DATE:
CHECKED BY:

SHEET NUMBER

A-1

OF 4 SHEETS

1405.2 SF 499.7 SF 35.0 SF

1939,90 SF

ELEVATION PLAN

SCALE: 1/4"=1'-0"

AREA SUMMARY

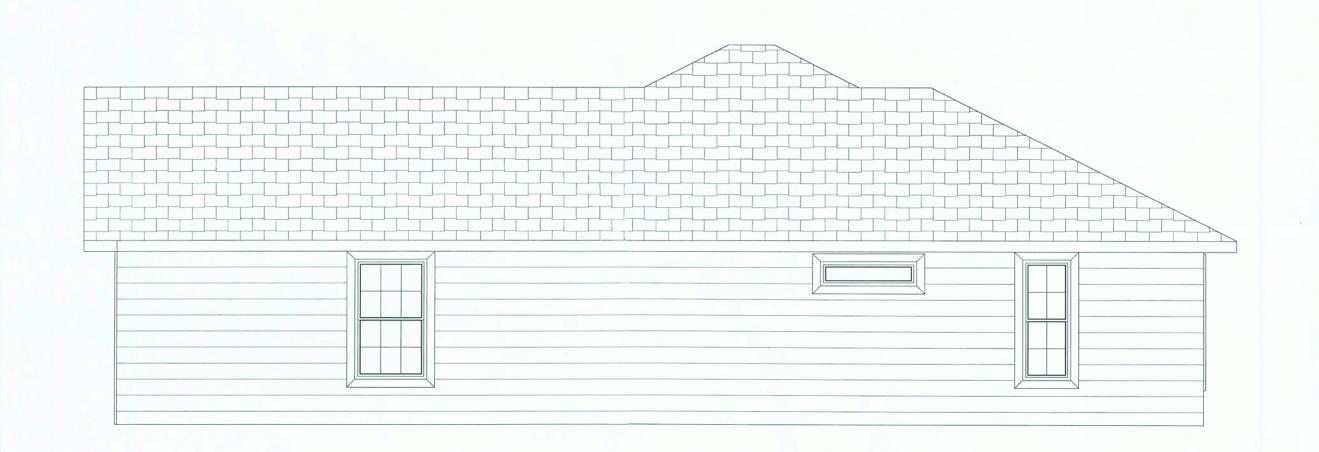
LIVING AREA

GARAGE

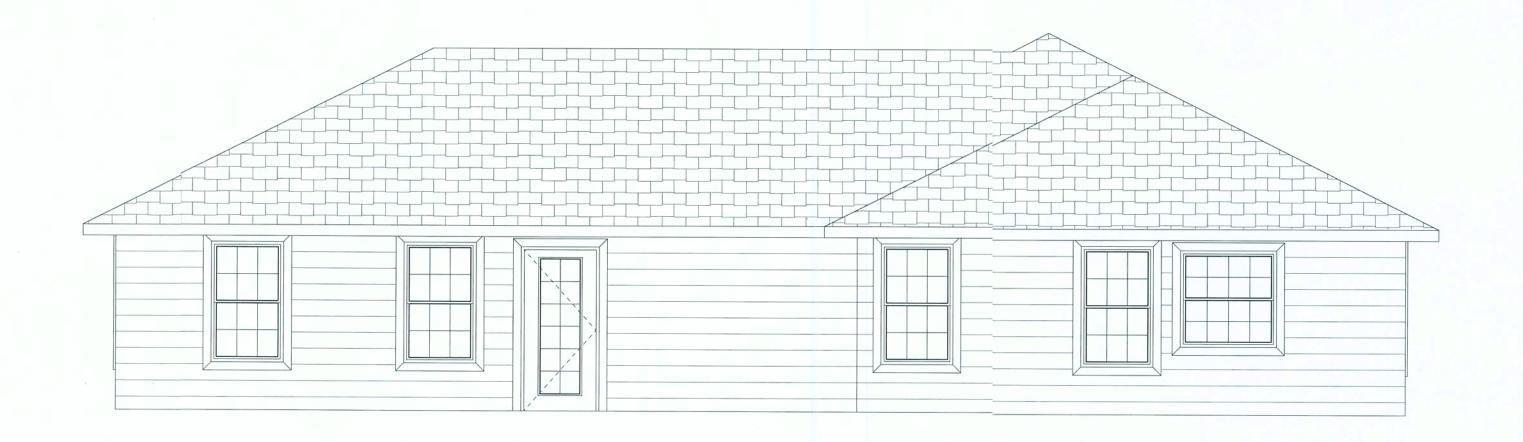
PORCHES

TOTAL AREA











BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

DATE: CHECKED BY:

SHEET NUMBER

A-2OF 4 SHEETS

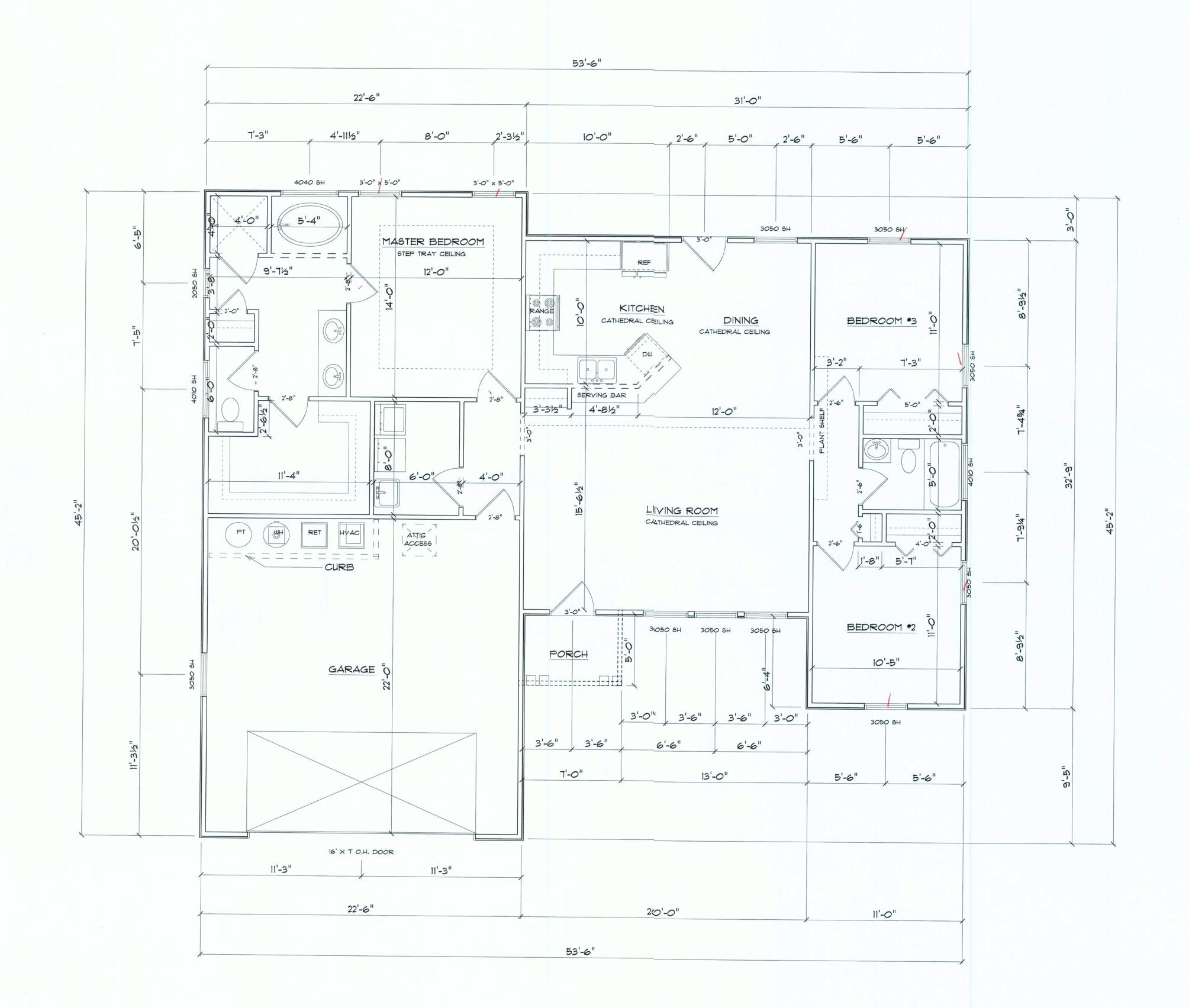
AREA SUMMARY

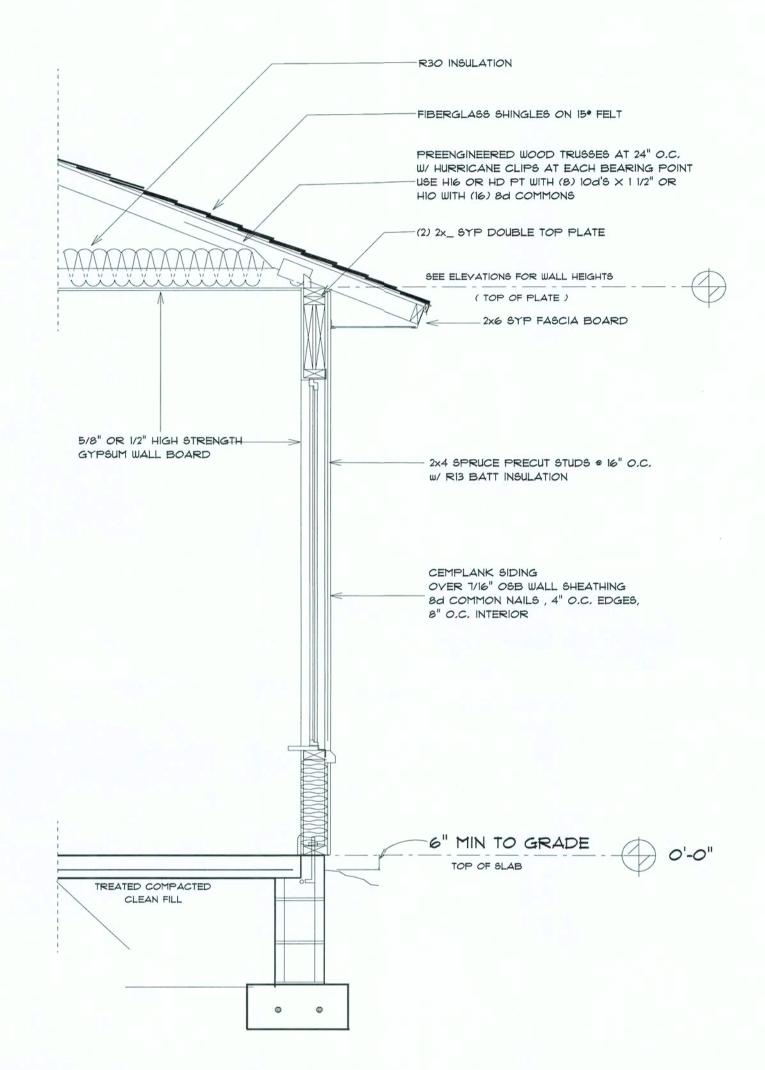
LIVING AREA GARAGE PORCHES TOTAL AREA

1405.2 SF 499.7 SF 35.0 SF 1939.90 SF

MAIN FLOORPLAN

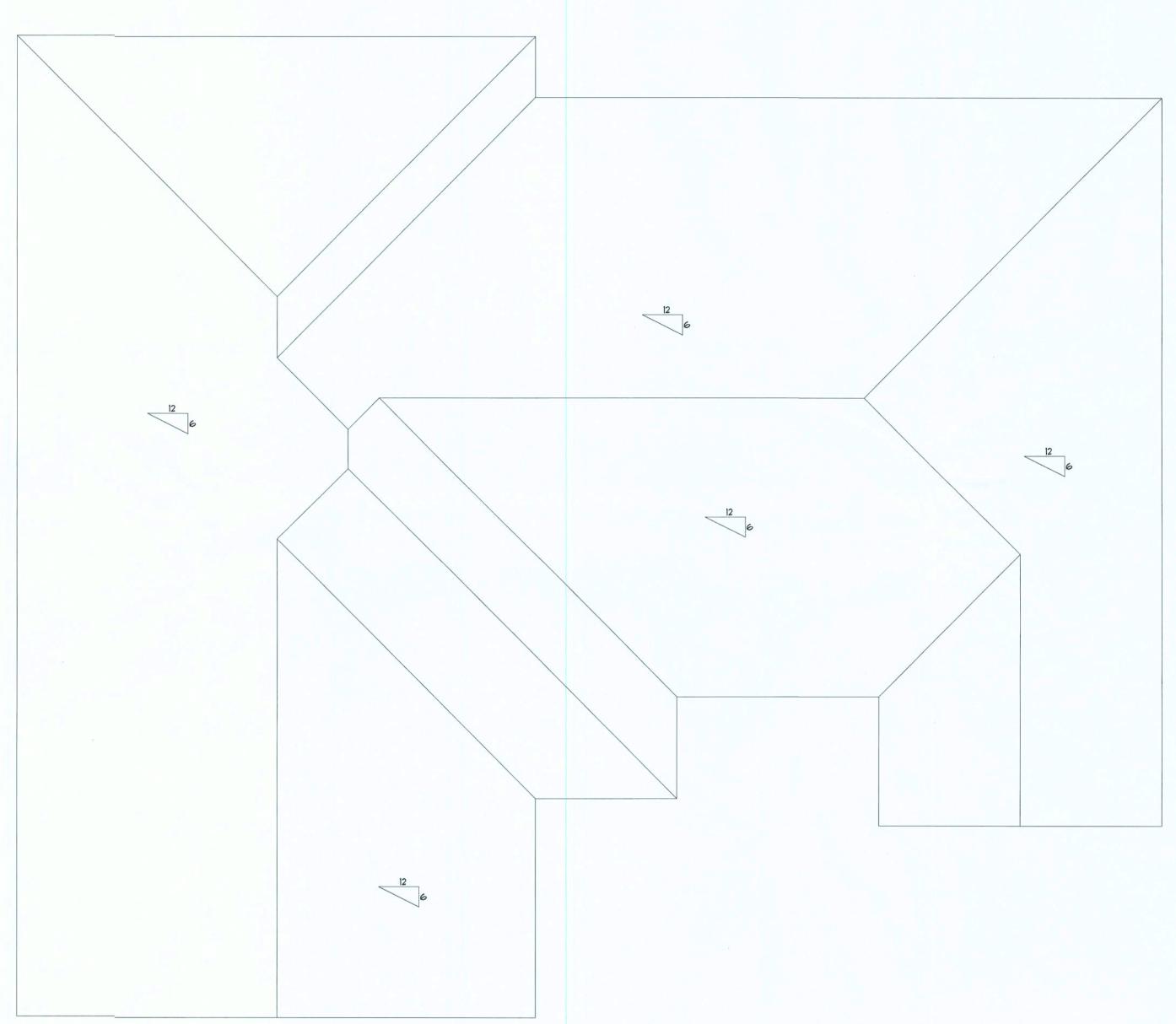
SCALE: 1/4"=1'-0"





TYPICAL WALL SECTION

2 × 4 STUD WALL W/ SIDING



SHEET NUMBER

OF 4 SHEETS

AREA SUMMARY

LIVING AREA GARAGE PORCHES TOTAL AREA

1405.2 SF 499.7 SF 35.0 SF 1939,90 SF

ROOF PLAN

SCALE: 1/4"=1'-0"

OR:

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

CHECKED BY:

ELECTRICAL	SYMBOL
ceiling fan spotlights 1	
Can Light	\Box
chandelier	<u>0</u> 0 00 00 00
fluorescent fixture	
vanity bar light	<u> </u>
wall mount 1	Q
electrical panel	11
Fan - Light Combo	
cable ty outlet	TV
light	
outlet	Ф
outlet 220v	Ф
outlet gfi	Фан
smoke detector	•
switch	\$
switch 3 way	\$3
telephone	∇

ELECTRICAL PLAN NOTES

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFCI CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

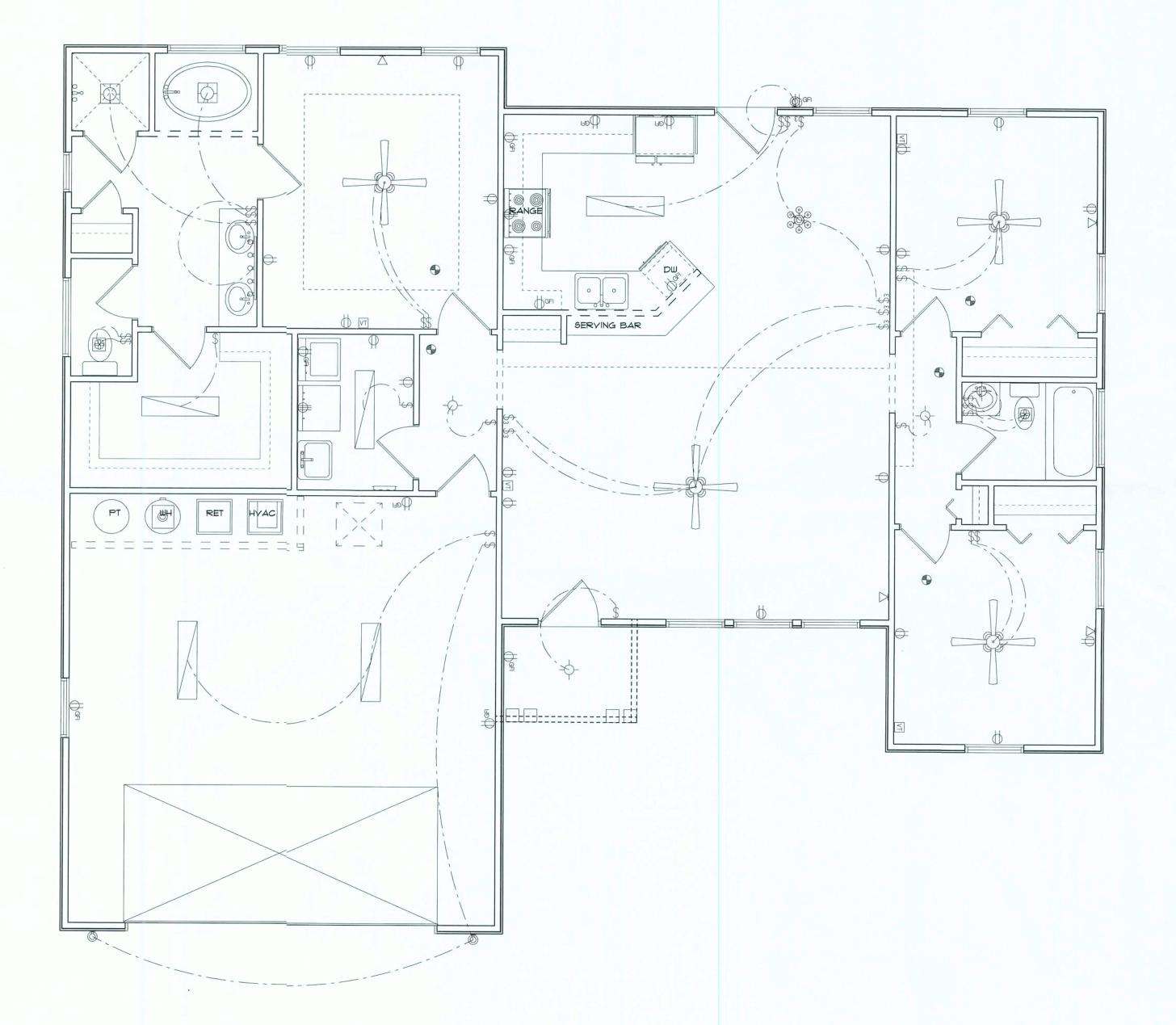
INSTALLATION SHALL BE PER NAT'L, ELECTRIC CODE,

ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, NCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROLTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE I COPY OF AS-BUILT DWGS TO OWNER & I COPY TO THE PERMIT ISSUING AUTHORITY.



3REEN ACRES LOT 1

BRIAN S. CRAWFOR

ARCHITECTURAL DESIGN

DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

DATE:

SHEET NUMBER

A-4

OF 4 SHEETS

1405.2 SF

499,7 SF

35.0 SF

1939,90 SF

ELECTRICAL PLAN

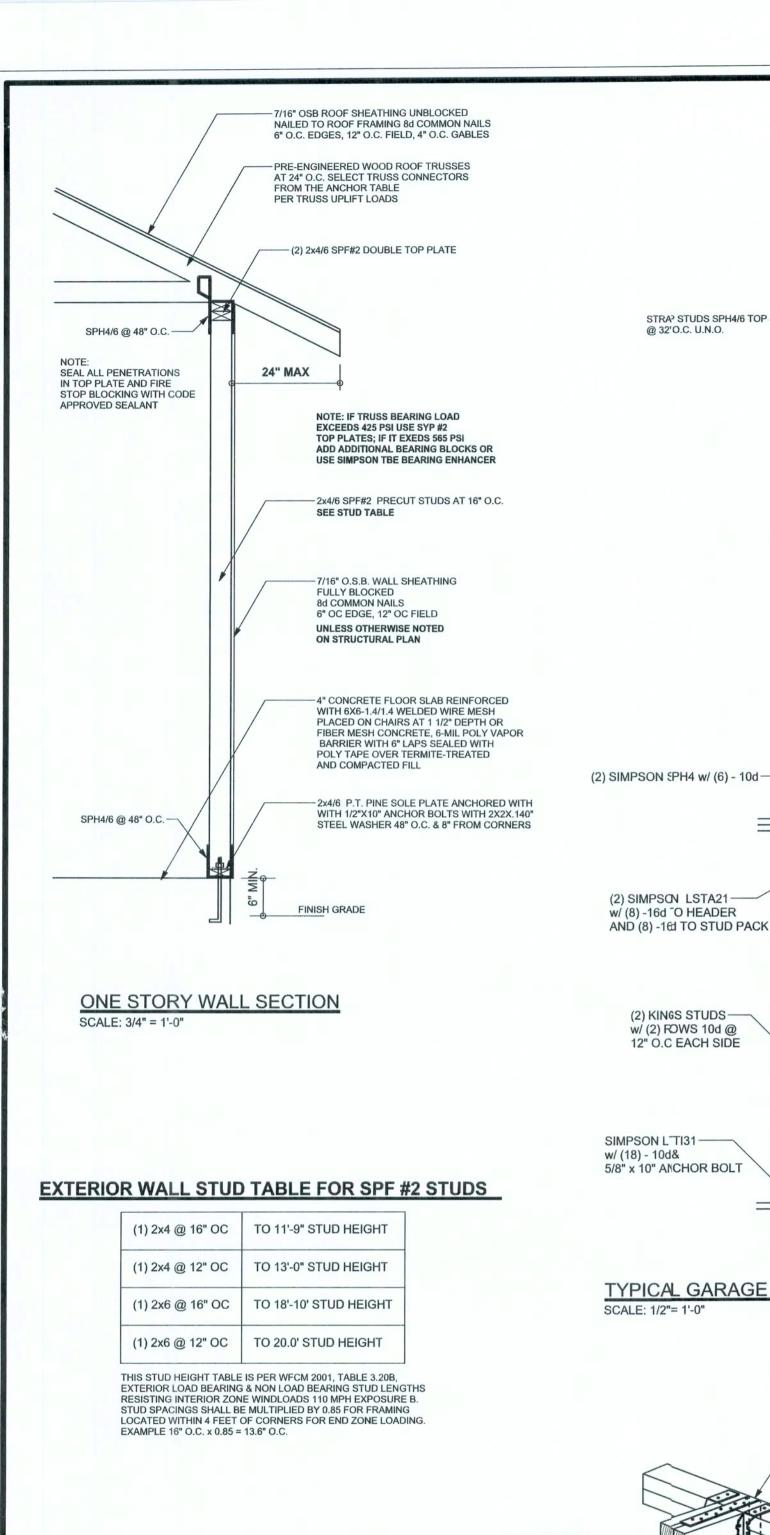
SCALE: 1/4"=1'-0"

AREA SUMMARY

LIVING AREA

GARAGE

PORCHES



SIMPSON H2.5A U.N.O.-SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

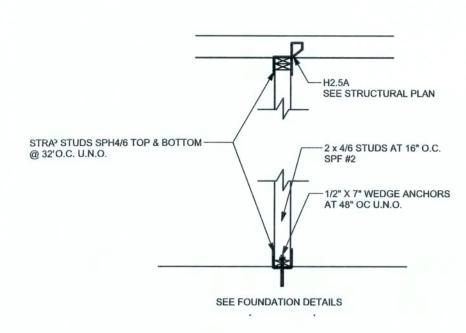
w/ (8) -16d TO HEADER AND (8) -16d TO POST

ANCHOR BOLT

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



(2) SIMPSON LSTA21-

w/ (8) -16d O HEADER

AND (8) -16d TO STUD PACK

(2) KINGS STUDS-

w/ (2) ROWS 10d @

12" O.C EACH SIDE

SIMPSON L'TI31-

5/8" x 10" ANCHOR BOLT

w/ (18) - 10d&

NAIL THRU 2x4 IITC BEAM W/4-16d

BEAM MAY BE ATTACHED IN

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

INTERIOR BEARING WALL

-(2) 2X4 SPF #2 TOP PLATE

-(2) JACKS STUDS

w/ (2) ROWS 10d @

12" O.C. EACH SIDE

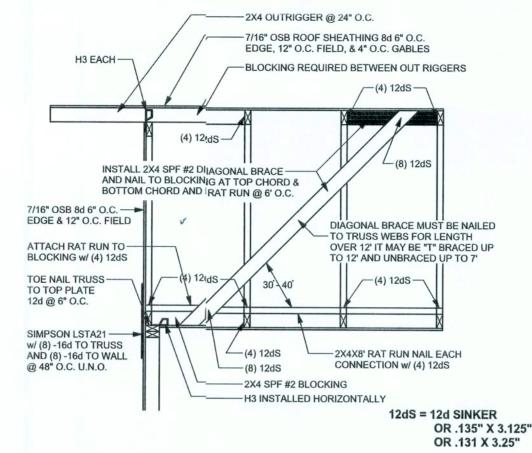
-FOUNDATION SEE

TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

SEE FOOTING DETAILS

—(2) 2X12 SYP #2 HEADER U.N.O SEE STRUCTURAL PLAN

SIMPSON SPH4 @ 48" O.C.



SPACE RAT' RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE: HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

16d (2) ROWS OF

STAGGER .131 x 3 1/4" GN

5" O.C.

4" O.C.

3" O.C.

5" O.C.

4" O.C.

3" O.C.

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

24" O.C.

18" O.C.

16" O.C.

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GN PER TABLE BELOW:

8' - 10'

11' - 15'

2x6SYP #2 DOOR BUCK --

SCALE: N.T.S.

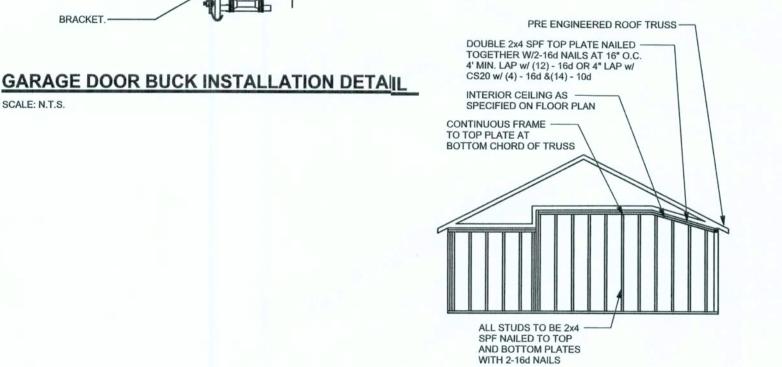
BRACKET. --

2X4 LADDER BEAM

DOOR WIDTH | 3/8" x 4" LAG

GRADE & SPECIES TABLE

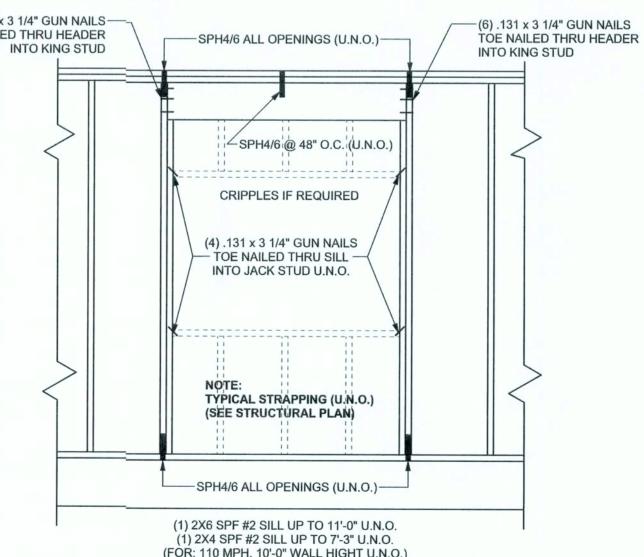
		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL**

IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED

SCALE: N.T.S.



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 31.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/64"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	ER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH A LLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	E CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND GHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 TS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE TH	ONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL AD ENGINEER IMMEDIATELY.
DESIGN, PLA	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, USS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL CATIONS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

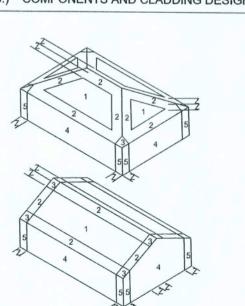
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	7
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d	,	
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
	< 2320	ABU88	18 - 16d		2-5/8" AB

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN DATA

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	WIN	ND LOADS PER FLORIDA BUILDING CODE 2004 RESI	DENTI	AL, SECTIO	N R301.2.1
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	ON	AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZOI UPPER HALF OF HILL OR ESCARPMENT 60FT IN EX	NTAL D (P. B. 3	IMENSION OFT IN EXP	OR 60 FT; NOT
1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	BUI	ILDING IS NOT IN THE HIGH VELOCITY HURRICANE .	ZONE		
2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	BUI	ILDING IS NOT IN THE WIND-BORNE DEBRIS REGIO	N		
3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	1.)	BASIC WIND SPEED = 110 MPH			
 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 	2.)	WIND EXPOSURE = B			
5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	3.)	WIND IMPORTANCE FACTOR = 1.0			
6.) MEAN ROOF HEIGHT = <30 FT 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	4.)	BUILDING CATEGORY = II			
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	5.)	ROOF ANGLE = 10-45 DEGREES			
(6.)	MEAN ROOF HEIGHT = <30 FT			
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLO	OSED B	UILDING)	
	8.)	COMPONENTS AND CLADDING DESIGN WIND PRE	ESSUR	ES (TABLE	R301.2(2))
		***	Zone		
Zone Effective Wind Area (ft2)		\\XT.		10	100



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	1	0		100
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	& Wind st Cas 5, 10	е	21.8	-29.1
8x7 Gar	age D	oor	19.5	-22.9
16x7 Ga	arage [Door	18.5	-21.0

DESIGN	LOADS		
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	 	
	30 PSF (SLEEPING ROOMS)		
	30 PSF (ATTICS WITH STORAGE)		
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)		
ROOF	20 PSF (FLAT OR <4:12)		
	16 PSF (4:12 TO <12:12)		
	12 PSF (12:12 AND GREATER)		

Cason Construction

VINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

tated dimensions supercede scaled

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orm or manner without first the express writte

CERTIFICATION: I hereby certify that I have

portions of the plan, relating to wind engineer comply with section R301.2.1, florida buildir

ode residential 2004, to the best of my

LIMITATION: This design is valid for one

MARK DISOSWAY

P.E. 53915

building, at specified location.

camined this plan, and that the applicable

nission and consent of Mark Disosway.

mensions. Refer all questions to

Mark Disosway, P.E. for resolution

Do not proceed without clarification

2056, 386-754-5419

REVISIONS

SOFTPLYN

Spec House Lot 1 Green Acres S/D

> ADDRESS: Lot 1 Green Acres S/D Columbia County, Florida

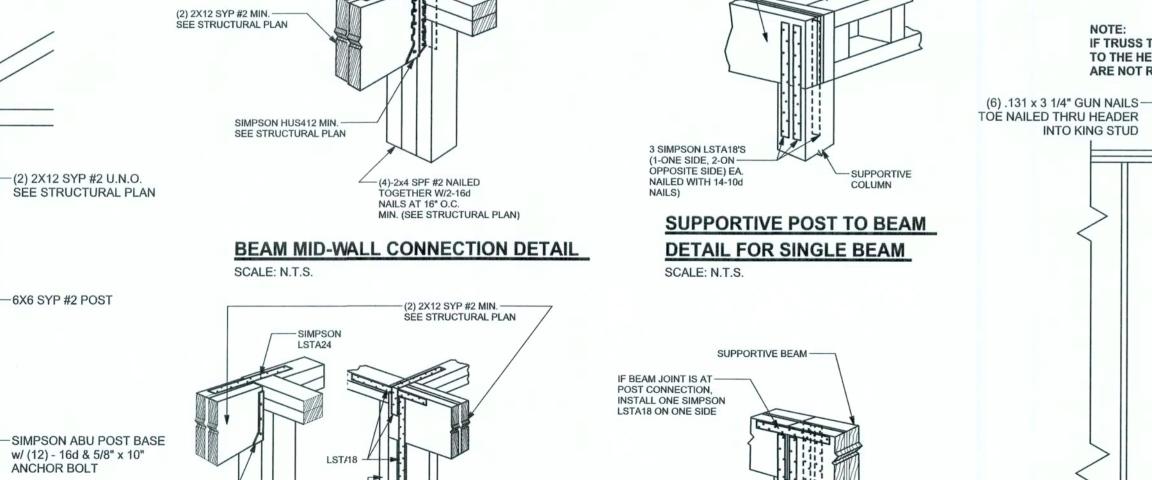
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRINTED DATE: October 31, 2007 STRUCTURAL B

FINALS DATE: 31 / Oct / 07

> JOB NUMBER: 710241 DRAWING NUMBER

> > OF 3 SHEETS



SUPPORTIVE -

4-SIMPSON LSTA18 ---(2-ONE SIDE, 2-ON └─3-1/2" P.T.

SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

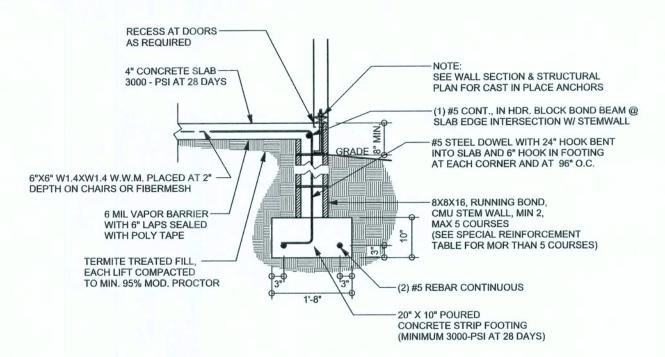
(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.) **TYPICAL HEADER STRAPING DETAIL**

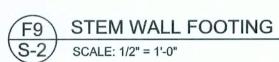
S(CALE: 1/2" = 1'-0"

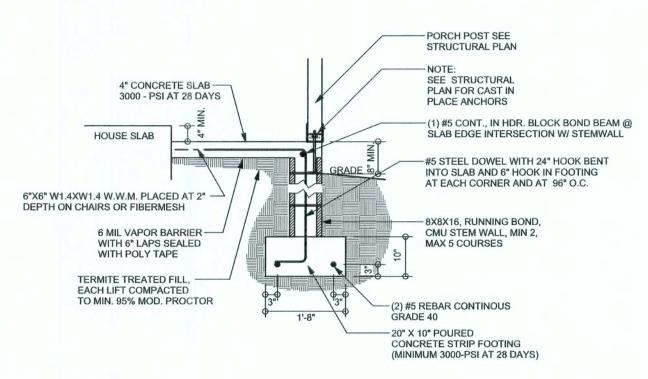
MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

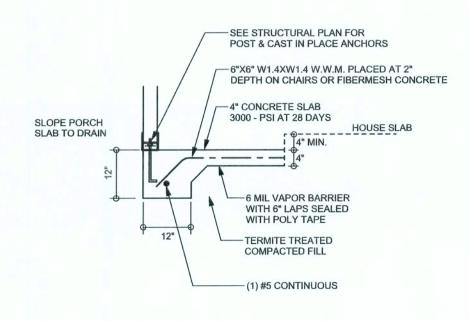
	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



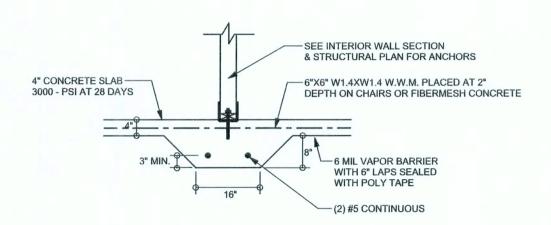




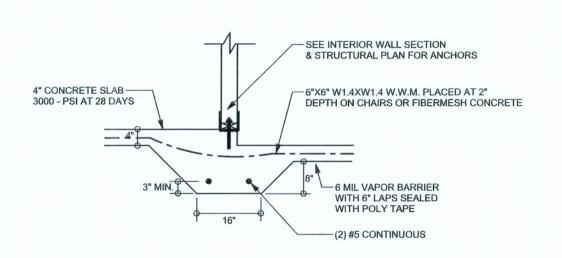
F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"

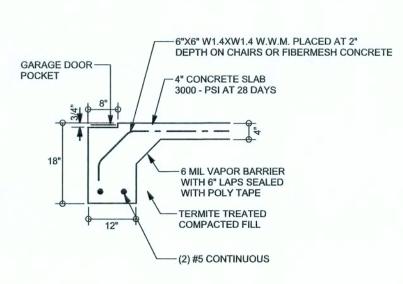


F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"

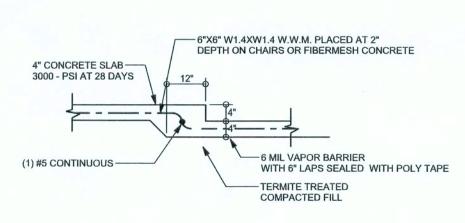
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

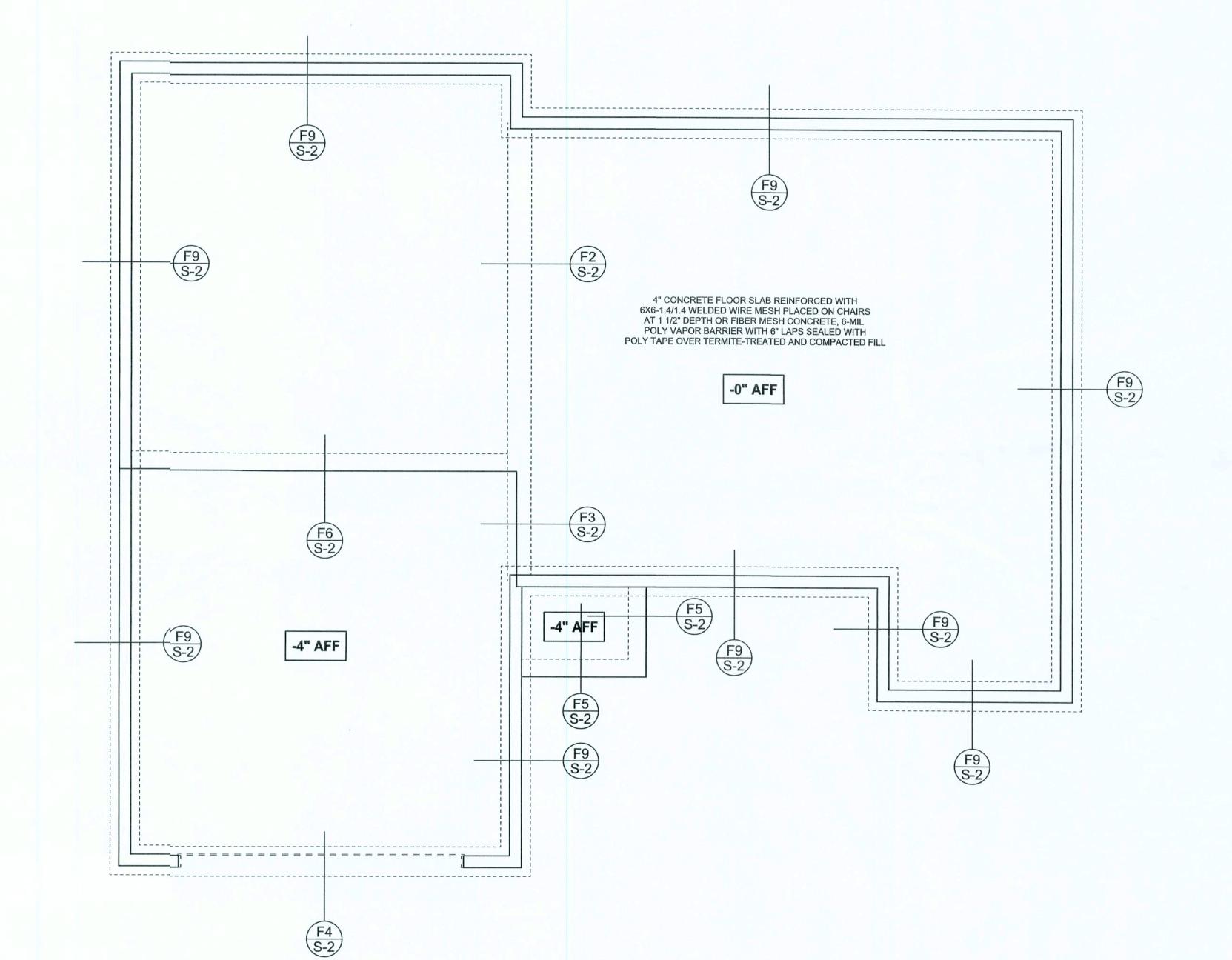
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	AL REINFOR B" CMU STEN INCHES O.C	MWALL	FOR 1	AL REINFOR 2" CMU STEI INCHES O.C	MWALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN ABOUT A DESIGN SOFTMARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable

portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

NARK DISOSWAY
P.E. \$3915

POVO

Cason Construction

Spec House Lot 1 Green Acres S/D

ADDRESS: Lot 1 Green Acres S/D Columbia County, Florida

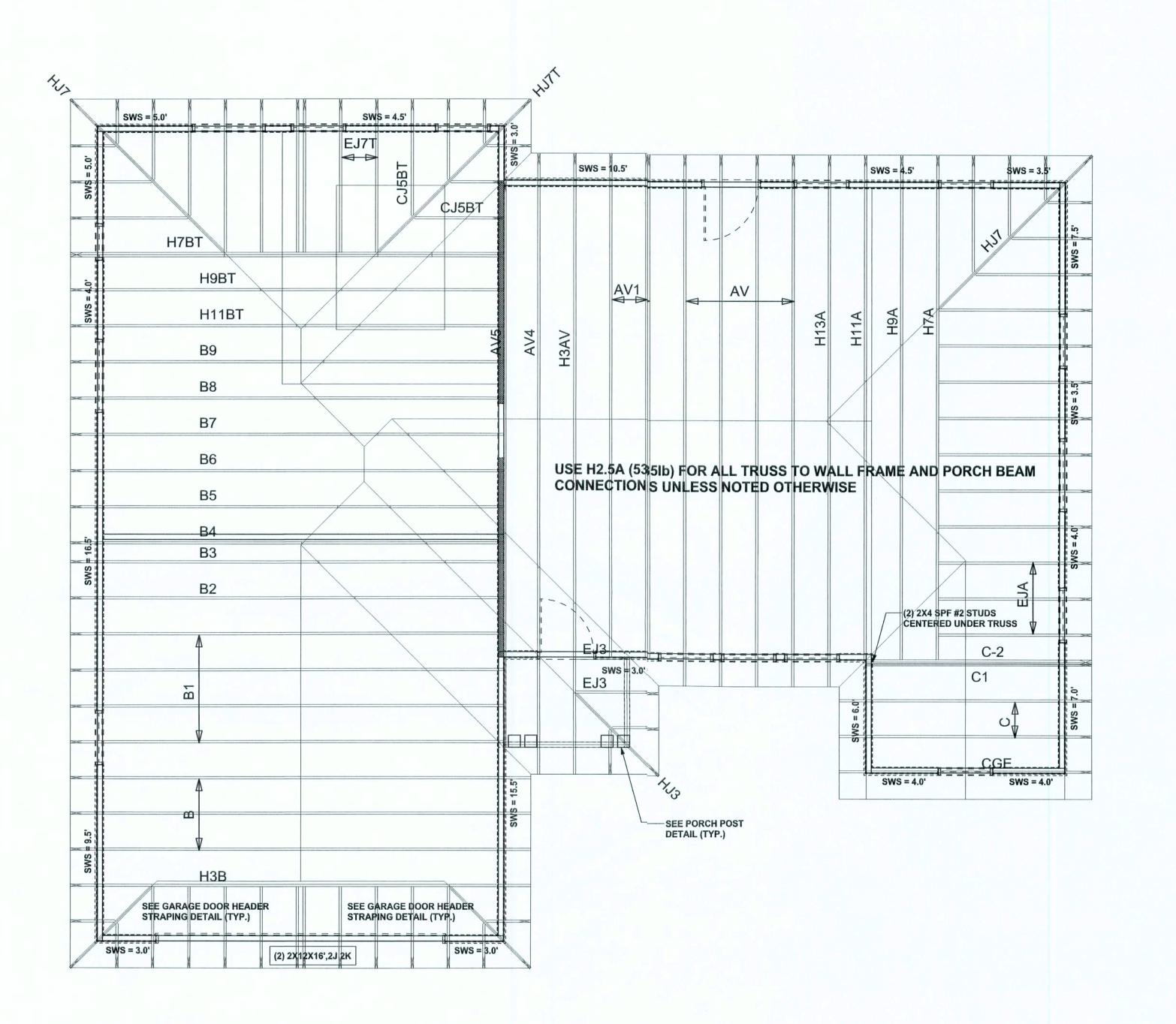
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
Oc:ober 31, 2007
STRUCTURAL BY

FINALS DATE: 31 / Oct / 07

> JOB NUMBER: 710241 DRAWING NUMBER

> > S-2 OF 3 SHEETS



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4

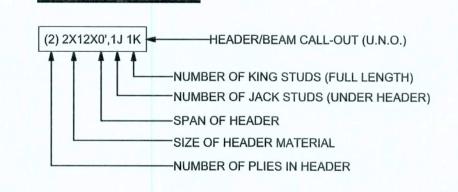
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	26.8'	81.5'
LONGITUDINAL	22.8'	45.0'

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, P0B 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
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form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at speci[†]ed location.

building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Cason Construction

Spec House Lot 1 Green Acres S/D

ADDRESS: Lot I Green Acres S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
October 31, 2007
STRUCTURAL BY:

FINALS DATE: 31 / Oct / 07

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

FURNISHED BY BUILDER. ANDERSON TRUSS

JOB #7-310

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

JOB NUMBER: 710241

710241
DRAWING NUMBER

S-3 OF 3 SHEETS