

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID: ITXS8228Z0121142351

Truss Fabricator: Anderson Truss Company

Job Identification: 9-253--Fill in later MOSES -- , **

Truss Count: 38

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

Handwritten signature

Seal Date: 12/21/2009

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

-Truss Design Engineer-
Doug Fleming

Florida License Number: 66648

1950 Marley Drive

Haines City, FL 33844

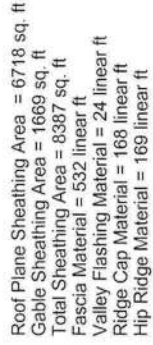
Details: A1101505-GBLLETIN-PB120-A1103005-BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	63092--CJ1		09355001	12/21/09
2	63093--HJ7		09355002	12/21/09
3	63094--CJ3		09355003	12/21/09
4	63095--EJ7		09355004	12/21/09
5	63096--M		09355005	12/21/09
6	63097--M2		09355006	12/21/09
7	63098--MGE		09355007	12/21/09
8	63099--C		09355008	12/21/09
9	63100--D		09355009	12/21/09
10	63101--D1		09355010	12/21/09
11	63102--D2		09355011	12/21/09
12	63103--DGE		09355012	12/21/09
13	63104--DPGE		09355013	12/21/09
14	63105--DP		09355014	12/21/09
15	63106--A13	✓	09355015	12/21/09
16	63107--A12	✓	09355016	12/21/09
17	63108--A11	✓	09355017	12/21/09
18	63109--A14	✓	09355018	12/21/09
19	63110--AP	✓	09355019	12/21/09
20	63111--A9	✓	09355020	12/21/09
21	63112--A8	✓	09355021	12/21/09
22	63113--A7	✓	09355022	12/21/09
23	63114--A1	✓	09355023	12/21/09
24	63115--A6	✓	09355024	12/21/09
25	63116--A2	✓	09355025	12/21/09
26	63117--A4	✓	09355026	12/21/09
27	63118--A3	✓	09355027	12/21/09
28	63119--A5	✓	09355028	12/21/09
29	63120--M-4		09355029	12/21/09
30	63121--M3		09355030	12/21/09
31	63122--M1		09355032	12/21/09
32	63123--AGE15	✓	09355038	12/21/09
33	63124--AGE	✓	09355033	12/21/09
34	63125--APGE		09355035	12/21/09
35	63126--M5		09355037	12/21/09
36	63127--GZA		09355036	12/21/09

#	Ref	Description	Drawing#	Date
37	63128--A10	✓	09355034	12/21/09
38	63129--CJ5		09355031	12/21/09

✓ = TRUSS WEB(S) BRACING REQ'D.





Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense

Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

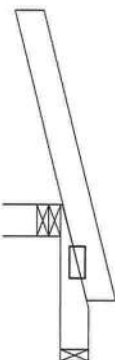
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

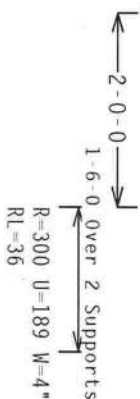
Roof overhang supports 2.00 psf soffit load.

3



R=90 Rm=92 U=56

2X4 (**) =



R=300 U=189 W=4"
RL=36

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY: 4

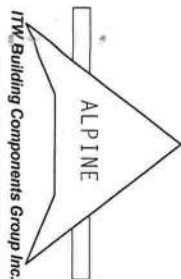
FL/-/4/-/R/-

Scale = .5"/Ft.

****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE MANUFACTURER OF THE THUSSES. 6000 ENTERPRISE LANE, MOHAWK, NY 13519 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE THUSSES IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF THUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (GALV/SS) ASH/ALUMINUM GRADE 40/60 (GALV/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF THUSSES AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2, 160B-2, 160C-2, 160D-2, 160E-2, 160F-2, 160G-2, 160H-2, 160I-2, 160J-2, 160K-2, 160L-2, 160M-2, 160N-2, 160O-2, 160P-2, 160Q-2, 160R-2, 160S-2, 160T-2, 160U-2, 160V-2, 160W-2, 160X-2, 160Y-2, 160Z-2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. THE SEAL ON THIS DRAWING INDICATES THE SUFFICIENCY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 63092
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCUR8228 09355001
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEON- 11992 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

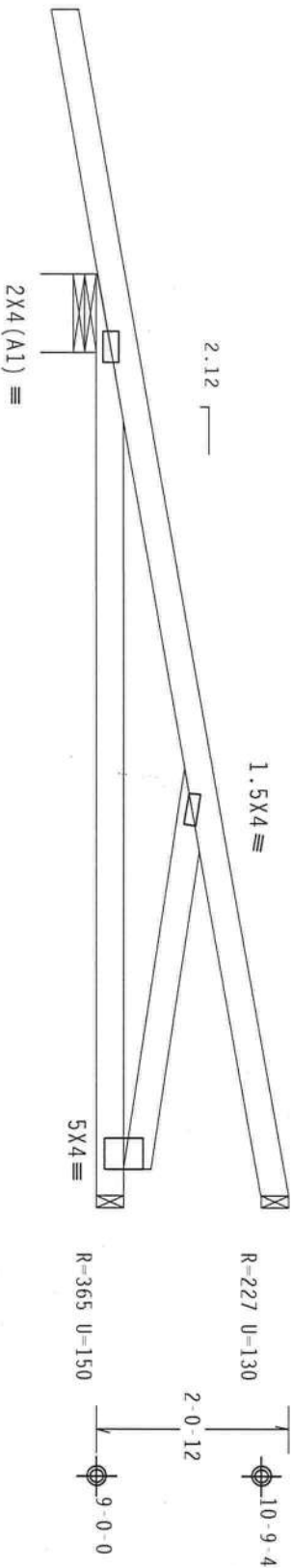
Hipjack supports 7'-0" setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)

FT/RT=10%(0%)/0(0)

9.02.00

QTY: 2

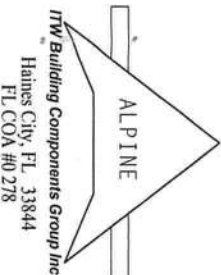
FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC. BY AIA/ASA AND TPI. CONNECTION PLATES ARE MADE OF 2018/16GA (OR HIGHER) ASTM A653 GRADE 40/50 (Q. KYN. 55) GALV. STEEL. APPLY 2. ALL TRUSS AND CHORD MEMBERS AND UNLESS OTHERWISE SPECIFIED OR NOTED ON THIS DESIGN, SECTION PER DRAWING OR TITW BCG. ALL TRUSS AND CHORD MEMBERS AND UNLESS OTHERWISE SPECIFIED OR NOTED ON THIS DESIGN, SECTION PER DRAWING OR TITW BCG. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2.



FL/-/4/-/-/R/-		Scale = .5"/Ft.	
TC LL	20.0 PSF	REF	R8228- 63093
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355002
BC LL	0.0 PSF	HC-ENG TCE/DF	
TOT. LD.	40.0 PSF	SEON-	62577
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

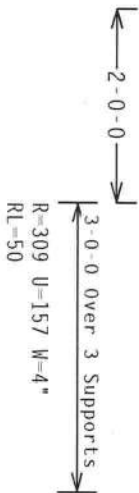
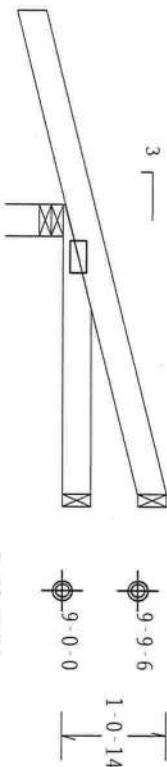
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf, $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:4

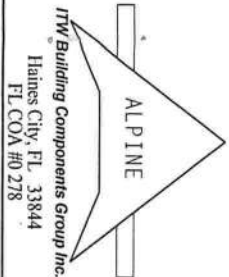
FL/-/4/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100
N. 10TH ST., SUITE 100, WISCONSIN, WI 53091) FOR SAFETY PRACTICES BEFORE PROCEEDING. UNLESS
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES BEFORE PROCEEDING. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN CONFORMANCE WITH
TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA) AND TPI. ITW BCG
CONNECTION PLATES ARE MADE OF 20/18/16GA (W/H/S/S) ASTM A653 GRADE 40/60 (W, W/H, S/S) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE FOR THE TRUSS COMPONENT
DESIGN & INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN & INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
BUILDING DESIGNER PER AIA/TPI 1 SEC. 2.



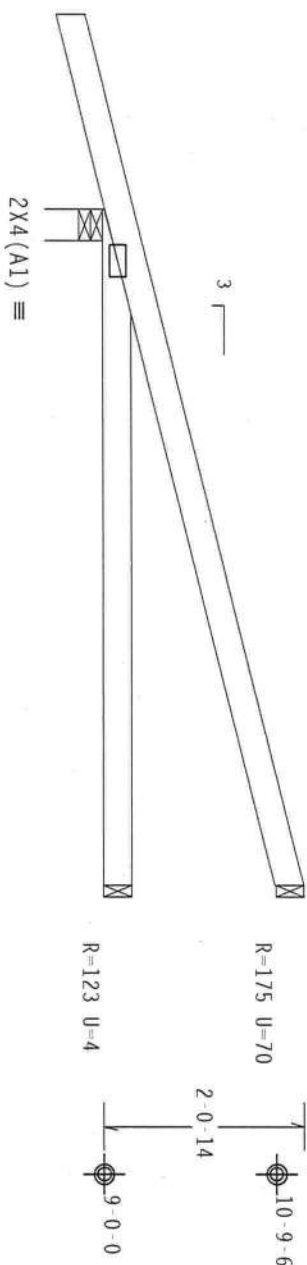
TC LL	20.0 PSF	REF R8228- 63094
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCUR8228 09355003
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEON- 62565
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Provide (2)	16d common nails (0.162"x3.5")	toe nailed at Top chord
Provide (2)	16d common nails (0.162"x3.5")	toe nailed at Bot chord



Scale = .5" / Ft.

2.00
DOUGLAS FLEMING
LICENSE
No. 66648
QTY



ALPINE

HW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

TC LL	20.0 PSF	REF	R8228- 63095
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCU8R8228 093550.
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEON-	62573
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228Z01

JREF - 1TXS8228Z01

THESE ARE THE RESULTS OF THE STUDY OF THE

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TCDL=5.0 psf, wind BC DL=5.0 psf $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

MFERS loads based on trusses located at least 7.50 ft. from roof



9.02.00

QTY:21 FL/-/4/-/-/R/-

Scale = .5"/Ft.

02.00
DOUGLAS FLEMING
LICENSE
No. 66648
QT

TC LL	20.0 PSF
TC DL	10.0 PSF

REF	R8228 - 63097
DATE	12/21/09

2004



ALPINE

Haines City, FL 33844
FL COA #0278

[illegible]

21.09

DUR.FAC.	1.25
SPACING	24.0"

JREF- 1TXS8228701

THIS WORK PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KUSS MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MFRS pressures.

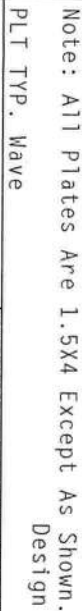
Right end vertical not exposed to wind pressure.

See DWG5 A11015050109 & GBLETTIN0109 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets $L/240$ live and $L/180$ total load.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.



BC2007Res/TP1-2002(Std)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:2

FL/-/4/-/-/R/-

Scale = .5"/Ft.

2.00
DOUGLAS FLEMING
LICENSE
No. 66648
QTY



ALPINE

Haimes City, FL 33844
FL COA #0278

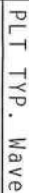
21 '09

TC LL	20.0 PSF	REF	R8228- 63098
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCHSR8228 09355007
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	62676
DUR.FAC.	1.25		
SPACING	24.0"	UREF-	1TXS8228Z01

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load.



9.02.00

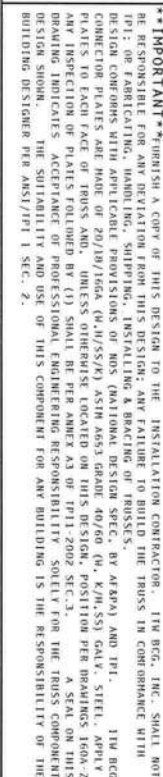
Scale = .5"/Ft.

****IMPORTANT**** OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR REVISIONS FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE OF FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

STATE OF

BC LL 0.0 PSF

HC-ENG TCE/DF



JREF- 1TXS8228Z01

(9-253--F111 in later MOSES --, ** (D)

Top chord 2x4 SP #2 Dense :12, 14 2x6 SP #2:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

:Lt Wedge 2x5 SP #3::Rt Wedge 2x5 SP #3:

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

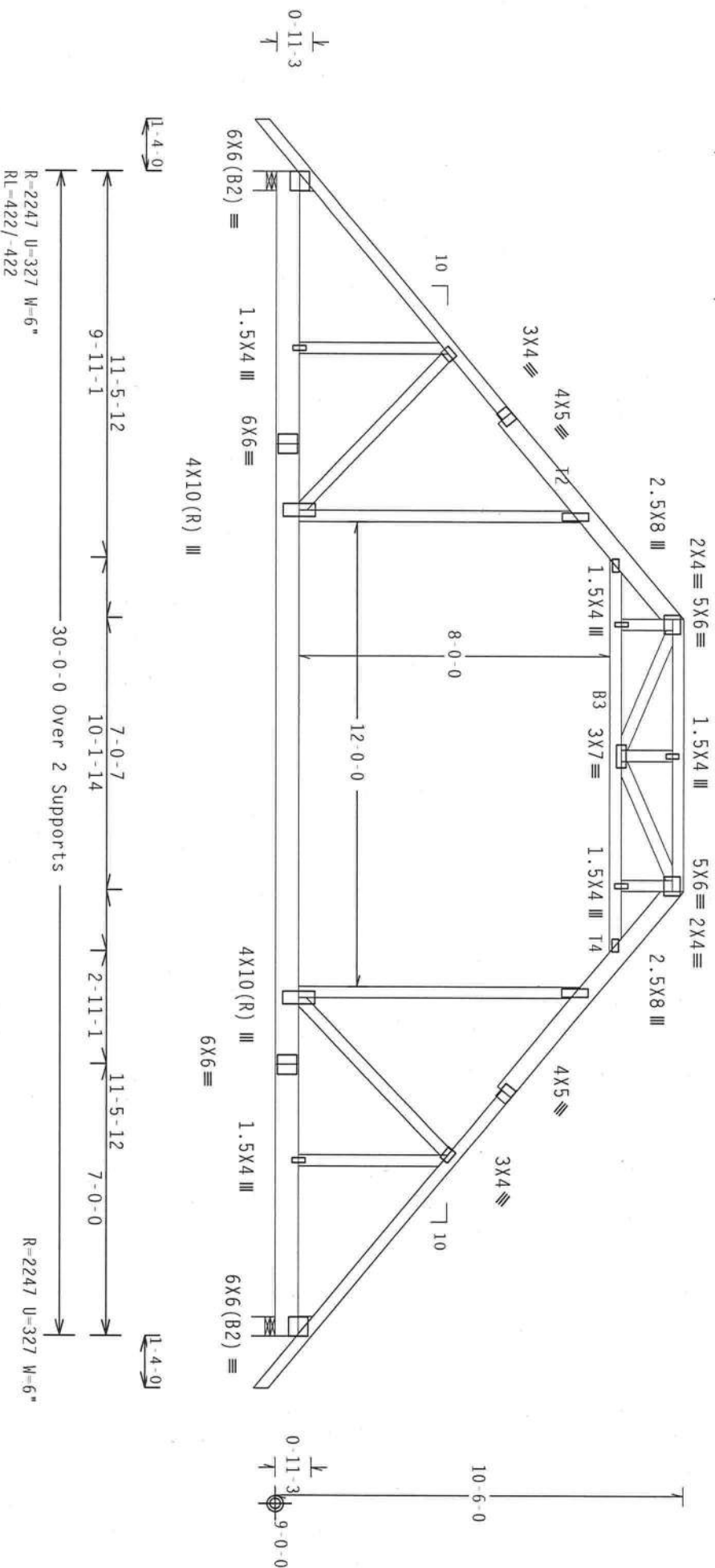
Deflection meets L/360 live and L/240 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 9-0-0 to 21-0-0.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

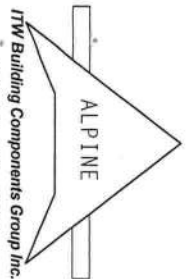
9.02.00

QTY: 10 FL/-/4/-/R/-

Scale = .25"/ft.

****WARNING**** THESE REQUIRED EXTREME CARE IN FABRICATING, MAINTAINING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICK (6000) TRUSS COMPANY OF AMERICA, 6000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICK (6000) TRUSS COMPANY OF AMERICA, 6000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF R8228- 63100
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCURS8228 09355009
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEON- 12011 REV
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228201

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110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpl(+/-)=0.18

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24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 9-0-0 to 21-0-0.



Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:5

Scale = .25" / ft.

2.00
DOUGLAS FLEMING
LICENSE
No. 66648
QT

TC LL	20.0 PSF	REF	R8228 - 63101
TC DL	10.0 PSF	DATE	12/21/09
RC DL	10.0 PSF	DBW	HCHSP8228 09355010

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

[illegible]

A circular professional engineer seal for Douglas Fleming, License No. 66648, State of Florida. The seal features the text "DOUGLAS FLEMING" at the top, "LICENSE" on the right, "No. 66648" in the center, "STATE OF FLORIDA" on the left, and "PROFESSIONAL ENGINEER" at the bottom. A signature is written over the license number. The seal is stamped on a document with a date stamp "2.00" and "01" at the top right.

TC LL	20.0 PSF	REF	R8228 - 63101
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355010
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	12013 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228Z01

~~2 COMPLETE TRUSSES REQUIRED~~

2 COMPLETE TRUSSES REQUIRED

Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.

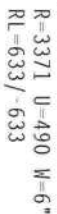
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 36.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC.

5X6 ≡ 9X9



Design Crit: FBC2007Res/TPI-2002(STD)

$$FT/RT=10\%(0\%)/0(0)$$

FL/-/4/-/-/R/-/

Scale = .25" / Ft.

DOUB
LICENSE
MING
No. 66648

ITW Building Components Group Inc.

Haines City, FL 33844
FL CO #0370

FL COA #0 278



TC LL	20.0 PSF	REF	R8228- 63102
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355011
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	12016 REV
DUR.FAC.	1.25		
SPACING	36.0"	JREF-	1TXS8228Z01

Top chord	2x4	SP	#2	Dense	:T2,	T4	2x6	SP	#2:
Bot chord	2x8	SP	#1	Dense	:B3,	2x4	SP	#2	Dense:

Roof overhang supports 2.00 psf soffit load.

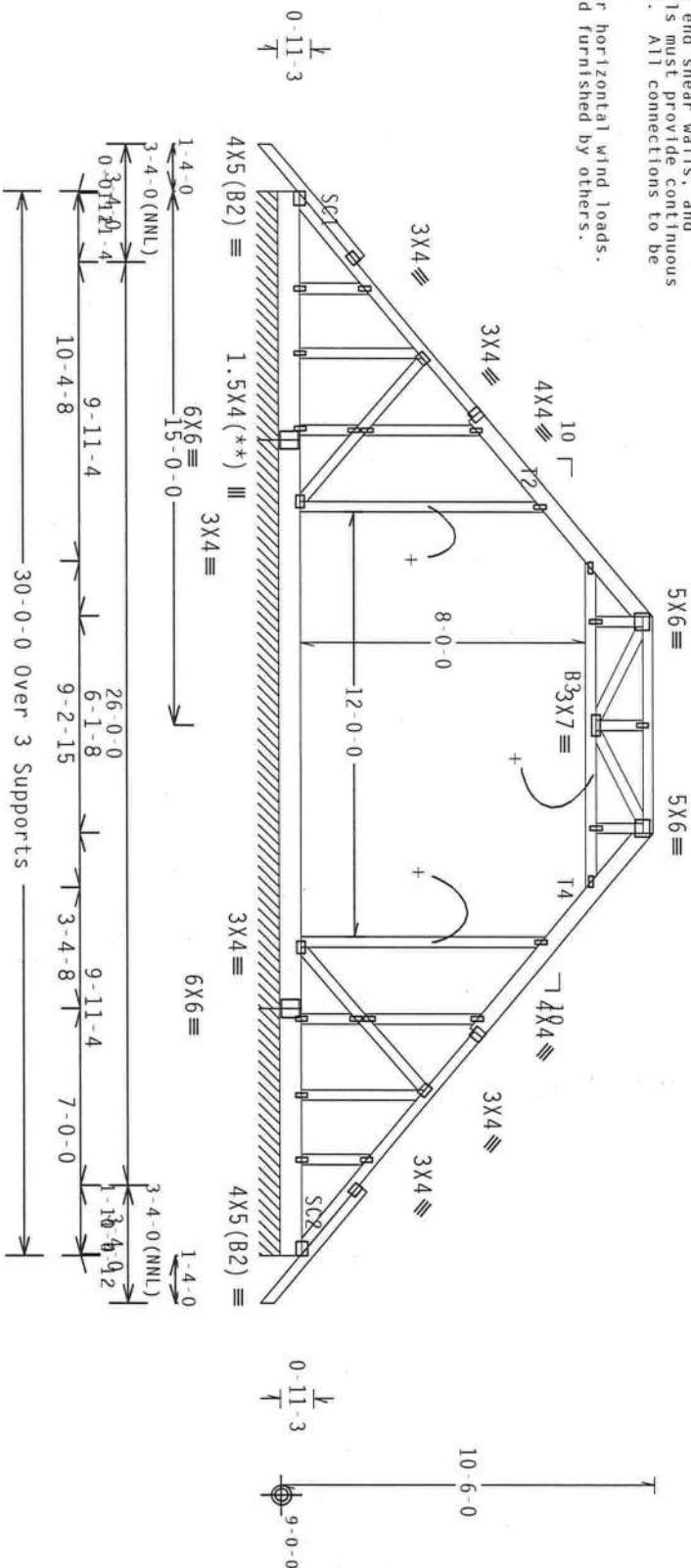
Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL).
Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates, 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 9-0-0 to 21-0-0.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

+ Member to be laterally braced for horizontal wind loads. Bracing system to be designed and furnished by others.



R=278 PLF U=34 PLF W=7-0-0
RL=68/-68 PLF R=183 PLF U=16 PLF W=16-0-0

R=285 PLF U=33 PLF W=7-0-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

$$FT/RT=10\%(0\%)/0(0)$$

9.02.00

QTY:2

FL/-/4/-/-/R/-

Scale = .1875"/ft.

BEER TO BEST BUILDING CONFORMANCE SAFETY INFORMATION), PROVIDED BY THE TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ARLINGTON, VA, 22204 AND AMERICAN TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LANE, MADISON, NJ 07093 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DISSESSMENT INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CHORDS.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 63103
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCU\$R8228 09355012
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	62767
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.78 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=2.0 psf, 1W=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

See DWGS A11030050109 & GBLLETT10109 for more requirements.

The Building Designer is responsible for the design of the
roof and ceiling diaphragms, gable end shear walls, and
supporting shear walls. Shear walls must provide continuous
lateral restraint to the gable end. All connections to be
designed by the Building Designer.

Special loads

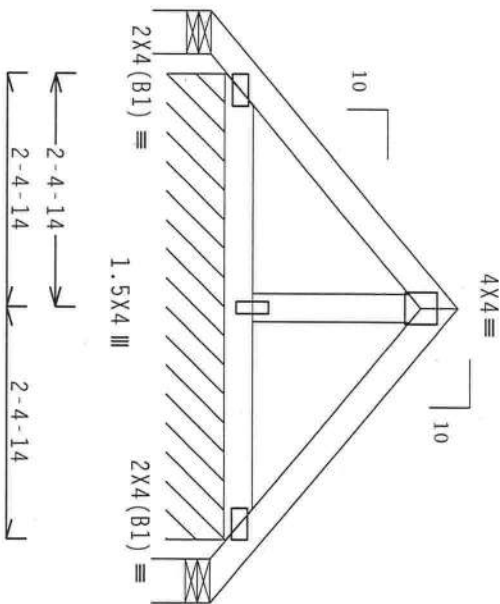
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 66 plf at 0.00 to 66 plf at 3.06
TC - From 66 plf at 3.06 to 66 plf at 6.13
BC - From 4 plf at 0.00 to 4 plf at 6.13

Truss spaced at 24.0" OC designed to support 1-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.

Deflection meets L/240 live and L/180 total load.

Refer to DWG PB1200109 for piggyback details.

For piggyback being used for gable ends, Gable Details
must also be applied as well as Piggyback Details.



19-6-12

2-4-14

R=42 RW=86 U=85 W=5.467"
R=42 RW=32 U=19 W=5.467"
RL=98/98 R=109 PLF U=110 PLF W=4-9-13

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:2 FL/-/4/-/-/R/-

Scale =.5"/ft.

****WARNING**** THESE REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE NATIONAL INSTITUTE OF CONSTRUCTION, 6300
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION), 5300
ENTERPRISE LANE, MODISON, WI 53119 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD TO THE TRUSS IN CONFORMANCE WITH
TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OR NOT GOVERNED BY SPEC. OF AISC AND TPI. THE BCG
CONNECTION PLATES ARE MADE OF 70/18/16GA (GALVANIZED) ASTM A653 GRADE 40/60 (40, 42, 55) GALV. STEEL. APPLY
THE BCG CONNECTION PLATES TO THE TRUSS AND, UNLESS OTHERWISE SPECIFIED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.
ANY INSPECTION OF QUALITY OF TRUSS AND, UNLESS OTHERWISE SPECIFIED ON THIS DESIGN, POSITION PER DRAWINGS T60A-2.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. THE BCG, INC. SHALL NOT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.

ALPINE



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 63104
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355013
BC LL	0.0 PSF	HC-ENG TCE/DF	
TOT.LD.	40.0 PSF	SEON-	62764
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228201

(9-253--Fill in later MOSES --, ** - DP)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.97 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=2.0 psf. Iw=1.00 GCPI(+/-)=0.18

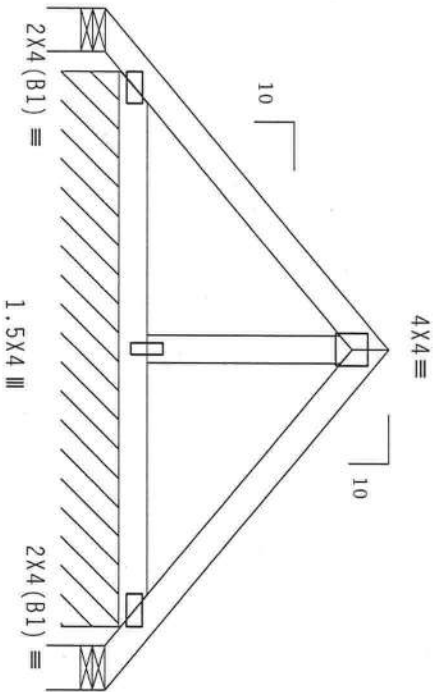
Wind reactions based on MMFRS pressures.

Refer to DWG PB1200109 for piggback details.

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 66 plf at 0.00 to 66 plf at 3.52
TC - From 66 plf at 3.52 to 66 plf at 7.04
BC - From 4 plf at 0.00 to 4 plf at 7.04

Deflection meets L/240 live and L/180 total load.



19-6-12

2'-9" 7

2'-10-6
2'-10-6
7'-0-7 Over 3 Supports
R=20 Rw=97 U=95 W=5.467"
RL=102/-102 R=87 PLF U=49 PLF W=5-8-12
R=20 Rw=30 U=27 W=5.467"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

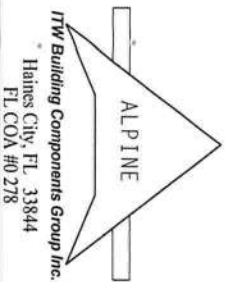
QTY: 16 FL/-/4/-/R/-

Scale = .5"/Ft.

****WARNING**** THUSSES ROUTED EXTERIOR CASE IN FABRICATION, MANUFACTURING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF THUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF 805 (ADDITIONAL DESIGN SPEC. BY ALKRA) AND TPI. PRODUCTION PLATES ARE MADE TO 2018/1604 (PLATE) ASH A653 GRADE 40/60 (Q, R, M, S) GALV. STEEL. APPLY TO ALL THUSSES. THIS DESIGN IS THE PROPERTY OF ITW BUILDING COMPONENTS GROUP INC. NO PART OF THIS DESIGN MAY BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, WITHOUT PERMISSION IN WRITING FROM ITW BUILDING COMPONENTS GROUP INC. THE ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



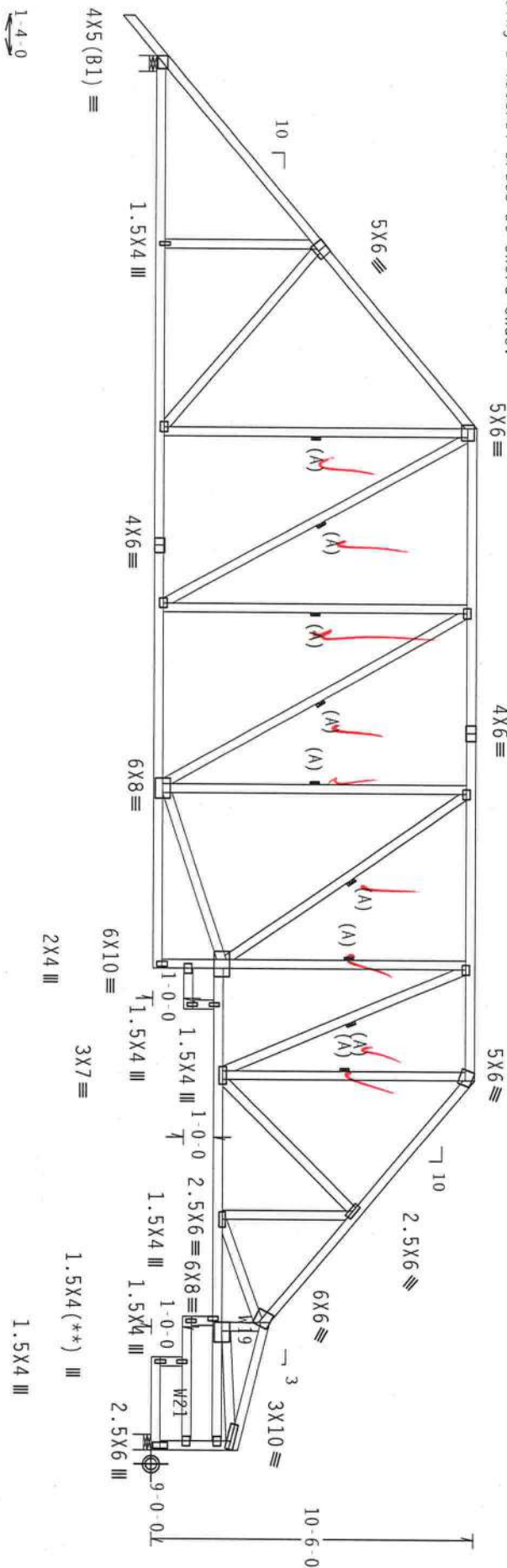
TC LL	20.0 PSF	REF R8228- 63105
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCURSR8228 09355014
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT. LD.	40.0 PSF	SEON- 63361
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228Z01

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.



110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI (+/-)=0.18

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

WUERS loads based on trusses located at least 15.00 ft. from roof edge.

9.02.00

QTY:3

FL/-/4/-/-/R/-/

Scale = .1875"/Ft.

WARNING: THESE RIGGING EXERCISE CASE IS FABRICATION, HANDLING, HOISTING, DRIPPING, INSTALLING, AND PRACTICING. REFER TO GC-1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE GIPSS PAPER INSTITUTE, 210 HORTON LANE, SUITE 312, ALEXANDRIA, VA, 22314 AND GC-2000 (TRUSS CONSTRUCTION), 6500 ENTERPRISE LANE, MONTICELLO, UT 84701 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. DANGERS NOT PREVIOUSLY INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228 - 63106
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355015
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	62863
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228Z01

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

 $3 \times 4 =$

edge.



RL=343/-364

R=2063 U=175 W=6"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0)$$

9.02.00

QTY: 2

FL/-/4/-/-/R/-/

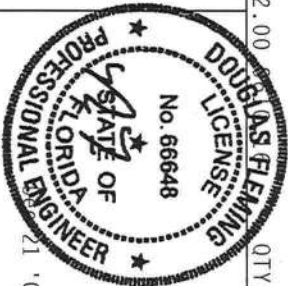
Scale = .1875"/Ft.

OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 63107
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355016
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	62923
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{cp1}(+/-)=0.18$

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

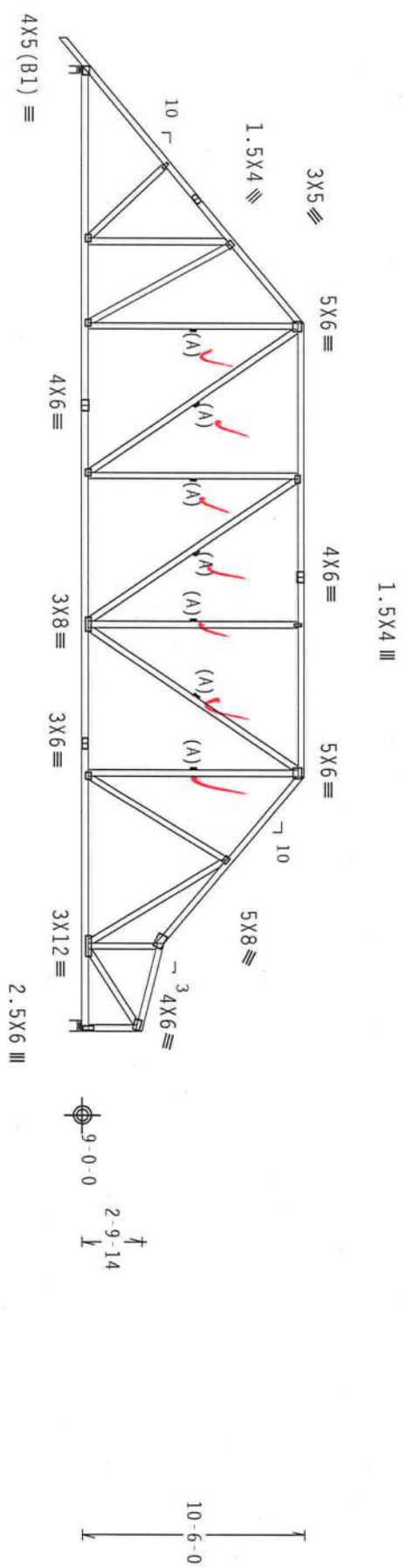
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



1-4-0
12-1-7
21-5-2
7-11-10
4-1-13
R=2008 U=198 W=4"
RL=343/-364
R=1877 U=176 W=6"

Note: All Plates Are 3x4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)
9.02.00

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

PLT TYP. Wave

QTY: 2

FL / - / 4 / - / - / R / -

TC LL	20.0 PSF	REF R8228- 63108
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCUR8228 09355017
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT. LD.	40.0 PSF	SEON- 62929
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1TXS8228Z01

Scale = .125"/Ft.

DOUGLAS, FLEMING

FLORIDA

PROFESSIONAL ENGINEER

No. 66648

21 '09

A14)

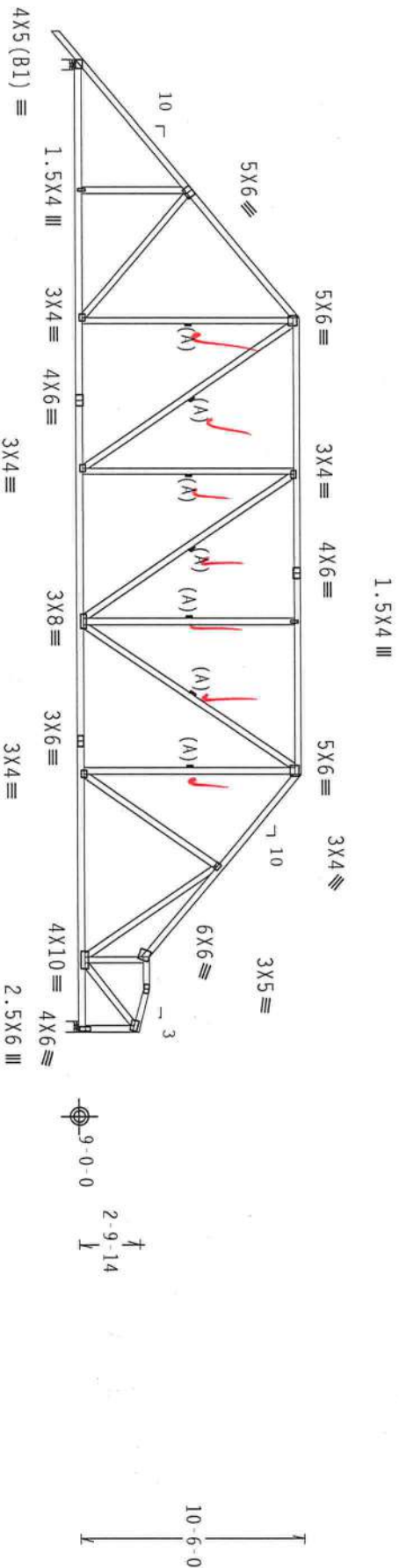
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT 1L, EXP C, wind TC, DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)-0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

WMERS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(Std)
FT/RT=10%(0%)/0(0)

QTY: 1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

****WARNING**** THROUS, REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING PER TO DCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PFI (GIBBS PATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AISC THROUS, COUNCIL OF AMERICA, ENTERPRISE LANE, MOUNTAIN, NJ 07071) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

2.00
DOUGLAS FLEMING
LICENSE
No. 66648
CITY

TC LL	20.0 PSF	REF	R8228- 63109
TC DL	10.0 PSF	DATE	12/21/09
RC DI	10.0 PSF	DDW	HCUSP8228 00355018

REF	R8228 - 63109
DATE	12/21/09
DBL	HCUCB8228 00355018

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

[illegible]

STATE OF
FLORIDA
PROFESSIONAL ENGINEER

BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 63037
DUR.FAC.	1.25	
SPACING	24.0"	JREF- ITXS8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

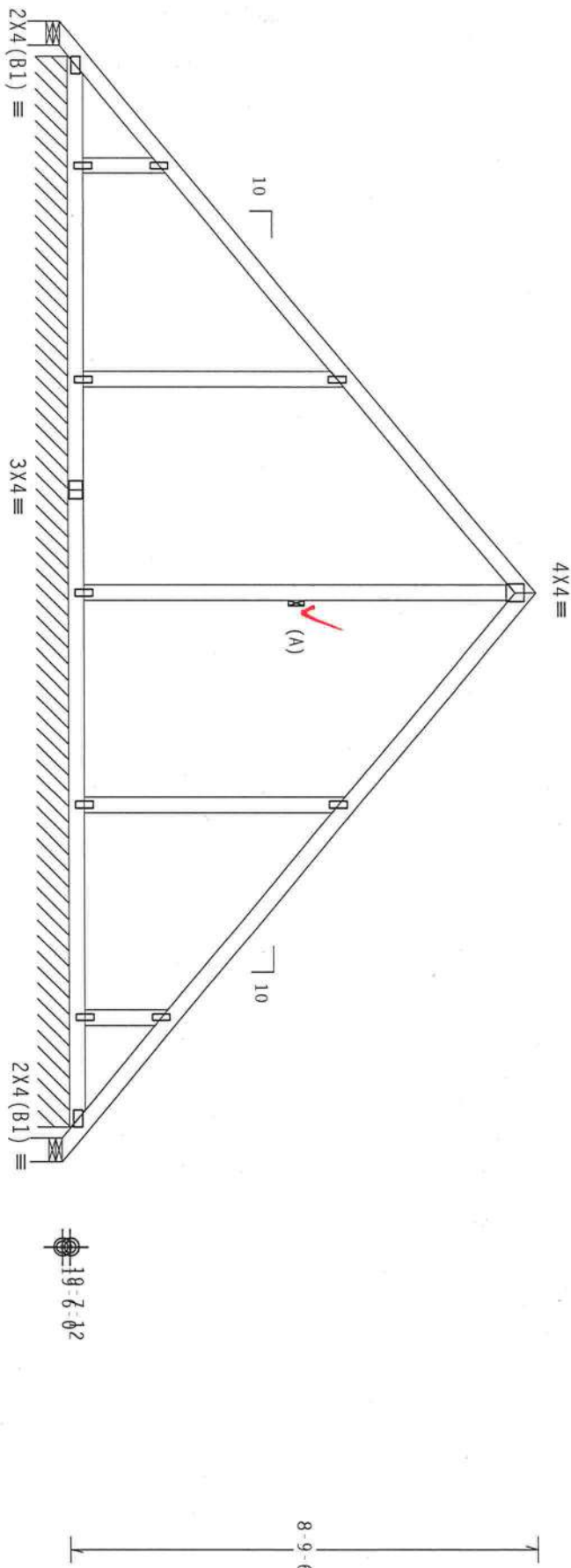
Deflection meets L/240 live and L/180 total load.

Refer to DWG PB1200109 for piggyback details.

110 mph wind, 23.96 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=2.0 psf. $I_w=1.00$ $GCP(+/)=0.18$

Wind reactions based on MMFRS pressures.

Truss passed check for 20 psf additional bottom chord live load
in areas with 42" high x 24" wide clearance.



R-26 RW=230 U=220 W=5.467"
RL=326/-326

R-85 PLF U=42 PLF W=20-1-6

R-25 U=4 W=5.468"

Note: All Plates Are 1.5x4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
PLT TYP. Wave

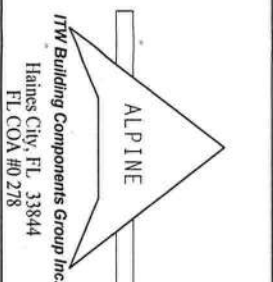
****WARNING**** TRUSSES BEARING EXTERIOR LOAD IN FABRICATION, MARKING, SHIPPING, INSTALLING AND BRACING
SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE FABRICATOR. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE
CORRECTNESS OF THE DESIGN. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF THE DESIGN.
ENTERPRISE LANE, MADISON, WI 53717 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED FIELD CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE FABRICATOR. THE FABRICATOR SHALL BE RESPONSIBLE FOR THE
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ENTERPRISE LANE, MADISON, WI 53717 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED FIELD CEILING.

9.02.00

QTY: 28 FL/-/4/-/-/R/-

Scale = .3125"/Ft.

			
ITW Building Components Group Inc.	Haines City, FL 33844	FL COA #0278	
TC LL	20.0 PSF	REF	R8228- 63110
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355019
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT. LD.	40.0 PSF	SEQN-	62962
DUR. FAC.	1.25		
SPACING	24.0"	UREF-	1TXS8228Z01

(A9)

** Negative reaction(s) of -530# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets $L/240$ live and $L/180$ total load.

Shim all supports to solid bearing.
MFERS loads based on trusses located at least 15.00 ft. from roof edge.



Scale = .125"/Ft.

2.00
OT
DOUGLAS FLEMING
LICENSE
No. 66648



ALPINE

Haines City, FL 33844
FL COA #0278

[illegible]

TC LL	20.0 PSF	REF	R8228- 63111
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355020
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEON-	71605
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228Z01

(9-253--F111 in later MOSES . ** (A8))

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

** Negative reaction(s) of -540# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 Gcpi(+/-)-0.18

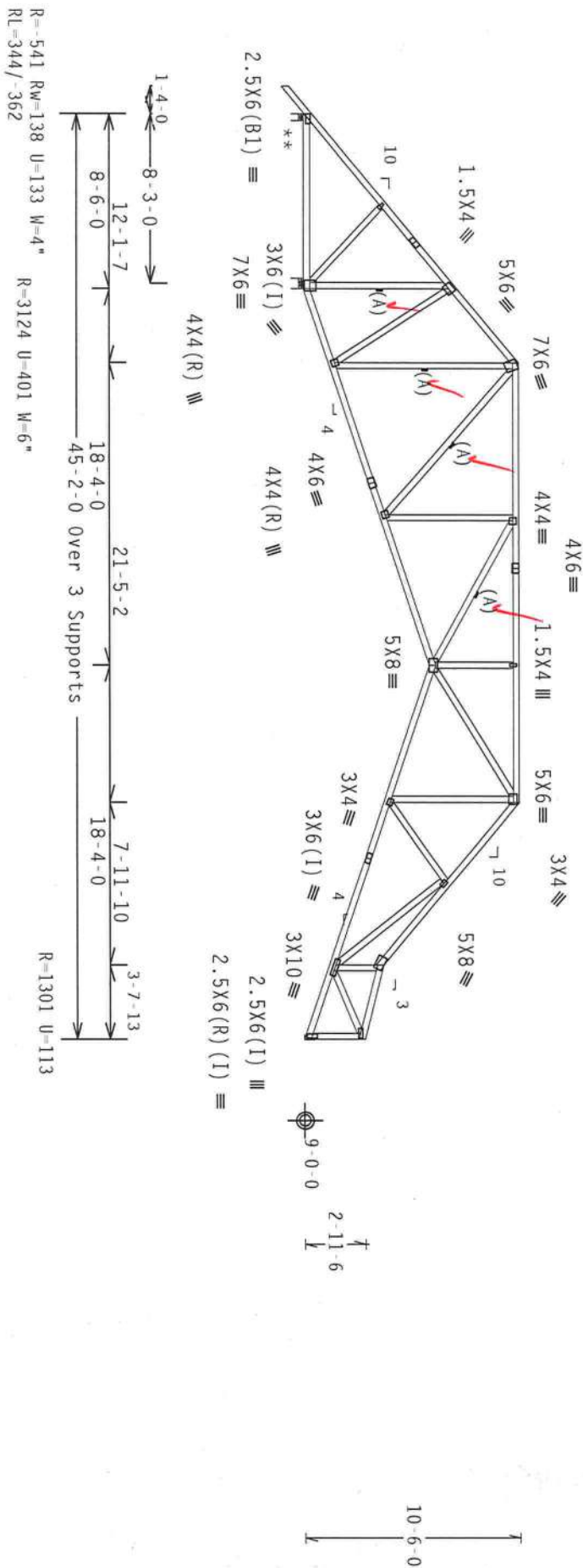
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0%)

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRING EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY. THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY. THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY.

A PROPERLY ATTACHED RIGID CEILING.

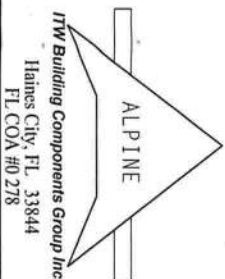
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY.

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THE TRUSS COMPANY SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPANY.



TC LL	20.0 PSF	REF	R8228- 63112
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 0935021
BC LL	0.0 PSF	HC-ENG TCE/DF	
TOT. LD.	40.0 PSF	SEQN-	71611
DUR. FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228Z01

A7

110 mph wind, 15.14 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT 11, EXP C, wind IQ DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 7.57 ft. from roof edge.



Design Crit: FBC2007Res/TP1-2002(STD,
FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0)$$

QTY: 1

FL/-/4/-/R/-

Scale = .1875"/ft.

[illegible]

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278



60

TC LL	20.0 PSF	REF R8228 - 63113
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCUR8228 0935022
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 63024
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228Z01

JREF - 1TXS8228Z01

A6)

110 mph wind, 15.14 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, Wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpl(+/-)=0.18

Wind reactions based on MFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

$$\overline{FT/RT} = 10\% (0\%) / 0 (0)$$

FL/-/4/-/-/R/-/

Scale = .1875"/ft.

DOUGLAS
LICENSE
No. 66648

7

OFFICE OF
THE
ATTORNEY GENERAL
STATE OF NEW YORK



21

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JREF - 1TXS8228Z01

110 mph wind, 15.14 ft mean hgt, ASCE 7-05, closed bldg, not located within 6.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcp(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load.

laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.



Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0)$$

QTY:1

FL/-/4/-/-/R/-/

Scale = .1875"/ft.

WARNING: THESE PANELS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRIVING. REFER TO BEST AVAILABLE ENGINEERING INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND TPI (TRUSS PLATE INSTITUTE) OF AMERICA, 6300 UNIVERSITY DRIVE, SUITE 100, ALEXANDRIA, VA. 22304. THESE PANELS ARE NOT TO BE USED FOR ANY OTHER PURPOSES. THE ENGINEER/DESIGNER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHANGING.

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THIS, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TROSS'S.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DRAWING, POSITION PER DRAWINGS 160A. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF T11-1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS.

DESIGN SHOW. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 63116
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355025
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	63077
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXSR8228Z01

A4)

110 mph wind, 15.14 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1w=1.00 GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.14" due to live load and 0.15" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

24" OC.

WMFR loads based on trusses located at least 7.57 ft. from roof edge.

edge.



Design Crit: FBC2007Res/TPI-2002(STD)

$$FT/RT=10\%(0\%)/0(0)$$

QTY:1

Scale = .1875"/Ft.

SHALL HAVE

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 63117
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355026
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	63115
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1TXS8228201

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110 mph wind, 15.14 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI (+/-)=0.18

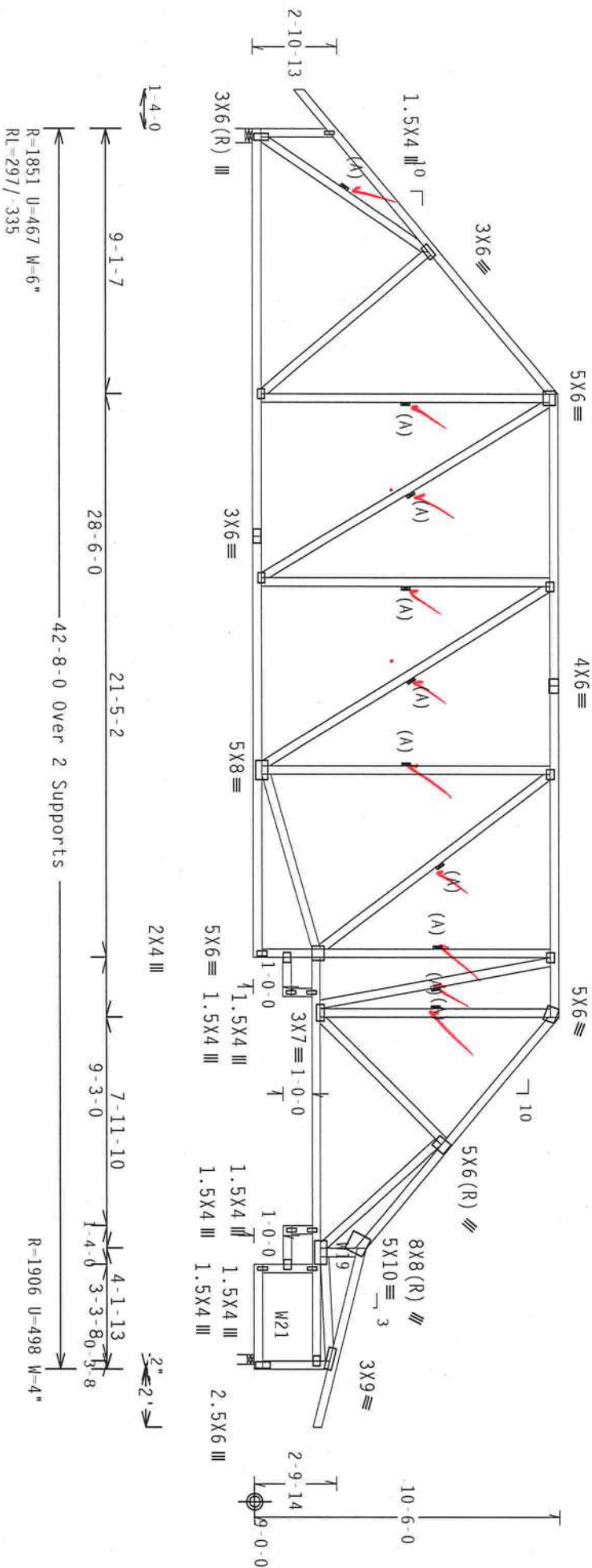
Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.14" due to live load and 0.15" due to dead load.

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24" OC.

MFERS loads based on trusses located at least 7.57 ft. from roof



Design Crit: FBC2007Res/TPI-2002(STD)

$$FT/RT=10\%(0\%)/0(0)$$

9.02.00

QTY:1

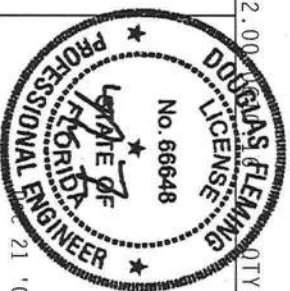
FL/-/4/-/-/R/-

Scale = .1875"/Ft.

[illegible]

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FLCOA #0278



21 '09

TC LL	20.0 PSF	REF	R8228- 63118
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355027
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	63120
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228201

A5)

110 mph wind, 15.14 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

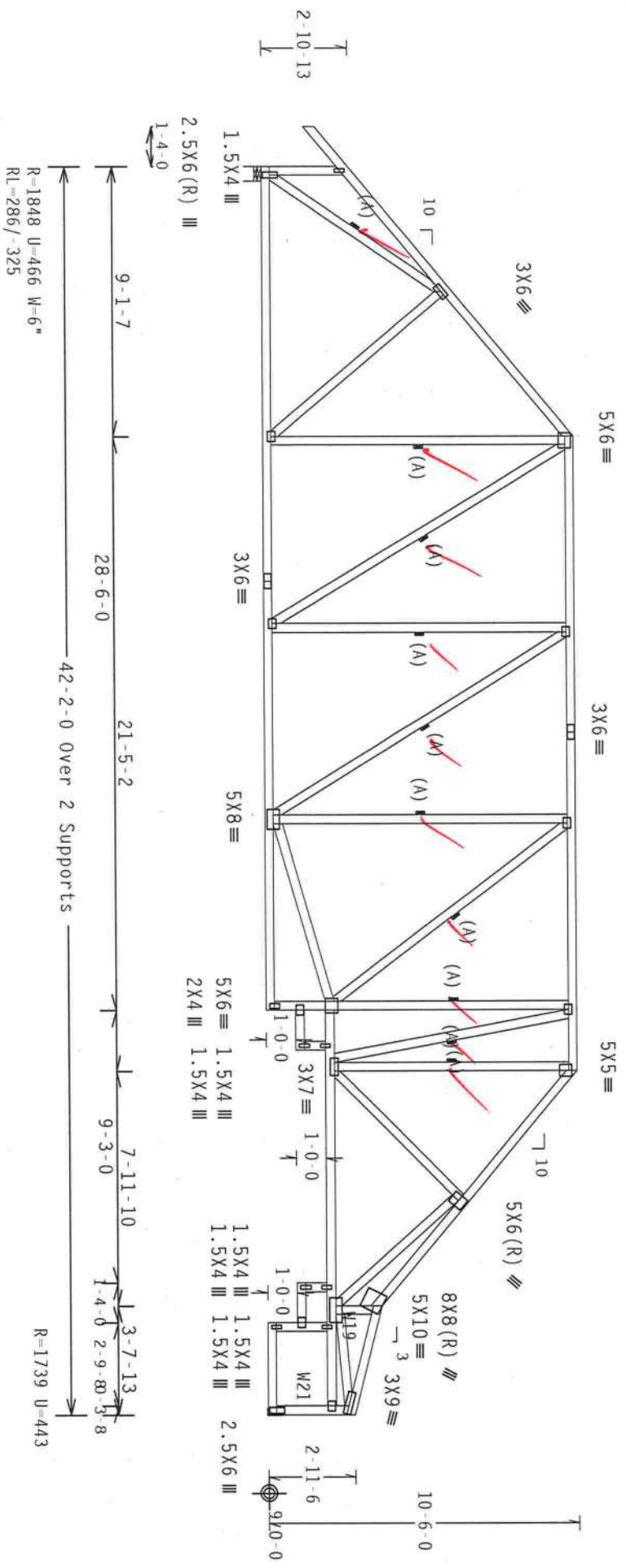
Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.15" due to live load and 0.16" due to dead load.

Review of structural models was running to brace all flat TC @

24" OC.

edge loads based on crusses located at least 1.5 ft. from roof

Design Crit: FBC2007Res/TP1-2002(STD)
ET/ST 100(0%) 10(0%)
$$FT/RT=10\%(0\%)/0(0)$$

QTY:1

$$\frac{FL}{-/4/-/-/R/-}$$

Scale = .1875"/Ft.

[illegible]

* **IMPORTANT** * Furnish a copy of this design to the installation contractor. ITW BCG, Inc. shall not be responsible for any errors or omissions, and failure to build the fence in accordance with this design.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AHS CHARTERED DESIGN SPEC., BY AFAPA AND TPI, TP1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTION PLATES TO THE TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/ISO 1 SEC. 2.



21.09

TC LL	20.0 PSF	REF	R8228- 63119
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355028
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT. LD.	40.0 PSF	SEQN-	63139
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1TXS8228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

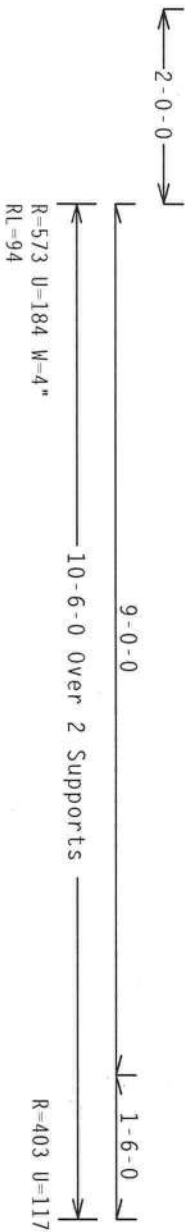
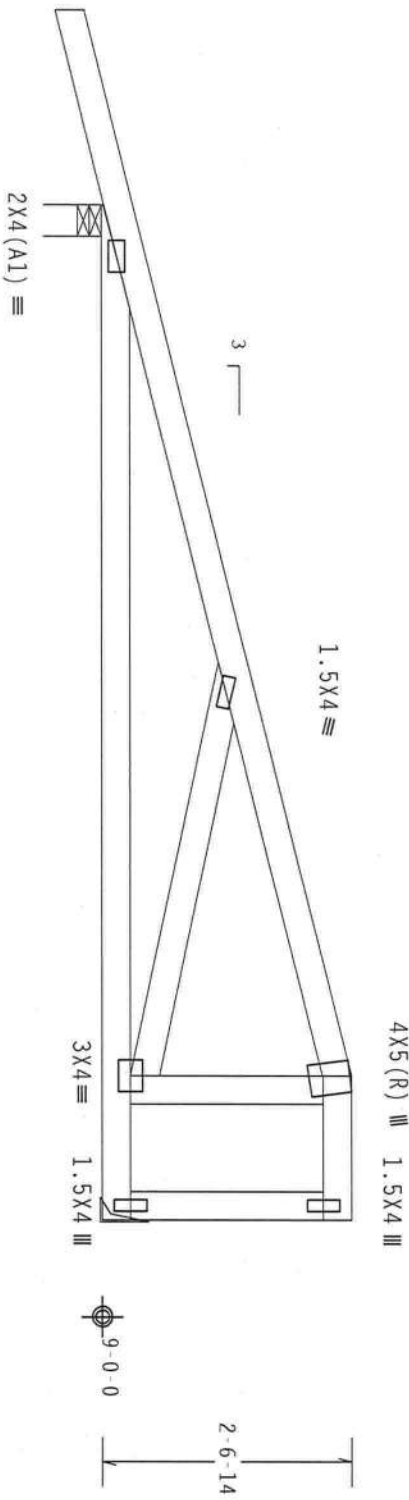
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)-0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:2

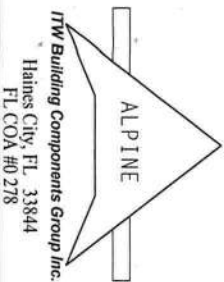
FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. BY ACPA AND TPI.

DESIGN CONTRACTORS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. PRODUCTION PLACES THE BCG, INC. (OR ITS SUBSIDIARY, BCG TRUSS SYSTEMS, 110 BCG DRIVE, SUITE 312, ALEXANDRIA, VA, 22314) IN A POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 63121
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355030
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT. LD.	40.0 PSF	SEON-	63193
DUR. FAC.	1.25		
SPACING	24.0"		
		JREF -	1TXS8228Z01

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpl(+/-)=0.18

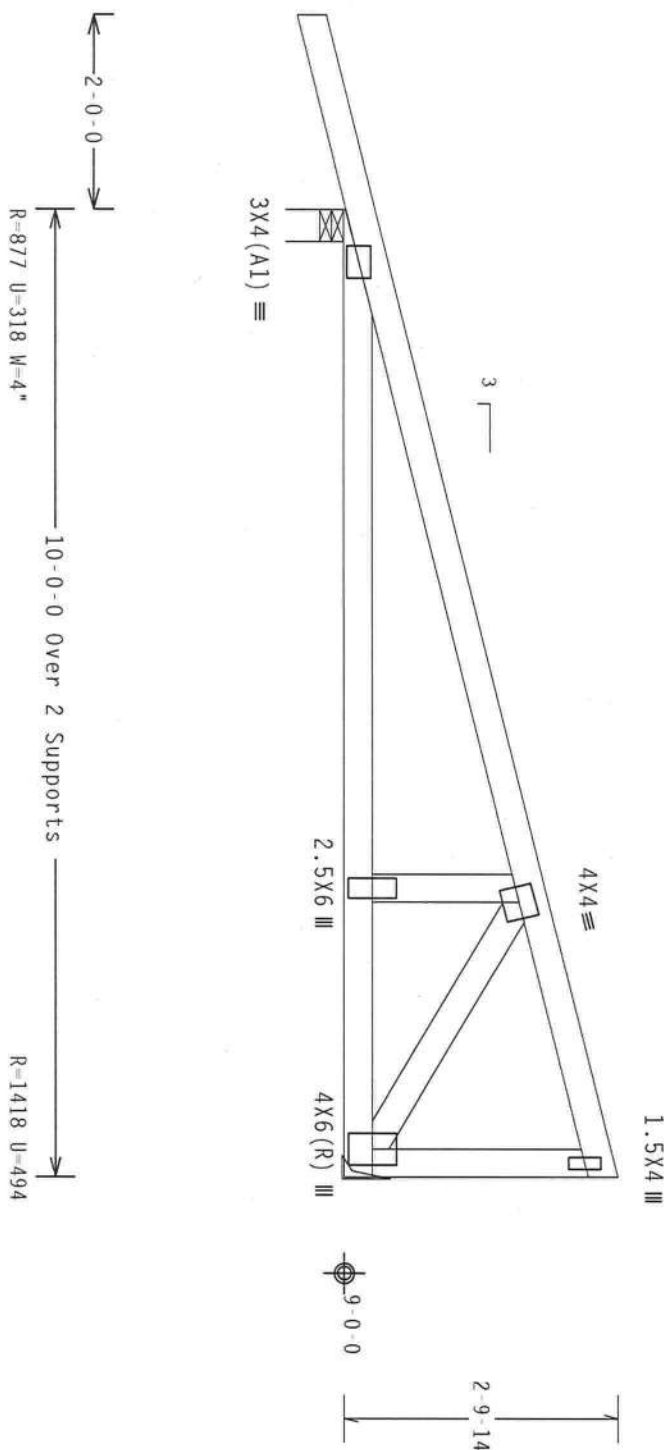
Wind reactions based on MWRFS pressures.

Roof overhang supports 2.00 psf soffit load.

	(number Dur.=1.25 / Plate Dur.=1.25)	
TC	From 61 pif at 2.00 to 61 pif at 10.00	
BC	From 4 pif at 2.00 to 4 pif at 0.00	
BC	From 20 pif at 0.00 to 20 pif at 10.00	
BC	956 1b Conc. Load at 7.06	
BC	403 1b Conc. Load at 9.06	

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:2

FL/-/4/-/-/R/-/-

Scale = .5" / Ft.

[illegible]

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

IP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 CONNECTION PLATES ARE MADE OF 20/18/16GA (W, H/S3/K) ASTM A653 GRADE 40/60 (W, X/H/S3) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/API 1 SEC. 2.

21.09

TC LL	20.0 PSF	REF R8228- 63122
TC DL	10.0 PSF	DATE 12/21/09
BC DL	10.0 PSF	DRW HCUR8228 093550
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 63243
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TXS8228Z01

JREF - 1TXS8228Z01

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Top chord 2x4 SP #2 Dense :83 2x6 SP #2:
Bot chord 2x4 SP #2 Dense :83 2x6 SP #2:
Webs 2x4 SP #3

:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures.

(A) 1x4 #3SR8 SPF-S or better "L" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Special loads

- TC - From Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) 66 plf at -1.33 to 66 plf at 12.12
- TC - From 66 plf at 12.12 to 66 plf at 33.55
- TC - From 66 plf at 33.55 to 66 plf at 45.67
- BC - From 5 plf at -1.33 to 5 plf at 0.00
- BC - From 20 plf at 0.00 to 20 plf at 45.67
- PLT- 936 LB Conc. Load at (37.73,15.58)
- PLT- 382 LB Conc. Load at (39.10,14.44)
- BC - 382 lb Conc. Load at (37.73, 39.10, 41.10)
- BC - 1418 lb Conc. Load at 43.10

The Building Designer shall evaluate and approve load magnitudes and locations as shown under "SPECIAL LOADS". Truss Engineer & Fabricator are not responsible for load magnitudes and locations.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

+ Member to be laterally braced for horizontal wind loads. Bracing system to be designed and furnished by others.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 7.25 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI(+/-)=0.18

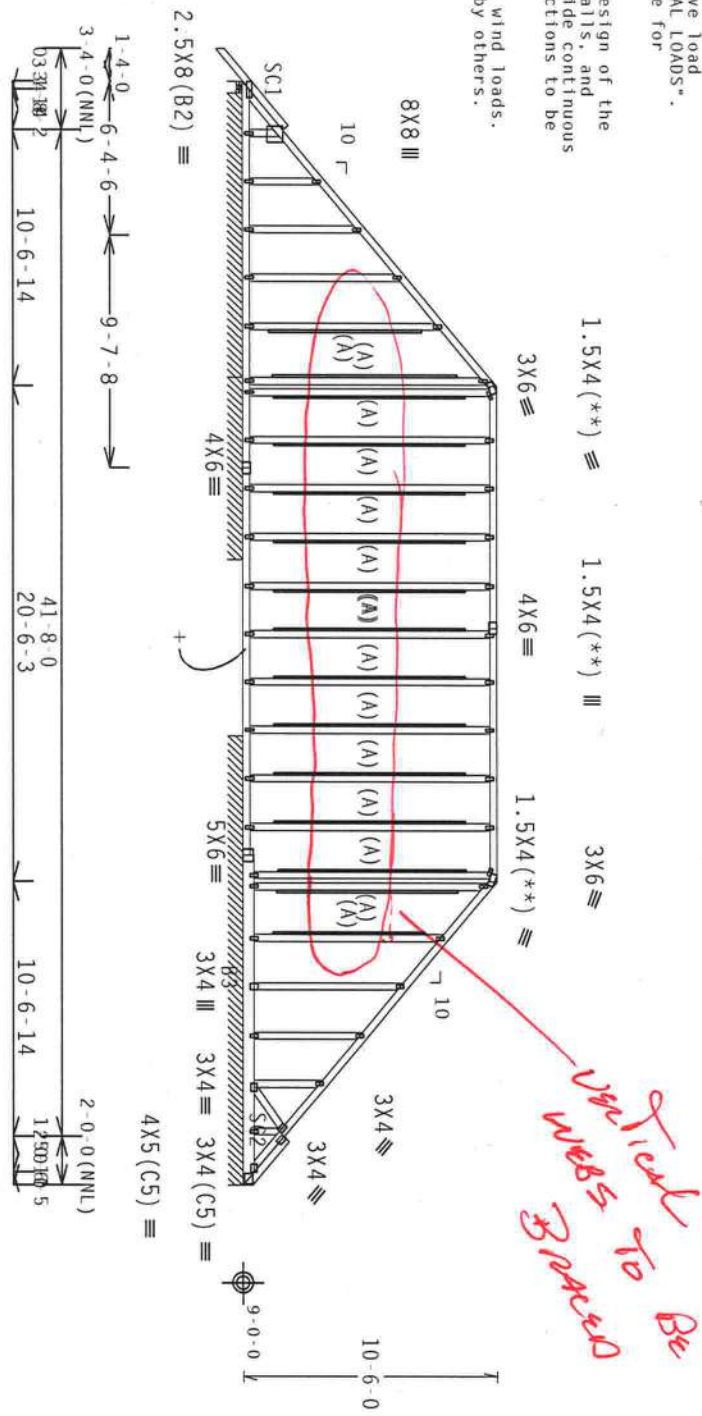
Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLETTIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

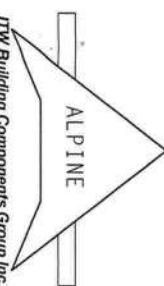
9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale =.125"/ft.

WARNING TRUSSES REQUIRE EXTERIOR GABLE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICKIWOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MANDALAY, UT 84701 FOR SAFETY PRACTICES PRIOR TO REPAIRING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 63123
TC DL	10.0 PSF	DATE	12/21/09
BC DL	10.0 PSF	DRW	HCUSR8228 09355038
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT. LD.	40.0 PSF	SEQN	12003 REV
DUR. FAC.	1.25	JREF	1TXS8228Z01
SPACING	24.0"		

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

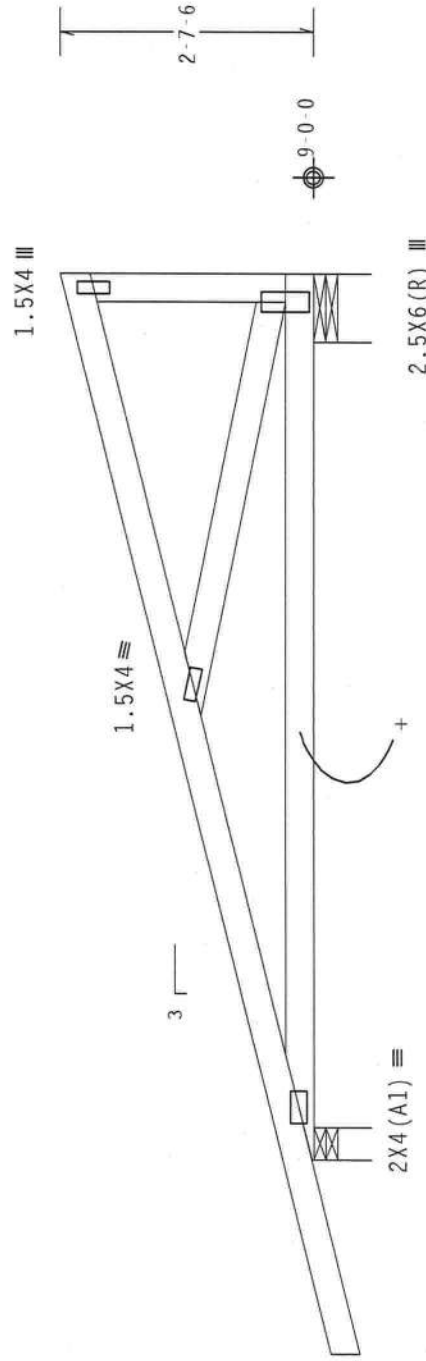
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 5.00 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

+ Member to be laterally braced for horizontal wind loads.
Bracing system to be designed and furnished by others.



9-2-0 Over 2 Supports

R=578 U=298 W=4"
RI=153

R=418 U=188 W=8.485"

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)	9.02.00

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/Ft.

WARNING: BOSSSES REQUIRE EXTREME CARE IN FABRICATION, TURNING, CUTTING, INSTALLING AND BRACING. THESE BEAMS ARE NOT TO BE USED FOR ANY OTHER PURPOSES. THE COMPANY'S FORMERLY INSTITUTED NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND WEA AND WEA TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, HADISON, NJ 07619, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

TC LL	20.0	PSF	REF	R8228-	63126
TC DL	10.0	PSF	DATE	12/21/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09355037
BC LL	0.0	PSF	HC-ENG	TCE/DF	
TOT.LD.	40.0	PSF	SEQN-	63358	
DUR.FAC.	1.25				
SPACING	24.0"				IDEE - 1TVS8228701

(9-253--Fill in later MOSES --, ** - GZA)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W7 2x6 SP #2:

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 16.25 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(v/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Girder supports 8-6-6 span to TC/BC split one face and 8-6-6 span to TC/BC split opposite face.

3 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 8.50" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.

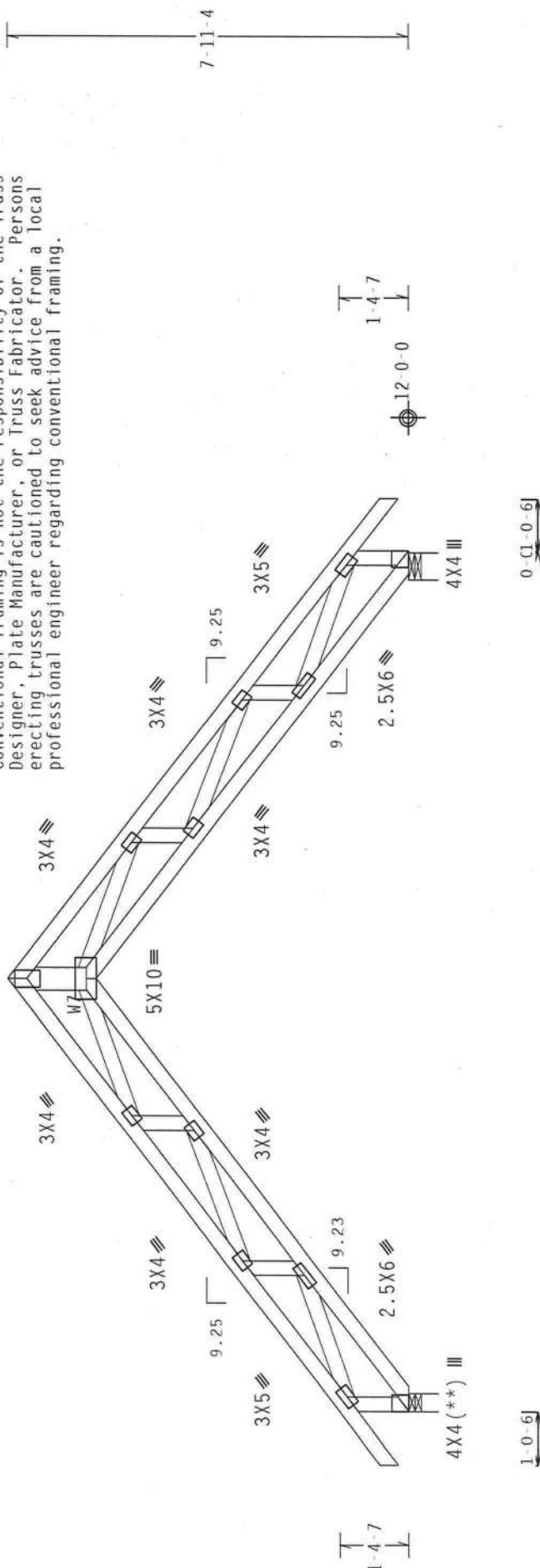
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.32" due to live load and 0.37" due to dead load.

Deflection meets L/240 live and L/180 total load.

Conventional framing is not the responsibility of the Truss Designer, Plate Manufacturer, or Truss Fabricator. Persons erecting trusses are cautioned to seek advice from a local professional engineer regarding conventional framing.



8-6-6
8-6-8
17'-0-10 Over 2 Supports
8-6-4
8-6-2
R-3186 U=795 W=4.334"
R-3218 U=802 W=6.501"
0'-0-6

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

FL/-4/-/-R/-

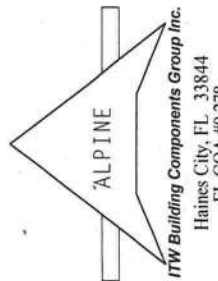
Scale = .3125"/Ft.

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	
SPACING	24.0"	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ITW BCG CONNECTIONS ARE MADE OF 2011/10/16GA (4.75/55/74) ASH 1603 GRADE 40/60 (N, K/2N/55) GALV. STEEL. APPLY ALL RECOMMENDATIONS OF TPI. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS SHALL BE PER AREA AS OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

*** Negative reaction(s) of -613# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpi(+/-)=0.18

0.13" due to dead load.

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

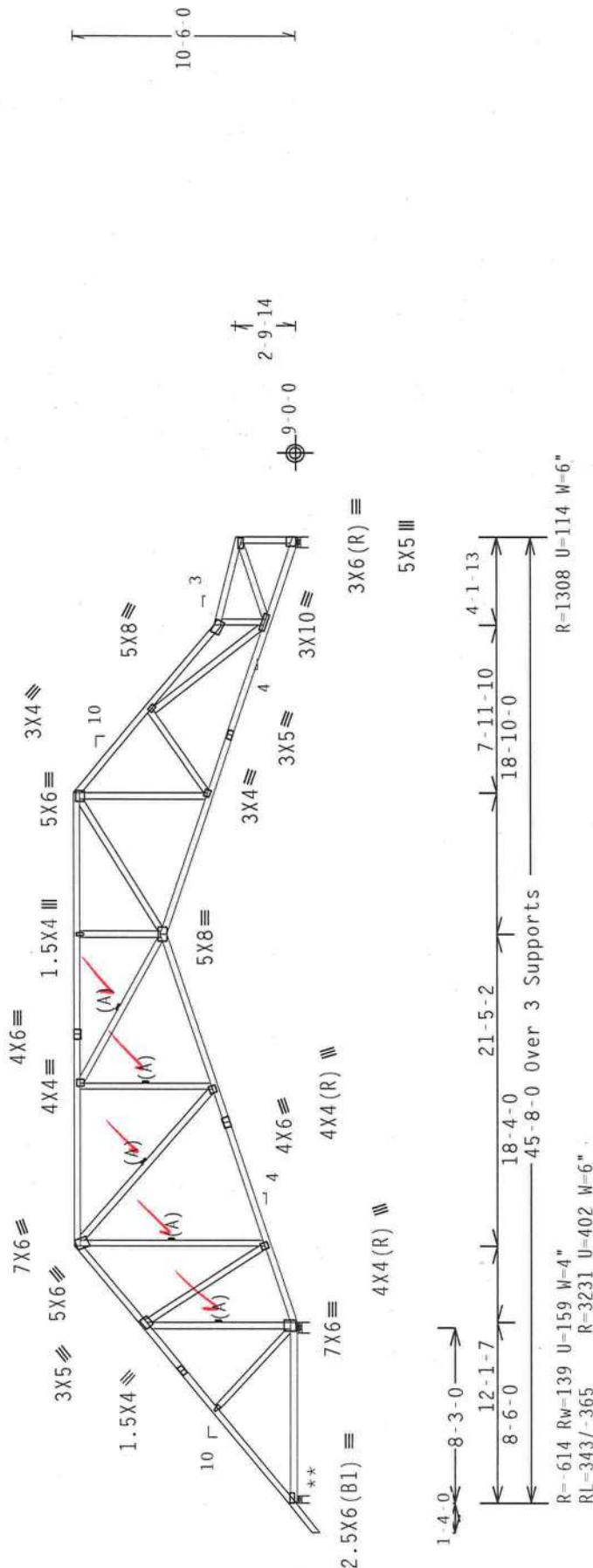
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

00

QTY:10 FL/-/4/-/-/R/-

Scale = .125"/Ft.

REF R8228- 63128

DATE 12/21/09

CO/IT/21 21/01/2021

ORW HCU5R8Z28 0935503

HC-ENG TCE/DF

71590 SEQN-

1000 JOURNAL OF CLIMATE

REF- 1TXS8228Z01

****WARNING**** THUSSES REQUIRE EXTERIOR CORE IN FABRICATION. THUSSES MUST BE PROPERLY ATTACHED TO THE TOP CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

* IMPORTANT ** UPON A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

THE USER COMING INTO POSSESSION OF THIS DOCUMENT DESIGNS AND SPECIFICATIONS SHALL BE LIMITED TO THE REG. INC. USUALLY COMING WITH APPLICABLE PROVISIONS OF MOST RECENT DESIGN CODES SUCH AS, AISC 89, AISC 90, AISC 91A, STEEL APPROX PLATES TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP1-1-2009 SEC-3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE CROSS COMPONENT OF THIS CONCEPT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNEX A OF TP1-1 SEC.-2.



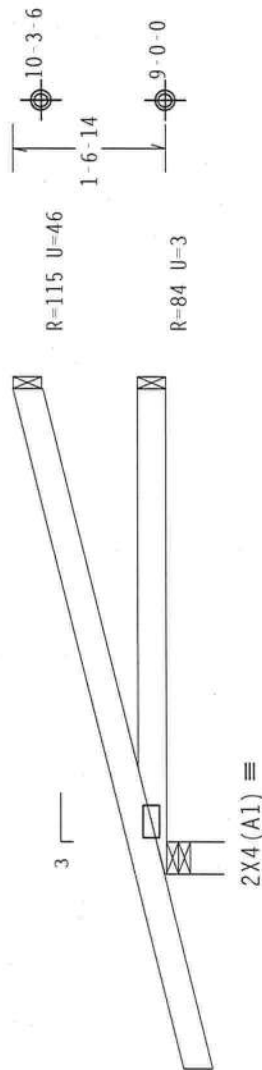
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w-1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



$\overbrace{\hspace{1.5cm}}^{2-0-0}$
 $\overbrace{\hspace{1.5cm}}^{5-0-0}$ Over 3 Supports
 R-368 U-154 W-4"
 RI=64

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5"/Ft.

FL/-/4/-/-/R/-

QTY: 4

00-10515-00

9.02.00

FT/RT=10%(0%)/0(0)

design critic: 10

PLT TYP. Wave

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, UNLOADING AND BRACING. PLEASE FOLLOW THE FOLLOWING GUIDELINES: TRUSSES ARE NOT TO BE OULDED/COMPONENT SAFETY INFORMATION, PUBLISHED BY IPI (SHIPPING PLATE INSTITUTE, 210 ROSEBUSH STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH ITPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES. THE TRUSS AND CONNECTION PLATES ARE MADE OF 2019/16GA. 48"X5/8" ASTM A575 GRADE 30/40 G. 48"X5/8" GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY ITPI SHALL BE PER AMEX AS OF ITPI-2009 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLICIT FOR THE SIGNATURE OF THE DESIGNER OF THIS PROJECT FOR THIS COMMENT FOR ANY HOLDING IS THE RESPONSIBILITY OF THE HOLDING DESIGNER PER AMEX ITPI 1, SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL

2X4 GABLE VERTICAL SPACING		BRACE NO.	(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
MAX GABLE VERTICAL LENGTH	SPF	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"
	HF	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"
	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"
	SP	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"
24" O.C.	DFL	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"
	STANDARD	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	10' 2"	12' 5"	13' 5"
	SPF	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	10' 2"	12' 5"	13' 5"
	HF	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"
16" O.C.	STANDARD	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"
	SPF	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"
	HF	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	14' 0"	14' 0"
	STANDARD	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"
12" O.C.	SPF	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"
	HF	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"
	STANDARD	4' 5"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"
	SPF	4' 11"	8' 5"	8' 5"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"
12" O.C.	HF	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"
	STANDARD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"
	SPF	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"
	HF	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"
12" O.C.	STANDARD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"
	SPF	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"
	HF	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"
	STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:			
GROUP A:			
SPRUCE-PINE-FIR	HEM-FIR		
#1 / #2 STANDARD	#2 STUD		
#3 STANDARD	#3 STUD		
DOUGLAS FIR-LARCH			
#3 STUD			
STANDARD			
GROUP B:			
HEM-FIR			
#1 & BTR			
#1			
DOUGLAS FIR-LARCH			
#1			
#2			

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

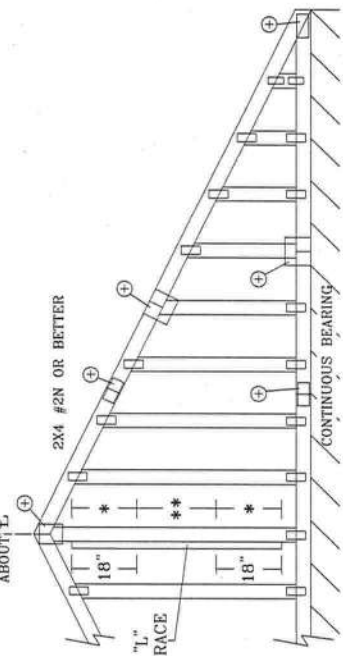
* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

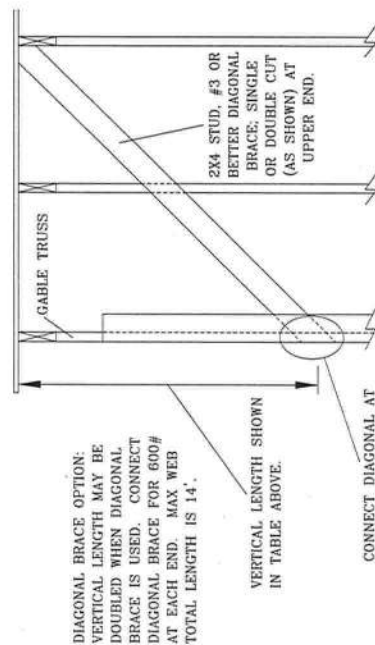
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



DIAGONAL BRACE OPTION: DOUBLED LENGTH WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Location shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. bracing of trusses. ITWBCG connector plates are made of 2018/16GA (W/S/K) ASTM A653 grade 37/40/60 (K/R/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joiner's responsibility of the Building Designer per ANST/TP1 1 Sec. 2. ITW-BCSI: www.itwbcg.com; TPI: www.tpiust.com; WTCA: www.abcdindustry.com; ICC: www.iccsafe.org



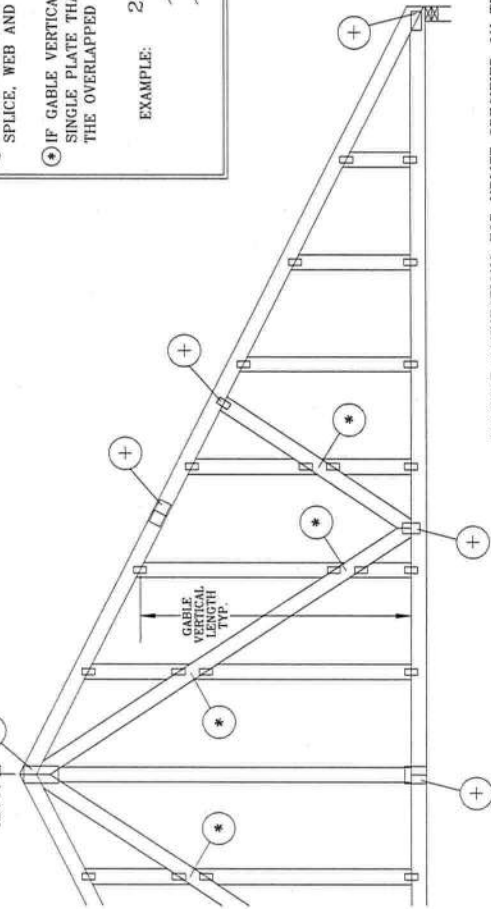
REF ASCE7-05-GABI1015
DATE 1/1/09
DRWG A11015050109

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

GABLE DETAIL FOR LET-IN VERTICALS

SYM. C.
ABOUT



GABLE TRUSS PLATE SIZES

- REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.
- REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.
- IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, $K_{zt} = 1.00$
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

END DRIVEN NAILS:

10d COMMON (0.148" X 3" MIN) NAILS AT 4" O.C. PLUS

(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:

10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE

WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015980109, A12015980109, A11015980109, A10015980109,

A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015020109, A12015020109, A11015020109, A10015020109,

A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015050109, A12015050109, A11015050109, A10015050109,

A13030050109, A12030050109, A11030050109, A10030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM

UNREINFORCED GABLE VERTICAL LENGTH.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow ITW Building Component Safety Information, by TPI and WCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/16GA (W/H/S/K) ASTM A653 grade 37/40/50 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. ITWBCG shall not be responsible for any deviation from this design or any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

ITW - BCG: www.itwbcg.com; TPI: www.tpinet.com; WCA: www.wcaindustry.com; ICC: www.iccsafe.org

REF LET-IN VERT

DATE 1/1/09

DRWG GBLLETTIN0109

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0"



Building Components Group Inc.

Earth City, MO 63045

120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZT=1.0.

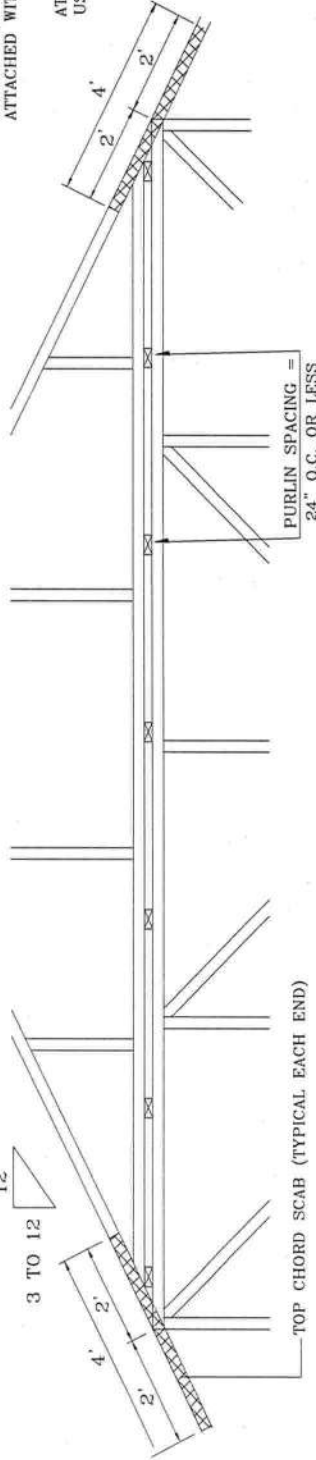
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

•• REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

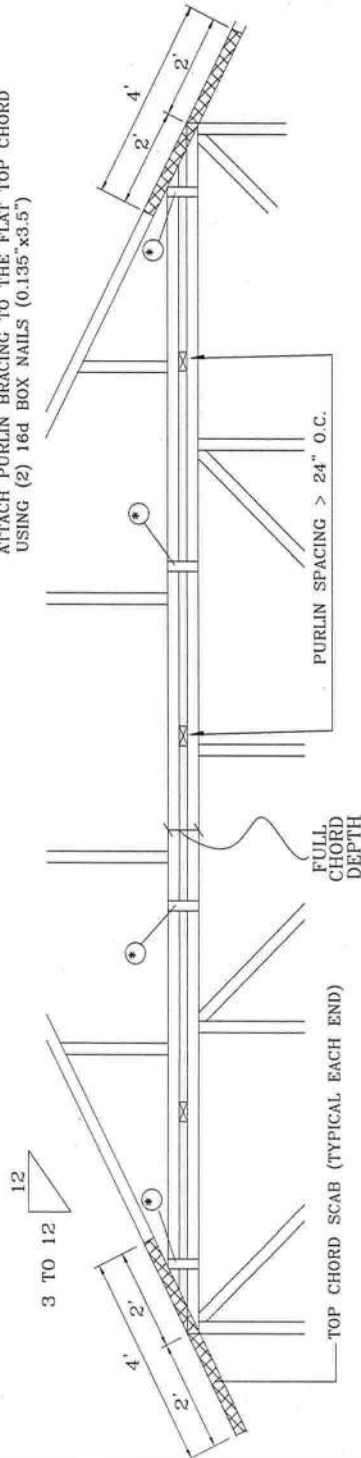
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



DETAIL B : PURLIN SPACING > 24" O.C.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

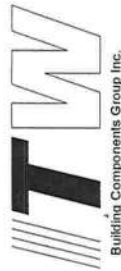
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

••WARNING•• READ AND FOLLOW ALL NOTES ON THIS SHEET Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow RCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing any building to building connections. Trusses shall be installed in accordance with the manufacturer's instructions. Trusses shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per RCSI sections B3 & B7. See this job's general notes page for more information.

••IMPORTANT•• FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any building to building connections shall be in accordance with the manufacturer's instructions. Trusses shall be installed in accordance with the manufacturer's instructions. Trusses shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per RCSI sections B3 & B7. See this job's general notes page for more information.



Earth City, MO 63045

• IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

- TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.
- PLYWOOD GUSSET
8"x8"x1/2" RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8" O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.
- 2x4 VERTICAL SCABS
2x4 SPF#2, FULL CHORD DEPTH SCABS @ 8" O.C. EACH FACE. STAGGERED 4" O.C. ATTACH WITH (3) 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB).
- 28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE OF @ O.C. ATTACH TEETH TO PIGGYBACK PLATE WITH (4) 0.120"x1.375" NAILS PER FACE PER PLATE. PIGGYBACK PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

REF PIGGYBACK
DATE 10/01/09
DRWG PB1201009



SPACING 24.0"

GABLE STUD REINFORCEMENT DETAIL

MAX GABLE VERTICAL LENGTH		2X4 GABLE VERTICAL SPACING		BRACE		NO BRACES	(1) 1X4 "L" BRACE •				(1) 2X4 "L" BRACE •				(2) 2X4 "L" BRACE ••				(1) 2X6 "L" BRACE •				(2) 2X6 "L" BRACE ••										
				GRADE	#1 / #2		GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B									
12" O.C.	SPF	H#1 / #2	#3	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"											
																							STUD	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"												
																						STUD	5' 6"	7' 3"	7' 3"	8' 5"	9' 9"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	STANDARD	3' 8"	4' 9"	6' 3"	7' 5"	8' 7"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"											
16" O.C.	SPF	H#1 / #2	#3	4' 1"	6' 8"	8' 7"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"											
																							STUD	5' 8"	7' 6"	7' 6"	10' 1"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	4' 7"	7' 3"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"												
																						STUD	6' 9"	8' 7"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	STANDARD	4' 4"	6' 9"	8' 7"	9' 3"	10' 3"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"											
24" O.C.	SPF	H#1 / #2	#3	4' 7"	8' 0"	9' 5"	9' 8"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"											
																							STUD	7' 8"	8' 8"	9' 5"	11' 3"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	5' 1"	8' 0"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"												
																						STUD	7' 11"	9' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

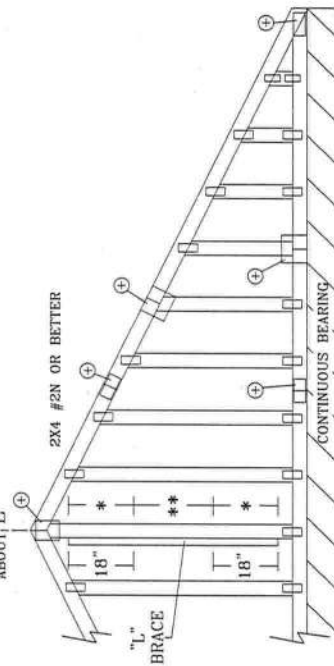
ATTACH EACH "L" BRACE WITH 10d NAILS.
(0.128"x3" min)

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES

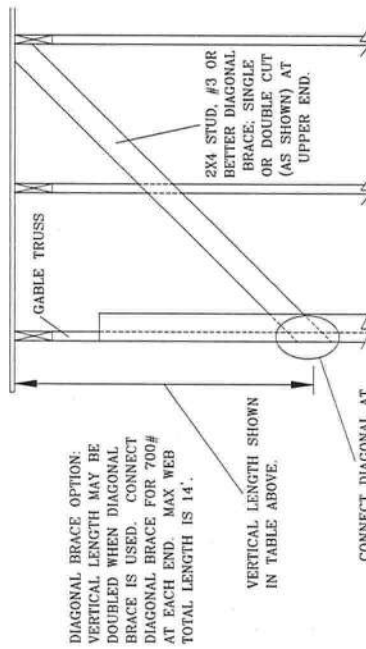
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL.

[illegible]

••IMPORTANT•• FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

WTW Building Components Group Inc. (WTBCC) shall not be responsible for any deviation from this design. WTBCC shall not be responsible for any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & erecting the truss. WTBCC connector plates are made of 20/18/16GA (W/H/S/K) ASTM A653 (W/H/S/K) on joint details. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TP1 Sec. 2.

WTBCC-HCC: www.wtbcc.com; TPI: www.tpi.net; WTA: www.abcdindustry.com; ICC: www.iccs.org

Earth City, MO 63045

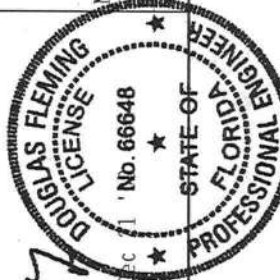
REF ASCE7-05-GABI1030

DATE 1/1/09

DRWG A11030050109

[illegible]

MAX. SPACING	24.0"
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CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING	SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4	
2X3 OR 2X4	2 ROWS	2X6	2-2X4	
2X6	1 ROW	2X4	1-2X6	
2X6	2 ROWS	2X6	2-2X4(*)	
2X8	1 ROW	2X6	1-2X8	
2X8	2 ROWS	2X6	2-2X6(*)	

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

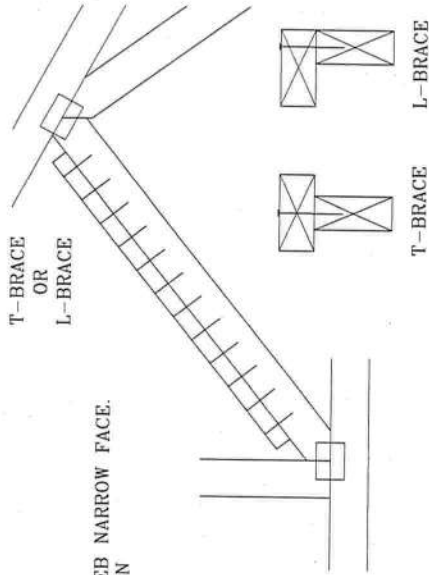
T-BRACING

OR

L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C.

BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH

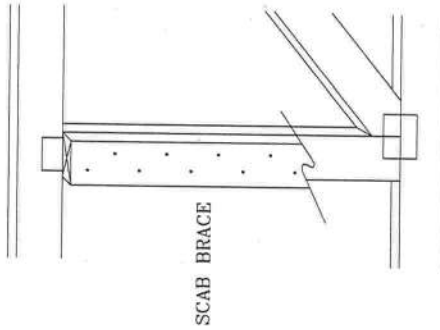


T-BRACE

L-BRACE

SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128" x 3", MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACE

WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing any work on trusses. Trusses shall provide temporary bracing per BCSI. Unless noted otherwise, top chord bracing shall be provided on all trusses and shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web and chord bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to follow the design, or any failure to follow the design. ITWBCG connector plates (K/W/H/S) galv. steel. Apply plates to each face of truss. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. ITW-BCG: www.itwbcg.com; TPI: www.tpiinst.com; WTCA: www.abcdindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045

TC LL PSF REF CLB SUBST.

TC DL PSF DATE 1/1/09

BC DL PSF DRWG BRCLBSUB0109

BC LL PSF

TOT. LD.

DUR. FAC.

SPACING

