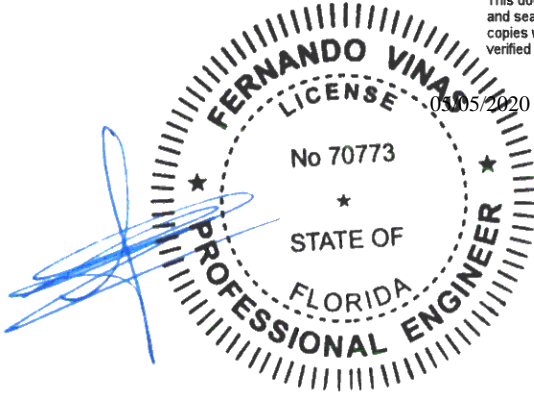


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Alpine, an ITW Company  
6750 Forum Drive, Suite 305  
Orlando, FL 32821  
Phone: (800)755-6001  
www.alpineitw.com



COA#0-278

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 20-4160
Job Description: Kerry Clancy	
Address: FL	

Job Engineering Criteria:			
Design Code: FBC 2017 RES		IntelliVIEW Version: 18.02.01B through 19.02.02B JRef #: 1WV02150001	
Wind Standard: ASCE 7-10	Wind Speed (mph): 130	Roof Load (psf): 20.00-10.00- 0.00-10.00	
Building Type: Closed		Floor Load (psf): None	

This package contains general notes pages, 15 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	126.20.0750.48443	A01
3	126.20.0752.11373	A03
5	126.20.0752.39537	B01
7	126.20.0752.47653	B03
9	126.20.0752.52773	V2
11	126.20.0752.55163	V4
13	126.20.0752.59500	V6
15	126.20.0753.05123	V8
17	GBLLETIN0118	
19	A14015ENC101014	
21	VAL160101014	

Item	Drawing Number	Truss
2	126.20.0750.50693	A02
4	126.20.0752.37307	A04
6	126.20.0752.41297	B02
8	126.20.0752.49853	V1
10	126.20.0752.54067	V3
12	126.20.0752.58040	V5
14	126.20.0753.01273	V7
16	A14030ENC101014	
18	BRCLBSUB0119	
20	A14015051014	

## **General Notes**

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

## **General Notes** (continued)

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

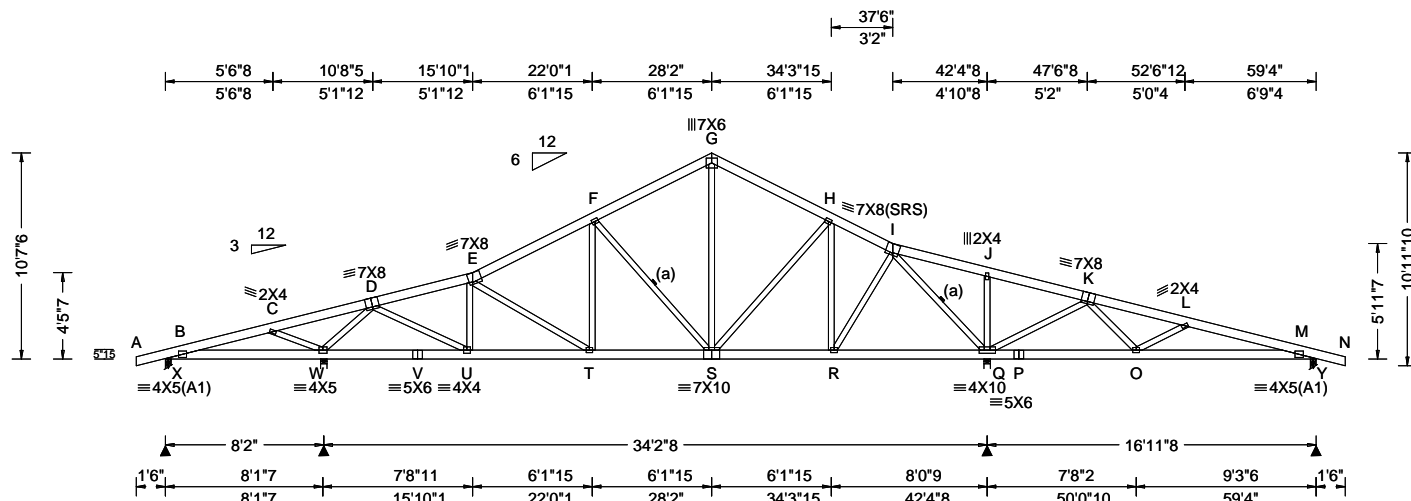
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

**References:**

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcindustry.com](http://www.sbcindustry.com).

SEQN: 339474 FROM: CDM	COMM Ply: 1 Qty: 4	Job Number: 20-4160 Kerry Clancy Truss Label: A01	Cust: R 215 JRef: 1WV02150001 T12 DrwNo: 126.20.0750.48443 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.93 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.054 T 999 360 VERT(CL): 0.108 T 999 240 HORZ(LL): 0.012 O - - HORZ(TL): 0.024 O - - Creep Factor: 2.0 Max TC CSI: 0.159 Max BC CSI: 0.139 Max Web CSI: 0.651  VIEW Ver: 18.02.01B.0321.08	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL X 279 -18 - / /74 /77 /213 W 2017 - / - / /1073 /42 - / Q 2529 - / - / /1250 /61 - / Y 574 - / - / /291 /73 - / <b>Non-Gravity</b> Wind reactions based on MWFRS X Brg Width = 3.5 Min Req = 1.5 W Brg Width = 4.0 Min Req = 1.5 Q Brg Width = 4.0 Min Req = 1.7 Y Brg Width = 3.5 Min Req = 1.5 Bearings X, W, Q, & Y are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x6 SP 2400f-2.0E;  
Bot chord: 2x6 SP 2400f-2.0E;  
Webs: 2x4 SP #3;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Plating Notes

All plates are 3X4 except as noted.

#### Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

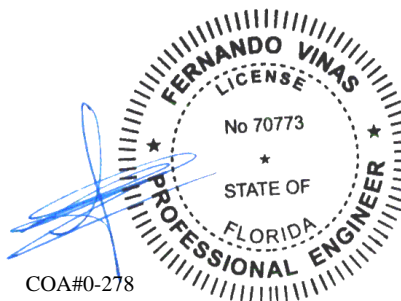
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10'-7-6.



COA#0-278

05/05/2020

#### Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - W	173 -569	T - S	1494 -230
W - V	594 -115	S - R	987 -89
V - U	594 -115	R - Q	644 -21
U - T	1724 -354	O - M	767 -60

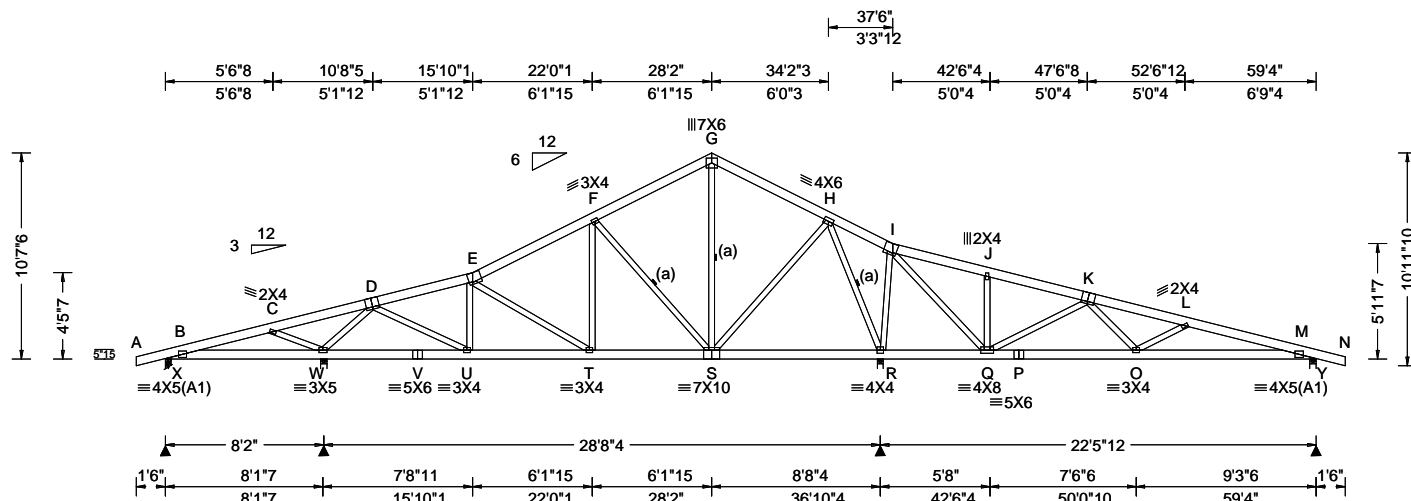
#### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - W	193 -523	H - R	159 -516
W - D	625 -2149	R - I	821 -149
D - U	1280 -313	I - Q	538 -2075
U - E	181 -516	Q - J	188 -382
T - F	375 -62	Q - K	221 -892
F - S	262 -705	K - O	601 -100
G - S	643 -218	O - L	181 -502

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

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AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339477 FROM: CDM	COMM Ply: 1 Qty: 6	Job Number: 20-4160 Kerry Clancy Truss Label: A02	Cust: R 215 JRRef: 1WV02150001 T10 DrwNo: 126.20.0750.50693 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.93 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.040 O 999 360 VERT(CL): 0.080 O 999 240 HORZ(LL): 0.012 I - - HORZ(TL): 0.022 I - - Creep Factor: 2.0 Max TC CSI: 0.121 Max BC CSI: 0.159 Max Web CSI: 0.633  VIEW Ver: 18.02.01B.0321.08	<b>Gravity</b> Loc R+ / R- / Rh / Rw / U / RL X 311 - / - / - /103 /77 /213 W 1673 - / - / - /887 /33 - /- R 2709 - / - / - /1313 /64 - /- Y 751 - / - / - /395 /78 - /- <b>Non-Gravity</b> Wind reactions based on MWFRS X Brg Width = 3.5 Min Req = 1.5 W Brg Width = 4.0 Min Req = 1.5 R Brg Width = 3.5 Min Req = 1.9 Y Brg Width = 4.0 Min Req = 1.5 Bearings X, W, R, & Y are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x6 SP 2400f-2.0E;  
Bot chord: 2x6 SP 2400f-2.0E;  
Webs: 2x4 SP #3;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Plating Notes

All plates are 7X8 except as noted.

#### Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

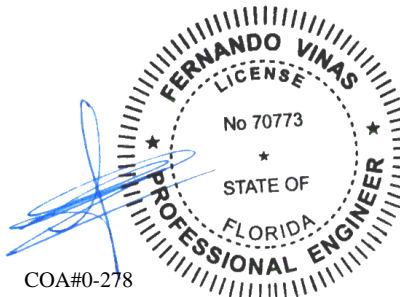
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10'-7-6\"/>



COA#0-278

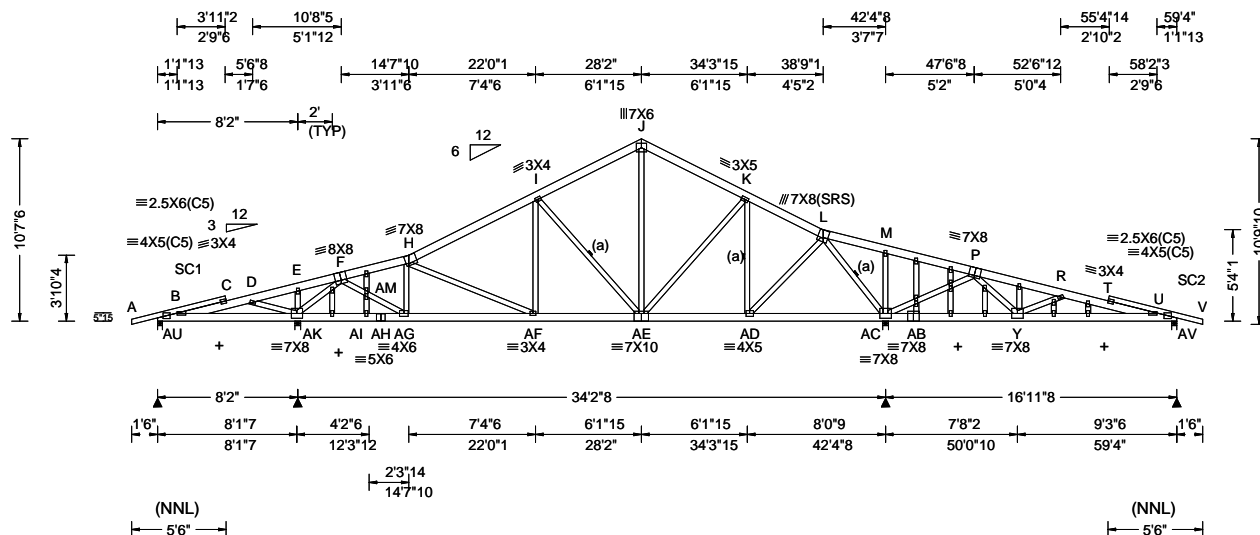
05/05/2020

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**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: [www.alpineitw.com](http://www.alpineitw.com); TPI: [www.tpinet.org](http://www.tpinet.org); SBCA: [www.sbcindustry.com](http://www.sbcindustry.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
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Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.076 AF 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.162 AF 999 240	AU 293	-	-	-	/117	/122	/349
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.017 M - -	AK 3029	-	-	-	/1299	/607	-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.037 M - -	AC 4095	-	-	-	/1551	/831	-
NCBCLL: 10.00	Mean Height: 15.00 ft		Creep Factor: 2.0	AV 715	-	-	-	/344	/208	-
Soffit: 2.00	TCDL: 5.0 psf		Max TC CSI: 0.280	Wind reactions based on MWFRS						
Load Duration: 1.25	BCDL: 5.0 psf	Code / Misc Criteria	Max BC CSI: 0.182	AU Brg Width = 3.5	Min Req = 1.5					
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Bldg Code: FBC 2017 RES	Max Web CSI: 0.870	AK Brg Width = 4.0	Min Req = 2.1					
	C&C Dist a: 5.93 ft	TPI Std: 2014		AC Brg Width = 4.0	Min Req = 3.0					
	Loc. from endwall: not in 13.25 ft	Rep Fac: No		AV Brg Width = 3.5	Min Req = 1.5					
	GCpi: 0.18	FT/RT:20(0)/10(0)		Bearings AU, AK, AC, & AV are a rigid surface.						
	Wind Duration: 1.60	Plate Type(s):	VIEW Ver: 18.02.01B.0321.08							
		WAVE								

**Lumber**  
 Top chord: 2x6 SP 2400f-2.0E;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3;  
 Stack Chord: SC1 2x4 SP 2400f-2.0E;  
 Stack Chord: SC2 2x4 SP 2400f-2.0E;

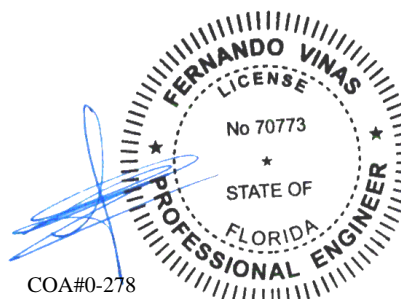
**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 2X4 except as noted.

**Loading**  
 Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Purlins**  
 In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 + MEMBER TO BE Laterally Braced For HORIZONTAL WIND LOADS.



COA#0-278

05/05/2020

Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1067 -261	J - K	677 -1958
C - D	972 -198	K - L	492 -1577
D - E	1417 -393	L - M	1851 -463
E - F	1373 -334	M - P	1877 -526
F - H	784 -2809	R - T	358 -790
H - I	794 -2820	T - U	382 -865
I - J	691 -1975		

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - AK	300 -894	AF - AE	2344 -314
AK - AI	1000 -140	AE - AD	1352 -71
AI - AH	998 -140	AC - AB	352 -569
AH - AG	998 -140	AB - Y	352 -571
AG - AF	2789 -569	Y - U	782 -219

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
D - AK	255 -470	J - AE	689 -344
AK - F	845 -3216	K - AD	306 -1186
F - AM	2031 -533	AD - L	1626 -359
AM - AG	2021 -534	L - AC	911 -3357
AG - H	325 -1025	AC - M	244 -592
H - AF	278 -482	AC - P	458 -1332
AF - I	377 -96	P - Y	892 -356
I - AE	422 -1165	Y - R	365 -841

**Maximum Gable Forces Per Ply (lbs)**

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCEA: www.sbceaindust.com; ICC: www.iccsafe.org

**ALPINE**  
 AN ITW COMPANY  
 6750 Forum Drive  
 Suite 305  
 Orlando FL, 32821

SEQN: 339482	GABL	Ply: 1	Job Number: 20-4160	Cust: R 215 JRef: 1WV02150001 T5
FROM: CDM		Qty: 1	Kerry Clancy	DrwNo: 126.20.0752.11373
Page 2 of 2			Truss Label: A03	JB / FV 05/05/2020

#### Additional Notes

Refer to General Notes for additional information  
See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

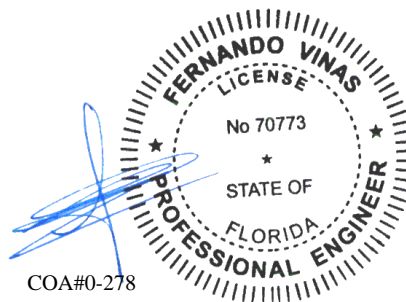
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10-7-6.

Gables Tens.Comp.

E -AK 207 -471



COA#0-278

05/05/2020

#### **\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

#### **\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

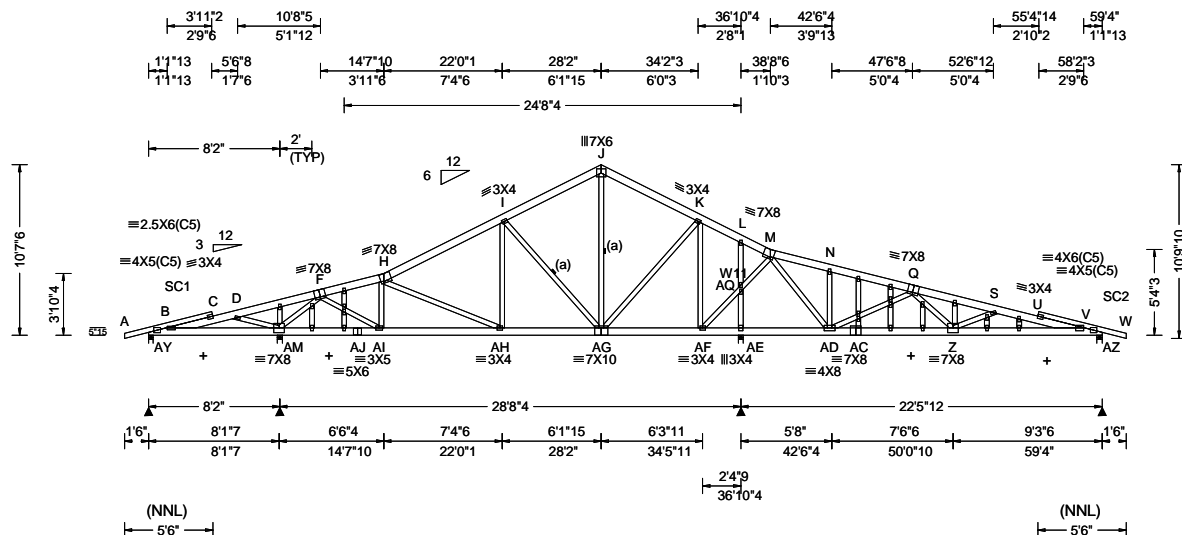
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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.096 AB 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.169 Y 999 240	AY 271	-13	-		/78	/78	/228
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.038 L - -	AM 1810	-	-		/969	/35	-
	EXP: C Kzt: NA		HORZ(TL): 0.074 L - -	AE 2472	-	-		/1221	/54	-
Des Ld: 40.00	Mean Height: 15.00 ft		Creep Factor: 2.0	AZ 863	-	-		/448	/80	-
NCBCLL: 10.00	TCDL: 5.0 psf		Max TC CSI: 0.586	Wind reactions based on MWFRS						
Soffit: 2.00	BCDL: 5.0 psf	Code / Misc Criteria	Max BC CSI: 0.276	AY	Brg Width = 3.5			Min Req = 1.5		
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	Bldg Code: FBC 2017 RES	Max Web CSI: 0.661	AM	Brg Width = 4.0			Min Req = 1.5		
Spacing: 24.0 "	C&C Dist a: 5.93 ft	TPI Std: 2014		AE	Brg Width = 3.5			Min Req = 1.7		
	Loc. from endwall: not in 13.00 ft	Rep Fac: No		AZ	Brg Width = 4.0			Min Req = 1.5		
	GCpi: 0.18	FT/RT:20(0)/10(0)		Bearings AY, AM, AE, & AZ are a rigid surface.						
	Wind Duration: 1.60	Plate Type(s):	VIEW Ver: 18.02.01B.0321.08							
		WAVE								

#### Lumber

Top chord: 2x6 SP 2400f-2.0E;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W11 2x4 SP #2;  
 Stack Chord: SC1 2x4 SP 2400f-2.0E;  
 Stack Chord: SC2 2x4 SP 2400f-2.0E;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Plating Notes

All plates are 2X4 except as noted.

#### Loading

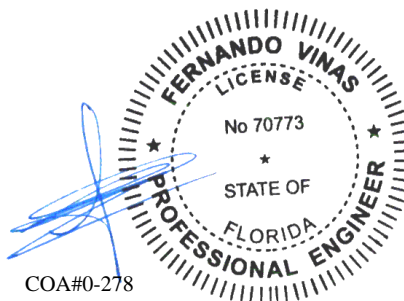
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

Wind loads based on MWFRS with additional C&C member design.

+ MEMBER TO BE Laterally Braced For HORIZONTAL WIND LOADS.



05/05/2020

**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!  
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 For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

**ALPINE**  
 AN ITW COMPANY  
 6750 Forum Drive  
 Suite 305  
 Orlando FL, 32821

SEQN: 339488	GABL	Ply: 1	Job Number: 20-4160	Cust: R 215 JRef: 1WV02150001 T21
FROM: CDM		Qty: 1	Kerry Clancy	DrwNo: 126.20.0752.37307
Page 2 of 2			Truss Label: A04	JB / FV 05/05/2020

#### Additional Notes

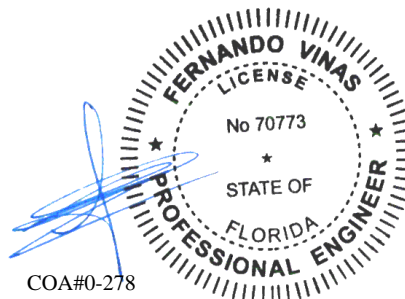
Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10-7-6.



COA#0-278

05/05/2020

#### **\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

#### **\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

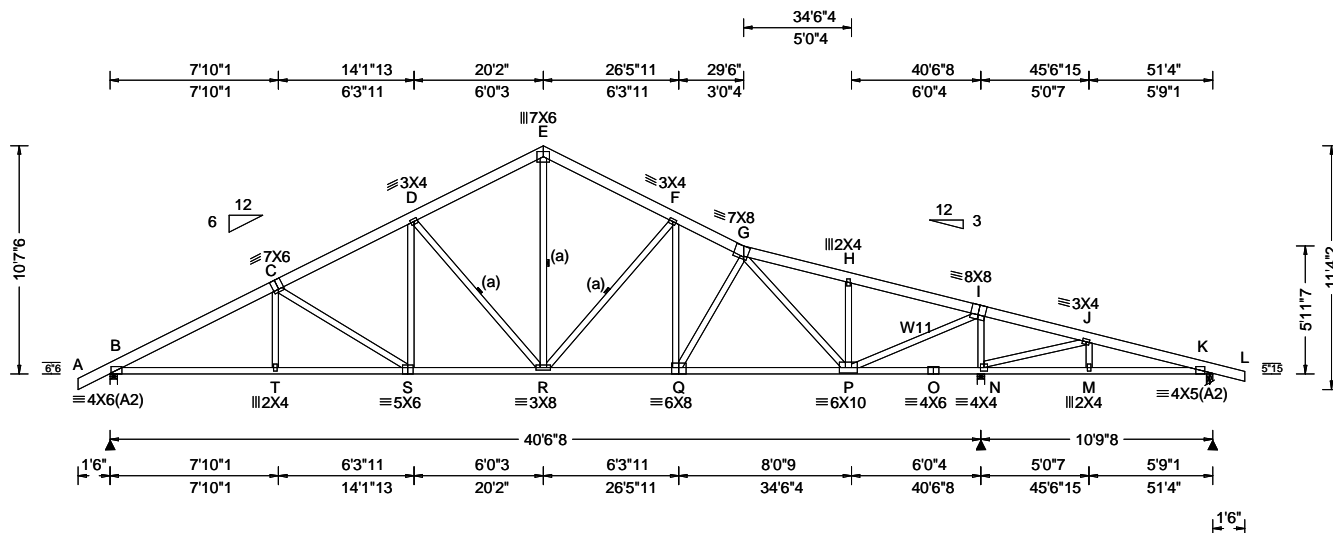
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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339492 FROM: CDM	COMN Ply: 1 Qty: 6	Job Number: 20-4160 Kerry Clancy Truss Label: B01	Cust: R 215 JRRef: 1WV02150001 T20 DrwNo: 126.20.0752.39537 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.13 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.119 S 999 360 VERT(CL): 0.231 S 999 240 HORZ(LL): 0.049 P - - HORZ(TL): 0.097 P - - Creep Factor: 2.0 Max TC CSI: 0.177 Max BC CSI: 0.361 Max Web CSI: 0.972  VIEW Ver: 18.02.01B.0321.08	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1813 -/- /- /1056 /300 /287 N 2578 -/- /- /1290 /440 /- K 319 -/34 /- /130 /73 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 N Brg Width = 4.0 Min Req = 1.8 K Brg Width = 3.5 Min Req = 1.5 Bearings B, N, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.

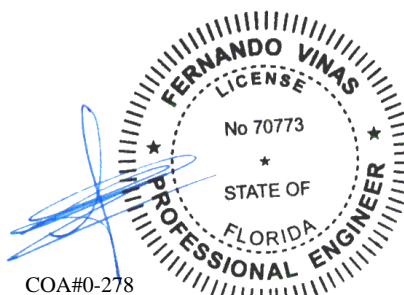
**Lumber**  
Top chord: 2x6 SP 2400F-2.0E;  
Bot chord: 2x4 SP M-31;  
Webs: 2x4 SP #3; W11 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Refer to General Notes for additional information  
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.  
The overall height of this truss excluding overhang is 10'-7-6.



05/05/2020

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For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1337 - 3083	G - H	935 - 1693
C - D	1234 - 2573	H - I	855 - 1704
D - E	1096 - 1969	I - J	1229 - 488
E - F	1135 - 1963	J - K	662 - 208
F - G	1277 - 2373		

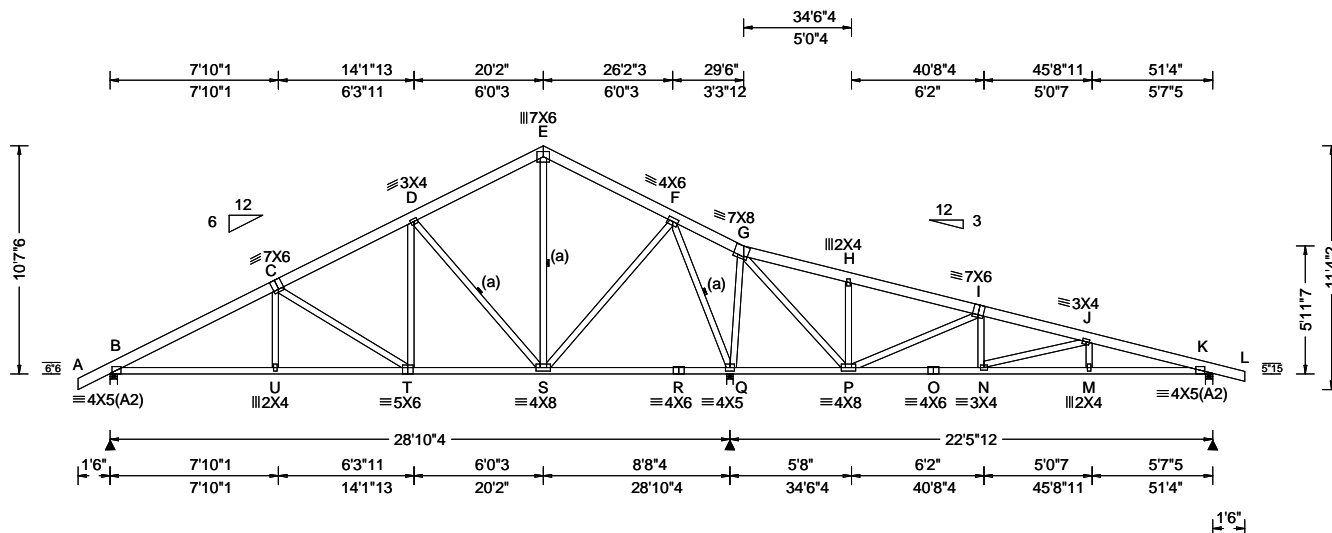
Chords	Tens.Comp.	Chords	Tens. Comp.
B - T	2652 - 1007	P - O	523 - 968
T - S	2651 - 1008	O - N	523 - 968
S - R	2212 - 782	N - M	244 - 627
R - Q	2085 - 779	M - K	244 - 621
Q - P	2190 - 876		

Webs	Tens.Comp.	Webs	Tens. Comp.
C - S	322 - 511	G - P	451 - 937
S - D	497 - 157	P - I	2748 - 1254
D - R	447 - 804	I - N	1057 - 2214
E - R	1273 - 657	N - J	404 - 798
R - F	466 - 609		

**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 342077 FROM: CDM	COMN Ply: 1 Qty: 6	Job Number: 20-4160 Kerry Clancy Truss Label: B02	Cust: R 215 JRRef: 1WV02150001 T2 DrwNo: 126.20.0752.41297 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.13 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.054 J 999 360 VERT(CL): 0.112 J 999 240 HORZ(LL): 0.024 M - - HORZ(TL): 0.046 M - - Creep Factor: 2.0 Max TC CSI: 0.136 Max BC CSI: 0.782 Max Web CSI: 0.914  VIEW Ver: 19.02.02B.0122.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1206 - / - / /23 /185 /287 Q 2856 - / - / /1391 /468 - / - K 713 - / - / /373 /140 - / - Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 Q Brg Width = 3.5 Min Req = 3.0 K Brg Width = 4.0 Min Req = 1.5 Bearings B, Q, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.

#### Lumber

Top chord: 2x6 SP 2400F-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Bracing

(a) Continuous lateral restraint equally spaced on member.

#### Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

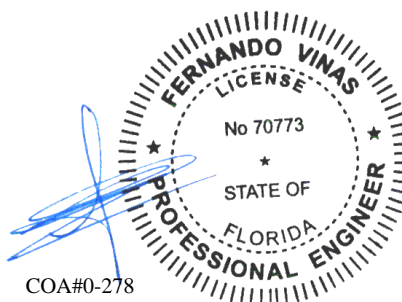
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10'-7".



COA#0-278

05/05/2020

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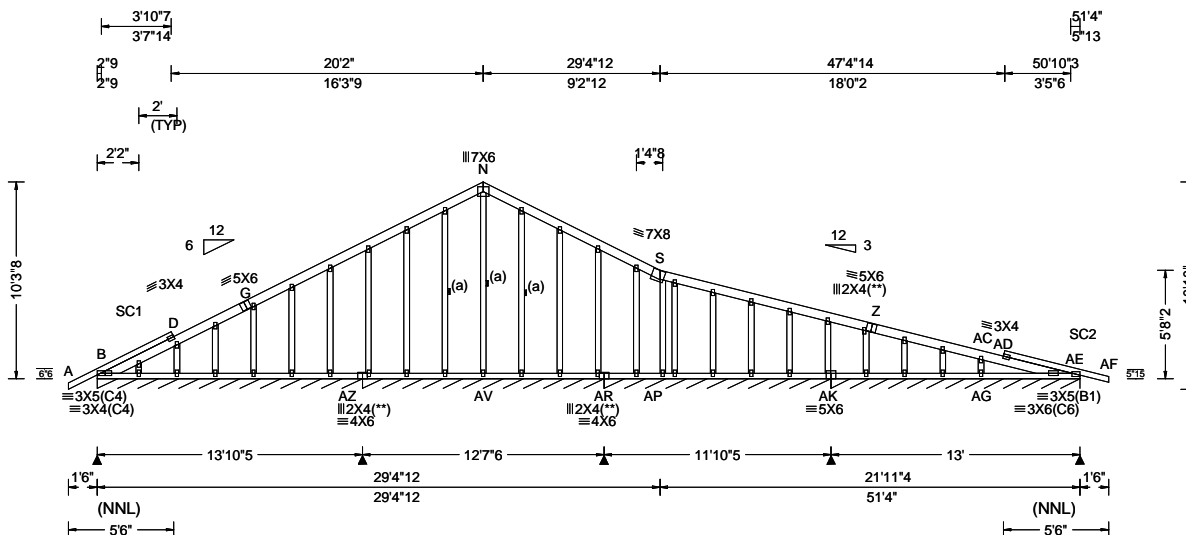
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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - U	1524 -434	Q - P	604 -1130
U - T	1523 -434	P - O	692 -204
T - S	1033 -184	O - N	692 -204
S - R	366 -603	N - M	1302 -479
R - Q	366 -603	M - K	1307 -478

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - T	351 -579	Q - G	474 -617
T - D	497 -174	G - P	1129 -611
D - S	463 -810	P - I	466 -1060
S - F	1246 -348	I - N	390 -87
F - Q	912 -2092	N - J	335 -628



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 AD 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 AD 999 240	B* 134	-	-		/76	/30	/32
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.006 P - -	AZ* 172	-	-		/64	/12	-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.009 P - -	AR* 133	-	-		/52	/31	-
NCBCLL: 10.00	Mean Height: 15.00 ft		Creep Factor: 2.0	AK* 121	-	-		/52	/27	-
Soffit: 2.00	TCDL: 5.0 psf		Max TC CSI: 0.459	Wind reactions based on MWFRS						
Load Duration: 1.25	BCDL: 5.0 psf	Bldg Code: FBC 2017 RES	Max BC CSI: 0.180	B Brg Width = 166	Min Req = -					
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.227	AZ Brg Width = 151	Min Req = -					
	C&C Dist a: 5.13 ft	Rep Fac: No		AR Brg Width = 142	Min Req = -					
	Loc. from endwall: Any	FT/RT:20(0)/10(0)		AK Brg Width = 156	Min Req = -					
	GCpi: 0.18	Plate Type(s):		Bearings B, AZ, AR, & AK are a rigid surface.						
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15	Members not listed have forces less than 375#						

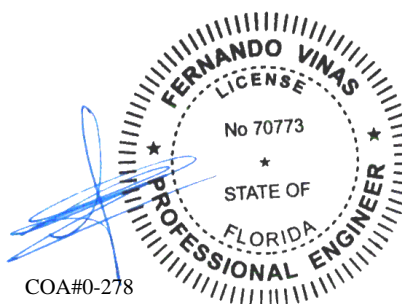
**Lumber**  
 Top chord: 2x6 SP 2400F-2.0E;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;  
 Stack Chord: SC1 2x4 SP #2;  
 Stack Chord: SC2 2x4 SP #2;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 2X4 except as noted.  
 (\*\*) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
 Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.



05/05/2020

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.  
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

6750 Forum Drive  
 Suite 305  
 Orlando FL, 32821

SEQN: 342084	GABL	Ply: 1	Job Number: 20-4160	Cust: R 215 JRef: 1WV02150001 T3
FROM: CDM		Qty: 2	Kerry Clancy	DrwNo: 126.20.0752.47653
Page 2 of 2			Truss Label: B03	JB / FV 05/05/2020

#### Additional Notes

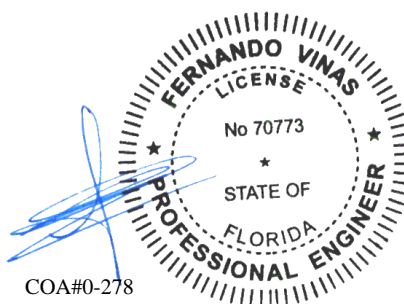
Refer to General Notes for additional information

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 10-3-8.



05/05/2020

#### **\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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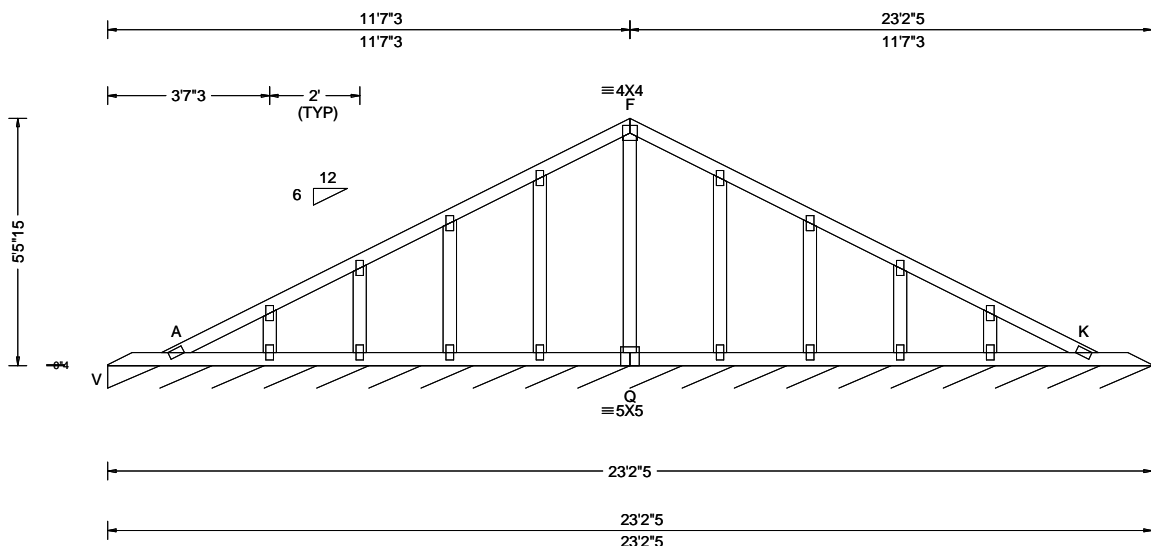
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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821



SEQN: 342087 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V1	Cust: R 215 JRef: 1WV02150001 T1 DrwNo: 126.20.0752.49853 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.07 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 G 999 360 VERT(CL): 0.002 G 999 240 HORZ(LL): -0.002 E - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.110 Max BC CSI: 0.040 Max Web CSI: 0.079  VIEW Ver: 19.02.02B.0122.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L* 122 /- /- /49 /20 /9 Wind reactions based on MWFRS L Brg Width = 278 Min Req = - Bearing V is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Plating Notes

All plates are 2X4 except as noted.

#### Loading

Truss designed to support 1-6-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

#### Wind

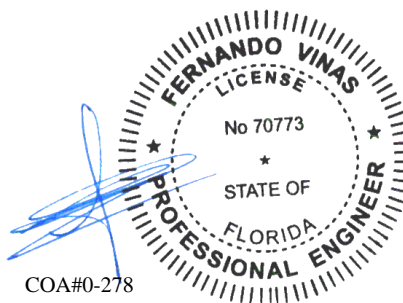
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWGS A14030ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 55-15.



COA#0-278

05/05/2020

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**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

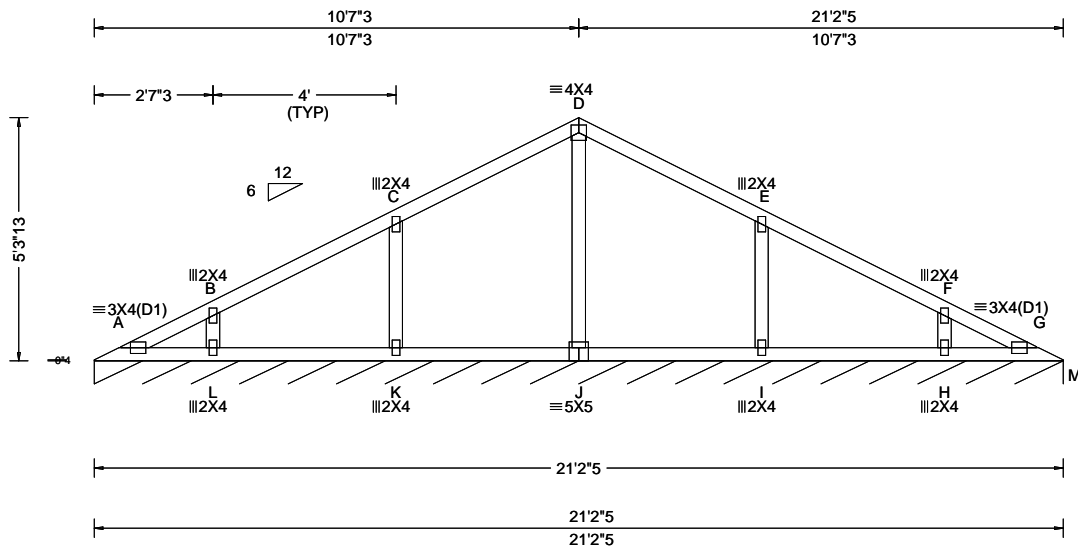
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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
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Orlando FL, 32821

SEQN: 339451 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V2	Cust: R 215 JRef: 1WV02150001 T6 DrwNo: 126.20.0752.52773 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.50 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 H 999 360 VERT(CL): 0.002 H 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.210 Max BC CSI: 0.110 Max Web CSI: 0.097  VIEW Ver: 18.02.01B.0321.08	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M* 81 /- /- /42 /13 /6 Wind reactions based on MWFRS M Brg Width = 254 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

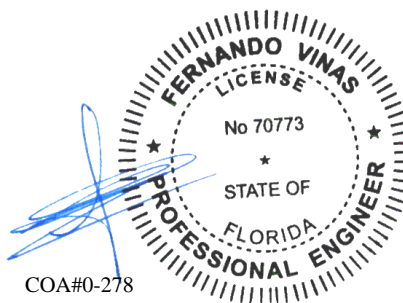
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 5'-3-13/16.



COA#0-278

05/05/2020

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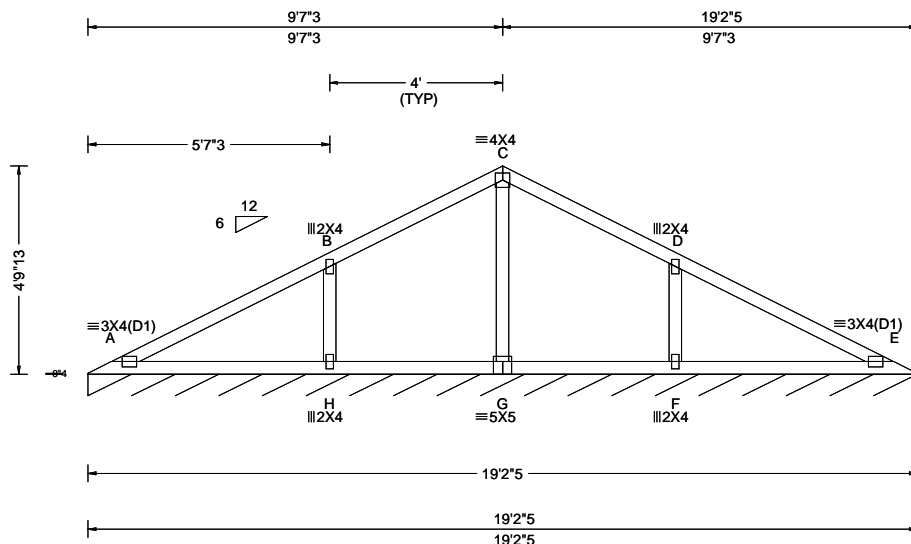
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6750 Forum Drive  
Suite 305  
Orlando FL, 32821



SEQN: 339471 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V3	Cust: R 215 JRef: 1WV02150001 T7 DrwNo: 126.20.0752.54067 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.75 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.018 H 999 360 VERT(CL): 0.036 H 999 240 HORZ(LL): 0.006 H - - HORZ(TL): 0.012 H - - Creep Factor: 2.0 Max TC CSI: 0.435 Max BC CSI: 0.243 Max Web CSI: 0.142  VIEW Ver: 18.02.01B.0321.08	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 81 /- /- /42 /2 /6 Wind reactions based on MWFRS I Brg Width = 230 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

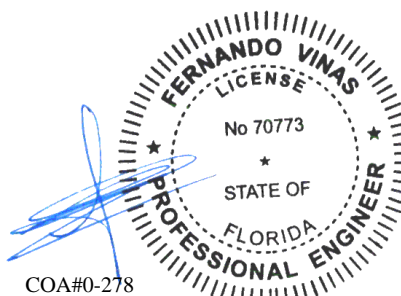
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 4-9-13.



COA#0-278

05/05/2020

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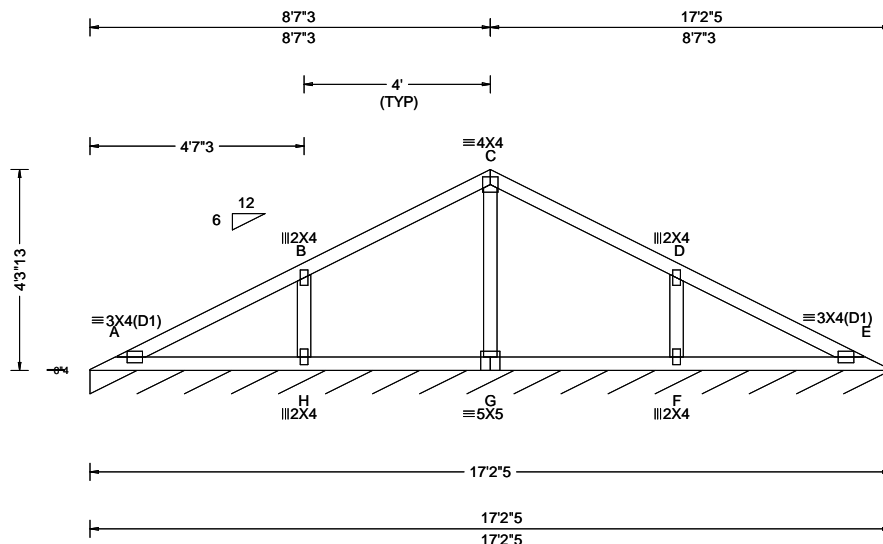
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AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339460 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V4	Cust: R 215 JRef: 1WV02150001 T8 DrwNo: 126.20.0752.55163 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 H 999 360 VERT(CL): 0.018 H 999 240 HORZ(LL): 0.003 H - - HORZ(TL): 0.006 H - - Creep Factor: 2.0 Max TC CSI: 0.335 Max BC CSI: 0.174 Max Web CSI: 0.096  VIEW Ver: 18.02.01B.0321.08	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 81 /- /- /41 /13 /6 Wind reactions based on MWFRS I Brg Width = 206 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

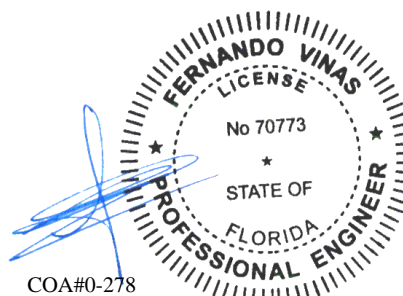
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 4'-3-13/16\"/>



COA#0-278

05/05/2020

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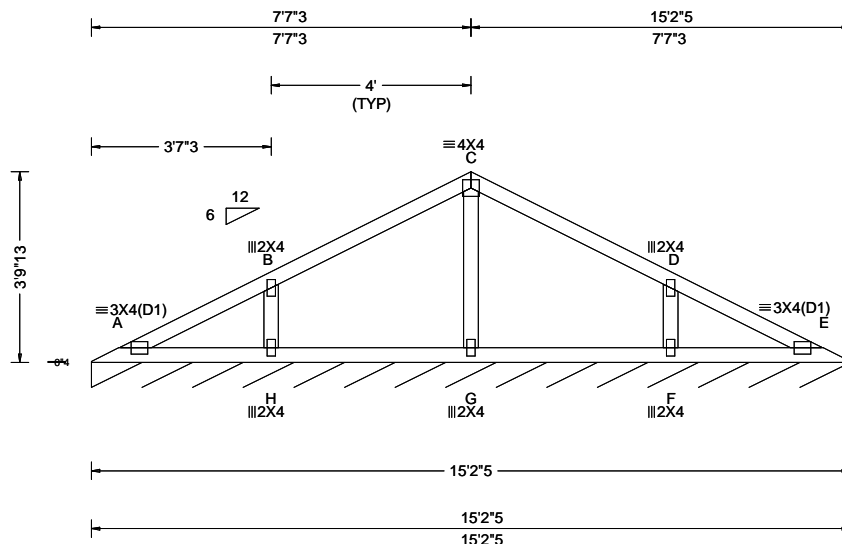
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**ALPINE**  
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6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339449 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V5	Cust: R 215 JRef: 1WV02150001 T13 DrwNo: 126.20.0752.58040 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.25 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 H 999 360 VERT(CL): 0.007 H 999 240 HORZ(LL): -0.001 F - - HORZ(TL): 0.003 F - - Creep Factor: 2.0 Max TC CSI: 0.271 Max BC CSI: 0.122 Max Web CSI: 0.065  VIEW Ver: 18.02.01B.0321.08	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 81 /- /- /41 /13 /6 Wind reactions based on MWFRS I Brg Width = 182 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

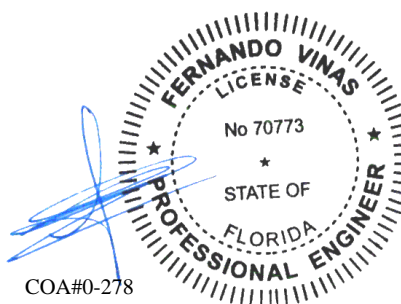
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 3-9-13.



05/05/2020

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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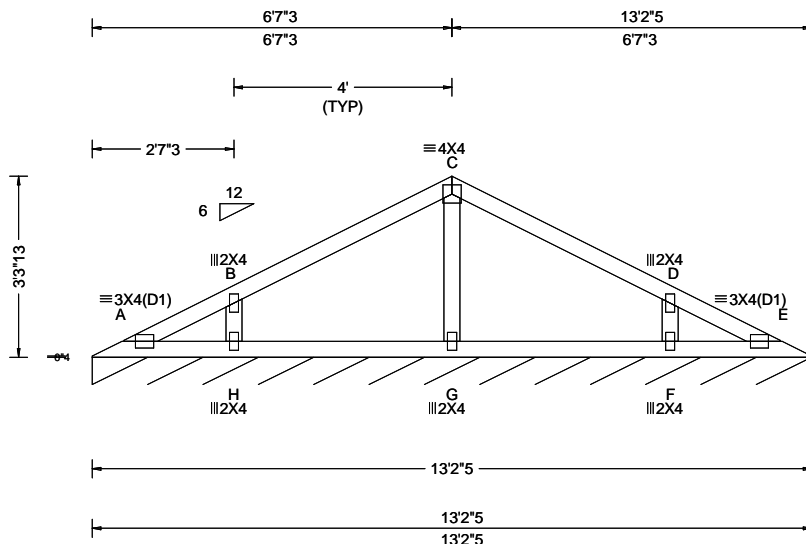
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AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339467 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V6	Cust: R 215 JRef: 1WV02150001 T14 DrwNo: 126.20.0752.59500 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.50 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 360 VERT(CL): 0.001 C 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.200 Max BC CSI: 0.114 Max Web CSI: 0.050  VIEW Ver: 18.02.01B.0321.08	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 81 /- /- /41 /12 /6 Wind reactions based on MWFRS I Brg Width = 158 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

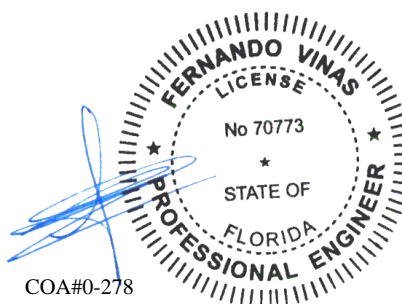
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 3-3-13.



COA#0-278

05/05/2020

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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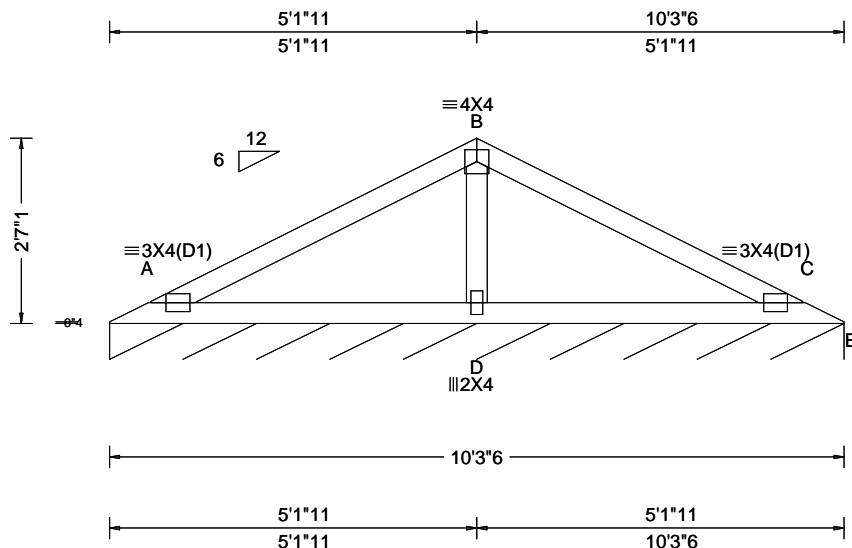
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For more information see this job's general notes page and these web sites: ALPINE: [www.alpineitw.com](http://www.alpineitw.com); TPI: [www.tpinet.org](http://www.tpinet.org); SBCA: [www.sbcindustry.com](http://www.sbcindustry.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339458 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V7	Cust: R 215 JRef: 1WV02150001 T15 DrwNo: 126.20.0753.01273 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.86 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.016 D 999 360 VERT(CL): 0.032 D 999 240 HORZ(LL): -0.006 D - - HORZ(TL): 0.013 D - - Creep Factor: 2.0 Max TC CSI: 0.360 Max BC CSI: 0.296 Max Web CSI: 0.107  VIEW Ver: 18.02.01B.0321.08	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 80 /- /- /40 /12 /6 Wind reactions based on MWFRS E Brg Width = 123 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. B - D 277 -530

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

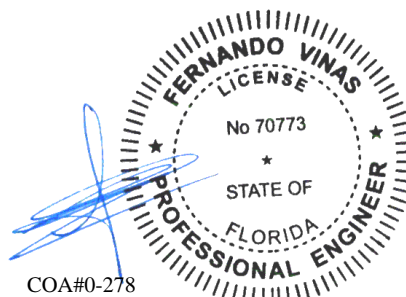
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 2'-7-1/2".



COA#0-278

05/05/2020

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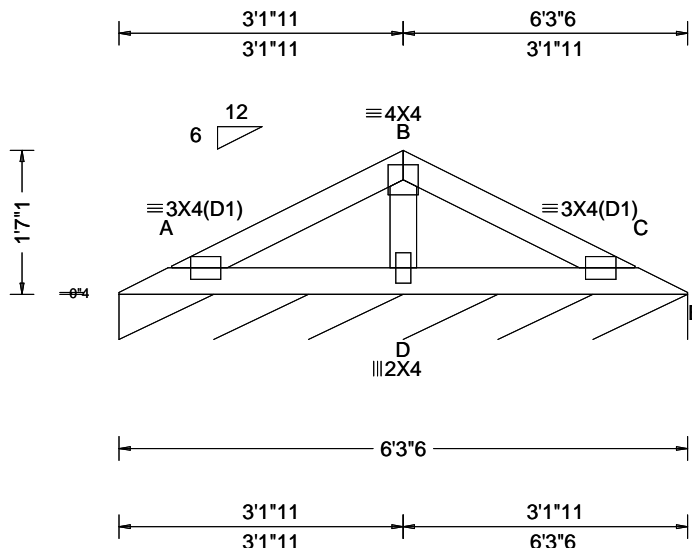
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6750 Forum Drive  
Suite 305  
Orlando FL, 32821

SEQN: 339444 FROM: CDM	VAL Ply: 1 Qty: 1	Job Number: 20-4160 Kerry Clancy Truss Label: V8	Cust: R 215 JRef: 1WV02150001 T16 DrwNo: 126.20.0753.05123 JB / FV 05/05/2020
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.36 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Code / Misc Criteria</b> Bldg Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 D 999 360 VERT(CL): 0.007 D 999 240 HORZ(LL): -0.001 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.111 Max BC CSI: 0.094 Max Web CSI: 0.048  VIEW Ver: 18.02.01B.0321.08	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 79 /- /- /38 /13 /5 Wind reactions based on MWFRS E Brg Width = 75.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

#### Wind

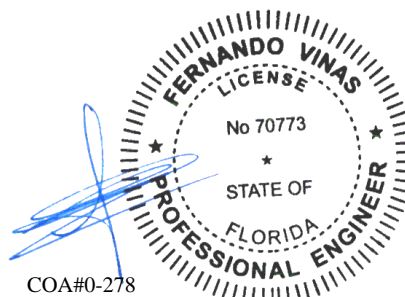
Wind loads based on MWFRS with additional C&C member design.

#### Additional Notes

Refer to General Notes for additional information

See DWG VAL160101014 for valley details.

The overall height of this truss excluding overhang is 1'-7-1/2\"/>



COA#0-278

05/05/2020

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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C,  $K_{zt} = 1.00$

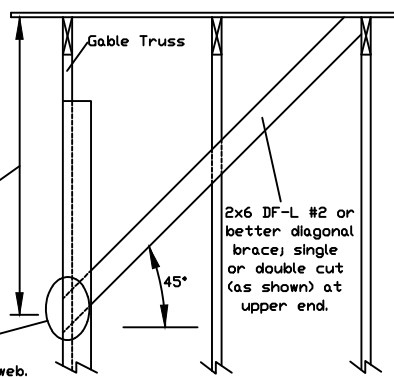
Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dr: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
	Spacing	Species			Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A
24" o.c.	24" o.c.	SPF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
			Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
			Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
		SP	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		DFL	Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
			#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
16" o.c.	16" o.c.	SPF	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
DFL	Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"		
	Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"		

Vertical length shown  
in table above.

Connect diagonal at midpoint of vertical web.



The diagram illustrates a roof truss system with a gabled roof. The roof is supported by a continuous bearing wall. The truss members are labeled with circled numbers 1 through 6. The roof slope is indicated as 18° on both sides. The vertical loads are represented by asterisks (\*) and double asterisks (\*\*). The reactions are shown as upward arrows at the supports. The diagram is labeled "Symm About" at the top center and "Continuous Bearing" at the bottom center. A note at the bottom right reads "Refer to chart above for max. allowable vertical loads." A circular stamp with the name "FERNANDO M. VILLAR" is visible in the bottom right corner.

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Southern Pine\*\*\*

#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir
#1 & Btr
#1

Southern Pine\*\*\*

#1	#1
#2	#2

\*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Wind Load deflection criterion is  $L/240$ .

Gable end supports load from 4' 0" outlookers  
with 2' 0" overhang, or 12" plywood overhang.

※※For (2) 'L' braces: space nails at 3' o.c.  
in 18' end zones and 6' o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



13723 Riverport Drive  
Suite 200  
Maryland Heights, MO 63043

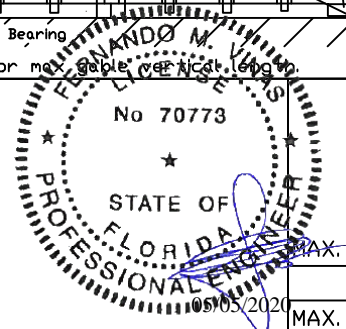
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~~MAX. TOT. LD. 60 PSF~~

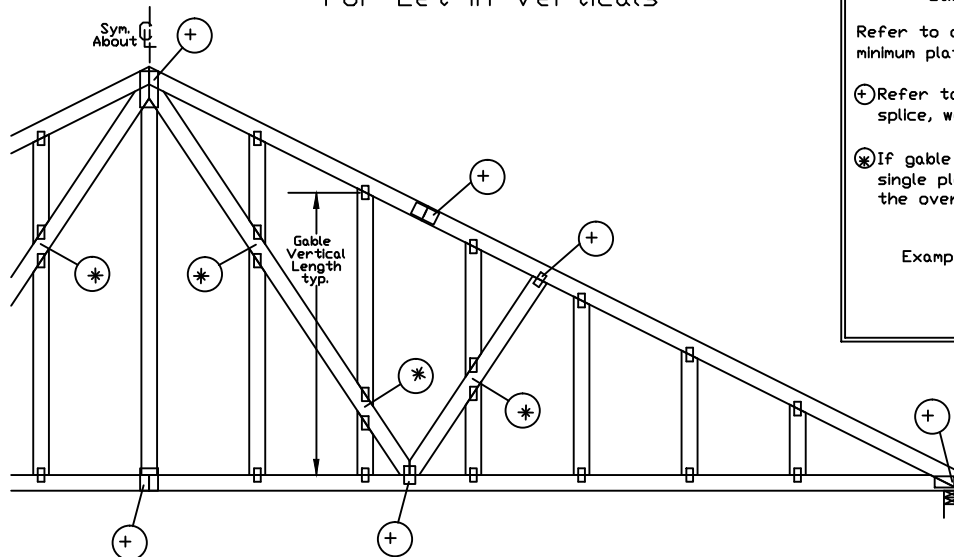
MAX. SPACING 24.0"

DATE	10/01/14
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DRWG A14030ENC101014



# Gable Detail For Let-in Verticals



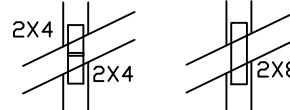
## Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

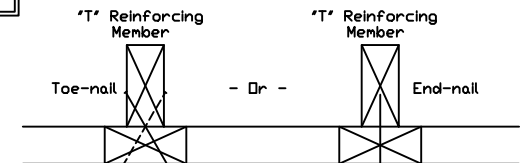
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



## 'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

## Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length  
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x3",min) Nails at 4' o.c. plus  
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus  
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

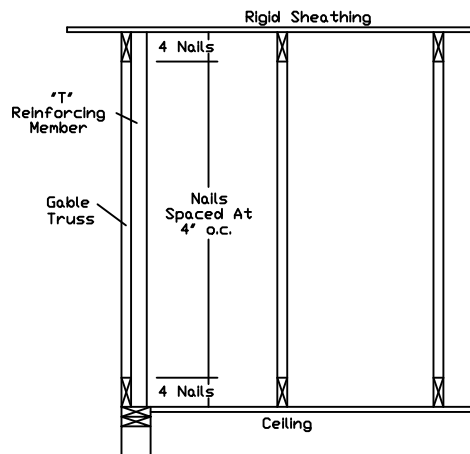
## ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,  
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

## ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,  
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,  
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,  
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,  
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,  
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,  
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,  
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



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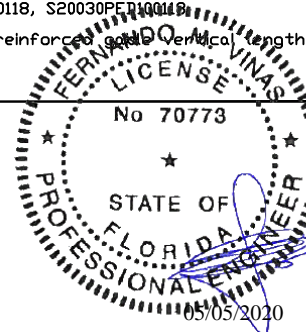
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13723 Riverport Drive  
Suite 200  
Maryland Heights, MO 63043



COA#0-278

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"



# CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

## Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

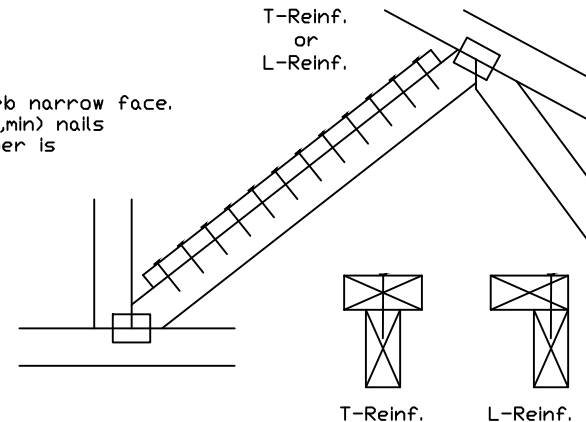
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*) Center scab on wide face of web. Apply (1) scab to each face of web.

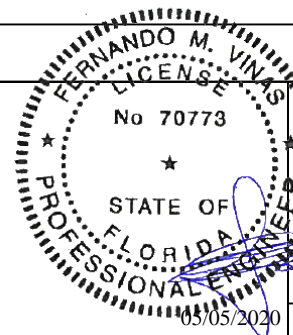
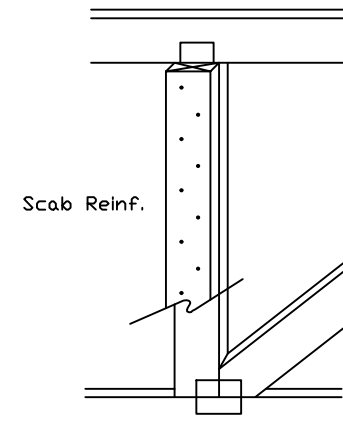
## T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



## Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



PSF	REF	CLR Subst.
PSF	DATE	01/02/19
PSF	DRWG	BRCLBSUB0119
PSF		
PSF		
DUR. FAC.		
SPACING		



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Maryland Heights, MO 63043

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# Gable Stud Reinforcement Detail

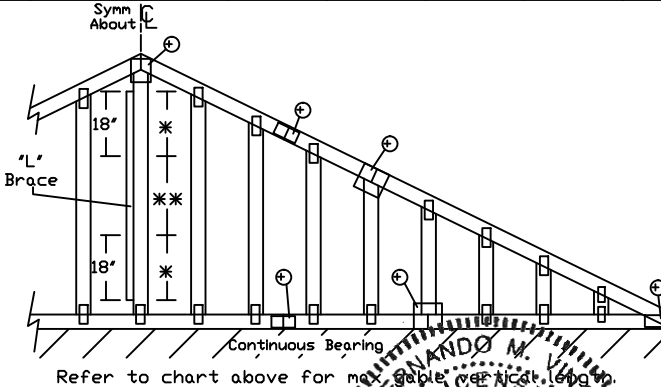
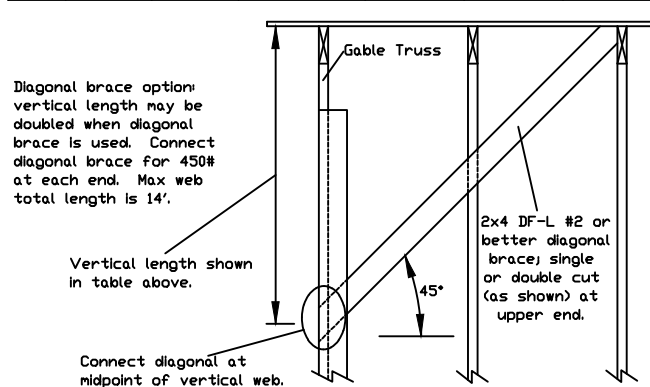
ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace *	
	Spacing	Species			Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A
24" o.c.	SPF	HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3'	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
		DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
	Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	14' 0"	
		#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	SPF	HF	Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
			#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
12" o.c.	SPF	HF	Stud	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"



Bracing Group Species and Grades:			
Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine****	
#3		#3	
Stud		Stud	
Standard		Standard	
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine****	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

\*\*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:  
Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
\* For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.  
\*\*For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.  
'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

**ALPINE**  
AN ITW COMPANY

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Suite 200  
Maryland Heights, MO 63043

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Professional Engineer  
No 70773  
STATE OF FLORIDA  
MAX. TOT. LD. 60 PSF  
MAX. SPACING 24.0'

REF ASCE7-10-GAB14015  
DATE 10/01/14  
DRWG A14015ENC101014

# GABLE STUD REINFORCEMENT DETAIL

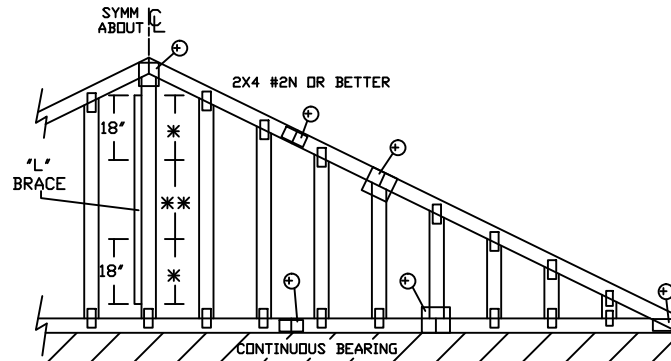
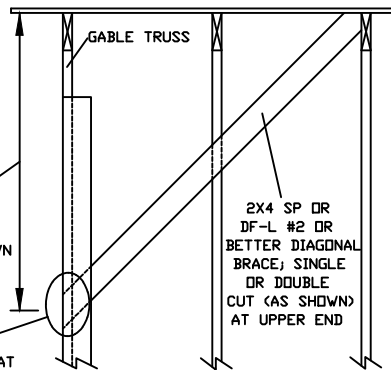
ASCE 7-05: 140 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH	2X4 GABLE VERTICAL		BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *	(1) 2X4 "L" BRACE *	(2) 2X4 "L" BRACE **	(1) 2X6 "L" BRACE *	(2) 2X6 "L" BRACE **					
	SPACING	SPECIES			GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
24" O.C.	SPF	HF	#1 / #2	3' 0"	4' 10"	5' 4"	6' 1"	6' 5"	7' 3"	7' 8"	9' 6"	10' 1"	11' 4"	12' 0"
			#3	2' 10"	4' 0"	4' 4"	5' 3"	5' 10"	7' 2"	7' 7"	8' 4"	9' 2"	11' 2"	11' 10"
			STUD	2' 10"	4' 7"	5' 0"	6' 0"	6' 4"	7' 2"	7' 7"	9' 5"	9' 11"	11' 2"	11' 10"
			STANDARD	2' 10"	4' 2"	4' 7"	5' 7"	6' 1"	7' 2"	7' 7"	8' 8"	9' 7"	11' 2"	11' 10"
	SP	DFL	#1	3' 3"	4' 11"	5' 4"	6' 2"	6' 6"	7' 4"	7' 9"	9' 8"	10' 3"	11' 6"	12' 2"
			#2	3' 2"	4' 9"	5' 2"	6' 2"	6' 6"	7' 4"	7' 9"	9' 8"	10' 2"	11' 5"	12' 1"
			#3	3' 0"	3' 10"	4' 2"	5' 1"	5' 7"	6' 10"	7' 7"	7' 11"	8' 9"	10' 9"	11' 11"
			STUD	3' 0"	4' 0"	4' 4"	5' 3"	5' 10"	7' 1"	7' 8"	8' 3"	9' 1"	11' 2"	12' 0"
16" O.C.	SPF	HF	#1 / #2	3' 5"	5' 11"	6' 2"	6' 11"	7' 4"	8' 3"	8' 9"	10' 11"	11' 6"	13' 0"	13' 9"
			#3	3' 3"	4' 10"	6' 2"	6' 6"	7' 2"	8' 2"	8' 8"	10' 2"	11' 2"	12' 10"	13' 7"
			STUD	3' 3"	5' 8"	6' 1"	6' 10"	7' 3"	8' 2"	8' 8"	10' 9"	11' 4"	12' 10"	13' 7"
			STANDARD	3' 3"	5' 1"	5' 7"	6' 9"	7' 3"	8' 2"	8' 8"	10' 8"	11' 4"	12' 10"	13' 7"
	SP	DFL	#1	3' 8"	6' 0"	6' 4"	7' 1"	7' 6"	8' 5"	8' 11"	11' 1"	11' 8"	13' 2"	13' 11"
			#2	3' 7"	5' 10"	6' 3"	7' 0"	7' 5"	8' 4"	8' 10"	11' 0"	11' 8"	13' 1"	13' 10"
			#3	3' 5"	4' 8"	5' 1"	6' 3"	6' 10"	8' 3"	8' 9"	9' 9"	10' 9"	13' 0"	13' 9"
			STUD	3' 5"	4' 10"	5' 4"	6' 5"	7' 1"	8' 3"	8' 9"	10' 1"	11' 2"	13' 0"	13' 9"
12" O.C.	SPF	HF	STANDARD	3' 4"	4' 3"	4' 8"	5' 9"	6' 3"	7' 9"	8' 6"	8' 11"	9' 10"	12' 2"	13' 5"
			#1 / #2	3' 10"	6' 6"	6' 10"	7' 8"	8' 1"	8' 3"	9' 8"	12' 0"	12' 8"	14' 0"	14' 0"
			#3	3' 7"	5' 7"	6' 2"	7' 6"	8' 0"	9' 0"	9' 6"	11' 9"	12' 6"	14' 0"	14' 0"
			STUD	3' 7"	6' 5"	6' 9"	7' 7"	8' 0"	9' 0"	9' 6"	11' 10"	12' 6"	14' 0"	14' 0"
	SP	DFL	STANDARD	3' 7"	5' 11"	6' 5"	7' 7"	8' 0"	9' 0"	9' 6"	11' 10"	12' 6"	14' 0"	14' 0"
			#1	4' 1"	6' 7"	6' 11"	7' 9"	8' 3"	9' 3"	9' 9"	12' 2"	12' 11"	14' 0"	14' 0"
			#2	4' 0"	6' 7"	6' 11"	7' 9"	8' 2"	9' 2"	9' 9"	12' 2"	12' 10"	14' 0"	14' 0"
			#3	3' 10"	5' 5"	5' 11"	7' 2"	7' 11"	9' 1"	9' 8"	11' 3"	12' 5"	14' 0"	14' 0"
			STUD	3' 10"	5' 7"	6' 2"	7' 5"	8' 1"	9' 1"	9' 8"	11' 8"	12' 8"	14' 0"	14' 0"
			STANDARD	3' 9"	4' 11"	5' 5"	6' 7"	7' 3"	8' 11"	9' 7"	10' 4"	11' 5"	14' 0"	14' 0"
			#1 / #2	3' 10"	6' 6"	6' 10"	7' 8"	8' 1"	8' 3"	9' 8"	12' 0"	12' 8"	14' 0"	14' 0"
			#3	3' 7"	5' 7"	6' 2"	7' 6"	8' 0"	9' 0"	9' 6"	11' 9"	12' 6"	14' 0"	14' 0"

DIAGONAL BRACE OPTION:  
VERTICAL LENGTH MAY BE  
DOUBLED WHEN DIAGONAL  
BRACE IS USED. CONNECT  
DIAGONAL BRACE FOR 960#  
AT EACH END. MAX WEB  
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN  
IN TABLE ABOVE.

CONNECT DIAGONAL AT  
MIDPOINT OF VERTICAL WEB.



REFER TO CHART ABOVE FOR MAX. GABLE VERTICAL LENGTH.

## BRACING GROUP SPECIES AND GRADES:

GROUP A:			
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD
DOUGLAS FIR-LARCH		SOUTHERN PINE	
#3	STUD	#3	STUD
STANDARD	STANDARD	STUD	STANDARD

GROUP B:			
HEM-FIR			
#1 & BTR	#1		
SOUTHERN PINE		DOUGLAS FIR-LARCH	
#1	#2	#1	#2

## GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 160 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH 'L' BRACE WITH 10d NAILS. (0.128"x3" min.)
- \* FOR (1) 'L' BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.
- \* FOR (2) 'L' BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6' O.C. BETWEEN ZONES.
- 'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

## GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	2X3
GREATER THAN 4' 0", BUT LESS THAN 10' 0"	3X4
GREATER THAN 10' 0"	3.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



13723 Riverport Drive  
Suite 200  
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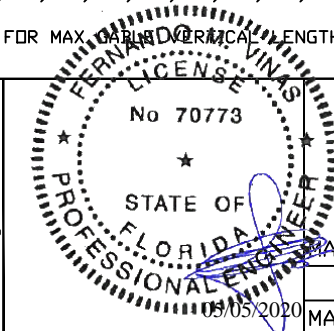
\*\*\*WARNING\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING  
\*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:  
ALPINE: [www.alpineitw.com](http://www.alpineitw.com) TPI: [www.tpinet.org](http://www.tpinet.org) SBCA: [www.sbcindustry.org](http://www.sbcindustry.org) ICC: [www.iccsafe.org](http://www.iccsafe.org)



REF ASCE7-05-GAB14015

DATE 10/01/14

DRWG A14015051014

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

Valley Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exp. C,  $K_{zt}=1.00$

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.

Bot Chord 2x4 SP #2N or SPF #1/#2 or better.

Weds 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

\*\* Attach each valley to every supporting truss with:

(2) 16d box (0.135" x 3.5") nails toe-nailed for

ASCE 7-10 160 mph. 30' Mean Height, Enclosed

Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00

Or

ASCE 7-10 140 mph. 30' Mean Height, Enclosed

Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are ITW BCG Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9' apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with properly attached, rated sheathing applied prior to valley truss installation.

 $\square r$ 

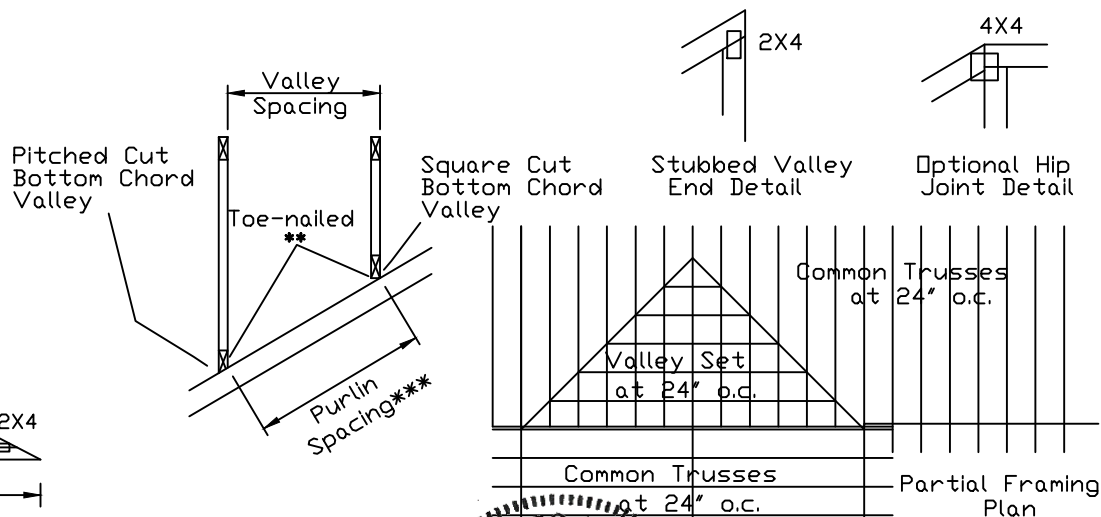
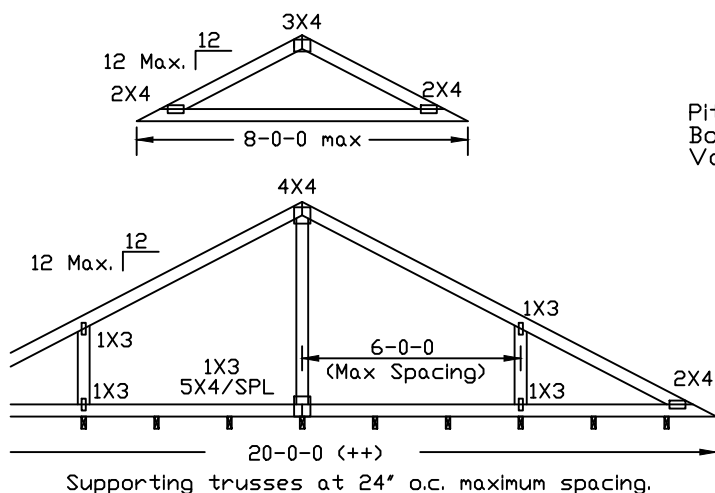
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

 $\square_r$ 

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

\*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



13723 Riverport Drive  
Suite 200  
Maryland Heights, MO 63043

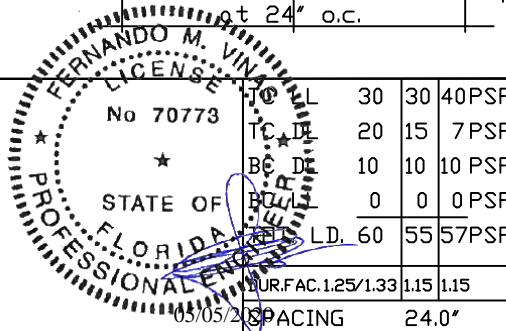
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[illegible]