

Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.

A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

AREA SUMMARY

LIVING AREA	2,378	S.F.
GARAGE AREA	595	S.F.
COVERED PORCH AREA	206	S.F.
ENTRY PORCH AREA	163	S.F.
TOTAL AREA	3,342	S.F.

OF 3 SHEETS

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

Garage fire separations shall comply with the following:

room used for sleeping purposes shall not be permitted.

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a

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DIMENSIONED

C**7** 19280-7 V OF OAKS

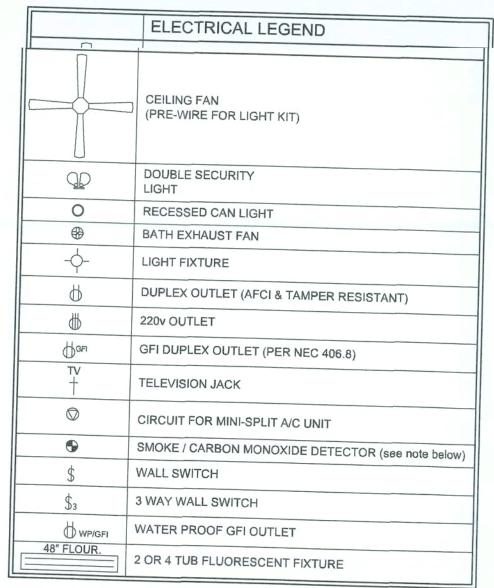
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ONTRACTING
NGS, FLORIDA

© WM DESIGN & A550CIATE 5. NC. 426 SW COMMERCE DR, STE 13 LAKE CITY, FL 32025 (386) 758-8406 will@willmyers.net

JOB NUMBER 20210324

SHEET NUMBER



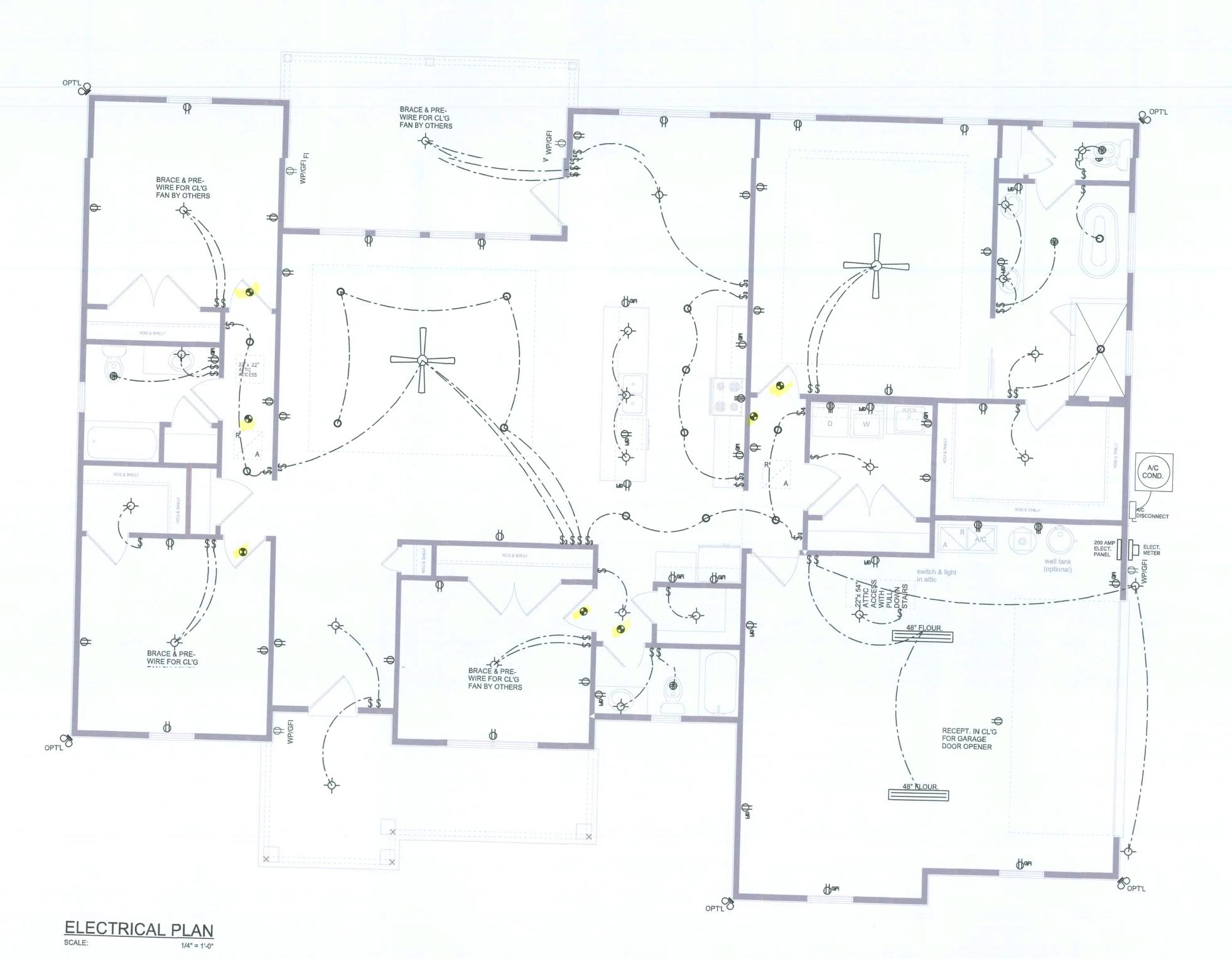
NOTE: ALL INTERIOR RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER NEC 406.11

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGH-EFFICIENCY LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR AND SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSE LECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



REVISIONS
April 15, 2021

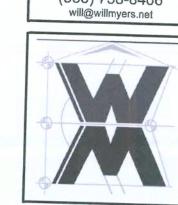
LOT 64, THE DAKS OF LAKE CPR.

LOT 64, THE DAKS OF LAKE CPR.

PROJECT ADDRESS: Lot 64, Oaks Cake City, Phase II. Parcel: 18-5S-17-09280-164

GIBRALTAR CONTRACTING, LLC.

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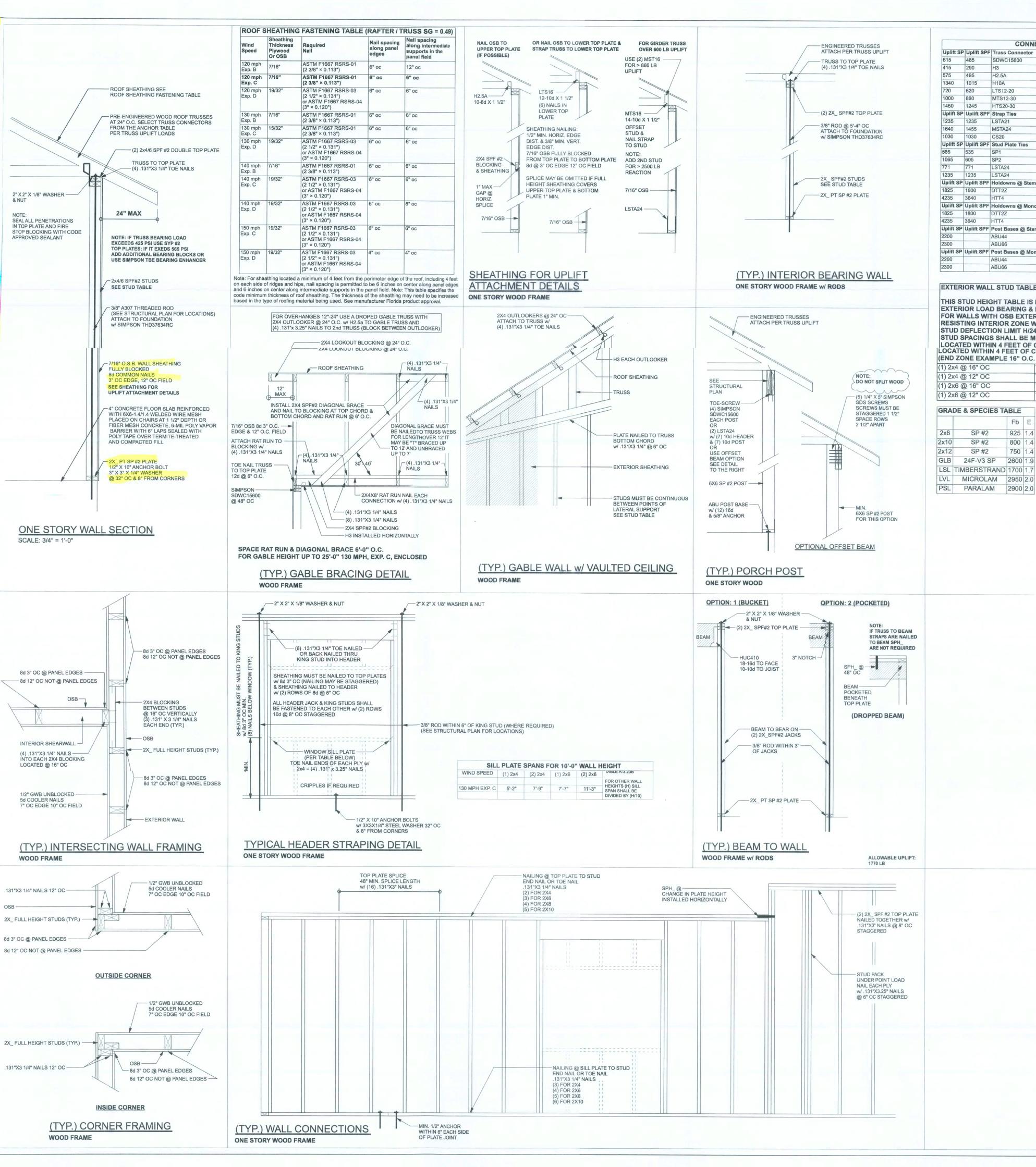


JOB NUMBER 20210324

SHEET NUMBER

A.3

OF 3 SHEETS



CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C. STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

TO 10'-1" STUD HEIGH TO 11'-2" STUD HEIGHT TO 15'-7" STUD HEIGHT TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE				
		Fb	Е	
2x8	SP #2	925	1.4	
2x10	SP #2	800	1.4	
2x12	SP #2	750	1.4	
GLB	24F-V3 SP	2600	1.9	
LSL	TIMBERSTRAND	1700	1.7	
LVL	MICROLAM	2950	2.0	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF TH TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING. UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

BEARING LOCATIONS. ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERA BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

> 2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

DOOR	WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' -	10'	24" OC	5" OC	5" OC
11'	- 15'	18" OC	4" OC	4" OC
16'	- 18'	16" OC	3" OC	3" OC

(TYP.) GARAGE DOOR BUCK INSTALLATION WOOD FRAME

DESIGN CRITERIA & LOAD	35:
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	120 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZON

COMPONENT & CLADING DESIGN PRESSURES 120 MPH (EXP C WIND AREA (FT2) INTERIOR END 4' FROM ALL OUTSIDE CORNER +25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd) +42.6(Vult) -46.2(Vult) +42.6(Vult) -57(Vult) GARAGE DOOR DESIGN PRESSURES 120 MPH (EXP C)

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd)

9x7 GARAGE DOOR

16x7 GARAGE DOOR

JOB NUMBER: 210541

S-1 OF 3 SHEETS

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Western Model The Oaks of Lak

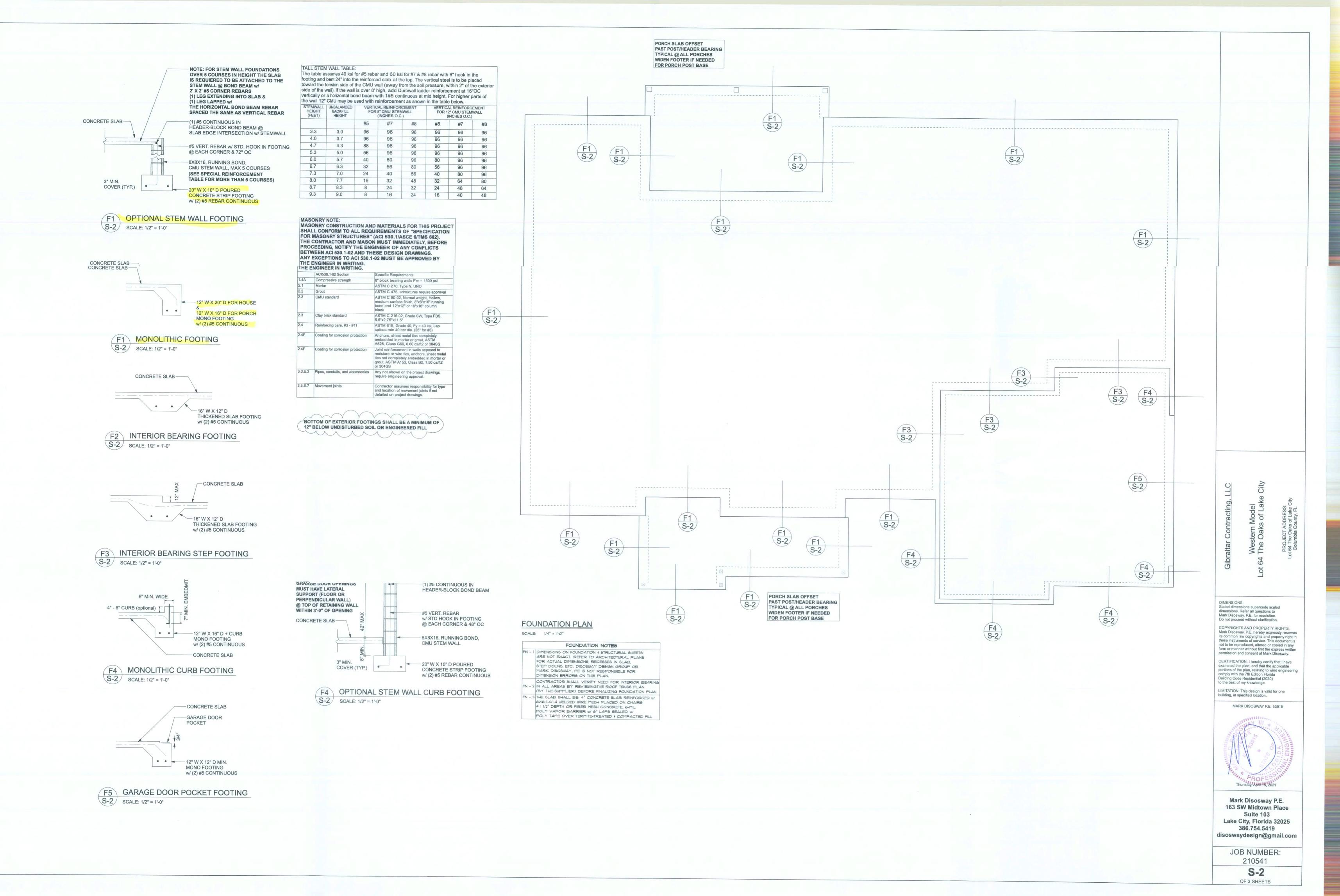
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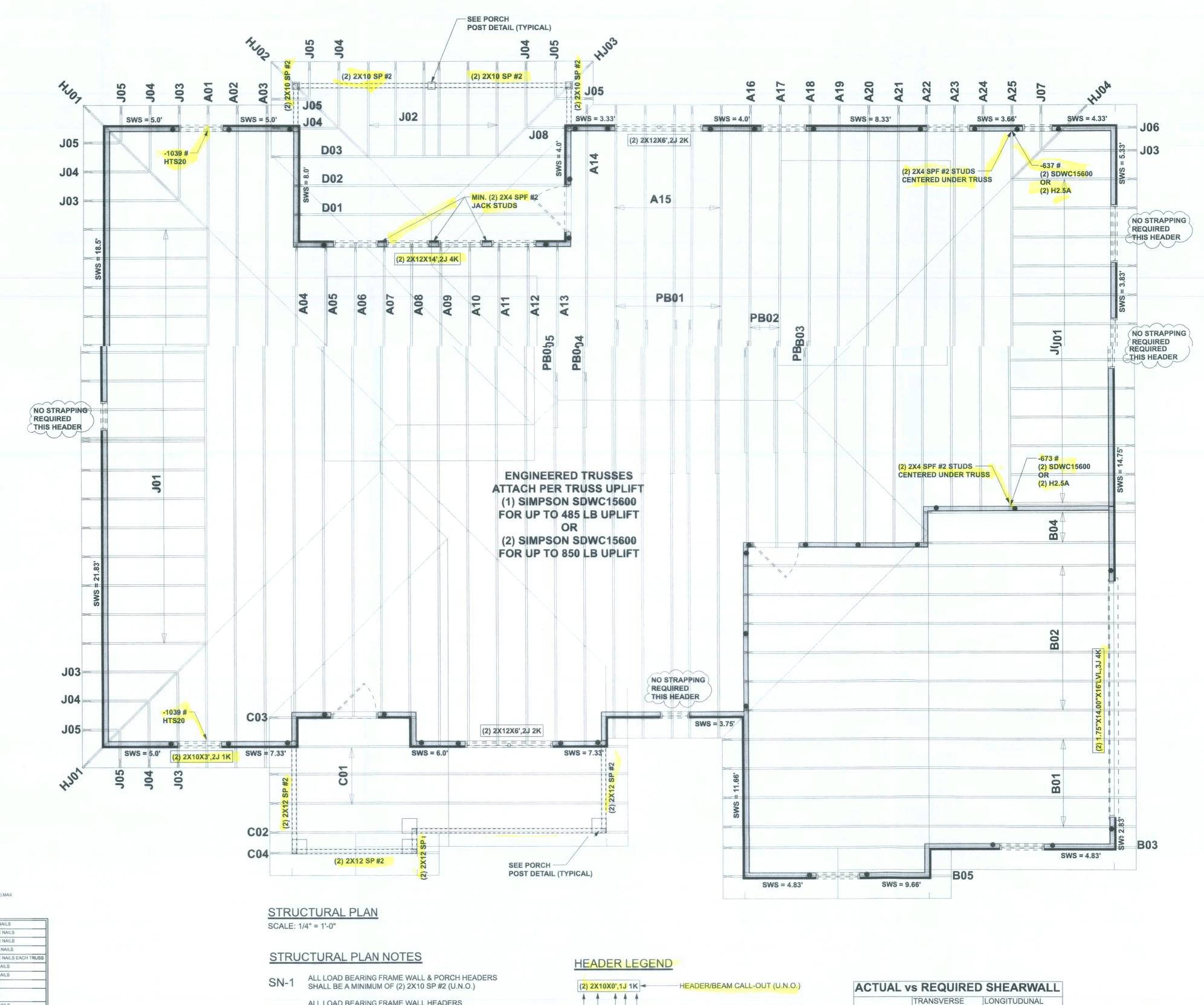
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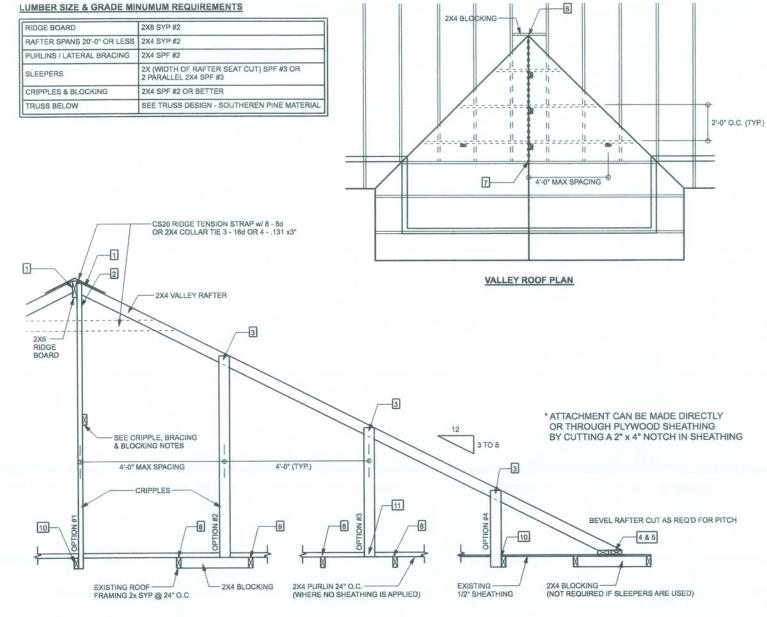
examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915









SECTION CUT PARALLEL TO VALLEY RAFTER

VALLEY ROOF PLAN MEMBER LEGEND

= = = TRUSS UNDER VALLEY FRAMING :====: VALLEY RAFTER OR RIDGE

CRIPPLE CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3 -16d OR 6131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6131 x 3" FACE NAILS

GENERAL NOTES

GENERAL NOTES

MAXIMUM RAFTER SPANS
6-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.

MAXIMUM ROOF AREA PER SUPPORT
16ft2 IN ZONES 2 & 3, 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN
= 16ft2 OR 2:0" X 9'-0" SPAN = 16ft2)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING IN MINIMUM
OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWNING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
- MAXIMUM WIND SPEED: 130 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM MEAN ROOF HEIGHT: 10 FEET
- MAXIMUM TOTAL LOADING: 40 psi
- MEETS FBC / ASCE 7-16 WIND REQUIREMENTS
- EXPOSURE CATEGORY "C", | = 1.0, Kzt = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES -2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 99% OF CRIPPLE LENGTH. CRIPPLES OVER 10" O'LONG REQUIRE TWO CLB'S OR BOTH FACES W "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD

- INSTALLED INTO RIDGE BOARD

- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL SETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED,

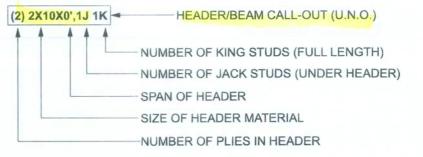
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL

FLOOR PLAN FOR ACTUAL DIMENSIONS PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



21775 LBF

16881 LBF

REQUIRED

19771 LBF

13252 LBF

THREADED ROD LEGEND

-INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD DIMENSIONS:

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO.

JOB #21-5392

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



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> JOB NUMBER: 210541

S-3 OF 3 SHEETS

ROOF OVER FRAMING & BRACING DETAIL