DATE 09/21/200		Imbia County Permit Expires One Yo			PERMIT 000026260
APPLICANT RO	BERT MCINTOSH	Termit Expires One 10	PHONE	754-8678	000020200
ADDRESS 28	NW CORIN	TH DR	LAKE CITY		FL 32055
OWNER GE	RALD & PATRICIA	HALE	PHONE	386.758.7857	
ADDRESS 244	4 SW CHALLI	ENGER LANE	LAKE CITY		FL 32025
CONTRACTOR	MICHAEL DELAH	IOZ	PHONE	754-8678	
LOCATION OF PR	OPERTY BA	SCOM NORRIS TO DEANN.	A,TR TO BAMBI,TR TO	WINGS TERRA	CE,TL
	TO	CHALLNGER,TR 3RD HOM	ME ON L.		
TYPE DEVELOPM	1ENT SCREEN I	ENCLOSURE ES	STIMATED COST OF CO	ONSTRUCTION	7200.00
HEATED FLOOR	AREA	TOTAL AR	EA	HEIGHT _	STORIES
FOUNDATION		WALLS	ROOF PITCH	FL	OOR
LAND USE & ZON	NING RSF-2		MAX	K. HEIGHT 3	5
Minimum Set Back		REET-FRONT 25.00	REAR	15.00	SIDE 10.00
	•				10.00
NO. EX.D.U. <u>1</u>	FLOOD 2	ZONE NA	DEVELOPMENT PER	MIT NO.	
PARCEL ID 07-	4S-17-08106-114	SUBDIVISIO	ON COUNTRY LANE	DING	
LOT 14 BL	OCK PH.	ASE UNIT	тот	AL ACRES 0.4	19
			177114		/
		SCC056689	1 Kalut	M. ltx	
Culvert Permit No.	Culvert Waiver			Applicant/Owner/	
EXISTING	<u>X07-373</u>	CS	 -	IH	N Name Paristant
Driveway Connectio	-	umber LU & Zoni	ing checked by App	proved for Issuance	e New Resident
COMMENTS: NO	C ON FILE				
				Cl l . // C .	sh 2513
				Check # or Ca	sn 2313
	FO	R BUILDING & ZONIN	NG DEPART M ENT	ONLY	(footer/Slab)
Temporary Power		Foundation	Jaka farra Barr	_ Monolithic _	
,, , , , , , , ,	date/app. by	01.1	date/app. by	C1 .1. A	date/app. by
Under slab rough-in		Slab _ date/app. by	date/app. by	Sheathing/N	Vailingdate/app. by
Framing			bove slab and below wood	d floor	autorupp. Oy
	date/app. by				date/app. by
Electrical rough-in		Heat & Air Duct		Peri. beam (Lintel	
.	date/app. by		date/app. by		date/app. by
Permanent power	date/app. by	C.O. Final	date/app. by	Culvert	date/app. by
M/H tie downs, block	king, electricity and plo	umbing		Pool	autorapp. oy
Reconnection		date/app			date/app. by
	date/app. by	Pump pole date	Utility Po e/app. by	date/app. by	_
M/H Pole		Travel Trailer	••	Re-roof	
date/app	. 		1,	-	
	p. by		date/app. by		date/app. by
BUILDING PERMIT				SURCHARGE	date/app. by
	T FEE \$ 40.00	CERTIFICATION FE	EE \$ 0.00	SURCHARGE	fee \$ 0.00
MISC. FEES \$	0.00 ZO	CERTIFICATION FE	EE\$ 0.00 FIRE FEE\$ 0.00	SURCHARGE WASTE	FEE \$ 0.00
	0.00 ZO	CERTIFICATION FE	EE\$ 0.00 FIRE FEE\$ 0.00	SURCHARGE WASTE	fee \$ 0.00
MISC. FEES \$	0.00 ZO MENT FEE \$	CERTIFICATION FE	EE\$ 0.00 FIRE FEE\$ 0.00	SURCHARGE WASTE	FEE \$ 0.00

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

cht 3513

Columbia County Building Permit Application

	91	2/2/0
For Office Use Offix Application w	ceived 917 By W Per	
Application Approved by	Plans Examiner OK 3	
Flood Zone Development Permit Zoning	RSF-2Land Use Plan Map	Category _ ~ ~ U
Comments		\
NOC FEH O Deed or PA & Site Plan Sta	re Road Info Parent Parcel #	
who to sale is a sale in		7508-425-
Name Authorized Person Signing Permit Mart M Inter		-104-0010
Address 289 MW Correction of hake lity	4 39022	ンペッション
Owners Name Hale, Gerald & Patricia	1 1 1	158-7867
911 Address 241 SW (Mallenger hane		
Contractors Name Michael A Dela Hoz	_ PATE AND ADDRESS OF THE PATE ADDRESS OF THE PATE AND	8628-626
Address 927 Hickory St Altamonte Sprin	52,41 25/11	
Fee Simple Owner Name & Address		
Bonding Co. Name & Address N/A		
Architect/Engineer Name & Address		
Mortgage Lenders Name & Address N/A		1 %
Circle the correct power company - FL Power & Light - Cla		
Property ID Number 7 - 48-17-08106-114 H1	_ Estimated Cost of Constructio	u 1300.00
Subdivision Name Country Landings	Lot N Block	
Priving Directions Bascon Morris to a tight	on Deanne then	righton
	ight on Challenger	B9 319
Nouse on the left		
Type of Construction You Enclosure	Number of Existing Dwellings o	n Property\
Total Acreage DACT Lot Size Do you need a - <u>Cu</u>		
Actual Distance of Structure from Property Lines - Front 107'	Side <u>30.10'</u> Side <u>10.</u>	$\frac{\lambda'}{\lambda'}$ Rear $\frac{80'}{\lambda'}$
Total Building Height Number of Stories	Heated Floor Area	Roof Pitch
Application is hereby made to obtain a permit to do work and	inetalistions as indicated I cert	ify that no work or
installation has commenced prior to the issuance of a permit	and that all work be performed t	to meet the standards of
all laws regulating construction in this jurisdiction.		ala a 111 hara da was be
OWNERS AFFIDAVIT: I hereby certify that all the foregoing in compliance with all applicable laws and regulating construction.	ormation is accurate and all wo on and zoning.	ork will be done in
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU	E OF COMMENCMENT MAY RE	SULT IN YOU PAYING
LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTIC	E OF COMMENCEMENT.	, CONSULT WITH TOOK
	11 m	111.11
Owner Builder or Authorized Person by Notarized Letter	Contractor Signature	N Willy
•	Contractors License Num	
STATE OF FLORIDA COUNTY OF COLUMBIA	Competency Card Number NOTARY STAMP/SEALURE	Nadean G.S. McIntosh
Sworn to (or affirmed) and subscribed before me		Commission # DD371494
this \\\ day of \(\sqrt{20 \delta\cdot} \).	1111 4 61 700 3	
	Water Soll Mediate	ended Troy Fain - Insurance, Inc. 809-385-7919
Personally known or Produced Identification	Notary Signature	Expires November 14, 2008 ended Tray Fein - Insurance, Inc. 808-385-7919 (Revised Sept. 2006

NOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice the Comprovement will be made to certain real property, and in accordan with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 07-43-17	1016-119 HX
1. Description of property: (legal description of	the property and street address or 911 address)
TRITA COUNTY MONUME	DRB 520:229,900-229,952-
36x7 MB 1009-1101)
44 pm (pullencer)	Con a
24 Sw Challenger	Inst:200712021085 Date:9/17/2007 Time:9:29 AM
	odding County Page 1 of 1
2. General description of improvement:	1 tondosure
3. Owner Name & Address	atricia Hale
AH Sw Challenger Lan	3303 Interest in Property OWN & F
4. Name & Address of Fee Simple Owner (if oth	er than owner):Y_Y\
· ·	
5. Contractor Name Wike Delay	Phone Number 366-754-8678
Address 927 Hickory St Pilo	100 Pel ver 16 18 17 com
6. Surety Holders Name	Phone Number
Mudiess	
- Amount of Bond	
7. Lender Name W/A	Phone Number
Address	Phone Number
8. Persons within the State of Florida designate served as provided by section 718.13 (1)(a) $\%$; Fig.	d by the Owner upon whom notices or other documents may be prida Statutes:
Name	Phone Number
	F 1 1 11 11 1
9. In addition to himself/herself the owner designation of the state o	gnates 110tid Citrol Enclosure
(a) 7. Phone Number of the designee	e a copy of the Lienor's Notice as provided in Section 713.13 (1)
10. Expiration date of the Notice of Commencem	ent (the expiration date is 1 (one) year from the date of recording
(Unless a different date is specified)	the expiration date is 1 (one) year from the date of recording
OTICE AS PER CHAPTER 713. Florida Statutos:	
/	nt and no one else may be permitted to sign in his/her stead.
1000	Sworn to (or affirmed) and subscribed before
\ \ \ (1/1/1	day of thisust 23 2007
Signature of Owner	NOTARY S Nadean G.S. McIntosh Commission # DD371494 Expires November 14, 2008
	Exhibs investible 14, 2006

Print

Columbia County Property Appraiser

DB Last Updated: 8/2/2007

Parcel: 07-4S-17-08106-114 HX

2007 Proposed Values

Tax Record Property Card Interactive GIS Map

New Super Homestead Taxable Value Calculator

Prev Search Result: 7 of 28

Next >>

GIS Aerial



Owner & Property Info

Owner's Name	HALE GERALD E &					
Site Address	CHALLENGER					
Mailing Address	PATRICIA R 244 SW CHALLENGER LANE LAKE CITY, FL 32025					
Use Desc. (code)	SINGLE FAM (000100)					
Neighborhood	7417.04	Tax District	2			
UD Codes	MKTA06	MKTA06 Market Area 06				
Total Land Area	0.498 ACRES					
Description	LOT 14 COUN 2687. WD 10	TTRY LANDINGS S/D. ORB 8 62-1106.	320-229, 906-2239, 952-			

Property & Assessment Values

Total Appraised Value		\$274,276.00
XFOB Value	cnt: (2)	\$3,456.00
Building Value	cnt: (1)	\$213,820.00
Ag Land Value	cnt: (0)	\$0.00
Mkt Land Value	cnt: (1)	\$57,000.00

Just Value		\$274,276.00
Class Value		\$0.00
Assessed Value		\$260,836.00
Exempt Value	(code: HX)	\$25,000.00
Total Taxable Value		\$235,836.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
10/18/2005	1062/1106	WD	I	Q		\$315,000.00
5/1/2002	952/2687	WD	I	U	99	\$100.00
6/28/2000	906/2239	WD	V	Q		\$30,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value			
1	SINGLE FAM (000100)	2002	Vinyl Side (31)	2177	5304	\$213,820.00			
	Note: All S.F. calculations are based on exterior building dimensions.								

Extra Features & Out Buildings

Code	Desc	Year Bit	Value	Units	Dims	Condition (% Good)
0166	CONC,PAVMT	2001	\$1,452.00	968.000	22 x 44 x 0	(.00)
0166	CONC,PAVMT	2003	\$2,004.00	1002.000	0 x 0 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	1.000 LT - (.498AC)	1.00/1.00/1.00/1.00	\$57,000.00	\$57,000.00

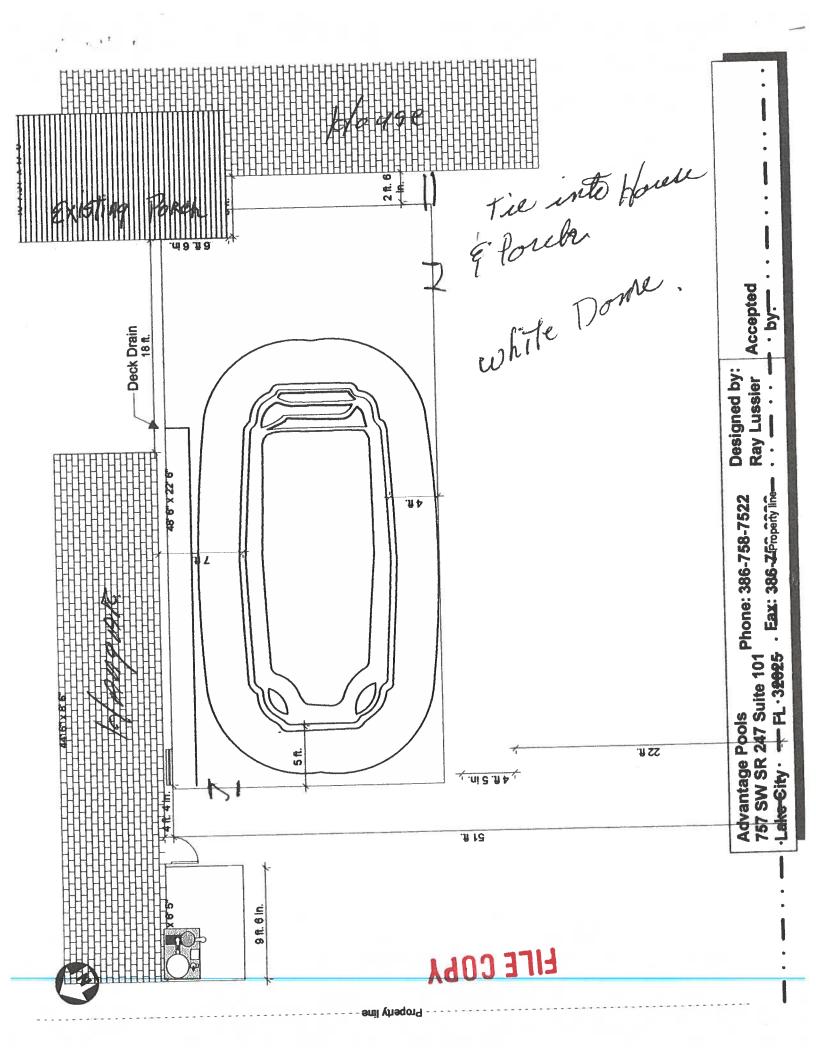
Columbia County Property Appraiser

DB Last Updated: 8/2/2007

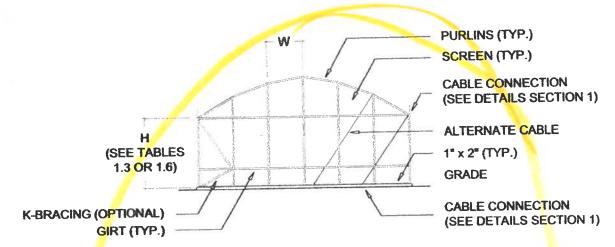
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7 of 28

Next >>

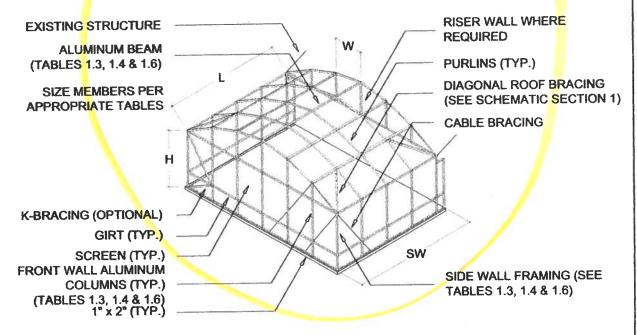


SECTION 1



TYPICAL DOME ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL & STRUCTURAL ENGINEERING P.O. Box 214368, South Daytona, Fl 32121

Telephone #: (386) 767-4774 Fax #: (386) 767-6556 Email: lebpe@bellsouth.net

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Table 1.7

Minimum Size Screen Enclosure Knee Braces and Anchoring Required

Aluminum 6063 T-6

Brace Length*	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 3'-0"		2" H-Channel With (3) #10 x 1/2" each leg of channel
		2" H-Channel With (4) #10 x 1/2" each leg of channel

^{*} Knee brace length shall be the horizontal and vertical length @ a 45° angle from the center of the connection to the face of the beam or upright.

Note:

- 1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
- 2. Cantilever beam detail shown on page 1-40 shall be used for transom wall to host structure attachment when knee brace length exceeds 6'-0".

Table 1.8 K-Bracing Fastening Schedule

	Number of #10 x 3/4" S.M.S. Required							
Maximum Wall Width =	Corner Post @ Top	Diagonals (K) per End	Intermediate Post @ Chair Rail	Corner Post @ Bottom	Plate to Sole Plate			
20'-0"	2	2	4	2	22			
30'-0"	2	2	4	2	2			
40'-0"	3	4	6	2	2			
50'-0"	4	5	8	3	3			
60'-0"	6	7	12	3	3			

Use front wall width when determining number of s.m.s. for the side wall K-bracing. Use side wall width when determining number of s.m.s. for the front and / or back wall K-bracing.

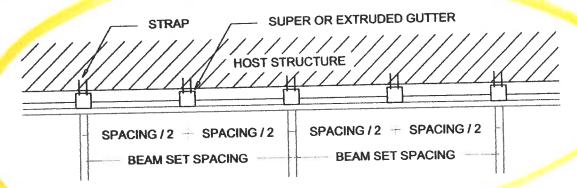
REVISED APRIL 2007

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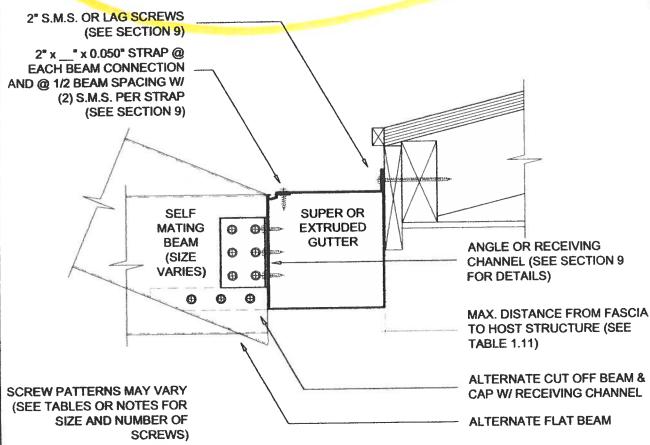
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STRAP LOCATION FOR SUPER OR EXTRUDED GUTTER REINFORCEMENT

SCALE: 3/8" = 1'-0"



SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

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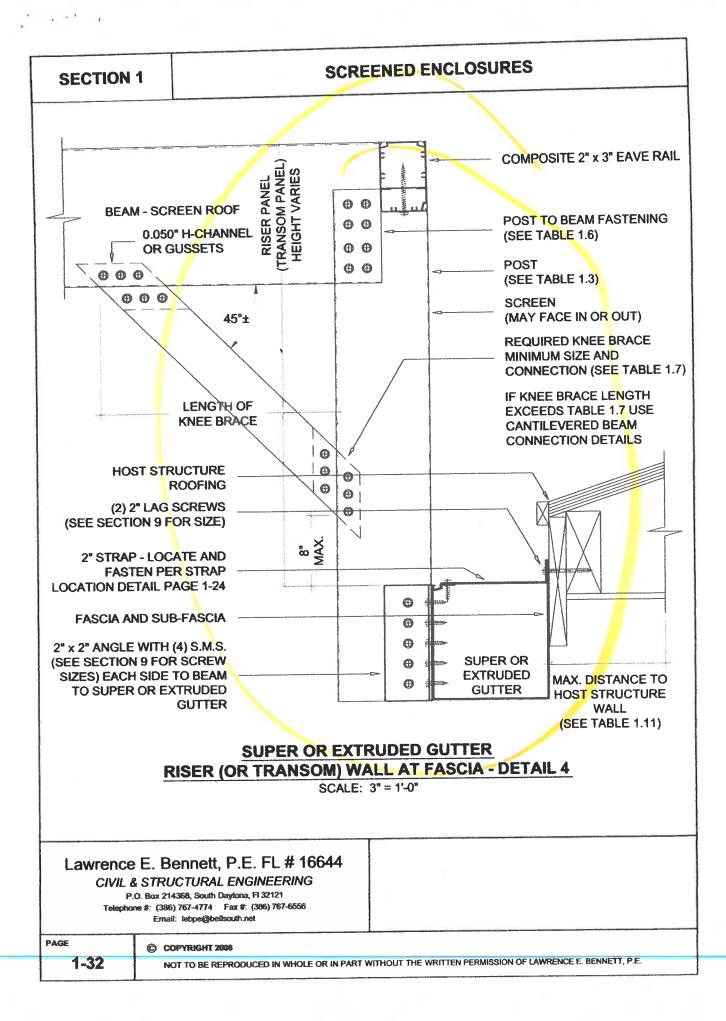
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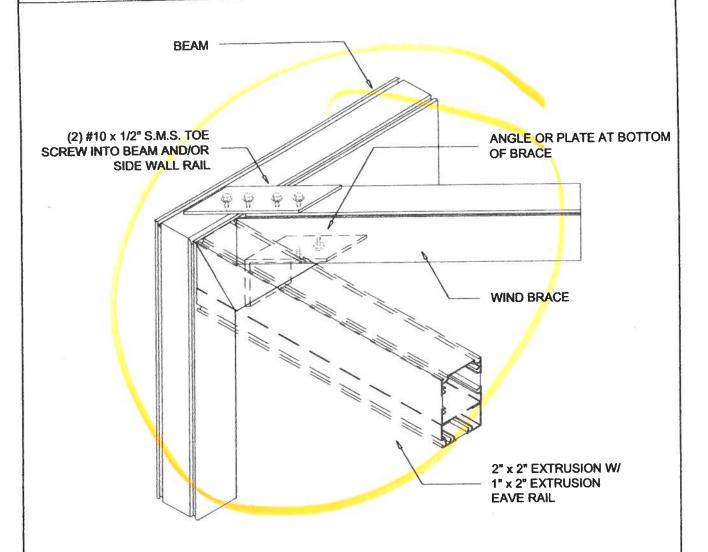
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SECTION 1



WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTES:

1. Wind bracing shall be provided at each side wall panel when enclosure projects more than (4) panels from host structure.

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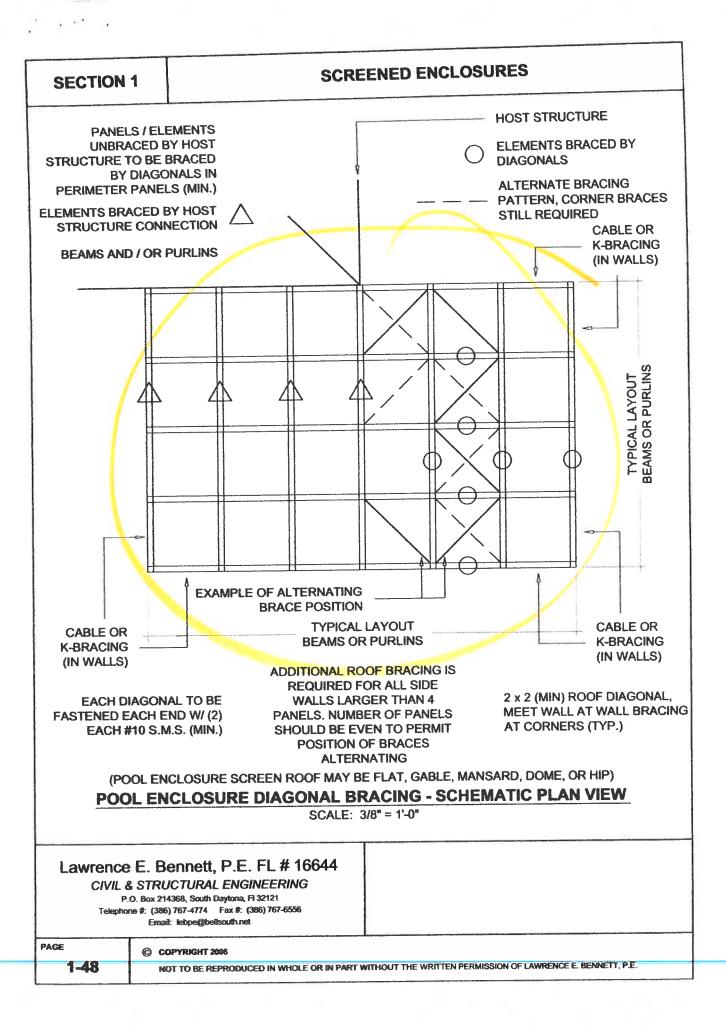
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CABLE BRACING

General Notes and Specifications:

- 1) The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:
 - a) FRONT WALL CABLES 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
/ 1/8°	445 Sq. Ft. / PAIR OF CABLES

* TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE:

FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft. SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft. TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 19 STAINLESS STEEL

CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

^{**} SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

c) To calculate the required pair of cables for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

NOTES:

- 1. Where wall height is such that a girt is required between the top or eave rail and the chair rail, (i.e. a mid-rise girt), then the front wall shall have two cable pairs and they shall be attached to the top rail and the mid-rise rail. If more than one additional girt is required between the top or eave rail and the chair rail, then there shall be an additional front wall cable pair at that girt also.
- 2. Side walls do not require cables until the side wall area is greater than 233 Sq. Ft.. The side wall cable may be attached at the mid-rise girt or the top rail.
- 3. Standard rounding off rules apply. ie: if the number of cables calculated is less than 2.5 pairs use two cables; if the number of cables calculated is 2.5 pairs or greater use 3 pairs of cables.
- 4. Additional roof bracing is required for all side walls larger than 4 panels. Number of panels shall be even and position shall be alternating.

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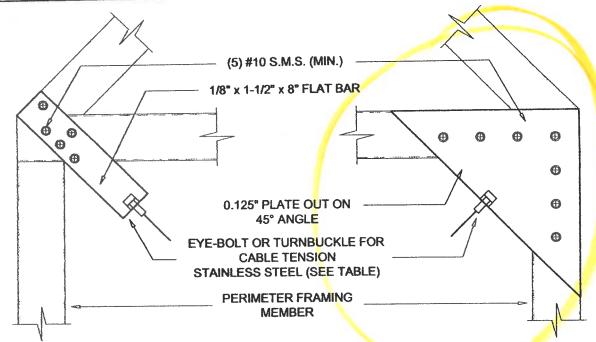
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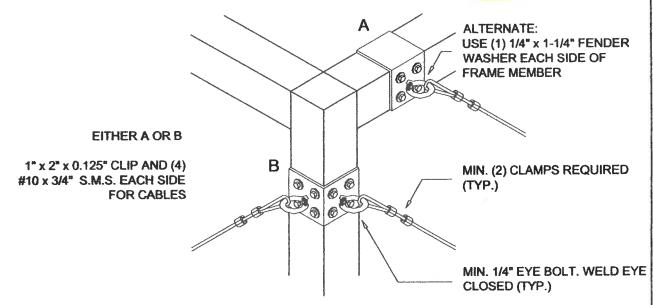
1-50

SECTION 1



TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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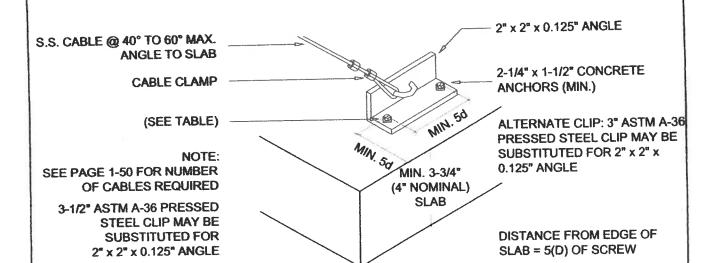
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SECTION 1



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0" SELECT ANCHOR FROM TABLE 9-1, MIN. SHEAR 607# FOR 3/32" CABLE AND 594# FOR 1/8" CABLE, FOR 3/32° CABLE (1) 1/4" x 1-1/2" CONCRETE ANCHOR (MIN.) @ 5d MIN. 5d (MIN.) 5-1/2" (6" NOMINAL) SLAB (MIN.) 5d (MIN.) 2500 P.S.I. CONCRETE 6 x 6 - 10 x 10 WELDED WIRE MESH OR FIBER MESH CONCRETE

ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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SECTION 1

SCREENED ENCLOSURES

0

PURLINS ANCHORED W/ CLIPS OR #10 SCREWS THROUGH PURLINS INTO SCREW BOSSES

EAVE RAILS SHALL BE STITCHED W/ #10 x 1-1/2" SMS @ 6" FROM EACH END AND 24" OC MAX.

FRONT AND SIDE BOTTOM
RAILS ATTACHED TO
CONCRETE W/ 1/4" x 2-1/4"
CONCRETE / MASONRY
ANCHORS @ PRIMARY &
SECONDARY ANGLES OR @ 6"
FROM EACH POST AND 24"
O.C. MAX. AND WALLS MIN. 1"
FROM EDGE OF CONCRETE

GIRTS ANCHORED W/ CLIPS
OR THROUGH #10 SCREWS
INTO SCREW BOSSES

1" x 2" OR 1" x 3"

PURLIN & CHAIR RAIL DETAIL

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

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SCREENED ENCLOSURES SECTION 1 2" x 2" x 0.063" PRIMARY ANGLE **EACH SIDE** #10 x 3/4" S.M.S. EACH SIDE **SCREEN** 'd' VARIES (SEE SCHEDULE NEXT PAGE) 4" SHOWN 1" x 2" O.B. BASE PLATE (TYP.) 5d* MINIMUM EDGE DISTANCE FROM EXTERIOR OF COLUMN SECONDARY 0 TO OUTSIDE EDGE OF SLAB 2" x (D - 2 ") x 0.063" ANGLE **(1)** BOLT Ø | * 5d DISTANCE EACH SIDE OF COLUMN W/ #10 0 4d 1-1/4" 1" 1/4" S.M.S. 3/8" 1-7/8" 1-1/2" (SEE SCHEDULE NEXT PAGE) GRADE **CONCRETE ANCHOR** (SEE SCHEDULE PAGE 65) 1-1/4" MIN. CONCRETE ANCHOR EMBEDMENT NOTE: DETAIL ILLUSTRATES 2500 P.S.I. CONCRETE OR TYPICAL 2" x 4" S.M.B. COLUMN 2" 5d ALTERNATE 2" x __ WOOD CONNECTION (MIN.) (MIN.) DECK SIDE VIEW TYPICAL S.M. OR SNAP SECTION COLUMN **SCREEN** #10 x 3/4" S.M.S. EACH SIDE (SEE SCHEDULE PAGE 1-65) **CONCRETE ANCHOR THRU** ANGLE OR WITHIN 6° OF PRIMARY 2" x 2" x 0.063" ANGLE UPRIGHT IF INTERNAL SCREWS INTO SCREW 1" x 2" BASE PLATE (TYP.) **BOSSES** 1-1/4" MIN. CONCRETE NOTE: SELECT CONCRETE ANCHOR EMBEDMENT **ANCHOR FROM TABLE 9.1** 6" (MAX.) 6" (MAX.) MAX. SPACING 24" O.C. FOR BOTH SIDES

FRONT VIEW

2" x 4" OR LARGER SELF MATING OR SNAP SECTION POST TO DECK DETAILS

SCALE: 3" = 1'-0"

NOTE:

- 1. FOR SIDE WALLS OF 2" x 4" OR SMALLER ONLY ONE ANGLE IS REQUIRED.
- 2. PREDRILL PAVERS W/ MIN. 1/4" MASONRY BIT.

Lawrence E. Bennett, P.E. FL # 16644

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1-64

SECTION 1

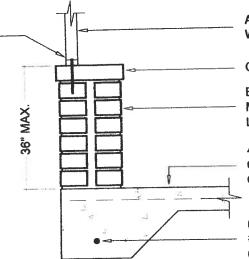
1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND ROWLOCK INTO FIRST COURSE OF BRICKS

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL

1" WIDE x 0.063" THICK STRAP

@ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4"

S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR FOOTING



ALUMINUM FRAME SCREEN WALL

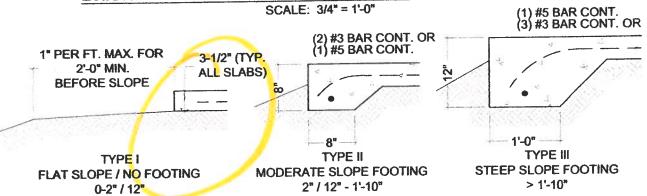
CAP BRICK

BRICK KNEEWALL TYPE 'S' MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO CONCRETE SLAB (SEE NOTES CONCERNING FIBER MESH)

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS



Notes for all foundation types:

- 1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
- 2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
- 3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure catagory in which case a type II footing is required.
- 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
- 5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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SECTION 1

SCREENED ENCLOSURES

Table 1.1 120

Allowable Spans for Primary Screen Roof Frame Members

Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered Tributary Load Width 'W' = Beam Spacing

Hollow Sections	3'-0"		4'-0"	•	5'-0"	•	6'-0'		7'-0"		8'-0"		90,	
	Allo	wabl	e Span 'l	. /	Point Loa	ed (P) or Unife	m	Load (U),					
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2°	РЪ	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7°	Pb	7'-7°	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud	19'-4"	υd	18'-6"	Ud	17'-9"	Ud
					ributary	Load	Width 7	N' =	Beam Sp	acin	9			
Self Mating Sections	3'-0'	•	4'-0'		5'-0'		6'-0'		7'-0"	_	8'-0"		30.	•
	Allo	wabl	e Span 1	2/	Point Lo	ad (P	or Unif	orm '	Load (U),	ben	ding (b),	defle	ection (d)	
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8°	Pd	11'-8"	Pd
2" x 5" x 0.050" x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud	15'-1"	Ud
2" x 6" x 0.050" x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3°	Ud	18'-5"	Ud	17'-8"	Ud
2" x 7" x 0.055" x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	23'-1"	Ud	21'-11"	Ud	20'-11"	Ud	20'-2"	Ud
2" x 8" x 0.072" x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5°	Ud	28'-7"	Ud	27'-2"	Ud	25'-11"	Ud	24'-11"	Ud
2" x 9" x 0.072" x 0.224"	39'-3"	Pd	35'-11"	Πq	33'-4"	Ud	31'-5"	Ud	29'-10"	Ud	28'-6"	Ud	27'-5"	υd
2" x 9" x 0.082" x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud	31'-11"	Ud	30'-7"	Ud	29'-5"	Ud
2" x 10" x 0.092" x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6°	Ud	34'-2°	Ud
				1	ributary	Load	Width 7	N' =	Beam Sp	acin	9			
Snap Sections	3'-0'	•	4'-0'		5'-0'		6'-0'		7'-0'		80.	•	3,-0,	•

				1	ributary	Load	Width 7	W' =	Beam Sp	acin	9			
Snap Sections	3'-0	*	4'-0'		5'-0'	•	6'-0'	l&	7'-0'	•	80,		3,-0,	•
	Allo	wabi	e Span 1	L' /	Point Lo	ad (P) or Unif	orm	Load (U),	ben	ding (b),	defle	ction (d)	
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud	20'-5"	Ud	19'-6"	Ud	18'-9"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud	23'-0"	Ud	22'-0"	Ud	21'-2"	Ud

Note:

- Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16. Structures larger than these limits shall have site specific engineering.
- Span is measured from center of beam and upright connection to fascia or wall connection.
- 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
- 6. Spans may be interpolated.
- 7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii. Example: Max. "L" for 2" x 4" x 0.050" hollow section with "W" = 5'-0" = 9'-1"

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Table 1.3 120 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 120 MPH, Exposure "B" or an applied load of 15 #sq. ft.

			1	rib	utary Lo	ad \	Width 'W	' =	Upright :	Spa	cing			
Hollow Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
			Allo	wa	ble Heig	ht "	H"/ ben	din	g (b), de	flec	tion (d)			
2" x 2" x 0.044"	6'-11"	Ь	5'-11"	ь	5'-3"	b	4'-9"	b	4'-4"	b	3'-11"	b	3'-8"	Ь
2" x 2" x 0.050"	7'-5"	d	6'-7"	ь	5'-10"	b	5'-3"	b	4'-10"	b	4'-5"	b	4'-2"	Ь
2" x 2" x 0.090"	8'-7"	d	7'-9"	đ	7'-4"	d	6'-8"	b	6'-1"	b	5'-8"	b	5'-4"	b
2" x 3" x 0.045"	7'-11"	d	7°-3°	d	7'-2"	d	6'-6"	d	5'-11"	b	5'-6"	ь	5'-2"	b
2" x 4" x 0.050"	10'-5"	b	8'-11"	b	7'-11"	b	7'-2"	b	6'-7"	b	6'-1"	b	5'-8"	Ь
2" x 5" x 0.062"	16'-0"	ь	13'-10"	b	12'-3"	b	11'-2"	b	10'-3"	b	9'-7"	b	8'-11"	ь

			1	rib	utary Lo	ad '	Width 'W	=	Upright S	Spa	cing			
Self Mating Sections	3'-0"		4'-0"		5'-0"		6'-0"	8	7'-0"		8'-0"		9'-0"	
			Allo	wa	ble Heig	ht "	H"/ ben	din	ıg (b), de	flec	ction (d)			
2" x 4" x 0.044 x 0.100"	11'-4"	d	10'-4"	d	9'-7"	d	8'-9"	b	8'-0"	b	7'-6"	b	6'-11"	b
2" x 5" x 0.050" x 0.100"	14'-0"	d	12'-9°	d	11'-10°	d	10'-10"	b	9'-11"	b	9'-3"	b	8'-8"	b
2" x 6" x 0.050" x 0.120"	16'-5"	d	14'-11°	d	13'-4"	b	12'-2"	b	11'-2"	b	10'-5"	b	9'-10"	b
2" x 7" x 0.055" x 0.120"	18'-9"	d	16'-3"	b	14'-6"	b	13'-2"	b	12'-2"	b	11'-4"	b	10'-8"	b
2" x 8" x 0.072" x 0.224"	23'-2"	d	21'-1"	d	19'-7"	d	18'-5"	d	17'-6"	d	16'-9"	d	16'-1"	d
2" x 9" x 0.072" x 0.224"	25'-5"	đ	23'-1"	d	21'-6"	d	20'-2"	d	19'-2"	d	17'-11"	b	16'-11"	b
2" x 9" x 0.082" x 0.310"	27'-4"	d	24'-10°	d	23'-0"	d	21'-8"	d	20'-7"	d	19'-8"	d	18'-11"	d
2" x 10" x 0.092" x 0.369"	31'-9"	đ	28'-10"	d	26'-9"	d	25'-2"	d	23'-11"	d	22'-10"	d	21'-11"	d

		Tributary Load Width W= Upright Spacing													
Snap Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	,	
•			Allo	wa	ble Heig	ht "	'H" / ben	dir	ıg (b), de	flec	tion (d)				
2" x 2" x 0.044"	6'-3"	d	5'-8"	đ	5'-4"	d	4'-11"	d	4'-6"	b	4'-2°	b	3'-10"	b	
2" x 3" x 0.045"	8'-6"	d	7'-8"	d	7'-2"	d	6'-6"	b	5'-10"	b	5'-4"	ь	4'-11"	Ь	
2" x 4" x 0.045"	10'-8"	đ	9'-8"	d	8'-6"	b	7'-7"	b	6'-10"	b	6'-3"	b	5'-9"	b	
2" x 6" x 0.062"	17'-5"	d	15'-10"	d	14'-8"	d	13'-10"	d	13'-1"	d	12'-7"	d	11'-11"	b	
2" x 7" x 0.062"	19'-8"	d	17'-10"	d	16'-7"	d	15'-7"	d	14'-10"	d	14'-1"	b	13'-2"	b	

Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 30' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
- 8. Spans may be interpolated.
- 9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Table 1.4 120 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 120 MPH, Exposure "B" or an applied load of 15 # / sq. ft.

9 4			-			
Δ	Sections	As Horizontals	Fastened To	Posts	With (Clips

				Tril	butary Lo	pad	Width Y	A. =	: Upright	Sp	acing			
Hollow Sections	3'-0"		4'-0"		5'-0"		60		7'-0"		80.,		9'-0"	_
(totton ocononio			Allowa	ble	Height	'H"	or Span	"L	"/ bend	ing	(b), defte	ecth		
2" x 2" x 0.044"	6'-11"	ь	5'-11"	b	5'-3"	b	4'-9"	Ь	4'-4"	b	3'-11"	Ь	3'-8"	Ь
2" x 2" x 0.050"	7'-5"	d	6'-7"	b	5'-10"	b	5'-3"	b	4'-10"	b	4'-5°	b	4'-2"	b
2" x 2" x 0.090"	8'-7"	d	7'-9"	d	7'-4"	d	6'-8"	b	6'-1"	Ь	5'-8"	Ь	5'-4"	ĮЬ
3" x 2" x 0.045"	7'-11"	d	6'-10"	b	6'-1"	b	5'-5"	b	4'-11°	b	4'-7°	b	4'-3"	b
3" x 2" x 0.070"	8'-11"	d	8'-1"	d	7'-3"	b	6'-6"	b	5'-11"	b	5'-7"	b	5'-2"	ΙÞ
2" x 3" x 0.045"	7'-11"	d	7'-3"	d	7'-2"	d	6'-6"	d	5'-11"	b	5'-6"	b	5'-2"	b
2" x 4" x 0.050"	10'-5"	b	8'-11"	b	7'-11"	b	7'-2"	b	6'-7"	b	6'-1"	b	5'-8"	IР
2" x 5" x 0.062"	16'-0"	Ь	13'-10"	b	12'-3"	ь	11'-2"	b	10'-3"	b	9'-7"	Ь	8'-11"	lp

	I	Tr	ibutary Load	Width W'	Upright Sp	acing	
Snap Sections	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	80.a	9'-0"
	100	Allowabl	e Height "H"			(b), deflect	on (d)
2" x 2" x 0.044"	6'-3" d	5'-8" d	5'-4" d	4'-11" d	4'-6" b	4'-2" b	3'-10" b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

				Tri	butary L	oad	Width V	N ' =	Upright	Sp	acing			
Hollow Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0°		8,-0,		9'-0"	,
			Allow	able	Height	"H"	or Span	"L	" / bend	Ing	(b), defle	ecti	on (d)	
2" x 2" x 0.044"	7'-9"	b	6'-8"	Ь	5'-10"	b	5'-3"	b	4'-10"	b	4'-5"	b	4'-2"	b
3" x 2" x 0.045"	8'-11"	b	7'-8"	Ь	6'-9"	b	6'-1"	b	5'-7"	b	5'-2"	b	4'-9"	b
3" x 2" x 0.070"	10'-8"	Ь	9'-1"	ь	8'-1"	b	7'-4"	b	6'-8"	b	6'-2"	b	5'-9"	b
2" x 3" x 0.045"	10'-7"	b	9'-1"	b	8'-1"	b	7'-3"	b	6'-8"	b	6'-2"	b	5'-9"	b
2" x 4" x 0.050"	11'-8"	ь	10'-0"	b	8'-10"	b	8'-0"	b	7'-4"	b	6'-10°	b	6'-4"	b
2" x 5" x 0.062"	17'-11"	Ь	15'-5"	b	13'-9"	b	12'-6"	Ь	11'-6°	b	10'-8"	b	10'-0"	b

2 X 3 X 4.002		1 14 4 14					
		Tri	ibutary Load	Width 'W'=	Upright Sp	acing	
Snap Sections	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
		Allowable	e Height "H"	or Span "L	"/ bending	(b), deflecti	on (d)
2" x 2" x 0.044"	8'-5" d	7'-2" b					4'-3" b

Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 30' in mean roof height.
- Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of $2^n \times 2^n \times 0.044^n$ min. and set @ 36^n in height are designed to be residential gardraits provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
- 8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
- 9. Spans/heights may be interpolated.
- 10. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

REVISED APRIL 2007

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Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright	Upright or	Minimum Purlin, Girt	Notes	Minimum Number of Screws*			Beam Stitching
or Post	Post/Beam	& Knee Brace Size		#8 x 1/2"	#10 x ½"	#12 x 1/2"	Screw at 24" OC
	2 x 3 SMB or H		Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H		Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H		Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H		Full Lap	14	12	10	#12
	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and	Gusset Plate Thickness		
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16 ^a	5/8"	2" x 7" x 0.055" x 0.120"	0.063*
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1º	2" x 9" x 0.072" x 0.224"	0.125°
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0306"	0.190°
5/16*	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250°
3/8"	1"	2 ^M		

^{* 0.082&}quot; wall thickness, 0.310" flange thickness

Connection Example:

 2^n x 7^n beam & 2^n x 5^n at beam & gusset plate, (14) #8 x $1/2^n$ sms & upright & gusset plate (14) #8 x $1/2^n$ sms ea. side of beam & upright.

Note:

- 1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
- 2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 3. The number of screws is based on the maximum allowable moment of the beam.
- 4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 5. Hollow splice connections can be made provided the connection is approved by the engineer.
- 6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
- 7. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
- 8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

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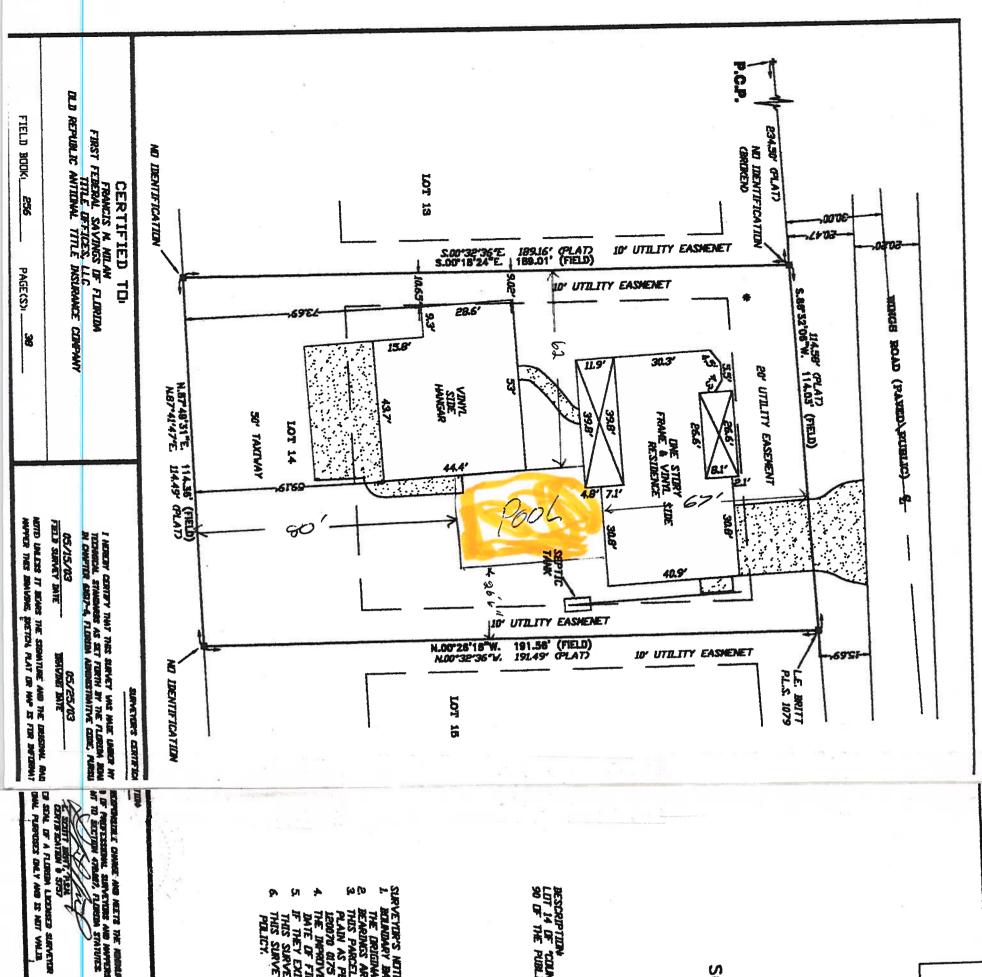
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^{** (1)} Stitching screw at 16" O.C. max.



EUF RECURDED IN PLAT BUIK 6, PAGE FLORIDA

11

30

1111

WELL

SATELLITE DISH

TELEPHINE BIX

ELECTRIC LINES

WIRE FENCE

CHAIN LINK FENCE

CHAIN LINK FENCE

A*X4* CONCRETE MONIMENT FOUND A*X4* CONCRETE MONIMENT SET IRON PIPE FOUND IRON PIN AND CAP SET POWER POLE WATER METER CENTERLINE

LEGEND

BOUNDARY SURVEY RANGE 17 EAST,

×

SECTION 7, COLUMBIA

TOWNSHIP 4 SOUTH, COUNTY, FLORIDA.

ACCURDANCE WITH THE RETRACEMENT OF REAL

ED III BE QUITSIBE THE 500 YEAR FLIXID TARY, 1988 COMMUNITY PANEL NUMBER E RATE MAPS ARE SUBJECT TO CHANGE, S SURVEY DRAYING ARE AS LOCATED ON TS AND/OR UTILITIES WERE LOCATED FOR

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SURVEYING

830 VEST DUVAL STREET LAKE GITY, FLURIDA 32055 (386)752-7163 FAX (386)752-5573 LAND SURVEYORS AND MAPPERS WORK ORDER # L-13769