

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 16" OC | TO 18'-10' STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

SEE STRUCTURAL PLAN

SCALE: N.T.S.

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TO THE HEADER THE SPH4/6 @ 48" O.C. FOR LESS THAN 3750 Ib UPLIFT USE ARE NOT REQUIRED 3 X 3 X 1/8" WASHER - NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT (7) .131 x 3 1/4" GUN NAILS ----SPH4/6 @ 48" O.C. (U.N.O.)/---(7) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD CRIPPLES IF REQUIRED TOE .131 x 3 1/4" GUN NAILS INTO OE NAILED THRU SILL NTO JACK STUD U.N.O. (SEE SPICAL STRAPPING (U.N.O.) EE STRUCTURAL PLAN (2) 2X4 SPI SPF #2 SILL UP TO 7'-6" U.N.O. (1) 2X4 SPI SPF #2 SILL UP TO 7'-8" U.N.O. (FOR: 120 MP SPF #2 SILL UP TO 5'-1" U.N.O. MPH, 10'-0" WALL HEIGHT U.N.O.)

FOR LESS THAN 1500 Ib UPLIFT USE

2 X 2 X 1/8" WASHER

TYPICAL 1 STORY
SCALE: 1/2" = 1'-0"
RY HEADER STRAPING DETAIL

MASONRY NOTES:

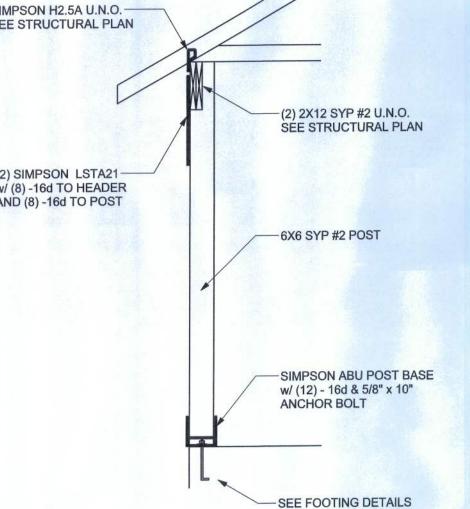
IF TRUSS TO WALL STRAPS ARE NAILED

		ACI530.1-02 Section		Cassific Requirements
	1.4A	Compressive strength	ion	Specific Requirements
	2.1	Mortar	ngth	8" block bearing walls F'm = 1500 psi
	2.2	Grout		ASTM C 270, Type N, UNO
	2.3	CMU standard		ASTM C 476, admixtures require approval
				ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
	2.3	Clay brick standard	d	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
	2.4 Reinfo	Reinforcing bars, #3 -		
			#3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
	2.4F Coating for corros	Coating for corrosion		
			ion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
	2.4F	Coating for corrosion		
			ion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
	3.3.E.2 Pipes, conduits, and			
			nd accessories	Any not shown on the project drawings require engineering approval.
	3.3.E.7	Movement joints		Contractor assumes responsibility for type

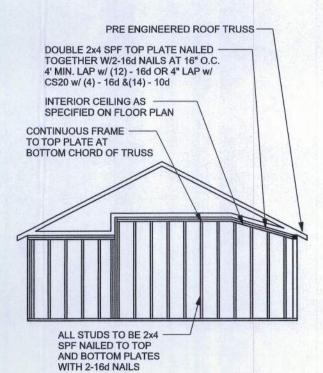
MASONRY CONSTRUCTION AND

CONFORM TO ALL REQUIREMENAND MATERIALS FOR THIS PROJECT SHALL STRUCTURES" (ACI 530.1/ASCE (MENTS OF "SPECIFICATION FOR MASONRY MUST IMMEDIATELY, BEFORE PFCE 6/TMS 602). THE CONTRACTOR AND MASON ANY CONFLICTS BETWEEN ACI & PROCEDING, NOTIFY THE ENGINEER OF ANY EXCEPTIONS TO ACI 530.1-(ACI 530.1-02 AND THESE DESIGN DRAWINGS.).1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section		
1.4A	Compressive strength	ion	Specific Requirements
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3.3.E.2	Pipes, conduits, and a		
194		nd accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints		Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL**

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	En. M
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT	7	22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4	31777		6-10d, 1 1/2"
< 1240	< 1065	SPH4	9		10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"	0.1965	1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		OIO AB
< 3335	< 3335	HPAHD22	16-16d	U Deva	
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		
< 2320	< 2320	ABU88	18 - 16d		1/2" AB
2020	2020	ADOOG	10 - 100		2-5/8" AB

GRADE & SPECIES TABLE

SYP #2

SYP #2 SYP #2

24F-V3 SP

LSL TIMBERSTRAND 1700

MICROLAM 2900

PARALAM 2900

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH 3/8" x 4" LAG 16d (2) ROWS OF STAGGER .131 x 3 1/4" GN

8' - 10' 24" O.C. 5" O.C. 5" O.C.

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

11' - 15' 18" O.C. 4" O.C.

16' - 18' 16" O.C. 3" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

2x6SYP #2 DOOR BUCK ----

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR

2x10

GLB

Fb (psi) E (10⁶ psi)

1.6

2.0

4" O.C.

1200

975

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS, TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURE AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND O SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB:
6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT F.BRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVEI MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMINT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGT! WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. D(NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BAS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY N ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CAL'S.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHOIS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUTYO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALLY N	D OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICHAR OT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK
CONFIRM SITE CONE BACKFILL HEIGHT, W	TIONS, FOUNDATION BEARING CAPACITY, GRADE AND ND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS REQUIREMENTS FOR	AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLAN O	DUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU MITS A CONTINUOUS LOAD PATH CONNECTION, CALL NEER IMMEDIATELY.
DESIGN, PLACEMENT	ANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, NNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE: REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIN	ND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1			
ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.			
BUII	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE			
BUII	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION			
1.)	BASIC WIND SPEED = 110 MPH			
2.)	WIND EXPOSURE = B			
3.)	WIND IMPORTANCE FACTOR = 1.0			
4.)	BUILDING CATEGORY = II			
5.)	ROOF ANGLE = 10-45 DEGREES			
6.)	MEAN ROOF HEIGHT = <30 FT			
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)			
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))			

Zone Effective Wind Area (ft2)

19.9 -21.8 18.1 -18.1

19.9 -25.5 18.1 -21.8

19.9 -25.5 18.1 -21.8

3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

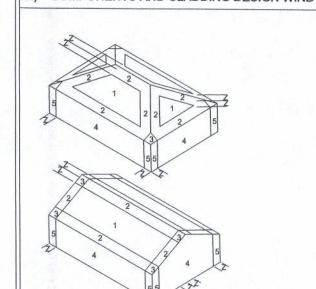
Doors & Windows 21.8 -29.1

8x7 Garage Door 19.5 -22.9

2 O'hg -40.6

Worst Case

(Zone 5, 10 ft2)



2	4 /2/ 5	16x7 Garage Door	18.5	-21.0			
	3 4 /2	4-					
	55 22		-	-			
	2/2		1	-			
DESIGN	LOADS						
FLOOR	R 40 PSF (ALL OTHER DWELLING ROOMS)						
	30 PSF (SLEEPING ROOMS)	1. 1. 1. 1. 1. 1.					
, m	30 PSF (ATTICS WITH STORAGE)						
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)						
ROOF	20 PSF (FLAT OR <4:12)						
	16 PSF (4:12 TO <12:12)	V vH II					
	12 PSF (12:12 AND GREATER)		3/ 10-				
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)						
SOIL BE	ARING CAPACITY 1000PSF						
NOT IN I	FLOOD ZONE (BUILDER TO VERIFY)						

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL

32056, 386-754-5419

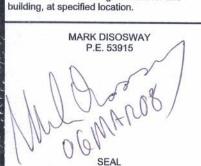
REVISIONS

SOFTPIAN

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not to be reproduced, altered or copied in any form or manner without first the express writt ermission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have amined this plan, and that the applicable rtions of the plan, relating to wind engine comply with section R301.2.1, florida buildin

ode residential 2004, to the best of my LIMITATION: This design is valid for one building, at specified location.



Sparks Construction Inc

Bryant Residence

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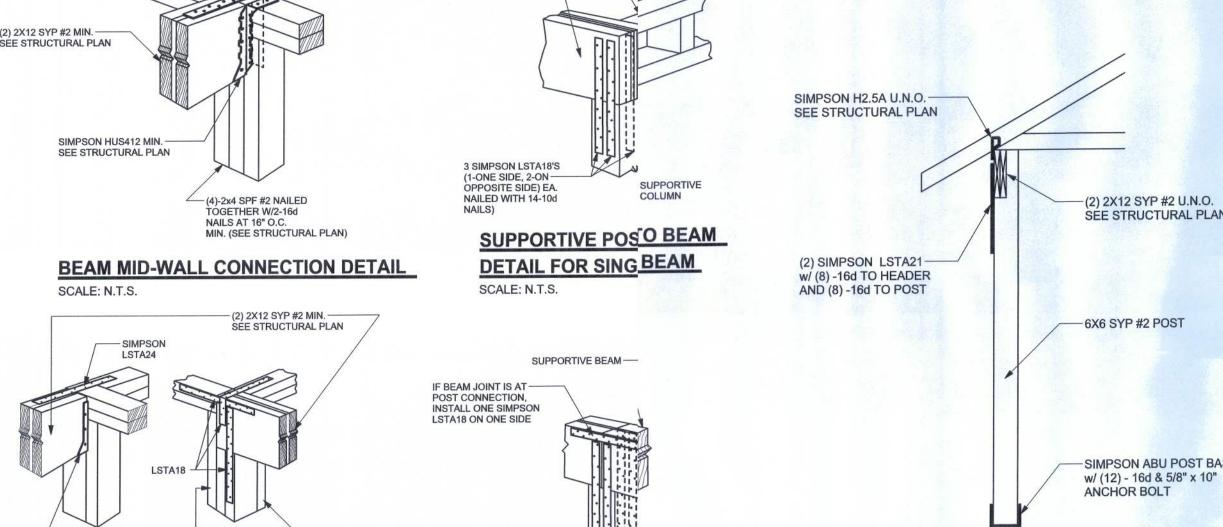
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FINALS DATE:

27 / Feb / 08 JOB NUMBER: 707261

> **S-1** OF 6 SHEETS

DRAWING NUMBER



SUPPORTIVE CENTER PO TO BEAM DETAIL

SUPPORTIVE

(2-ONE SIDE,2-ON

OTHER SIDE)

