

DATE 06/13/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024622

APPLICANT LINDA RODER PHONE 752-2281

ADDRESS 387 SW KEMP COURT LAKE CITY FL 32055

OWNER ISAAC CONSTRUCTION PHONE 719-7143

ADDRESS 318 NW ZACK DRIVE LAKE CITY FL 32055

CONTRACTOR ISAAC CONSTRUCTION PHONE 719-7143

LOCATION OF PROPERTY 90W, TR ON BROWN RD, TL ON ZACK DR, LOT ON LEFT JUST PAST
KELLY LAKE CT, CORNER LOT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 92900.00

HEATED FLOOR AREA 1858.00 TOTAL AREA 2553.00 HEIGHT 1 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 22

Minimum Set Back Requirements: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 28-3S-16-02372-443 SUBDIVISION EMERALD LAKES

LOT 143 BLOCK PHASE UNIT TOTAL ACRES

000001112

Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor

CULVERT 06-0111-N BK JH

Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, REQUIRES SILT FENCE ON SOUTH PROPERTY LINE

Check # or Cash 5607

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 465.00 CERTIFICATION FEE \$ 12.77 SURCHARGE FEE \$ 12.77

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 590.54

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

LOT 143
emerald lakes

left message 6/12/06

Columbia County Building Permit Application

Revised 9-23

For Office Use Only: Application # 0605-90 Date Received 5/25 By TW Permit # 1112/2462
Application Approved by - Zoning Official BKA Date 07.06.06 Plans Examiner OK JTH Date 8-12-06
Flood Zone X B1 Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res Low Density
Comments note Requires Silt Fence on South Property Line

Applicants Name Linka or Melanie Roder Phone 752-2281
Address 387 S.W. Kemp Ct. Lake City FL 32024
Owners Name Isaac Construction Phone 719-7143
911 Address 318 NW Zach Dr Lake City FL 32055
Contractors Name Isaac Bratkovich Phone 719-7143
Address PMB 338 2109 U.S. Hwy 90 Lake City FL 32055
Fee Simple Owner Name & Address NA
Bonding Co. Name & Address NA
Architect/Engineer Name & Address Daniel Shaheen / Mark Disasway
Mortgage Lenders Name & Address Mercantile
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
Property ID Number 28-35-16-02372-443 Estimated Cost of Construction 120K
Subdivision Name Emerald Lakes Lot 143 Block Unit Phase 4
Driving Directions 90 W. Turn Ron Brown Rd. Turn L on Zach Dr. Lot on left just past Kelly Lake Ct. (corner lot, 1st lot)

Type of Construction SFD Number of Existing Dwellings on Property 0
Total Acreage .5 Lot Size Do you need a Culvert Permit or Culvert Waiver or Have an Existing Driv
Actual Distance of Structure from Property Lines - Front 66' Side 30' Side 30' Rear 53'
Total Building Height 22'-2" Number of Stories 1 Heated Floor Area 1858 Roof Pitch 8-12
Porch 148 Garage 547 TOTAL 2553

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) 590.54 Contractor Signature Isaac Bratkovich
STATE OF FLORIDA COUNTY OF COLUMBIA Barbara C. Webster Contractors License Number CBC 05932
Commission # DD329279 Competency Card Number
Expires July 2, 2008 NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed before me
this 22 day of May 2006
Personally known X or Produced Identification Notary Signature Barbara C Webster

This Instrument Prepared by & return to:
Name: **Chris Travis, an employee of**
TITLE OFFICES, LLC
Address: **1089 SW MAIN BLVD.**
LAKE CITY, FLORIDA 32025
File No. 05Y-11028CT

Inst: 2005030401 Date: 12/07/2005 Time: 15:15
Doc Stamp-Deed : 301.00
mk DC, P. Dewitt Cason, Columbia County B: 1067 P: 1426

Parcel I.D. #: 02372-443

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

THIS WARRANTY DEED Made the 1st day of December, A.D. 2005, by

TODD E. WIDERGREN and SHEILA WIDERGREN, HIS WIFE, hereinafter called the grantors, to
ISAAC CONTRUCTION, INC, A FLORIDA CORPORATION, having its principal place of business at
144 SW WATERFORD COURT, SUITE 101, LAKE CITY, FL. 32025, hereinafter called the grantee:

(Wherever used herein the terms "grantors" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantors, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, do hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in **Columbia County, State of FLORIDA**, viz:

Lot 143, EMERALD LAKES, Phase 4, according to the map or plat thereof as recorded in Plat Book 6, Page 151-152, of the Public Records of Columbia County, FLORIDA.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.

And the grantors hereby covenant with said grantee that they are lawfully seized of said land in fee simple; that they have good right and lawful authority to sell and convey said land, and hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

In Witness Whereof, the said grantors have signed and sealed these presents, the day and year first above written.

Signed, sealed and delivered in the presence of:

Martha Bryan
Witness Signature

Printed Name

Regina Simpkins
Witness Signature

Printed Name

Todd E. Widergren
L.S.
TODD E. WIDERGREN

Address:
384 NW PALM DRIVE,
LAKE CITY, FL. 32055

Sheila Widergren
L.S.
SHEILA WIDERGREN

Address:
384 NW PALM DRIVE,
LAKE CITY, FL. 32055

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 1st day of December, 2005, by **TODD E. WIDERGREN and SHEILA WIDERGREN**, who are known to me or who have produced Drivers License as identification.

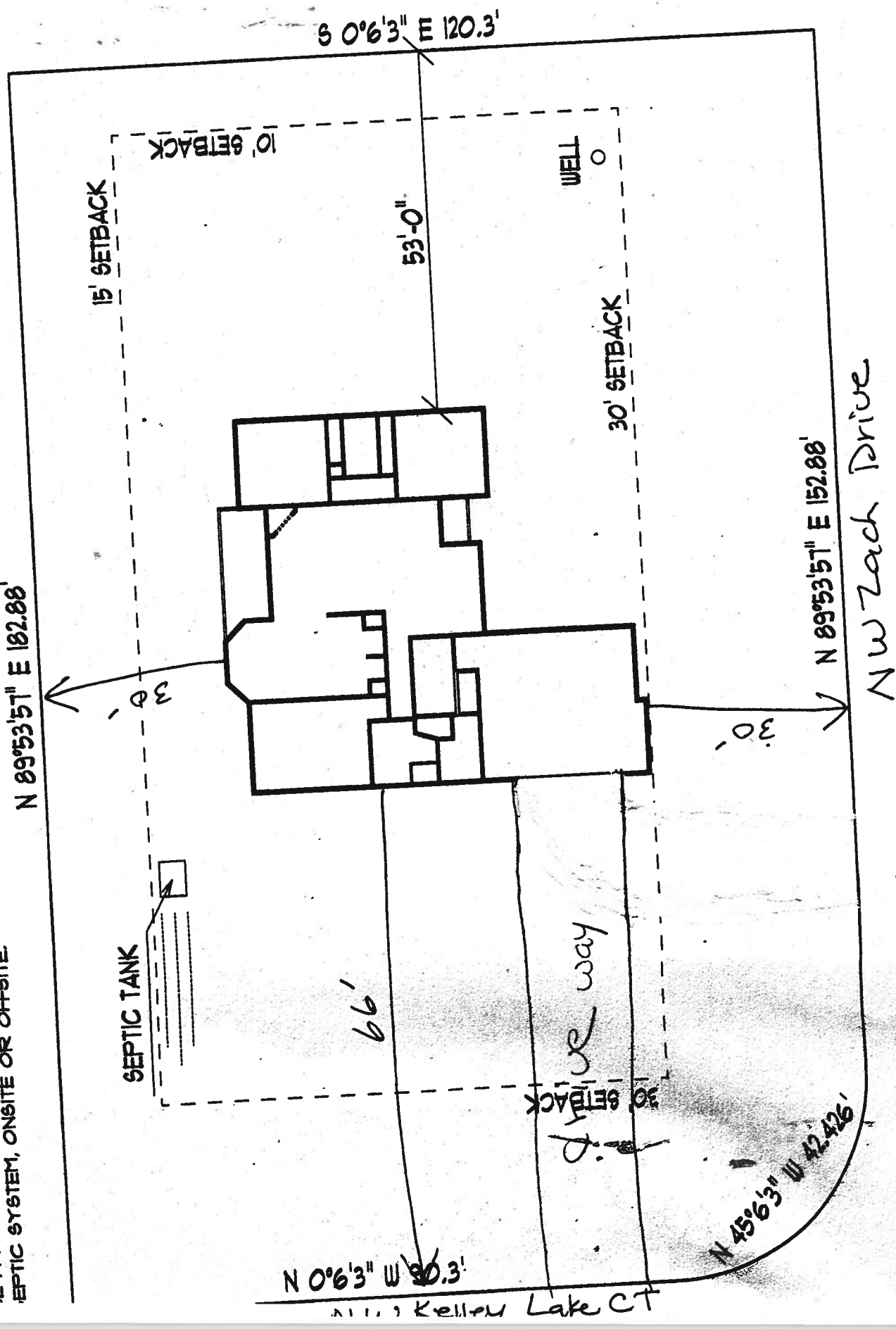
Martha Bryan
Notary Public
My commission expires



Martha Bryan
Commission # DD232534
Expires August 10, 2007
Bonded Title Plan - Insurance, Inc. 800-385-7019

02372-443

POTABLE WATER SYSTEM, ONSITE OR OFFSITE
LOCATION OF POTABLE WATER SYSTEM MUST BE A MIN. 15' DISTANCE FROM ANY EXISTING SEPTIC SYSTEM, ONSITE OR OFFSITE.



SITE PLAN

SCALE: 1" = 20'
MAY 10 11 AM '08

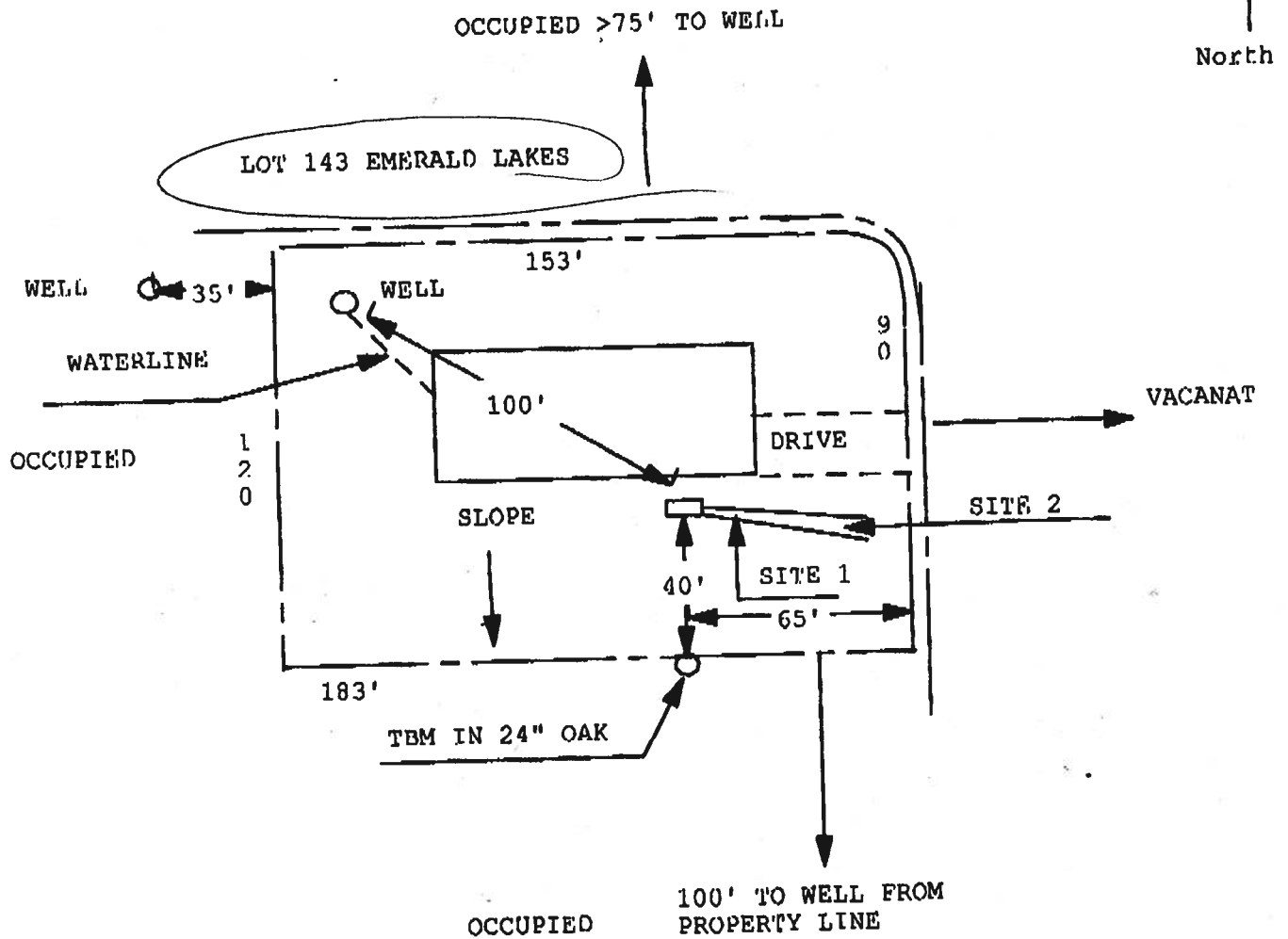


Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 010-0111N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

ISAAC CONST. CR# 05-3300



1 inch = 50 feet

Site Plan Submitted By Paul Lloyd

Date 1/6/06

Plan Approved ☒ Not Approved ☐

Date 2-13-06

By M. O. Z.

Columbia CPHU

Notes:



Phone (386) 755-3611

Fax (386) 752-5381

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code (FBC) 104.2.6)

Aspen Pest Control, Inc.**(386) 755-3611****State License # - JB109476****State Certification # - JF104376****Lot 143 Harwell Place (Emerald Lakes), Lake City, FL**

Address of Treatment or Lot/Block of Treatment

Bora-Care Wood Treatment – 23% Disodium Octaborate Tetrahydrate

Method of Termite Prevention Treatment – Soil Barrier, Wood Treatment, Bait System, Other

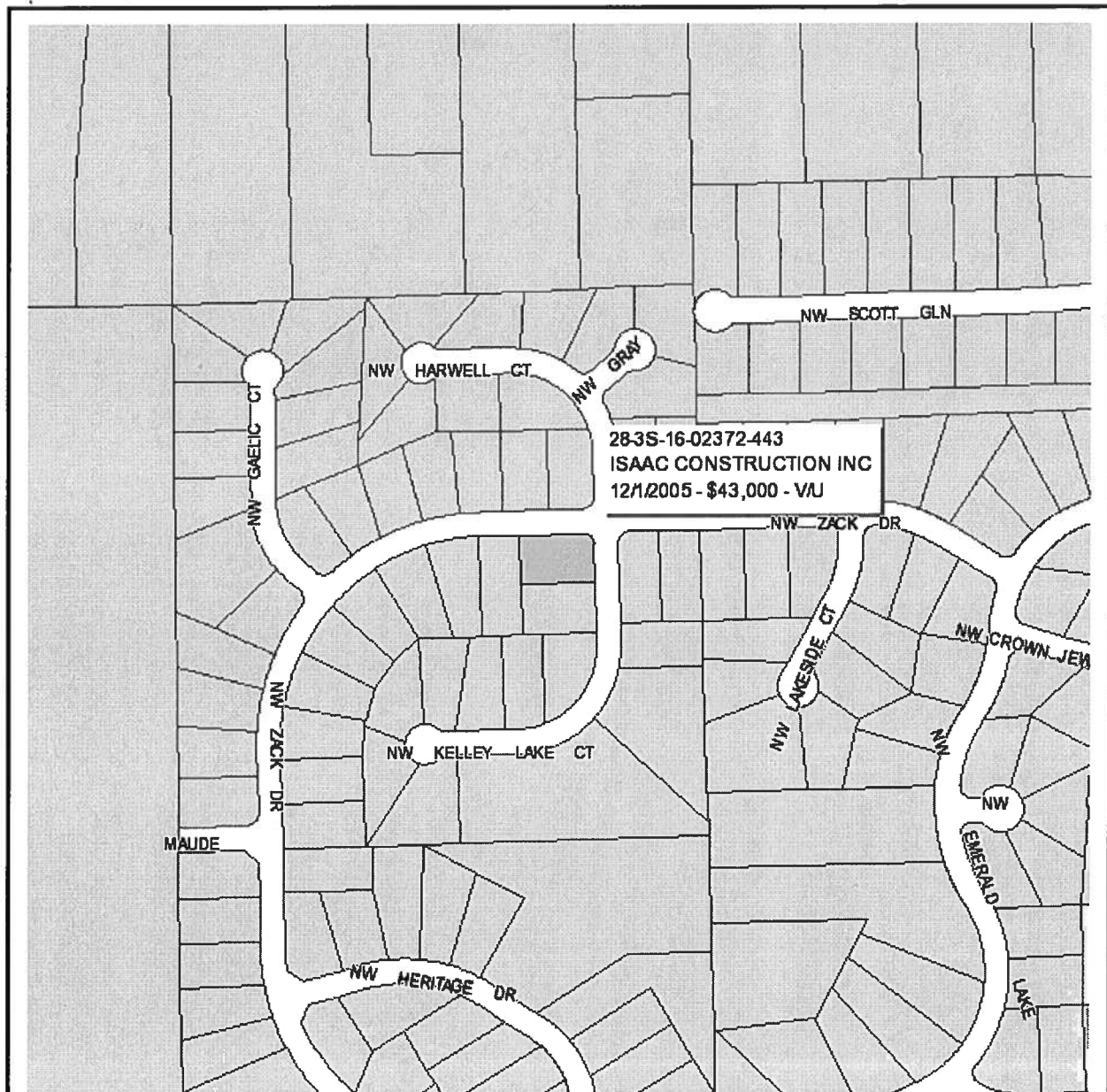
Application onto Structural Wood

Description of Treatment

The above named structure will receive a complete treatment for the prevention of subterranean termites at the dried-in stage of construction. Treatment is done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services and according to EPA registered label directions as stated in Florida Building Code Section 1861.1.8.

Celia Orjales
Authorized Signature

1-26-06
Date



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 28-3S-16-02372-443 - VACANT (000000)

Name: ISAAC CONSTRUCTION INC	LandVal	\$25,500.00
Site:	BldgVal	\$0.00
144 SW WATERFORD COURT	ApprVal	\$25,500.00
Mail: SUITE 101	JustVal	\$25,500.00
LAKE CITY, FL 32025	Assd	\$25,500.00
Sales 12/1/2005 \$43,000.00V / U	Exmpt	\$0.00
Info 10/13/2005 \$30,000.00V / Q	Taxable	\$25,500.00
4/15/2005 \$22,000.00V / U		

0 170 340 510 ft



This information, GIS Map Updated: 5/5/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: **THE MORICE, LOT 143**
Address: **Lot: 143, Sub: Emerald Lakes, Plat:**
City, State: **LAKE CITY, FL**
Owner: **ISAAC CONSTRUCTION**
Climate Zone: **North**

Builder: **ISAAC CONSTRUCTION**
Permitting Office: **Columbia**
Permit Number: **24622**
Jurisdiction Number: **221000**

1. New construction or existing	New	—
2. Single family or multi-family	Single family	—
3. Number of units, if multi-family	1	—
4. Number of Bedrooms	3	—
5. Is this a worst case?	Yes	—
6. Conditioned floor area (ft²)	1858 ft²	—
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		—
a. U-factor:	Description Area	
(or Single or Double DEFAULT)	7a. (Dble Default)	260.3 ft²
b. SHGC:		
(or Clear or Tint DEFAULT)	7b. (Clear)	260.3 ft²
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 209.0(p) ft	—
b. N/A		—
c. N/A		—
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 198.0 ft²	—
b. Frame, Wood, Exterior	R=13.0, 1632.0 ft²	—
c. N/A		—
d. N/A		—
e. N/A		—
10. Ceiling types		
a. Under Attic	R=30.0, 1858.0 ft²	—
b. N/A		—
c. N/A		—
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 150.0 ft	—
b. N/A		—
12. Cooling systems		
a. Central Unit	Cap: 36.0 kBtu/hr	—
	SEER: 13.00	—
b. N/A		—
c. N/A		—
13. Heating systems		
a. Electric Heat Pump	Cap: 36.0 kBtu/hr	—
	HSFP: 7.20	—
b. N/A		—
c. N/A		—
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons	—
	EF: 0.92	—
b. N/A		—
c. Conservation credits		—
(HR-Heat recovery, Solar		—
DHP-Dedicated heat pump)		—
15. HVAC credits		CF, —
(CF-Ceiling fan, CV-Cross ventilation,		—
HF-Whole house fan,		—
PT-Programmable Thermostat,		—
MZ-C-Multizone cooling,		—
MZ-H-Multizone heating)		—

Glass/Floor Area: 0.17

Total as-built points: 26689
Total base points: 28816

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [Signature]

DATE: 5/4/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [Signature]

DATE: 5-19-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joist members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 75%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING				Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multipler X	Credit = Total Multiplier
Number of Bedrooms	X	Multiplier	= Total							
3		2635.00	7905.0	40.0	0.92	3		1.00	2635.00	1.00 7905.0
				As-Built Total:						7905.0

CODE COMPLIANCE STATUS							
BASE				AS-BUILT			
Cooling Points	+	Heating Points	+ Hot Water Points = Total Points	Cooling Points	+	Heating Points	+ Hot Water Points = Total Points
10464		10447	7905 28816	7470		11315	7905 26689

PASS



WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

BASE			AS-BUILT			
INFILTRATION Area X BWPM = Points			Area X	WPM	=	Points
1858.0	-0.59	-1098.2	1858.0	-0.59		-1098.2
Winter Base Points:		16651.1	Winter As-Built Points:			20556.1
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier X Credit Multiplier = Heating Points
16651.1	0.6274	10448.9	(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0			
			20556.1	1.000	(1.069 x 1.189 x 0.93)	0.474 1.000 11314.6
			20556.1	1.00	1.162	0.474 1.000 11314.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ormt Len Hgt		Area X WPM X WOF = Points				
.18	1858.0	12.74	4280.8	Double, Clear	E	1.5	9.0	53.7	18.79	1.02	1024.3
				Double, Clear	E	5.0	10.0	13.3	18.79	1.12	280.6
				Double, Clear	E	5.0	4.0	9.0	18.79	1.35	227.9
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8
				Double, Clear	N	1.5	5.0	16.0	24.58	1.00	394.7
				Double, Clear	S	1.5	1.5	4.0	13.30	2.73	145.3
				Double, Clear	W	1.5	6.0	60.0	20.73	1.02	1272.9
				Double, Clear	W	8.0	10.0	24.0	20.73	1.14	568.5
				Double, Clear	N	1.5	6.0	20.0	24.58	1.00	492.7
				Double, Clear	NW	1.5	7.5	21.0	24.30	1.00	510.8
				Double, Clear	SW	3.0	7.5	21.0	16.74	1.16	408.6
				Double, Clear	W	1.5	7.5	35.0	20.73	1.01	735.4
				As-Built Total:				307.0			6645.6
WALL TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM = Points	
Adjacent		198.0	3.60	712.8	Frame, Wood, Adjacent		13.0	198.0	3.30	653.4	
Exterior		1632.0	3.70	6038.4	Frame, Wood, Exterior		13.0	1632.0	3.40	5548.8	
Base Total:		1830.0	6751.2		As-Built Total:		1830.0		6202.2		
DOOR TYPES				Area X BWPM = Points		Type		Area X WPM = Points			
Adjacent		20.0	11.50	230.0	Exterior Wood		20.0	12.30	246.0		
Exterior		68.0	12.30	836.4	Exterior Wood		48.0	12.30	590.4		
					Adjacent Wood		20.0	11.50	230.0		
Base Total:		88.0	1066.4		As-Built Total:		88.0		1066.4		
CEILING TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM X WCM = Points	
Under Attic		1858.0	2.05	3808.9	Under Attic		30.0	1858.0	2.05 X 1.00	3808.9	
Base Total:		1858.0	3808.9		As-Built Total:		1858.0		3808.9		
FLOOR TYPES				Area X BWPM = Points		Type		R-Value		Area X WPM = Points	
Slab		209.0(p)	8.9	1860.1	Slab-On-Grade Edge Insulation		0.0	209.0(p)	18.80	3929.2	
Raised		0.0	0.00	0.0							
Base Total:		1860.1		As-Built Total:		209.0		3929.2			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

BASE				AS-BUILT			
INFILTRATION Area X BSPM = Points				Area X SPM = Points			
1858.0 10.21 18870.2				1858.0 10.21 18870.2			
Summer Base Points: 24529.5				Summer As-Built Points: 26324.9			
Total Summer X System = Cooling Points Multiplier Points				Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)			
24529.5 0.4266 10464.3				<small>(sys 1: Central Unit 36000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)</small> 26325 1.00 (1.09 x 1.147 x 0.91) 0.263 0.950 7469.9 26324.9 1.00 1.138 0.263 0.950 7469.9			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Omt Len Hgt		Area X SPM X SOF = Points				
.18	1858.0	20.94	6702.2	Double, Clear	E	1.5	9.0	53.7	42.06	0.97	2189.1
				Double, Clear	E	5.0	10.0	13.3	42.06	0.73	406.9
				Double, Clear	E	5.0	4.0	9.0	42.06	0.46	175.3
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8
				Double, Clear	N	1.5	5.0	16.0	19.20	0.92	281.3
				Double, Clear	S	1.5	1.5	4.0	35.87	0.52	74.7
				Double, Clear	W	1.5	6.0	60.0	38.52	0.91	2111.2
				Double, Clear	W	8.0	10.0	24.0	38.52	0.58	539.7
				Double, Clear	N	1.5	6.0	20.0	19.20	0.94	360.4
				Double, Clear	NW	1.5	7.5	21.0	25.87	0.96	521.3
				Double, Clear	SW	3.0	7.5	21.0	40.16	0.75	630.2
				Double, Clear	W	1.5	7.5	35.0	38.52	0.95	1279.8
				As-Built Total:		307.0			9721.8		
WALL TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM = Points	
Adjacent		198.0	0.70	138.6	Frame, Wood, Adjacent		13.0	198.0	0.60	118.8	
Exterior		1632.0	1.70	2774.4	Frame, Wood, Exterior		13.0	1632.0	1.50	2448.0	
Base Total:		1830.0	2913.0		As-Built Total:		1830.0		2566.8		
DOOR TYPES				Area X BSPM = Points		Type		Area X SPM = Points			
Adjacent		20.0	2.40	48.0	Exterior Wood		20.0	6.10	122.0		
Exterior		68.0	6.10	414.8	Exterior Wood		48.0	6.10	292.8		
					Adjacent Wood		20.0	2.40	48.0		
Base Total:		88.0	462.8		As-Built Total:		88.0		462.8		
CEILING TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM X SCM = Points	
Under Attic		1858.0	1.73	3214.3	Under Attic		30.0	1858.0	1.73 X 1.00	3214.3	
Base Total:		1858.0	3214.3		As-Built Total:		1858.0		3214.3		
FLOOR TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM = Points	
Slab		209.0(p)	-37.0	-7733.0	Slab-On-Grade Edge Insulation		0.0	209.0(p)	-41.20	-8610.8	
Raised		0.0	0.00	0.0							
Base Total:			-7733.0		As-Built Total:		209.0		-8610.8		

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.3

The higher the score, the more efficient the home.

ISAAC CONSTRUCTION, Lot: 143, Sub: Emerald Lakes, Plat: , LAKE CITY, FL,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	1858 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 260.3 ft ²		HSPF: 7.20
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 260.3 ft ²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 209.0(p) ft	a. Electric Resistance	Cap: 40.0 gallons
b. N/A			EF: 0.92
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Adjacent	R=13.0, 198.0 ft ²	(HR-Heat recovery, Solar	
b. Frame, Wood, Exterior	R=13.0, 1632.0 ft ²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	CF,
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 1858.0 ft ²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 150.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: _____

City/FL Zip: _____



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.0)

FROM :

FAX NO. : 386-755-7022

Sep. 17 2002 01:52PM P1

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1334
FAX (904) 755-7022
JACKSONVILLE, FLORIDA 32055
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

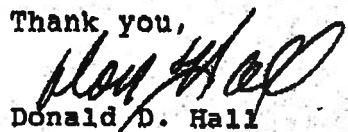
June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,


Donald D. Hall
DDH/jk

THIS INSTRUMENT WAS PREPARED BY:
TERRY MCDAVID 06-440
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Permit
600024622
758-2160
Attn: Janice

PERMIT NO. _____

TAX FOLIO NO.: _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:

Lot 143, EMERALD LAKES, Phase 4, a subdivision according to the plat thereof as recorded in Plat Book 6, Pages 151-152 of the public records of Columbia County, Florida.

2. General description of improvement: Construction of Dwelling

3. Owner information:

a. Name and address: ISAAC HOLDINGS, INC. f/k/a ISAAC CONSTRUCTION, INC., 2109 W US Highway 90, Suite 170 PMB 338, Lake City, FL 32055

b. Interest in property: Fee Simple

c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: ISAAC CONSTRUCTION, LLC

2109 W US Highway 90, Suite 170 PMB 338, Lake City, FL 32055; Phone No.: (386) 719-7143

5. Surety n/a

a. Name and address:
b. Amount of bond:

6. Lender: Mercantile Bank

187 SE Baya Drive, Lake City, FL 32025

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: NONE

8. In addition to himself, Owner designates Clarence B. Cannon, III, at Mercantile Bank, 187 SE Baya Drive, Lake City, FL 32025 to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

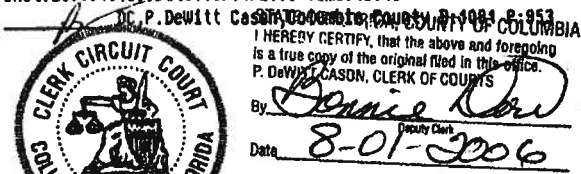
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified). July 27, 2007.

ISAAC HOLDINGS, INC.

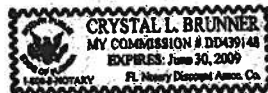
Isaac Bratkovich
By: ISAAC BRATKOVICH, President

The foregoing instrument was acknowledged before me this 27th day of July, 2006, by ISAAC BRATKOVICH, President of ISAAC HOLDINGS, INC. f/k/a ISAAC CONSTRUCTION, INC. who are personally known to me and who did not take an oath.

Inst:2006018125 Date:08/01/2006 Time:13:19



Crystal L. Brunner
Notary Public
My commission expires: _____



By: *Janice*
Date: 8-01-2006

**Columbia County Building Department
Culvert Permit**

**Culvert Permit No.
000001112**

DATE 06/13/2006 PARCEL ID # 28-3S-16-02372-443
APPLICANT LINDA RODER PHONE 752-2281
ADDRESS 387 SW KEMP COURT LAKE CITY FL 32055
OWNER ISAAC CONSTRUCTION PHONE 719-7143
ADDRESS 318 NW ZACK DRIVE LAKE CITY FL 32055
CONTRACTOR ISAAC CONSTRUCTION PHONE 719-7143
LOCATION OF PROPERTY 90W, TR ON BROWN RD, TL ON ZACK DR, LOT ON LEFT, JUST PAST
KELLY LAKE COURT, CORNER LOT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD LAKES 143

SIGNATURE

INSTALLATION REQUIREMENTS

☒ X

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

☐

Culvert installation shall conform to the approved site plan standards.

☐

Department of Transportation Permit installation approved standards.

☐

Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



#24622

Mark Disosway, P.E.

POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4817

11 Aug 2006

Building and Zoning, Columbia County, Florida

Re: Site Evaluation, ISAAC CONSTRUCTION, SPEC HOUSE, 144 SW WATERFORD COURT, SUITE 101, LAKE CITY, FL 32025, Columbia County, FL

Dear Building Inspector:

The elevation of the top of the stem wall foundation as prepped for slab by builder is clearly less than one foot above the elevation of the county road, SW WATERFORD COURT at a point immediately in front of the house.

Based on topo maps, FEMA Flood Insurance Rate Map, and visual inspection the proposed finished floor elevation, the top of the stem wall foundation as indicated by string lines which vary from 6" above natural grade to 6' above grade, is at an adequate elevation to avoid flooding.

Flood Zone of Home Site: Zone X; Based on the FEMA rate map, attached.

Home Site Natural Grade, Elevation: about 130 - 140 ft; Based on topo map, attached.

Zone A flood zone 600' to south of home site, Elevation: about 125 ft; Based on topo map, attached..

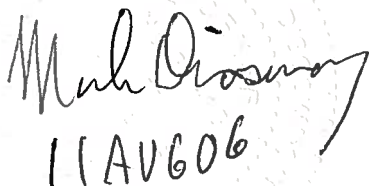
Proposed Finished Floor Elevation: 6" - 72" above natural grade in the perimeter of the house.

Observations: This house is higher (about 15 ft) than the county road behind it and nearby isolated flood Zone A established by FEMA based on estimation of the flood zone elevations from the topo map. There is clearly continuous slope drainage to this isolated flood zone and on to Lake Lona which is about 25' lower. The finished floor is about 8 - 12 feet higher than two neighboring houses which were recently built at over one foot above the road.

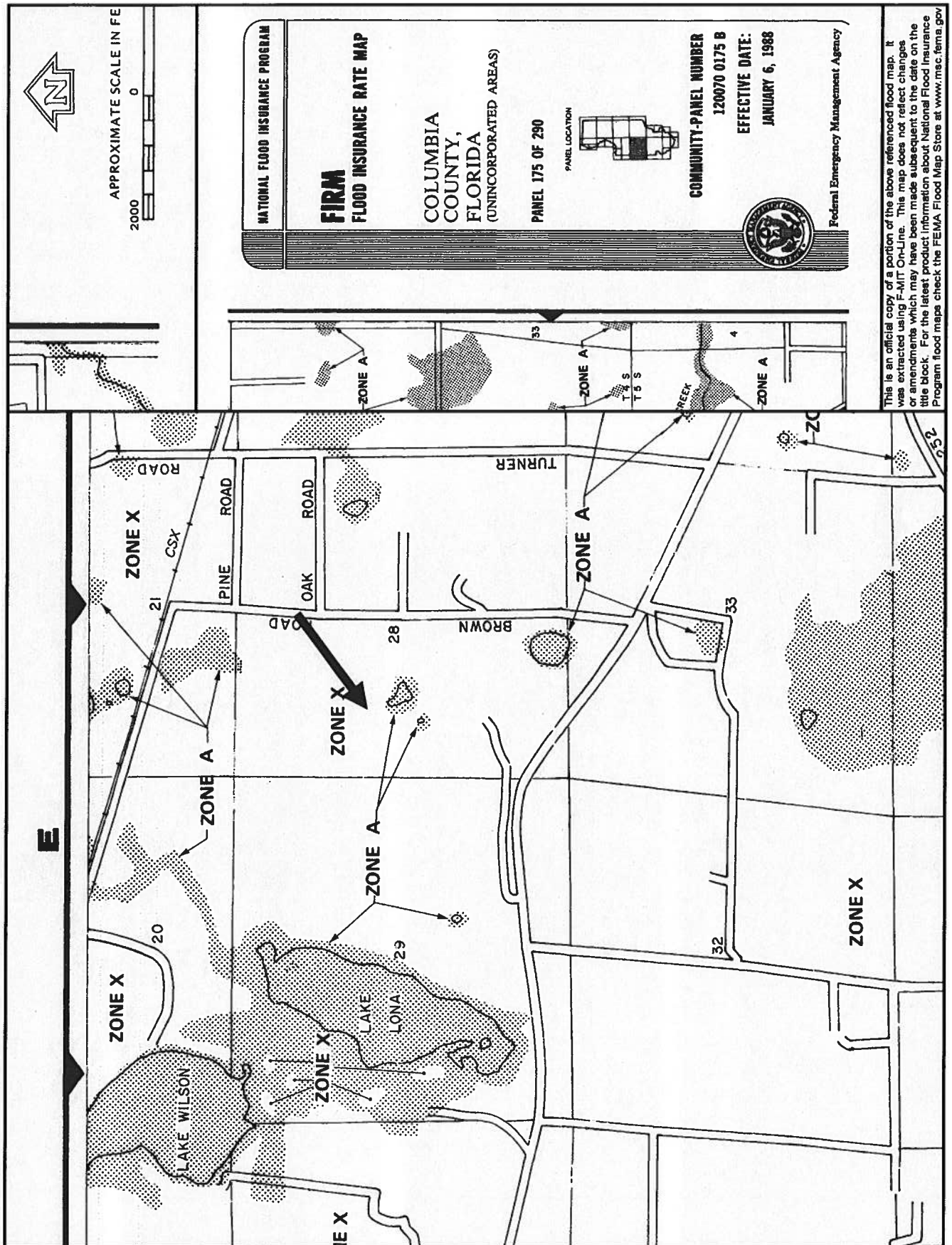
The finished floor elevation must be minimum 6" above finished grade per FBC2004. The finished grade should slope down from that elevation for another 6" within 12 feet away from the house in all directions so that all runoff drains away from the house. The owner must maintain the swales, slopes, and ditch to provide free drainage to the ditch and prevent any possibility of storm water backing up into the house.

The owner should be aware that if free drainage is not maintained thru fields and across roads and thru culverts to the sink holes, or if future development in the area causes increased storm water run off, or if rainfall occurs with greater flooding effect than the design storm, the level of the nearby Zone A could rise higher than anticipated and his house would be more susceptible to flooding.

Sincerely,


11 AUG 06

Mark Disosway, PE



R403.1 General.

All exterior walls shall be supported on continuous solid or fully grouted masonry or concrete footings, wood foundations, or other approved structural systems which shall be of sufficient design to accommodate all loads according to Section R301 and to transmit the resulting loads to the soil within the limitations as determined from the character of the soil. Footings shall be supported on undisturbed natural soils or engineered fill.

R403.1.1 Minimum size.

Minimum sizes for concrete and masonry footings shall be as set forth in Table R403.1 and Figure R403.1(1). The footing width, W , shall be based on the load-bearing value of the soil in accordance with Table R401.4.1. Spread footings shall be at least 6 inches (152 mm) in thickness. Footing projections, P , shall be at least 2 inches (51 mm) and shall not exceed the thickness of the footing. The size of footings supporting piers and columns shall be based on the tributary load and allowable soil pressure in accordance with Table R401.4.1. Footings for wood foundations shall be in accordance with the details set forth in Section R403.2, and Figures R403.1(2) and R403.1(3).

R403.1.4 Minimum depth.

All exterior footings shall be placed at least 12 inches (305 mm) below the undisturbed ground surface.

R403.1.5 Slope.

The top surface of footings shall be level. The bottom surface of footings shall not have a slope exceeding one unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footings or where the slope of the bottom surface of the footings will exceed one unit vertical in ten units horizontal (10-percent slope).

R403.1.6 Foundation anchorage.

When braced wall panels are supported directly on continuous foundations, the wall wood sill plate or cold-formed steel bottom track shall be anchored to the foundation in accordance with this section. The wood sole plate at exterior walls on monolithic slabs and wood sill plate shall be anchored to the foundation with anchor bolts spaced a maximum of 6 feet (1829 mm) on center. There shall be a minimum of two bolts per plate section with one bolt located not more than 12 inches (305 mm) or less than seven bolt diameters from each end of the plate section. Bolts shall be at least 1/4 inch (12.7 mm) in diameter and shall extend a minimum of 7 inches (178 mm) into masonry or concrete. Interior bearing wall sole plates on monolithic slab foundations shall be positively anchored with approved fasteners. A nut and washer shall be tightened on each bolt to the plate. Sills and sole plates shall be protected against decay and termites where required by Sections R319 and R320. Cold-formed steel framing systems shall be fastened to the wood sill plates or anchored directly to the foundation as required in Section R505.3.1 or R603.1.1. Exception: Foundation anchor straps, spaced as required to provide equivalent anchorage to 1/2-inch-diameter (12.7 mm) anchor bolts.

R403.1.6.1 Reserved.

R403.1.7 Footings on or adjacent to slopes.

The placement of buildings and structures on or adjacent to slopes steeper than 1 unit vertical in 3 units horizontal (33.3-percent slope) shall conform to Sections R403.1.7.1 through R403.1.7.4.

R403.1.7.1 Building clearances from ascending slopes.

In general, buildings below slopes shall be set a sufficient distance from the slope to provide protection from slope drainage, erosion and shallow failures. Except as provided in Section R403.1.7.4 and Figure R403.1.7.1, the following criteria will be assumed to provide this protection. Where the existing slope is steeper than one unit vertical in one unit horizontal (100-percent slope), the toe of the slope shall be assumed to be at the intersection of a horizontal plane drawn from the top of the foundation and a plane drawn tangent to the slope at an angle of 45 degrees (0.79 rad) to the horizontal. Where a retaining wall is constructed at the toe of the slope, the height of the slope shall be measured from the top of the wall to the top of the slope.

R403.1.7.2 Footing setback from descending slope surfaces.

Footings on or adjacent to slope surfaces shall be founded in material with an embedment and setback from the slope surface sufficient to provide vertical and lateral support for the footing without detrimental settlement. Except as provided for in Section R403.1.7.4 and Figure R403.1.7.1, the following setback is deemed adequate to meet the criteria. Where the slope is steeper than one unit vertical in one unit horizontal (100-percent slope), the required setback shall be measured from an imaginary plane 45 degrees (0.79 rad) to the horizontal, projected upward from the toe of the slope.

R403.1.7.3 Foundation elevation.

On graded sites, the top of any exterior foundation shall extend above the elevation of the street gutter at point of discharge or the inlet of an approved drainage device a minimum of 12 inches (305 mm) plus 2 percent. Alternate elevations are permitted subject to the approval of the building official, provided it can be demonstrated that required drainage to the point of discharge and away from the structure is provided at all locations on the site.

R403.1.7.4 Alternate setback and clearances.

Alternate setbacks and clearances are permitted, subject to the approval of the building official. The building official is permitted to require an investigation and recommendation of a qualified engineer to demonstrate that the intent of this section has been satisfied. Such an investigation shall include consideration of material, height of slope, slope gradient, load intensity and erosion characteristics of slope material.

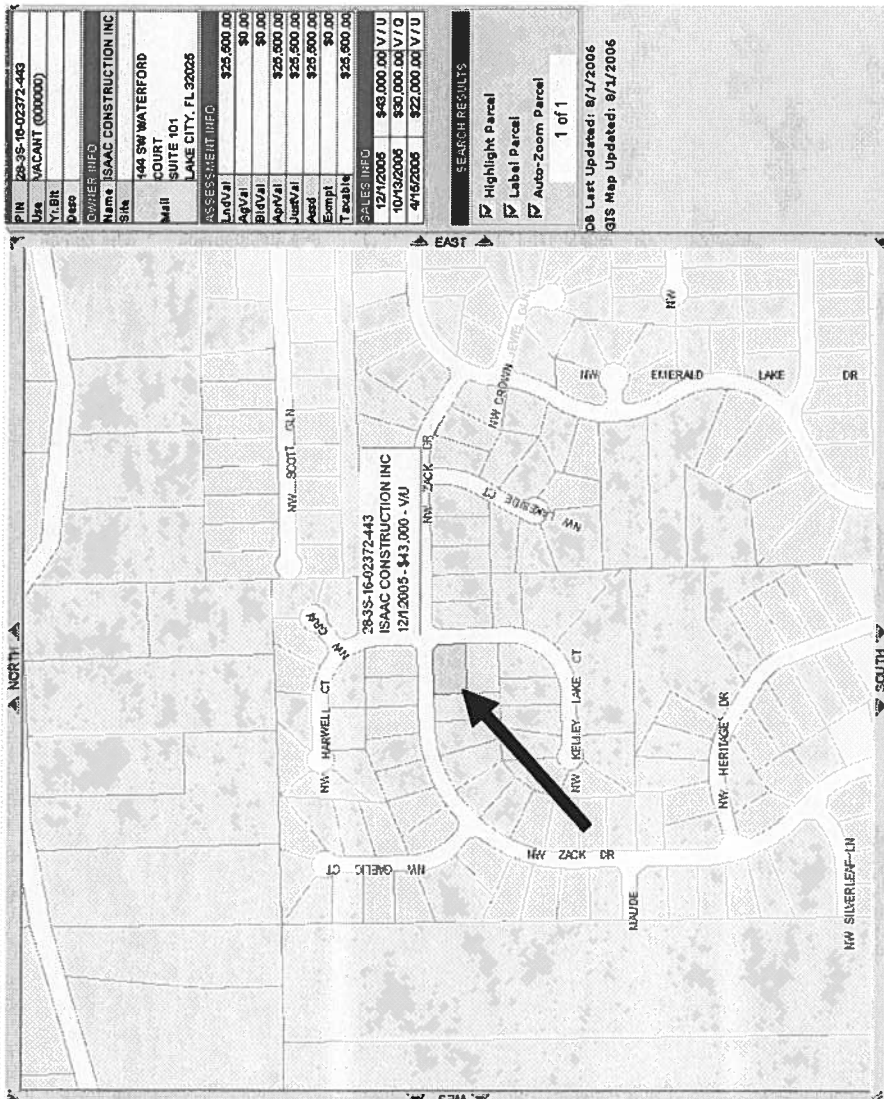
R403.1.8 Foundations on expansive soils.

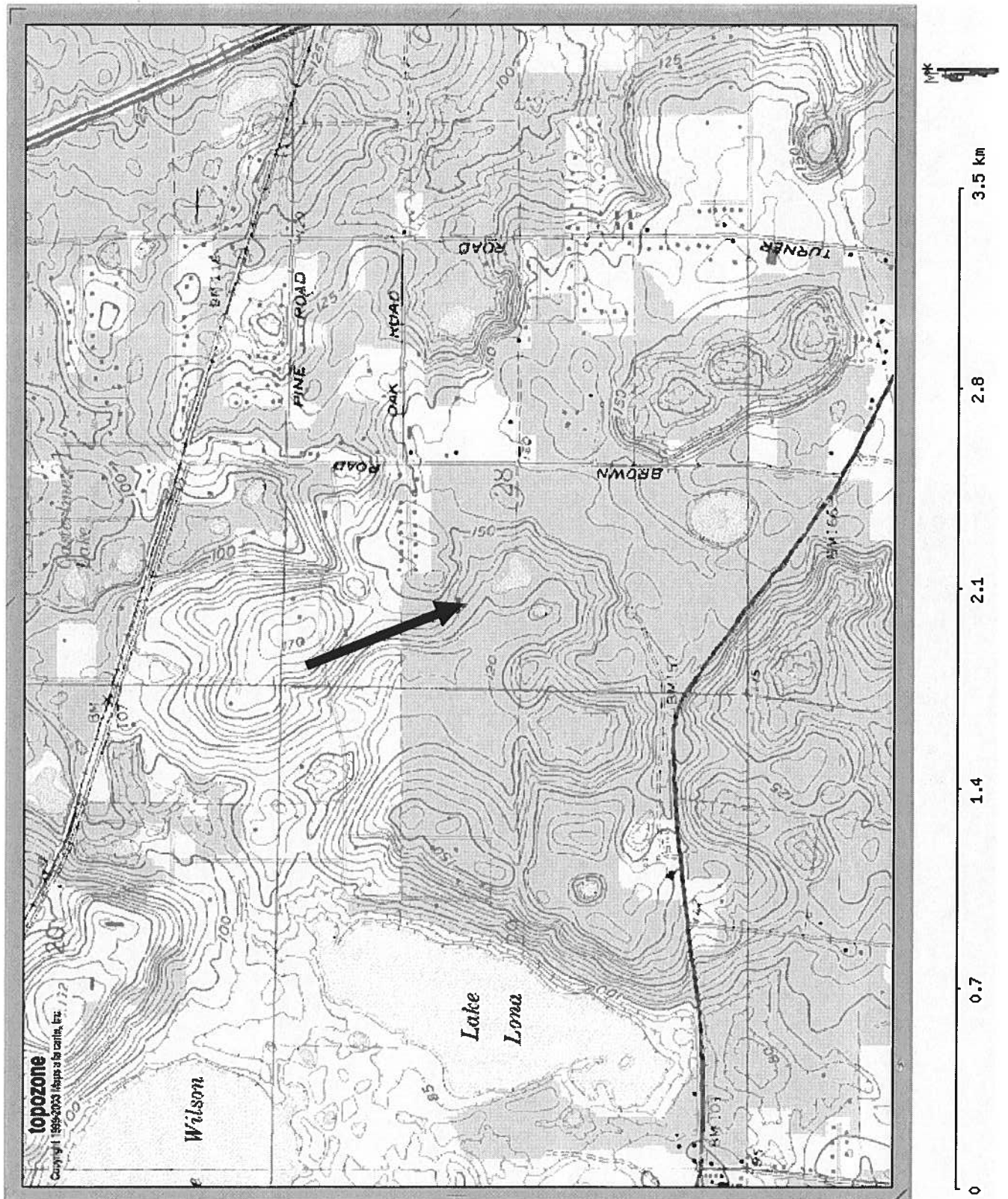
Foundation and floor slabs for buildings located on expansive soils shall be designed in accordance with Section 1805.8 of the Florida Building Code, Building. Exception: Slab-on-ground and other foundation systems which have performed adequately in soil conditions similar to those encountered at the building site are permitted subject to the approval of the building official.

R403.1.8.1 Expansive soils classifications.

Soils meeting all four of the following provisions shall be considered expansive, except that tests to show compliance with items 1, 2 and 3 shall not be required if the test prescribed in item 4 is conducted:

1. Plasticity Index (PI) of 15 or greater, determined in accordance with ASTM D 4318.
2. More than 10 percent of the soil particles pass a No. 200 sieve (75 mm), determined in accordance with ASTM D 422.
3. More than 10 percent of the soil particles are less than 5 micrometers in size, determined in accordance with ASTM D 422.
4. Expansion Index greater than 20, determined in accordance with ASTM D 4829.







Phone (386) 755-3611

Fax (386) 752-5381

24622

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code (FBC) 104.2.6)

Aspen Pest Control, Inc.

(386) 755-3611

State License # - JB109476

State Certification # - JF104376

, Lot 143 Harwell Place (Emerald Lakes), Lake City, Fl

Address of Treatment or Lot/Block of Treatment


Bora-Care Wood Treatment – 23% Disodium Octaborate Tetrahydrate

Method of Termite Prevention Treatment – Soil Barrier, Wood Treatment, Bait System, Other

Application onto Structural Wood

Description of Treatment

The above named structure will receive a complete treatment for the prevention of subterranean termites at the dried-in stage of construction. Treatment is done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services and according to EPA registered label directions as stated in Florida Building Code Section 1861.1.8.


Authorized Signature
Date

Benji Logstrom

Mark Disosway, P.E.

POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4817

ISAAC
CONSTRUCT

11 Aug 2006

Building and Zoning, Columbia County, Florida

*24622

Re: Site Evaluation, ISAAC CONSTRUCTION, SPEC HOUSE, 144 SW WATERFORD COURT, SUITE 101, LAKE CITY, FL 32025, Columbia County, FL

Dear Building Inspector:

The elevation of the top of the stem wall foundation as prepped for slab by builder is clearly less than one foot above the elevation of the county road, SW WATERFORD COURT at a point immediately in front of the house.

Based on topo maps, FEMA Flood Insurance Rate Map, and visual inspection the proposed finished floor elevation, the top of the stem wall foundation as indicated by string lines which vary from 6" above natural grade to 6' above grade, is at an adequate elevation to avoid flooding.

Flood Zone of Home Site: Zone X; Based on the FEMA rate map, attached.

Home Site Natural Grade, Elevation: about 130 - 140 ft; Based on topo map, attached.

Zone A flood zone 600' to south of home site, Elevation: about 125 ft; Based on topo map, attached..

Proposed Finished Floor Elevation: 6" - 72" above natural grade in the perimeter of the house.

Observations: This house is higher (about 15 ft) than the county road behind it and nearby isolated flood Zone A established by FEMA based on estimation of the flood zone elevations from the topo map. There is clearly continuous slope drainage to this isolated flood zone and on to Lake Lona which is about 25' lower. The finished floor is about 8 - 12 feet higher than two neighboring houses which were recently built at over one foot above the road.

The finished floor elevation must be minimum 6" above finished grade per FBC2004. The finished grade should slope down from that elevation for another 6" within 12 feet away from the house in all directions so that all runoff drains away from the house. The owner must maintain the swales, slopes, and ditch to provide free drainage to the ditch and prevent any possibility of storm water backing up into the house.

The owner should be aware that if free drainage is not maintained thru fields and across roads and thru culverts to the sink holes, or if future development in the area causes increased storm water run off, or if rainfall occurs with greater flooding effect than the design storm, the level of the nearby Zone A could rise higher than anticipated and his house would be more susceptible to flooding.

Sincerely,

Mark Disosway
11 SEP 06

Mark Disosway, PE

R403.1 General.

All exterior walls shall be supported on continuous solid or fully grouted masonry or concrete footings, wood foundations, or other approved structural systems which shall be of sufficient design to accommodate all loads according to Section R301 and to transmit the resulting loads to the soil within the limitations as determined from the character of the soil. Footings shall be supported on undisturbed natural soils or engineered fill.

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Minimum sizes for concrete and masonry footings shall be as set forth in Table R403.1 and Figure R403.1(1). The footing width, W , shall be based on the load-bearing value of the soil in accordance with Table R401.4.1. Spread footings shall be at least 6 inches (152 mm) in thickness. Footing projections, P , shall be at least 2 inches (51 mm) and shall not exceed the thickness of the footing. The size of footings supporting piers and columns shall be based on the tributary load and allowable soil pressure in accordance with Table R401.4.1. Footings for wood foundations shall be in accordance with the details set forth in Section R403.2, and Figures R403.1(2) and R403.1(3).

R403.1.4 Minimum depth.

All exterior footings shall be placed at least 12 inches (305 mm) below the undisturbed ground surface.

R403.1.5 Slope.

The top surface of footings shall be level. The bottom surface of footings shall not have a slope exceeding one unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footings or where the slope of the bottom surface of the footings will exceed one unit vertical in ten units horizontal (10-percent slope).

R403.1.6 Foundation anchorage.

When braced wall panels are supported directly on continuous foundations, the wall wood sill plate or cold-formed steel bottom track shall be anchored to the foundation in accordance with this section. The wood sole plate at exterior walls on monolithic slabs and wood sill plate shall be anchored to the foundation with anchor bolts spaced a maximum of 6 feet (1829 mm) on center. There shall be a minimum of two bolts per plate section with one bolt located not more than 12 inches (305 mm) or less than seven bolt diameters from each end of the plate section. Bolts shall be at least 1/2 inch (12.7 mm) in diameter and shall extend a minimum of 7 inches (178 mm) into masonry or concrete. Interior bearing wall sole plates on monolithic slab foundations shall be positively anchored with approved fasteners. A nut and washer shall be tightened on each bolt to the plate. Sills and sole plates shall be protected against decay and termites where required by Sections R319 and R320. Cold-formed steel framing systems shall be fastened to the wood sill plates or anchored directly to the foundation as required in Section R505.3.1 or R603.1.1. Exception: Foundation anchor straps, spaced as required to provide equivalent anchorage to 1/2-inch-diameter (12.7 mm) anchor bolts.

R403.1.6.1 Reserved.

R403.1.7 Footings on or adjacent to slopes.

The placement of buildings and structures on or adjacent to slopes steeper than 1 unit vertical in 3 units horizontal (33.3-percent slope) shall conform to Sections R403.1.7.1 through R403.1.7.4.

R403.1.7.1 Building clearances from ascending slopes.

In general, buildings below slopes shall be set a sufficient distance from the slope to provide protection from slope drainage, erosion and shallow failures. Except as provided in Section R403.1.7.4 and Figure R403.1.7.1, the following criteria will be assumed to provide this protection. Where the existing slope is steeper than one unit vertical in one unit horizontal (100-percent slope), the toe of the slope shall be assumed to be at the intersection of a horizontal plane drawn from the top of the foundation and a plane drawn tangent to the slope at an angle of 45 degrees (0.79 rad) to the horizontal. Where a retaining wall is constructed at the toe of the slope, the height of the slope shall be measured from the top of the wall to the top of the slope.

R403.1.7.2 Footing setback from descending slope surfaces.

Footings on or adjacent to slope surfaces shall be founded in material with an embedment and setback from the slope surface sufficient to provide vertical and lateral support for the footing without detrimental settlement. Except as provided for in Section R403.1.7.4 and Figure R403.1.7.1, the following setback is deemed adequate to meet the criteria. Where the slope is steeper than one unit vertical in one unit horizontal (100-percent slope), the required setback shall be measured from an imaginary plane 45 degrees (0.79 rad) to the horizontal, projected upward from the toe of the slope.

R403.1.7.3 Foundation elevation.

On graded sites, the top of any exterior foundation shall extend above the elevation of the street gutter at point of discharge or the inlet of an approved drainage device a minimum of 12 inches (305 mm) plus 2 percent. Alternate elevations are permitted subject to the approval of the building official, provided it can be demonstrated that required drainage to the point of discharge and away from the structure is provided at all locations on the site.

R403.1.7.4 Alternate setback and clearances.

Alternate setbacks and clearances are permitted, subject to the approval of the building official. The building official is permitted to require an investigation and recommendation of a qualified engineer to demonstrate that the intent of this section has been satisfied. Such an investigation shall include consideration of material, height of slope, slope gradient, load intensity and erosion characteristics of slope material.

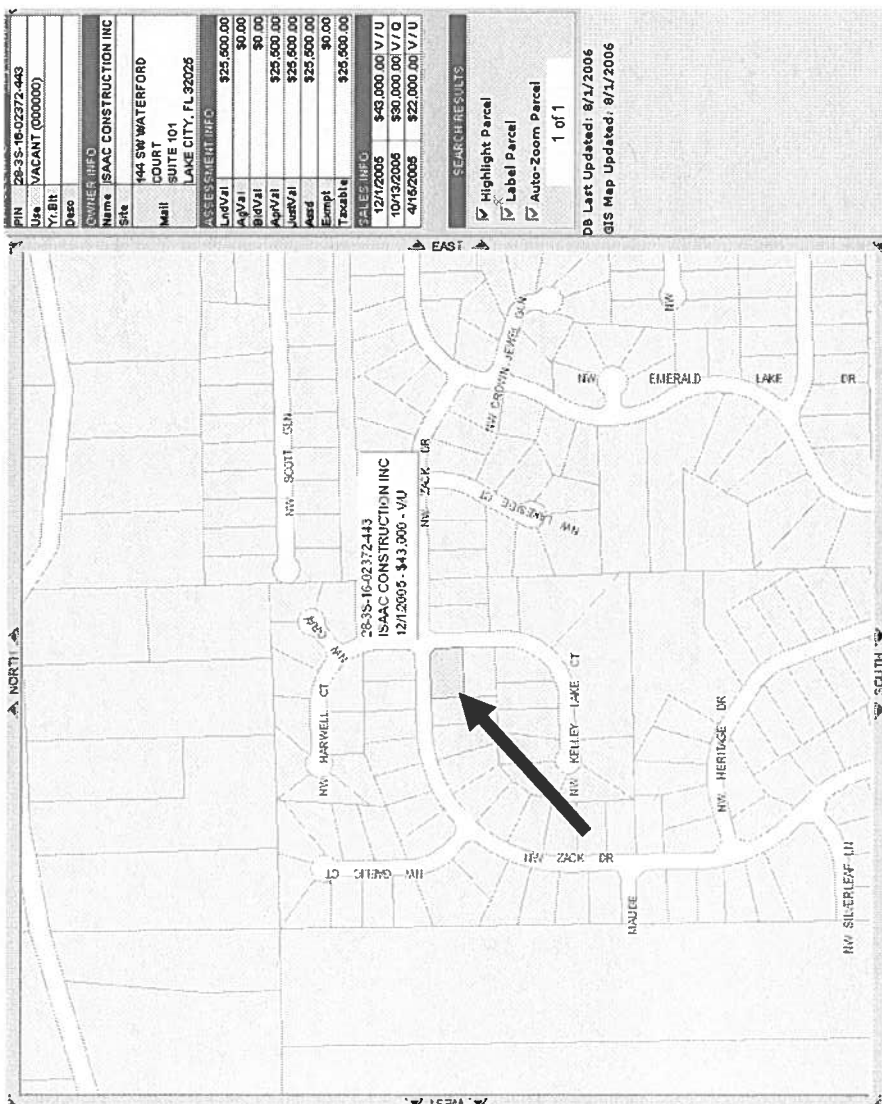
R403.1.8 Foundations on expansive soils.

Foundation and floor slabs for buildings located on expansive soils shall be designed in accordance with Section 1805.8 of the Florida Building Code, Building. Exception: Slab-on-ground and other foundation systems which have performed adequately in soil conditions similar to those encountered at the building site are permitted subject to the approval of the building official.

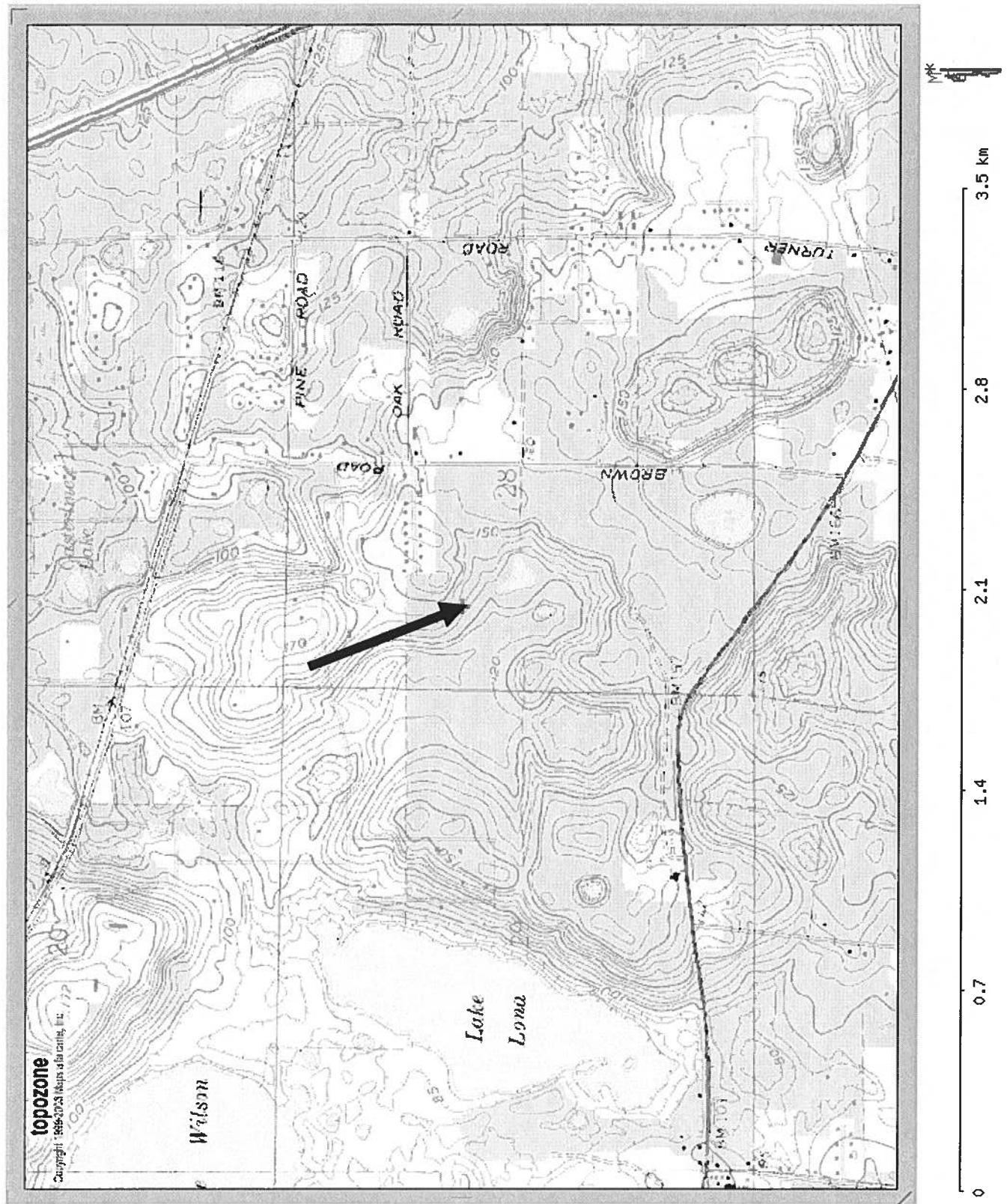
R403.1.8.1 Expansive soils classifications.

Soils meeting all four of the following provisions shall be considered expansive, except that tests to show compliance with Items 1, 2 and 3 shall not be required if the test prescribed in Item 4 is conducted:

1. Plasticity Index (PI) of 15 or greater, determined in accordance with ASTM D 4318.
2. More than 10 percent of the soil particles pass a No. 200 sieve (75 mm), determined in accordance with ASTM D 422.
3. More than 10 percent of the soil particles are less than 5 micrometers in size, determined in accordance with ASTM D 422.
4. Expansion Index greater than 20, determined in accordance with ASTM D 4829.



wind00





Joe,
this has a great slope to the
South may want to check concerning
the height of Block being set. May
Require additional Engineering.
R.

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0605-90**

Contractor: Isaac Construction Owner: Isaac Construction Lot 143 Phase 4
Emerald Lakes Subdivision.

On the date of June 1, 2006 application 0605-90 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0605-90 when making reference to this application.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please apply the following building codes in the garage area FRC-2004 sections R309 R309.1 Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm)

in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. In the garage area show compliance with the FRC-2004 sections R309.1.1 Duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. In the garage area show compliance with the FRC-2004 sections R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

2. On the electrical plan show the location of the electrical panel is shown, also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Joe Haltiwanger

A red ink signature of Joe Haltiwanger, written in a cursive style, positioned over the text 'Plan Examiner'.

Plan Examiner
Columbia County Building Department



FLORIDA BUILDING CODE

- Overview
- User Registration
- Organization Registration
- User Authentication
- Organization Search
- Organization Approval Status

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Cancel

Search

Result List for Organizations

Displaying 1-1 of 1

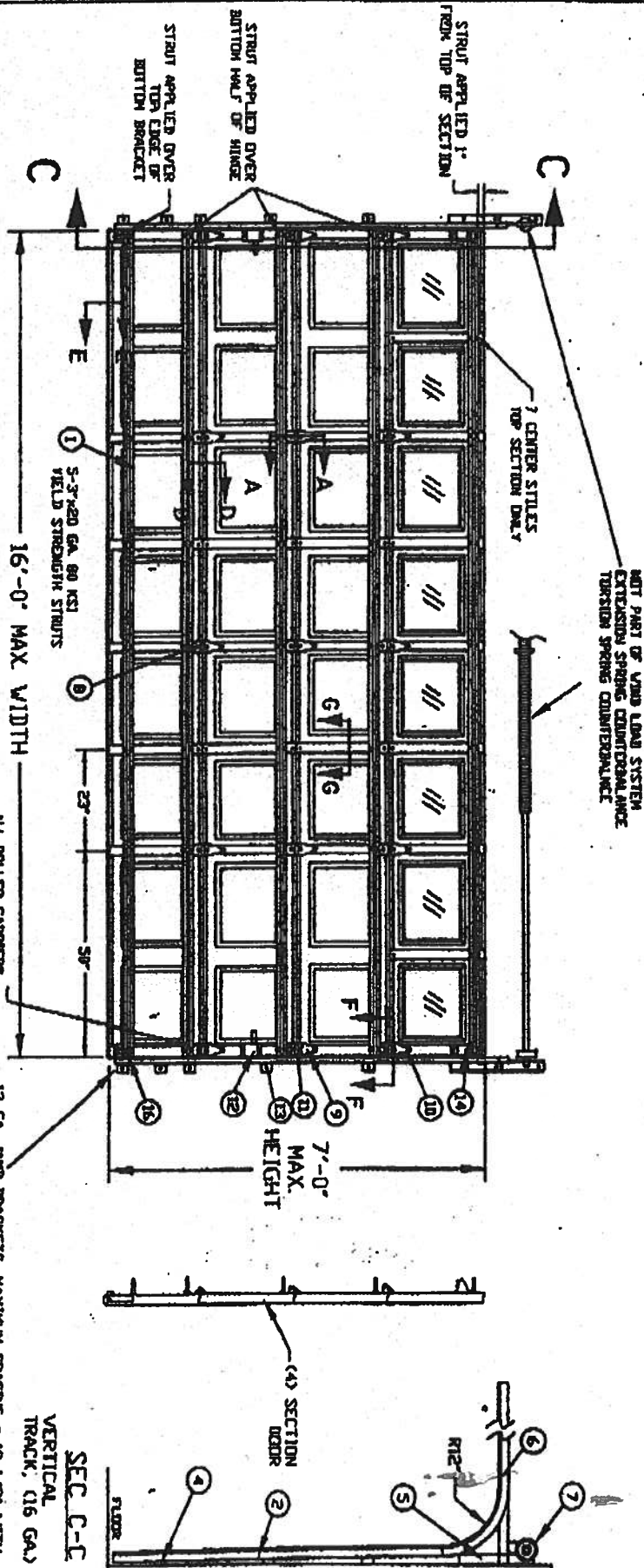
Name	City	Contact	Phone	Type	Expire	Status
General American Door	Montgomery	James Campbell	630-593000	Product Manufacturer	01/01/2009	Approved
Org Code: FDM System ID: 3585						Site Link: www.gadco.com

Displaying 1-1 of 1

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NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF SECTION AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT: 21'
3. SECTION HEIGHTS OF 21' AND 19.5' ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. VARIOUS MAY BE INSTALLED IN THE TOP SECTION, AS TESTED WITH 1/4" BSS GLASS OR EQUIVALENT OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MAXIMUM LENGTH OF ROLLER STICH IS 3/4" OR AS TESTED
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOW.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEE SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE AL OR AS TESTED.
9. DROP IN TYPE OF INSULATION IS OPTIONAL.

**INSIDE ELEVATION**

16'-0" MAX WIDTH

ALL ROLLER CARRIERS AND ROLLERS ARE 14 GA

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR, END BRACKET NEAR THE HORIZONTAL 5" OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION

SEC C-C
VERTICAL TRACK, (16 GA.)

DESIGN LOAD +200 PSF & -200 PSF
TEST LOAD +300 PSF & -300 PSF

TEST REPORTS ON FILE VIDEO 10/19/00 002933

GAUGED DOORS

SERIES 7400, EXTERIOR STEEL - 407 MIN OAS TESTED
SERIES 7825, EXTERIOR STEEL - 407 MIN A
SERIES 7524, EXTERIOR STEEL - 404 MIN A
(TESTED WITH VARIOUS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL LITE STYLE SPACING	STRUTS 60 KSI		VERTICAL TRACK
16'	7'	23"	SIZE	QTY.	
			3"	5	2 IN.



GENERAL AMERICAN DOOR COMPANY
5050 BASLINE ROAD
MONTICEMERY, IL 60058

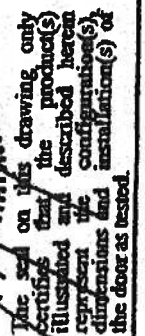
DESIGN LOADS: 10' x 7' MAX. RAISED PANEL STEEL DOOR - VERTICAL TRACK, (16 GA.)

DESIGN LOADS: 10' x 7' MAX. RAISED PANEL STEEL DOOR - VERTICAL TRACK, (16 GA.)
PAGE 1 OF 2
DRAWING NUMBER: V13220-1

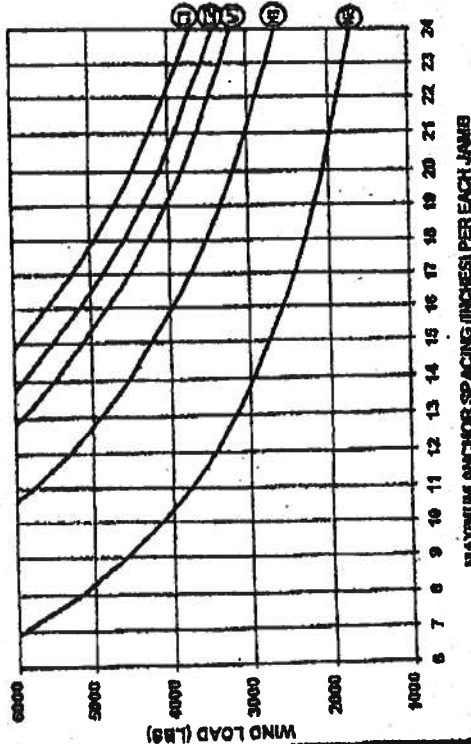
The seal on this drawing only represents the product(s) illustrated and the product(s) of the door as tested.



REPORT No. 2202



WIND LOAD vs ANCHOR SPACING



MAXIMUM ANCHOR SPACING (INCHES) PER EACH JAMB

DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)
 LOAD FT²

EXAMPLE

30 LBS X 16 FT WIDE X 8 FT HIGH = 3840 LBS
 FT²

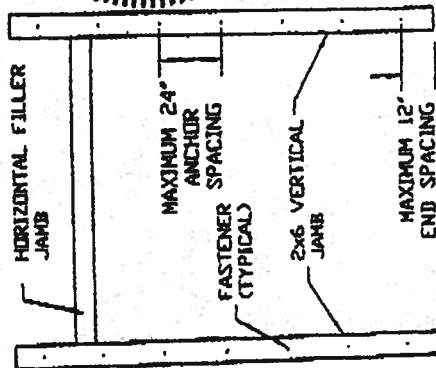
⑩ USE 22" SPACING

⑨ USE 21" SPACING

⑧ USE 19" SPACING

SEE NOTE 11 FOR ADDITIONAL
 REQUIRED 2X6 VJMB JAMB ANCHORS

HORIZONTAL FILLER JAMB

SEAL
PE No. 024280ENGINEER
ROGER R. KETYAN

3/8/2002

2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2X6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

- 1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.
- 2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTS 10," CURRENT EDITION.
- 3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
- 4) WOOD FRAME BUILDINGS STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.
- 5) REINFORCED CMU OR CONCRETE 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2500 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- 6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- 7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"
- 8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.
- 9) WASHERS ARE REQUIRED ON ALL FASTENERS.
- 10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- 11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

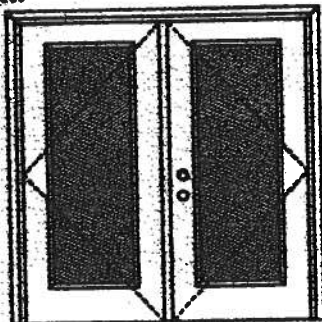


GENERAL AMERICAN DOOR COMPANY
 5600 BASELINE ROAD
 MORTGENTHE, IL 60538

WIND MINE	DESIGNED BY	DATE	BY
NO. 8-30-99	REVISED		
JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		DRAWING NUMBER AUS60	

XX**Glazed Outswing Unit**

DDP-WL-JH162-02

WOOD-EDGE STEEL DOORS**APPROVED ARRANGEMENT:**

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:**1/4 GLASS:**

100 Series



122, 125 Series



126 Series



680 Series



822 Series

1/2 GLASS:

105 Series*



106, 160 Series*



120 Series*



200 Series*



12 RL, 25 RL, 24 RL Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson
EntrySystems

March 28, 2002

Our continuing program of product improvement makes specifications, designs and product detail subject to change without notice.

PREMDORE Collection
Premium Quality Doors



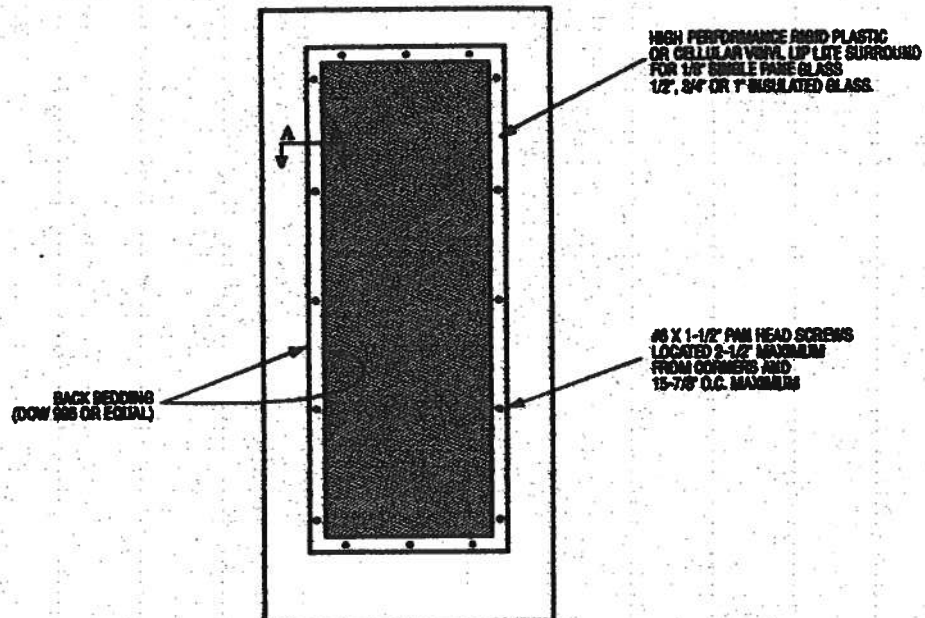
Exclusively from

Masonite

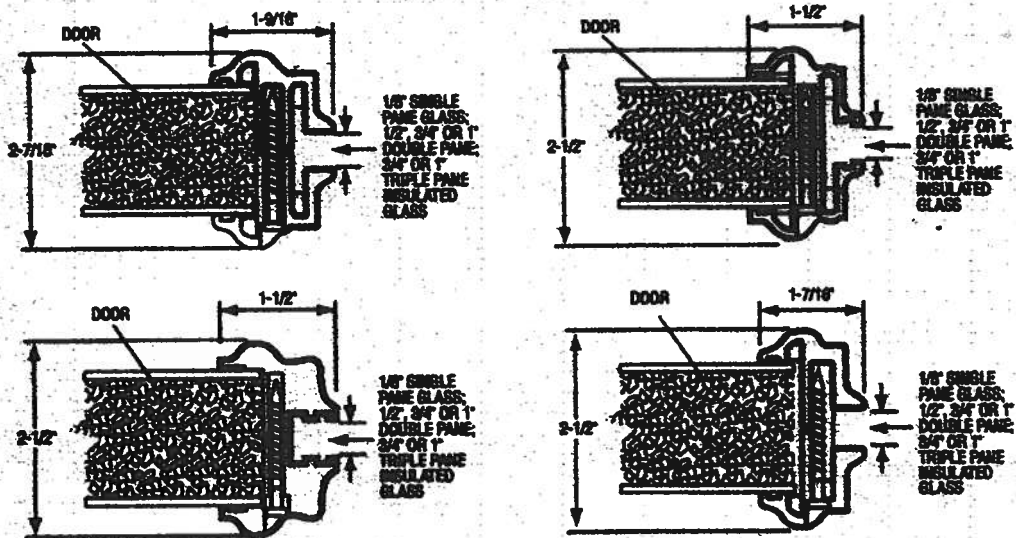
Masonite International Corporation

IMAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A TYPICAL RIGID PLASTIC LIP LITE SURROUND



March 29, 2002
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Exclusively from
Masonite
Masonite International Corporation

XX

Glazed Outswing Unit

COP-VL-JH4162-02

WOOD-EDGE STEEL DOORS**APPROVED DOOR STYLES:****3/4 GLASS:**

404 Series



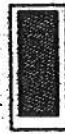
410 Series



450 Series

FULL GLASS:

100 Series



114, 120, 122 Series



162 Series



140 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1884-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Johnson
EntrySystems

March 29, 2002

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PRENDRE
Premium Quality Doors



Exclusively from

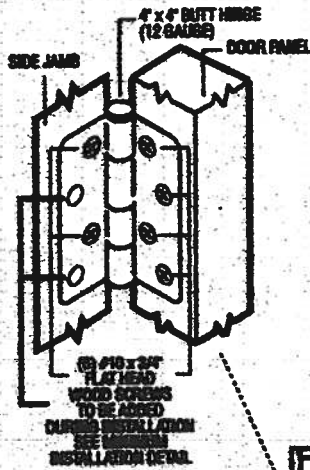
Masonite

Masonite International Corporation

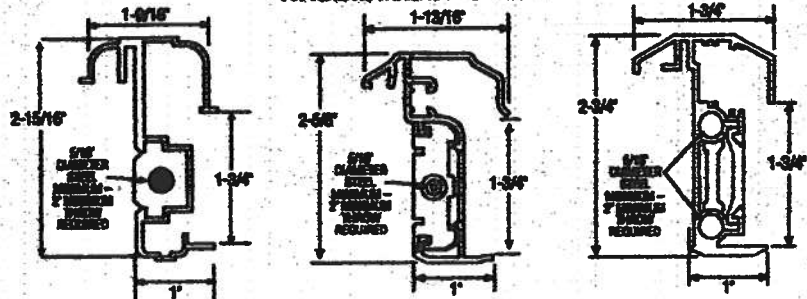
XX
Unit

OUTSWING UNITS WITH DOUBLE DOOR

TYPICAL HINGE ATTACHMENT

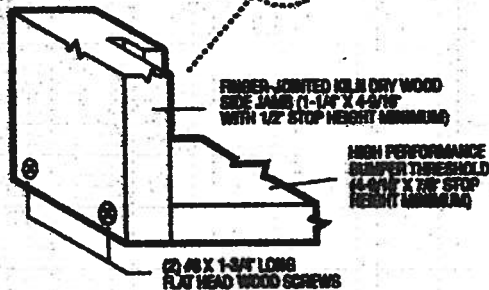


TYPICAL ASTRAL PROFILES

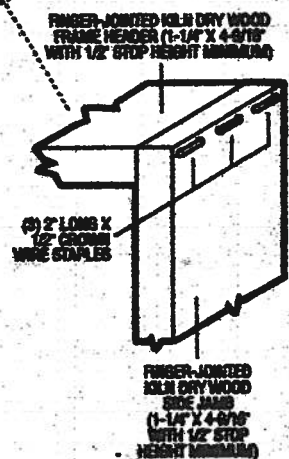


ALUMINUM EXTRUDED ASTRAL (0.06" MINIMUM WALL THICKNESS) WITH ADDED REINFORCEMENT INSERTS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDRICAL LATCH BOLT LOCATIONS. ATTACH WITH #6 X 1" PAN HEAD SCREWS - LOCATE 1" FROM EACH END MINIMUM AND 22" O.C. MAXIMUM.

TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT



TYPICAL HEADER & SIDE JAMB ATTACHMENT



March 20, 2002
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PREMDORCA
Premium Quality Doors



Exclusively from

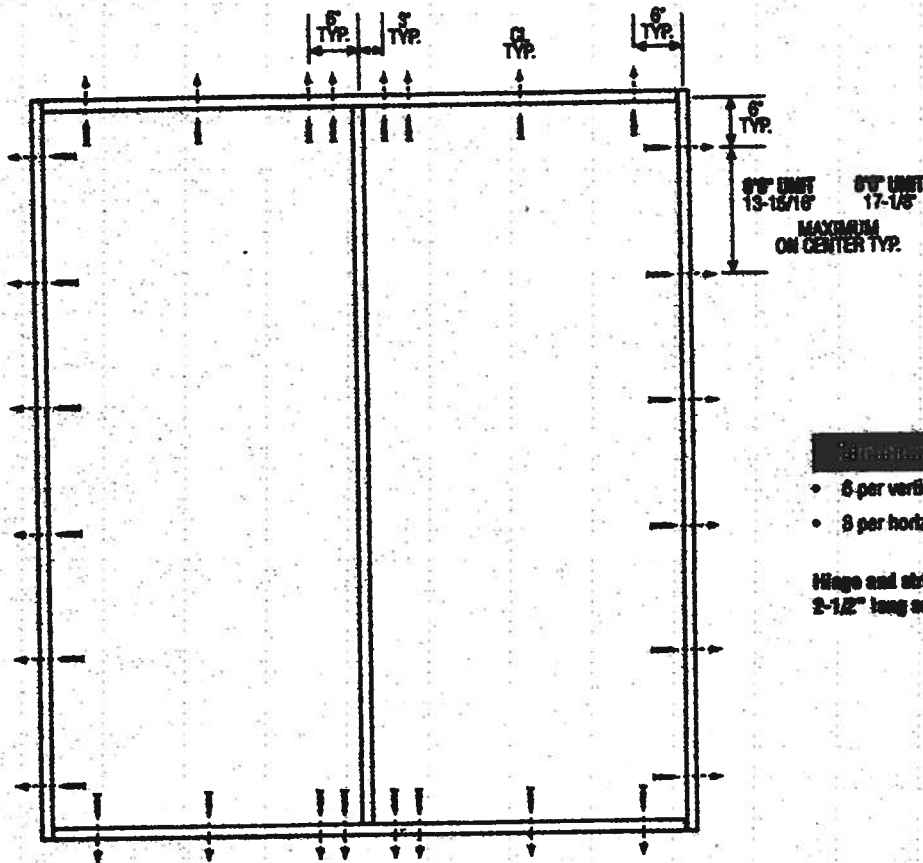
Masonite

Masonite International Corporation

XX
Unit

IMD-WL-MA0002-02

DOUBLE DOOR



- Minimum Fastener Spacing**
- 6 per vertical framing member
 - 3 per horizontal framing member

Hinge and strike plates require two 2-1/2\"

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16\"
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4\"
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 29, 2002
Our continuing program of product improvement unless specifications,
design and product detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation



FEB - 4 REC'D

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

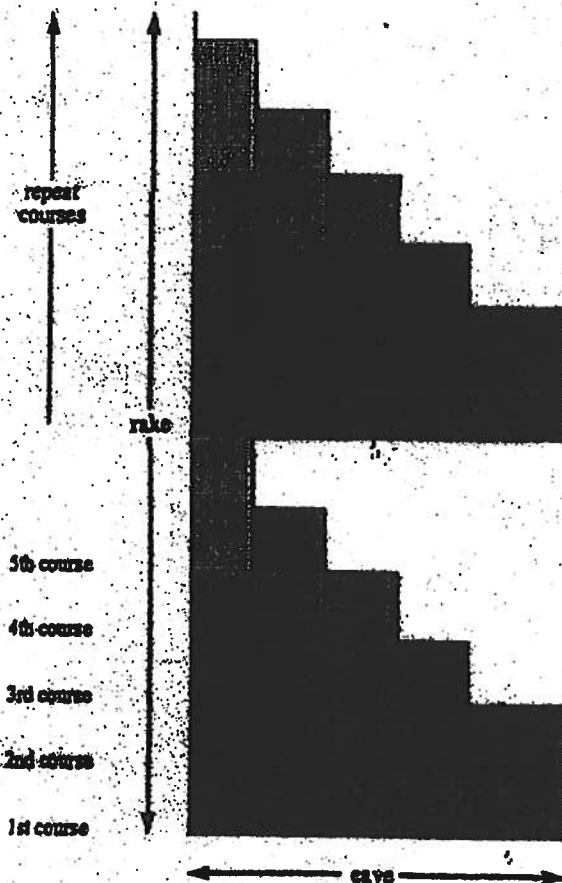
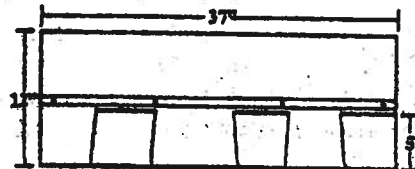
TAMKO Roofing Products, Inc.

CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-841-1925



Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"



The 4 cuts in the first 10 courses:



In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.



Application Instructions for

• **Glass-Seal**
• **Glass-Seal AR**

• **Elite Glass-Seal®**
• **Elite Glass-Seal® AR**

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS. THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER. IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and ridges.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thick and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.

FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.

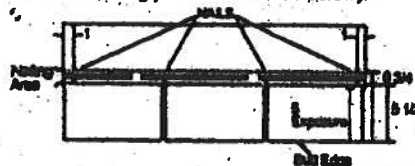
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These

conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

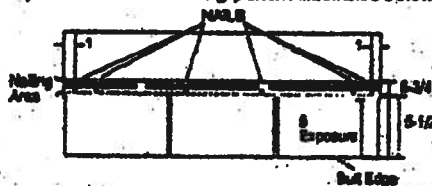
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagrams and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) **Standard Fastening Pattern.** (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below.)



2) **Mansard or High Wind Fastening Pattern.** (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fasteners per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

Visit Our Web Site at
www.tamko.com

Central District	220 West 4th St., Joplin, MO 64801	800-841-4691
Northeast District	4600 Tamko Dr., Frederick, MD 21701	800-368-2066
Southeast District	2300 36th St., Tuscaloosa, AL 35401	800-228-2666
Southwest District	7910 S. Central Exp., Dallas, TX 75216	800-443-1834
Western District	8300 East 43rd Ave., Denver, CO 80216	800-530-8868

0791

TAMKO

ROOFING PRODUCTS

(CONTINUED FROM Pg. 2)

Glass-Seal Glass-Seal AR

Elite Glass-Seal® Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nailing procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tab from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter-shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 1.

Second and Subsequent Courses: According to the off-set application method you choose to use, remove the appropriate length from the

rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.

VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Past® or a minimum 50 lb. roof roofing in the valley. Nail the fast only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- **CAUTION:** Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

Visit Our Web Site at
www.tamko.com

Central District 220 West 4th St., Joplin, MO 64801
Northeast District 4500 Tamko Dr., Frederick, MD 21701
Southeast District 2300 35th St., Tuscaloosa, AL 35401
Southwest District 7910 S. Central Exp., Dallas, TX 75216
Western District 5300 East 43rd Ave., Denver, CO 80216

800-841-4691
800-358-2066
800-228-2658
800-443-1834
800-530-8868

07/01



(CONTINUED from Pg. 3)

• Glass-Seal
• Glass-Seal AR

• Elite Glass-Seal®
• Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

18. HIP AND RIDGE FASTENING DETAIL.

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

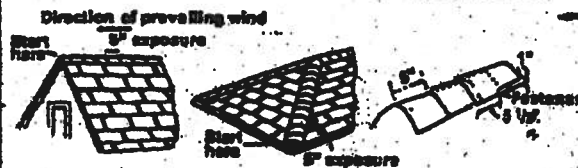
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.

Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHEN BENDING SHINGLES IN COOL WEATHER.

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.



THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.



MADE IN THE U.S.A.
TAMKO ROOFING PRODUCTS, INC.

IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE

In this paragraph "You" and "Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

Visit Our Web Site at
www.tamko.com

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Northeast District
Southeast District
Southwest District
Western District

220 West 4th St., Joplin, MO 64801
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2300 35th St., Tuscaloosa, AL 35401
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5300 East 43rd Ave., Denver, CO 80216

800-841-4891
800-388-2055
800-228-2658
800-443-1634
800-530-8888

07/01

I

**AAMA/NWDA 101/L8.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft ²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
De-glazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hesa
Mark A. Hesa, Technician

MAH:alb

Allen P. Reeves
1 APRIL 2002



II

Architectural Testing

AAMA/NWDA 101/LS-2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethtown, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/LS-2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

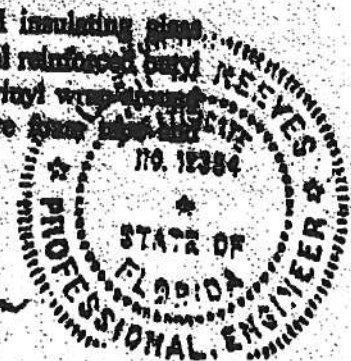
Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 3/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a rustal reinforced poly spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen H. Reeves
1 APRIL 2002



III

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reeves
1 APRIL 2002



IV

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft ²	0.3 cfm/ft ² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/L.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.42" 0.43"	0.26" max. 0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.02" 0.02"	0.18" max. 0.18" max.
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Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Degreasing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"	0.26" max.
	@ 47.2 psf (negative)	0.46"	0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	
	@ 70.8 psf (negative)	0.05"	

Allen H. Reeves
1 APRIL 2002



VI

01-41134.01
Page 5 of 5

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:



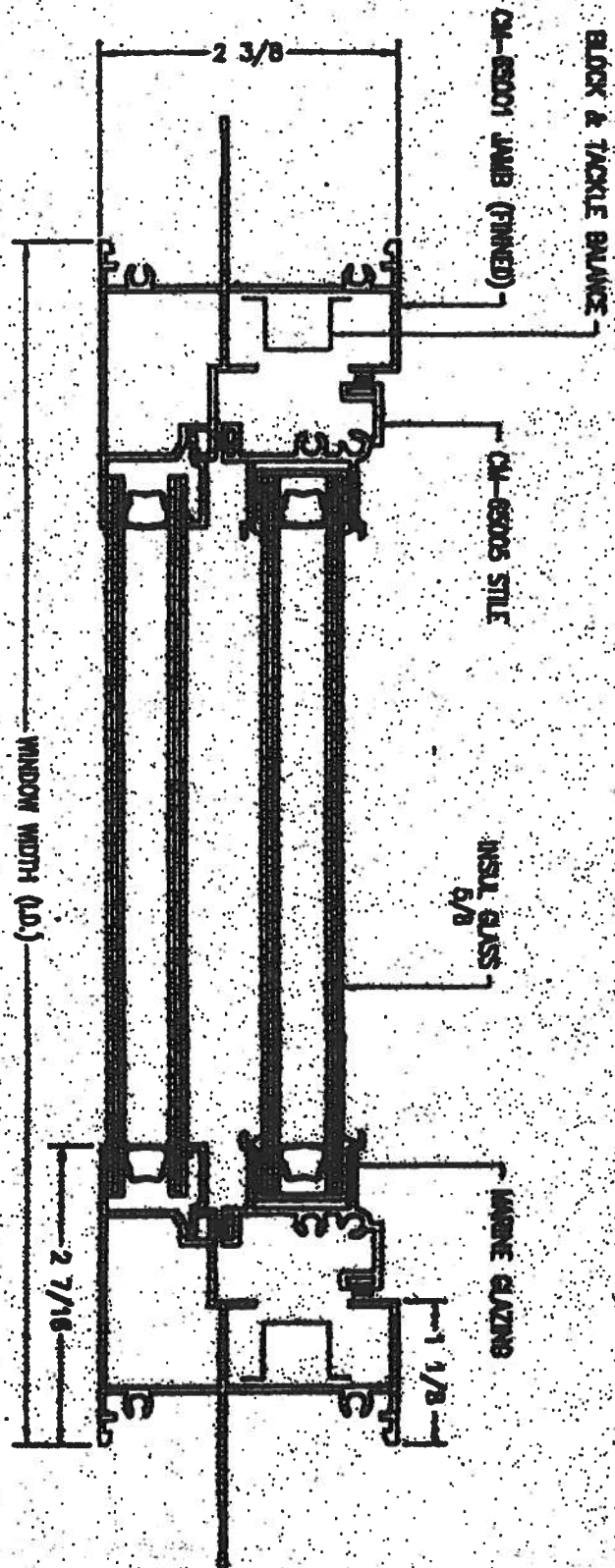
Mark A. Heas
Technician

MAH:nlb
01-41134.01

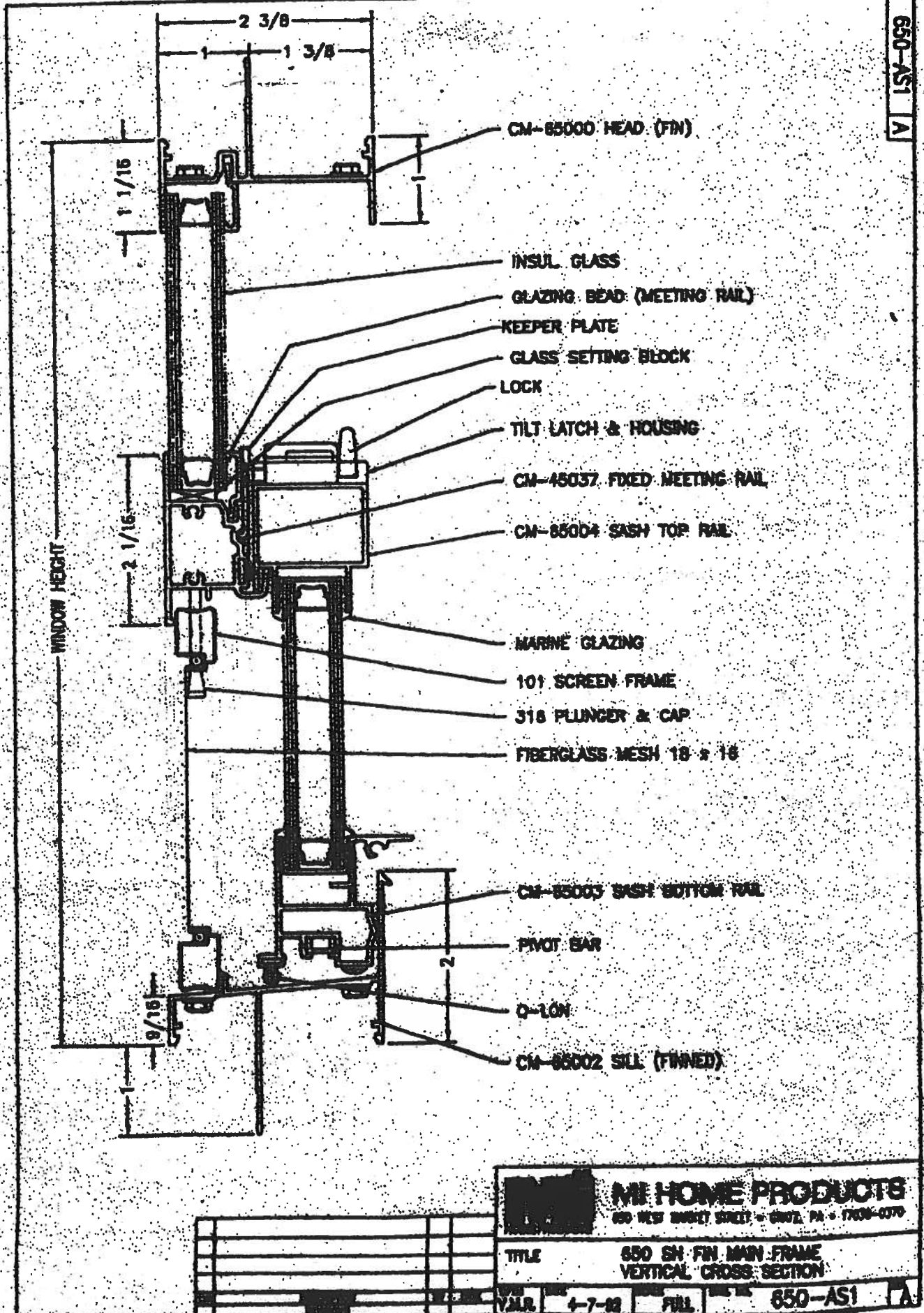


Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002





MI HOME PRODUCTS	
100 King Street, Suite 100 - Grand, Pa. 17030-0200	
650 SH FIN JAMB FRAME INSULATED GLASS HORIZONTAL CROSS SECTION	
DATE	REV
4-1-82	FULL
650-AS2	B



650-AS1 A

Residential System Sizing Calculation

Summary

ISAAC CONSTRUCTION

LAKE CITY, FL

Project Title:
THE MORICE, LOT 143

Code Only
Professional Version
Climate: North

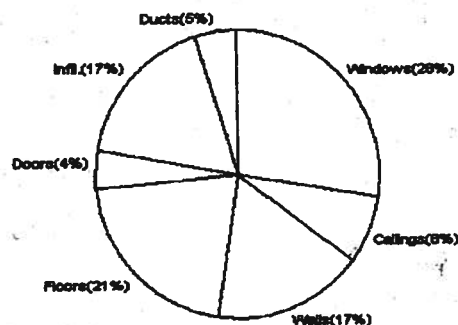
5/4/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature	31 F	Summer design temperature	93 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	39 F	Summer temperature difference	18 F
Total heating load calculation	31303 Btuh	Total cooling load calculation	30101 Btuh
Submitted heating capacity	36000 Btuh	Submitted cooling capacity	36000 Btuh
Submitted as % of calculated	115.0 %	Submitted as % of calculated	119.6 %

WINTER CALCULATIONS

Winter Heating Load (for 1858 sqft)

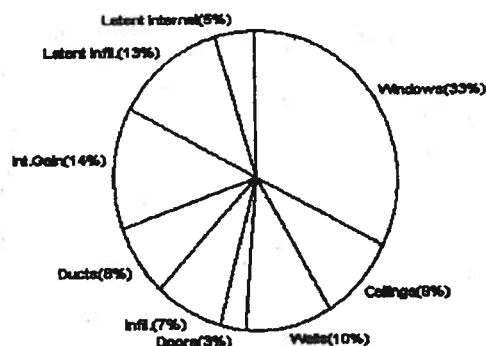
Load component		Load	
Window total	307 sqft	8688	Btuh
Wall total	1830 sqft	5376	Btuh
Door total	88 sqft	1404	Btuh
Ceiling total	1858 sqft	2415	Btuh
Floor total	209 ft	6604	Btuh
Infiltration	124 cfm	5325	Btuh
Subtotal		29812	Btuh
Duct loss		1491	Btuh
TOTAL HEAT LOSS		31303	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1858 sqft)

Load component		Load	
Window total	307 sqft	9873	Btuh
Wall total	1830 sqft	3046	Btuh
Door total	88 sqft	878	Btuh
Ceiling total	1858 sqft	2638	Btuh
Floor total		0	Btuh
Infiltration	109 cfm	2150	Btuh
Internal gain		4100	Btuh
Subtotal(sensible)		22686	Btuh
Duct gain		2269	Btuh
Total sensible gain		24954	Btuh
Latent gain(Infiltration)		3766	Btuh
Latent gain(internal)		1380	Btuh
Total latent gain		5146	Btuh
TOTAL HEAT GAIN		30101	Btuh



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: *[Signature]*

DATE: 5/4/06

Manual J Winter Calculations

Residential Load - Component Details (continued)

ISAAC CONSTRUCTION

Project Title:
THE MORICE, LOT 143

Code Only
Professional Version
Climate: North

LAKE CITY, FL

5/4/2006

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

ISAAC CONSTRUCTION

Project Title:
THE MORICE, LOT 143

Code Only
Professional Version
Climate: North

LAKE CITY, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

5/4/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	53.7	28.3	1519 Btuh
2	2, Clear, Metal, DEF	N	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	N	9.0	28.3	255 Btuh
4	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
6	2, Clear, Metal, DEF	E	4.0	28.3	113 Btuh
7	2, Clear, Metal, DEF	S	60.0	28.3	1698 Btuh
8	2, Clear, Metal, DEF	S	24.0	28.3	679 Btuh
9	2, Clear, Metal, DEF	W	20.0	28.3	588 Btuh
10	2, Clear, Metal, DEF	SW	21.0	28.3	594 Btuh
11	2, Clear, Metal, DEF	SE	21.0	28.3	594 Btuh
12	2, Clear, Metal, DEF	S	35.0	28.3	990 Btuh
Window Total			307		8688 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	198	1.6	317 Btuh
2	Frame - Exterior	13.0	1632	3.1	5059 Btuh
Wall Total			1830		5376 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Exter		48	17.9	861 Btuh
3	Wood - Adjac		20	9.2	184 Btuh
Door Total			88		1404 Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1858	1.3	2415 Btuh
Ceiling Total			1858		2415 Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	209.0 ft(p)	31.6	6604 Btuh
Floor Total			209		6604 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	18580(sqft)	124	5325 Btuh
	Mechanical			0	0 Btuh
Infiltration Total				124	5325 Btuh

Totals for Heating	Subtotal	29812 Btuh
	Duct Loss(using duct multiplier of 0.05)	1491 Btuh
	Total Btuh Loss	31303 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

ISAAC CONSTRUCTION

LAKE CITY, FL

Project Title:
THE MORICE, LOT 143

Code Only
Professional Version
Climate: North

5/4/2008

Totals for Cooling	Subtotal	22686 Btuh
	Duct gain(using duct multiplier of 0.10)	2269 Btuh
	Total sensible gain	24954 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	3766 Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380 Btuh
	Latent other gain	0 Btuh
	TOTAL GAIN	30101 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

ISAAC CONSTRUCTION

LAKE CITY, FL

Project Title:
THE MORICE, LOT 143

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F

5/4/2006

Window	Type Pane/SHGC/U/InSh/ExSh Omt	Overhang Len Hgt	Window Area(sqft)			HTM		Load
			Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, DEF, N, N	N 1.5 9	53.7	0.0	53.7	22	22	1181 Btuh
2	2, Clear, DEF, N, N	N 5 10	13.3	0.0	13.3	22	22	283 Btuh
3	2, Clear, DEF, N, N	N 5 4	9.0	0.0	9.0	22	22	198 Btuh
4	2, Clear, DEF, N, N	N 1.5 6	30.0	0.0	30.0	22	22	660 Btuh
5	2, Clear, DEF, N, N	W 1.5 5	16.0	1.0	15.0	22	72	1103 Btuh
6	2, Clear, DEF, N, N	E 1.5 1.5	4.0	3.0	1.0	22	72	139 Btuh
7	2, Clear, DEF, N, N	S 1.5 6	60.0	30.0	30.0	22	37	1770 Btuh
8	2, Clear, DEF, N, N	S 8 10	24.0	24.0	0.0	22	37	528 Btuh
9	2, Clear, DEF, N, N	W 1.5 6	20.0	0.5	19.5	22	72	1416 Btuh
10	2, Clear, DEF, N, N	SW 1.5 7.5	21.0	6.1	14.9	22	62	1059 Btuh
11	2, Clear, DEF, N, N	SE 3 7.5	21.0	13.6	7.4	22	62	756 Btuh
12	2, Clear, DEF, N, N	S 1.5 7.5	35.0	35.0	0.0	22	37	770 Btuh
Window Total			307					9873 Btuh
Walls	Type	R-Value	Area		HTM		Load	
	1 Frame - Adjacent	13.0	198.0		1.0		206 Btuh	
2	Frame - Exterior	13.0	1632.0		1.7		2840 Btuh	
Wall Total			1830.0				3046 Btuh	
Doors	Type	R-Value	Area		HTM		Load	
	1 Wood - Exter		20.0		10.0		200 Btuh	
2	Wood - Exter		48.0		10.0		479 Btuh	
3	Wood - Adjac		20.0		10.0		200 Btuh	
Door Total			88.0				878 Btuh	
Ceilings	Type/Color	R-Value	Area		HTM		Load	
	1 Under Attic/Dark	30.0	1858.0		1.4		2638 Btuh	
Ceiling Total			1858.0				2638 Btuh	
Floors	Type	R-Value	Size		HTM		Load	
	1 Slab-On-Grade Edge Insulation	0.0	209.0 ft(p)		0.0		0 Btuh	
Floor Total			209.0				0 Btuh	
Infiltration	Type	ACH	Volume		CFM=		Load	
	Natural	0.35	18580		108.6		2150 Btuh	
					0		0 Btuh	
Infiltration Total					109		2150 Btuh	
Internal gain		Occupants	Btuh/occupant		Appliance		Load	
		6	X 300 +		2300		4100 Btuh	

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-3S-16-02372-443

Building permit No. 000024622

Use Classification SFD, UTILITY

Fire: 39.06

Permit Holder ISAAC CONSTRUCTION

Waste: 117.25

Owner of Building ISAAC CONSTRUCTION

Total: 156.31

Location: 318 NW ZACK DRIVE(EMERALD LAKES, LOT 143)

Date: 03/02/2007

Harry Dick

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1SXA487-Z0217150931

Truss Fabricator: Anderson Truss Company
Job Identification: 6-034--Isaac Construction #143 Emerad Lakes -- , **
Truss Count: 43
Model Code: Florida Building Code 2004
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.24.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: CNBRGBLK-BRCLBSUB-A11015EE-GBLLETIN-PIGBACKB-

Seal Date: 05/17/2006

-Truss Design Engineer-

Arthur R. Fisher

Florida License Number: 59687

1950 Marley Drive

Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	00437--A1		06137010	05/17/06
2	00438--A2		06137016	05/17/06
3	00439--A3		06137003	05/17/06
4	00440--A4		06137007	05/17/06
5	00441--A5		06137006	05/17/06
6	00442--A6		06137005	05/17/06
7	00443--A7		06137029	05/17/06
8	00444--B1		06137013	05/17/06
9	00445--B2		06137011	05/17/06
10	00446--B3		06137012	05/17/06
11	00447--B4		06137013	05/17/06
12	00448--B5		06137009	05/17/06
13	00449--B6		06137010	05/17/06
14	00450--B7		06137028	05/17/06
15	00451--C1-GE		06137008	05/17/06
16	00452--C2		06137015	05/17/06
17	00453--D1-GE		06137036	05/17/06
18	00454--D2		06137038	05/17/06
19	00455--D3		06137039	05/17/06
20	00456--D4G		06137002	05/17/06
21	00457--HJ7		06137033	05/17/06
22	00458--EJ7		06137019	05/17/06
23	00459--J5		06137018	05/17/06
24	00460--J3		06137034	05/17/06
25	00461--J1		06137035	05/17/06
26	00462--HJA		06137017	05/17/06
27	00463--HJK		06137020	05/17/06
28	00464--EJK1		06137022	05/17/06
29	00465--EJK2		06137024	05/17/06
30	00466--J1K		06137021	05/17/06
31	00467--JA1		06137004	05/17/06
32	00468--JA2		06137012	05/17/06
33	00469--K1G		06137026	05/17/06
34	00470--K2		06137030	05/17/06
35	00471--K3		06137032	05/17/06
36	00472--AP1		06137023	05/17/06

#	Ref	Description	Drawing#	Date
37	00473--BP1		06137014	05/17/06
38	00474--BP2		06137011	05/17/06
39	00475--BP3		06137027	05/17/06
40	00476--R1-GE		06137025	05/17/06
41	00477--R2		06137031	05/17/06
42	00478--R3G		06137037	05/17/06
43	00479--S1-GE		06137001	05/17/06





Scale: 3/32" = 1'

7719-7143

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

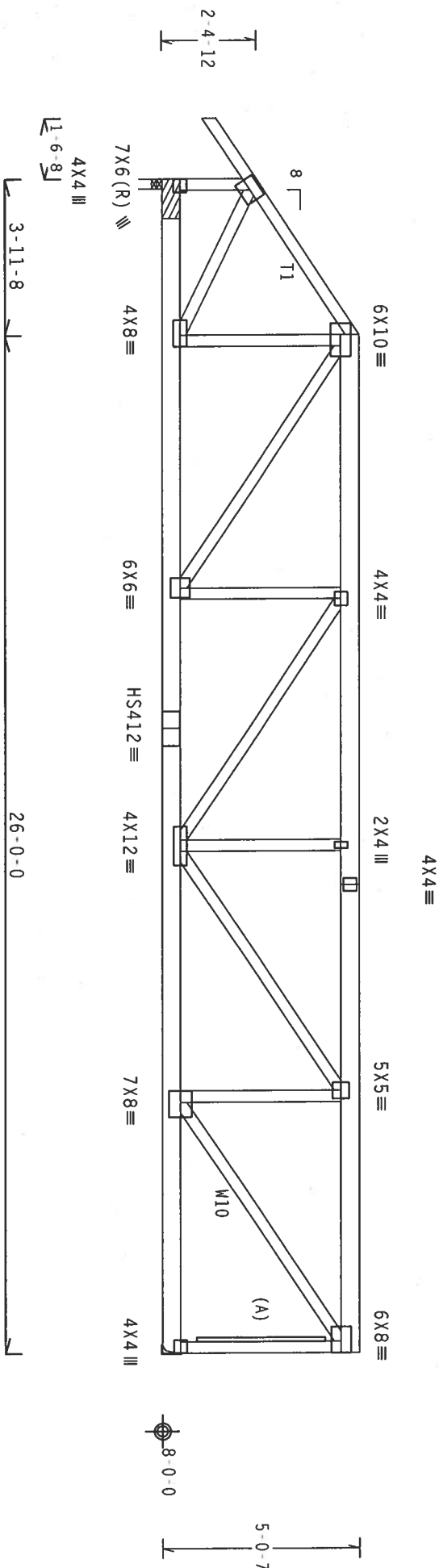
#1 hip supports 7-0-0 jacks.

Left side jacks have 3-11-8 setback with 0-0-0 cant and 1-6-8 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-6-8 overhang. side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

Bearing blocks: Nail type: 12d, Box or Gun (0.128"x3.25", min.), nails
BRG X-10C #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
1 0.000" 1 12" 4 Match Truss
Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGblk1103 for additional information.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. 20 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/10(0) \quad 7.24.1$$

EXCISE

FL	-	4	-	-	-	R	-
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Scale = .25" / Ft.


* **WARNING** * **RUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC51 1-3 (BUILDING COMPONENT SPECIFIC INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 5839 N. D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TOP CHORD CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AITPA) AND TPI.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING SPECIFICATIONS, FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR THE CONSTRUCTION OF THE BUILDING SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE TRUSS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC 8853, AISC 880, AISC 360, AISC 308, AISC 301, AISC 305, AISC 304, AISC 303, AISC 302, AISC 301, AISC 300, AISC 290, AISC 288, AISC 287, AISC 286, AISC 285, AISC 284, AISC 283, AISC 282, AISC 281, AISC 280, AISC 279, AISC 278, AISC 277, AISC 276, AISC 275, AISC 274, AISC 273, AISC 272, AISC 271, AISC 270, AISC 269, AISC 268, AISC 267, AISC 266, AISC 265, AISC 264, AISC 263, AISC 262, AISC 261, AISC 260, AISC 259, AISC 258, AISC 257, AISC 256, AISC 255, AISC 254, AISC 253, AISC 252, AISC 251, AISC 250, AISC 249, AISC 248, AISC 247, AISC 246, AISC 245, AISC 244, AISC 243, AISC 242, AISC 241, AISC 240, AISC 239, AISC 238, AISC 237, AISC 236, AISC 235, AISC 234, AISC 233, AISC 232, AISC 231, AISC 230, AISC 229, AISC 228, AISC 227, AISC 226, AISC 225, AISC 224, AISC 223, AISC 222, AISC 221, AISC 220, AISC 219, AISC 218, AISC 217, AISC 216, AISC 215, AISC 214, AISC 213, AISC 212, AISC 211, AISC 210, AISC 209, AISC 208, AISC 207, 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-287, AISC -288, AISC -289, AISC -290, AISC -291, AISC -292, AISC -293, AISC -294, AISC -295, AISC -296, AISC -297, AISC -298, AISC -2

R-3258 U-180 H-Simpson HUS26	
w/ (6) 16d, 0.162"x2.5" nails in Truss	
w/ (14) 16d, 0.162"x2.5" nails in Girder	
Girder is (2)2x6 min. So.Pine	
FL/-/4/-/-/R/-	Scale = .25"/ft.
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"
JREF	15XA487_202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

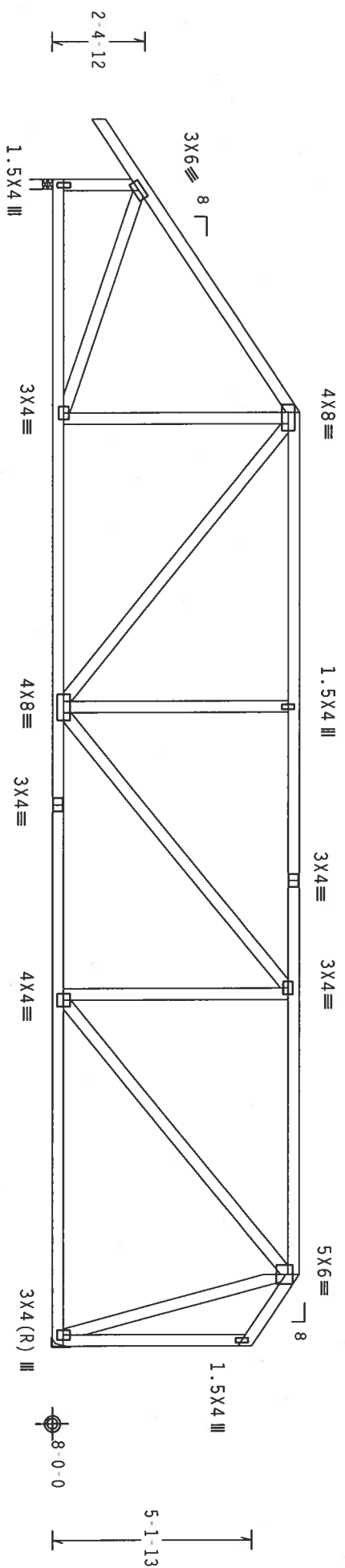
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



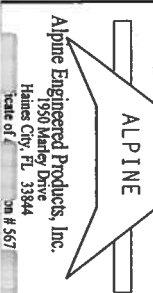
R-1368 U-180 W-3"
29-11-8 Over 2 Supports
1-10-0

PLT TYP. Wave

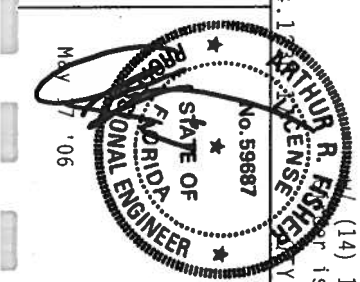
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 503 DOWNTOWN DR., SUITE 200, FARMINGTON, CT 06031 AND PER A GOOD TRUSS COMPANY OF AMERICA, 6500 LEBANON RD., FARMINGTON, CT 06031. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AREA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H./S) ASTM A653 GRADE 40/60 (W. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1950 Main St.
Haines City, FL 33844
On # 567



TC LL	20.0 PSF	REF R487-- 438
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUR487 06137016
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6769
DUR.FAC.	1.25	
COATING	24.0"	URFF-15XA487 202

Scale = .25"/ft.

R-1256 U-180 H-Simpson HUS26
W/ (4) 10d Common, 0.148"x3.0" nails in Truss
(14) 10d Common, 0.148"x3.0" nails in Girder
or is (2) 2x6 min. So. Pine

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240
criteria for brittle and flexible wall coverings.

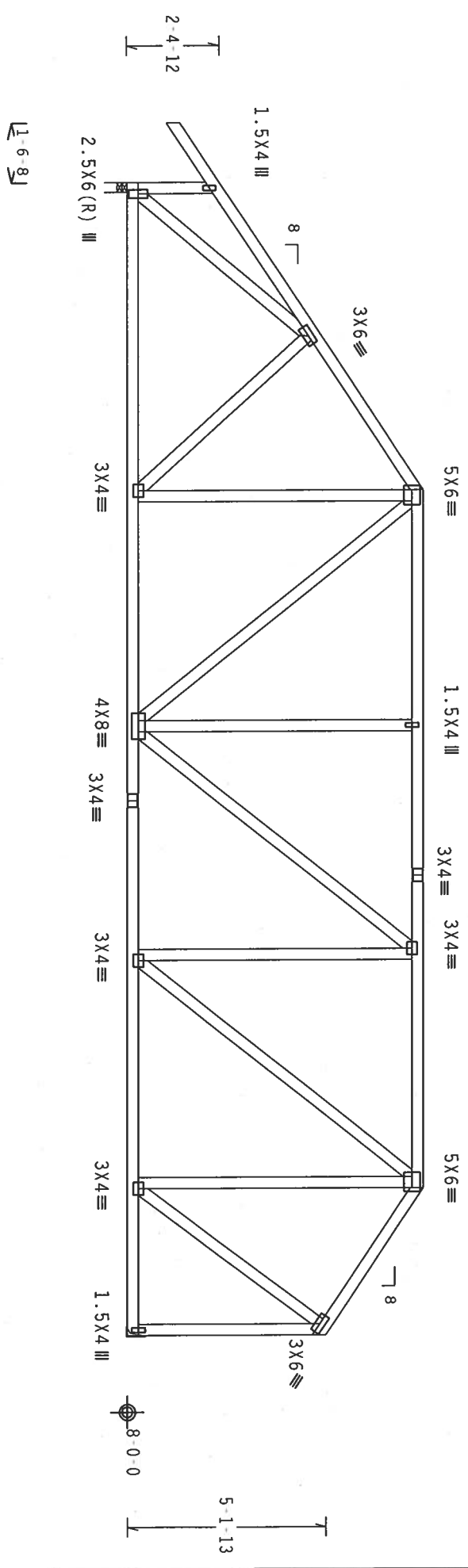
In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind
BC DL-5.0 psf.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and
calculations. Conditions may exist that require different connections
than indicated. Refer to manufacturer publication for additional
information.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.13



R-1256 U-180 H-Simpson HUS26	
w/ (4) 10d Common, 0.148"x3.0" nails in Truss	
(14) 10d Common, 0.148"x3.0" nails in Girder	
So. Pine	
FL/-14/-1/-R/-	Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DOWBORO RD., SUITE 200, MADISON, VT 05750) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 EXTENSIVE LN., SUITE 100, FARMINGTON, CT 06031) FOR ADDITIONAL INFORMATION. THE TRUSS MANUFACTURER SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (NATIONAL DESIGN SPEC. BY AF&PA AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/X) ASTM A653 GRADE 40/60 (K, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.
1950 Highway 100
Haines City, FL 33844
Phone # 888-367-3671
Fax # 888-367-3672

TC LL	20.0 PSF	REF R487-- 439
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUR487 06137003
BC LL	0.0 PSF	HC-ENG JB/AF
TOT. LD.	40.0 PSF	SEGN- 6777
DUR. FAC.	1.25	
SPACING	24.0"	JREC- 1SX487.202

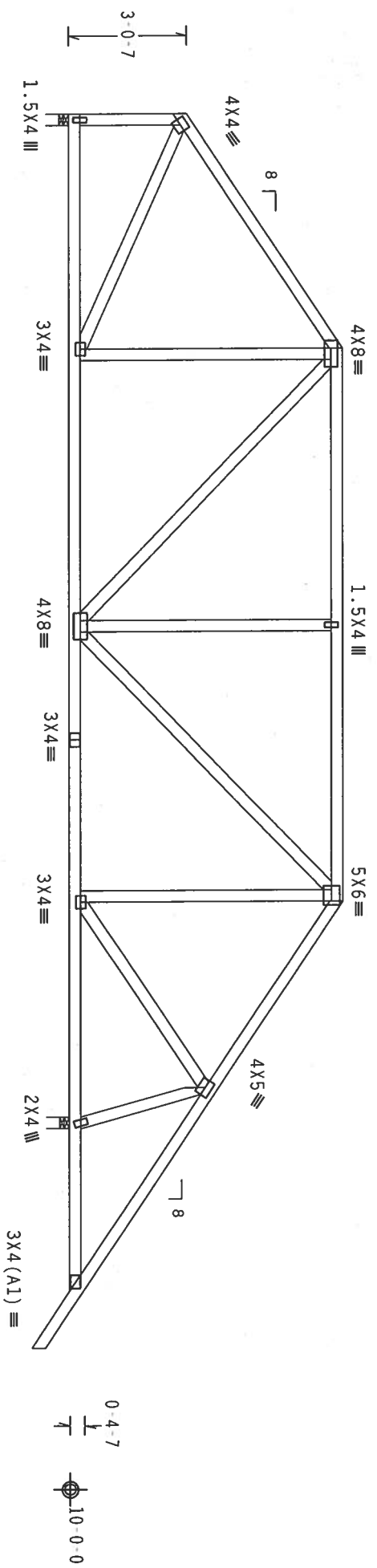
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

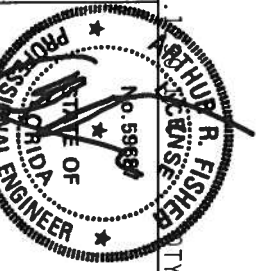


R=1036 U=180 W=3.5"
R=1602 U=180 W=3.5"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPT (TRUSS PLATE INSTITUTE, 583 DUNDAS ST. E., SUITE 200, MISSISSAUGA, ONT. L4X 1L3) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., SUITE 100, FARMINGTON, CT 06031) FOR SAFETY PRACTICES. FAILURE TO FOLLOW THESE PRACTICES MAY RESULT IN PERSONAL INJURY OR DEATH. TRUSS MANUFACTURERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA AND TPI). ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/N/S/X) ASTM A653 GRADE 40/60 (K, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE

Alpine Engineered Products, Inc.
1930 Hatterly Drive
Haines City, FL 33844
Phone: 888-367-5671
Fax: 888-367-5672

FL	/	4	/	-	/	R	-	Scale = .25" / Ft.
TC LL								20.0 PSF
TC DL								10.0 PSF
BC DL								10.0 PSF
BC LL								0.0 PSF
TOT. LD.								40.0 PSF
DUR. FAC.								1.25
COATING								24.0"
DATE								05/17/06
DRW								HCSR487 06137007
HC-ENG								JB/AF
SECN								6785

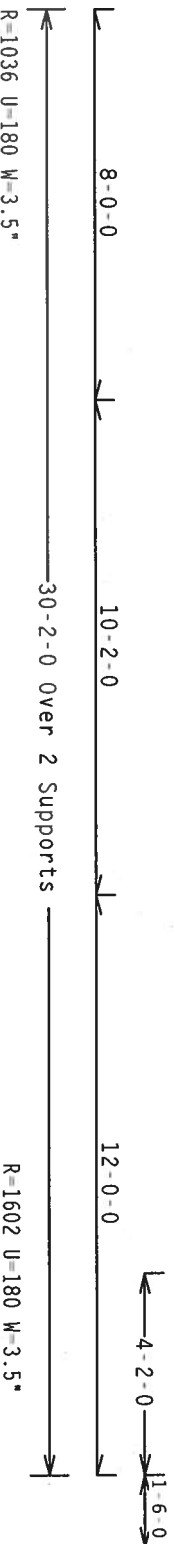
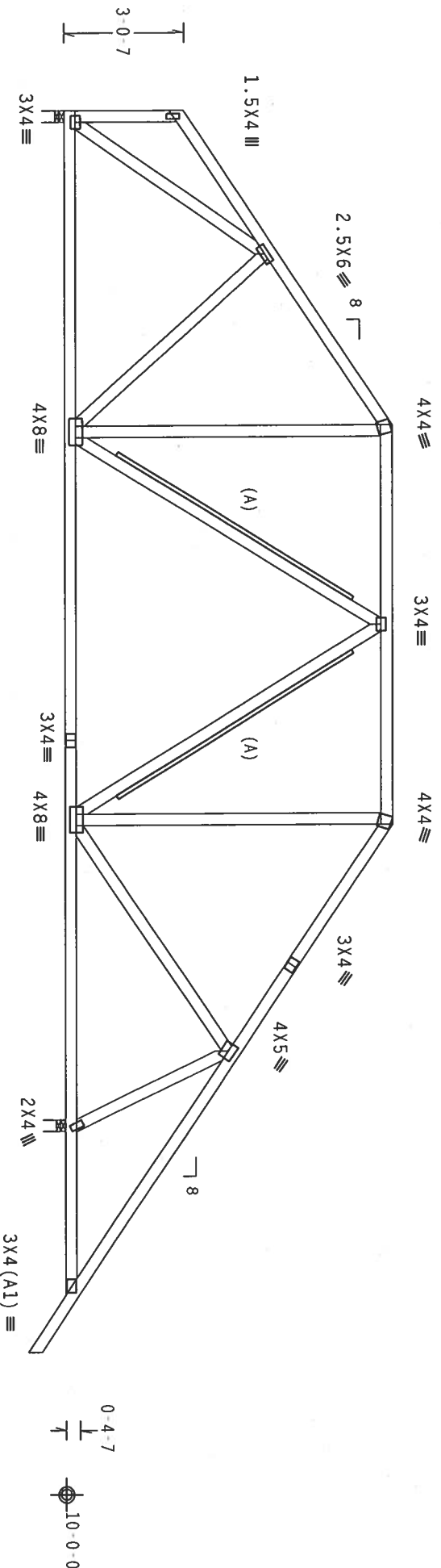
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with
8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

STY:1 FL/-/4/-/R/-

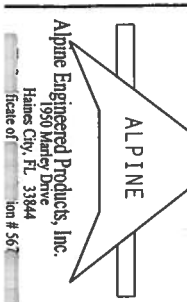
Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 583 D'AMORE DR., SUITE 200, MADISON, WI 53719, AND WCA (WOOD TRUSS CONSTRUCTION) AND WCA (WOOD TRUSS CONSTRUCTION) FOR MORE INFORMATION. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (4/H/S/X) ASTM A653 GRADE 40/60 (K. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF R487-- 441
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137006
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEON- 6796
DUR.FAC.	1.25	
SPACING	24.0"	

JRCF- 15X487 202



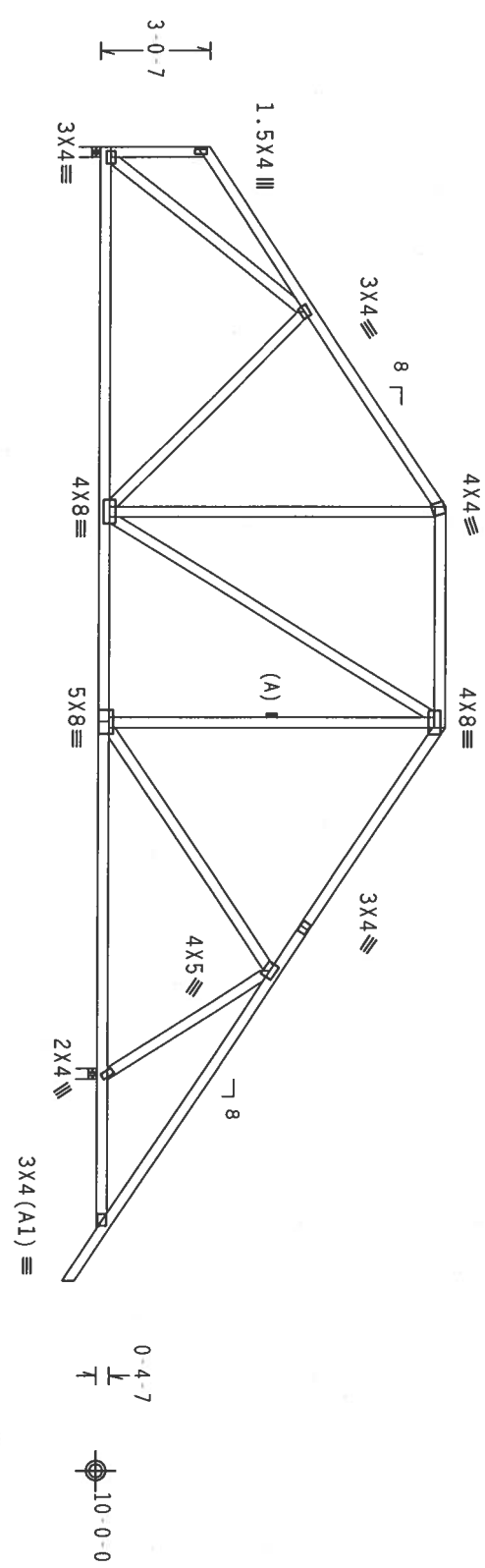
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



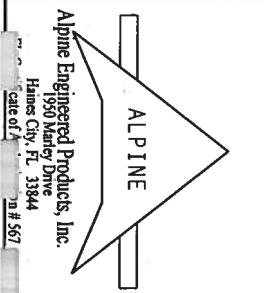
10-0-0 6-2-0 14-0-0
1-6-0
4-2-0
R-1036 U=180 W=3.5"
30-2-0 Over 2 Supports
R-1602 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

FL/-/4/-/R/-

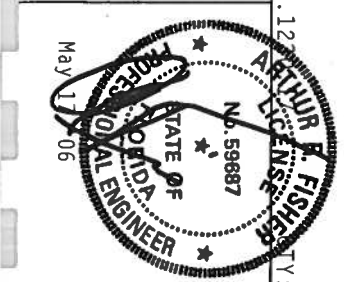
Scale = .1875"/ft.



ALPINE
Engineered Products, Inc.
1990 Marley Drive
Haines City, FL 33844
Phone # 567

WARNING TRUSSES REQUIRE EXISTING MADE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 FOR BUILDING CODES. THIS TRUSS IS DESIGNED FOR A 150 PSF DEAD LOAD AND A 150 PSF LIVE LOAD. THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY NDS) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W/H/S/K) ASTM A653 GRADE 40/50 (W/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNEX 1 SEC. 2.



TC LL	20.0 PSF	REF	R487-- 442
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCSR487 06137005
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT. LD.	40.0 PSF	SEQN	6806
DUR. FAC.	1.25		
SPACING	24.0"		

JRFF-15X4187 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

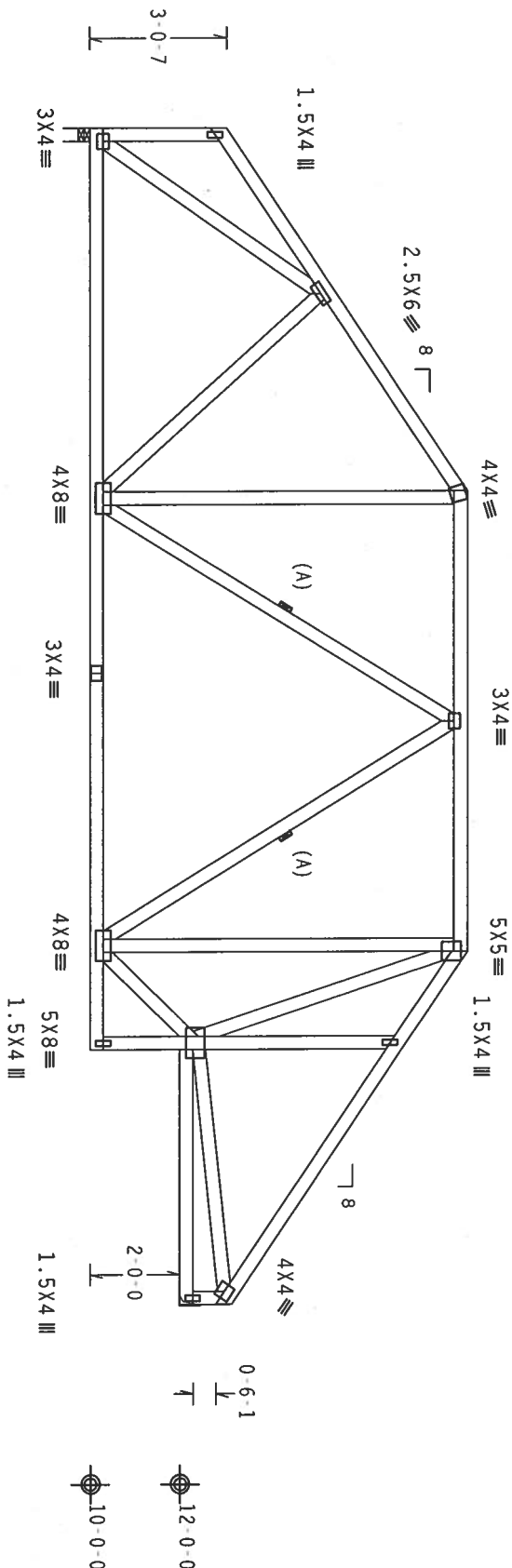
(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24

TY.5 FL/-14/-1-R/-

Scale = .25"/ft.

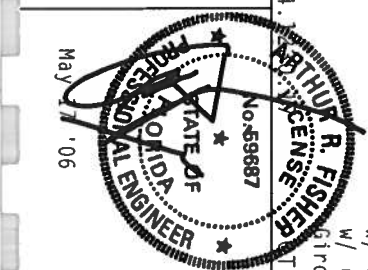
REF R487-- 443
DATE 05/17/06
DRW HCUSR487 06137029
HC-ENG JB/AF
SEQN- 6827

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2831 W. 10TH AVE., SUITE 100, DENVER, CO 80202, FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S) ASTM A653 GRADE 40/60 (W. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1950 Motor Drive
Haines City, FL 33844
Tel: 888-255-2557
Fax: 888-255-2557



TC LL	20.0 PSF	REF R487-- 443
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137029
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6827
DUR.FAC.	1.25	
CDACING	24.0"	JREF- 15X4487 202

Case of *...* on # 567

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240
Criteria for brittle and flexible wall coverings.

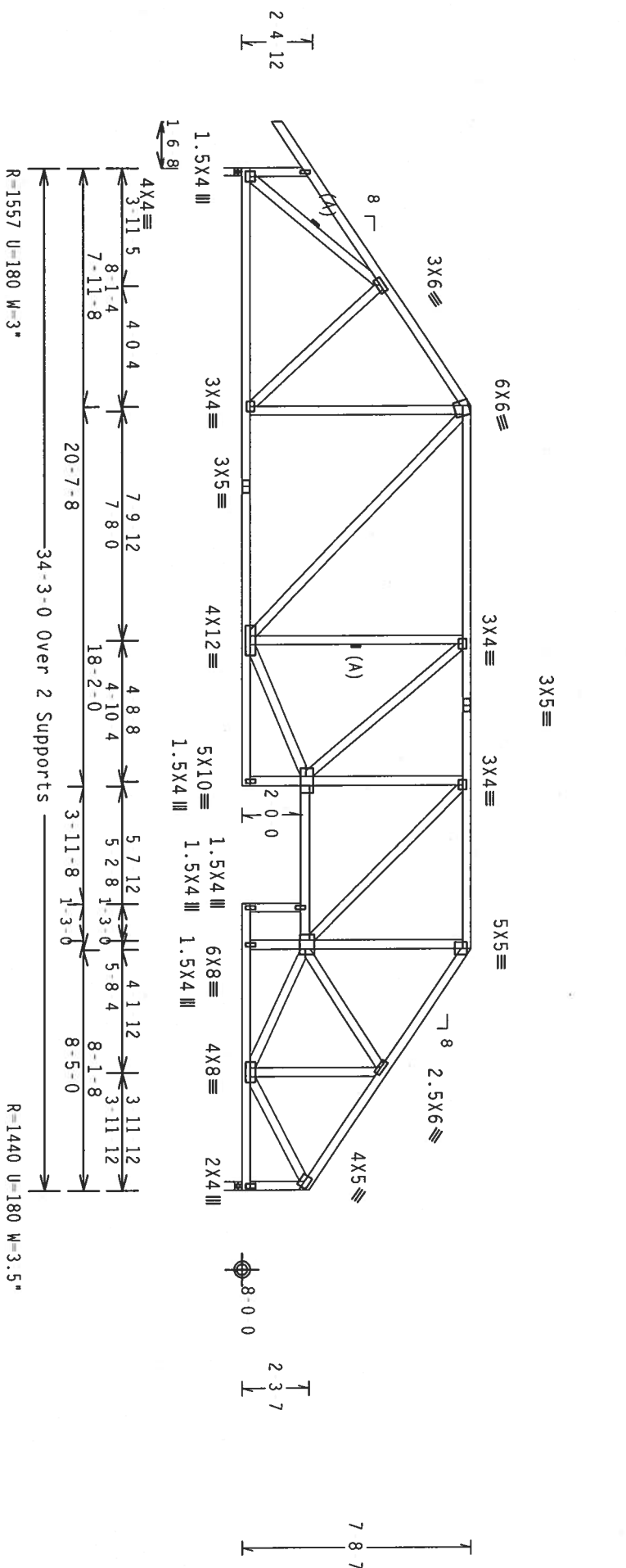
Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.

LATERALLY BRACE BOTTOM CHORD ABOVE FILLER AT 24" O.C.,
INCLUDING A LATERAL BRACE AT CHORD ENDS.

110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL 5.0 psf, wind
BC DL 5.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

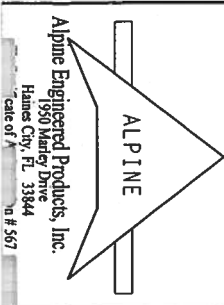
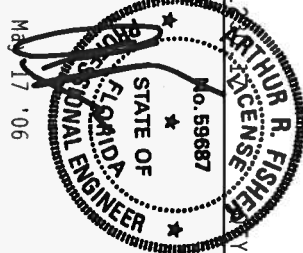
7.24.1

Scale = .1875"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO ACES 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 MONROE DR., SUITE 200, HOUSTON, TEXAS 77059) AND PERFORMING THESE FUNCTIONS. ALL TRUSSES MUST BE INSTALLED TO THE DESIGNER'S SPECIFICATIONS. THE SUBMITTAL OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/M/S) ASTM A653 GRADE 40/50 (W. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487-- 446
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137012
BC LL	0.0 PSF	HC-ENG	DF/AF
TOT.LD.	40.0 PSF	SEQN-	104072
DUR.FAC.	1.25		
COATING	24.0"	JRFF-	15X4497 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240
Criteria for brittle and flexible wall coverings.

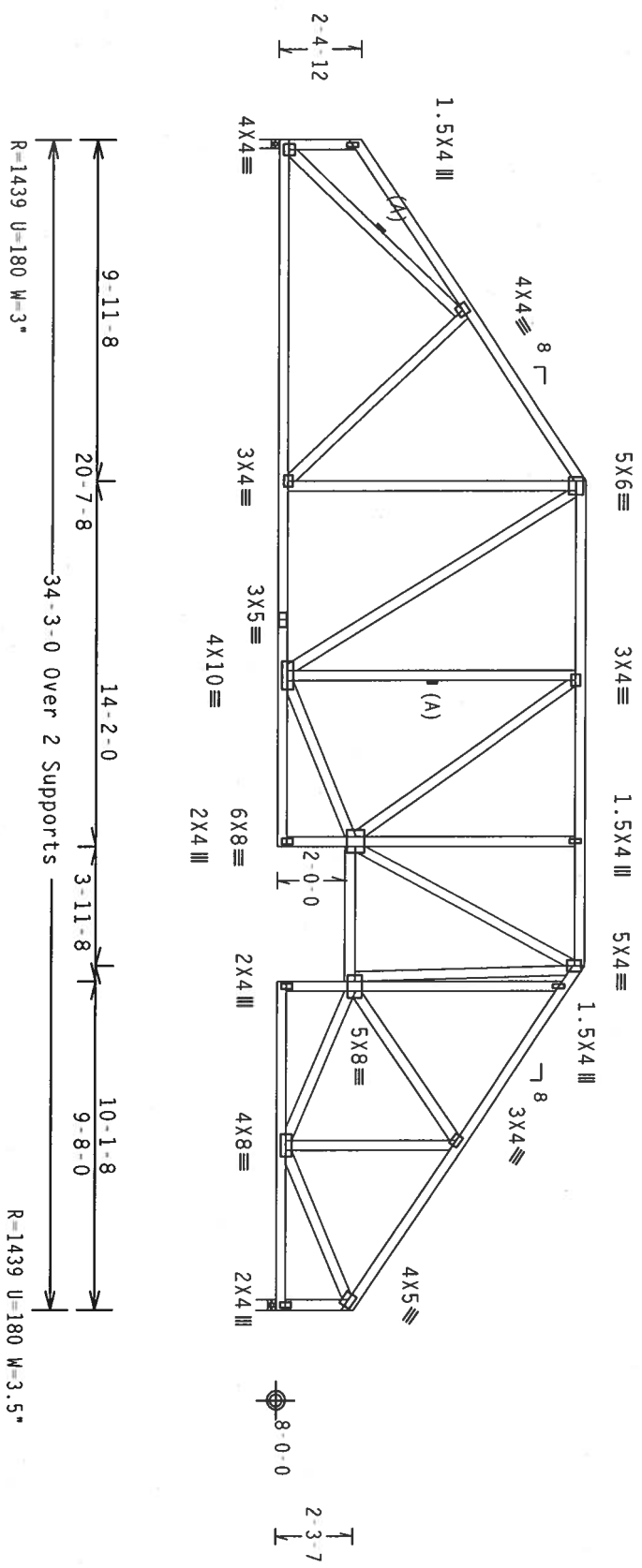
In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

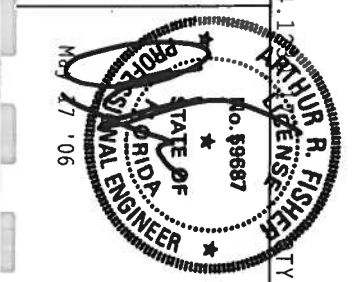
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSSES ARE TO BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. THE USER SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION AND BRACING OF THE TRUSSES. THE USER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W/H/S) ASTM A653 GRADE 40/60 (W, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487-- 447
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137013
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6902
DUR.FAC.	1.25		
SPACING	24.0"	URFF-	15XA487 202

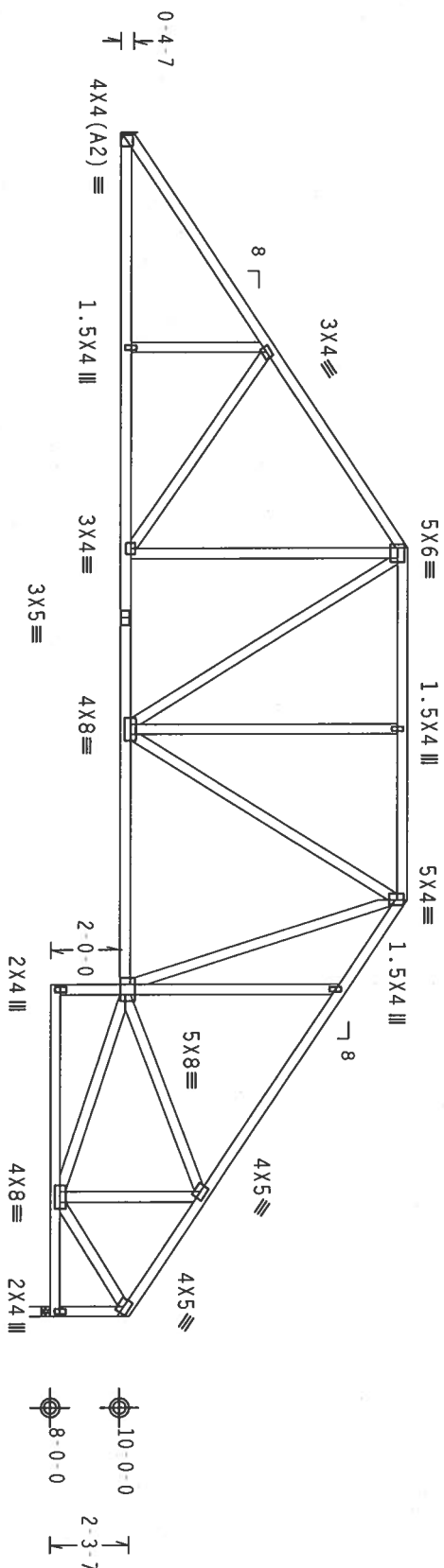
Scale = .1875"/Ft.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT I, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace
 24" OC BC @ 24" OC

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1446 U=180 H=Simpson HUS26
W/ (4) 10d Common, 0.148"x3.0" nails in Truss
W/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2)2X6 min. So.Pine

R=1436 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/10(0)$$

PROPERTY: 1

FL/4/4/R/

Scale = .1875"/Ft.

*"MAINLINE" *TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO SCG1-1 TO (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 580 S. OXFORD DR., SUITE 200, MADISON, WI 53715) AND WCA (WOOD RINGS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES RELATIVE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED LIGID CEILING.

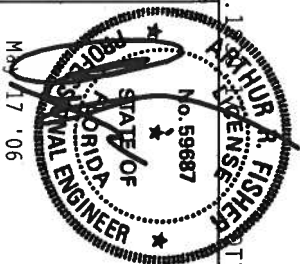
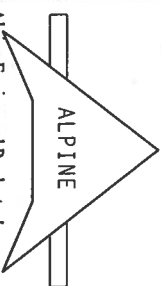
****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR**

PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ALPINE

Alpine Engineered Products, Inc.

Haines City, FL 33844

icate of _____ on # 567



TC LL	20.0 PSF	REF	R487 - 448
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137009
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6882
DUR.FAC.	1.25		
SPACING	24.0"	JRFF-15XA187	Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

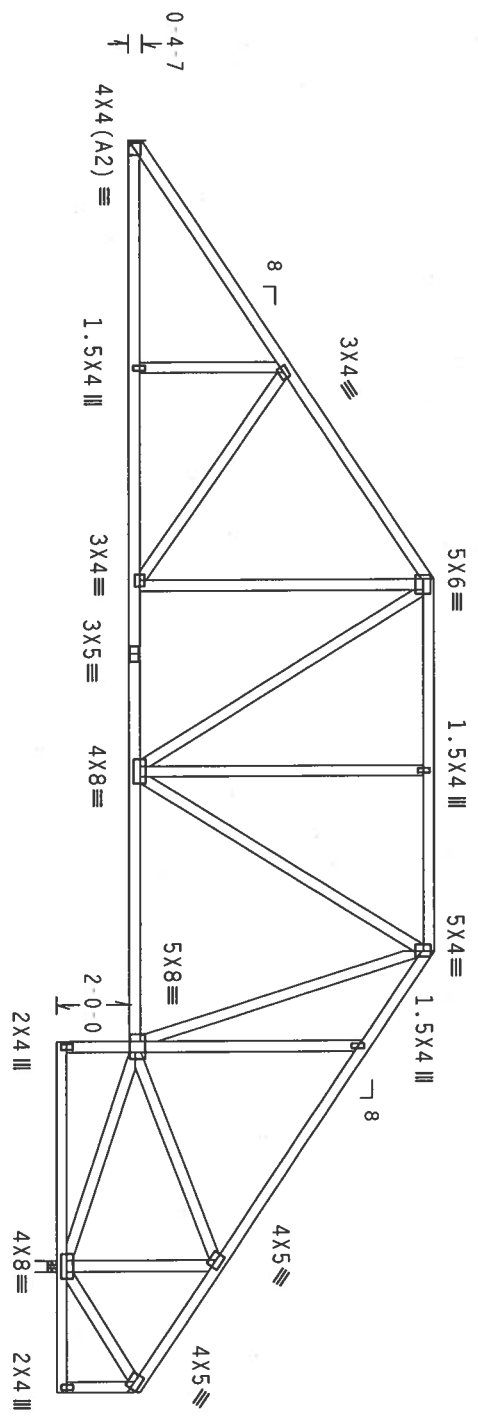
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



R-1283 U-180 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.13

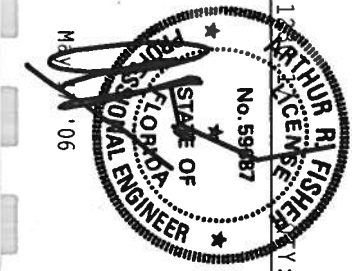
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 583 DORRISTON DR., SUITE 200, MADISON, WI 53713, AND WPCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 CENTERVILLE LN., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMANCE WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/X) ASTM A653 GRADE 40/60 (K. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.
1500 Marley Drive
Haines City, FL 33844

Scale of 1/8" = 1'-0"



TC LL	20.0 PSF	REF R487-- 449
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUR487 06137010
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEON- 6887
DUR.FAC.	1.25	
BRACING	24.0"	JREF- 1SX487 202

Scale = .1875"/ft.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

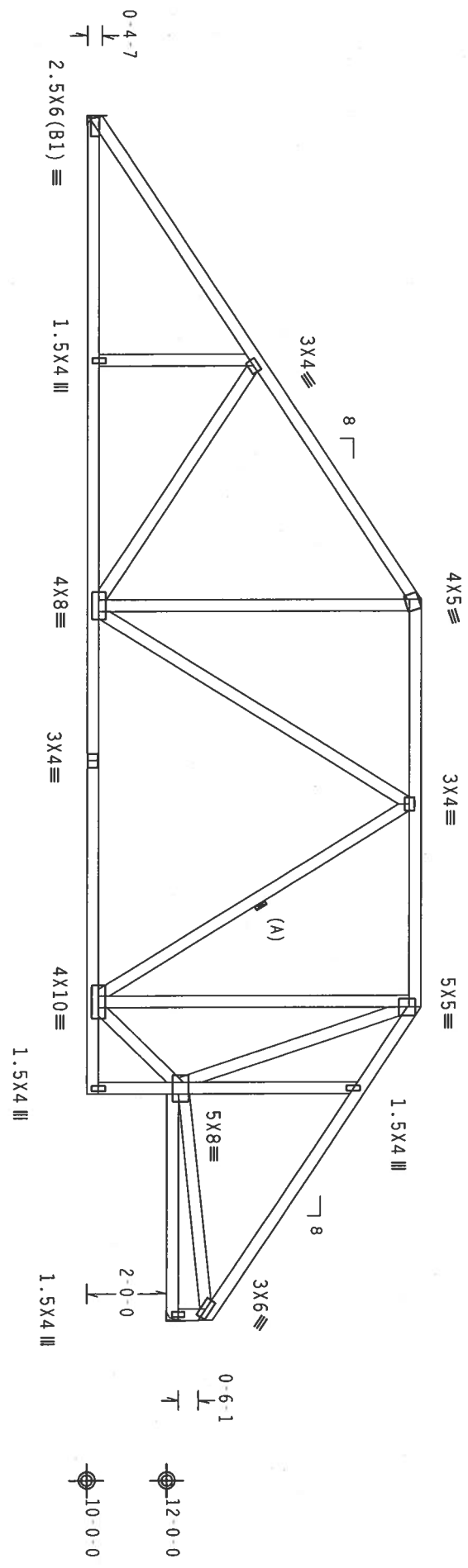
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace, 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5" min.) nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



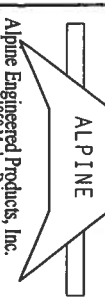
R-1260 U-180 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in girder
Girder is (2)2X6 min. So.Pine

R-1249 U-180 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in girder
Girder is (2)2X6 min. So.Pine

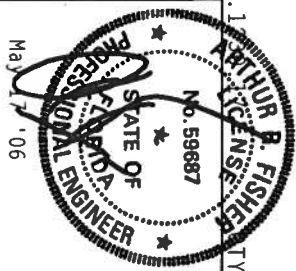
PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE). SEE INSTRUCTIONS FOR PROPER BRACING AND RIGID CEILING. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PAI AND TPI).



Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Scale of 1/8" = 1'-0"



TC LL	20.0 PSF	REF R487-- 450
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137028
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6840
DUR.FAC.	1.25	
SPACING	24.0"	JREF-1SXA487 202

Scale = .25"/ft.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
: Stack Chord SC1 2x4 SP #2 Dense:
: Stack Chord SC2 2x4 SP #2 Dense:

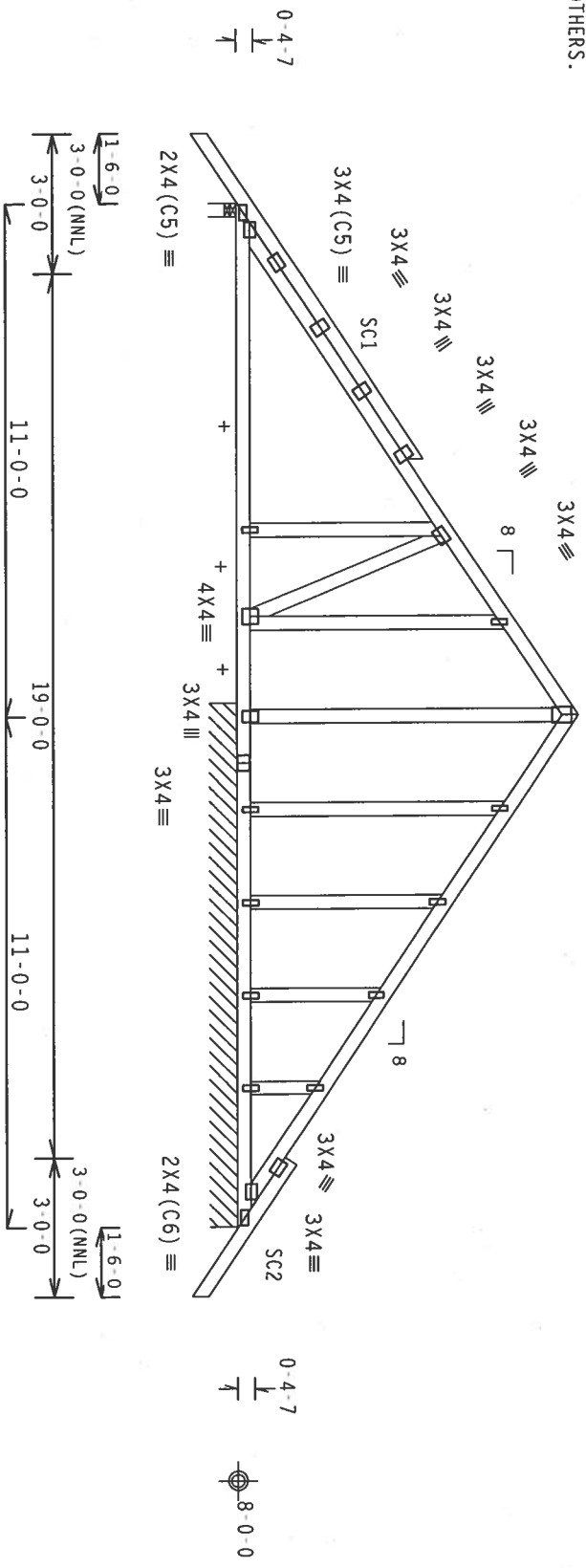
SPECIAL LOADS

LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25	
TC - From	64 PLF at -1.50 to 64 PLF at 5.50
TC - From	106 PLF at 5.50 to 106 PLF at 11.00
TC - From	106 PLF at 11.00 to 106 PLF at 23.50
BC - From	5 PLF at -1.50 to 5 PLF at 0.00
BC - From	20 PLF at 0.00 to 20 PLF at 22.00
BC - From	5 PLF at 22.00 to 5 PLF at 23.50

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

+ MEMBER TO BE Laterally Braced For Horizontal Wind Loads.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED Bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
See DWS A11015EE0405 & GBLLETIN0405 for more requirements.
Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



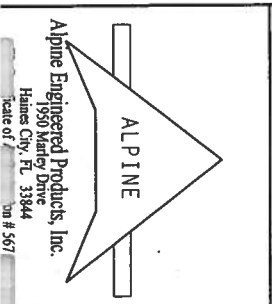
R-666 U-180 W-3.5"
R-245 PLF U-22 PLF W-11-3-8

Note: All plates are 1.5x4 except as shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24

ARTHUR R. FISHER
Professional Engineer
No. 69687
May 11 '06

Scale = .25"/ft.

FL	-/4	-/R	-/R
TC LL	20.0 PSF	REF R487 -	451
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW HCUSR487	06137008
BC LL	0.0 PSF	HC-ENG UB/AF	
TOT.LD.	40.0 PSF	SEON-	6993 REV



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 585 D'ONOFIO DR., SUITE 200, MODISON, IL 60450 AND KICK-BOOTS THESE CONDITIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/H/S/K) ASTM A653 GRADE 40/60 (K. K/H/S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

COATING	DUR.FAC.	1.25	24.0"
JRF-15X487	202		

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Scale = .25" / ft.

N. 59687

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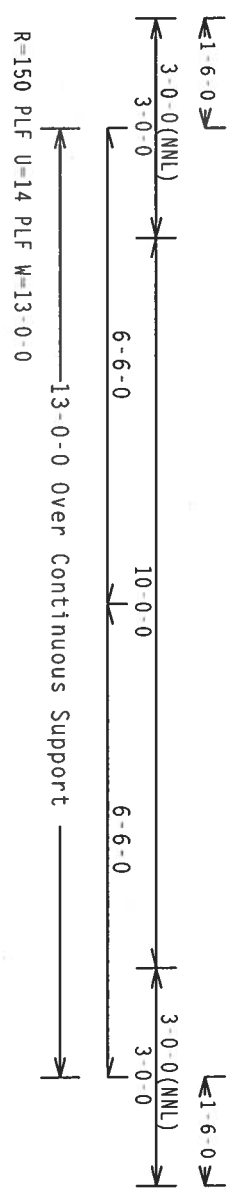
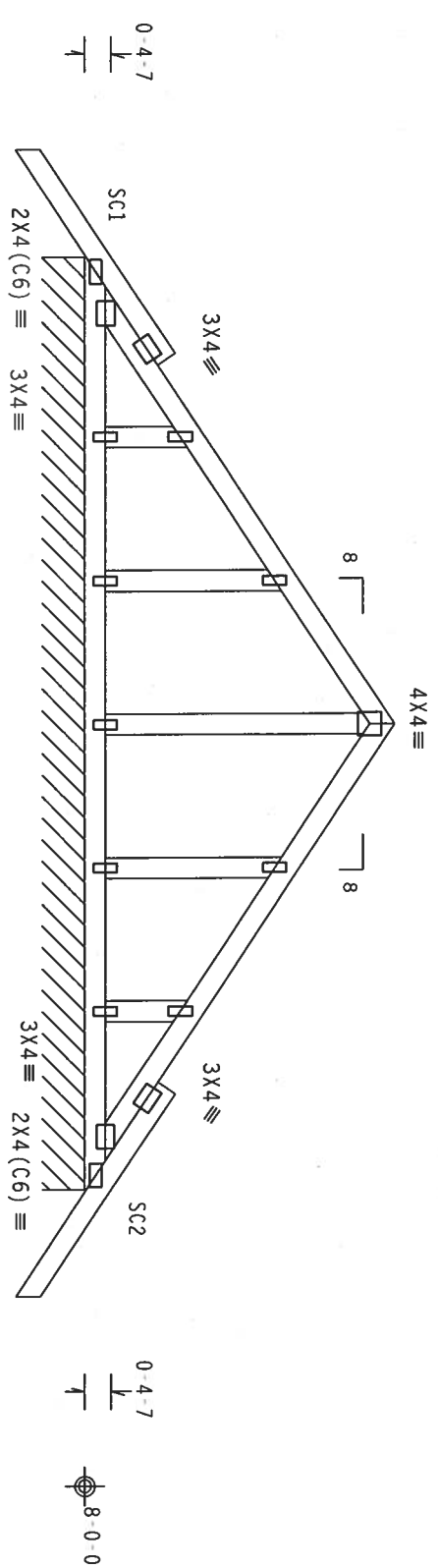
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

See DWGS A11015EE0405 & 6BLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

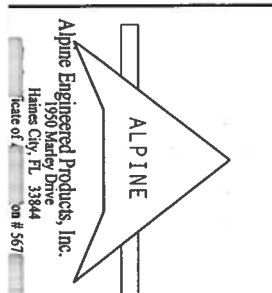
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



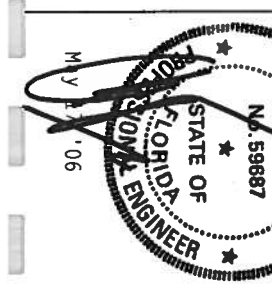
Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25/10(0) 7.24.1

Scale = .375"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 1563 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., DUNDAS ST. W., UNIT 10, MISSISSAUGA, ONTARIO L4X 1L3) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERING PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

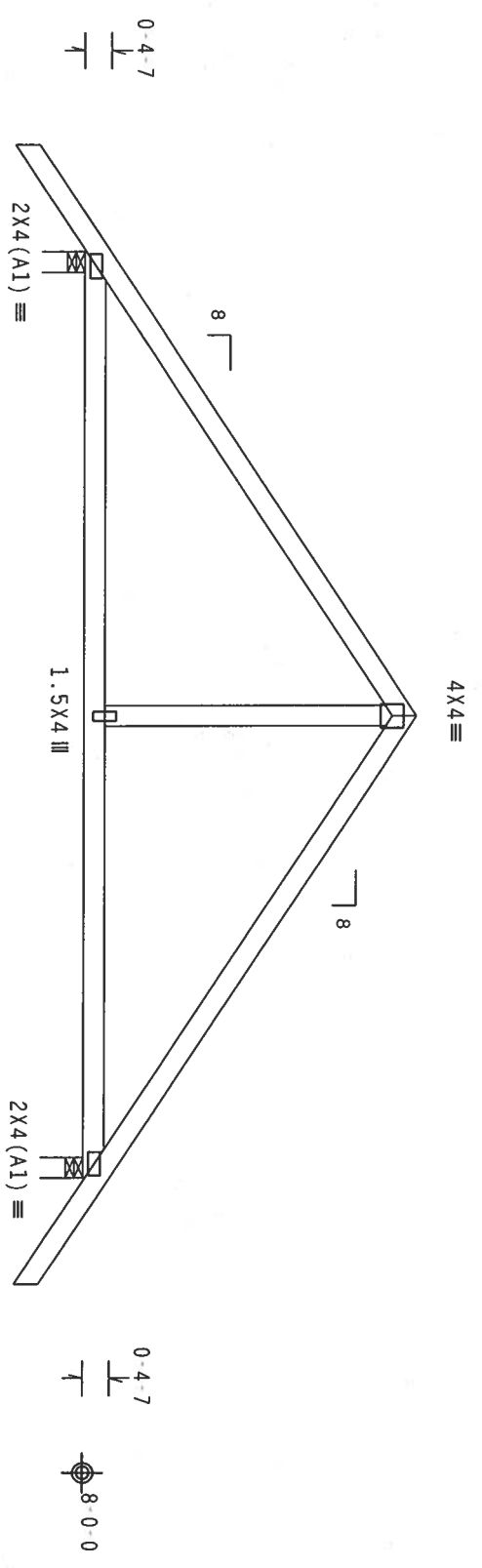


TC LL	20.0 PSF	REF R487-- 453
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUR487 06137036
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEON- 6743 REV
DUR.FAC.	1.25	
SPACING	24.0"	JRFF- 1SX487 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)



FL/-/4/-/1-/R/-

Scale = .375"/ft.

ALPINE

Alpine Engineering Products, Inc.
1950 Hardy Drive
Haines City, FL 33844
Tel # 567

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DENVER ST., SUITE 100, DENVER, CO 80202) FOR ADDITIONAL INFORMATION. TRUSS CONNECTOR OF AMERICA, 6500 LEXINGTON LN., MADISON, AL 37129, FOR SAFETY PRACTICES. THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

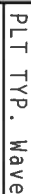
****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/S/K) ASTM A653 GRADE 40/50 (W, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DATE	17 '06
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TC LL	20.0 PSF	REF	R487 - 454
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137038
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN	6739
DUR.FAC.	1.25		
SPACING	24.0"	JRCC	15XAA07 202

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.


$$Cq/RT=1.00(1.25)/10(0)$$

7.24.

PERMITS

2 FL/-/4/-/-/R/.

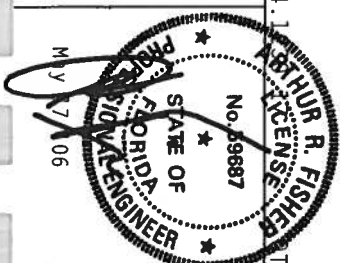
Scale = .375"/Ft.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR**

PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PCA) AND TPI. ALTHOUGH CONNECTIONS MUST BE MADE IN ACCORDANCE WITH THE AIA/PCA DESIGN SPECIFICATION, THE COMPANY SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE CONNECTIONS.

CONNECTION PLATES MADE OF 2018/1870 (M, H/S/R) ASIN A553 GRADE 40/60 (M, K/H/S) GALV. DRILLING, BOLA 2 PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEE THE THREE COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



SPACING 24.0"

JREF 1SXA 07 202

2 COMPLETE TRUSSES REQUIRED
Nailing Schedule: (12d Box or Gun 0.128"x3).

Nailing Schedule: (12d_Box_or_Gun_(0.128"x3.25",_min.)_nails)

Top Chord:	1 Row	@ 12.00"	0. c.
Bot Chord:	1 Row	@ 3.25"	0. c.

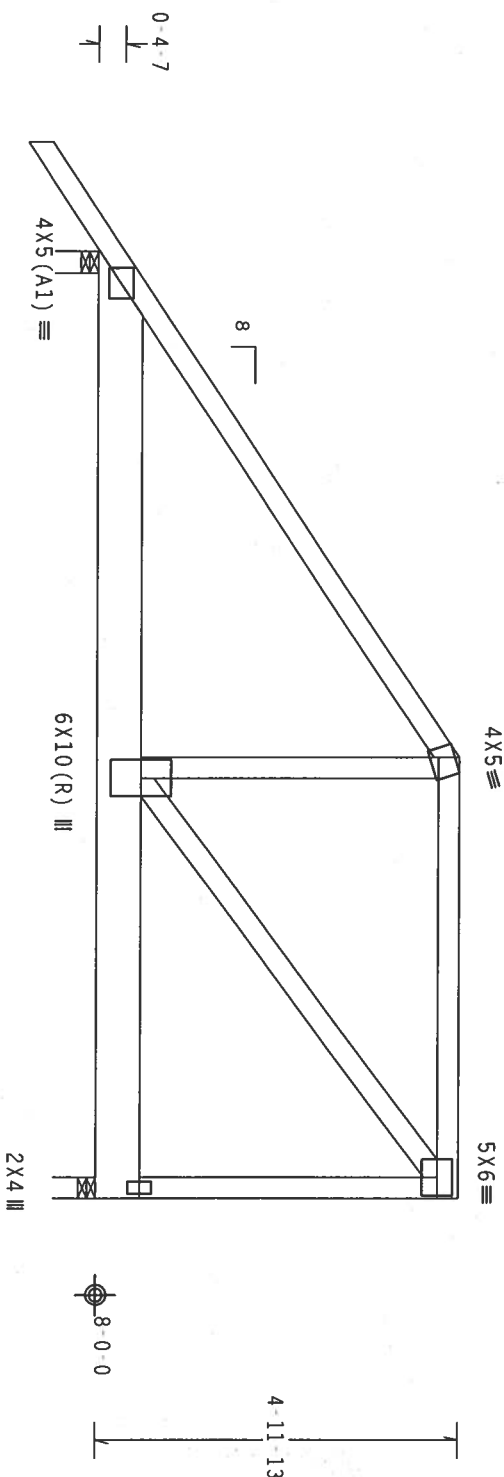
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

	(1) LUMBER DUR. FAC. = 1.25	(2) PLATE DUR. FAC. = 1.25
TC From	64 PLF at 1.50 to	64 PLF at 13.00
BC From	5 PLF at 1.50 to	5 PLF at 0.00
BC From	20 PLF at 0.00 to	20 PLF at 13.00
BC 3258 LB Conc.	Load at 7.06	
BC 1256 LB Conc.	Load at 9.06, 11.06	

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/10(0)$$

CONFIDENTIAL

FL/4/R/

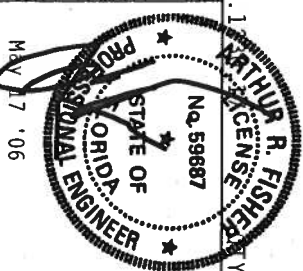
Scale = .375" / Ft.

*HAINING** TRUSSES REQUIRE EXPERT CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RC61-1-3 (BUILDING COMPONENT SPECIFIC INFORMATION), PUBLISHED BY TPI (TRUSS PRACTICE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53718) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE IN MADISON, WI 53719) FOR SPECIFIC PRACTICES RELATIVE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED LIGID CEILING.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR**

ALPINE
Alpine Engineered Products, Inc.

Haines City, FL 33844
 Telephone # 567



TC LL	20.0 PSF	REF	R487 - - 456
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137002
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6861
DUR.FAC.	1.25		
SPACING	24.0"	JRFF-	1SXA487 202

JREF - 1SXA127 202

TOP chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

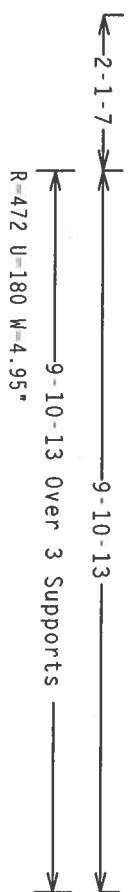
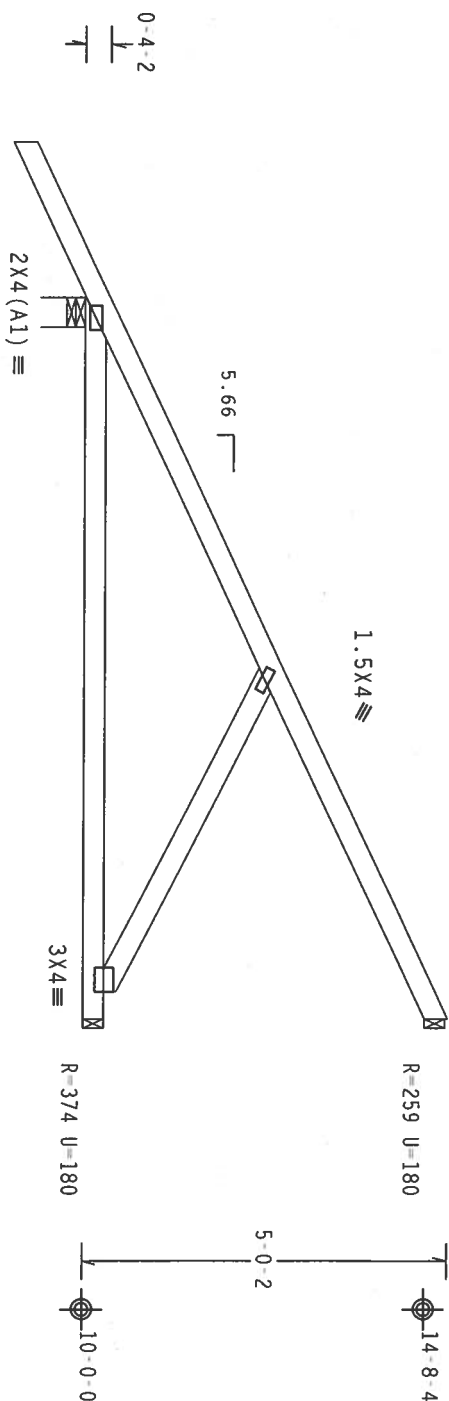
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7'-0" setback jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



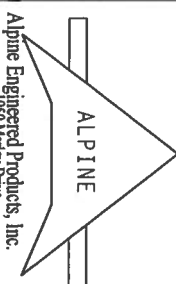
PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

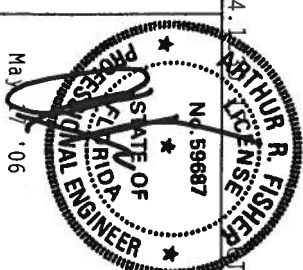
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 103 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 563 MADISON, WI 53719 FOR SAFETY INFORMATION. TRUSS CHORDS OF AMERICA, 6300 ENTERPRISE, E.M. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

DESIGN CONFORMS WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (W/ K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
Haines City, FL 33844
D# 567



FL	/4	/1	/R	Scale = .375"/Ft.
TC LL	20.0	PSF	REF R487 - 457	
TC DL	10.0	PSF	DATE 05/17/06	
BC DL	10.0	PSF	DRW HCUR487 06137033	
BC LL	0.0	PSF	HC-ENG JB/AF	
TOT.LD.	40.0	PSF	SEQN - 6718	
DUR.FAC.	1.25			

SPACING 24.0" JRF-15X4x87-202

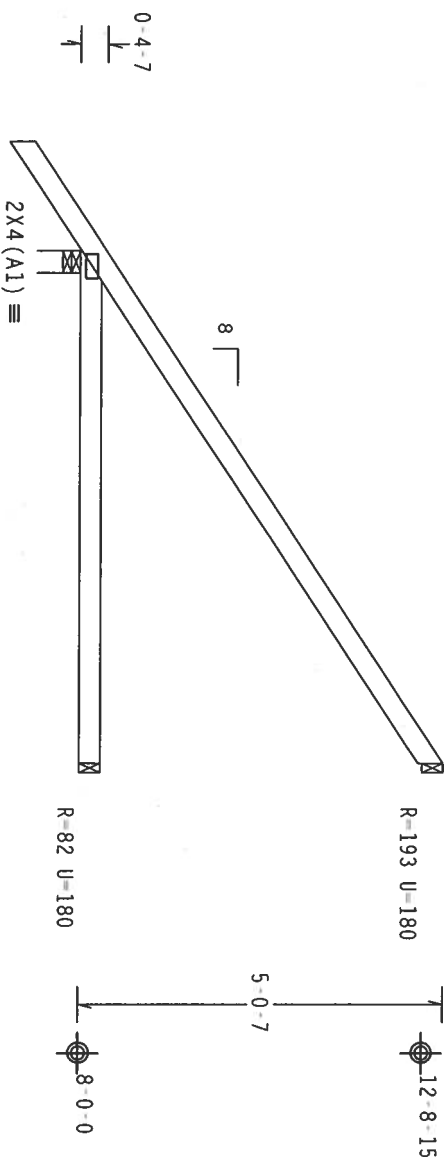
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC. BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



≤1-6-0

7-0-0 Over 3 Supports
R-417 U-180 W-3.5"

PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.1

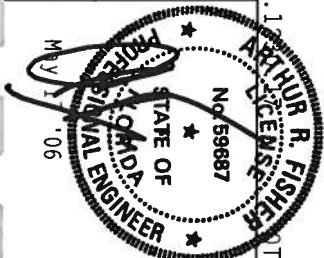
Scale = .375"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
PROVIDE 16D COMMON NAILS TO SECURE TRUSSES TO ROOF DECK OR WALLS. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGNER OR OTHER PARTY SHALL BE RESPONSIBLE. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. AND IS NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM. WITHOUT PERMISSION IN WRITING FROM ALPINE ENGINEERED PRODUCTS, INC. THIS DESIGN IS THE PROPERTY OF ALPINE ENGINEERED PRODUCTS, INC. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Date of: 20# 567



TC LL	20.0 PSF	REF	R487 - 458
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137019
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT. LD.	40.0 PSF	SEQN-	6615
DUR. FAC.	1.25		
SPACING	24.0"		

JREF-15XAA87-202

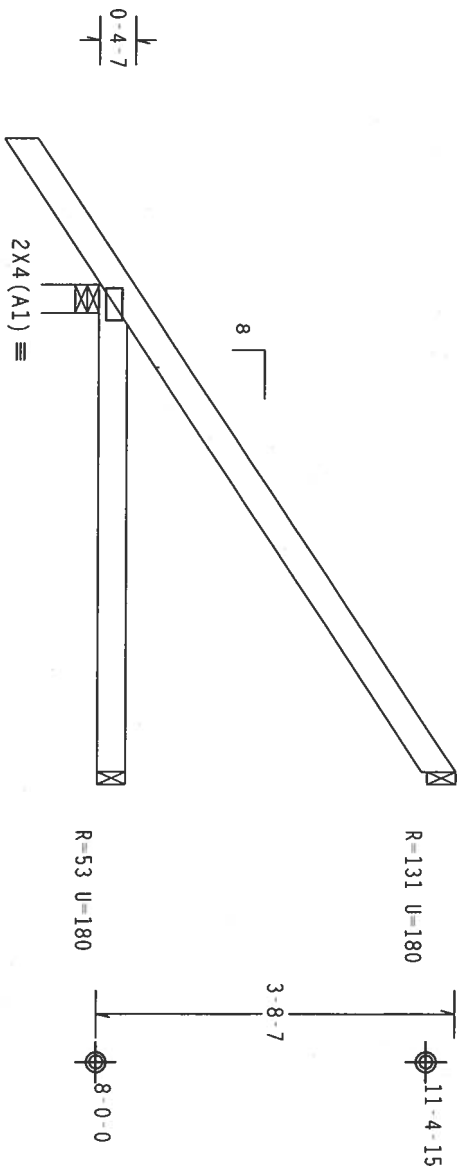
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.1

FL/-/4/-/R/-

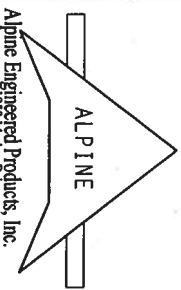
Scale = .5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCS1 1003 BUILDING CORP. SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 OR FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA 603 GRADE 40/60 (K, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

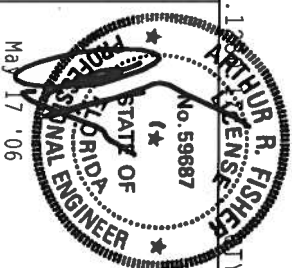
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1990 Marley Drive
Haines City, FL 33844

Date of: 5/17/06

Job # 557



TC LL	20.0 PSF	REF R487-- 459
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137018
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 6619
DUR.FAC.	1.25	
SPACING	24.0"	

JRFF-15XAM87 202

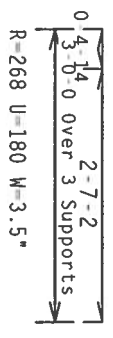
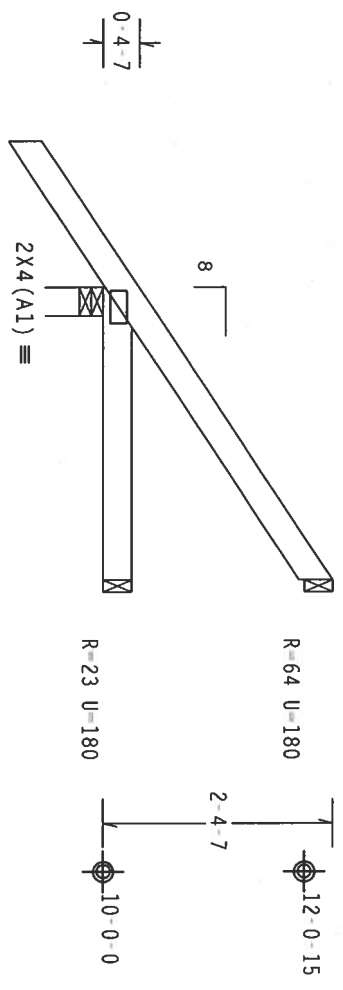
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

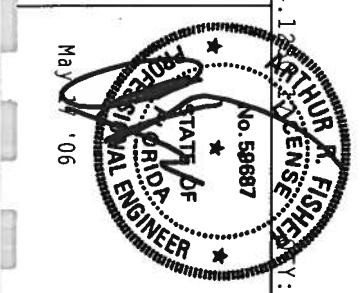
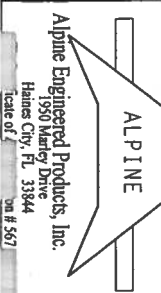
7.24.12

FL/-4/-/R/-

Scale =.5"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 568 MADISON AVE, SUITE 100, FARMINGTON, CT 06030) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/5/5) ASTM A653 GRADE 40/50 (W. K/H-5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R487-- 460
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137034
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 6624
DUR.FAC.	1.25	
SPACING	24.0"	JREF- JSXA487 Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

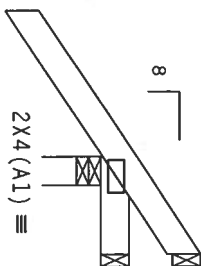
In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.

1
0-4-7



R=59 U=180
R=14 U=180

10-8-15
10-0-0

1-0-7

1-6-0

1-0-0 Over 3 Supports

R=261 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.1

QTY:2 FL/-/4/-/R/-

Scale =.5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST PRACTICES FOR TRUSS FABRICATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563
N. 10TH AVE., SUITE 200, CHICAGO, IL 60642, TEL: 773.321.1111, FAX: 773.321.1112, WWW.TPI-TRUSS.COM)
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
ALPINE ENGINEERED
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE
TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES,
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. ALPINE
CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/U/S) ASTM A653 GRADE 40/50 (W. K/H.S) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
BUILDING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

No.59687

STATE OF

FLORIDA

REGISTERED

ENGINEER

May 17, 2006

TC LL	20.0 PSF	REF R487-- 461
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137035
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6626
DUR.FAC.	1.25	
SPACING	24.0"	DRFF- 15XA487 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

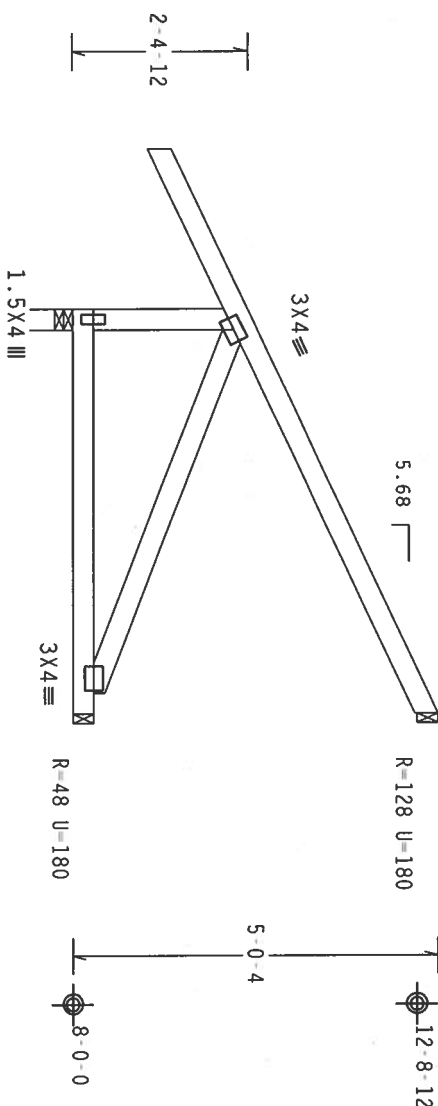
Hipjack supports 3-11-0 setback jacks with no webs.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



←2-2-3→
←5-6-7 Over 3 Supports →
R-221 U-180 W-3.493"

PLT TYP. Wave

Design Cr1t: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.1

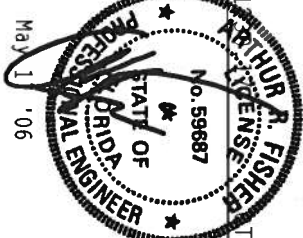
TY:1

FL/-4/-/-R/-

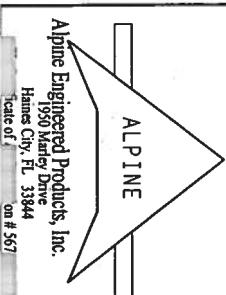
Scale = .375"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATING INSTITUTE, 3631 W. 10TH AVE., SUITE 100, DENVER, CO 80202, 303-733-1100) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S) ASTM A653 GRADE 40/60 (K, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



CD	ACING	24.0"	JRFE - 15XA487 202
DUR.	FAC.	1.25	
TOT.	LD.	40.0 PSF	SEON - 6734
BC	DL	10.0 PSF	DRW HCUSR487 06137017
BC	LL	0.0 PSF	HC-ENG JB/AF
TC	DL	10.0 PSF	DATE 05/17/06
TC	LL	20.0 PSF	REF R487 - 462



Alpine Engineered Products, Inc.
1950 W. 10th Ave.
Haines City, FL 33844
State of FL on # 567

In lieu of structural panels or rigid ceiling use purtins to brace TC @ 24" OC, BC @ 24" OC.

The following trusses need concentrated loads at the end of their overhangs: 2-9-0 span/setback member on the 1-6-0 cant side requires 28 lbs and the 2-9-0 span/setback member on the 1-6-0 cant side requires 28 lbs.

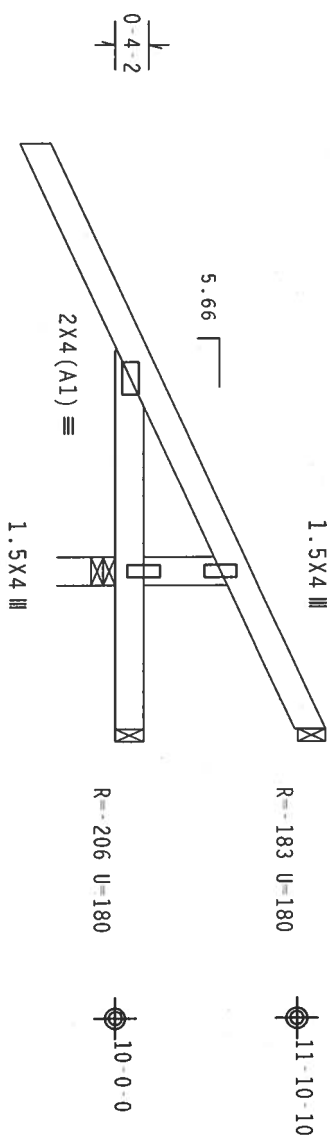
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Sub-fascia beam assumptions: 4-3-0 sub-fascia beam on the 1-6-0 cantilever side. 4-3-0 sub-fascia beam on the 1-6-0 cantilever side.

Hipjack supports 2-9-0 setback jacks with 1-6-0 cantilever one face; 1-6-0 cantilever opposite face.

Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



2-2-2

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

7.24.12

TY:1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

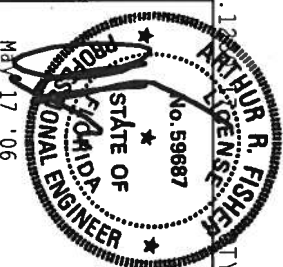
*WARNING: *ROSSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO RC61-1-3 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, 563 O'HEARNO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRECAUTIONS BEFORE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TIGHT CEILING.

*****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR**

TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE



TC LL	20.0 PSF	REF	R487-- 463
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCSR487 06137020
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6705
DUR.FAC.	1.25		
SPACING	24.0"	JRFF-	1SXA487_Z02

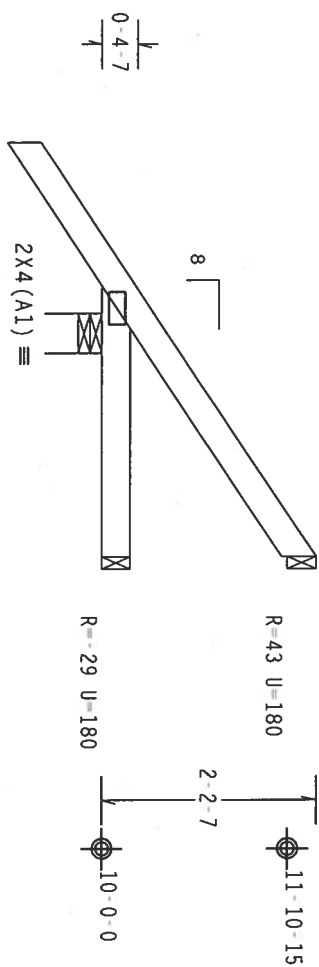
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at -1.50 to 64 PLF at 2.75
BC - From 5 PLF at -1.50 to 5 PLF at 0.00
TC - From 20 PLF at 0.00 to 20 PLF at 2.75
TC - 28 LB Conc. Load at -1.50

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



1-6-0 0-3-0
2-9-0 Over 3 Supports
R=348 U=180 W=4.95"

PLT TYP. Wave

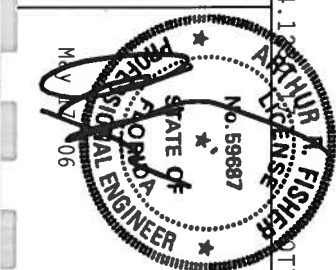
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

QTY: 1 FL/-/4/-/R/-

Scale = .5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE BUILDING COMPONENT SAFETY AND OPERATION, BUILT UNDER THE TPI (TRUSS PLATE) INSTALLATION, 363 DOWNSIDE, SUITE 200, 1517191 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S) ASTM A653 GRADE 40/60 (W. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



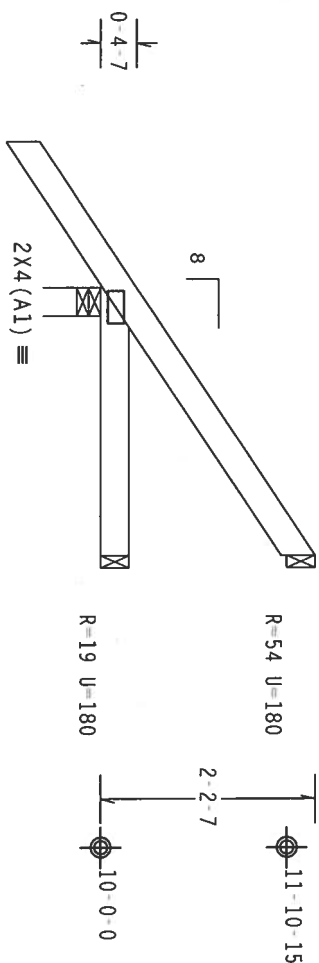
TC LL	20.0 PSF	REF R487-- 464
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137022
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6711
DUR.FAC.	1.25	
SPACING	24.0"	

JREF-15XAA87_202

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED blg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.



1-6-0

2-9-0
2-9-0 over 3 Supports
R=261 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/10(0)$$

7.24.12

PROPERTY: 4

FL/4/-/R/-

Scale = .5" / Ft.

WARNING FUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO GC51 1.0 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TROSS PLATE INSTITUTE), 563 D'ONOFIO DR., SUITE 200, MAISON, IL 57139, AND WICA (WOOD TROSS COUNCIL OF AMERICA), 6300 ENTERPRISE LN, MAISON, IL 57139, FOR SAFETY PRACTICES RELATIVE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CHORD CEILING.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.**

TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING

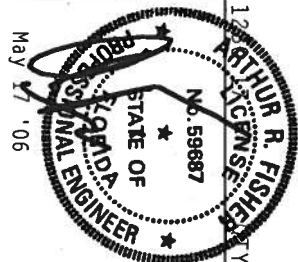
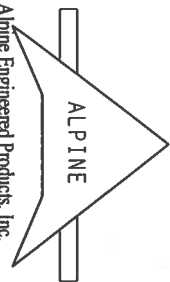
CONNECTOR PLATES ARE MADE OF 20/18/16GA (M.H/S/K) ASTM A653 GRADE 40/60 (M. K/H.S) GALV. STEEL. APPLY

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE ARCHITECT.

Alpine Engineered Products, Inc.

Scale of 1 to 5 on # 567



TC LL	20.0 PSF	REF	R487 - - 465
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 0613/024
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	6628
DUR.FAC.	1.25		
SPACING	24.0"	JRFF-	15XA487 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

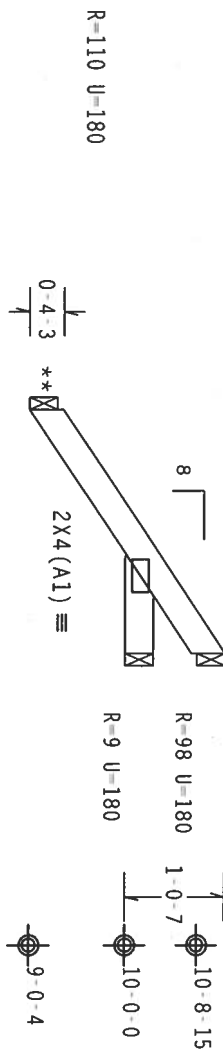
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

** PROVIDE CONNECTION FOR 180# UP OR DOWN AT FASCIA BEAM. FASCIA BEAMS AND THEIR CONNECTIONS TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

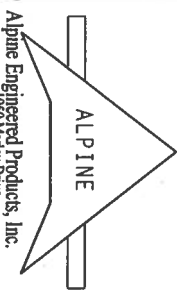
7.24.1

FL/-4/-/R/-

Scale =.5"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (TPI-2002(STD)/FBC) AND MICHAEL BAKER CORP. OF AMERICA, 6300 ENTERPRISE LN, MAISON VILLAGE, TX 77955 FOR SAFETY PRACTICES. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

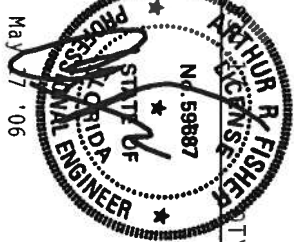
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002(STD)/FBC OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI-2002(STD)/FBC. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844

Date of: 7/10/06

PL # 567



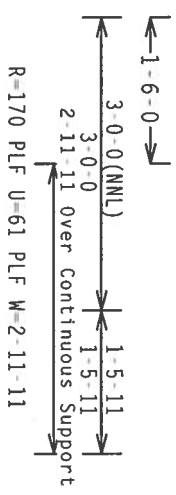
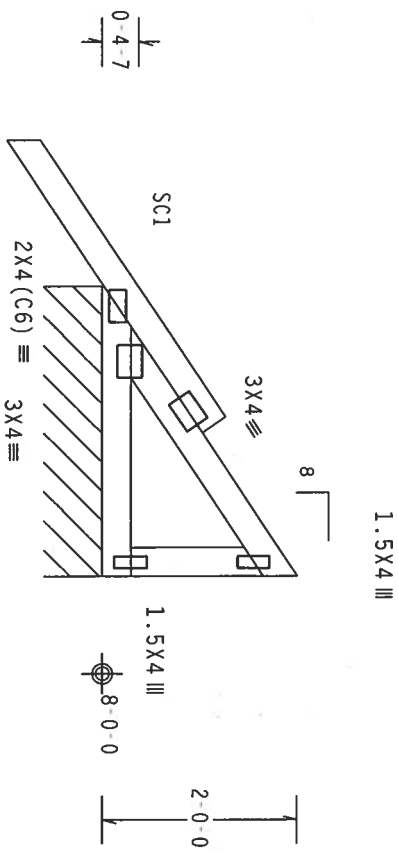
TC LL	20.0 PSF	REF	R487 - 466
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCSR487 06137021
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN	6686
DUR.FAC.	1.25		
SPACING	24.0"		

REF-15X487-202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #2 Dense:

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

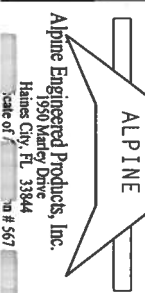


PLT TYP. Wave

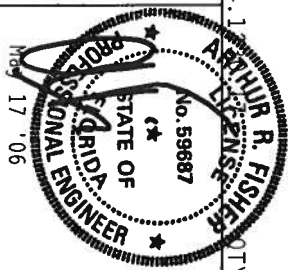
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51.103 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DUNSTON DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6500 LENTHERISE LN., DUBLIN, OH 43017) FOR ADDITIONAL INFORMATION. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P) AND TPI. ALPINE



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Right end vertical not exposed to wind pressure.
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



TC LL	20.0 PSF	REF R487-- 467
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137004
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6635 REV
DUR.FAC.	1.25	
SPACING	24.0"	URF- 15XA487 202

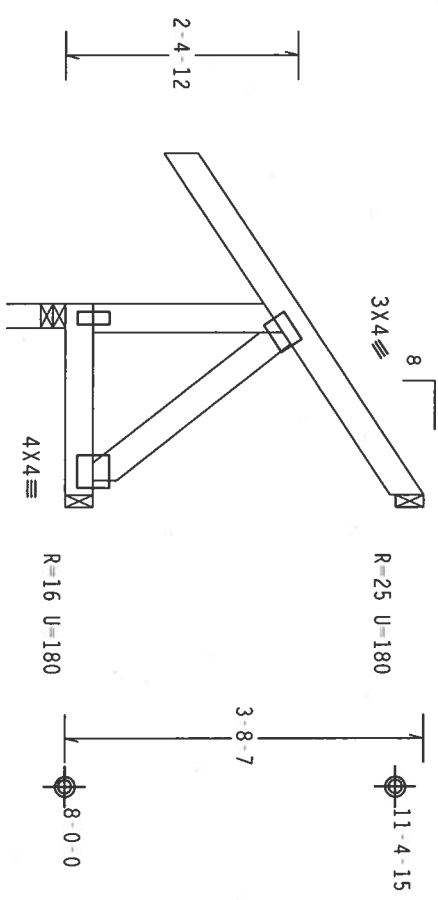
Scale = .5"/Ft.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



1-11-8 Over 3 Supports
R=230 U-180 W=3"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

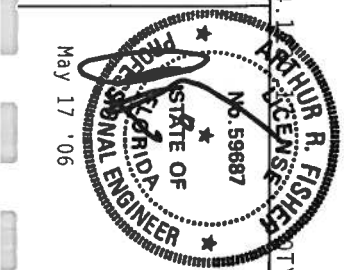
7.24.1

FL/-4/-1/-R/-

Scale = .5"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 MONROE RD., SUITE 200, MADISON, WI 53719) AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LN, MADISON, WI 53719) FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. APPLY CONNECTOR PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487-- 468
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137012
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6641
DUR.FAC.	1.25		
SPACING	24.0"	DRWF	15X4x87 202

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3 : W4 2x4 SP #2 Dense:

SPECIAL LOADS

TC - From	64 PLF at 0.00 to 64 PLF at 12.33
BC - From	20 PLF at 0.00 to 20 PLF at 12.33
TC - 183 LB Conc. Load at	2.75
TC - 56 LB Conc. Load at	2.81
TC - 54 LB Conc. Load at	4.81
BC - 1446 LB Conc. Load at	1.73
BC - 224 LB Conc. Load at	2.81
BC - 1283 LB Conc. Load at	3.73
BC - 19 LB Conc. Load at	4.81, 6.81, 8.81, 10.81
BC - 1260 LB Conc. Load at	9.73, 11.73

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

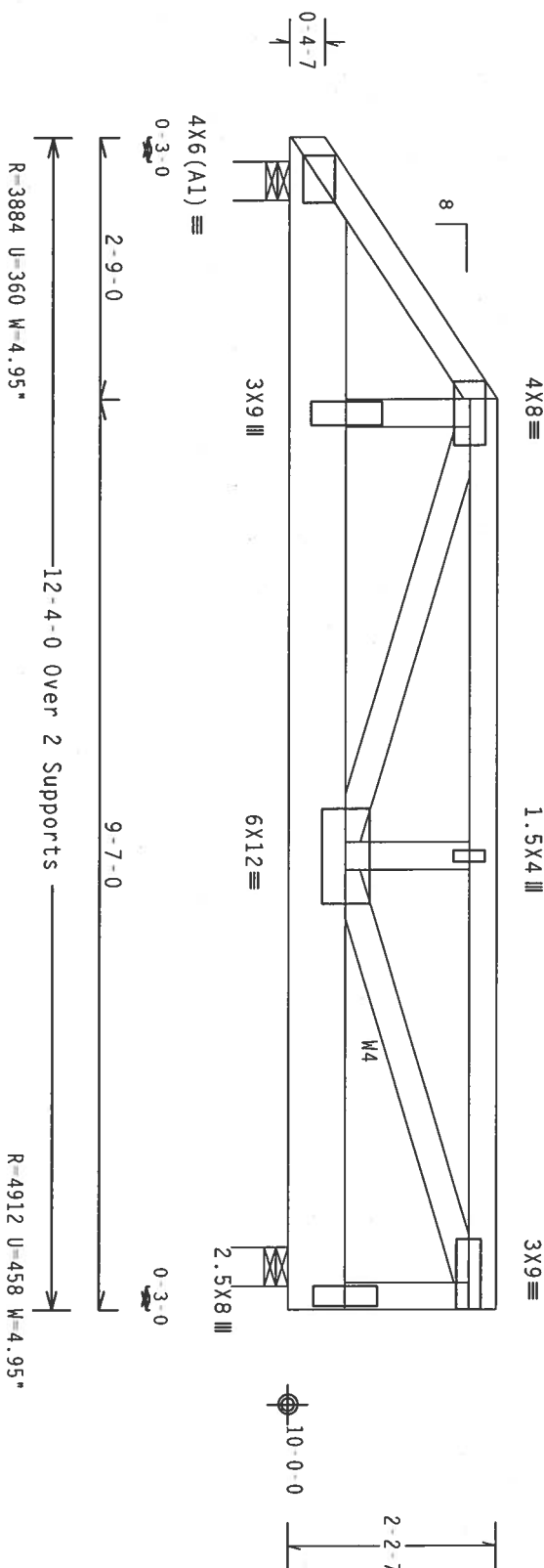
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Box or Gun (0.128"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @5.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



PLT TYP. Wave

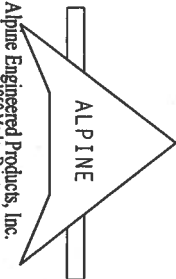
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.12

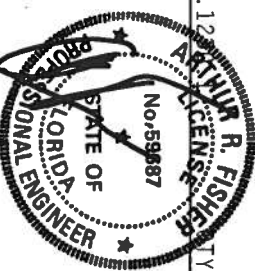
Scale = .5"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 DOWNTOWN DR., SUITE 200, MADISON, WI 53703) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN, DOWNTOWN DR., SUITE 200, MADISON, WI 53703) FOR TRUSS CONSTRUCTION DETAILS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/S/S) ASTM A653 GRADE 40/50 (W. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1990 Valley Drive
Haines City, FL 33844
Tel: 888-367-5677

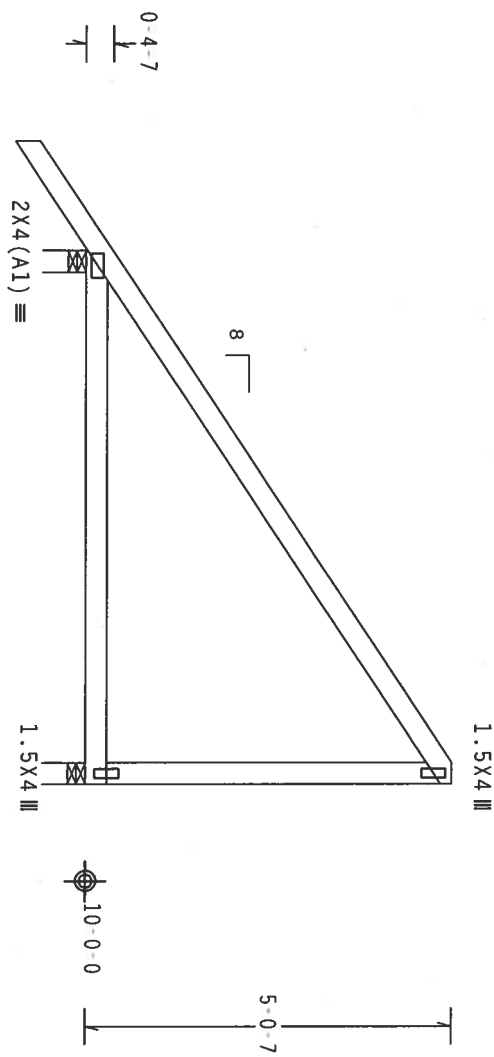


TC LL	20.0 PSF	REF R487-- 469
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUR487 06137026
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6892
DUR.FAC.	1.25	
SPACING	24.0"	
DRF	15X4487 202	

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Right end vertical not exposed to wind pressure.
#1 hip supports 7'-0" jacks with no webs.



7-3-8 Over 2 Supports
R-449 U=180 W=3.5"
R-753 U=180 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)



FL/-/4/-/R/-

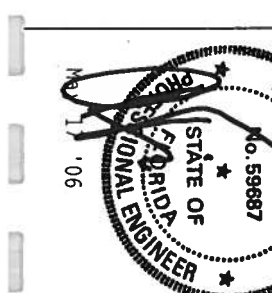
Scale = .375"/ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Page of 1
on # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS DESIGN AND THE TRUSS FABRICATOR SHALL BE RESPONSIBLE FOR THE TRUSS FABRICATION. THE TRUSS FABRICATOR SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S) ASTM A653 GRADE 40/50 (W. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF. R487-- 470
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137030
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6969
DUR.FAC.	1.25	
SPACING	24.0"	

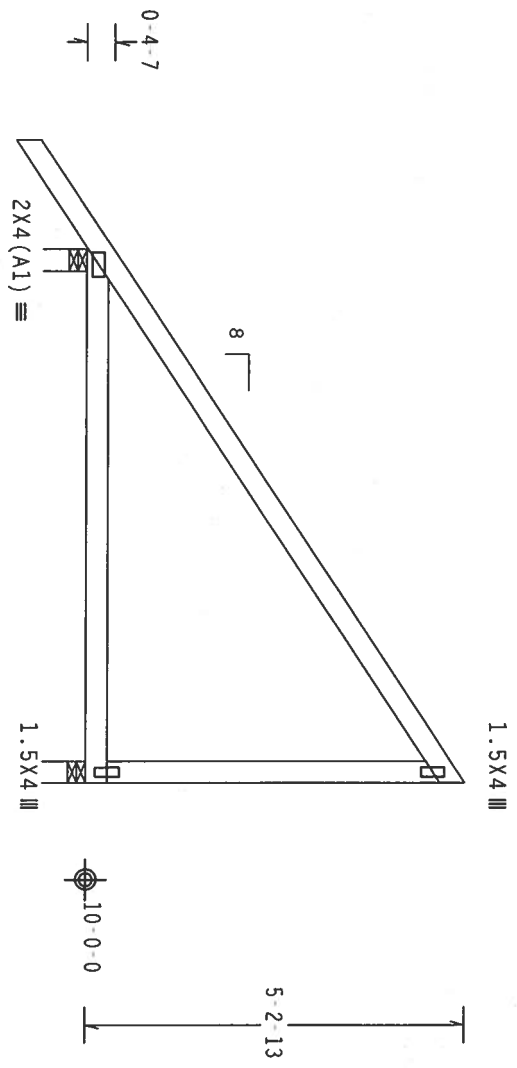
JRFF-15X487 202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.
Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



1-6-0

7-3-8 Over 2 Supports
R-429 U=180 W=3.5" R-287 U=180 W=3.5"

PLT TYP. Wave

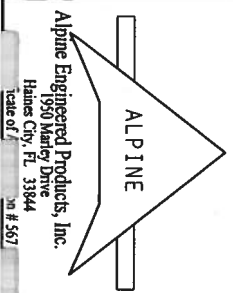
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583
HARDISON AVE, SUITE 100, HUNTSVILLE, AL 35893) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN,
HUNTSVILLE, AL 35893) FOR FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. UNLESS OTHERWISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

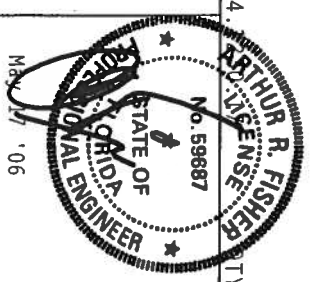
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE
DESIGN CONFORMS WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.
CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (W, K/H-S) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Date of: 05/17/06
In #567



TC LL	20.0 PSF	REF	R487 - - 471
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCSR487 06137032
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6723
DUR.FAC.	1.25		
SPACING	24.0"		

Scale = .375"/ft.

JRFF-15XAA07 202

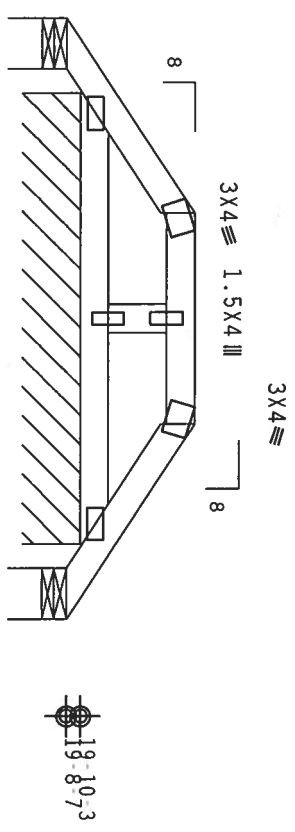
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGGYBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 20.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

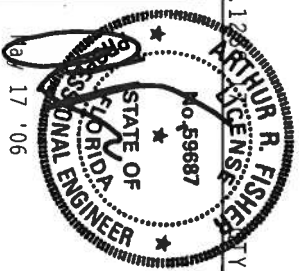
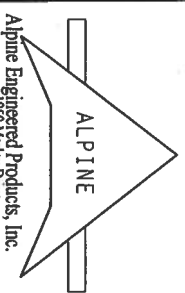


R-17 U-180 W-6.31"
R-76 PLF U-39 PLF W-4-7-7
R-17 U-180 W-6.31"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. ALL TRUSSES SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC) 360-10, 15TH EDITION, 2010. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

IMPORTANT TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/A/S) ASTM A653 GRADE 40/60 (4. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.



TC LL	20.0 PSF	REF	R487-- 472
TC DL	10.0 PSF	DATE	05/17/06
BC DL	2.0 PSF	DRW	HCUR487 06137023
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	32.0 PSF	SEQN-	6672
DUR.FAC.	1.25		
SPACING	24.0"	DRFF-	15XAAR7 202

Scale = .5"/ft.

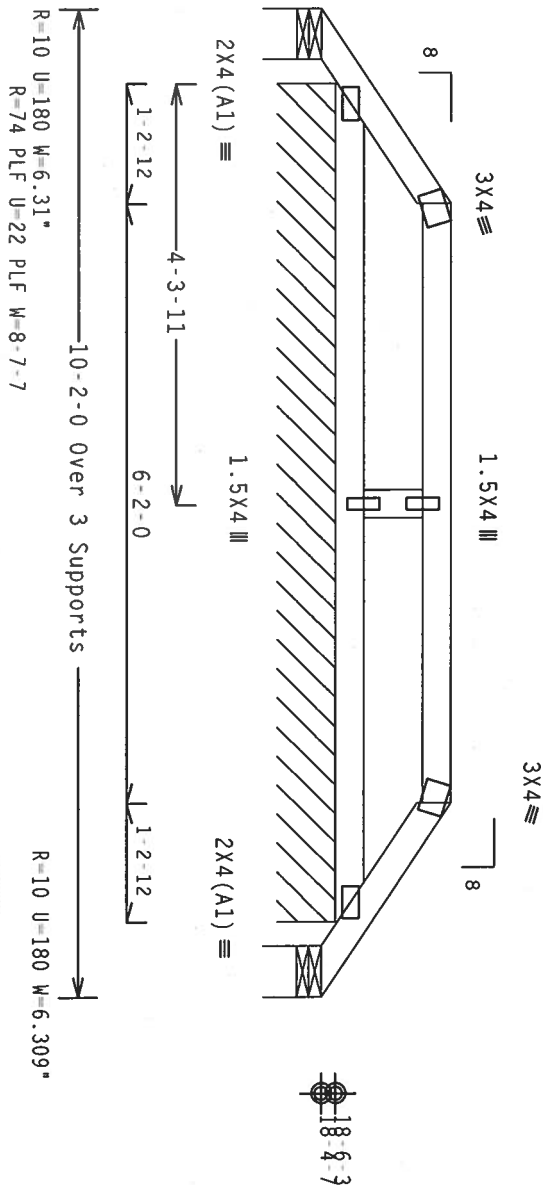
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGGYBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 19.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



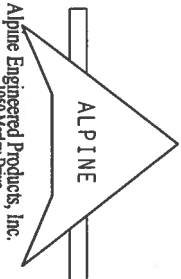
PLT TYP. Wave

Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24

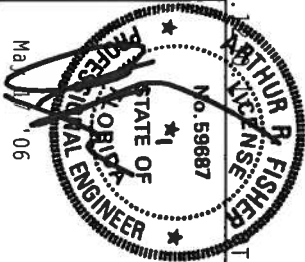
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 OAKWOOD DR., SUITE 200, FARMINGTON, CT 06031) AND TPI (TRUSS PLATE INSTITUTE, 583 OAKWOOD DR., SUITE 200, FARMINGTON, CT 06031) FOR ADDITIONAL INFORMATION. THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-H/S) ASTM A653 GRADE 40/60 (W, K/H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
Haines City, FL 33844
Date of Issue: 05/17/06



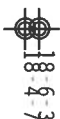
FL/-/4/-/R/-

Scale = .5"/ft.

TC LL	20.0 PSF	REF R487-- 473
TC DL	10.0 PSF	DATE 05/17/06
BC DL	2.0 PSF	DRW HCUR487 06137014
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN- 6651
DUR.FAC.	1.25	
SPACING	24.0"	DRFF- 15XAAR7 202

110 mph wind, 19.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.



Scale = .5" / Ft.

No. 59687

ALPINE ENGINEERED

$$\frac{BC \cdot LL}{0.0}$$

May 17, '06

SPACING 24.0"

JRFF - ISXAA87 202

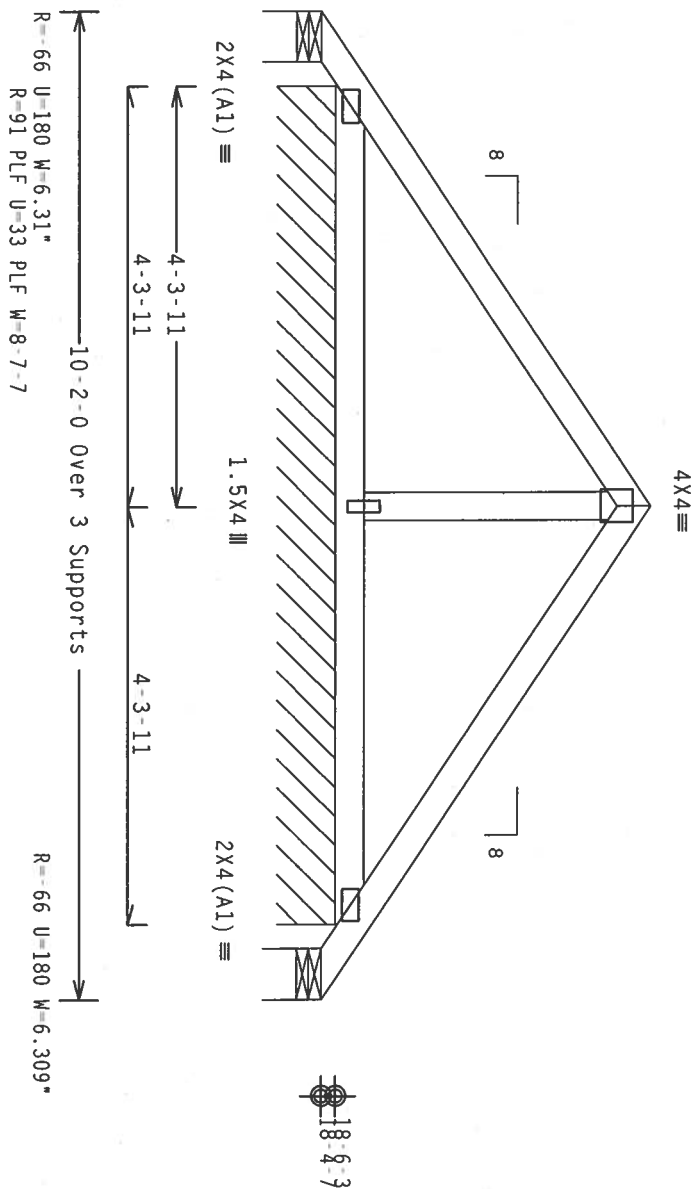
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 20.06 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.24.12

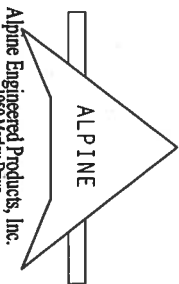
FL/-/4/-/R/-

Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 983 DOWNSBORO RD, SUITE 100, DOWNSBORO, VT 05237) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

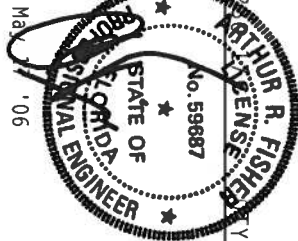
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (W/H/S/N) ASTM A653 GRADE 40/50 (W. K/M/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
1990 Marley Drive
Haines City, FL 33844

Scale of: 1/8" = 1'-0"
Sheet # 567



TC LL	20.0 PSF	REF	R487 - 475
TC DL	10.0 PSF	DATE	05/17/06
BC DL	2.0 PSF	DRW	HCUSR487 06137027
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	32.0 PSF	SEQN-	6662
DUR.FAC.	1.25		
CDACING	24.0"	JRFF	15XA487 202

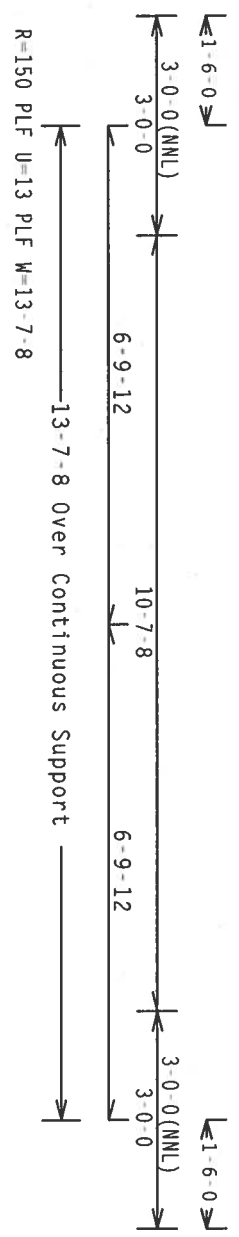
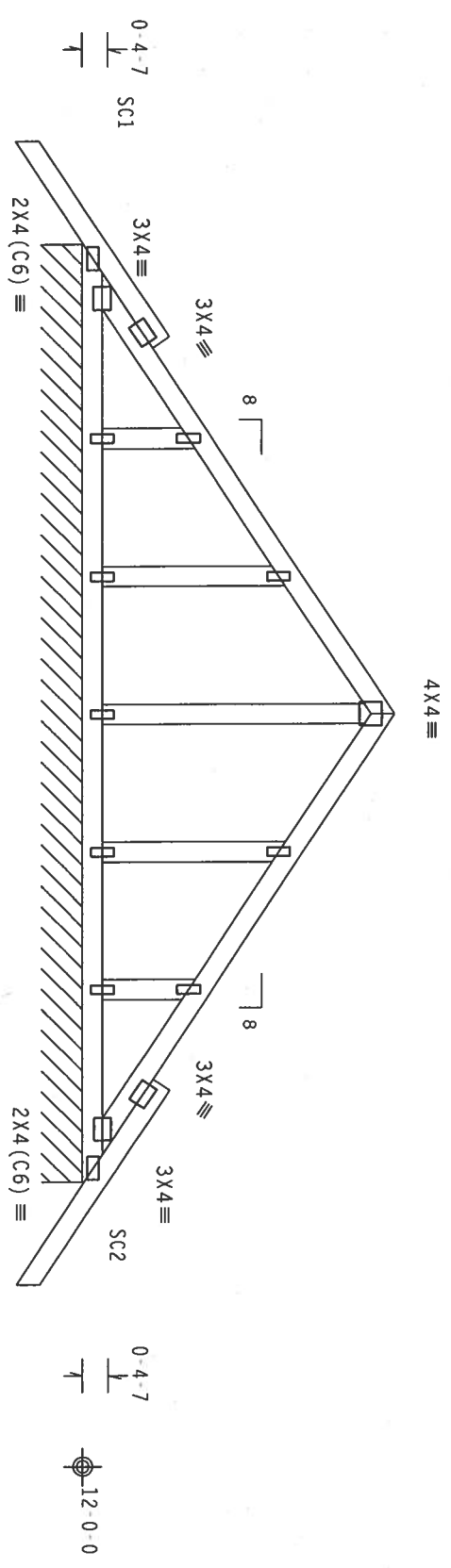
Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3
 :Stack Chord SC1 2x4 SP #2 Dense:
 :Stack Chord SC2 2x4 SP #2 Dense:

See DWGS A11015E0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
 Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Note: All Plates Are 1.5X4 Except As Shown.

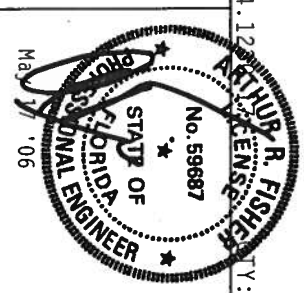
PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.24.12

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 585 DUNSTON DR., SUITE 200, MADISON, WI 53719) AND TRCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE BL., FARMINGTON, CT 06031) FOR ADDITIONAL INFORMATION. THE TRUSS MANUFACTURER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Alpine Engineered Products, Inc.
 1950 Hwy 100
 Haines City, FL 33844
 Phone 888-567-1067



TC LL	20.0 PSF	REF R487 - - 476
TC DL	10.0 PSF	DATE 05/17/06
BC DL	10.0 PSF	DRW HCUSR487 06137025
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 6856 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 15XA487 202

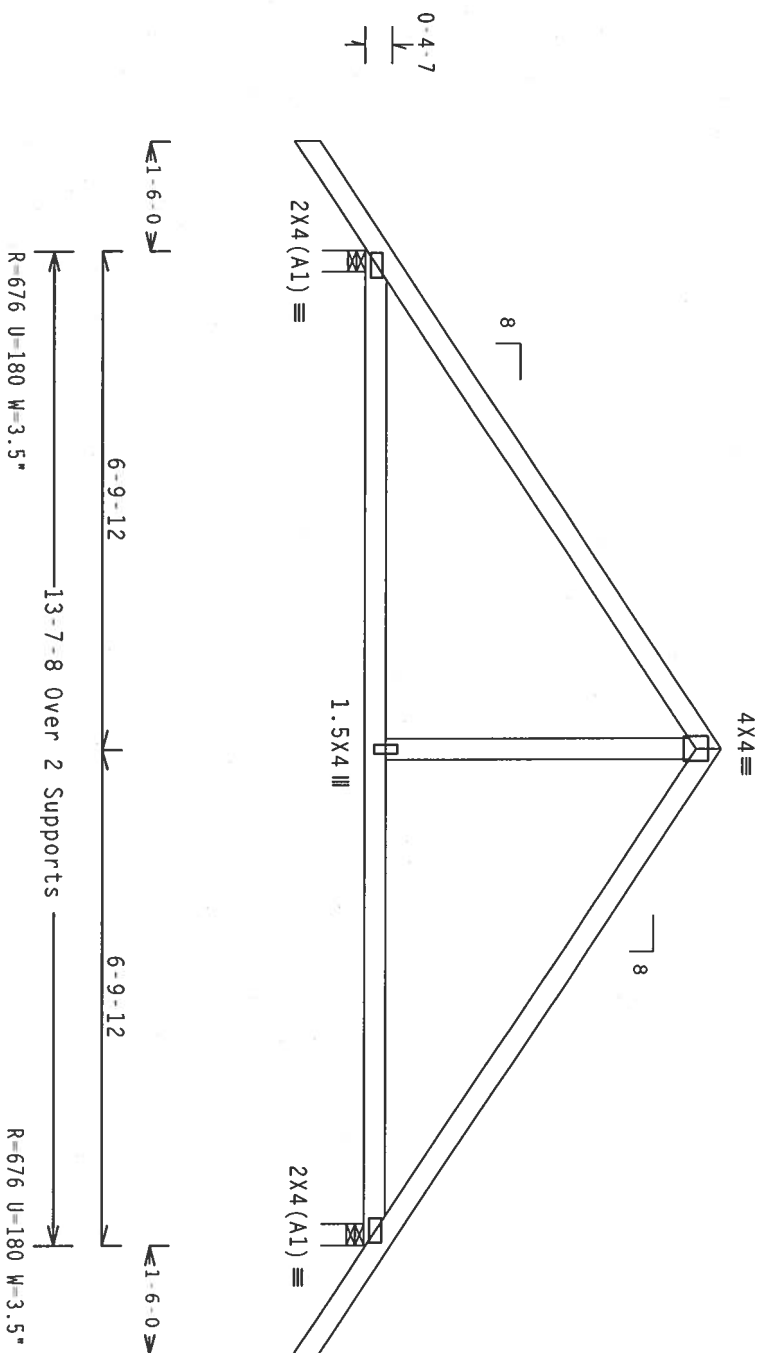
Scale = .375"/Ft.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @
24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



PLT TYP. Wave

Design Cr1t: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

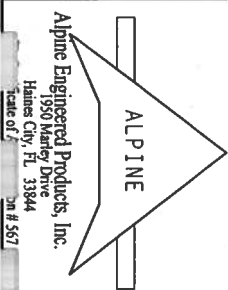
7.24

QTY: 2 FL/-/4/-/R/-

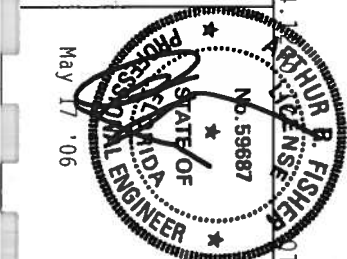
Scale = .375"/Ft.

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TC LL	20.0 PSF	REF	R487 - 477
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCUSR487 06137031
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	6852
DUR.FAC.	1.25		
SPACING	24.0"	JRFFC -	15XAA87 Z02

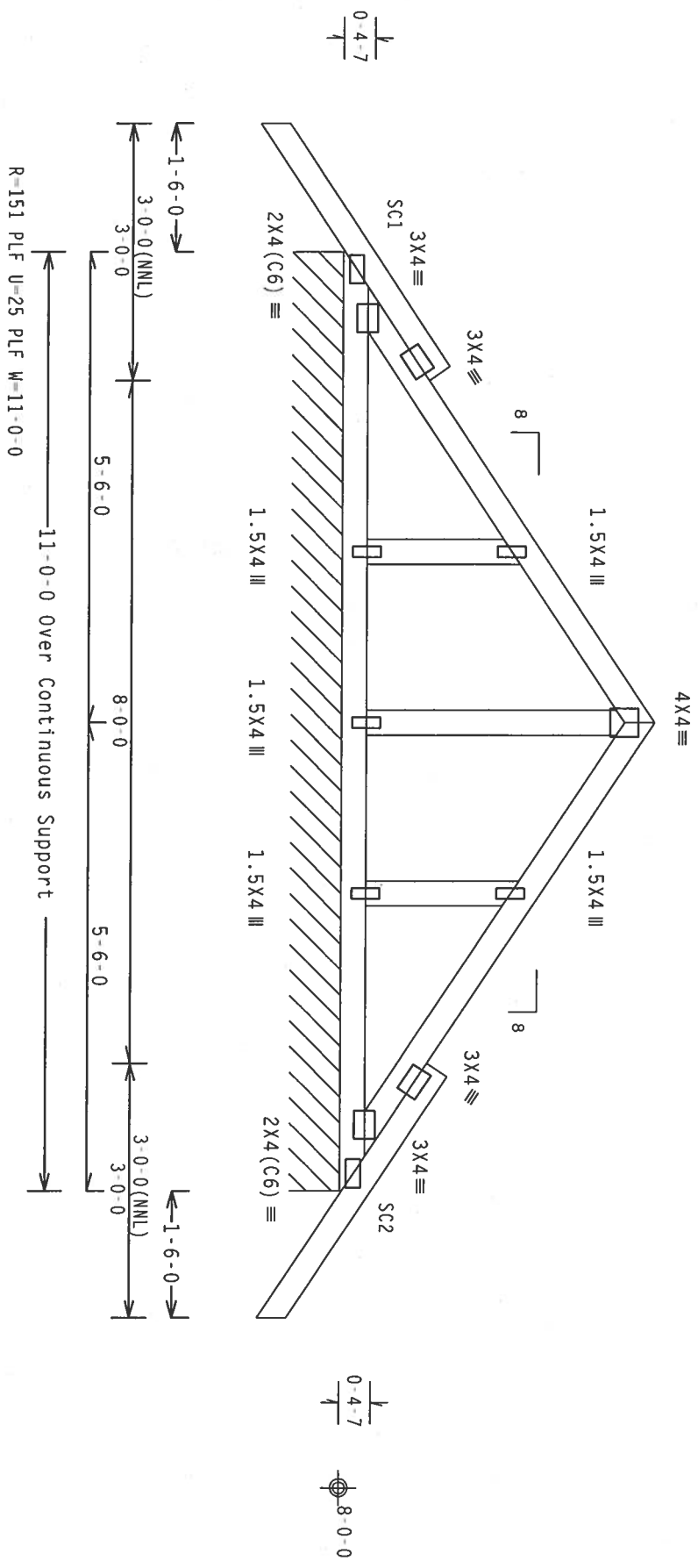
TOP Chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Stack Chord SC1 2x4 SP #2 Dense:
Stack Chord SC2 2x4 SP #2 Dense:

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25/10(0)) 7.24.1

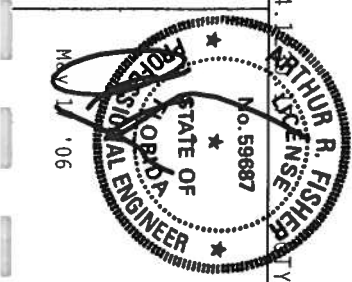
FL/-/4/-/R/-

Scale = .5"/ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Tel: 888-251-5672

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****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OR DAMAGE TO TRUSSES IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ACP&A) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W/H/S/K) ASTM A653 GRADE 40/60 (K, K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R487-- 479
TC DL	10.0 PSF	DATE	05/17/06
BC DL	10.0 PSF	DRW	HCURS487 06137001
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SECON	6974 REV
DUR.FAC.	1.25		
SPACING	24.0"		

JREF - 15XAA87 202

BEARING BLOCK NAIL SPACING DETAIL

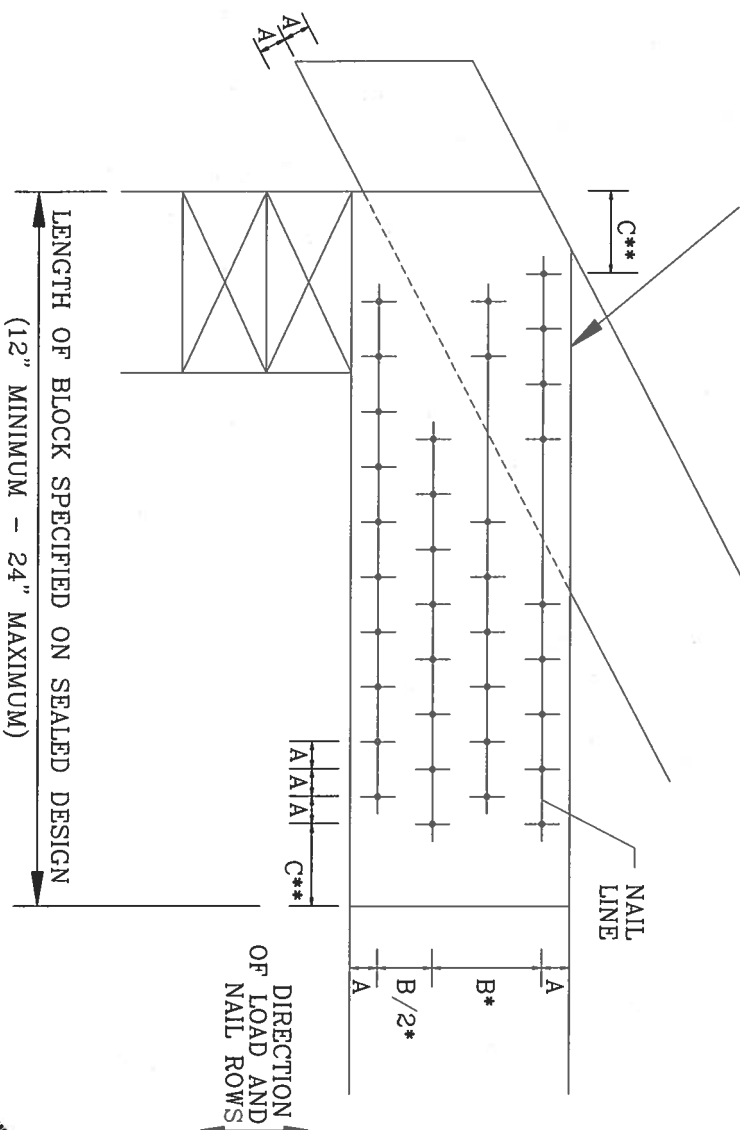
MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
 • SPACING MAY BE REDUCED BY 50%
 • SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.

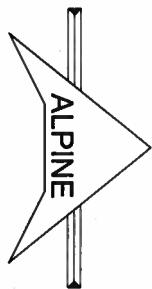


NAIL TYPE	CHORD SIZE				
	2X4	2X6	2X8	2X10	2X12
8d BOX (0.113"x2.5")	3	6	9	12	15
10d BOX (0.128"x3")	3	5	7	10	12
12d BOX (0.128"x3.25")	3	5	7	10	12
16d BOX (0.135"x3.5")	3	5	7	10	12
20d BOX (0.148"x4")	2	4	5	6	8
8d COMMON (0.131"x2.5")	3	5	7	10	12
10d COMMON (0.148"x3")	2	4	6	8	10
12d COMMON (0.148"x3.25")	2	4	6	8	10
16d COMMON (0.162"x3.5")	2	4	6	8	10
0.120"x2.5" GUN	3	6	8	11	14
0.131"x2.5" GUN	3	5	7	10	12
0.120"x3.0" GUN	3	6	8	11	14
0.131"x3.0" GUN	3	5	7	10	12

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	
8d BOX (0.113"x2.5")	3/4"	1 3/8"	1 3/4"	
10d BOX (0.128"x3")	7/8"	1 5/8"	2"	
12d BOX (0.128"x3.25")	7/8"	1 5/8"	2"	
16d BOX (0.135"x3.5")	7/8"	1 5/8"	2 1/8"	
20d BOX (0.148"x4")	1"	1 7/8"	2 1/4"	
8d COMMON (0.131"x2.5")	7/8"	1 5/8"	2"	
10d COMMON (0.148"x3")	1"	1 7/8"	2 1/4"	
12d COMMON (0.148"x3.25")	1"	1 7/8"	2 1/4"	
16d COMMON (0.162"x3.5")	1"	2"	2 1/2"	
0.120"x2.5" GUN	3/4"	1 1/2"	1 7/8"	
0.131"x2.5" GUN	7/8"	1 5/8"	2"	
0.120"x3.0" GUN	3/4"	1 1/2"	1 7/8"	
0.131"x3.0" GUN	7/8"	1 5/8"	2"	

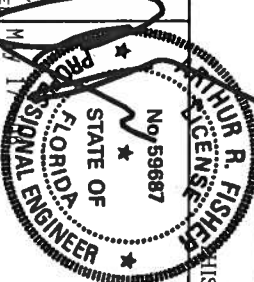
THIS DRAWING REPLACES DRAWING B139 AND CNBRGK0699



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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REF	BEARING BLOCK
DATE	11/26/03
DRWG	CNBRGK1103
-ENG	SJP/KAR

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

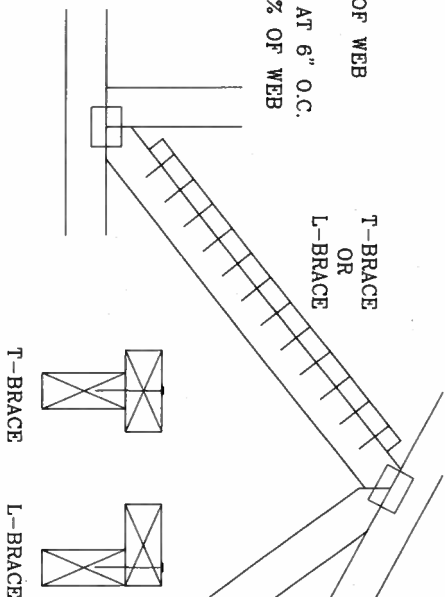
ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

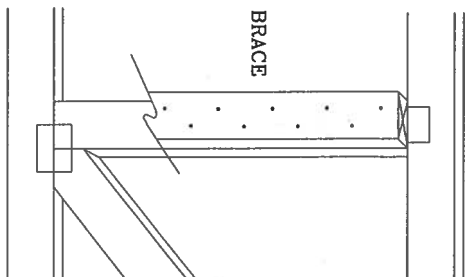
(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

T-BRACING
OR
L-BRACING:
APPLY TO EITHER SIDE OF WEB
NARROW FACE
ATTACH WITH 16d NAILS AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d OR .128"x3" GUN
NAILS AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER
No. 59867
STATE OF FLORIDA
Professional Engineer

TC LL
TC DL
BC DL
BC LL
TOT. LD.
DUR. FAC.
SPACING

PSF
PSF
PSF
PSF
PSF
PSF
PSF

REF
DATE 11/26/03
DRWG BRCB SUB1103
-ENG MLH/KAR

CLB SUBST.

MAX GABLE VERTICAL LENGTH															
GABLE SPACING	2x4 VERTICAL SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **			
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B				
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"		
			#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	
			STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	
	HF	STANDARD	#1	4' 3"	6' 8"	5' 2"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"		
			#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
			#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	
	DFL	STANDARD	#1	4' 0"	6' 3"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	
			#2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF	#1 / #2	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
			STUD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	
			#3	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
HF	STANDARD	#1	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"		
16" O.C.	DFL	STANDARD	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"		
			#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
			STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF	STANDARD	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	STANDARD	#1	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
	12" O.C.	SPF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
				#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
				STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
HF		STANDARD	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL		STANDARD	#1	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	

GROUP A:	
SPRUCE-PINE-FIR	HEM-FIR
#1 / #2 STANDARD	#2 STUD
#3 STUD	#3 STANDARD
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR	
#1 & BTR	
#1	

SOUTHERN PINE

#1	
#2	

DOUGLAS FIR-LARCH

#1	
#2	

CABLE TRUSS DETAIL NOTES:

E LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

ABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS

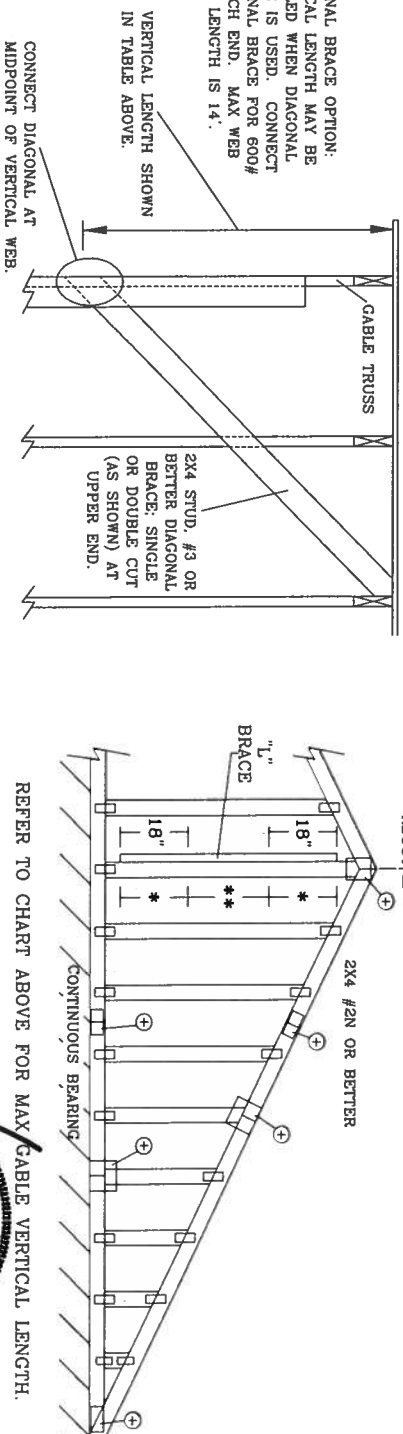
FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES

FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1/4 OR 2/3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2/4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX. CABLE VERTICAL LENGTH.

DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR 600#
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
IN TABLE ABOVE.

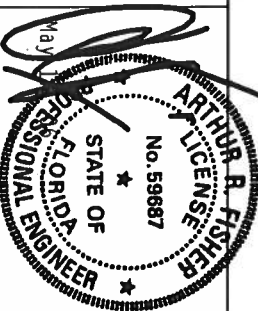
CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL WEB.



ALPINE ENGINEERED PRODUCTS, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 5653 DUNDREID DR., SUITE 200, MAISON, WI 53579) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. DEARBORN ST., CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPECIFICATION AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/28/16/64 (W/H/S/D) ASTM A653 GRADE 40/50 (W/H/S/D) STEEL. STEEL CONNECTOR PLATES TO EACH PAIR OF 20/28/16/64 (W/H/S/D) ASTM A653 GRADE 40/50 (W/H/S/D) STEEL TRUSSES SHALL BE 16" LONG. ALL DIMENSIONS SHALL BE IN INCHES UNLESS OTHERWISE SPECIFIED. SEE PARAGRAPH A3 OF TPI 1-2002, SEC. 3. THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SEAL FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.

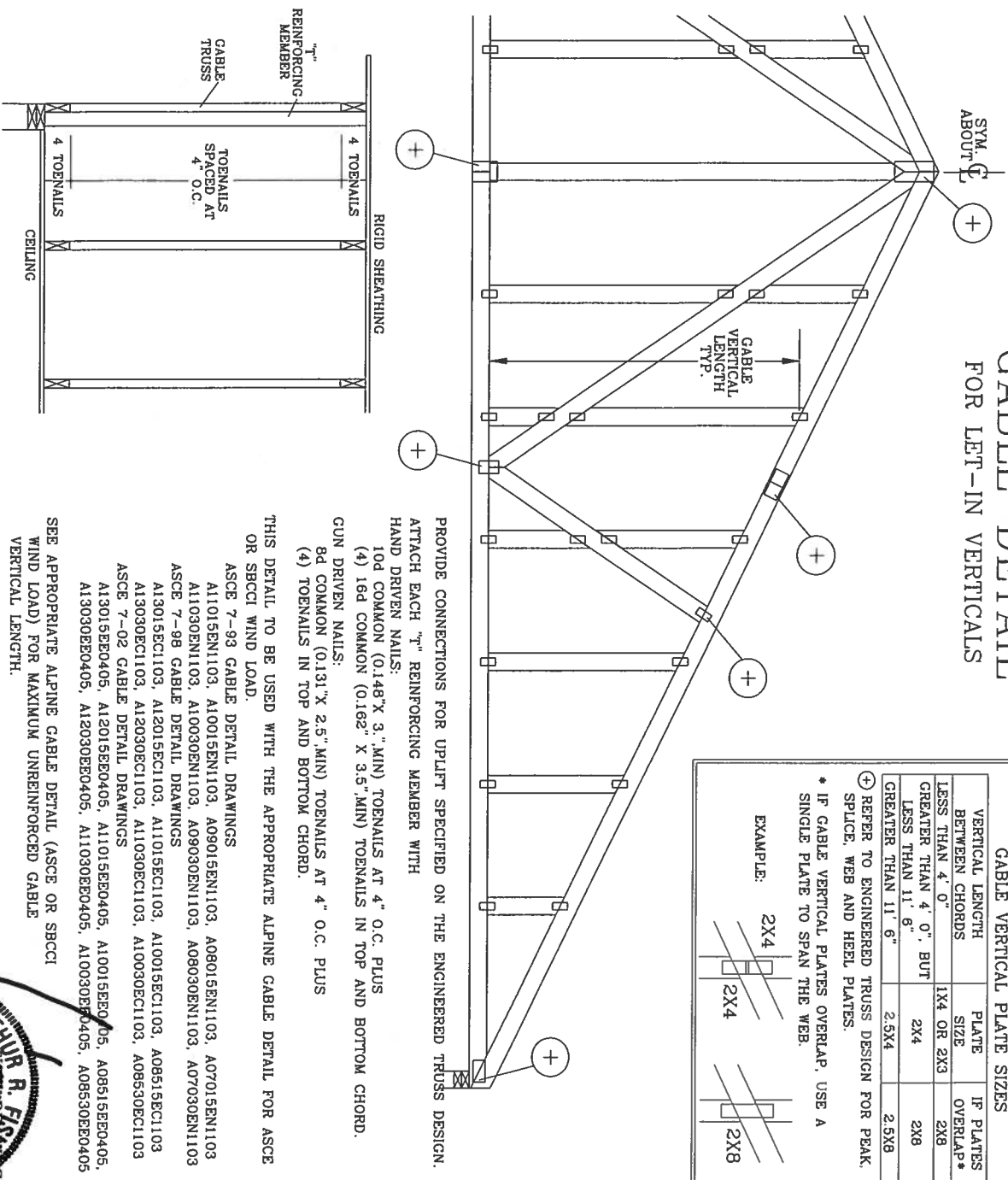


MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	04/15/05
DRWG	A11015EE0405
—ENG	

CABLE DETAIL FOR LET-IN VERTICALS



CABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1x4 OR 2x3	2x8
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2x4	2x8
GREATER THAN 11' 6"	2.5x4	2.5x8

* REFER TO ENGINEERED TRUSS DESIGN FOR PEAK SPLICE, WEB AND HEEL PLATES.

EXAMPLE: 2x4 2x4 2x8

* IF CABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN. ATTACH EACH "T" REINFORCING MEMBER WITH HAND DRIVEN NAILS:

(4) 16d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A11015EN1103, A10015EN1103, A08015EN1103, A07015EN1103

A11030EN1103, A10030EN1103, A08030EN1103, A07030EN1103

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015EC1103, A12015EC1103, A11015EC1103, A08515EC1103

A13030EC1103, A12030EC1103, A11030EC1103, A08530EC1103

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015EE0405, A12015EE0405, A11015EE0405, A08515EE0405

A13030EE0405, A12030EE0405, A11030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE CABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 DONDRIER DR., SUITE 200, MADISON, VI. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TYP. CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA C/V/S/K/ ASH A653 GRADE, 40/60 C/V/S/K/ GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FLINN
STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 59687

REPLACES DRAWINGS GAB98117 876.719 & HC26294035

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MBR.	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	10 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2x4

"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2x4 "T" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"

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2x4 "T" REINFORCING MEMBER

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TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #3 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

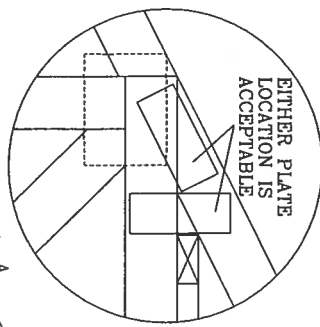
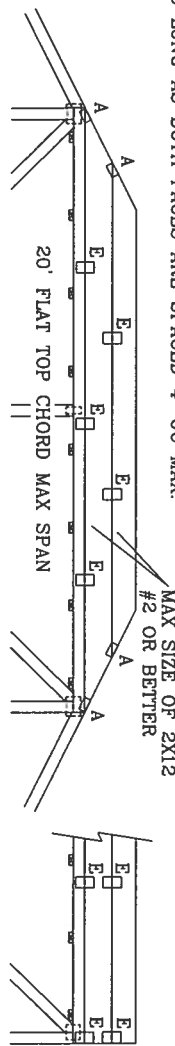
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

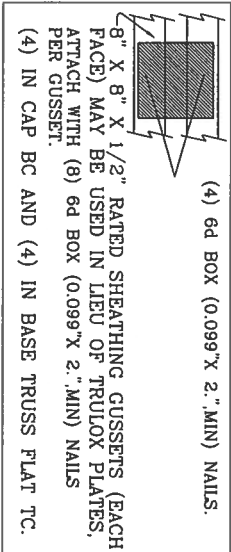
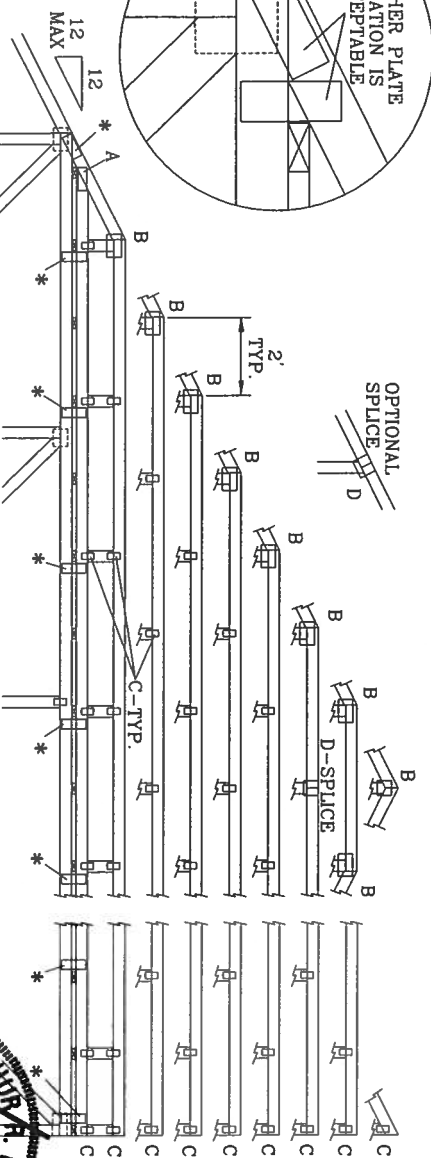
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C,
WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, SBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E.*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

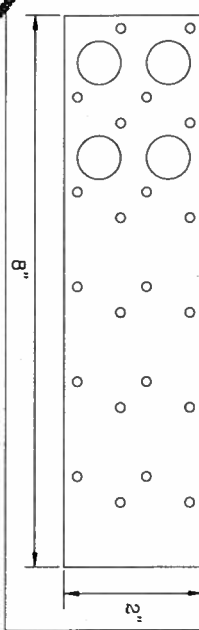


130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C,
WIND TC DL=5 PSF, WIND BC DL=5 PSF

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5" MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "I" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5" MIN) NAILS AT 4" OC.

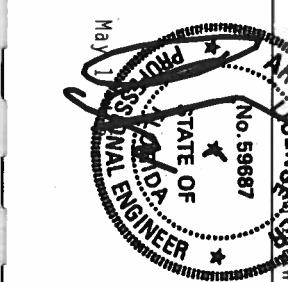
* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 DUNDON DR., SUITE 200, MADISON, WI 53719, AND VITA C/VOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA C.V./S/KS ASTM A553 GRADE, 40/60 C.V./H/S. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.



WIND REPLACES DRAWINGS 634.016 634.017 & 847.045	REF PIGGYBACK
MAX LOADING	DATE 04/14/05
55 PSF AT	DRWG PIGBACKB0405
1.33 DUR. FAC.	-ENG DLJ/KAR
50 PSF AT	
1.25 DUR. FAC.	
47 PSF AT	
1.15 DUR. FAC.	
SPACING 24.0"	