

FILED 07/06/2006

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000024712

APPLICANT JOHN SHERMAN PHONE 303-1981
ADDRESS 12592 S US HIGHWAY 441 LAKE CITY FL 32025
OWNER HARRY & LAMANDA MOTE PHONE
ADDRESS 301 SW FAUL COURT LAKE CITY FL 32024
CONTRACTOR HAYGOOD HOMES PHONE 752-3496
LOCATION OF PROPERTY 47S, TL ON FAUL COURT, 4TH LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 134700.00
HEATED FLOOR AREA 2694.00 TOTAL AREA 3949.00 HEIGHT STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB
LAND USE & ZONING A-3 MAX. HEIGHT 22
Minimum Set Back Requirements: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 02-5S-16-03427-011 SUBDIVISION THE OAKS
LOT 2 BLOCK B PHASE UNIT TOTAL ACRES

CRC1326715
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
EXISTING 06-0567-N BK JH Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD,

Check # or Cash 2514

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 675.00 CERTIFICATION FEE \$ 19.75 SURCHARGE FEE \$ 19.75
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 789.50
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

789.50

Revised 9-23-04

For Office Use Only Application # 0606-46 Date Received 6/15/06 By G Permit # 24712
Application Approved by - Zoning Official BLK Date 22.06.06 Plans Examiner OK JH Date 6-23-06
Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments Section 2.3.1 Legal Nonconforming Lot of Record

Applicants Name John R. Sherman Brenda Haygood Phone 752-3496
Address 12592 S. US Hwy 441 LC 32025 John 397-0600
Owners Name Harry & Lamanda Mote Phone NT
911 Address 301 SW Faul Ct LC 32024
Contractors Name Haygood Homes, Inc Phone 752-3496
Address 12592 S US Hwy 441 LC cell 303-1981
Fee Simple Owner Name & Address Coast Bank of Florida 6205 Cortez Rd West
Bonding Co. Name & Address NA Bradenton, FL 34210
Architect/Engineer Name & Address Pat Haygood Marty Humphries
Mortgage Lenders Name & Address Coast Bank of Florida
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
Property ID Number 02-55-16-03427-011 Estimated Cost of Construction 200,00.
Subdivision Name The Oaks Lot 2 Block B Unit Phase
Driving Directions SR 47 South, Turn left on Faul Court
4th lot on left

Type of Construction new home KFD Number of Existing Dwellings on Property 0
Total Acreage 2 Lot Size 2ac Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 85' Side 150' Side 97.88' Rear 129.1'
Total Building Height 22' 5" Number of Stories 1 Heated Floor Area 2694 Roof Pitch 8/12
PORCH 608 GARAGE 647 BONUS RM 464 TOTAL 3949

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

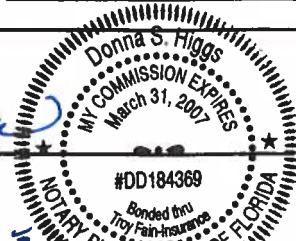
[Signature]
Owner Builder or Agent (Including Contractor)

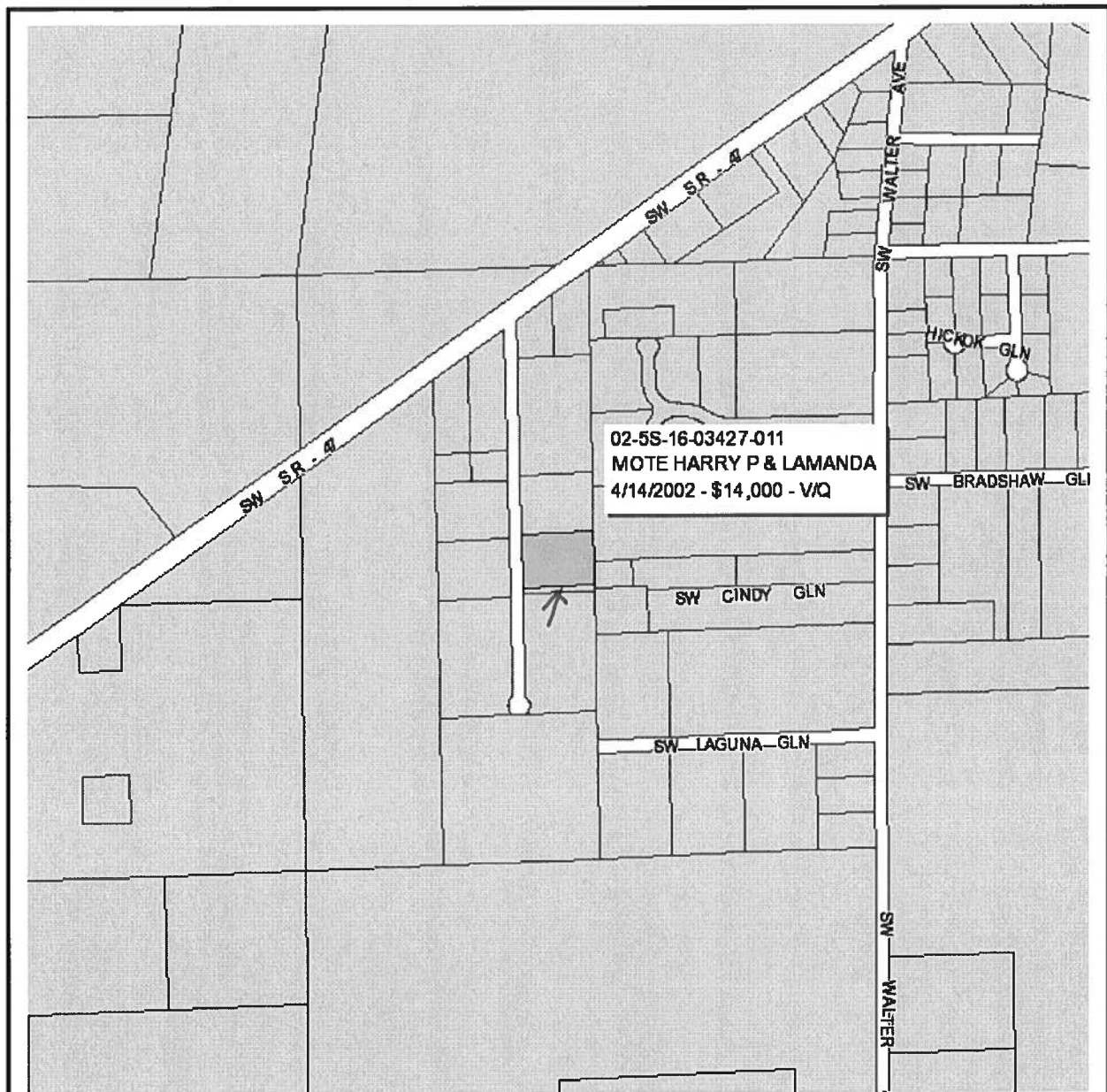
STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 14 day of June 2006.
Personally known ✓ or Produced Identification

[Signature]
Contractor Signature
Contractors License Number CRC1326715
Competency Card Number
NOTARY STAMP/SEAL

[Signature]
Notary Signature





02-5S-16-03427-011
MOTE HARRY P & LAMANDA
4/14/2002 - \$14,000 - V/Q

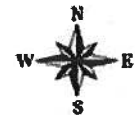
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 02-5S-16-03427-011 - VACANT (000000)

Name: MOTE HARRY P & LAMANDA	LandVal	\$24,120.00
Site: THE OAKS S/D	BldgVal	\$0.00
Mail: 208 NE JERRY GLN	ApprVal	\$24,120.00
LAKE CITY, FL 32055	JustVal	\$24,120.00
Sales 4/14/2002 \$14,000.00 V / Q	Assd	\$24,120.00
Info 9/27/1996 \$12,500.00 V / Q	Exmpt	\$0.00
	Taxable	\$24,120.00

0 0.05 0.1 0.15 mi



This information, GIS Map Updated: 5/5/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

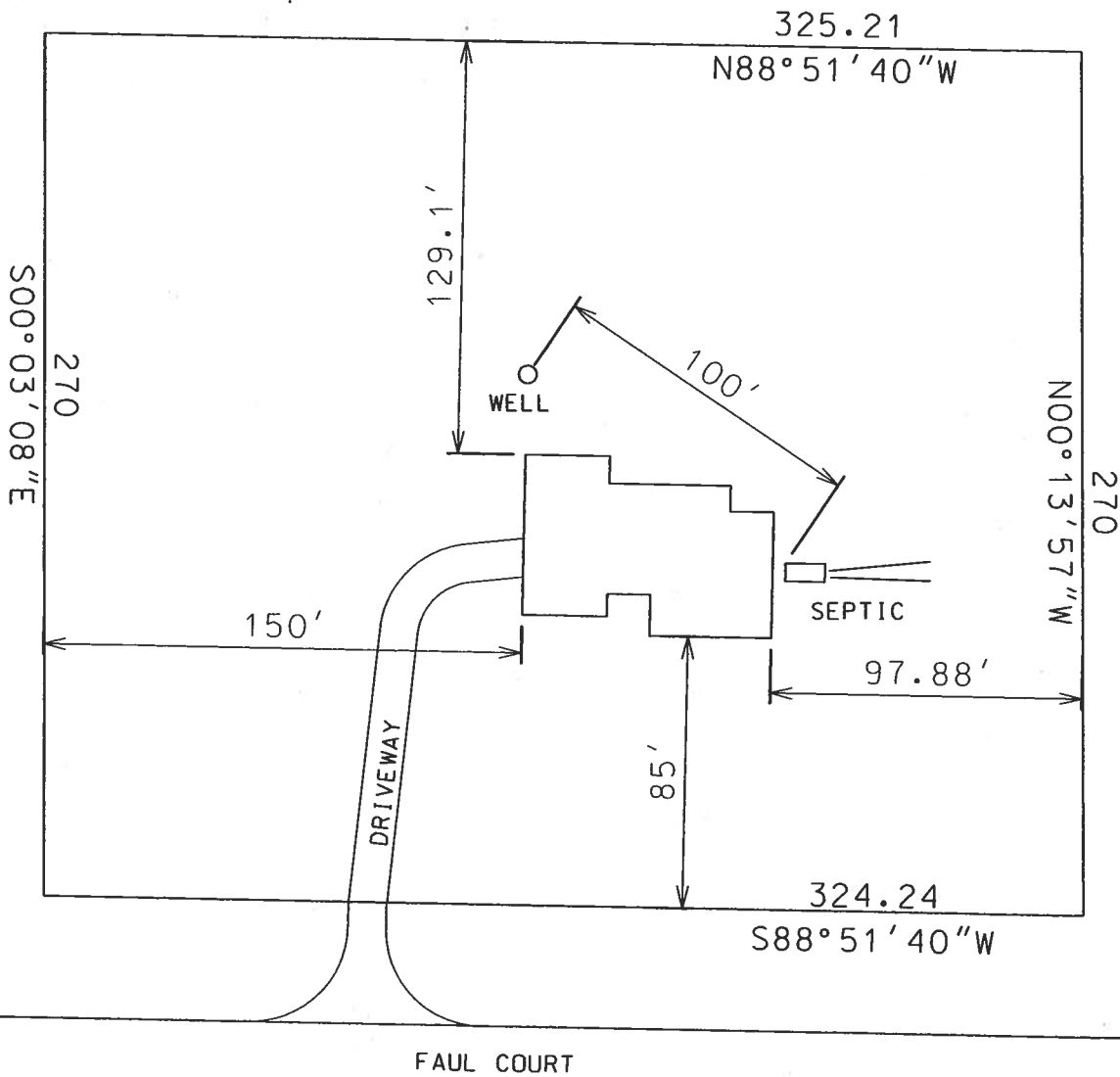
SITE LOCATION

HARRY P. & LAMANDA P. MOTE
301 SW Faul Ct.
Lake City Fl 32025

PID 02-5S-16-03427-011

STATE ROAD 47 SOUTH, TURN LEFT ON FAUL COURT.

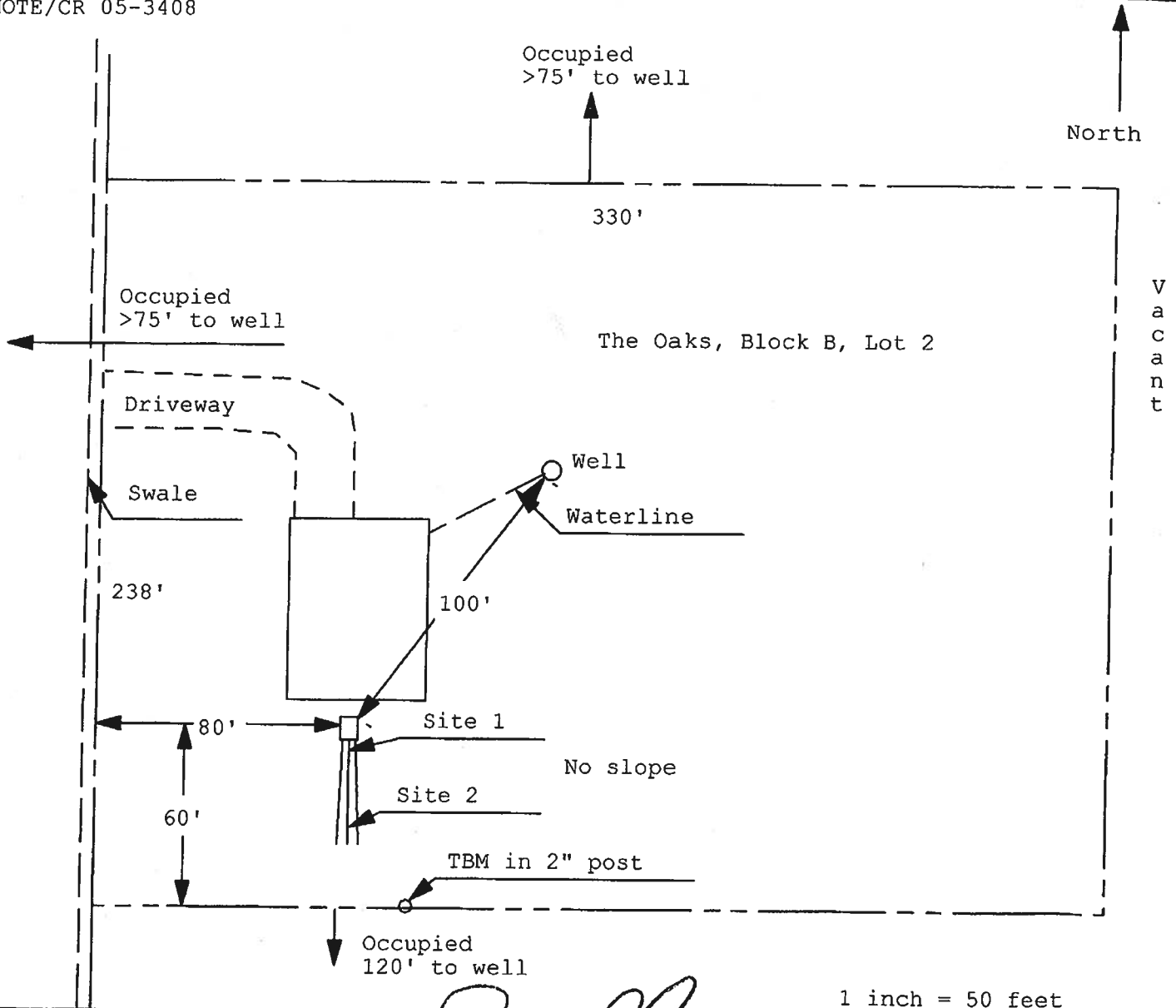
4th - LOT ON THE LEFT.



**Application for Onsite Sewage Disposal System
Construction Permit. Part II Site Plan**
Permit Application Number: 06-0567N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

MOTE/CR 05-3408



Site Plan Submitted By Paul Lloyd Date 3/7/06
 Plan Approved ☒ Not Approved ☐ Date 6-21-06

By Salbi Graddy, ESII **Columbia CHD** CPHU

Notes: _____

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 5/31/2006 DATE ISSUED: 6/1/2006

ENHANCED 9-1-1 ADDRESS:

301 SW FAUL CT

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

02-5S-16-03427-011

Remarks:

SOUTH 270 FEET FOR LOT 2 BLOCK B THE OAKS S/D

Address Issued By:


Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

276

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXX NORTH FIRST STREET~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

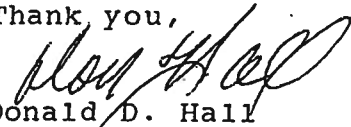
June 12, 2002

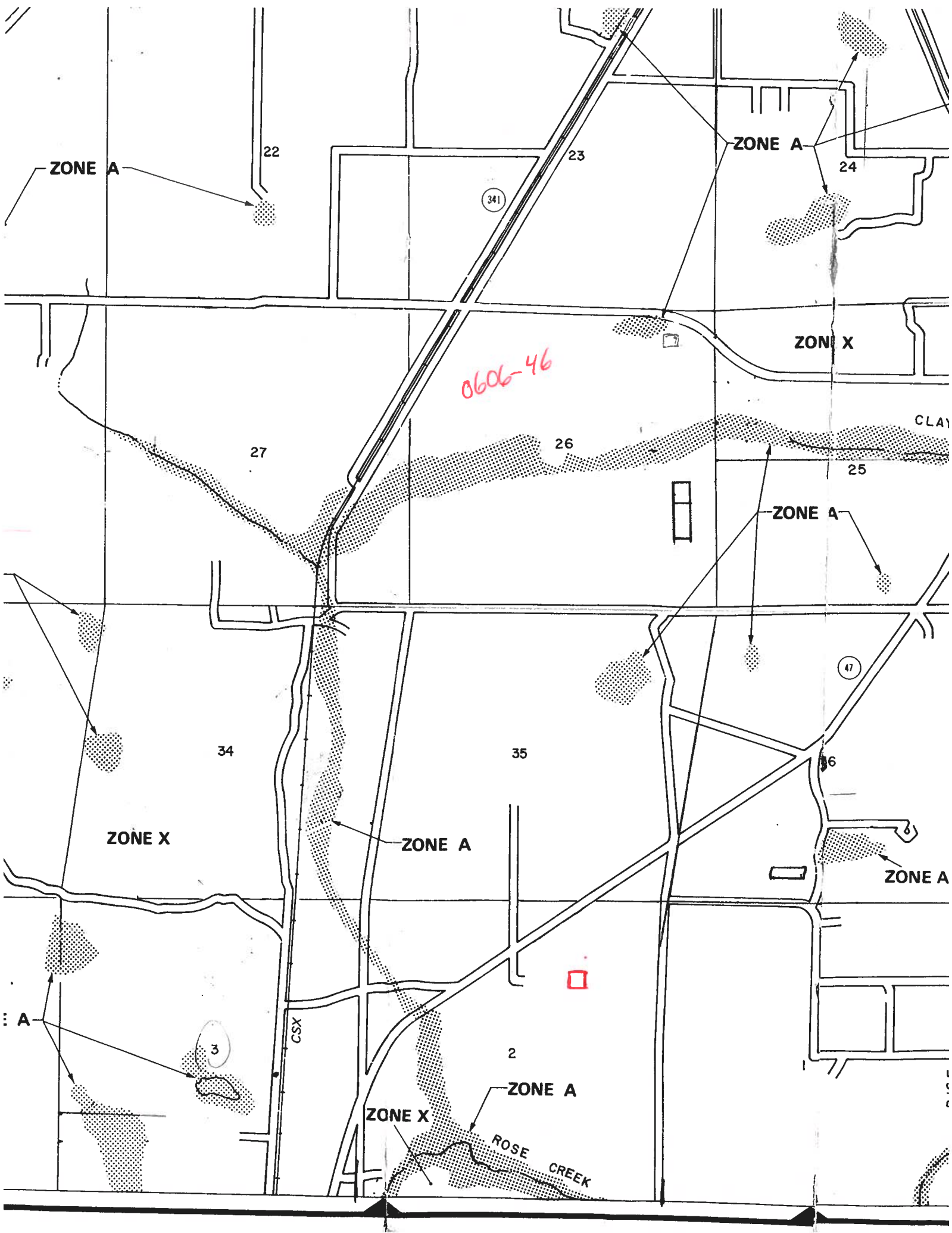
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,


Donald D. Hall
DDH/jk



ZONE A

ZONE A

ZONE X

0606-46

CLAY

ZONE A

ZONE X

ZONE A

ZONE A

ZONE A

ZONE X

ROSE CREEK

22

23

24

341

27

26

25

34

35

47

36

3

2

CSX

FORM 600B-04

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION
Residential Component Prescriptive Method B

NORTH 12 3

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME: AND ADDRESS:	Mote		BUILDER:	Haygood Homes, Inc.	
			PERMITTING OFFICE:		
OWNER:	Harry LaManda Mote		PERMIT NO.:	24712	JURISDICTION NO.:
			CLIMATE ZONE:	1 ☐ 2 ☐ 3 ☐	

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

Please Print

CK

1. Compliance package chosen (A-E)
2. New construction or addition
3. Single-family detached or multiple-family attached
4. If multiple-family—No. of units covered by this submission
5. Is this a worst case? (yes/no)
6. Conditioned floor area (sq. ft.)
7. Predominant eave overhang (ft.)
8. Glass type and area:
 - a. Clear glass
 - b. Tint, film or solar screen
9. Percentage of glass to floor area
10. Floor type, area or perimeter, and insulation:
 - a. Slab-on-grade (R-value)
 - b. Wood, raised (R-value)
 - c. Wood, common (R-value)
 - d. Concrete, raised (R-value)
 - e. Concrete, common (R-value)
11. Wall type, area and insulation:
 - a. Exterior:
 1. Masonry (Insulation R-value)
 2. Wood frame (Insulation R-value)
 - b. Adjacent:
 1. Masonry (Insulation R-value)
 2. Wood frame (Insulation R-value)
12. Ceiling type, area and insulation:
 - a. Under attic (Insulation R-value)
 - b. Single assembly (Insulation R-value)
13. Air distribution system: Duct insulation, location
Test report (attach if required)
14. Cooling system:
(Types: central, room unit, package terminal A.C., gas, none)
15. Heating system:
(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)
16. Hot water system:
(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)

1.	B	
2.	new	
3.	Single	
4.		
5.	yes	
6.	2694	
7.	2	
	Single Pane	Double Pane
8a.	sq. ft. 602	sq. ft.
8b.	sq. ft.	sq. ft.
9.	22 %	
10a.	R = 0	266 lin. ft.
10b.	R =	sq. ft.
10c.	R =	sq. ft.
10d.	R =	sq. ft.
10e.	R =	sq. ft.
11a-1	R =	sq. ft.
11a-2	R = 13	2374 sq. ft.
11b-1	R =	sq. ft.
11b-2	R =	sq. ft.
12a.	R = 30	sq. ft. 2694
12b.	R =	sq. ft.
13.	R = 6	
14a.	Type: Central	
14b.	SEER/EER: 13	
14c.	Capacity: 4 ton	
15a.	Type: Heat Pump	
15b.	HSPF/COP/AFUE:	
15c.	Capacity: 50 gal	
16a.	Type: elect	
16b.	EF: 88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Brian Haygood DATE: 6/13/06

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: [Signature] DATE: 6/13/06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL:

DATE:

APPENDIX 13-D

TABLE 6B-1

MINIMUM REQUIREMENTS

Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED	
GLASS	Max. % of Glass to Floor Area	15%	15%	20%	20%	25%	15 %	
	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☐	DT: ☐
	Overhang	1'4"	2'	2'	2'	2'	2 FEET	
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE					EXT: R = ADJ: R = COM: R =	
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD-FRAME WALLS R-11					EXT: R = 13 ADJ: R = COM: R =	
CEILING		R-30	R-30	R-30	R-30	R-30	UNDER ATTIC: R = COMMON: R =	
FLOORS	Slab-On-Grade	R-0					R = 0	
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R =	
	Raised Concrete	R-7					R =	
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R =	
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	R = 6 COND. ☐	
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	SEER = 13	
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					HSPF =	
HOT WATER SYSTEM	Electric Resistance**	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	AFUE =	
	Gas & Oil**	MINIMUM EF OF .59				NATURAL GAS ONLY (SEE BELOW)	EF = 88	
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☐ EF = HRU: ☐ EF = SOLAR: ☐ EF =	

* Single package units minimum SEER=9.7, HSPF = 6.6.

** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 612.1 ABC.3.2 for minimum code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multistory house.

Wall, Ceiling and Floor Insulation Values: The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multiple-family building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a state-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTACs equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of Section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed," an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options." See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .92 or greater, or natural gas systems with EF .59 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES			
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	NA
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	NA
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 psig.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

LOAN NUMBER: 7011012486

STATE OF Florida, COUNTY OF Columbia NOTICE OF COMMENCEMENT24112
Harry & Lamanda
Mote

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: SEE EXHIBIT A ATTACHED HERETO AND MADE A PART THEREOF
XXX SW FAUL CT, LAKE CITY, FL 32024
2. General description of improvements: Single Family Residence
3. Owner information:
 - a. Name(s): HARRY P MOTE and
 - b. Address: 208 NE JERRY GLN, LAKE CITY, FL 32055
 - c. Interest in property: Fee Simple
 - d. Name and address of fee simple titleholder (if other than Owner): N/A
4. Contractor: HAYGOOD HOMES INC.
12592 S. U.S. HWY 441, LAKE CITY, FL 32025
5. Surety
 - a. Name and address: None
 - b. The amount of bond: N/A
6. Lender COAST BANK OF FLORIDA - P. O. BOX 150, BRADENTON, FLORIDA 34206-0150
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7, Florida Statutes: N/A
8. In addition to himself, Owner designates ** to receive a copy of the Licnor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
**COAST BANK OF FLORIDA - Attn: Wanda M. Farr - P. O. Box 150, Bradenton, Florida 34206-0150 (941-345-1473)
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified):

Harry P. Mote
Borrower: HARRY P MOTELamanda Mote
Borrower: LAMANDA F. MOTE

Borrower:

Borrower:

STATE OF Florida Columbia COUNTY SS:

The forgoing instrument was acknowledge before me this 12th day of June, 2006
by HARRY P MOTE and who is personally known to me or has produced driver license
as identification and who did not take an oath.
My Commission expires:

Notary Public

Matthew D. Rocco

Matthew Rocco
My Commission 00150709
Expires September 17, 2006

Inst:2006014430 Date:06/15/2006 Time:09:29

DC,P.Dewitt Cason,Columbia County B:1086 P:2266

- 24712

COLUMBIA COUNTY OFFICIAL CERTIFICATE

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 02-5S-16-03427-011

Building permit No. 24712

Use Classification SINGLE FAMILY DWELLING

Fire: 39.06

Permit Holder HAYGOOD HOMES

Waste: 117.25

Owner of Building HARRY & LAMANDA MOTE

Total: \$ 156.31

Location: 301 SW FAUL COURT, LAKE CITY, FL (THE OAKS, LOT 2)

Date: 3-13-07

Harry Mote

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)



January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

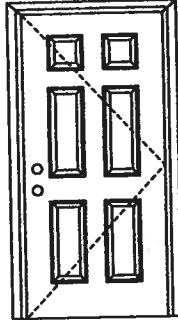
Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS**APPROVED ARRANGEMENT:****Note:**

Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itsmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door

Maximum unit size = 3'0" x 6'8"

Design Pressure

+66.0/-66.0

limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

Johnson
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from

Masonite
Masonite International Corporation

X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).



State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etsmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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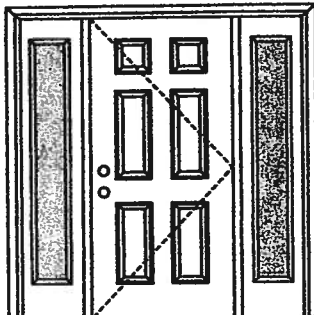
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Masonite
Masonite International Corporation

OXO

Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS**APPROVED ARRANGEMENT:**

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website (www.itsmto.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure

+57.0/-57.0 with maximum sidelite panel width of 1'2"

+45.0/-45.0 with maximum sidelite panel width of 3'0"

limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panels, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0004-02.

APPROVED DOOR STYLES:

Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

Johnson
EntrySystems

June 17, 2002

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PREMIER Collection
Premium Quality Doors



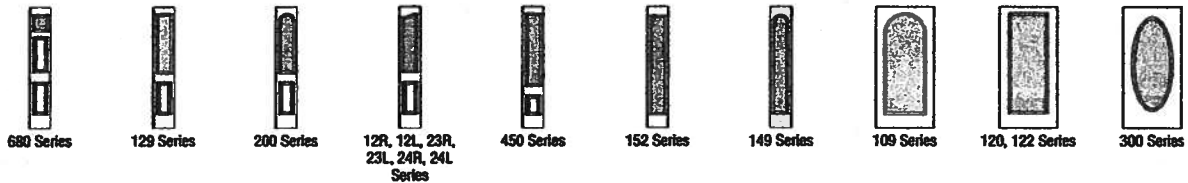
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Masonite International Corporation

OXO

Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS**APPROVED SIDELITE STYLES:****CERTIFIED TEST REPORTS:**

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL-210-1880-7, 9, 10, 12;
NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itsmko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Johnson
EntrySystems™

June 17, 2002
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Exclusively from

Masonite
Masonite International Corporation



**AAMA/WDMA 101/I.S. 2-97
TEST REPORT**

Rendered to:

JORDAN COMPANIES

**SERIES/MODEL: 8500
TYPE: PVC Single Hung Window**

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft ²
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com



AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No: 02-48976.02
Test Date: 02/25/04
Report Date: 02/26/04
Expiration Date: 02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

<u>Paragraph</u>	<u>Title of Test</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1) @ 1.57 psf (25 mph)	0.21 cfm/ft ²	0.30 cfm/ft ²
<i>Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.</i>			
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		
<i>Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."</i>			
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

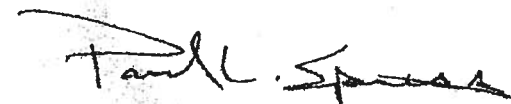
Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance:</u>			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds) @ 40.0 psf (positive) @ 40.0 psf (negative)	0.45" 0.52"	(See Note #3) (See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 60.0 psf (positive) @ 60.0 psf (negative)	0.03" 0.03"	0.16" max. 0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.



Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager



Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

DAJ/jb
02-48976.02

** LAMAR BOOZER **
 900 EAST PUTNAM STREET
 LAKE CITY, FL 32055

PROJECT: CUSTOM
 CLIENT: PAT HAYGOOD (MOTE)
 DATE: 4 17 06

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

NAME: PAT HAYGOOD (MOTE)
 ADDRESS:
 CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	85	2,773	0	2,964	2,964
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,219	4,389	0	2,400	2,400
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462	462
16-G CEILING R-30 INSULATION	2,639	2,627	0	2,627	2,627
22-A SLAB ON GRADE NO EDGE INSUL	111	4,046	0	0	0
SUBTOTALS FOR STRUCTURE:		4,094	14,681	0	8,453
PEOPLE	10	0	0	3,000	3,000
APPLIANCES	0	0	800	1,500	2,300
DUCTWORK	0	734	0	1,841	1,841
INFILTRATION W.CFM: 0.0 S.CFM: 235.9	0	0	7,859	5,449	13,308
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				20,243	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		15,415	8,659	20,243	28,902

SUPPLY CFM AT 20 DEG DT: 920 CFM PER SQUARE FOOT: 0.520
 SQUARE FT. OF ROOM AREA: 2,639 SQUARE FOOT PER TON: 734.482

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 15.415 MBH
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 4.409 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR
FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS
ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Plans Examiner

☒

1

All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.

☒

Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.

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Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

☒

Wind-load Engineering Summary, calculations and any details required

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC
 - a. Basic wind speed (MPH)
 - b. Wind importance factor (I) and building category
 - c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
 - d. The applicable internal pressure coefficient
 - e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional

Elevations including:

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

Q

□

①

□

□

b

c. Crawl space (if applicable)



b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiteicide or alternative method)
11. Slab on grade
 - a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)



c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:



a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer



b) Floor joist size and spacing



c) Girder size and spacing



d) Attachment of joist to girder



e) Wind load requirements where applicable



Plumbing Fixture layout

Electrical layout including:



a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified



b) Ceiling fans



c) Smoke detectors



d) Service panel and sub-panel size and location(s)



e) Meter location with type of service entrance (overhead or underground)



f) Appliances and HVAC equipment



g) Arc Fault Circuits (AFCI) in bedrooms



HVAC information



a) Manual J sizing equipment or equivalent computation



b) Exhaust fans in bathroom



Energy Calculations (dimensions shall match plans)



Gas System Type (LP or Natural) Location and BTU demand of equipment



Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

AN

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit.
5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is \$10.00
6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$5.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$25.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 758-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

Mote Residence Columbia County FL
Wind Load Analysis Requirements
(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:

Footprint: 78' x 50' "L" shaped home with front and rear covered porches
(see plan 0609 by Haygood Homes)
Walls: 2x4-16" O.C. with 7/16" OSB sheathing minimum, hardiboard lap siding
and 1/2" gypsum wall board interior.
Roof Structure: Pre-engineered roof trusses and 1/2" OSB sheathing min.
Roof Type: Gable construction (analyzed for 2' eave overhang and porch areas)
Foundation: footer with stemwall, with slab construction

Windload Data and Exposure:

Basic Wind Speed = 110 mph
Importance Factor = 1.0
Exposure category = B
Height and Exposure Adjustment Coefficient = 1.0
Residential Occupancy = Group R3
Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)
Mean roof height = 16'
Roof Cross Slope = 3.5:12, 7:12 & 8:12
Eave Overhang= (Analyzed for 2'eave overhang and porches)
Wall Height = 8'
Shear Wall locations = exterior walls only(all walls 3' in length or greater)
Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing: Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be installed At horizontal joints in sheathing.

Roof sheathing: Shall be 1/2" Oriented Strand Board(OSB) nailed with 8d common nails 3" on center at panel ends and eave overhang areas and 6" on center elsewhere.

Top wall plate: Nail with 1-16d common nail 12" O.C.(average)


5-16-06

Strapping and Anchor Requirements:

truss to exterior wall plate and porch beam locations: install one Simpson model H10 hurricane anchor at each location. at first 5 trusses at the left and right side of the front porch install an additional H5 anchor at the exterior wall locations(front and rear exterior walls).

wall strap tie requirements: At top and bottom of wall install one Simpson model SP4 at each side of each door or window under 4' or less in width. For doors or windows greater than 4' in width install 2-SP4's each side top and bottom. For garage door opening install 2-SPH4's top and bottom each side. All other wall locations install SP4's top and bottom of wall 4' on center.

Dormers: See attached detail

Porch Columns: Install Simpson model ABU44,ABU46 or ABU66 and Simpson model AC4Max or AC6Max(ACE4Max or ACE6Max may be used for end columns)

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:

At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)

Foundation Requirements:

Stemwall: Minimum size of footer shall be 10" x 20" wide with 2-#5 rebar continuous and 1-#5 vertical rebar 48" on center. All cells shall be filled with concrete. ½" anchor bolts with 2" washers shall be installed 3' on center and 8" from corners each way and at each side of door openings. (3000 psi concrete min.) Porch footer may be reduced to 16" in width.(Note: foundation designed using an allowable bearing pressure of 1000 psf)

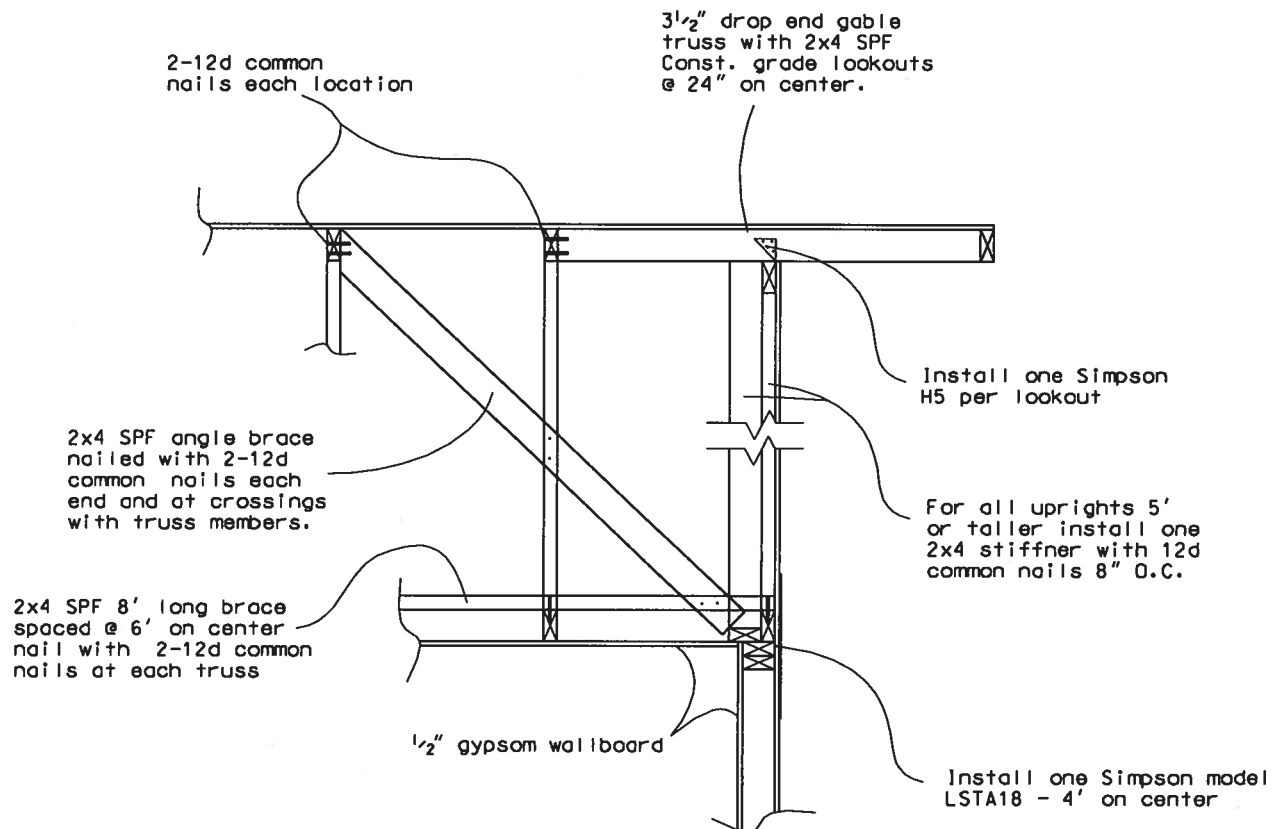
Header Requirements:

Windows, Doors & Porch Beams: Header shall be 2 - #2 SYP 2x10's with ½" plywood/OSB between. .

Garage Door Beams: Header shall be 2-#2 SYP 2x12's with ½" plywood/OSB between

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

Marty D. Dwyer
5-16-06

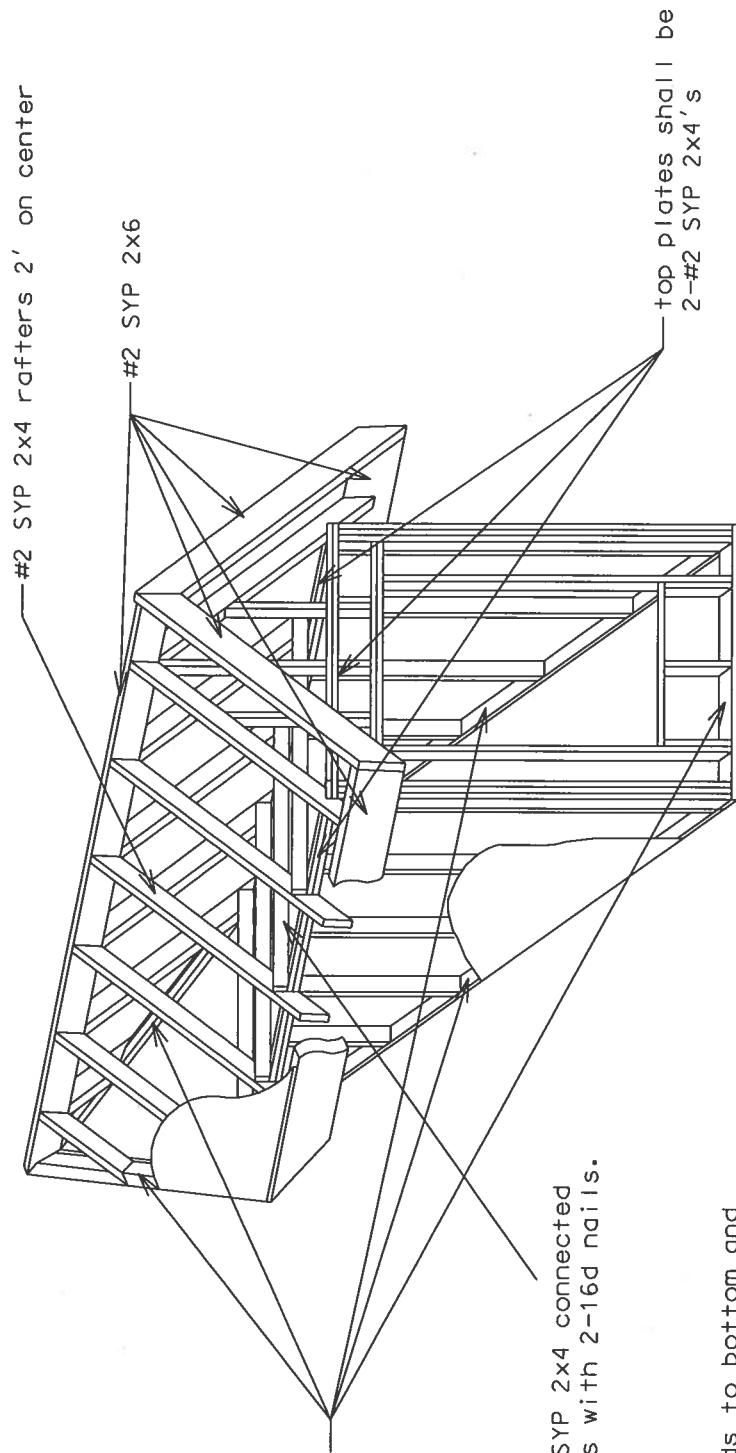


GABLE END BRACING
DETAIL (N.T.S.)

Marty J. Humphries
5-16-06

Mote Residence
Columbia County, FL

DETAIL PREPARED BY:
MARTY J. HUMPHRIES P.E. # 51976
7932 240TH ST., O'BRIEN, FL 32071



bottom plates shall be #2 SYP 2x4's screwed to roof decking with 1- 2 1/2" #10 wood screw 6" O.C. (average)

Tie beam shall be #2 SYP 2x4 connected at each end to rafters with 2-16d nails.

Note:

Connect studs to bottom and top plates with 2-16d nails and connect roof rafters to ridge plate with 2-16d nails. Also connect fascia plate to rafters with 2-16d nails at each location.

top plates shall be 2-#2 SYP 2x4's

DORMER FRAMING DETAIL

Marty J. Murphy
5-16-06

NEW! The H2.5A is symmetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0.

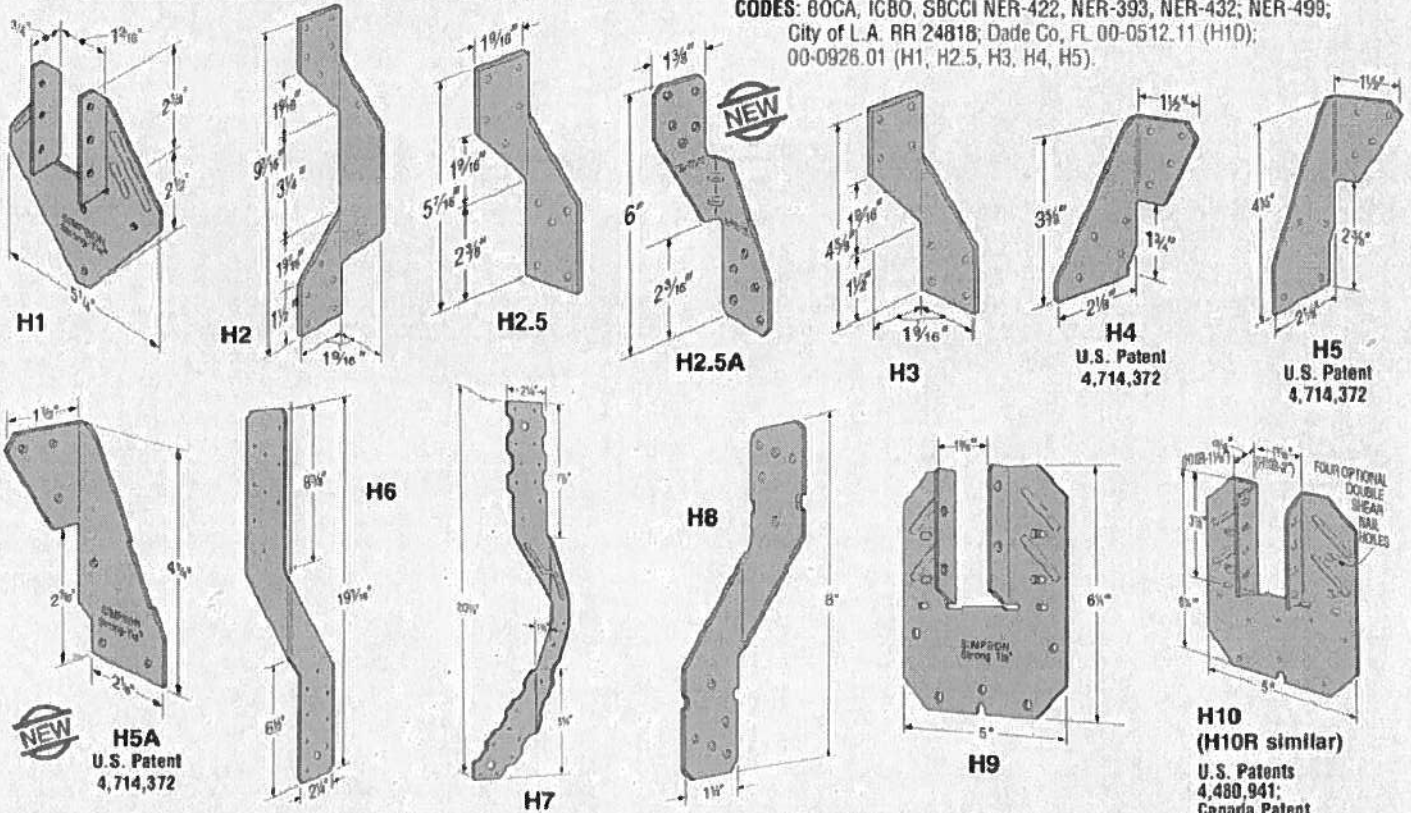
MATERIAL: See table

FINISH: Galvanized; H10-2, H11Z-Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION: • Use all specified fasteners. See General Notes.

- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co. FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5).



Model No.	Ga	Fasteners			Uplift Avg Ull	Doug-Fir Larch/So. Pine Allowable Loads ^{1,2}				Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads ^{1,2}				Uplift Load with 8dx1½ Nails (133 & 160)
		To Rafters/ Truss	To Plates	To Studs		Uplift		Lateral (133/160)			Uplift		Lateral (133/160)		
						(133)	(160)	F ₁	F ₂		(133)	(160)	F ₁	F ₂	
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS"x1½	5-SDS"x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.

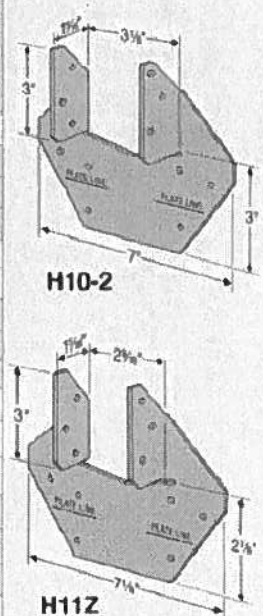
2. Allowable loads are for one anchor. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).

4. The H9KT is sold in 20 piece packs with screws.

5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.

MATERIAL: Z clips—see table. A21 and A23—18 ga.; all other A angles—12 ga.

FINISH: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

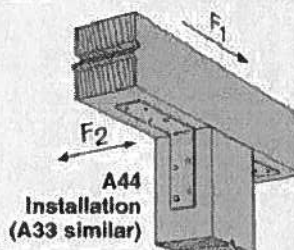
- Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.

CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

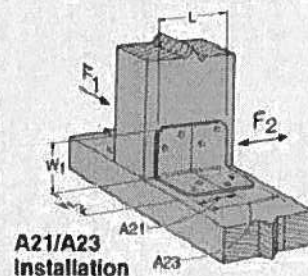
Model No.	Dimensions			Fasteners				Avg Ult F ₂	Allowable Loads ¹ DF/SP			
	W ₁	W ₂	L	Base		Post			(133)		(160)	
				Bolts	Nails	Bolts	Nails		F ₁	F ₂	F ₁	F ₂
A21	2	1½	1¾	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1¾	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4¾	4¾	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5½	5½	1½	2-½	—	2-½	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-¾	—	3-¾	—	N/A	N/A	N/A	N/A	N/A
A24	3½	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3½	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

Model No.	Ga	Dimensions					Fasteners ¹ (Total)	Avg Ullt	Allowable Download (125)
		W	H	B	TF				
Z2	20	2½	1½	1½	1½	4-10d×1½	1507	465	
Z4	12	1½	3½	2½	1¼	2-16d	1450	465	
Z6	12	1½	5½	2	1¼	2-16d	1517	485	
Z28	28	2½	1½	1½	1½	10d×1½ ¹	—	—	
Z38	28	2½	2½	1½	1½	10d×1½ ¹	—	—	
Z44	12	2½	3½	2	1¾	4-16d	2800	865	

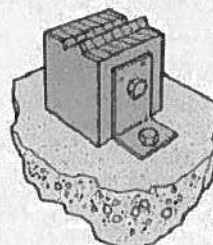
1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.



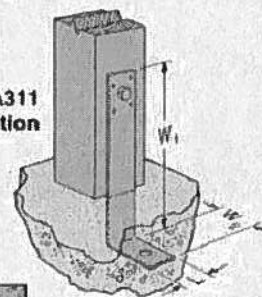
A44 Installation (A33 similar)



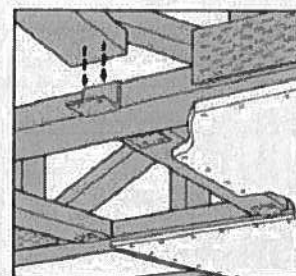
A21/A23 Installation



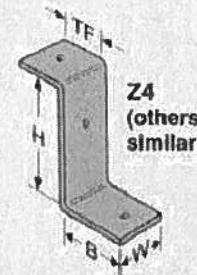
A24 Installation



A311 Installation



Typical Z2 Installation



Z4 (others similar)

SP/SPH/RSP4 STUD PLATE TIES

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

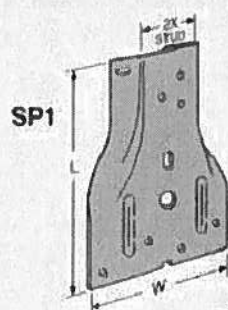
MATERIAL: SPH—18 gauge, all others—20 gauge **FINISH:** Galvanized

INSTALLATION: • Use all specified fasteners; see General Notes.

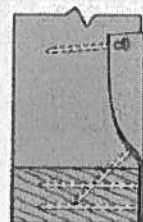
- SP—one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

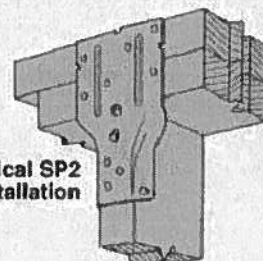
Model No.	Dimensions		Fasteners		Avg Ullt	Allowable Uplift Loads DF/SP	
	W	L	Stud ¹	Plate		(133) ²	(160) ²
SP1	3½	5½	6-10d	4-10d	1950	585	585
SP2	3½	6½	6-10d	6-10d	3300	890	1065
SP3	4½	6½	6-10d	6-10d	3467	890	1065
SP4	3½	7½	6-10d×1½	—	2917	735	885
SP5	4½	5½	6-10d	4-10d	1950	585	585
SP6	5½	7½	6-10d×1½	—	2917	735	885
SP8	7½	8½	6-10d×1½	—	2917	735	885
SPH4	3½	8½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
SPH6	5½	9½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
SPH8	7½	8½	10-10d×1½	—	3993	1240	1240
			12-10d×1½	—	4470	1360	1360
RSP4 (1)	2½	4½	4-8d×1½	4-8d×1½	1032	315	315
RSP4 (2)	2½	4½	4-8d×1½	4-8d×1½	1445	450	450



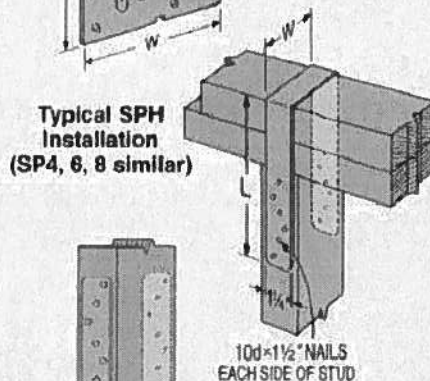
SP1



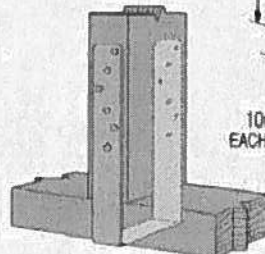
SP1 Nailing Profile



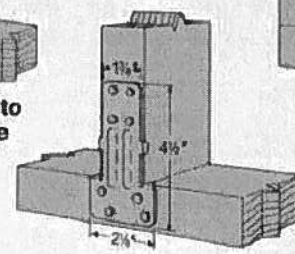
Typical SP2 Installation



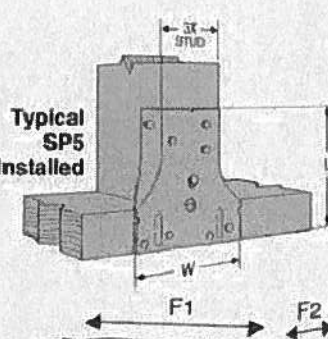
Typical SPH Installation (SP4, 6, 8 similar)



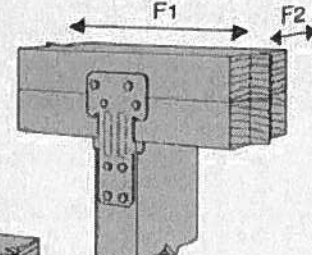
Typical SPH4 Stud to Single Bottom Plate



(1) Typical RSP4 Stud to Single Bottom Plate



Typical SP5 Installed



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

1. SP1, 2, 3 and SP5; drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4—see installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement

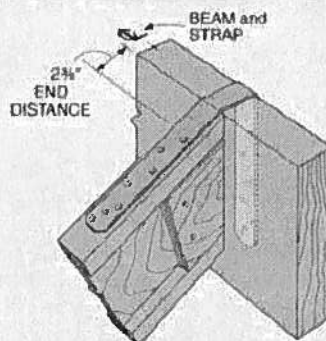
SIMPSON
Strong-Tie
CONNECTIONS

Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 100x1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and

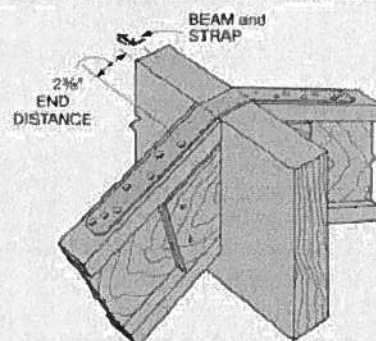
Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22);
City of L.A. RR 25119, RR 25149, RR 25281.



Model No.	Ga	Dimensions		Fasteners (Total)		Allowable Tension Loads		
		W	L	Nails		Floor (100)	(133)	(160)
RPS18	16	1 1/2	18 1/2	12-16d		810	1080	1295
RPS22		1 1/2	22 1/2	16-10d		905	1205	1445
RPS28		1 1/2	28 1/2	12-16d		810	1080	1295
LSTA9		1 1/2	9	8-10d		450	605	725
LSTA12	20	1 1/2	12	10-10d		565	755	905
LSTA15		1 1/2	15	12-10d		680	905	1085
LSTA18		1 1/2	18	14-10d		790	1055	1265
LSTA21		1 1/2	21	16-10d		905	1205	1295
LSTA24		1 1/2	24	18-10d		1015	1295	1295
ST292		2 1/2	9 1/2	12-16d		790	1055	1130
ST2122		2 1/2	12 1/2	16-16d		1070	1425	1505
ST2115		2 1/2	16 1/2	10-16d		450	600	600
ST2215	18	2 1/2	16 1/2	20-16d		1270	1695	1695
LSTA30		1 1/2	30	22-10d		1255	1870	1715
LSTA36		1 1/2	36	28-10d		1480	1715	1715
LSTI49		3 1/2	49	32-10dx1 1/2		1455	1940	2330
LSTI73	18	3 1/2	73	48-10dx1 1/2		2185	2910	3495
MSTA9		1 1/2	9	8-10d		455	610	730
MSTA12		1 1/2	12	10-10d		570	760	910
MSTA15		1 1/2	15	12-10d		685	910	1095
MSTA18	18	1 1/2	18	14-10d		800	1065	1275
MSTA21		1 1/2	21	16-10d		910	1215	1460
MSTA24		1 1/2	24	18-10d		1025	1370	1640
MSTA30		1 1/2	30	22-10d		1265	1685	2025
MSTA36	16	1 1/2	36	26-10d		1495	1995	2135
ST6215		2 1/2	16 1/2	20-16d		1330	1775	2130
ST6224		2 1/2	23 1/2	28-16d		1890	2520	2630
ST9		1 1/2	9	8-16d		530	705	850
ST12	16	1 1/2	11 1/2	10-16d		665	885	1065
ST18		1 1/2	17 1/2	14-16d		900	1200	1200
ST22		1 1/2	21 1/2	18-16d		1025	1370	1370
MSTC28		3	28 1/2	36-16d sinkers		2070	2760	3310
MSTC40	14	3	40 1/2	52-16d sinkers		2990	3985	4740
MSTC52		3	52 1/2	62-16d sinkers		3555	4740	4740
MSTC66		3	65 1/2	76-16d sinkers		4390	5855	5855
MSTC78		3	77 1/2	76-16d sinkers		4390	5855	5855
ST6236	12	2 1/2	33 1/2	40-16d		2575	3430	3430
FHA6		1 1/2	6 1/2	8-16d		550	735	885
FHA9		1 1/2	9	8-16d		550	735	885
FHA12		1 1/2	11 1/2	8-16d		550	735	885
FHA18	12	1 1/2	17 1/2	8-16d		550	735	885
FHA24		1 1/2	23 1/2	8-16d		550	735	885
FHA30		1 1/2	30	8-16d		550	735	885
MST126		2 1/2	26	26-10dx1 1/2		1130	1510	1810
MST136	10	2 1/2	36	36-10dx1 1/2		1565	2090	2505
MST148		2 1/2	48	48-10dx1 1/2		2135	2850	3420
MST160		2 1/2	60	60-10dx1 1/2		2780	3680	4415
MST172		2 1/2	72	72-10dx1 1/2		3310	4415	4725

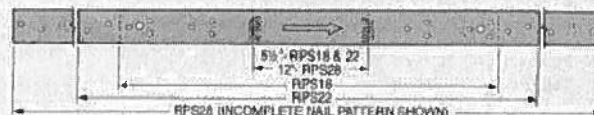


Typical LSTA Installation
(hanger not shown)

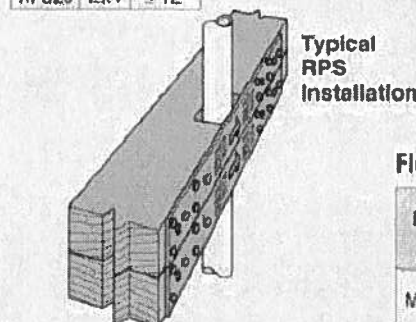


Typical LSTA Installation
(hanger not shown)

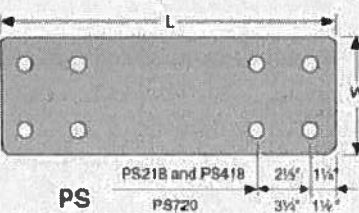
Model No.	Plate	Notch Width
RPS18	2x4	≤ 5 1/2"
RPS22	2x6	≤ 5 1/2"
RPS28	2x4	≤ 12"



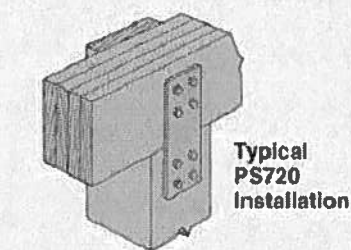
RPS



Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions	Bolts
		W L Qty Dia	
PS218	7	2 18 4 1/2	
PS418		4 18 4 1/2	
PS720		6 1/2 20 8 1/2	

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load	
			(133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	56-16d	5800	5800
MST136	18	14-10dx1 1/2	810	975
	16	16-10dx1 1/2	930	1115
MST148	18	26-10dx1 1/2	1545	1855
	16	28-10dx1 1/2	1660	1990
MST160	18	38-10dx1 1/2	2330	2800
	16	40-10dx1 1/2	2455	2945
MST172	18	50-10dx1 1/2	3065	3680
	16	52-10dx1 1/2	3190	3830

Model No.	Ga	Dimensions		Fasteners (Total)		Allowable Tension Loads					
		W	L	Nails	Bolts	Nails			Bolts ⁵		
				Qty	Dia	Floor (100)	(133)	(160)	Floor (100)	(133)	(160)
MST27	12	2 1/2	27	30-16d	4 1/2	2070	2760	2790	1295	1725	2070
MST37		2 1/2	37 1/2	42-16d	6 1/2	2860	3815	3815	1825	2435	2920
MST48		2 1/2	48	48-16d	8 1/2	3345	4460	4460	2225	2970	3560
MST60		2 1/2	60	56-16d	10 1/2	4350	5800	5800	2670	3565	4275
MST72	10	2 1/2	72	56-16d	10 1/2	4350	5800	5800	2670	3565	4275
HST2		2 1/2	21 1/2	—	8 1/2	—	—	—	3130	4175	5005
HST5		5	21 1/2	—	12 1/2	—	—	—	6385	8510	10210
HST3		3	25 1/2	—	8 1/2	—	—	—	4645	6195	7435
HST6	3	6	25 1/2	—	12 1/2	—	—	—	9350	12465	14955

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1 1/2" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3/4") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST-2 1/2"; HST2 and HST5-4"; HST3 and HST6-4 1/2".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

Locking prongs insert into concrete. The one-piece design assures maximum strength.

MATERIAL: 12 gauge. **FINISH:** Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.

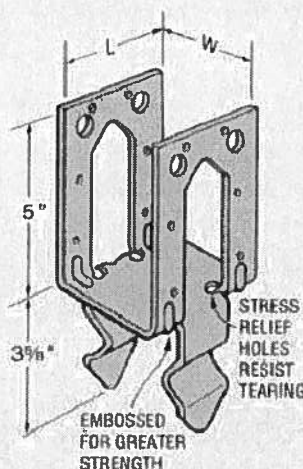
- Holes are provided for installation with either 16d commons or 1/2" bolts for PB66 and PB66R; all other models use 16d commons only.
- A 2" minimum sidecover is required to obtain the full load.
- Not recommended for non-top-supported installations such as fences.

CODES: BOCA, ICBO, SBCCI NER-443, City of LA RR 25149.

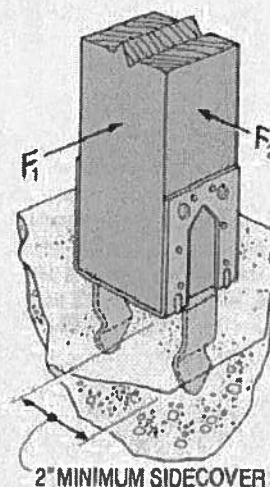
Dade Co. 00-0512.11 (PB44).

Model No.	Dimensions		Uplift Avg Ull	Allowable Loads				
	W	L		12-16d Nails (133 & 160)			2-½ MB	
				Uplift	F ₁	F ₂	Uplift (133 & 160)	
PB44	3⅝	3⅝	4267	1365	765	1325	—	
PB44R	4	3⅝	4267	1365	765	1325	—	
PB46	5⅝	3⅝	4267	1365	765	1325	—	
PB46R	6	3⅝	4267	1365	765	1325	—	
PB66	5⅝	5⅝	5143	1640	765	1325	1640	
PB66R	6	5⅝	5143	1640	765	1325	1640	

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed.



PB



Typical PB Installation

AC/LPC/LCE POST CAPS

The LCE4's universal design provides high capacity while eliminating the need for rights and lefts.

The AC MAX design allows for higher load capacity to match comparable post bases.

LPC—Adjustable design allows greater connection versatility.

MATERIAL: LCE4—20 ga; AC, ACE, LPC4—18 ga; LPC6—16 ga

FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 5.

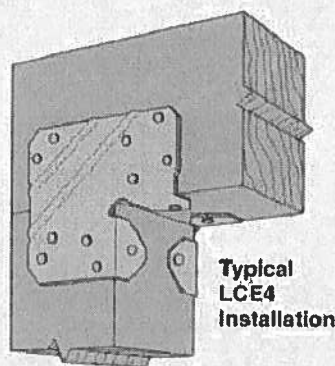
INSTALLATION: • Use all specified fasteners. See General Notes.

- Install all models in pairs. LPC—2 1/2" beams may be used if 10d x 1 1/2" nails are substituted for 10d commons.

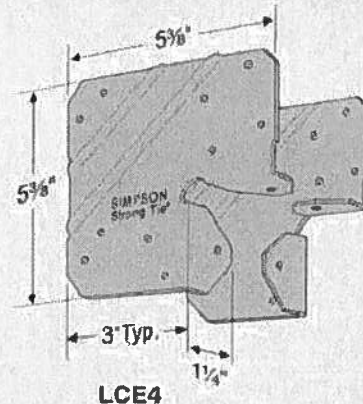
CODES: BOCA, ICBO, SBCCI NER-421, NER-443, NER-469.

City of LA RR 25076; Dade County, FL 99-0623.04 (LPC)

and Dade County, FL 99-0713.05 (AC, ACE).



Typical LCE4 Installation



LCE4

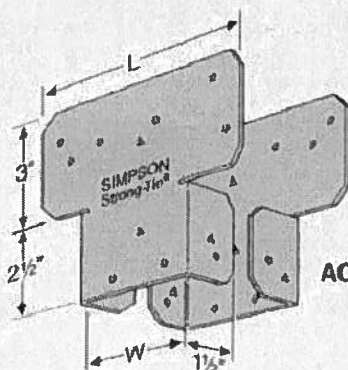
Model No.	Dimensions		Total No. Fasteners		Uplift Avg Ull	Allowable Loads (133 & 160) ¹	
	W	L	Beam	Post		Uplift	Lateral
AC4 MIN	3 3/8	6 1/2	12-16d	8-16d	4467	1430	715
AC4 MAX	3 3/8	6 1/2	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7 1/2	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4 1/2	8-16d	6-16d	4215	1070	715
ACE4 MAX	—	4 1/2	10-16d	10-16d	6238	1785	1070
AC6 MIN	5 1/2	8 1/2	12-16d	8-16d	4467	1430	715
AC6 MAX	5 1/2	8 1/2	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9 1/2	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6 1/2	8-16d	6-16d	4537	1070	715
ACE6 MAX	—	6 1/2	10-16d	10-16d	6432	1785	1070
LPC4	3 3/8	3 1/2	8-10d	8-10d	2333	760	325
LPC6	5 1/8	5 1/2	8-10d	8-10d	2817	915	490
LCE4	—	5 1/2	14-16d	10-16d	5518	1600	1425

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.

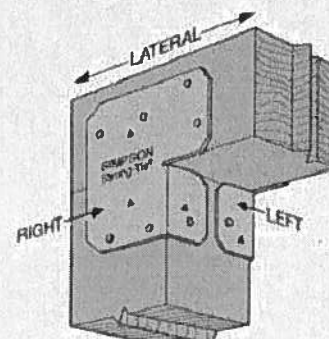
2. Loads apply only when used in pairs.

3. LPC lateral load is in the direction of the beam's axis.

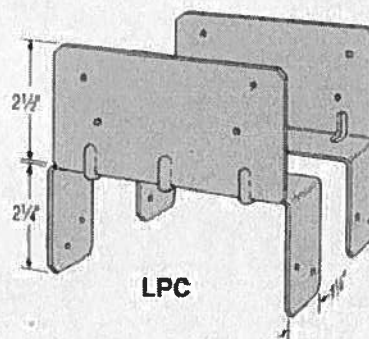
4. MIN nailing quantity and load values — fill all round holes; MAX nailing quantities and load values — fill round and triangle holes.



AC



Typical ACE Installation



LPC

The AB is a fully-adjustable post base which offers moisture protection and finished hardware appearance.

Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

MATERIAL: AB—12 ga plates, 16 ga base cover, all others—see table.
FINISH: Galvanized. Some products available in Z-MAX.

see Corrosion-Resistance, page 5.

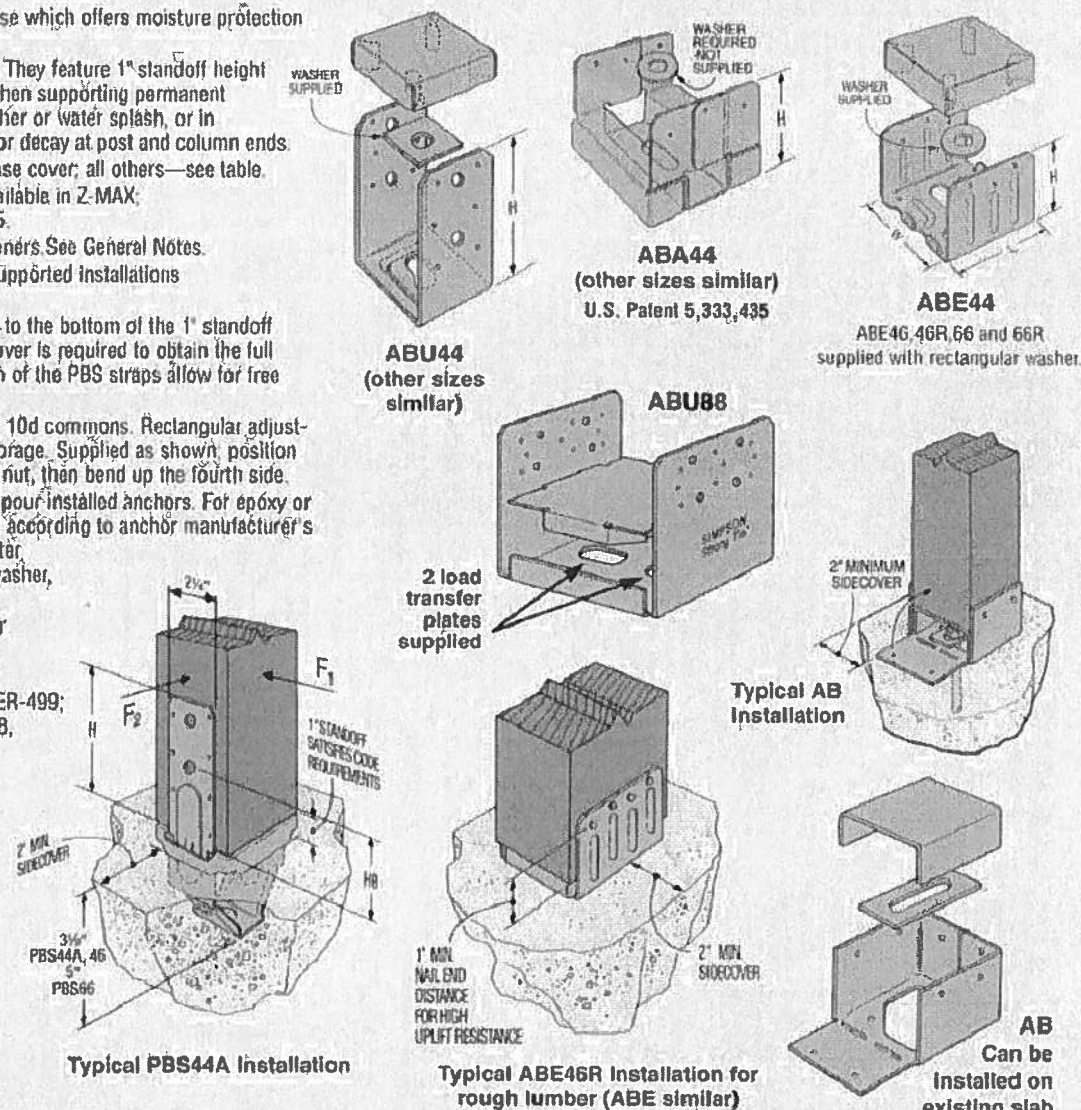
INSTALLATION: • Use all specified fasteners. See General Notes.

- Not recommended for non-top-supported installations such as fences.
- PBS embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full load for PBS. Holes in the bottom of the PBS straps allow for free concrete flow.
- AB—Post nail holes are sized for 10d commons. Rectangular adjustment plate assumes 1/2" dia anchorage. Supplied as shown, position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors.

CODES: BOCA, ICBO, SBCCI NER-393, NER-422, NER-432, NER-469, NER-499; ICBO 5870, City of L.A. RR 24818, RR 25064, 25074, 25158; Dade Co FL 99-0713.05 (ABA, ABE), 00-0512.11 (ABU).

Model No.	Dimensions	Allowable Downloads (100)
	W L	
ABA44	3 3/4 3 3/4	4065
ABA44R	4 4 1/2	4065
ABA46	3 3/4 5 1/2	4165
ABA46R	4 6 6	4165
ABE66	5 5 1/2	5335
ABE66R	6 6 6	5335

1. Loads may not be increased for short-term loading.



Typical PBS44A Installation

Typical ABE46R Installation for rough lumber (ABE similar)

AB Can be installed on existing slab

Model No.	Nominal Post Size	Material		Dimensions				Fasteners				Uplift Avg Ulf	Allowable Loads									
		Base (Ga)	Strap (Ga)	W	L	H	HB	Anch. Dia	Post		Uplift (133)		Uplift (160)		F _t (133 & 160)		F _r (133 & 160)		Down (100)			
									Nails	Bolts Qty Dia	Nails		Bolts	Nails	Bolts	Nails	Bolts	Nails		Bolts		
ABA44	4x4	16	16	3 3/4	3 3/4	3 3/4	—	1/2	6-10d	—	—	2120	555	—	555	—	—	—	—	—	6000	
ABE44	4x4	16	16	3 3/4	3 3/4	2 1/2	—	1/2	6-10d	—	—	1893	520	—	520	—	—	—	—	—	8665	
ABU44	4x4	16	12	3 3/4	3 3/4	1 1/2	1 1/2	1/2	12-16d	2	1/2	7833	2200	1800	2200	2160	—	—	—	—	6665	
PBS44A	4x4	12	14	3 3/4	2 1/2	1 1/2	3 3/4	—	14-16d	2	1/2	7733	2400	2400	2400	2400	1165	230	885	885	6665	
ABA44R	RGH 4x4	16	16	4 1/4	3 3/4	2 1/2	—	1/2	6-10d	—	—	2120	555	—	555	—	—	—	—	—	8000	
ABE44R	RGH 4x4	16	16	4	3 3/4	2 1/2	—	1/2	6-10d	—	—	1893	400	—	400	—	—	—	—	—	8665	
ABE46	4x6	12	16	3 3/4	5 1/2	4 1/2	—	1/2	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335	
PBS46	4x6	12	14	3 3/4	2 1/2	6 3/8	3 3/4	—	14-16d	2	1/2	7733	2400	2400	2400	2400	1165	360	885	885	9335	
ABA46	4x6	14	14	3 3/4	5 1/2	3 1/2	—	1/2	8-16d	—	—	2967	700	—	700	—	—	—	—	—	9435	
ABU46	4x6	12	12	3 3/4	5 1/2	7 1/2	2 1/2	1/2	12-16d	2	1/2	8633	2255	2300	2300	2300	—	—	—	—	10335	
ABE46R	RGH 4x6	12	16	4 1/4	5 1/2	3 1/2	—	1/2	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335	
ABA46R	RGH 4x6	14	14	4 1/4	5 1/2	2 1/2	—	1/2	8-16d	—	—	2967	935	—	935	—	—	—	—	—	12000	
PBS66	6x6	12	12	5 1/2	2 1/2	6 3/8	3 3/4	—	14-16d	2	1/2	13100	2630	3560	3160	4000	1865	570	1700	1700	9335	
ABA66	6x6	14	14	5 1/2	5 1/2	3 1/2	—	1/2	8-16d	—	—	3050	720	—	720	—	—	—	—	—	10665	
ABE66	6x6	12	14	5 1/2	5 1/2	3 1/2	—	1/2	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000	
ABU66	6x6	12	10	5 1/2	5 1/2	6 3/8	1 1/2	1/2	12-16d	2	1/2	8900	2300	2300	2300	2300	—	—	—	—	12000	
ABA66R	RGH 6x6	14	14	6 3/8	5 1/2	2 1/2	—	1/2	8-16d	—	—	3050	985	—	985	—	—	—	—	—	12665	
ABE66R	RGH 6x6	12	14	6 3/8	5 1/2	2 1/2	—	1/2	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000	
ABU88*	8x8	12	14	7 1/2	7 1/2	7 1/2	—	2 1/2	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335	
ABU88R	RGH 8x8	12	14	8	7 1/2	7 1/2	—	2 1/2	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335	

1. Uplift and lateral loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.

2. Downloads may not be increased for short-term loading.

3. Specifier to design concrete for shear capacity.

4. ABU88 and ABU88R may be installed with 8-SDS4X3 wood screws for the same table load.



From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0606-46**

Contractor: Haygood Homes Owners Harry & Lamanda Mote Lot 2 block B the Oaks Subdivision

On the date of June 21, 2006 application 0606-46 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0606-46 when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.
2. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.
3. This is to clarify Florida Residential Code 2004 Section R309 Garage and Carports: R309.2 Separation required. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.
4. On the electrical plans please indicate the electrical panel location and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
5. The dwelling elevation drawing show three dormers to be constructed on the roof truss system. Please have Mr. Marty Humphries show a detail

Done

Done

W with
the Wand
load

plan which will provide a construction diagram and material to be used for these dormers, to include material size and attachment method of the dormers to the trusses.

Done
6. Show the type material and thickness of the flooring material for the bonus room area.

Done
7. The bonus room area is considered as a habitable space, the window area shall meet the requirements of the FRC-2004 sections R310.1.1 Minimum opening area. All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m²). With the minimum net clear opening height shall be 24 inches (610 mm) and a minimum net clear opening width shall be 20 inches (508 mm). Please verify that the 3050 window will meet the above requirements.

Joe Haltiwanger



Plan Examiner
Columbia County Building Department



NOTICE OF INSPECTION AND/OR TREATMENT

24712

Date of Inspection

10 / 24 / 06

Date of Treatment

Bora-care

Pesticide Used

subterranean Termites

Wood-Destroying Organisms Treated

****Notice****

It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address: °

Pestmaster Services of Lake City

879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025

Account: CONTRACTORS
Job: HAYGOOD-MOTE
Designer: A. HIGSMITH
Checker: M. MURRAY
Date: 05-24-06

Permit Number: _____ Lot Number: _____

Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: HAYGOOD-MOTE - MOTE

Standard Loading:

T.C. Live	20 psf
T.C. Dead	10 psf
B.C. Live	0 psf
B.C. Dead	10 psf
Total	40 psf

**ROBBINS
ENGINEERING, INC.**P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135**Engineering Index Sheet**

Index Page 1 of 1

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06052406	05/19/2006		19

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)

ANSI/ASCE 7-02
Wind Speed - 110 mph
Mean Roof Ht. - 15 ft.
Exposure Category - B
Occupancy Factor - 1.00
MWFRS
Enclosed

Notes: Refer to individual
truss design drawings for
special loading
conditions.

Date Mark			Date Mark			Date Mark			Date Mark		
1	05/19/06	A1	2	05/19/06	A2	3	05/19/06	A3	4	05/19/06	A4
5	05/19/06	B1	6	05/19/06	B2	7	05/19/06	C1	8	05/19/06	C2
9	05/19/06	D1	10	05/19/06	D2	11	05/19/06	D3	12	05/19/06	D4
13	05/19/06	D5	14	05/19/06	D6	15	05/19/06	D7	16	05/19/06	D8
17	05/19/06	D9	18	05/19/06	P1	19	05/19/06	P2			

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



HAYGOOD-MOTE

Mark

A1

Ouan

4

Type

SE

Span

510300

P1-H1

7

Left OH

0

Right OH

C

Engineering

T06052406

U# J#HAYGOOD-MOTE MOTE



Scale: 0.121" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 457.4 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 4	SP-#2
BC	0.21	2x 6	SP-#2
WB	0.60	2x 4	SP-#2
WG	---	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	18- 7- 8
TC 2- 0- 0		18- 7- 8	24- 7- 8
TC Cont.		24- 7- 8	51- 3- 0
BC Cont.		0- 0- 0	51- 3- 0
WB 1 rows CLB on DD-AA			
WB 1 rows CLB on AA-B			
WB 1 rows CLB on I -O			
Attach CLB with (2)-10d nails			
at each web.			

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplift Lbs	Size In-Sx	Req'd In-Sx
Q	331	62	3- 8	1- 8
			H _z =	-234
R	1723	250	3- 8	1-13
S	1709	249	3- 8	1-13
E	338	62	3- 8	1- 8
			H _z =	212

Membr	CSI	P	Lbs	Axl-CSI-Bnd
1	1	1	1	1
2	2	2	2	2
3	3	3	3	3
4	4	4	4	4
5	5	5	5	5
6	6	6	6	6
7	7	7	7	7
8	8	8	8	8
9	9	9	9	9
10	10	10	10	10
11	11	11	11	11
12	12	12	12	12
13	13	13	13	13
14	14	14	14	14
15	15	15	15	15
16	16	16	16	16
17	17	17	17	17
18	18	18	18	18
19	19	19	19	19
20	20	20	20	20
21	21	21	21	21
22	22	22	22	22
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81	81	81	81	81
82	82	82	82	82
83	83	83	83	83
84	84	84	84	84
85	85	85	85	85
86	86	86	86	86
87	87	87	87	

-----Top Chords-----						
A	-T	0.10	97	T	0.01	0.09
T	-CC	0.16	33	T	0.01	0.15
C	-Z	0.22	237	T	0.05	0.17
Z	-DD	0.17	656	C	0.00	0.17
DD	-B	0.17	605	C	0.00	0.17
B	-C	0.28	786	C	0.00	0.28
C	-O	0.28	917	C	0.00	0.28
O	-D	0.28	1169	C	0.00	0.28
D	-V	0.28	1073	C	0.00	0.28
V	-Y	0.34	344	T	0.06	0.28
Y	-E	0.14	53	T	0.01	0.13

-----Bottom Chords-----						
A	-Q	0.03	66	C	0.00	0.03
Q	-BB	0.03	226	T	0.00	0.03
BB	-R	0.08	182	T	0.00	0.08
R	-AA	0.14	203	C	0.00	0.14
AA	-I	0.18	593	T	0.04	0.14
I	-H	0.21	1018	T	0.07	0.14
H	-G	0.17	1040	T	0.12	0.05
G	-S	0.19	324	C	0.00	0.19
S	-E	0.19	158	T	0.00	0.19

	Webs		
Q -T	0.02	243	C
T -BB	0.01	67	T
BB-CC	0.02	131	T
CC-R	0.08	258	C
R -Z	0.60	1436	C
Z -AA	0.19	1057	T
DD-AA	0.06	243	C
AA-B	0.14	353	C
B -I	0.07	397	T
I -C	0.03	241	T
I -O	0.11	401	C
H -O	0.03	234	T
H -D	0.03	52	C
G -D	0.15	595	C
G -V	0.27	1511	T
S -V	0.18	1388	C
S -Y	0.07	312	C

TL Defl	-0.08"	in AA-I	L/999
LL Defl	-0.02"	in S -E	L/999
Shear //	Grain	in B -C	0.23

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

BASIC CROSS RANK LIST						
Plate	LOCK	20 Ga,	Gross Area			
Plate	RHS	20 Ga,	Gross Area			
Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	5.0x	7.0	0.1-0.4	0.7	
T	LOCK	3.0x	7.0	Ctr	Ctr	0.47
CC	LOCK	3.0x	7.0	Ctr	Ctr	0.47
Z	LOCK	5.0x	7.0-0.3	0.5	0.86	
DD	LOCK	2.0x	4.0	Ctr	Ctr	0.55
B	LOCK	6.0x	6.0	0.1-4.2	0.91	
C	LOCK	6.0x	6.0-1.0-3.7	0.75		
O	LOCK	3.0x	7.0	Ctr	Ctr	0.52
D	LOCK	6.0x	6.0	Ctr	Ctr	0.88
V	LOCK	3.0x	7.0	Ctr	Ctr	0.53
Y	LOCK	5.0x	7.0	0.1	0.5	0.87
E	LOCK	5.0x	7.0	Ctr	Ctr	0.87
E	LOCK	3.0x	7.0	Ctr	Ctr	0.00
Q	LOCK	2.0x	4.0	Ctr	Ctr	0.55
BB	LOCK	4.0x	6.0	Ctr	Ctr	0.72
R	LOCK	7.0x	8.0	Ctr-0.8	0.72	
AA	LOCK	4.0x	8.0	Ctr	Ctr	0.55

I	LOCK	7.0x	8.0	Ctr-0.8	0.72
H	LOCK	4.0x	6.0	Ctr Ctr	0.47
G	LOCK	7.0x	8.0	Ctr-0.8	0.72
S	LOCK	4.0x	6.0	Ctr Ctr	0.76

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1436 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

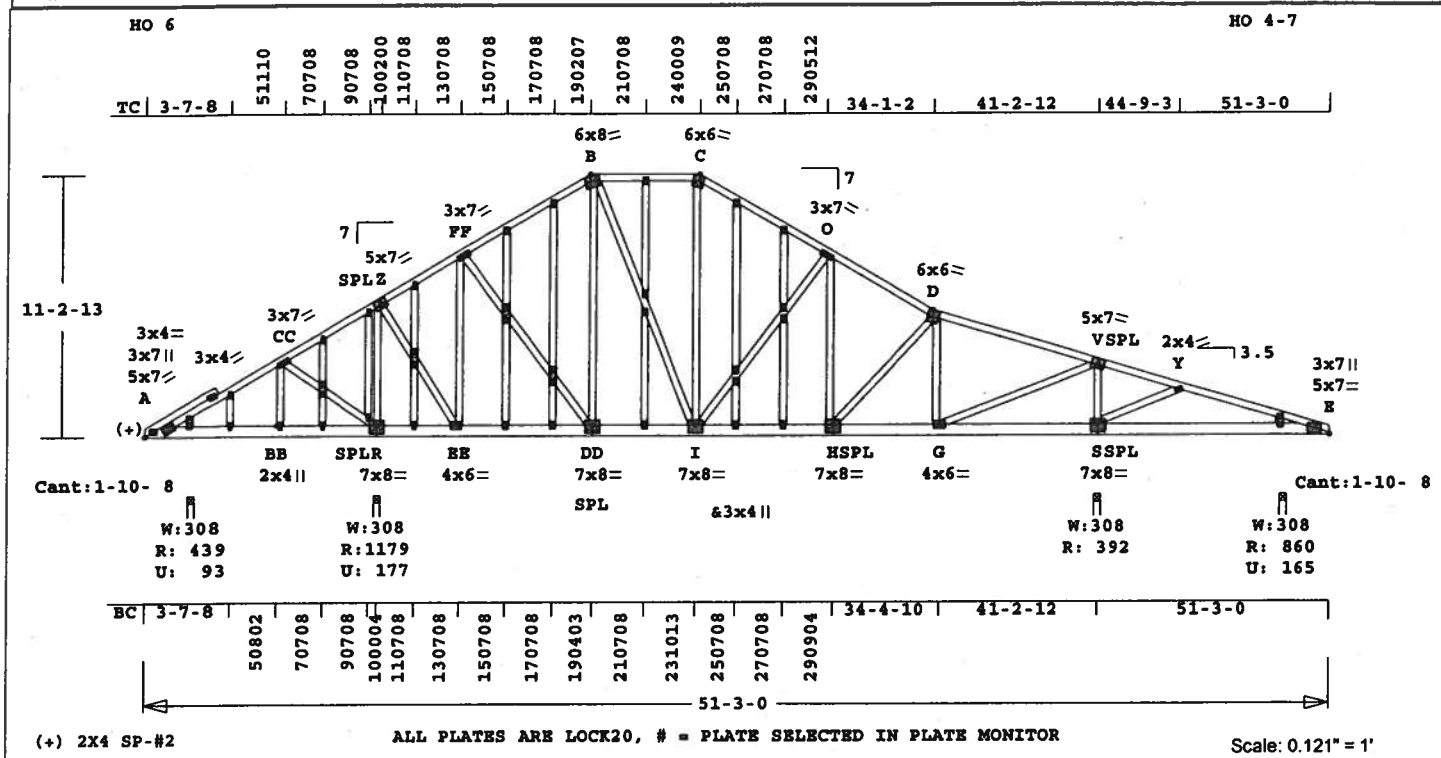


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	A2	1	SP	510300	7	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 1422 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	A3	1	SP	510300	7	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---

Member	Size	SP	#2
TC	0.59	2x 4	SP-#2
BC	0.58	2x 6	SP-#2
WB	0.51	2x 4	SP-#2
GW	0.09	2x 4	SP-#2
WG	---	2x 6	SP-#2
EX E -E	2x 4	SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 2- 7
TC 2- 0- 0	19- 2- 7	24- 0- 9
TC Cont.	24- 0- 9	51- 3- 0
BC Cont.	0- 0- 0	51- 3- 0

Loading Live Dead (psf)

Member	Live	Dead
TC	20.0	10.0
BC	0.0	10.0
Total	20.0	20.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd

Member	Lbs	Lbs	In-Sx	In-Sx
A	440	93	3- 8	1- 8
			Hx =	-132
R	1179	177	3- 8	1- 8
S	392	0	3- 8	1- 8
E	860	165	3- 8	1- 8
			Hx =	-58

Membr CSI P Lbs Ax1-CSI-Bnd

-----Top Chords-----

Member	CSI	P	Lbs	Ax1-CSI-Bnd
A -KK	0.09	482	C	0.00 0.09
KK-CC	0.05	286	C	0.00 0.05
CC-MM	0.05	59	T	0.00 0.05
MM-QQ	0.08	82	T	0.01 0.07
QQ-Z	0.04	62	C	0.00 0.04
Z -SS	0.04	455	C	0.00 0.04
SS-PP	0.03	331	C	0.00 0.03
PP-WW	0.05	491	C	0.00 0.05
WW-AO	0.12	482	C	0.00 0.12
AO-B	0.28	451	C	0.00 0.28
B -GG	0.42	469	C	0.00 0.42
GG-C	0.59	1496	C	0.03 0.56
C -O	0.56	1372	T	0.21 0.35
O -D	0.50	843	T	0.15 0.35

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 583.4 LBS

Member	Weight	Area	Perimeter
D -V	0.52	492	C 0.00 0.52
V -Y	0.40	1762	C 0.02 0.38
Y -E	0.40	2070	C 0.02 0.38

-----Bottom Chords-----

Member	Weight	Area	Perimeter
A -LL	0.14	392	T 0.04 0.10
LL-BB	0.12	192	T 0.02 0.10
BB-PP	0.06	192	T 0.02 0.04
PP-R	0.07	220	T 0.02 0.05
R -VV	0.05	89	T 0.00 0.05
VV-EE	0.02	133	T 0.00 0.02
EE-ZZ	0.05	231	T 0.02 0.03
ZZ-AA	0.06	307	T 0.03 0.03
AA-DD	0.05	387	T 0.04 0.01
DD-JJ	0.06	393	T 0.04 0.02
JJ-I	0.42	1474	T 0.17 0.25
I -H	0.25	706	C 0.00 0.25
H -G	0.30	137	T 0.01 0.29
G -S	0.39	481	T 0.05 0.34
S -E	0.58	2002	T 0.24 0.34

-----Webs-----

Member	Weight	Area	Perimeter
BB-CC	0.02	172	T
CC-OO	0.04	263	C 0.00 0.04
OO-RR	0.08	351	C 0.00 0.08
RR-R	0.08	387	C 0.00 0.08
R -Z	0.28	754	C
Z -TT	0.11	585	T 0.10 0.01
TT-EE	0.09	536	T 0.09 0.00
EE-FF	0.27	380	C
FF-XX	0.04	223	T 0.04 0.00
XX-AA	0.01	57	T 0.01 0.00
AA-DD	0.01	88	C 0.01 0.00
DD-B	0.06	92	T
B -HH	0.01	89	C 0.01 0.00
HH-HH	0.04	0	T 0.00 0.04
HH-I	0.16	1038	C 0.00 0.16
I -C	0.51	2804	T
I -O	0.04	523	C
H -O	0.02	177	T
H -D	0.09	871	C
G -D	0.00	344	C
S -V	0.11	1211	C
S -Y	0.03	398	C

-----Gable Webs-----

Member	Weight	Area	Perimeter
LL-KK	0.04	222	T
OO-MM	0.00	19	T
PP-OO	0.00	49	C
RR-QQ	0.01	153	C 0.01 0.00
TT-SS	0.01	103	T 0.01 0.00
VV-TT	0.01	57	T 0.01 0.00
ZZ-XX	0.00	76	C
AA-AO	0.00	44	T
A3-AA	0.01	116	C 0.01 0.00
HH-GG	0.08	918	C 0.08 0.00
JJ-HH	0.09	1079	C 0.09 0.00

TL Defl -0.26" in H -G L/999

LL Defl -0.14" in H -G L/999

Shear // Grain in D -V 0.30

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 7.0 Ctr-0.9 0.75
A LOCK 3.0x 7.0 Ctr Ctr 0.00
KK LOCK 2.0x 4.0 Ctr Ctr 0.55
CC LOCK 3.0x 7.0 Ctr Ctr 0.47
MM LOCK 2.0x 4.0 Ctr Ctr 0.55
QQ LOCK 2.0x 4.0 Ctr Ctr 0.55
Z LOCK 5.0x 7.0-0.3 0.5 0.86
SS LOCK 2.0x 4.0 Ctr Ctr 0.55
FF LOCK 3.0x 7.0 Ctr Ctr 0.51
WW LOCK 3.0x 4.0 Ctr Ctr 0.36
AA LOCK 3.0x 4.0 Ctr Ctr 0.36
B LOCK 6.0x 8.0 1.1-3.9 0.80
GG LOCK 3.0x 4.0 Ctr Ctr 0.36
C LOCK 6.0x 6.0-1.0-3.7 0.75
O LOCK 3.0x 7.0 Ctr Ctr 0.51
D LOCK 6.0x 6.0 Ctr Ctr 0.88
V LOCK 5.0x 7.0 0.1 0.5 0.87
Y LOCK 2.0x 4.0 Ctr Ctr 0.55
E LOCK 5.0x 7.0 Ctr Ctr 0.87
E LOCK 3.0x 7.0 Ctr Ctr 0.00
LL LOCK 2.0x 4.0 Ctr Ctr 0.55
BB LOCK 2.0x 4.0 Ctr Ctr 0.55
PP LOCK 2.0x 4.0 Ctr Ctr 0.55
R LOCK 7.0x 8.0 Ctr-0.8 0.72
VV LOCK 2.0x 4.0 Ctr Ctr 0.55

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	A3	1	SP	510300	7	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

EE LOCK 4.0x 6.0 Ctr Ctr 0.50
 ZZ#LOCK 3.0x 4.0 Ctr Ctr 0.36
 A3#LOCK 3.0x 4.0 Ctr Ctr 0.36
 DD LOCK 7.0x 8.0 Ctr-0.8 0.72
 JJ#LOCK 3.0x 4.0 Ctr Ctr 0.36
 I LOCK 7.0x 8.0 Ctr-0.9 0.71
 H LOCK 7.0x 8.0 Ctr-0.8 0.72
 G LOCK 4.0x 6.0 Ctr Ctr 0.50
 S LOCK 7.0x 8.0 Ctr-0.8 0.72
 OO LOCK 4.0x 8.0 Ctr Ctr 0.74
 RR LOCK 2.0x 4.0 Ctr Ctr 0.55
 TT LOCK 4.0x10.0 Ctr Ctr 0.90
 XX LOCK 4.0x10.0 Ctr Ctr 0.78
 A2 LOCK 4.0x10.0 Ctr Ctr 0.78
 HH#LOCK 3.0x 4.0 Ctr Ctr 0.50
 II#LOCK 3.0x 4.0 Ctr Ctr 0.51

= Plate Monitor used
 4 Gable studs to be attached
 with 2.0x4.0 plates each end.

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004

WARNING Do Not Cut overframe
 member between outside of
 truss and first tie-plate
 to inside of heel plate.

Design checked for 10 psf non-
 concurrent LL on BC.

Refer to Gen Det 3 series for
 web bracing and plating.

NOTE: USER MODIFIED PLATES

This design may have plates
 selected through a plate
 monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main
 Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Max comp. force 2070 Lbs

Quality Control Factor 1.25

Engineering
T06052406

HO 3-14

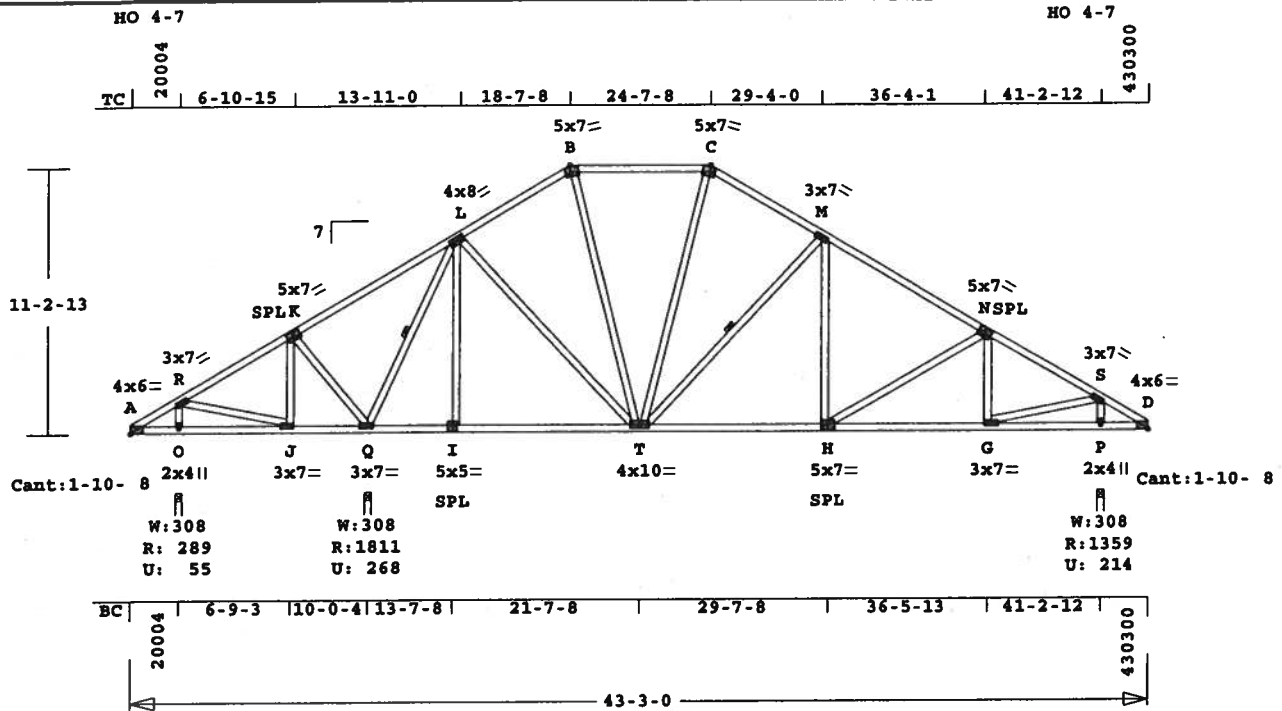


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	A4	8	SP	510300	3.5	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 User-defined wind-exposed BC
 regions --From-- ---To---
 2- 0- 4 10- 2- 0
 41- 0- 8 49- 4- 8
 Max comp. force 1528 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	B1	6	HIPP	430300	7	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



This truss is NOT symmetric.
Proper orientation is essential.

ALL PLATES ARE LOCK20

Scale: 0.123" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 362.5 LBS

P LOCK 2.0x 4.0 Ctr Ctr 0.50

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ----Lumber-----
TC 0.44 2x 4 SP-#2
BC 0.46 2x 4 SP-#2
WB 0.41 2x 4 SP-#2

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 18- 7- 8
TC 2- 0- 0 18- 7- 8 24- 7- 8
TC Cont. 24- 7- 8 43- 3- 0
BC Cont. 0- 0- 0 43- 3- 0
WB 1 rows CLB on Q -L
WB 1 rows CLB on T -M
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
O	289	56	3- 8	1- 8
			Hx =	-239
Q	1811	268	3- 8	1-15
P	1359	214	3- 8	1- 8
			Hx =	240

Membr	CSI	P Lbs	Ax1	CSI-Bnd
-----Top Chords-----				
A -R	0.10	88 T	0.01	0.09
R -K	0.42	79 T	0.02	0.40
K -L	0.44	358 T	0.04	0.40
L -B	0.36	816 C	0.00	0.36
B -C	0.32	720 C	0.00	0.32
C -M	0.36	816 C	0.00	0.36
M -N	0.38	1242 C	0.03	0.35
N -S	0.37	1394 C	0.01	0.36
S -D	0.13	64 T	0.01	0.12
-----Bottom Chords-----				
A -O	0.09	62 C	0.00	0.09

O -J	0.09	237 T	0.00	0.09
J -Q	0.09	173 T	0.00	0.09
Q -I	0.25	349 T	0.03	0.22
I -T	0.38	349 T	0.03	0.35
T -H	0.46	1073 T	0.11	0.35
H -G	0.41	1216 T	0.12	0.29
G -P	0.18	244 T	0.00	0.18
P -D	0.09	45 T	0.00	0.09
-----Webs-----				
O -R	0.01	195 C		
R -J	0.01	66 C		
J -K	0.02	177 T		
K -Q	0.13	407 C		
Q -L	0.41	1579 C	1 Br	
I -L	0.03	251 T		
L -T	0.09	494 T		
B -T	0.03	196 T		
T -C	0.03	196 T		
T -M	0.22	543 C	1 Br	
H -M	0.05	361 T		
H -N	0.13	164 C		
G -N	0.04	215 C		
G -S	0.23	1296 T		
P -S	0.12	1261 C		

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691

ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 0.3 0.2 0.76
R LOCK 3.0x 7.0 Ctr Ctr 0.44
K LOCK 5.0x 7.0-0.3 0.5 0.81
L LOCK 4.0x 8.0 Ctr Ctr 0.89
B LOCK 5.0x 7.0 0.9-3.3 0.71
C LOCK 5.0x 7.0-0.9-3.3 0.71
M LOCK 3.0x 7.0 Ctr Ctr 0.46
N LOCK 5.0x 7.0 0.3 0.5 0.81
S LOCK 3.0x 7.0 Ctr Ctr 0.44
D LOCK 4.0x 6.0-0.3 0.2 0.76
O LOCK 2.0x 4.0 Ctr Ctr 0.50
J LOCK 3.0x 7.0 Ctr Ctr 0.91
Q LOCK 3.0x 7.0-0.9 Ctr 0.59
I LOCK 5.0x 5.0 Ctr-0.5 0.83
T LOCK 4.0x10.0 Ctr Ctr 0.67
H LOCK 5.0x 7.0 Ctr-0.5 0.83
G LOCK 3.0x 7.0 Ctr Ctr 0.91

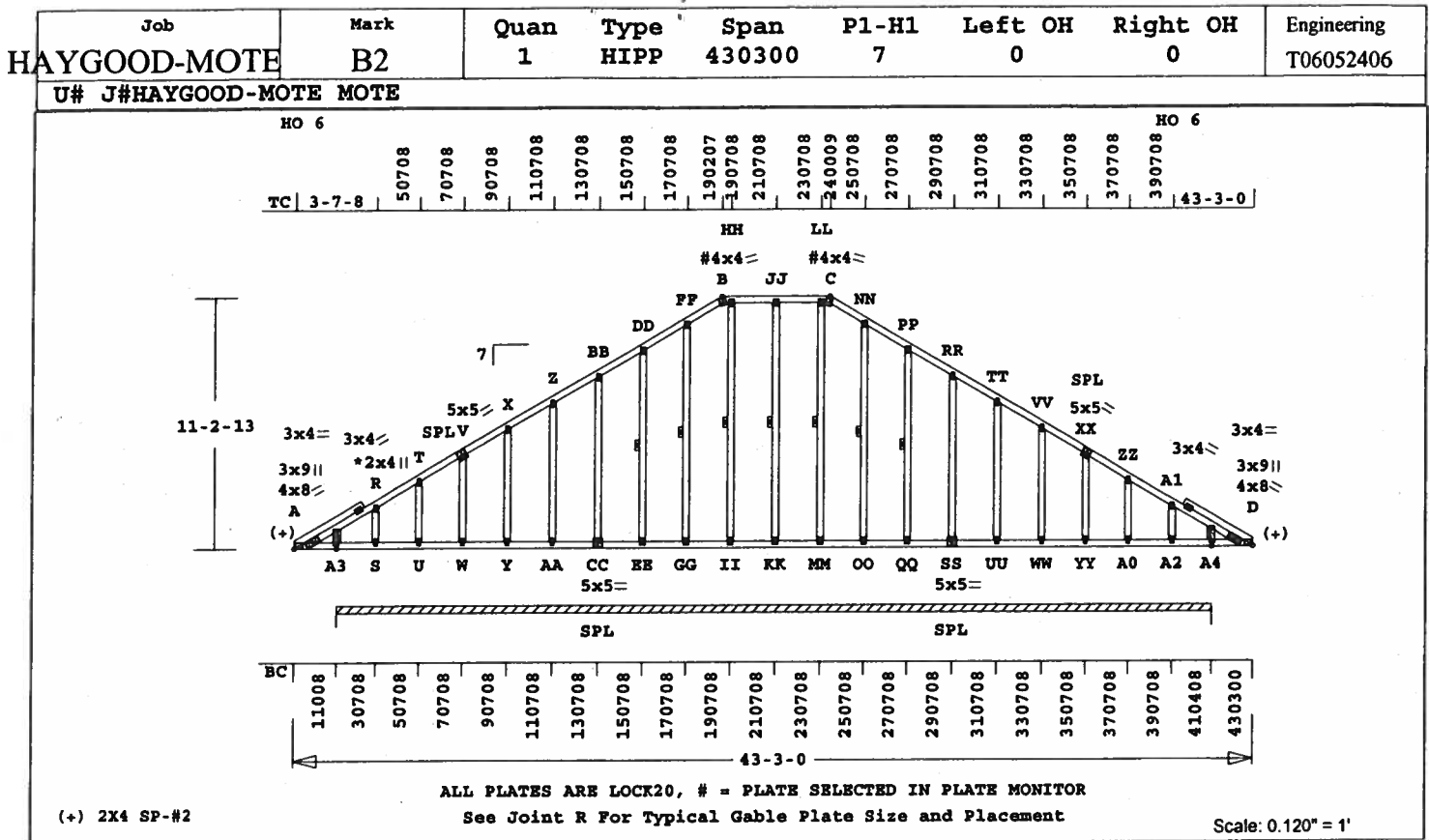
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 1579 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™		APPROX. TRUSS WEIGHT: 426.7 LBS	
Online Plus -- Version 19.0.020		REPORT: NER 691	
RUN DATE: 19-MAY-06		ROBBINS ENGINEERING, INC.	
CSI -Size- ----Lumber----		BASED ON SP LUMBER	
TC 0.06 2x 4 SP-#2		USING GROSS AREA TEST.	
BC 0.06 2x 4 SP-#2		Plate - LOCK 20 Ga, Gross Area	
GW 0.08 2x 4 SP-#2		Plate - RHS 20 Ga, Gross Area	
WG --- 2x 8 SP-#2		Jt Type Plt Size X Y JSI	
Brace truss as follows:		A LOCK 4.0x 8.0 0.9 0.2 0.71	
O.C. From To		A LOCK 3.0x 9.0 Ctr Ctr 0.00	
TC Cont. 0- 0- 0 19- 2- 7		R LOCK 2.0x 4.0 Ctr Ctr 0.00	
TC 2- 0- 0 19- 2- 7 24- 0- 9		T LOCK 2.0x 4.0 Ctr Ctr 0.00	
TC Cont. 24- 0- 9 43- 3- 0		V LOCK 5.0x 5.0 0.3 0.5 0.81	
BC Cont. 0- 0- 0 43- 3- 0		X LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on EE-DD		Z LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on GG-FF		BB LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on II-HH		DD LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on KK-JJ		FF LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on MM-LL		BH LOCK 4.0x 4.0 0.3-0.8 0.80	
WB 1 rows CLB on OO-NN		HH LOCK 2.0x 4.0 Ctr Ctr 0.00	
WB 1 rows CLB on QQ-PP		JJ LOCK 2.0x 4.0 Ctr Ctr 0.00	
Attach CLB with (2)-10d nails		LL LOCK 2.0x 4.0 Ctr Ctr 0.00	
' at each web.		CH LOCK 4.0x 4.0-0.9-1.6 0.95	
Loading Live Dead (psf)		NN LOCK 2.0x 4.0 Ctr Ctr 0.00	
TC 20.0 10.0		PP LOCK 2.0x 4.0 Ctr Ctr 0.00	
BC 0.0 10.0		RR LOCK 2.0x 4.0 Ctr Ctr 0.00	
Total 20.0 20.0 40.0		TT LOCK 2.0x 4.0 Ctr Ctr 0.00	
Spacing 24.0"		VV LOCK 2.0x 4.0 Ctr Ctr 0.00	
Lumber Duration Factor 1.25		XX LOCK 5.0x 5.0 0.3 0.5 0.81	
Plate Duration Factor 1.25		ZZ LOCK 2.0x 4.0 Ctr Ctr 0.00	
TC Fb=1.15 Fc=1.10 Ft=1.10		A1 LOCK 2.0x 4.0 Ctr Ctr 0.00	
BC Fb=1.10 Fc=1.10 Ft=1.10		D LOCK 4.0x 8.0-0.9 0.2 0.71	
Plus 6 Wind Load Case(s)		S LOCK 3.0x 9.0 Ctr Ctr 0.00	
Plus 1 UBC LL Load Case(s)		U LOCK 2.0x 4.0 Ctr Ctr 0.00	
Jt React Uplift Size Req'd			
Lbs Lbs In-Sx In-Sx			
Cont. Brg 1-10- 8 to 41- 4- 8			
3460 538 Hz = 240			
Membr CSI P Lbs Ax1-CSI-Bnd			
-----Top Chords-----			
A -R 0.06 127 C 0.00 0.06			
R -T 0.05 110 C 0.00 0.05			
T -V 0.03 100 C 0.00 0.03			
V -X 0.03 86 C 0.00 0.03			
X -Z 0.03 73 C 0.00 0.03			
Z -BB 0.03 110 T 0.00 0.03			
BB-DD 0.03 151 T 0.00 0.03			
DD-FF 0.03 196 T 0.00 0.03			
FF-B 0.04 224 T 0.02 0.02			
B -HH 0.03 214 T 0.00 0.03			
HH-JJ 0.04 214 T 0.02 0.02			
-----Bottom Chords-----			
A -S 0.06 0 T 0.00 0.06			
S -U 0.03 0 T 0.00 0.03			
U -W 0.02 0 T 0.00 0.02			
W -Y 0.02 0 T 0.00 0.02			
Y -AA 0.02 0 T 0.00 0.02			
AA-CC 0.02 0 T 0.00 0.02			
CC-EE 0.02 0 T 0.00 0.02			
EE-GG 0.02 0 T 0.00 0.02			
GG-II 0.02 0 T 0.00 0.02			
II-KK 0.02 0 T 0.00 0.02			
KK-MM 0.02 0 T 0.00 0.02			
MM-OO 0.02 0 T 0.00 0.02			
OO-QQ 0.02 0 T 0.00 0.02			
QQ-SS 0.02 0 T 0.00 0.02			
SS-UU 0.02 0 T 0.00 0.02			
UU-VV 0.02 0 T 0.00 0.02			
VV-XX 0.02 0 T 0.00 0.02			
XX-YY 0.02 0 T 0.00 0.02			
YY-A0 0.02 0 T 0.00 0.02			
A0-A2 0.03 0 T 0.00 0.03			
A2-D 0.06 0 T 0.00 0.06			
-----Gable Webs-----			
S -R 0.01 145 C			
U -T 0.01 113 C			
W -V 0.02 121 C			
Y -X 0.04 119 C			
AA-Z 0.06 120 C			
CC-BB 0.08 119 C			
EE-DD 0.03 120 C			
GG-FF 0.04 118 C			
II-HH 0.04 108 C			
KK-JJ 0.04 123 C			
MM-LL 0.04 108 C			
OO-NN 0.04 118 C			
QQ-PP 0.03 120 C			
SS-RR 0.08 119 C			
UU-TT 0.06 120 C			
VV-WV 0.04 119 C			
YY-XX 0.02 121 C			
A0-ZZ 0.01 113 C			
A2-A1 0.01 145 C			
TL Defl 0.00" in A2-A4 L/999			
LL Defl 0.00" in A2-A4 L/999			
Shear // Grain in A3-R 0.09			
Plates for each ply each face.			
PLATING CONFORMS TO TPI.			

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	B2	1	HIPP	430300	7	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

W LOCK 2.0x 4.0 Ctr Ctr 0.00
 Y LOCK 2.0x 4.0 Ctr Ctr 0.00
 AA LOCK 2.0x 4.0 Ctr Ctr 0.00
 CC LOCK 5.0x 5.0 Ctr-0.5 0.83
 EE LOCK 2.0x 4.0 Ctr Ctr 0.00
 GG LOCK 2.0x 4.0 Ctr Ctr 0.00
 II LOCK 2.0x 4.0 Ctr Ctr 0.00
 KK LOCK 2.0x 4.0 Ctr Ctr 0.00
 MM LOCK 2.0x 4.0 Ctr Ctr 0.00
 OO LOCK 2.0x 4.0 Ctr Ctr 0.00
 QQ LOCK 2.0x 4.0 Ctr Ctr 0.00
 SS LOCK 5.0x 5.0 Ctr-0.5 0.83
 UU LOCK 2.0x 4.0 Ctr Ctr 0.00
 WW LOCK 2.0x 4.0 Ctr Ctr 0.00
 YY LOCK 2.0x 4.0 Ctr Ctr 0.00
 A0 LOCK 2.0x 4.0 Ctr Ctr 0.00
 A2 LOCK 2.0x 4.0 Ctr Ctr 0.00

= Plate Monitor used

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
 NOTES AND SYMBOLS SHEET FOR
 ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
 Mayo Truss Co. Inc.
 Analysis Conforms To:
 FBC2004

WARNING Do Not Cut overframe
 member between outside of
 truss and first tie-plate
 to inside of heel plate.

Design checked for 10 psf non-

concurrent LL on BC.

Prevent truss rotation at all
bearing locations.

Refer to Gen Det 3 series for
web bracing and plating.

NOTE: USER MODIFIED PLATES

This design may have plates
 selected through a plate
 monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main
Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

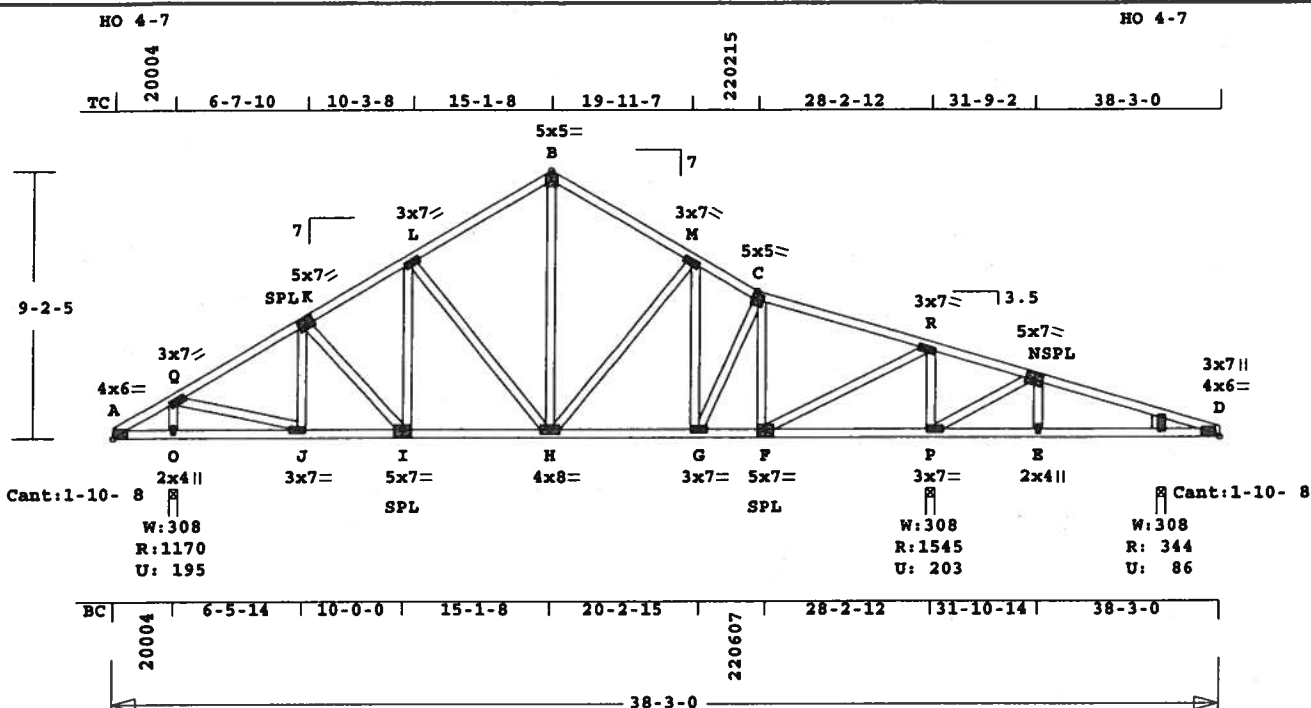
BC Dead Load : 5.0 psf

Max comp. force 145 Lbs

Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	C1	6	SP	380300	7	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



ALL PLATES ARE LOCK20

Scale: 0.151" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 294.4 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

TC	BC	WB	WG	CSI -Size-	Lumber----
0.37	0.24	0.24	---	2x 4	SP-#2
				2x 4	SP-#2
				2x 4	SP-#2
				2x 6	SP-#2

Brace truss as follows:

TC Cont.	BC Cont.	O.C.	From	To
0- 0- 0	0- 0- 0	38- 3- 0		
0- 0- 0	0- 0- 0	38- 3- 0		

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
O	1170	195	3- 8	1- 8
			Hx =	-179
P	1545	203	3- 8	1-10
D	345	86	3- 8	1- 8
			Hx =	153

Membr	CSI	P	Lbs	Ax1-CSI-Bnd
-----Top Chords-----				
A -Q	0.18		48 T	0.01 0.17
Q -K	0.17		1096 C	0.00 0.17
K -L	0.17		1064 C	0.00 0.17
L -B	0.17		827 C	0.00 0.17
B -M	0.17		826 C	0.00 0.17
M -C	0.15		978 C	0.00 0.15
C -R	0.31		865 C	0.00 0.31
R -N	0.37		347 T	0.06 0.31
N -D	0.11		30 T	0.00 0.11
-----Bottom Chords-----				
A -O	0.10		37 T	0.00 0.10
O -J	0.10		196 T	0.00 0.10
J -I	0.20		944 T	0.15 0.05
I -H	0.24		923 T	0.15 0.09
H -G	0.23		852 T	0.14 0.09

G -F	F -P	P -E	E -D	0.20	0.17	0.17	0.06	835 T	325 C	146 T	146 T	0.14	0.00	0.00	0.06

-----Webs-----

O -Q	Q -J	J -K	K -I	I -L	L -H	H -B	H -M	G -M	G -C	F -C	F -R	P -R	P -N	E -N
0.10	0.18	0.03	0.02	0.02	0.24	0.10	0.16	0.01	0.00	0.14	0.23	0.17	0.08	0.02
1072 C	997 T	195 C	63 C	187 T	327 C	529 T	217 C	95 T	42 T	506 C	1293 T	1260 C	375 C	138 T

TL Defl	LL Defl	LL Cant	Shear //	-0.08"	-0.03"	0.00"	Grain in	H -G	L/999	A -O	L/999	C -R	0.24

Plates for each ply each face.

PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 6.0 Ctr Ctr 0.72
Q LOCK 3.0x 7.0 Ctr Ctr 0.43
K LOCK 5.0x 7.0-0.3 0.5 0.76
L LOCK 3.0x 7.0 Ctr Ctr 0.46
B LOCK 5.0x 5.0 Ctr Ctr 0.68
M LOCK 3.0x 7.0 Ctr Ctr 0.46
C LOCK 5.0x 5.0 Ctr Ctr 0.93
R LOCK 3.0x 7.0 Ctr Ctr 0.47
N LOCK 5.0x 7.0 0.1 0.5 0.77
D LOCK 4.0x 6.0 Ctr Ctr 0.84
D LOCK 3.0x 7.0 Ctr Ctr 0.00
O LOCK 2.0x 4.0 Ctr Ctr 0.46
J LOCK 3.0x 7.0 Ctr Ctr 0.88
I LOCK 5.0x 7.0 Ctr-0.5 0.77
H LOCK 4.0x 8.0 Ctr Ctr 0.43
G LOCK 3.0x 7.0 Ctr Ctr 0.55
P LOCK 5.0x 7.0 Ctr-0.5 0.77
P LOCK 3.0x 7.0 Ctr Ctr 0.43
E LOCK 2.0x 4.0 Ctr Ctr 0.46

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

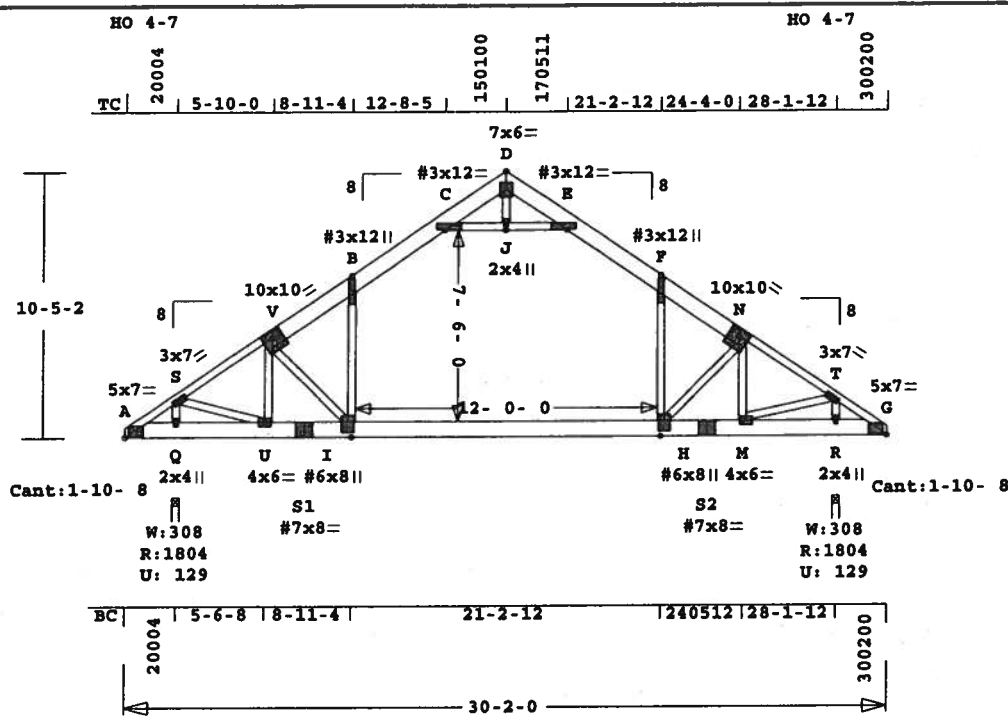
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1260 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D1	3	ATI2	300200	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.132" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 304.5 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ----Lumber-----
TC 0.23 2x 4 SP-#2
EX V -D 2x 8 SP-#1
EX D -N 2x 8 SP-#1
BC 0.37 2x 8 SP-#2
EX S1-S2 2x 8 SP-#1
WB 0.32 2x 4 SP-#2
ACT 0.21 2x 4 SP-#2
AWT 0.01 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 30- 2- 0
BC Cont. 0- 0- 0 30- 2- 0

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00
plf - Live Dead From To
TC V 40 20 0.0' 30.2'
BC V 0 20 0.0' 30.2'
TC V 0 10 9.1' 12.7'
TC V 0 10 17.5' 21.1'
BC V 80 10 9.1' 21.1'
MA V 0 10 12.9' 17.3'
MA V 0 10 0.3' 5.6'
MA V 0 10 0.3' 5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Q	1805	130	3- 8	1-15
			Hx =	-214
R	1804	130	3- 8	1-15
			Hx =	215

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A -S 0.12 113 C 0.00 0.12

S -V	0.23	1712	C	0.02	0.21
V -B	0.76	2105	C	0.00	0.76
B -C	0.76	1478	C	0.00	0.76
C -D	0.68	480	T	0.03	0.65
D -E	0.68	480	T	0.03	0.65
E -F	0.76	1478	C	0.00	0.76
F -N	0.76	2105	C	0.00	0.76
N -T	0.23	1712	C	0.02	0.21
T -G	0.12	113	C	0.00	0.12
-----Bottom Chords-----					
A -Q	0.16	98	T	0.01	0.15
Q -U	0.16	238	T	0.01	0.15
U -S1	0.37	1446	T	0.14	0.23
S1 -I	0.80	1446	T	0.11	0.69
I -H	0.82	1571	T	0.13	0.69
H -S2	0.80	1446	T	0.11	0.69
S2 -M	0.37	1446	T	0.14	0.23
M -R	0.16	238	T	0.01	0.15
R -G	0.16	98	T	0.01	0.15

Q -S	0.17	1489	C		
S -U	0.32	1424	T		
U -V	0.16	881	C		
V -I	0.08	347	T		
I -B	0.21	1123	T		
H -F	0.21	1123	T		
H -N	0.08	347	T		
M -N	0.16	881	C		
M -T	0.32	1424	T		
R -T	0.17	1489	C		
-----Attic Chords (Top)-----					
C -J	0.21	2082	C	0.21	0.00
J -E	0.21	2082	C	0.21	0.00
-----Attic Webs (Top)-----					
J -D	0.01	51	T		

TL Defl	-0.55"	in I -H	L/568
LL Defl	-0.39"	in I -H	L/798
LL Cant	0.03"	in R -G	L/799
Shear //	Grain	in B -C	0.56

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 7.0 Ctr-0.6 0.59
S LOCK 3.0x 7.0 Ctr Ctr 0.65
V LOCK 10.0x10.0 0.8-1.2 0.59
B# LOCK 3.0x12.0 Ctr 1.3 0.33
C# LOCK 3.0x12.0-0.8 Ctr 0.45
D LOCK 7.0x 6.0 Ctr Ctr 0.56
E# LOCK 3.0x12.0 0.8 Ctr 0.45
F# LOCK 3.0x12.0 Ctr 1.3 0.33
N LOCK 10.0x10.0-0.8-1.2 0.59

T LOCK	3.0x 7.0	Ctr Ctr	0.62
G LOCK	5.0x 7.0	Ctr-0.6	0.59
Q LOCK	2.0x 4.0	Ctr Ctr	0.41
U LOCK	4.0x 6.0	Ctr Ctr	0.50
S1# LOCK	7.0x 8.0	Ctr Ctr	0.37
I# LOCK	6.0x 8.0	Ctr-0.9	0.46
H# LOCK	6.0x 8.0	Ctr-0.9	0.46
S2# LOCK	7.0x 8.0	Ctr Ctr	0.37
M LOCK	4.0x 6.0	Ctr Ctr	0.62
R LOCK	2.0x 4.0	Ctr Ctr	0.41
J LOCK	2.0x 4.0	Ctr Ctr	0.41

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-
concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate
monitor.

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D1	3	ATI2	300200	8	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main

Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Unbalanced Loads Checked

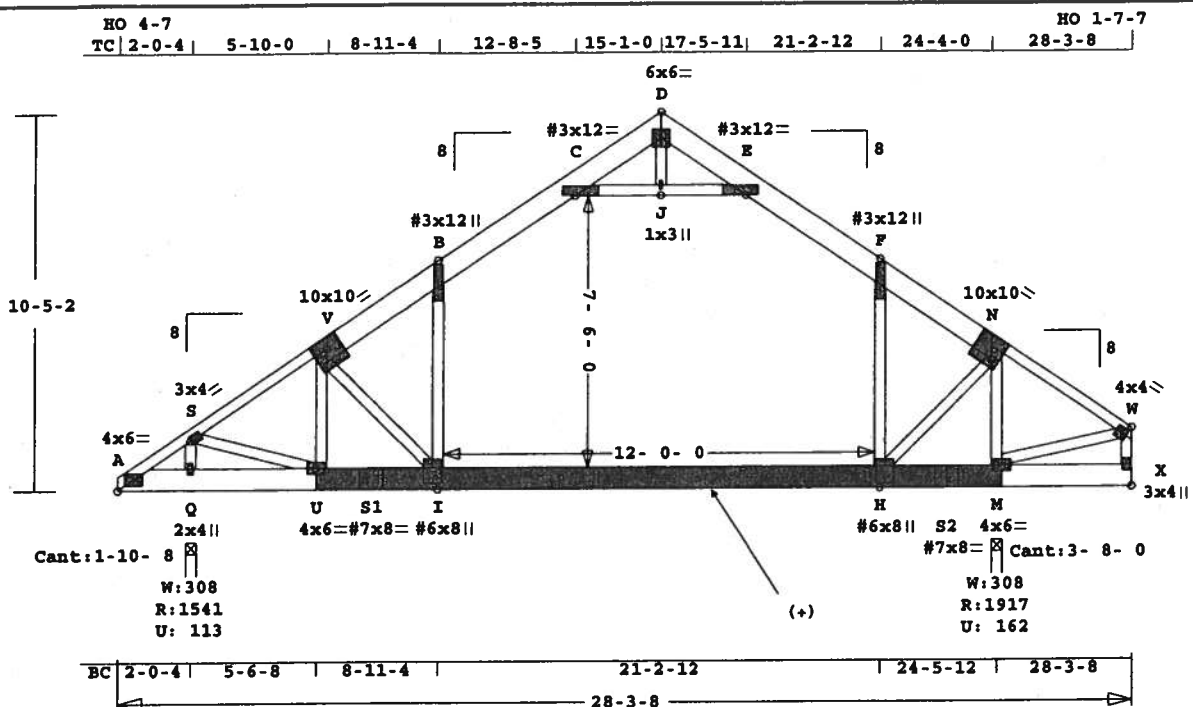
Load Factors = 1.00 and 0.00

Max comp. force 2105 Lbs

Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D2	11	ATI2	280308	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 365.3 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ----Lumber----

TC	0.29	2x 4	SP-#2
EX V -D	2x 8	SP-#1	
EX D -N	2x 8	SP-#1	
BC	0.24	2x 8	SP-#2
EX S1-S2	2x 8	SP-#1	
WB	0.45	2x 4	SP-#2
ACT	0.13	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2

SCB (1) 2x 8 SP-#2 (+)

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 3- 8
BC Cont.	0- 0- 0	28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00			
Plate Duration Factor	1.00			
plf - Live	Dead	From	To	
TC V	40	20	0.0'	28.3'
BC V	0	20	0.0'	28.3'
TC V	0	10	9.1'	12.7'
TC V	0	10	17.5'	21.1'
BC V	80	10	9.1'	21.1'
MA V	0	10	12.9'	17.3'
MA V	0	10	0.3'	5.6'
MA V	0	10	0.3'	5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
Q	1542	114	3- 8	1-10
			Hx =	-209
M	1917	162	3- 8	2- 1
			Hx =	231

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----

A -S	0.11	73 C	0.00	0.11
S -V	0.24	1427 C	0.01	0.23
V -B	0.81	1549 C	0.00	0.81
B -C	0.81	1004 C	0.00	0.81
C -D	0.25	338 T	0.01	0.24
D -E	0.58	224 T	0.00	0.58
E -F	0.58	1139 C	0.00	0.58
F -N	0.11	1303 C	0.00	0.11
N -W	0.29	181 T	0.05	0.24

Bottom Chords

A -Q	0.13	63 T	0.00	0.13
Q -U	0.13	225 T	0.00	0.13
U -S1	0.19	1207 T	0.15	0.04
S1 -I	0.22	1207 T	0.09	0.13
I -H	0.63	1083 T	0.10	0.53
H -S2	0.53	188 T	0.00	0.53
S2 -M	0.24	188 T	0.00	0.24
M -X	0.05	15 C	0.00	0.05

Webbs

Q -S	0.15	1287 C
S -U	0.27	1211 T
U -V	0.09	493 C
V -I	0.06	232 C
I -B	0.18	1000 T
H -F	0.06	404 T
H -N	0.42	1821 T
M -N	0.45	2401 C
M -W	0.02	146 C
X -W	0.01	88 T

Attic Chords (Top)

C -J	0.13	1324 C	0.13	0.00
J -E	0.13	1324 C	0.13	0.00

Attic Webs (Top)

J -D	0.01	44 T
------	------	------

TL Defl	-0.33"	in S1-I	L/808
LL Defl	-0.22"	in I -H	L/999
LL Cant	0.04"	in A -Q	L/573
Shear // Grain		in B -C	0.43

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area	
Plate - RHS 20 Ga, Gross Area	
Jt Type	Plt Size X Y JSI
A LOCK	4.0x 6.0 Ctr Ctr 0.62
S LOCK	3.0x 4.0 Ctr Ctr 0.99
V LOCK	10.0x10.0 0.8-1.2 0.57
B# LOCK	3.0x12.0 Ctr 1.3 0.29
C# LOCK	3.0x12.0-0.8 Ctr 0.28
D LOCK	6.0x 6.0 Ctr Ctr 0.60
E# LOCK	3.0x12.0 0.8 Ctr 0.28
F# LOCK	3.0x12.0 Ctr 1.3 0.27
N LOCK	10.0x10.0-0.8-1.2 0.63
W LOCK	4.0x 4.0 Ctr Ctr 0.73

Q LOCK	2.0x 4.0 Ctr Ctr 0.40
U LOCK	4.0x 6.0 Ctr Ctr 0.48
S1#LOCK	7.0x 8.0 Ctr Ctr 0.36
I# LOCK	6.0x 8.0 Ctr-1.1 0.40
H# LOCK	6.0x 8.0 Ctr-1.8 0.65
S2#LOCK	7.0x 8.0 Ctr Ctr 0.66
M LOCK	4.0x 6.0 Ctr Ctr 0.61
X LOCK	3.0x 4.0 Ctr Ctr 0.89
J LOCK	1.0x 3.0 Ctr Ctr 0.81

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
(+) Fasten each scab with 2 row(s)
of 10d nails at 6 in o.c.
along entire length.
Design checked for 10 psf non-
concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates
selected through a plate

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

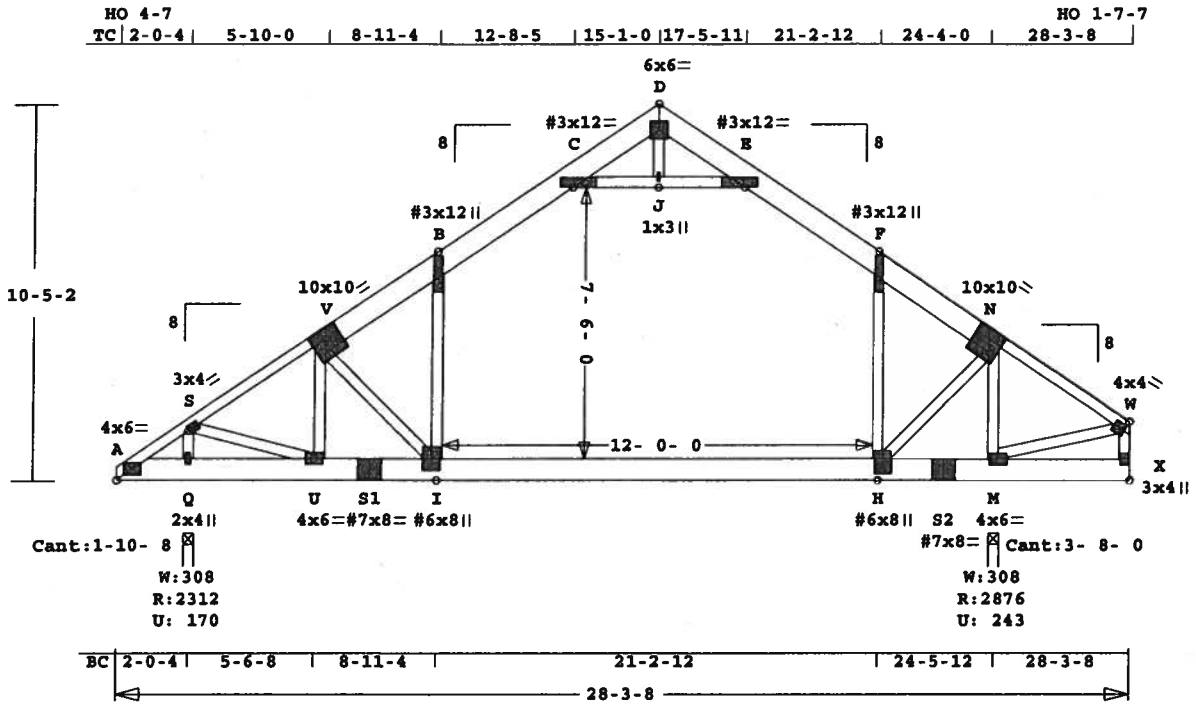


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D2	11	ATI2	280308	8	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

monitor.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 2401 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D3	2*2P	ATI2	280308	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 293.2 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

* 2-Ply Truss *

CSI -Size- ----Lumber----

TC	0.25	2x 4	SP-#2
EX V -D	2x 8	SP-#1	
EX D -N	2x 8	SP-#1	
BC	0.41	2x 8	SP-#2
EX S1-S2	2x 8	SP-#1	
WB	0.31	2x 4	SP-#2
ACT	0.11	2x 4	SP-#2
AWT	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC	2- 0- 0	0- 0- 0 28- 3- 8
BC	2- 0- 0	0- 0- 0 28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			36.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Attic Loading

Lumber Duration Factor	1.00			
Plate Duration Factor	1.00			
plf - Live	Dead	From	To	
TC V	60	30	0.0'	28.3'
BC V	0	30	0.0'	28.3'
TC V	0	15	9.1'	12.7'
TC V	0	15	17.5'	21.1'
BC V	120	15	9.1'	21.1'
MA V	0	15	12.9'	17.3'
MA V	0	15	0.3'	5.6'
MA V	0	15	0.3'	5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
Q	2312	171	3- 8	1- 8
			Hx =	-314
M	2876	243	3- 8	1- 9
			Hx =	347

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -S	0.09	109	C	0.00	0.09
S -V	0.20	2140	C	0.01	0.19
V -B	0.70	2324	C	0.00	0.70
B -C	0.70	1506	C	0.00	0.70
C -D	0.22	508	T	0.01	0.21
D -E	0.50	337	T	0.00	0.50
E -F	0.50	1708	C	0.00	0.50
F -N	0.09	1954	C	0.00	0.09
N -W	0.25	271	T	0.04	0.21
-----Bottom Chords-----					
A -Q	0.10	95	T	0.00	0.10
Q -U	0.10	338	T	0.00	0.10
U -S1	0.18	1811	T	0.12	0.06
S1 -I	0.29	1811	T	0.07	0.22
I -H	0.97	1624	T	0.08	0.89
H -S2	0.89	282	T	0.00	0.89
S2 -M	0.41	282	T	0.00	0.41
M -X	0.04	23	C	0.00	0.04
-----Webs-----					
Q -S	0.11	1931	C		
S -U	0.20	1817	T		
U -V	0.04	739	C		
V -I	0.02	349	C		
I -B	0.13	1500	T		
H -F	0.04	606	T		
H -N	0.31	2731	T		
M -N	0.22	3602	C		
M -W	0.01	220	C		
X -W	0.01	133	T	WindLd	
-----Attic Chords (Top)-----					
C -J	0.11	1986	C	0.11	0.00
J -E	0.11	1986	C	0.11	0.00
-----Attic Webs (Top)-----					
J -D	0.00	67	T		

TL Defl	-0.36"	in I -H	L/739
LL Defl	-0.26"	in I -H	L/999
LL Cant	0.03"	in A -Q	L/763
Shear //		Grain in I -H	0.47

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	- LOCK 20 Ga, Gross Area			
Plate	- RHS 20 Ga, Gross Area			
Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 6.0	Ctr	Ctr	0.62
S LOCK	3.0x 4.0	Ctr	Ctr	0.74
V LOCK	10.0x10.0	0.8-1.2	0.57	
B# LOCK	3.0x12.0	Ctr	1.3	0.27
C# LOCK	3.0x12.0	0.8	Ctr	0.27
D LOCK	6.0x 6.0	Ctr	Ctr	0.60
E# LOCK	3.0x12.0	0.8	Ctr	0.27
F# LOCK	3.0x12.0	Ctr	1.3	0.27

N LOCK	10.0x10.0	0.8-1.2	0.57	
W LOCK	4.0x 4.0	Ctr	Ctr	0.73
Q LOCK	2.0x 4.0	Ctr	Ctr	0.40
U LOCK	4.0x 6.0	Ctr	Ctr	0.48
S1#LOCK	7.0x 8.0	Ctr	Ctr	0.36
I# LOCK	6.0x 8.0	Ctr	Ctr	0.40
H# LOCK	6.0x 8.0	Ctr	1.1	0.43
S2#LOCK	7.0x 8.0	Ctr	Ctr	0.50
M LOCK	4.0x 6.0	Ctr	Ctr	0.61
X LOCK	3.0x 4.0	Ctr	Ctr	0.89
J LOCK	1.0x 3.0	Ctr	Ctr	0.80

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
pattern. (1/2" bolts -OR-
SDS3 screws -OR- 10d nails
as each layer is applied.)
-----Spacing (In)-----

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



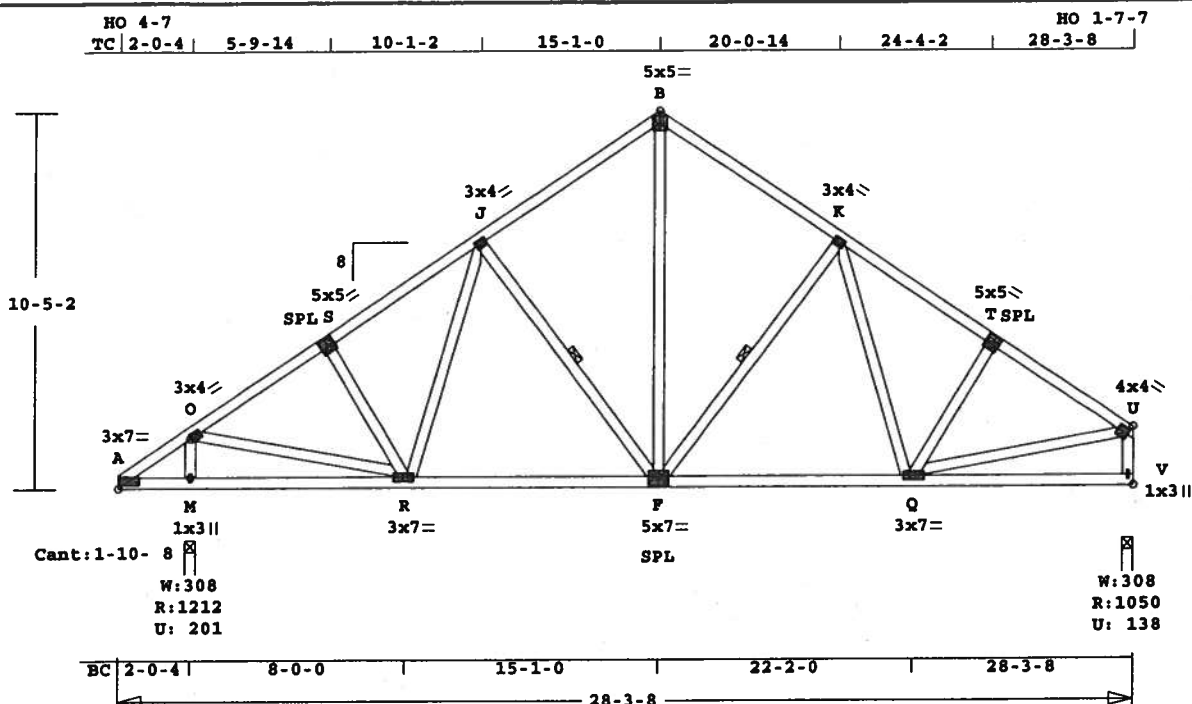
Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D3	2*2P	ATI2	280308	8	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

	Rows	Nails	Screws	Bolts
TC	1	12	24	0
BC	2	12	24	0
WB	1	8	8	

Provide connection to bearing
 for 346 Lbs Horiz Reaction
 Design checked for 10 psf non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.
 NOTE: USER MODIFIED PLATES
 This design may have plates
 selected through a plate
 monitor.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 3602 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D4	2	TR	280308	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



ALL PLATES ARE LOCK20

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 235.4 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---
TC 0.21 2x 4 SP-#2
BC 0.34 2x 4 SP-#2
WB 0.20 2x 4 SP-#2

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 28- 3- 8
BC Cont. 0- 0- 0 28- 3- 8
WB 1 rows CLB on J -F
WB 1 rows CLB on F -K
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
M	1213	202	3- 8	1- 8
			Hz =	-217
V	1050	139	3- 8	1- 8
			Hz =	240

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -O	0.13	48 C	0.01 0.12
O -S	0.13	1173 C	0.01 0.12
S -J	0.21	1044 C	0.00 0.21
J -B	0.21	827 C	0.00 0.21
B -K	0.20	827 C	0.00 0.20
K -T	0.20	1070 C	0.00 0.20
T -U	0.14	1198 C	0.00 0.14

-----Bottom Chords-----			
A -M	0.16	51 T	0.00 0.16
M -R	0.22	227 T	0.00 0.22
R -F	0.33	857 T	0.08 0.25
F -Q	0.34	867 T	0.08 0.26
Q -V	0.26	222 T	0.00 0.26

-----Webs-----			
M -O	0.10	1090 C	
O -R	0.18	1020 T	
S -R	0.05	190 C	
R -J	0.02	163 T	
J -F	0.06	289 C	1 Br
F -B	0.20	621 T	
B -K	0.07	306 C	1 Br
K -Q	0.02	190 T	
Q -T	0.05	213 C	
T -U	0.19	1032 T	
V -U	0.09	998 C	WindLd

TL Defl -0.10" in R -F L/999
LL Defl -0.04" in Q -V L/999
LL Cant 0.00" in A -M L/999
Shear // Grain in J -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	Type	Plt Size	X	Y	JSI
A LOCK	3.0x	7.0	1.0	0.7	0.82
O LOCK	3.0x	4.0	Ctr	Ctr	0.72
S LOCK	5.0x	5.0-0.3	0.5	0.65	
J LOCK	3.0x	4.0	Ctr	Ctr	0.67
B LOCK	5.0x	5.0	Ctr	Ctr	0.58
K LOCK	3.0x	4.0	Ctr	Ctr	0.67
T LOCK	5.0x	5.0	0.3	0.5	0.65
U LOCK	4.0x	4.0	Ctr	Ctr	0.73
M LOCK	1.0x	3.0	Ctr	Ctr	0.81
R LOCK	3.0x	7.0-0.5	Ctr	0.91	
F LOCK	5.0x	7.0	Ctr-0.5	0.67	
Q LOCK	3.0x	7.0	0.5	Ctr	0.91
V LOCK	1.0x	3.0	Ctr	Ctr	0.81

REVIEWED BY:

Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main

Wind-Force Resistance System.
Wind Speed: 110 mph

Mean Roof Height: 15-0
Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 1198 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



HAYGOOD-MOTE

U# J#HAYGOOD-MOTE MOTE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 252.5 LBS

Tampa, FL 33682

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
CW	0.10	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 3- 8
BC Cont.	0- 0- 0	28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber	Duration	Factor	1.25
Plate	Duration	Factor	1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
O	1213	202	3- 8	1- 8
			Hz =	-216
V	1050	139	3- 8	1- 8
			Hz =	239

Membr	CSI	P Lbs	Axl-CSI-Bnd
-----Top Chords-----			
A -Q	0.14	52 T	0.01 0.13
Q -S	0.14	1170 C	0.01 0.13
S -T	0.27	1046 C	0.00 0.27
T -B	0.27	925 C	0.00 0.27
B -M	0.35	1230 C	0.00 0.35
M -N	0.24	1241 C	0.02 0.22
N -U	0.21	1402 C	0.01 0.20
-----Bottom Chords-----			
A -O	0.13	48 T	0.00 0.13
O -J	0.28	224 T	0.00 0.28
J -G	0.28	44 C	0.00 0.28
F -H	0.22	746 T	0.07 0.15
H -E	0.28	1203 T	0.20 0.08
D -V	0.05	20 C	0.00 0.05
-----Chord-Webs-----			

G - F	0.08	108	T	0.01	0.07
F - B	0.10	438	T	0.07	0.03
D - E	0.03	50	T	0.00	0.03
E - N	0.07	113	C	0.00	0.07

-----Webb

O	-Q	0.10	1103	C
Q	-J	0.18	1022	T
S	-J	0.04	177	C
J	-T	0.04	74	T
J	-F	0.16	919	T
T	-F	0.16	239	C
B	-H	0.24	552	T
H	-M	0.12	291	C
H	-N	0.07	190	C
E	-U	0.22	1201	T
E	-V	0.03	228	T
V	-U	0.09	1017	C WindLd

TL Defl	-0.17"	in J	-G	L/999
LL Defl	-0.08"	in J	-G	L/999
LL Cant	0.00"	in A	-O	L/999
Shear //	Grain	in T	-B	0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate	-	LOCK	20	Ga,	Gross	Area
Plate	-	RHS	20	Ga,	Gross	Area
Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	3.0x	7.0	1.0	0.7	0.8
Q	LOCK	3.0x	4.0	Ctr	Ctr	0.72
S	LOCK	5.0x	5.0	-0.3	0.5	0.65
T	LOCK	3.0x	4.0	Ctr	Ctr	0.93
B#	LOCK	6.0x	8.0	-0.1	-1.5	0.78
M	LOCK	1.0x	3.0	Ctr	Ctr	0.80
N	LOCK	5.0x	7.0	0.3	0.5	0.69
U	LOCK	4.0x	4.0	Ctr	Ctr	0.73
O	LOCK	1.0x	3.0	Ctr	Ctr	0.81
J	LOCK	3.0x	9.0	Ctr	Ctr	0.96
G	LOCK	2.0x	4.0	Ctr	Ctr	0.58
F#	LOCK	6.0x10.0	-0.1	1.3	0.58	
H	LOCK	3.0x	7.0	Ctr	Ctr	0.60
E	LOCK	5.0x	9.0	Ctr	0.8	0.58
D	LOCK	2.0x	4.0	Ctr	Ctr	0.58
V	LOCK	4.0x	4.0	Ctr	Ctr	0.61

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

```

Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1402 Lbs
Quality Control Factor 1.25

```

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



HAYGOOD-MOTE

Mark
D6

Qua
3

Type
SP

Span
300200

P1-H1
8

Left OH
0

Right OH
0

Engineering
T06052406

NO 4-

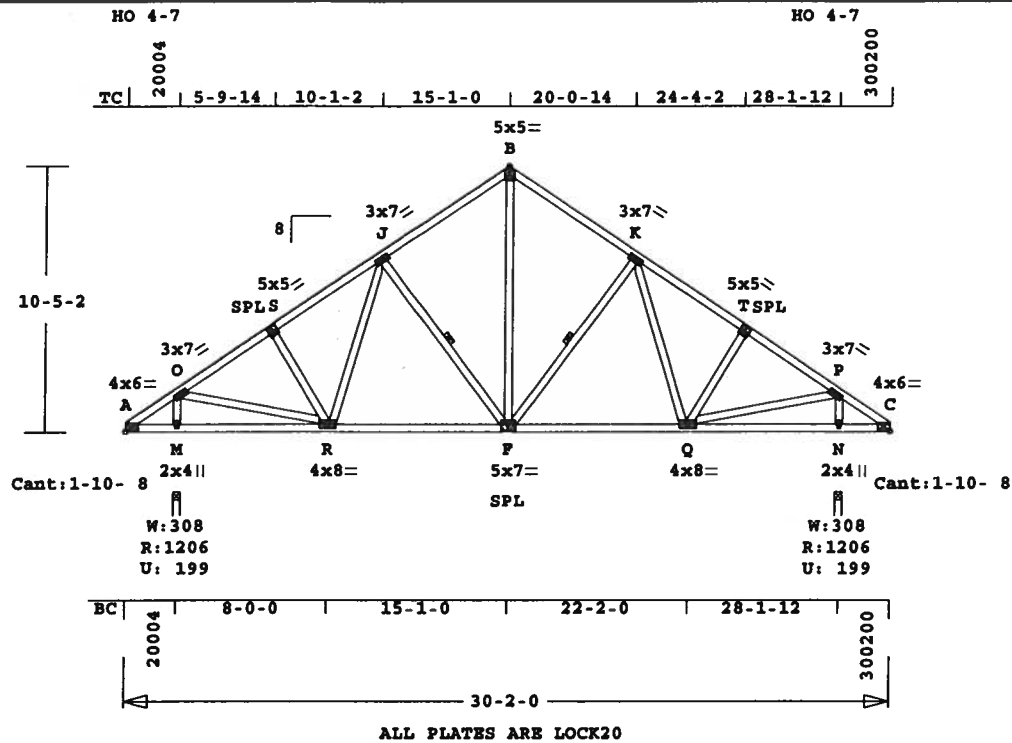


Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 260.3 LBS

A circular professional engineer seal for Philip J. O'Regan. The outer ring contains the text "PHILIP J. O'REGAN" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside this ring, the word "LICENSE" is at the top and "STATE OF FLORIDA" is at the bottom, also separated by two stars. In the center, the license number "No. 58126" is printed. A large, stylized signature is written across the center of the seal, overlapping the license number and the state name.

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D7	1	TR	300200	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



Scale: 0.132" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 243.2 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---
TC 0.21 2x 4 SP-#2
BC 0.34 2x 4 SP-#2
WB 0.19 2x 4 SP-#2

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 30- 2- 0
BC Cont. 0- 0- 0 30- 2- 0
WB 1 rows CLB on J -F
WB 1 rows CLB on F -K
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
Lbs	Lbs	In-Sx	In-Sx	
M	1207	199	3- 8	1- 8
			Hx =	-222
N	1207	199	3- 8	1- 8
			Hx =	223

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -O	0.13		48 C	0.01	0.12
O -S	0.13		1164 C	0.01	0.12
S -J	0.21		1036 C	0.00	0.21
J -B	0.21		818 C	0.00	0.21
B -K	0.21		818 C	0.00	0.21
K -T	0.21		1036 C	0.00	0.21
T -P	0.13		1164 C	0.01	0.12
P -C	0.13		48 C	0.01	0.12

-----Bottom Chords-----					
A -M	0.16	51 T	0.00	0.16	
M -R	0.22	232 T	0.00	0.22	
R -F	0.34	849 T	0.08	0.26	
F -Q	0.34	849 T	0.08	0.26	
Q -N	0.22	232 T	0.00	0.22	
N -C	0.16	51 T	0.00	0.16	

-----Webs-----					
M -O	0.10	1084 C			
O -R	0.18	1012 T			
S -R	0.05	190 C			
R -J	0.02	163 T			
J -F	0.06	290 C	1 Br		
F -B	0.19	611 T			
F -K	0.06	290 C	1 Br		
K -Q	0.02	163 T			
Q -T	0.05	190 C			
Q -P	0.18	1012 T			
N -P	0.10	1084 C			

TL Defl -0.10" in R -F L/999
LL Defl -0.04" in R -F L/999
LL Cant 0.00" in A -M L/999
Shear // Grain in J -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.5	0.4	0.65
O	LOCK	3.0x	7.0	Ctr	Ctr	0.42
S	LOCK	5.0x	5.0-0.3	0.5	0.67	
J	LOCK	3.0x	7.0	Ctr	Ctr	0.54
B	LOCK	5.0x	5.0	Ctr	Ctr	0.60
K	LOCK	3.0x	7.0	Ctr	Ctr	0.54
T	LOCK	5.0x	5.0	0.3	0.5	0.67
P	LOCK	3.0x	7.0	Ctr	Ctr	0.42
C	LOCK	4.0x	6.0-0.5	0.4	0.65	
M	LOCK	2.0x	4.0	Ctr	Ctr	0.41
R	LOCK	4.0x	8.0	Ctr	Ctr	0.70
F	LOCK	5.0x	7.0	Ctr	-0.5	0.69
Q	LOCK	4.0x	8.0	Ctr	Ctr	0.70
N	LOCK	2.0x	4.0	Ctr	Ctr	0.41

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main

Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 1164 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan

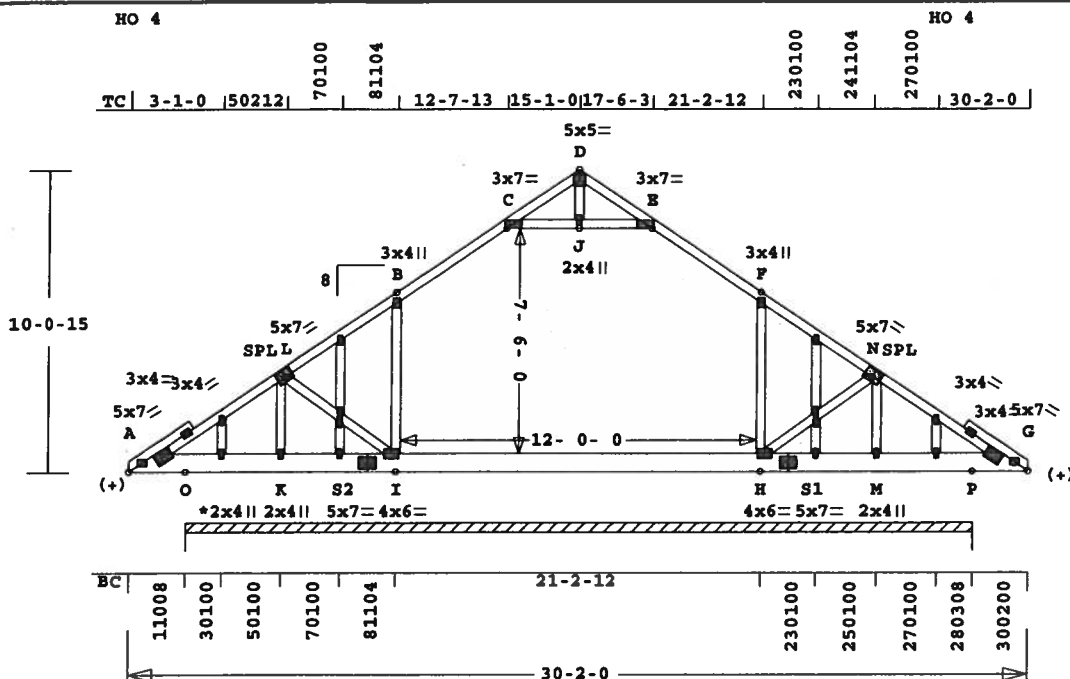
License #: 58126

Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D8	1	ATI2	300200	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



(+) 2X4 SP-#2

See * For Typical Gable Plate Size and Placement

Scale: 0.155" = 1'

Robbins Engineering, Inc./Online Plus[™] APPROX. TRUSS WEIGHT: 264.2 LBS

Online Plus -- Version 19.0.024
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---
TC 0.31 2x 4 SP-#2
BC 0.84 2x 8 SP-#2
WB 0.11 2x 4 SP-#2
ACT 0.03 2x 4 SP-#2
AWT 0.00 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 30- 2- 0
BC Cont. 0- 0- 0 30- 2- 0

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00
plf - Live Dead From To
TC V 40 20 0.0' 30.2'
BC V 0 20 0.0' 30.2'
TC V 0 10 9.1' 12.7'
TC V 0 10 17.5' 21.1'
BC V 80 10 9.1' 21.1'
MA V 0 10 12.9' 17.3'
MA V 0 10 0.3' 5.6'
MA V 0 10 0.3' 5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd
Lbs Lbs In-Sx In-Sx
Cont. Brg 1-10- 8 to 28- 3- 8
3609 259 Hz = 212

Membr CSI P Lbs Axl-CST-Bnd
-----Top Chords-----
A -L 0.17 76 C 0.00 0.17
L -B 0.31 508 C 0.00 0.31
B -C 0.31 521 C 0.00 0.31
C -D 0.21 151 C 0.00 0.21
D -E 0.21 151 C 0.00 0.21
E -F 0.31 521 C 0.00 0.31
F -N 0.31 508 C 0.00 0.31

N -G 0.17 76 C 0.00 0.17
-----Bottom Chords-----
A -O 0.12 68 T 0.00 0.12
O -K 0.12 0 T 0.00 0.12
K -S2 0.19 0 T 0.00 0.19
S2 -I 0.84 0 T 0.00 0.84
I -H 0.84 0 T 0.00 0.84
H -S1 0.84 0 T 0.00 0.84
S1 -M 0.19 0 T 0.00 0.19
M -P 0.12 0 T 0.00 0.12
P -G 0.12 68 T 0.00 0.12

-----Webs-----
K -L 0.10 738 C
L -I 0.10 454 T
I -B 0.11 309 C
H -F 0.11 309 C
H -N 0.10 454 T
M -N 0.10 738 C
-----Attic Chords (Top)-----
C -J 0.03 307 C 0.03 0.00
J -E 0.03 307 C 0.03 0.00
-----Attic Webs (Top)-----
J -D 0.00 31 T

TL Defl -0.24" in I -H L/999
LL Defl -0.19" in I -H L/999
LL Cant 0.00" in O -O L/999
Shear // Grain in S2-I 0.79

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 5.0x 7.0 Ctr-0.9 0.59
L LOCK 5.0x 7.0-0.3 0.5 0.67
B LOCK 3.0x 4.0 Ctr Ctr 0.28
C LOCK 3.0x 7.0 Ctr Ctr 0.28
D LOCK 5.0x 5.0 Ctr Ctr 0.60
E LOCK 3.0x 7.0 Ctr Ctr 0.28
F LOCK 3.0x 4.0 Ctr Ctr 0.28
N LOCK 5.0x 7.0 0.3 0.5 0.67
G LOCK 5.0x 7.0 Ctr-0.9 0.59
K LOCK 2.0x 4.0 Ctr Ctr 0.41
S2 LOCK 5.0x 7.0 Ctr Ctr 0.62
I LOCK 4.0x 6.0 Ctr Ctr 0.41
H LOCK 4.0x 6.0 Ctr Ctr 0.41
S1 LOCK 5.0x 7.0 Ctr Ctr 0.62
M LOCK 2.0x 4.0 Ctr Ctr 0.41
J LOCK 2.0x 4.0 Ctr Ctr 0.41

6 Gable studs to be attached
with 2.0x4.0 plates each end.

REVIEWED BY:

Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe
member between outside of
truss and first tie-plate
to inside of heel plate.
Design checked for 10 psf non-
concurrent LL on BC.
Prevent truss rotation at all
bearing locations.
Refer to Gen Det 3 series for
web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15'-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

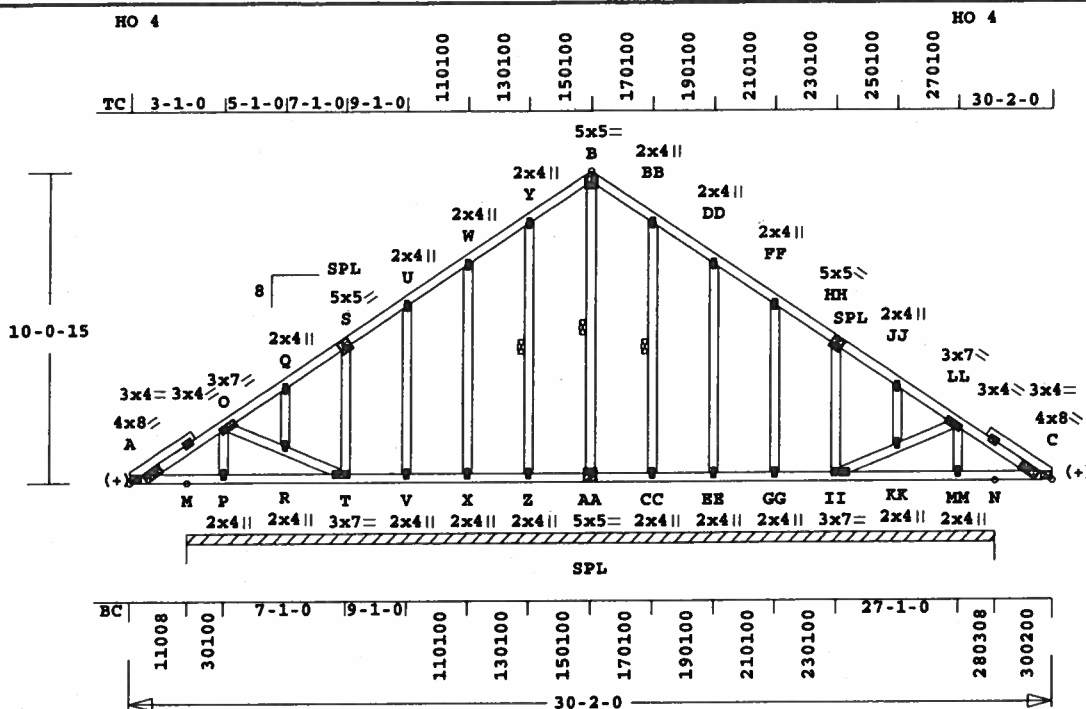


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D8	1	ATI2	300200	8	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

Unbalanced Loads Checked
 Load Factors = 1.00 and 0.00
 Max comp. force 738 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D9	1	TR	300200	8	0	0	T06052406

U# J#HAYGOOD-MOTE MOTE



(+) 2X4 SP-#2

ALL PLATES ARE LOCK20

Scale: 0.160" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 273.4 LBS

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---
TC 0.12 2x 4 SP-#2
BC 0.07 2x 4 SP-#2
WB 0.09 2x 4 SP-#2
GW 0.07 2x 4 SP-#2

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 30- 2- 0
BC Cont. 0- 0- 0 30- 2- 0
WB 1 rows CLB on Z -Y
WB 1 rows CLB on AA-B
WB 1 rows CLB on CC-BB
Attach CLB with (2)-10d nails
at each web.

Loading Live Dead (psf)
TC 20.0 10.0
BC 0.0 10.0
Total 20.0 20.0 40.0
Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd
Lbs Lbs In-Sx In-Sx
Cont. Brg 1-10- 8 to 28- 3- 8
2413 399 Hz = 215

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A -O 0.12 134 T 0.02 0.10
O -Q 0.10 112 C 0.00 0.10
Q -S 0.09 96 T 0.01 0.08
S -U 0.08 62 T 0.00 0.08
U -W 0.04 46 T 0.01 0.03
W -Y 0.03 68 T 0.00 0.03
Y -B 0.03 114 T 0.00 0.03
B -BB 0.03 114 T 0.00 0.03
BB-DD 0.03 68 T 0.00 0.03
DD-FF 0.04 46 T 0.01 0.03
FF-HH 0.08 62 T 0.00 0.08
HH-JJ 0.09 96 T 0.01 0.08
JJ-LL 0.10 112 C 0.00 0.10
LL-C 0.12 134 T 0.02 0.10
-----Bottom Chords-----
A -M 0.07 109 C 0.00 0.07
M -P 0.06 0 T 0.00 0.06

P -T	0.06	0 T	0.00	0.06
T -V	0.06	0 T	0.00	0.06
V -X	0.02	0 T	0.00	0.02
X -Z	0.02	0 T	0.00	0.02
Z -AA	0.02	0 T	0.00	0.02
AA-CC	0.02	0 T	0.00	0.02
CC-EE	0.02	0 T	0.00	0.02
EE-GG	0.02	0 T	0.00	0.02
GG-II	0.06	0 T	0.00	0.06
II-MM	0.06	0 T	0.00	0.06
MM-N	0.06	0 T	0.00	0.06
N -C	0.07	109 C	0.00	0.07

-----Webs-----
O -R 0.09 125 T 0.02 0.07
R -T 0.08 135 T 0.01 0.07
II-KK 0.08 135 T 0.01 0.07
KK-LL 0.09 125 T 0.02 0.07
-----Gable Webs-----
P -O 0.02 294 C
R -Q 0.04 37 C 0.00 0.04
T -S 0.04 177 C
V -U 0.04 101 C
X -W 0.07 121 C
Z -Y 0.03 125 C 1 Br
AA-B 0.04 137 C 1 Br
CC-BB 0.03 125 C 1 Br
EE-DD 0.07 121 C
GG-FF 0.04 101 C
II-HH 0.04 177 C
KK-JJ 0.04 37 C 0.00 0.04
MM-LL 0.02 294 C

TL Defl -0.01" in II-MM L/999
LL Defl 0.00" in P -T L/999
LL Cant 0.00" in A -M L/999
Shear // Grain in A -O 0.10

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 4.0x 8.0 1.0 0.4 0.59
O LOCK 3.0x 7.0 Ctr Ctr 0.42
Q LOCK 2.0x 4.0 Ctr Ctr 0.41
S LOCK 5.0x 5.0-0.3 0.5 0.67
U LOCK 2.0x 4.0 Ctr Ctr 0.41
W LOCK 2.0x 4.0 Ctr Ctr 0.41
Y LOCK 2.0x 4.0 Ctr Ctr 0.41
B LOCK 5.0x 5.0 Ctr Ctr 0.60
BB LOCK 2.0x 4.0 Ctr Ctr 0.41
DD LOCK 2.0x 4.0 Ctr Ctr 0.41
FF LOCK 2.0x 4.0 Ctr Ctr 0.41
HH LOCK 5.0x 5.0 0.3 0.5 0.67
JJ LOCK 2.0x 4.0 Ctr Ctr 0.41

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

WARNING Do Not Cut overframe
member between outside of
truss and first tie-plate

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

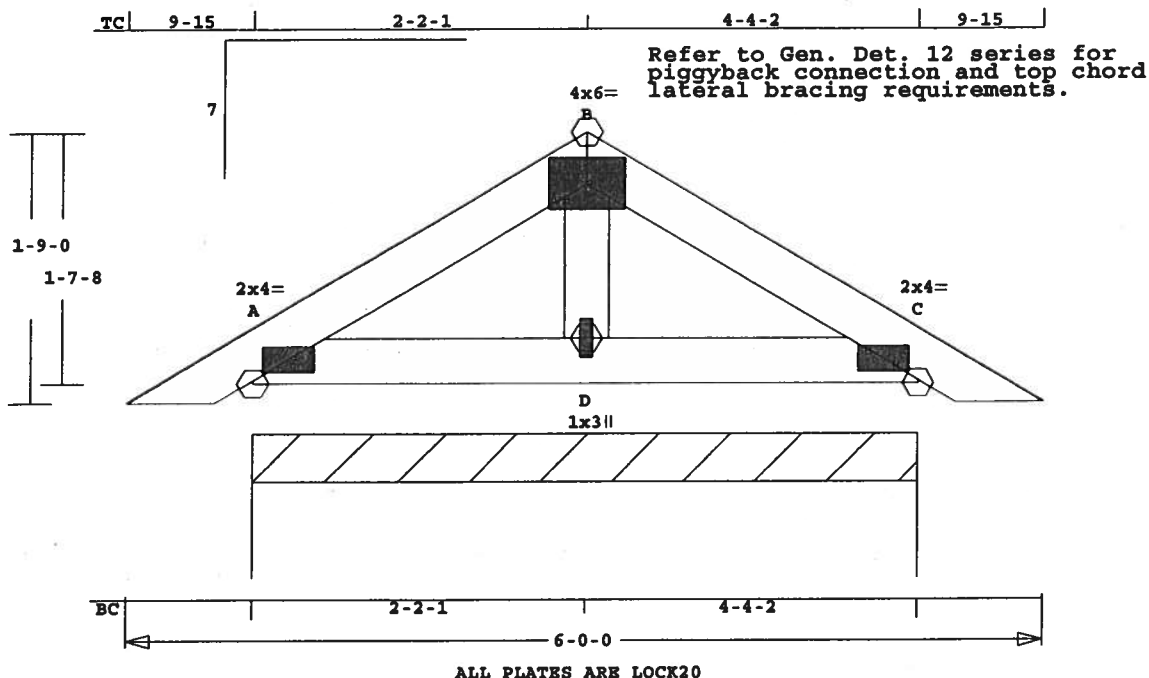


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	D9	1	TR	300200	8	0	0	T06052406
U# J#HAYGOOD-MOTE MOTE								

to inside of heel plate.
 Design checked for 10 psf non-
 concurrent LL on BC.
 Prevent truss rotation at all
 bearing locations.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as a Main
 Wind-Force Resistance System.
 Wind Speed: 110 mph
 Mean Roof Height: 15-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 Zone location: Exterior
 TC Dead Load : 5.0 psf
 BC Dead Load : 5.0 psf
 Max comp. force 294 Lbs
 Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	P1	19	TR	60000	7	9-15	9-15	T06052406

U# J#HAYGOOD-MOTE MOTE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.2 LBS

D - C 0.02 1 T 0.00 0.02

-----Webs-----

D - B 0.00 41 C

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

CSI -Size- ---Lumber---
TC 0.02 2x 4 SP-#2
BC 0.02 2x 4 SP-#2
WB 0.00 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	6- 0- 0	
BC Cont.	0- 0- 0	6- 0- 0	

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	4- 4- 2	
	453	65	Hz =	24

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A - B	0.02		75 C	0.00	0.02
B - C	0.02		75 C	0.00	0.02
-----Bottom Chords-----					
A - D	0.02		1 T	0.00	0.02

TL Defl 0.00" in D - C L/999
LL Defl 0.00" in D - C L/999
Shear // Grain in A - B 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.62

B LOCK 4.0x 6.0 Ctr Ctr 0.36

C LOCK 2.0x 4.0 Ctr Ctr 0.62

D LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Refer to Gen Det 3 series for
web bracing and plating.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main

Wind-Force Resistance System.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

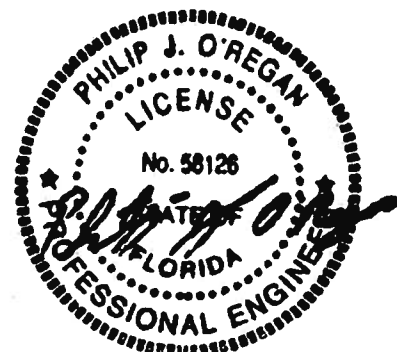
TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Max comp. force 75 Lbs

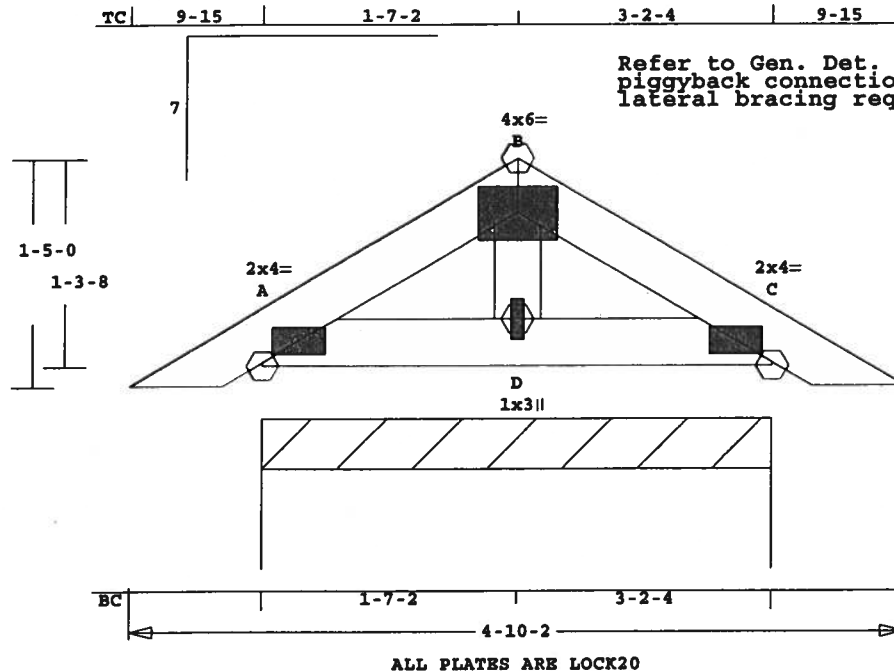
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
HAYGOOD-MOTE	P2	2	TR	41002	7	9-15	9-15	T06052406

U# J#HAYGOOD-MOTE MOTE



Scale: 0.834" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 17.9 LBS

D - C 0.01 1 T 0.00 0.01

concurrent LL on BC.

-----Webs-----

Refer to Gen Det 3 series for web bracing and plating.

Online Plus -- Version 19.0.020
RUN DATE: 19-MAY-06

D - B 0.00 37 C

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main Wind-Force Resistance System.

CSI -Size- ----Lumber----
TC 0.01 2x 4 SP-#2
BC 0.01 2x 4 SP-#2
WB 0.00 2x 4 SP-#2

TL Defl 0.00" in D - C L/999
LL Defl 0.00" in D - C L/999
Shear // Grain in A - B 0.03

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

Zone location: Exterior

TC Dead Load : 5.0 psf

BC Dead Load : 5.0 psf

Max comp. force 45 Lbs

Quality Control Factor 1.25

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4-10- 2
BC Cont.	0- 0- 0	4-10- 2

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.62

B LOCK 4.0x 6.0 Ctr Ctr 0.36

C LOCK 2.0x 4.0 Ctr Ctr 0.62

D LOCK 1.0x 3.0 Ctr Ctr 0.75

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan

License #: 58126

Address: P.O. Box 280055, Tampa, FL 33682

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 3- 2- 4		
	361	53	Hz =	17

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

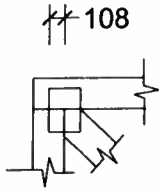
Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A - B	0.01		45 C	0.00	0.01
B - C	0.01		45 C	0.00	0.01
-----Bottom Chords-----					
A - D	0.01		1 T	0.00	0.01



Date Sealed: 5/10/2006

ROBBINS ENG. GENERAL NOTES & SYMBOLS

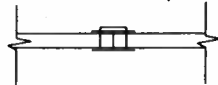
PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE

(3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

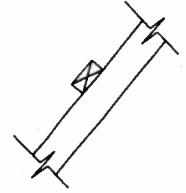
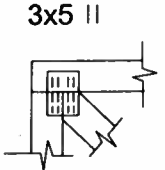


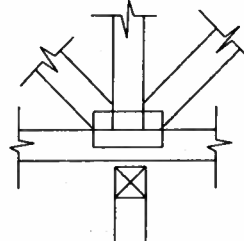
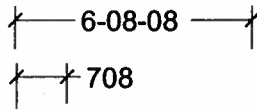
PLATE SIZE AND ORIENTATION



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd.
Tampa, FL 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbseng.com