DATE 09/13/2007 Columbia County	<u> </u>	PERMIT
This Permit Expires One Ye APPLICANT MARK HADDOX	ear From the Date of Issue PHONE 755-2411	000026232
ADDRESS P.O. BOX 1755	LAKE CITY	— FL 32056
OWNER GEORGE MORTENSEN	PHONE	
ADDRESS 250 SW VARGAS WAY	FT. WHITE	— FL 32038
CONTRACTOR WOODMAN PARK/WILLIAM WOOD	PHONE 755-2411	
LOCATION OF PROPERTY 47S, TL ON JEANLEA, TL ON V	ARGAS WAY, 3RD LOT ON LEFT	_
TYPE DEVELOPMENT SFD,UTILITY EST	TIMATED COST OF CONSTRUCTION	N 117100.00
HEATED FLOOR AREA 2342.00 TOTAL ARE	EA _4074.00 HEIGHT	STORIES 1
FOUNDATION CONC WALLS FRAMED F	ROOF PITCH 7/12	FLOOR SLAB
LAND USE & ZONING A-3	MAX. HEIGHT	21
Minimum Set Back Requirments: STREET-FRONT 30.00	REAR 25.00	SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE X PP	DEVELOPMENT PERMIT NO.	
		
PARCEL ID 27-6S-16-03951-105 SUBDIVISIO		
LOT 5 BLOCK PHASE UNIT	TOTAL ACRES _	
000001449 VNV058182	My Hely	
Culvert Permit No. Culvert Waiver Contractor's License Num		er/Contractor
CULVERT 07-713 BK	JH	Y
Driveway Connection Septic Tank Number LU & Zonir	ng checked by Approved for Issue	nce New Resident
COMMENTS: ELEVATION DETERMINATION LETTER SUBMITT		nnce New Resident
	ED 12" ABOVE EXISTING GRADE	
COMMENTS: ELEVATION DETERMINATION LETTER SUBMITT NOC ON FILE	ED 12" ABOVE EXISTING GRADE Check # or	
COMMENTS: ELEVATION DETERMINATION LETTER SUBMITT NOC ON FILE FOR BUILDING & ZONIN	ED 12" ABOVE EXISTING GRADE Check # or	
COMMENTS: ELEVATION DETERMINATION LETTER SUBMITTED NOC ON FILE FOR BUILDING & ZONIN Temporary Power Foundation	ED 12" ABOVE EXISTING GRADE Check # or IG DEPARTMENT ONLY Monolithic	Cash 1349 (footer/Slab)
COMMENTS: ELEVATION DETERMINATION LETTER SUBMITT NOC ON FILE FOR BUILDING & ZONIN Temporary Power Foundation date/app. by	Check # or G DEPARTMENT ONLY Monolithic date/app. by	Cash 1349 (footer/Slab) date/app. by
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

Columbia County Building Permit Application

For Office Use Only Application # 0709-14 Date Recei	ved 9/5/07 By 4 Permit # 126232	
Application Approved by - Zoning Official Date // Date	Plans Examiner Ok Jit Date 9-7-07	
Flood Zone	Land Use Plan Map Category A-3	_
Comments Elevation Determination Letter Submitted 12"	" above existing grade	
□ NOC tvEH Deed or PA Site Plan □ State R	load Info 🛘 Parent Parcel # 🖟 Development Perm	nit
	Fax 755- 86861	
Name Authorized Person Signing Permit Mark Hadley	Phone 755-1411	
Address P.U. B.x 1755 Loler ct	Y FL SLUSG	
Owners Name Garage Mortensen	Phone	
911 Address 250 Sw Varges wing	it white Deuss	
Contractors Name Wood man Pork 13 (Ir	5 Phone 255-2411	
Address 6'- V 13-x 1755 Loke	City FL 32056	
Fee Simple Owner Name & Address		
Bonding Co. Name & Address		
Architect/Engineer Name & Address	p p	
Mortgage Lenders Name & Address People & 150	ink hake city	
Circle the correct power company - FL Power & Light - Clay Ele	ec Suwannee Valley Elec Progressive Energ	av
Property ID Number 17-65-16 3951-105 Est	ilmated Cost of Construction) 80 000 00	21.
Subdivision Name Fux wood 5/0	Lot Block_ Unit Phase \	_
Driving Directions 47 South to J	eanlea (1eft) to	
Verges way (1ett) graperty	3rd laft	
8		
Type of Construction Residential Num	nber of Existing Dwellings on Property	_
Total Acreage Lot Size Do you need a <u>Culvert</u>	Permit or Culvert Waiver of Have an Existing Dr	der
Actual Distance of Structure from Property Lines - Front 1		_
Total Building Height 1 1 3 5/31 Number of Stories Hea	rted Floor Area 134) Roof Pitch 7-11	
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit and tall laws regulating construction in this jurisdiction. OWNERS AFFIDAVIT: I hereby certify that all the foregoing informed.	allations as indicated. I certify that no work or that all work be performed to meet the standards o	─ >f
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information compliance with all applicable laws and regulating construction and	nd zoning.	
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTENLENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF	ID TO OBTAIN EMANCING COMELY TARITH MOUNT	{
Owner Builder or Authorized Person by Notarized Letter	Contractor Signature	 > >
STATE OF FLORIDA COUNTY OF COLUMBIA	Contractors License Number CDC OSS/8 Competency Card Number NOTARY STAMP/SEAL	<u>.</u> _
Sworn to (or affirmed) and subscribed before me	My Commission DD293888 Expires February 24, 2008	i
this 24 day of August 2007.	Drenda Deris	
Personally known or Produced Identification	Notary Signature (Revised Sept. 200	- 06

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 07-251 POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

PERMIT	MO		
PERMIT	NO.		

TAX FOLIO NO.: R03951-105

NOTICE OF COMMENCEMENT

STATE OF FLORIDA COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:

Lot 5 of FOXWOOD PHASE ONE, according to the plat thereof as recorded in Plat Book 8, Page 86, public records of Columbia County, Florida.

- 2. General description of improvement: Construction of Dwelling
- Owner information:
 - Name and address: George Mortensen, 5 Stockton Commons, Yaphank, NY 11980
 - b. Interest in property: Fee Simple
 - c. Name and address of fee simple title holder (if other than Owner):
- Contractor: Woodman Park Builders Post Office Box 3535, Lake City, FL 32056

Contractor's Telephone Number: (386) 755-2411

- 5. Surety
 - a. Name and address: None
- Lender: PEOPLES STATE BANK, 350 SW Main Blvd., Lake City, Florida 32025.

Lender's Telephone Number: 386-754-0002

- 7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None
- 8. In addition to himself, Owner designates LONNIE T. HALTIWANTER of PEOPLES STATE BANK, 350 SW Main Blvd., Lake City, Florida 32025, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
- 9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

GEORGE MORTENSEN

The foregoing instrument was acknowledged before me this 14th day of August, 2007, by GEORGE MORTENSEN, who is personally known to me and did not take an oath.

Notary Rublic My commission expires:

STATE OF FLORIDA COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing

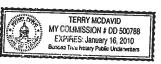
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS

Bu Laron Lagle

Deputy Clerk

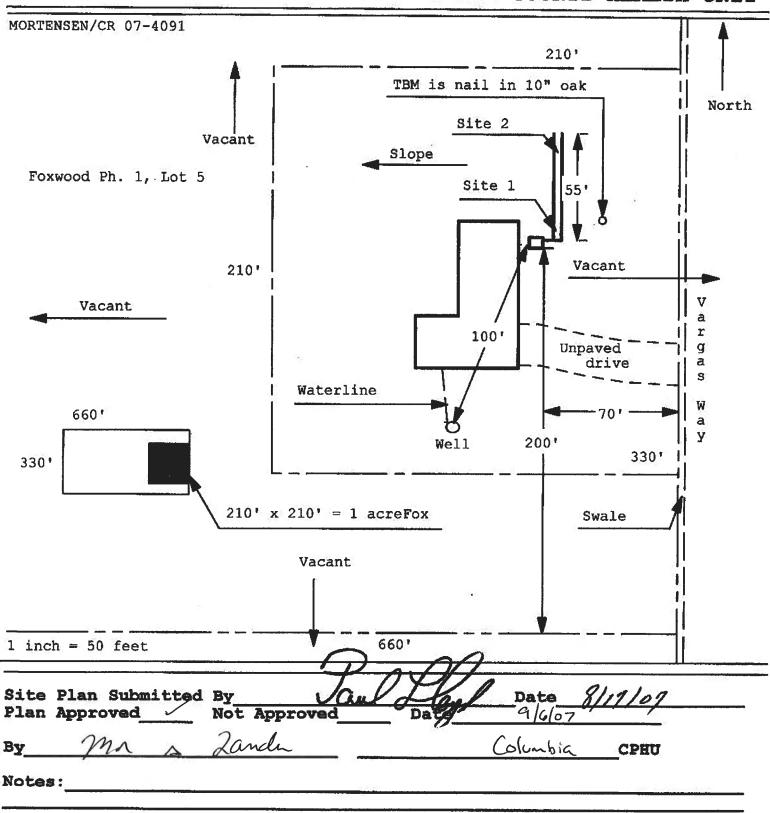
Date_08-21-2007





Application for ourside sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: (1/2-1/2)

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Mark Disosway, P.E.

POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-269-4871

August 20, 2007

Building and Zoning, Columbia County, Florida

Re: Site Evaluation, Mortensen Residence, 447 SW Eva Terrace, Columbia County, Florida, 27-6S-16-03951-105, Columbia County, FL

Dear Building Inspector:

I have reviewed the Flood Insurance Rate Map and NGS topographic map and performed a site evaluation for the Mortensen Residence, SW Vargas Way, Columbia County, Florida, 27-6S-16-03951-105, Columbia County, FL. The natural grade elevation at the perimeter of the proposed home site as shown on the permit is less than one foot above the nearby county road that it fronts on, SW Vargas Way. The lot is in Zone X on the FEMA rate map, attached. The builder states that although the proposed home site is less than 1 foot above the road it is on a knoll and the road slopes down steadily to the south.

Based on my personal inspection of the site if the finished floor elevation is set at least 12" above natural grade it will be higher than the land to the south and west of the lot and storm water will flow down to a large local depression which is about 15 feet lower than the proposed home site and is not a flood zone per the FEMA data.

Based on topo maps, FEMA data, and visual inspection the proposed finished floor elevation is at an adequate elevation to avoid flooding.

The finished floor elevation must be minimum 6" above finished grade per FBC2001. The finished grade should slope down from that elevation for another 6" within 12 feet away from the house and hanger in all directions so that all runoff drains away from the house. The owner must maintain the swales, slopes, and ditch to provide free drainage to the ditch and prevent any possibility of storm water backing up into the house.

The owner should be aware that if free drainage is not maintained thru fields and subdivisions across roads and thru culverts to the local depression, or if future development in the area causes increased storm water run off, or if rainfall occurs with greater flooding effect than the design storm, the level of the creek could rise higher than anticipated and nearby Zone A areas could be larger than indicated by FEMA and his house would be more susceptible to flooding. Owner should verify topo data by shooting grades to make sure there is a continuous downward slope to the creek on the path traced on the attached topo map.

Sincerely,

MULCIOSU

20AU607

Mark Disosway, PE

SECTION 1804 FOOTINGS AND FOUNDATIONS; §1804.1.1 Foundations shall be built on undisturbed soil or properly compacted fill material. Foundations shall be constructed of materials described in this chapter.

§1804.1.2 Pile foundations shall be designed and constructed in accordance with §1805.

§1804.1.3 The bottom of foundations shall extend no less than 12 inches (305 mm) below finish grade.

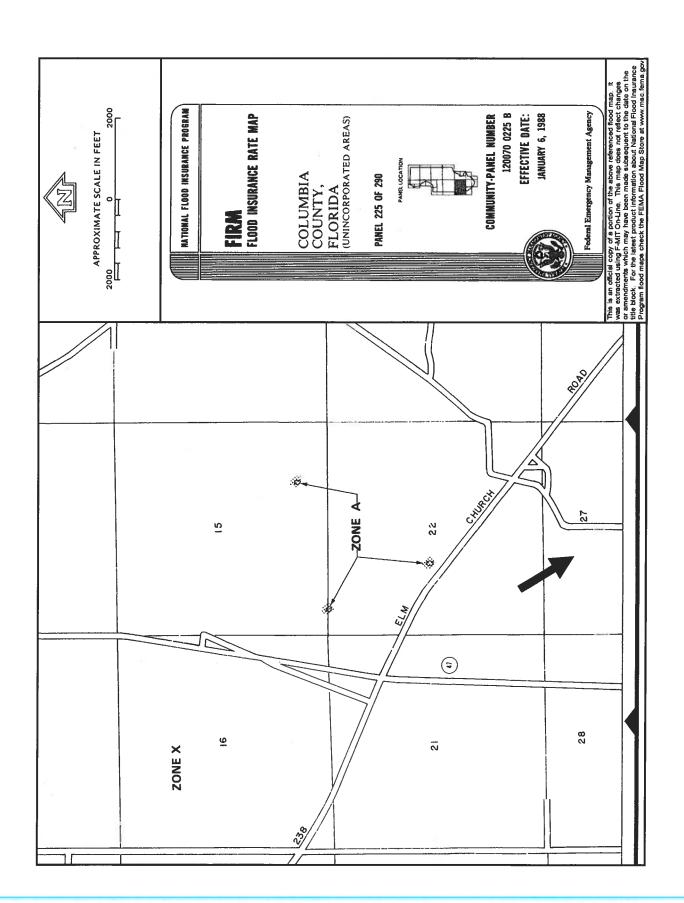
§1804.1.4 Temporary buildings and buildings not exceeding one story in height and 400 sq ft (37 m^2) in area shall be exempt from these requirements.

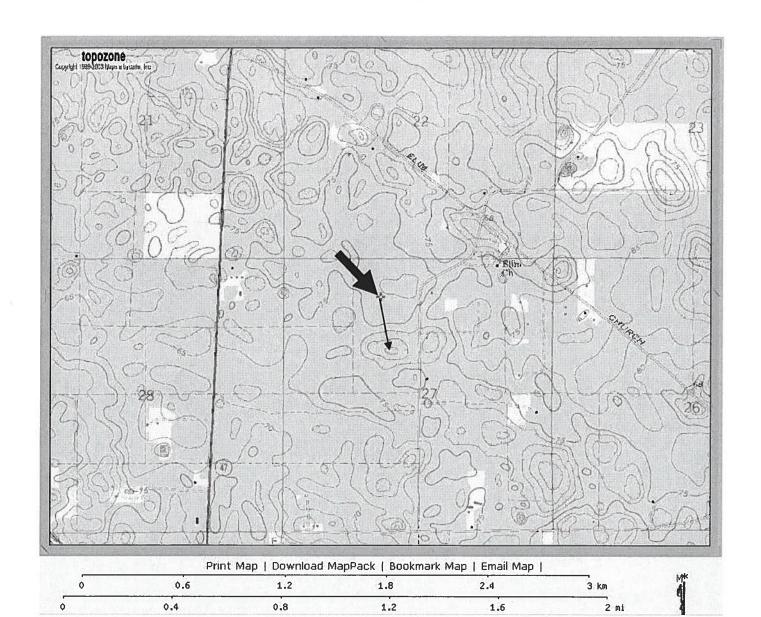
§1804.1.5 Excavations for foundations shall be backfilled with soil which is free of organic material, construction debris and large rocks.

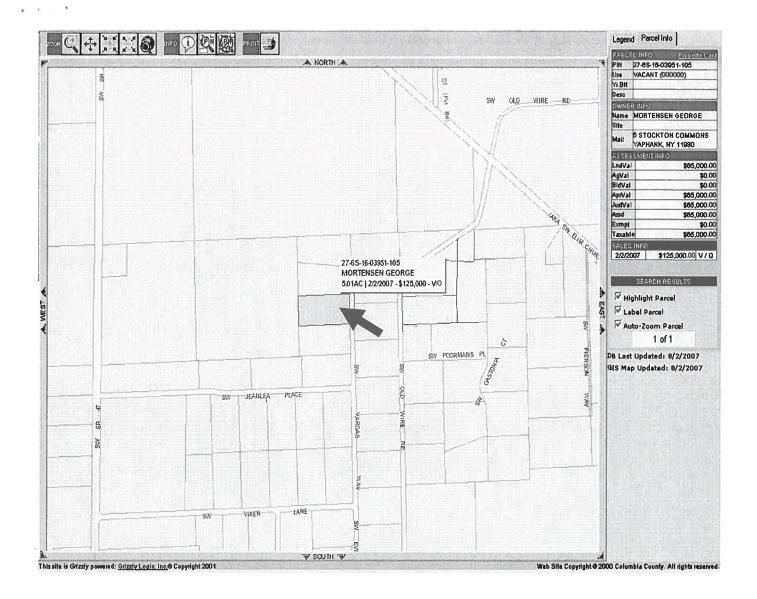
§1804.1.6 Where water impacts the ground from a roof valley, downspout, scupper or other rain water collection or diversion device, provisions shall be made to prevent soil erosion and direct the water away from the foundation.

§1804.1.7 Finish grade shall be sloped away from the foundation for drainage.

§1804.1.8 The area under footings, foundations and concrete slabs on grade shall have all vegetation, stumps, roots and foreign materials removed prior to their construction. Fill material shall be free of vegetation and foreign material.







Prepared by and return to: Carol Fitzgerald Paralegal Jose I. Moreno PA 126 NW 76th Drive Suite B Gainesville, FL 32607 352-332-4422 File Number: 07-0048

Inst:2007063353 Date:02/12/2007 Time:09:35
Doc Stamp Deed: 875.00
DC,P.DeWitt Cason,Columbia County B:1110 P:1320

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this 2nd day of February, 2007 between North Florida Timbertand, Inc. whose post office address is PO Box 1147, Newberry, FL 32669, grantor, and George Mortensen, a single man whose post office address is 5 Stockton Commons, Yaphank, NY 11980, grantee:

(Whenever used herein the terms "grantor" and "grantor" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS (\$10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida to-wit:

Lot 5, Foxwood Phase One (1), as per plat thereof, recorded in Plat Book 8, Page 86-96, of the Public Records of Columbia County, Florida.

Parcel Identification Number: 27-6S-16-03951-105

THIS IS NOT THE HOMESTEAD PROPERTY OF THE GRANTOR OR GRANTEE

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

State of Florida County of Alachua

The foregoing instrument was acknowledged before me this 2nd day of February, 2007 by Ernest Vargas, President of North Florida Timberland, Inc., on behalf of the corporation. He/she [] is personally known to me or [X] has produced a driver's license as identification.

[Notary Seal]

NOTARY PUBLIC-STATE OF FLORIDA

JOSE I. MORENO

Commission # DD584478

Expires: SEP 18, 2010

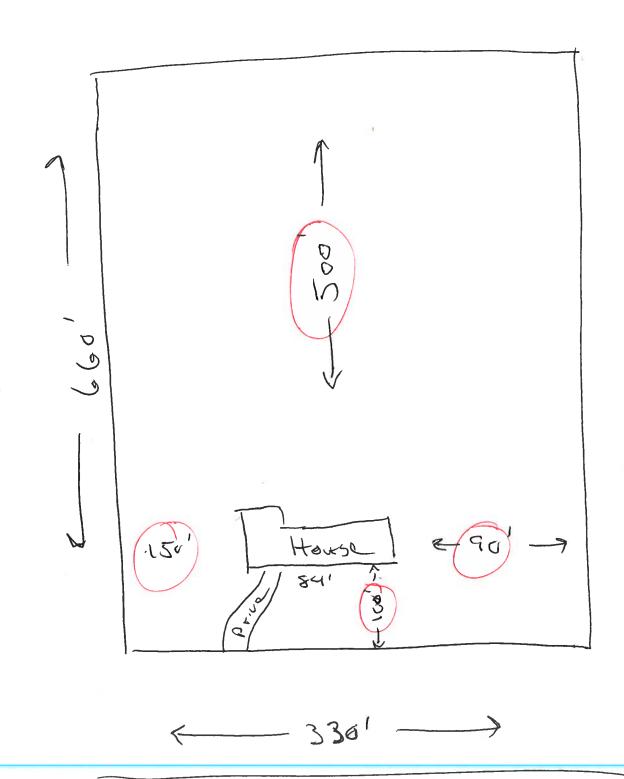
BONDED THRU ALLANTIC BONDING CO., INC.

Notary Public
Printed Name:
My Commission Expires:

DoubleTime=

Site Plan

W



J

Vargg way

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, F1. 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiscountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

6/14/2007

DATE ISSUED:

6/14/2007

ENHANCED 9-1-1 ADDRESS:

250

SW VARGAS

WAY

FORT WHITE

FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

27-6S-16-03951-105

Remarks:

LOT 5 FOXWOOD S/D PHASE 1

Address Issued By:

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

803

Approved Address

JUN 1 4 2007

911Addressing/GIS Dept

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL	REQUIRE	MENTS: Two (2) complete sets of plans containing the following:
Applicant	Plans Exam	
9	0	All drawings must be clear, concise and drawn to scale ("Optional"
		details that are not used shall be marked void or crossed off). Square
		footage of different areas shall be shown on plans.
0/	0	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
		Site Plan including:
		a) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if
		applicable, and all utility easements.
		d) Provide a full legal description of property.
4		Wind-load Engineering Summary, calculations and any details required
		Plans or specifications must state compliance with FBC Section 1609.
		The following information must be shown as per section 1603.1.4 FBC
		a. Basic wind speed (3-second gust), miles per hour (km/hr).
		b. Wind importance factor, Iw, and building classification from Table
		1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.
		c. Wind exposure, if more than one wind exposure is utilized, the
		wind exposure and applicable wind direction shall be indicated.
	•	d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
		e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and
	ē	cladding materials not specifally designed by the registered design professional.
		Elevations including:
0		a) All sides
9	0	b) Roof pitch

c) Overhang dimensions and detail with attic ventilation

a /	В	d) Location, size and height above roof of chimneys.
0 /		e) Location and size of skylights
0/	/ === D	f) Building height
D/		e) Number of stories
	_	Floor Plan including: a) Rooms labeled and dimensioned.
d'	0	a) Rooms labeled and difficultioned. b) Shear walls identified.
0/	/ 🗀	c) Show product approval specification as required by Fla. Statute 553.842 and
	/ "	Fla. Administrative Code 9B-72 (see attach forms).
m/		d) Show safety glazing of glass, where required by code.
	/ "	e) Identify egress windows in bedrooms, and size.
		f) Eireplace (gas vented), (gas non-vented) or wood burning with
V	u	hearth, (Please circle applicable type).
	/ 0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and
		handrails.
0		h) Must show and identify accessibility requirements (accessible bathroom)
		Foundation Plan including:
6		a) Location of all load-bearing wall with required footings indicated as standard
. /		or monolithic and dimensions and reinforcing.
0		b) All posts and/or column footing including size and reinforcingc) Any special support required by soil analysis such as piling
0	0	d) Location of any vertical steel.
0		Roof System:
h/		a) Truss package including:
U	u	1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
		2. Roof assembly (FBC 106.1.1.2) Roofing system, materials,
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
		b) Conventional Framing Layout including:
		1. Rafter size, species and spacing
		2. Attachment to wall and uplift3. Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
		Wall Sections including:
		a) Masonry wall
		1. All materials making up wall
		2. Block size and mortar type with size and spacing of reinforcement
		 3. Lintel, tie-beam sizes and reinforcement 4. Gable ends with rake beams showing reinforcement or gable truss
		4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
		5. All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating)
	2	7. Fire resistant construction (if required)
		8. Fireproofing requirements9. Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
		a. Vapor retarder (6mil. Polyethylene with joints lapped 6
	迎	inches and sealed)
	148	b. Must show control joints, synthetic fiber reinforcement of
		Welded fire fabric reinforcement and supports
		11. Indicate where pressure treated wood will he placed
		12. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity

c. Crawl space (if applicable)

/-		
0	0	b) Wood frame wall
		All materials making up wall
	41	2. Size and species of studs
	6	3. Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		hinge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers)
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
	*	8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
	0	a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
	-	c. Crawl space (if applicable)
		c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
		Engineer of Architect)
0	П	Floor Framing System:
C)		a) Floor truss package including layout and details, signed and sealed by Florida
	0	Registered Professional Engineer
		b) Floor joist size and spacing
		c) Girder size and spacing
		d) Attachment of joist to girder
	0	e) Wind load requirements where applicable
u /		Plumbing Fixture layout
01/		Electrical layout including:
	0	a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
100	0	b) Ceiling fans
	0	c) Smoke detectors
		d) Service panel and sub-panel size and location(s)
		e) Meter location with type of service entrance (overhead or underground)
12	- 0	f) Appliances and HVAC equipment
	0	g) Arc Fault Circuits (AFCI) in bedrooms
		h) Exhaust fans in bathroom
4/		HVAC information
2	0	a) Energy Calculations (dimensions shall match plans)
<i>₽4</i> ∩	· 0	b) Manual J sizing equipment or equivalent computation
0	0	c) Gas System Type (LP or Natural) Location and BTU demand of equipment
5	0	Disclosure Statement for Owner Builders
	0	***Notice Of Commencement Required Before Any Inspections Will Be Done
سل ال		Private Potable Water
		a) Size of pump motor
		b) Size of pressure tank
		c) Cycle stop valve if used

Directions

HT South to Jeanlea (1eft)

Jeanlea to Vargas (1ett)

to property on 1eft.

Pat Lynch LYNCH DRILLING

P. O. BOX 934 Branford, FL 32008-0934 (386) 935-1076

Woodman Park Bld George Montensen 27-65-16-03951-105

DATE: 8- 24.07

4" Water well complete with 4" black water well steel casing, 1HP submersible pump (20 gpm) with 1 1/4" galvanized drop pipe, and 81 gallon captive air tank (21.9 gallon drawdown) (maximum 100 feet included)

Additional footage over 100 feet will be charged at \$8.00 per foot.

Suwannee River Water Management District - well permit

Estimated total package

Well will be complete at the well site. We do not include electrical nor plumbing connections from the well to the home and/or power pole.

Prices on estimates are subject to change, if estimate is over 30 days old, unless specific arrangements are made to extend limit. Estimated depths are available upon request and after review of the specified location.

Note: Columbia County base price SRWMD permit + footage as applicable.

THANK YOU!

Seller shall retain title to the described merchandise until such merchandise has been paid for by the buyer, however, buyer shall have the right to use, display, move, prepare, or otherwise deal with the merchandise solely in connection with the sale of such merchandise to buyers in the ordinary course of business. The merchandise delivered hereby is to be paid for upon delivery and if not paid for within thirty (30) days after receipt, interest and service charges shall accrue at the rate of 1 1/2% per month; this charge is equivalent to an interest rate of 18% per annum from the date of receipt. In the event it shall become necessary for seller to collect the purchase price, or any part thereof, buyer agrees to pay to seller all of the cost of collection including reasonable attorney's fees and all incidental damages suffered by the seller. The buyer shall have five (5) days after receipt to notify seller of any defects or shortages in the merchandise. If buyer has not so notified seller within such five-day period such rights shall have waived and such merchandise shall be deemed to have been received in good condition. Seller warrants that the merchandise is merchantable and free from defects in material and workmanship. Seller makes no other express or implied warranties and does not warrant that the merchandise is fit for any particular purpose. Buyer further agrees that the site of this contract and place for payment is Suwannee County, Florida. The buyer acknowledges acceptance of the above stated items and conditions if this sale by his receipt and retention for five days the merchandise shipped or delivered by the seller.

NOT RESPONSIBLE FOR QUALITY OF WATER

Project Name:

WOODMAN PARK BUILDERS

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

MORTENSEN RESIDENCE

Address: City, State: Owner: MORTENSEN Climate Zone: North	Permitting Office: COLUMBIA COUNTY Permit Number: 26232 Jurisdiction Number: 221000
New construction or existing	12. Cooling systems
2. Single family or multi-family Single family	a. Central Unit Cap: 36.0 kBtu/hr
3. Number of units, if multi-family 1	SEER: 13.00
4. Number of Bedrooms 4	b. Central Unit Cap: 18.0 kBtu/hr
5. Is this a worst case? No	SEER: 13.00
6. Conditioned floor area (ft²) 2342 ft²	c. N/A
7. Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default)	_
a. U-factor: Description Area	13. Heating systems
(or Single or Double DEFAULT) 7a. (Dble, U=0.9) 42.0 ft ² b. SHGC:	a. Electric Heat Pump Cap: 36.0 kBtu/hr HSPF: 8.20
(or Clear or Tint DEFAULT) 7b. (Clear) 239.0 ft ²	b. Electric Heat Pump Cap: 18.0 kBtu/hr
8. Floor types	HSPF: 8.50
a. Slab-On-Grade Edge Insulation R=0.0, 183.0(p) ft	c. N/A
b. N/A	
c. N/A	14. Hot water systems
9. Wall types	a. Electric Resistance Cap: 50.0 gallons
a. Frame, Wood, Exterior R=13.0, 1610.5 ft ²	EF: 0.93
b. Frame, Wood, Adjacent R=0.0, 905.0 ft ²	b. N/A
c. Frame, Wood, Adjacent R=13.0, 369.0 ft ²	
d. N/A	c. Conservation credits
e. N/A	(HR-Heat recovery, Solar
10. Ceiling types	DHP-Dedicated heat pump)
a. Under Attic R=30.0, 2342.0 ft ²	15. HVAC credits
b. N/A	(CF-Ceiling fan, CV-Cross ventilation,
c. N/A	HF-Whole house fan,
11. Ducts(Leak Free)	PT-Programmable Thermostat,
a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 240.0 ft	MZ-C-Multizone cooling,
b. Sup: Unc. Ret: Unc. AH: Attic Sup. R=6.0, 140.0 ft	MZ-H-Multizone heating)
	
Glass/Floor Area: 0.10 Total as-built p	oints: 32655 points: 34838
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.	Review of the plans and specifications covered by this calculation indicates compliance

PREPARED BY: Sarry Kesmondo a/C with the Florida Energy Code. Before construction is completed DATE: / lu this building will be inspected for compliance with Section 553.908 I hereby certify that this building, as designed, is in compliance Florida Statutes. with the Florida Energy Code. **BUILDING OFFICIAL:** OWNER/AGENT: ___ DATE: DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,, PERMIT#:

	BASE					AS-	BU	ILT				
GLASS TYPES .18 X Conditio Floor Ar	ned X BS	SPM = P	oints	Type/SC	Ov Ornt	erhang Len	Hgt	Area X	SPI	иx	SOF	= Points
.18 2342	0	18.59	7837.0	1.Double,U=0.87,Clear	E	5.0						
.10 2542	.0	10.55	1031.0	2.Double,U=0.87,Clear	E	5.U 7.5	6.0 8.0	40.0 14.0	42.0 42.0		0.56	942.0
				3.Double,U=0.87,Clear	SE	7.5 7.0	6.0			-	0.53	314.0
2				4.Double,U=0.87,Clear	SE	7.0 1.5	5.0	20.0 4.0	42.7		0.46	396.0
				5.Double,U=0.87,Clear	NE	7.0	6.0	20.0	42.7 29.5		0.83	142.0
				6.Double,U=0.87,Clear	N	7.5	6.0	40.0	19.2	-	0.55	325.0
				7.Double,U=0.87,Clear	W	7.5 7.5	4.0			-	0.68	519.0
				8.Double,U=0.87,Clear	W	7.5 7.5	8.0	9.0	38.5		0.42	143.0
				9.Double,U=0.87,Clear	W	7.5 7.5		14.0	38.5		0.55	294.0
							7.0	42.0	38.5		0.51	830.0
				10.Double,U=0.87,Clear	W	1.5	6.0	20.0	38.5		0.91	703.0
				11.Double,U=0.87,Clear	S	1.5	5.0	12.0	35.8		0.81	347.0
				12.Double,U=0.87,Clear	SW	1.5	5.0	4.0	40.1	ь	0.84	134.0
				As-Built Total:				239.0				5089.0
WALL TYPES	Area X	BSPM =	= Points	Туре		R-\	/alue	e Area	X	SPN	1 =	Points
Adjacent	1274.0	0.70	891.8	1. Frame, Wood, Exterior		1	3.0	1610.5		1.50		2415.8
Exterior	1610.5	1.70	2737.9	2. Frame, Wood, Adjacent			0.0	905.0		2.20		1991.0
				3. Frame, Wood, Adjacent		1	3.0	369.0		0.60		221.4
Base Total:	2884.5		3629.7	As-Built Total:				2884.5				4628.1
DOOR TYPES	Area X	BSPM =	= Points	Туре				Area	X	SPN	1 =	Points
Adjacent	21.0	2.40	50.4	1.Exterior Wood				21.0		6.10		128.1
Exterior	21.0	6.10	128.1	2.Adjacent Wood				21.0		2.40		50.4
				•								55.1
Base Total:	42.0		178.5	As-Built Total:				42.0				178.5
CEILING TYPES	Area X	BSPM =	Points	Туре		R-Value	e /	Area X	SPM	x sc	:M =	Points
Under Attic	2342.0	1.73	4051.7	1. Under Attic		3	0.0	2342.0	1.73 X	1.00	-	4051.7
Base Total:	2342.0		4051.7	As-Built Total:				2342.0				4051.7
FLOOR TYPES	Area X	BSPM =	Points	Туре		R-\	/alue	Area	Х	SPM	=	Points
Slab 1 Raised	183.0(p) 0.0	-37.0 0.00	-6771.0 0.0	1. Slab-On-Grade Edge Insula	ation		0.0	183.0(p	-4	1.20	,	-7539.6
Base Total:		0.00	-677 1.0	As-Built Total:				183.0				-7539.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,,	PERMIT #:

BASE	AS-BUILT						
INFILTRATION Area X BSPM = Points	Area X SPM = Points						
2342.0 10.21 23911.8	2342.0 10.21 23911.8						
Summer Base Points: 32837.6	Summer As-Built Points: 30319.5						
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)						
	(sys 1: Central Unit 36000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS) 30320						
32837.6 0.3250 10672.2	30319.5 1.00 1.134 0.260 1.000 8940.8						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,, PERMIT #:

	BASE			1		AS-	BUI	LT				
GLASS TYPES .18 X Condition Floor Ar		NPM = F	Points	Type/SC	Ove Ornt	rhang Len	Hgt	Area X	WP	мх	WOF	= Points
.18 2342.	0	20.17	8503.0	1.Double,U=0.87,Clear	E	5.0	6.0	40.0	18.7	9	1.24	931.0
				2.Double,U=0.87,Clear	E	7.5	8.0	14.0	18.7	9	1.27	333.0
				3.Double,U=0.87,Clear	SE	7.0	6.0	20.0	14.7	1	2.09	614.0
				4.Double,U=0.87,Clear	SE	1.5	5.0	4.0	14.7	1	1.14	67.0
				5.Double,U=0.87,Clear	NE	7.0	6.0	20.0	23.5	7	1.05	494.0
				6.Double,U=0.87,Clear	N	7.5	6.0	40.0	24.5	8	1.02	1003.0
				7.Double,U=0.87,Clear	W	7.5	4.0	9.0	20.7	3	1.22	227.0
				8.Double,U=0.87,Clear	W	7.5	8.0	14.0	20.7	3	1.16	336.0
				9.Double,U=0.87,Clear	W	7.5	7.0	42.0	20.7	3	1.17	1022.0
				10.Double,U=0.87,Clear	W	1.5	6.0	20.0	20.7	3	1.02	424.0
			12	11.Double,U=0.87,Clear	s	1.5	5.0	12.0	13.3	0	1.20	191.0
				12.Double,U=0.87,Clear	SW	1.5	5.0	4.0	16.7	4	1.09	73.0
				As-Built Total:				239.0				5715.0
WALL TYPES	Area X	BWPM :	= Points	Туре		R-	Value	Area	Х	WPM	1 =	Points
Adjacent	1274.0	3.60	4586.4	1. Frame, Wood, Exterior			13.0	1610.5		3.40		5475.7
Exterior	1610.5	3.70	5958.9	2. Frame, Wood, Adjacent			0.0	905.0		10.40		9412.0
				3. Frame, Wood, Adjacent			13.0	369.0		3.30		1217.7
Base Total:	2884.5		10545.3	As-Built Total:		0		2884.5				16105.4
DOOR TYPES	Area X	BWPM =	= Points	Туре	-			Area	Х	WPN	1 =	Points
Adjacent	21.0	11.50	241.5	1.Exterior Wood				21.0		12.30		258.3
Exterior	21.0	12.30	258.3	2.Adjacent Wood				21.0		11.50		241.5
Base Total:	42.0		499.8	As-Built Total:				42.0				499.8
CEILING TYPES	Area X	BWPM =	= Points	Туре	R	-Value	e Ar	ea X W	PM 2	x wc	:M =	Points
Under Attic	2342.0	2.05	4801.1	1. Under Attic			30.0	2342.0	2.05 X	1.00		4801.1
Base Total:	2342.0		4801.1	As-Built Total:				2342.0				4801.1
FLOOR TYPES	Area X	BWPM =	= Points	Туре		R-	Value	Area	Х	WPN	l =	Points
Slab	183.0(p)	8.9	1628.7	1. Slab-On-Grade Edge Insu	ılation		0.0	183.0(p		18.80		3440.4
Raised	0.0	0.00	0.0					•				
Base Total:			1628.7	As-Built Total:				183.0				3440.4

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

	· · · · · · · · · · · · · · · · · · ·
ADDRESS: ,,,	PERMIT #:

BASE	AS-BUILT					
INFILTRATION Area X BWPM = Poin	Area X WPM = Points					
2342.0 -0.59 -1381	8 2342.0 -0.59 -1381.8					
Winter Base Points: 24596.	1 Winter As-Built Points: 29179.9					
Total Winter X System = Heating Points Multiplier Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)					
	(sys 1: Electric Heat Pump 36000 btuh ,EFF(8.2) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 29179.9 0.667 (1.069 x 1.000 x 1.00) 0.416 1.000 8647.9 (sys 2: Electric Heat Pump 18000 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Att(AH),R6.0					
24596.1 0.5540 13626.	29179.9 0.333 (1.069 x 1.000 x 1.10) 0.401 1.000 4588.5 29179.9 1.00 1.108 0.411 1.000 13287.7					

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT									
WATER HEA Number of Bedrooms	TING X	i Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	x	Tank X Ratio	Multiplier	X Credit Multiplie		tal
4		2635.00		10540.0	50.0	0.93	4		1.00	2606.67	1.00	1042 1042	

	CODE COMPLIANCE STATUS												
BASE							AS	-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
10672	E	13626		10540		34838	8941		13288		10427		32655

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal	
		efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.6

The higher the score, the more efficient the home.

MORTENSEN, , , ,

1.	New construction or existing	New	12	. Cooling systems		
1. 2.	Single family or multi-family	Single family	_	a. Central Unit	Cap: 36.0 kBtu/hr	
2. 3.	Number of units, if multi-family	Single family	_ '	a. Central Onit	SEER: 13.00	-
	Number of Bedrooms	1	_	b. Central Unit	Cap: 18.0 kBtu/hr	
4. 5.	Is this a worst case?	No.	_	b. Central Offit	SEER: 13.00	
		2342 ft ²	_	- N/A	SEEK. 13.00	-
6.	Conditioned floor area (ft²)		_ '	c. N/A		-
7.	Glass type 1 and area: (Label reqd. I	•		** .*		-
a.	U-factor:	Description Area		Heating systems	0 0(010, 0	
	(or Single or Double DEFAULT)	7a. (Dble, U=0.9) 42.0 ft ²		a. Electric Heat Pump	Cap: 36.0 kBtu/hr	-
b.	SHGC:				HSPF: 8.20	-
	(or Clear or Tint DEFAULT)	7b. (Clear) 239.0 ft^2	_ '	b. Electric Heat Pump	Cap: 18.0 kBtu/hr	_
8.	Floor types				HSPF: 8.50	_
a.	Slab-On-Grade Edge Insulation	R=0.0, 183.0(p) ft	_ '	c. N/A		_
b.	N/A					_
c.	N/A		14	. Hot water systems		
9.	Wall types		;	a. Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Exterior	R=13.0, 1610.5 ft ²	-		EF: 0.93	_
b.	Frame, Wood, Adjacent	R=0.0, 905.0 ft ²	_	b. N/A		
c.	Frame, Wood, Adjacent	R=13.0, 369.0 ft ²	_			
d.	N/A		_ ,	c. Conservation credits		_
e.	N/A		_	(HR-Heat recovery, Solar		
10.	Ceiling types			DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 2342.0 ft ²	15	. HVAC credits		_
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A			HF-Whole house fan,		
11.	Ducts(Leak Free)		_	PT-Programmable Thermostat,		
	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 240.0 ft		MZ-C-Multizone cooling,	8	
	Sup: Unc. Ret: Unc. AH: Attic	Sup. R=6.0, 140.0 ft		MZ-H-Multizone heating)		
0.	345. G.G. 744. G.G. 711	54p. 12 6.5, 2 6.5 6	_			
Con in th	rtify that this home has complic struction through the above end his home before final inspection and on installed Code compliant	ergy saving features which a. Otherwise, a new EPL I	h will be i	nstalled (or exceeded)	OF THE STATE	ROIS!
	Ider Signature:		Date:			151
Dul.	idei signature.		Date			

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTMdesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

Address of New Home:

RRODUCTAPPROVAL SPECIFICATION SHEET

	F G D Bross Burne G
cation: Project Name:	
cation: Project Name:	
CONTROL OF THE PROPERTY OF THE	

As required by Florida Statute 653.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number (s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory 💖	Manufacturer	Product Description	Approval Number(s
A. EXTERIOR DOORS		· Batta and the second of the	FL. 4242.1
1. Swinging	THE RESERVE THE PROPERTY OF THE PARTY OF THE	Haracon Colonia Charles Colonia Colonia	
2. Sliding	Free Har.		
3. Sectional	ing for the constant of the con-	o segui i i i i i i i i i i i i i i i i i i	
4. Roll up 5. Automatic			
6. Other	and value as size of the second		
B. WINDOWS		N 19-17 COV ASSESSMENT CONTRACTOR	
1. Single hung	HIGH.	IIII / F1214.10	
2. Horizontal Slider			FL. 6029.7
3. Casement ::	(4) Art Comment Street	of a constraint with the supplier of the second	
3. Casement	WKIH-Bost WI	Han & A0015	
4. Double Hung 5. Fixed	The state of the s	HANDER BUT A WEST THE CONTROL OF THE	
6. Awning			
7. Pass through	The state of the s		
8. Projected	, y principalista en periodo de la periodo d		
9. Mullion	Marketing		
10. Wind Breaker	Marketine		
11 Dual Action			
12. Other		Cay and the Control of the Control o	
THE RESERVE OF THE PARTY OF THE		Addition Section 1 and 1	
		Service Control of the Control of th	
		Marine to a settle-ing state of a	FL. 889 -122
The state of the s	W. C.		10.001 700
4. Storefronts	Mark the second		
E CONTROL DE LA	Section of the sectio		
6. Wali louver	is in the second		
7. Glass block	30	· 图 17 Sept. 12 Sept. 19 Sept	A DESTRUCTION OF THE PROPERTY
8. Membrane	Section 1985	The state of the s	
9. Greenhouse	Authorite Control of the Control of	THE BOOK OF THE PARTY WAS AND THE PARTY OF T	
THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.	The state of the s	III TALE TO LETTE THE CHARLEST THE CONTROL OF THE C	
	A	British and the second	Shirdica Itie
ROOFING PRODUCTS		MAR Sunder Revention of the second	728.4, 728.5
1. Asphalt Shingles	GIV.	Shinales	120.11/20.5
2. Underlayments	Will Wiese of Apple 2 de la	304€	FL. 18/4.3
. Roofing Fasteners			7 FL. 1814.1
Non-structural Metal Rf	Wheeling Chy	rug st 150 CQ. D'enty Branc	
	到于1 000年的第三人称单位的	Market Service	F1.5190
. Modified Bitumen	distribution of the second	e dan tenga bahan pagan da sakan pangan sa angah ana	
Single Ply Roofing Sys &	A STATE OF THE STA		
. Roofing Tiles	The state of the s	NGS COURT PER SET SERVICE CONTROL OF THE COURT	
. Roofing Insulation		C TOTAL CONTRACTOR OF THE CONT	
. Waterproofing	Shirt Company	responsibilities are partitionally the second of the control of the second of the seco	AND CANADA
. Wood shingles /shakes		The state of the s	8-551/-1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1
Roofing State	\$10 7.5 Personal Control	CONTROL OF THE PROPERTY OF THE	Q11_224V



Project Information for:

L250968

Builder:

Woodman Park Builders, Inc.

Address:

250 Southwest Vargas

...

Ft. White, FL 32038

County:

Columbia

Truss Count:

50

Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002
Truss Design Load Information:

Gravity:

Roof (psf): 42.0 Floor (psf): N/A

Wind Standard: ASCE 7-02

One and (multi) 440

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

William G. Wood Florida Certified Building Contractor License No. Case

Address: Woodman Bork Builders, Inc., B.O. Bay 2525, July 2525, July 2525

Wind Exposure: B

Address: Woodman Park Builders, Inc. P.O. Box 3535 Lake City, Florida 32026

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

25

26

27

28

J1884260

J1884261

J1884262

J1884263 | T14A

| T12

T13

T14

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg.#	Truss ID	Date	No.	Drwg. #	Truss ID	Date
1	J1884236	CJ1	8/24/07	29	J1884264	T15	8/24/07
2	J1884237	CJ3	8/24/07	30	J1884265	T16	8/24/07
3	J1884238	CJ3A	8/24/07	31	J1884266	T17	8/24/07
4	J1884239	CJ5	8/24/07	32	J1884267	T18	8/24/07
5	J1884240	CJ5A	8/24/07	33	J1884268	T19	8/24/07
6	J1884241	CJ7	8/24/07	34	J1884269	T20	8/24/07
7	J1884242	EJ7	8/24/07	35	J1884270	T21	8/24/07
8	J1884243	EJ7A	8/24/07	36	J1884271	T22	8/24/07
9	J1884244	EJ8	8/24/07	37	J1884272	T23	8/24/07
10	J1884245	HJ11	8/24/07	38	J1884273	T25	8/24/07
11	J1884246	HJ9	8/24/07	39	J1884274	T26	8/24/07
12	J1884247	HJ9A	8/24/07	40	J1884275	T27	8/24/07
13	J1884248	PB01	8/24/07	41	J1884276	T28	8/24/07
14	J1884249	PB02	8/24/07	42	J1884277	T29	8/24/07
15	J1884250	T02	8/24/07	43	J1884278	T30	8/24/07
16	J1884251	T03	8/24/07	44	J1884279	T31	8/24/07
17	J1884252	T04	8/24/07	45	J1884280	T32	8/24/07
18	J1884253	T05	8/24/07	46	J1884281	T35	8/24/07
19	J1884254	T06	8/24/07	47	J1884282	T35A	8/24/07
20	J1884255	T07	8/24/07	48	J1884283	T35G	8/24/07
21	J1884256	T08	8/24/07	49	J1884284	T36	8/24/07
22	J1884257	T09	8/24/07	50	J1884285	T38	8/24/07
23	J1884258	T10	8/24/07				
24	J1884259	T11	8/24/07				
				-			

8/24/07

8/24/07

8/24/07

8/24/07

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	CJ1	JACK	2	1		J1884236
	001	UNOIN .		'	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:11 2007 Page 1

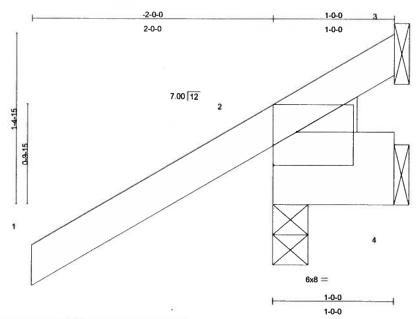


Plate Of	fsets (X,Y): [2:Edge,0-3-5]				·						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.00	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 9 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP No.1D

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=95(load case 6)

Max Uplift 2=-251(load case 6), 3=-90(load case 1)

Max Grav 2=256(load case 1), 4=14(load case 2), 3=104(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/41, 2-3=-99/85

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.07 and 2 = 0.00

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) All hearings assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Les Truss Coston Engineer Horida Ps. No., 2-1869 1199 Casstel Bay Sivri Coynton Gasch, FL 55466

August 24,2007

Scale = 1:9.2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	CJ1	JACK	2	1	J1884236
	001	J. C.			Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:11 2007 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard



August 24,2007



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	CJ3	JACK	2	1		J1884237
			2000-2200-000		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:11 2007 Page 1

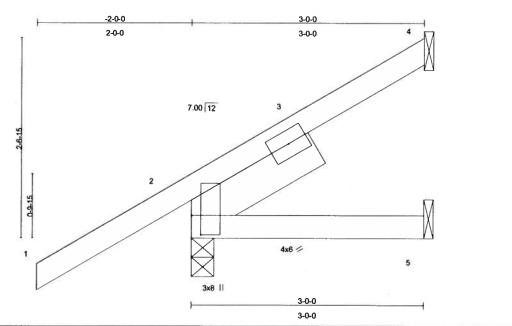


Plate Off	sets (X,Y	(): [2:0-3-0,0-1-7]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates increase	1.25	TC	0.21	Vert(LL)	-0.00	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.06	Vert(TL)	-0.01	2-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 18 lb	

L	JM	В	E	R
---	----	---	---	---

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

SLIDER

Left 2 X 6 SYP No.1D 1-10-1

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=43/Mechanical, 2=239/0-3-8, 5=15/Mechanical

Max Horz 2=145(load case 6)

Max Uplift 4=-51(load case 6), 2=-163(load case 6)

Max Grav 4=43(load case 1), 2=239(load case 1), 5=44(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-80/7, 3-4=-32/13

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.14, 2 = 0.06 and 3 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2



August 24,2007

Scale = 1:14.3

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connect
-Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent broncing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erectic
and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
8300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onorito Drive, Madison, WI 5370



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	CJ3	JACK	2	1	J1884237
220000	000	o lo		'	Job Reference (optional)

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4 and 163 lb uplift at joint 2.

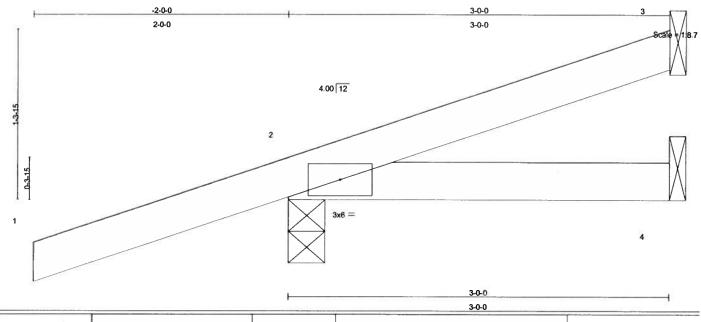
LOAD CASE(S) Standard

August 24,2007



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	CJ3A	JACK	12	1		J1884238
				<u> </u>	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:12 2007 Page 1



		1		1								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)	, ,					Weight: 12 lb	
		1										

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=88(load case 4)

Max Uplift 3=-23(load case 7), 2=-242(load case 4), 4=-27(load case 4) Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/33, 2-3=-41/5

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

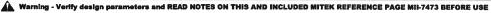
2 = 0.10

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 3, 242 lb uplift at joint 2 and 27 lb uplift at joint 4. Continued on page 2

Julius Lee Tues Cesian Engineer Plonas PE No. 24809 1499 Çaskiel Bay Biva Bovnton Besch, PL 86466

August 24,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-11 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	1
L250968	CJ3A	JACK	12	1	J1884238	
		JAON	12	'	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:12 2007 Page 2

LOAD CASE(S) Standard

August 24,2007



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	CJ5	JACK	2	4		J1884239
L250900	CJS	JACK		'	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:12 2007 Page 1

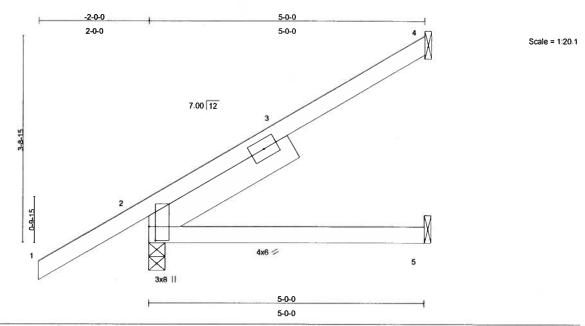


Plate Of	fsets (X,Y	<u>'): [2:0-3-0,0-1-7]</u>										
LOADIN	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.03	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.17	Vert(TL)	-0.05	2-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

SLIDER

Left 2 X 6 SYP No.1D 2-11-15

BRACING

TOP CHORD

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

Structural wood sheathing directly applied or

bracing.

REACTIONS (lb/size) 4=111/Mechanical, 2=288/0-3-8, 5=25/Mechanical

Max Horz 2=199(load case 6)

Max Uplift 4=-121(load case 6), 2=-162(load case 6)

Max Grav 4=111(load case 1), 2=288(load case 1), 5=74(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-97/8, 3-4=-77/42

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.16, 2 = 0.07 and 3 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truss Cesign Engineer Fibride PE No. 341898 1406 Castal Ray Blod Boynton Wesch, PL bo+56

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI/TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCS-I or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	CJ5	JACK				J1884239
L250966	CJ5	JACK	2	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:12 2007 Page 2

NOTES

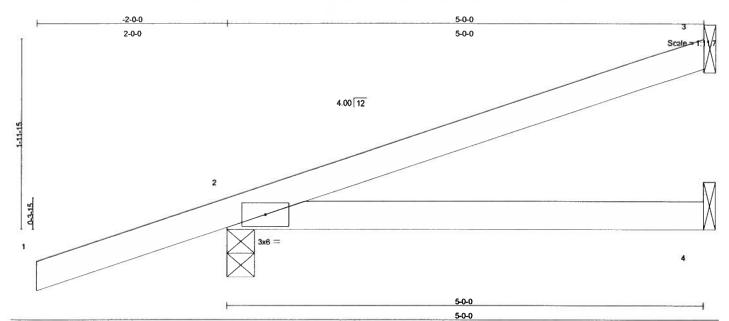
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 4 and 162 lb uplift at joint 2.

LOAD CASE(S) Standard

August 24,2007



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK			
						J1884240		
L250968	CJ5A	JACK	6	1				
					Job Reference (optional)			
Builders FirstSource Lake City FL 32055			6.300 s Feb 15 2006 MiTek Industries, Inc., Fri Aug 24 09:39:13 2007, Page 1					



LOADIN TCLL TCDL	20.0 7.0	SPACING Plates Increase Lumber Increase	2-0-0 1.25 1.25	CSI TC BC	0.26 0.24	DEFL Vert(LL) Vert(TL)	in 0.09 -0.05	(loc) 2-4 2-4	I/defl >663 >999	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	BC2004/TPI2002 (Matrix)		rix)						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING**

TOP CHORD Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=118(load case 4)

Max Uplift 3=-74(load case 4), 2=-273(load case 4), 4=-46(load case 4)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-55/26

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 3, 273 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	CJ5A	JACK	_		J1884240
L230900	C33A	JACK	0	'	Job Reference (optional)

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LOAD CASE(S) Standard



August 24,2007



Job Truss Truss Type Qty Ply **WOODMAN PARK** J1884241 L250968 CJ7 **JACK** 1 Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 24 12:04:52 2007 Page 1 2-0-0 7-0-0 4.00 12 0-3-15 7-0-0 7-0-0 CSI LOADING (psf) **SPACING** 2-0-0 **DEFL** in (loc) I/defl L∕d **PLATES GRIP TCLL** 20.0 1.25 Plates Increase TC 0.45 Vert(LL) 0.31 2-4 >263 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.44 Vert(TL) -0.16>508 240 **BCLL** 10.0 **WB** 0.00 Rep Stress Incr YES Horz(TL) -0.00 3 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 25 lb **LUMBER BRACING** TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 BOT CHORD 2 X 4 SYP No.2 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=155/Mechanical, 2=352/0-3-8, 4=44/Mechanical

Max Horz 2=107(load case 4)

Max Uplift 3=-82(load case 4), 2=-239(load case 4), 4=-63(load case 5) Max Grav 3=155(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-84/39

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.47

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 3, 239 lb uplift at joint 2 and 63 lb uplift at joint 4.

LOAD CASE(S) Standard



August 24,2007



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPT1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job Truss Truss Type Qty Ply WOODMAN PARK
L250968 EJ7 MONO TRUSS 9 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:14 2007 Page 1

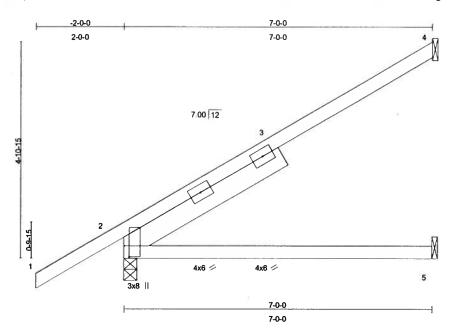


Plate Of	fsets (X,Y	'): [2:0-3-0,0-1-7], [2:	0-0-0,0-0-	0], [2:0-	0-0,0-0-0)]						
LOADIN	.,	SPACING	2-0-0	CSI	0.46	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.08	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.13	2-5	>621	240	}	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 36 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

SLIDER

Left 2 X 6 SYP No.1D 4-1-13

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=165/Mechanical, 2=346/0-3-8, 5=42/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 4=-103(load case 6), 2=-111(load case 6)

Max Grav 4=165(load case 1), 2=346(load case 1), 5=87(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-124/4, 3-4=-101/63

BOT CHORD 2-5=0/0

JOINT STRESS INDEX

2 = 0.82, 2 = 0.04, 2 = 0.04 and 3 = 0.00

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 Colhi HDER shiping and 111 lb uplift at joint 2.

Truse Cesion Engineer Plofide PE No. 3-1829 1 100 Castal Bay Slvd Coynton Cesch. PL 60406

August 24,2007

Scale = 1:25.2

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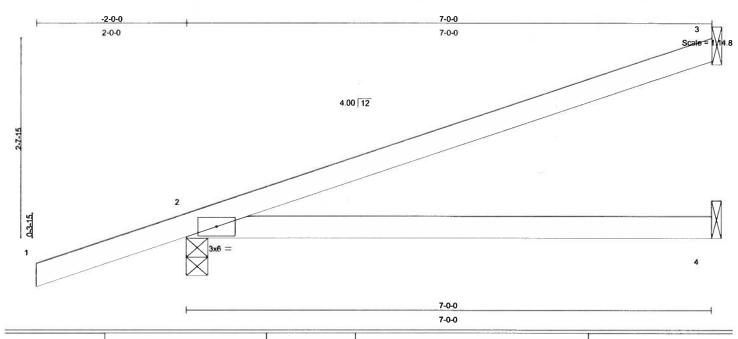
Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884242
L250968	EJ7	MONO TRUSS	9	1	
					Job Reference (optional)
Builders FirstSourc	Builders FirstSource, Lake City, FI 32055			MiTek	Industries, Inc. Fri Aug 24 09:39:14 2007 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1.050000	C 17A	MONO TRUES			J1884243
L250968	EJ7A	MONO TRUSS	1	1	Lab Data and William 6
					Job Reference (optional)

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LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.31	2-4	>263	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.16	2-4	>508	240		
BCLL 10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TI	PI2002	(Mati	rix)						Weight: 25 lb)

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=155/Mechanical, 2=352/0-3-8, 4=44/Mechanical

Max Horz 2=107(load case 4)

Max Uplift 3=-82(load case 4), 2=-239(load case 4), 4=-63(load case 5) Max Grav 3=155(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-84/39

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

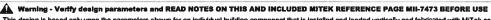
2 = 0.47

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); n=zuīt; ICDL=4.zpsi, DODE=0.opoi, State of the control 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 3, 239 lb uplift at joint 2 and 63 lb uplift at joint 4.

LOAD CASE(S) Standard

August 24,2007



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August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

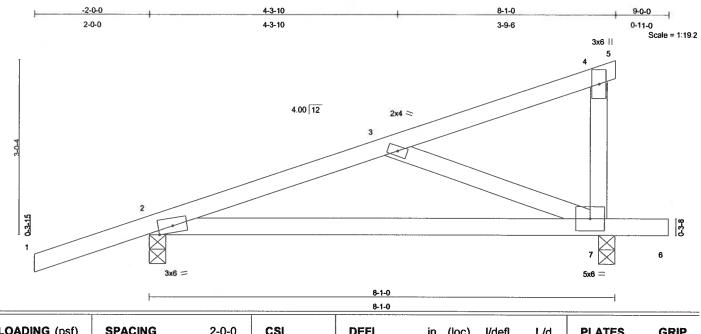
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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	EJ8	MONO TRUSS	16	1		J1884244
	200	Mone mode	,0	•	Job Reference (optional)	

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L	U	M	В	E	R
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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 8-4-14 oc

bracing.

REACTIONS (lb/size) 7=246/0-3-8, 2=376/0-3-8

Max Horz 2=119(load case 4)

Max Uplift 7=-162(load case 4), 2=-253(load case 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-368/386, 3-4=-64/87, 4-5=-1/0, 4-7=-91/72

BOT CHORD 2-7=-491/312, 6-7=0/0

WEBS 3-7=-302/401

JOINT STRESS INDEX

2 = 0.77, 3 = 0.19, 4 = 0.28 and 7 = 0.77

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 7 and 253 lb uplift at joint 2. Continued on page 2

Julius Les Trues Cesign Engineer Morida Ett No. 3-1889 1400 Cossell Bay Blyd Soynton Besch, FL 66466

August 24,2007

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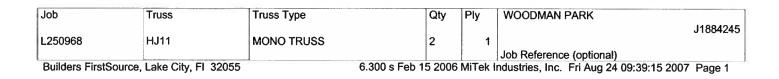
Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
1.250069	EJ8	MONO TRUCE	16			J1884244
L250968	EJ8	MONO TRUSS	16	1		
					Job Reference (optional)	

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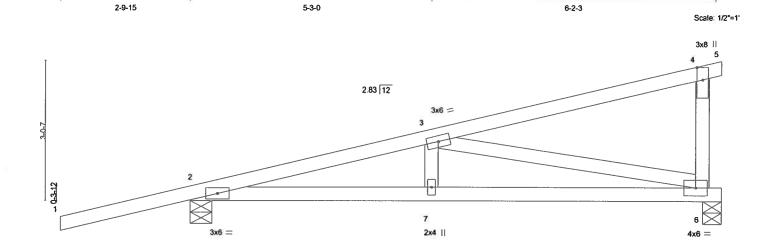
LOAD CASE(S) Standard

Julius Les Truss Ossian Engineer Plonds PE No. 3-1869 1-100 Cassial Ray Sivil Bovnton Beach, PL 85486





5-3-0



		530									0-1-8	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.08	`6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.46	Vert(TL)	-0.10	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	` ′					Weight: 53 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

-2-9-15

WEBS

2 X 4 SYP No.3 *Except*

4-6 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

5-9-1 oc purlins, except end verticals.

11-5-3

Rigid ceiling directly applied or 6-7-4 oc

bracing.

REACTIONS (lb/size) 6=699/0-4-15, 2=525/0-5-11

Max Horz 2=228(load case 3)

Max Uplift 6=-522(load case 3), 2=-469(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1072/670, 3-4=-228/42, 4-5=-15/0, 4-6=-416/321 **BOT CHORD**

2-7=-820/1027, 6-7=-820/1027

WEBS 3-7=-69/179, 3-6=-883/714

JOINT STRESS INDEX

2 = 0.69, 3 = 0.29, 4 = 0.73, 6 = 0.47 and 7 = 0.13

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 522 Ib uplift at joint 6 and 469 lb uplift at joint 2. Continued on page 2

August 24,2007

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J188	4245
L250968	HJ11	MONO TRUSS	2	1		
					Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-150(F=-48, B=-48), 4=-150(F=-48, B=-48)-to-5=-156(F=-51, B=-51), 2=-0(F=5,

B=5)-to-6=-29(F=-9, B=-9)



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	HJ9	MONO TRUSS			J18	84246
L250900	пја	MONO TRUSS	1	1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:15 2007 Page 1

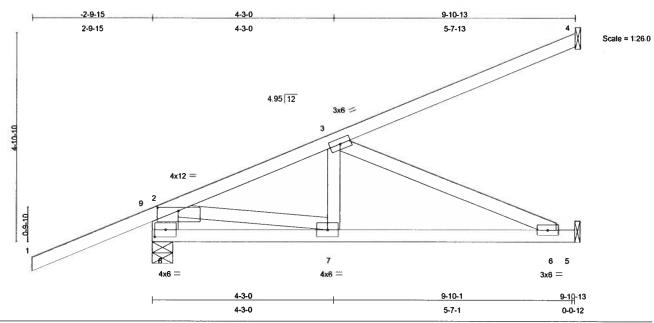


Plate Offsets (X,Y):	[2:0-5-14,0-0-14], [8:0-3-0,0-2-1]

LOADIN	.,	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.04	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.11	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.28	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)						Weight: 53 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

2-8 2 X 8 SYP No.1D

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 8=446/0-5-11, 4=262/Mechanical, 5=221/Mechanical

Max Horz 8=317(load case 5)

Max Uplift 8=-238(load case 5), 4=-243(load case 5), 5=-90(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-8=-441/228, 1-9=0/58, 2-9=0/58, 2-3=-456/53, 3-4=-124/72

BOT CHORD 7-8=-386/215, 6-7=-295/427, 5-6=0/0

WEBS 2-7=-516/823, 3-7=-51/143, 3-6=-465/322

JOINT STRESS INDEX

2 = 0.95, 3 = 0.14, 6 = 0.13, 7 = 0.33 and 8 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 Colline to the colline of the collin

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884246
L250968	HJ9	MONO TRUSS	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:15 2007 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

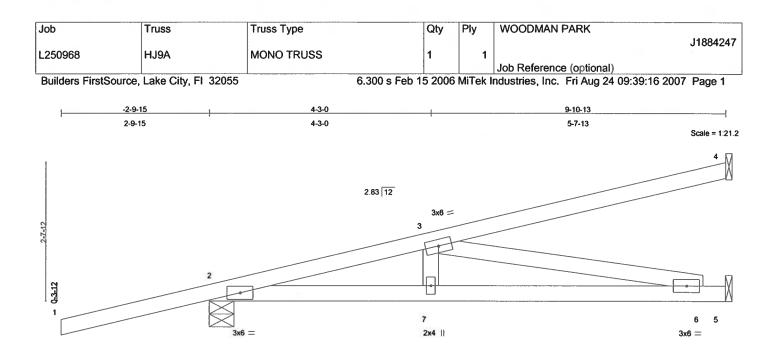
Vert: 1-9=-54

Trapezoidal Loads (plf)

Vert: 9=0(F=27, B=27)-to-2=-4(F=25, B=25), 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 8=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee Truss Desian Chainear Plonds PE No. 34869 1100 Cassell Bay Blvd Boynton Besch, FL 88466





		1		4-3-0						9-10-1		9-10-13
				4-3-0		-				5-7-1		0-0-12
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	0.12	6-7	>956	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.44	Vert(TL)	-0.14	6-7	>839	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 43 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-0-8 oc

bracing.

REACTIONS (lb/size) 4=271/Mechanical, 2=456/0-5-11, 5=215/Mechanical

Max Horz 2=178(load case 3)

Max Uplift 4=-210(load case 3), 2=-431(load case 3), 5=-166(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-891/600, 3-4=-67/46 2-7=-707/858, 6-7=-707/858, 5-6=0/0

BOT CHORD

3-7=-90/184, 3-6=-874/720

JOINT STRESS INDEX

2 = 0.73, 3 = 0.29, 6 = 0.24 and 7 = 0.13

NOTES

WEBS

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 4, 431 lb uplift at joint 2 and 166 lb uplift at joint 5.

Julius Les Truss Cesion Engineer Plonde Pt. No. 34869 1196 Cassid Bay Blvd Goynton Seach, FL 88465

August 24,2007

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI + as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Orive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	HJ9A	MONO TRUSS	1	1		J1884247
			'		Job Reference (optional)	

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)



WOODMAN PARK Job Truss Truss Type Qty Ply J1884248 PB01 L250968 VALLEY 1 Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:16 2007 Page 1 2-8-13 5-5-9 2-8-13 2-8-13 Scale = 1:11.7 4x6 = 7.00 12 5 3x6 = 3x6 = 2x4 5-3-10 0-1-15 5-3-10 LOADING (psf) **SPACING** 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES GRIP** in 20.0 1.25 TCLL Plates Increase TC 0.18 Vert(LL) -0.01 6 >999 360 MT20 244/190 **TCDL** 7.0 Lumber Increase 1.25 BC 0.15 Vert(TL) -0.01 >999 240 6 **BCLL** 10.0 * Rep Stress Incr WB 0.04 YES 0.01 Horz(TL) 5 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 16 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2 **TOP CHORD** Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 5-5-9 oc purlins. 2 X 4 SYP No.3 **WEBS BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=165/0-4-0, 5=165/0-4-0

Max Horz 1=-42(load case 4)

Max Uplift 1=-35(load case 6), 5=-35(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-74/48, 2-3=-275/153, 3-4=-275/153, 4-5=-74/48

BOT CHORD 2-6=-91/235, 4-6=-91/235

WEBS 3-6=-39/110

JOINT STRESS INDEX

2 = 0.39, 3 = 0.12, 4 = 0.39 and 6 = 0.08

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Continued on page 2

Truss Ossion Engineer Florida PE No. 34800 1 100 Creatal Bay Blyd Goynton Gesch, FL 55455

August 24,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884248
L250968	PB01	VALLEY	9	1	
				<u></u>	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:17 2007 Page 2

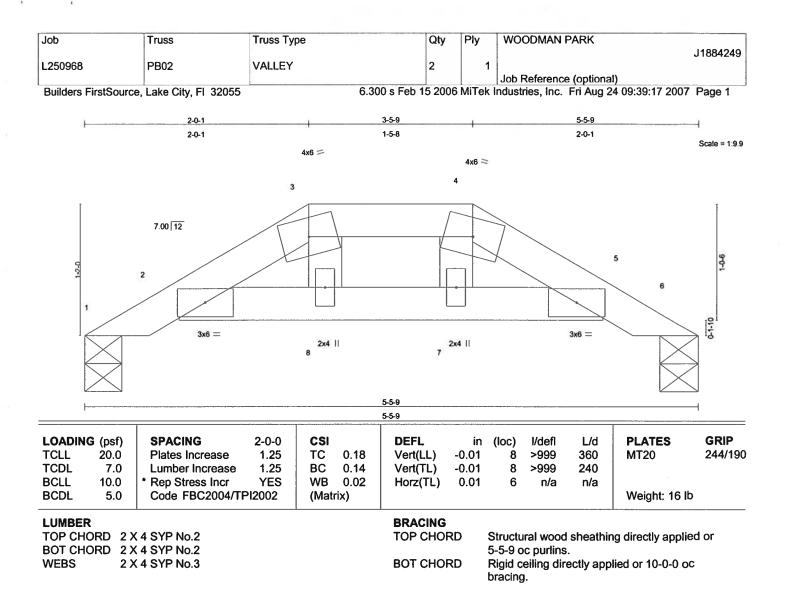
NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1 and 35 lb uplift at joint 5.
- 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truss Ceston Engineer Manda Ms. No.: 24809 1400 Casatel Bay Blvd. Goynton Gesch, Mt. 25456





REACTIONS (lb/size) 1=165/0-4-0, 6=165/0-4-0

Max Horz 1=-30(load case 4)

Max Uplift 1=-31(load case 6), 6=-31(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-74/55, 2-3=-312/217, 3-4=-282/219, 4-5=-312/217, 5-6=-74/55 TOP CHORD

BOT CHORD 2-8=-160/272, 7-8=-164/282, 5-7=-160/272

WEBS 3-8=-19/64, 4-7=-19/64

JOINT STRESS INDEX

2 = 0.36, 3 = 0.08, 4 = 0.08, 5 = 0.36, 7 = 0.04 and 8 = 0.04

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884249
L250968	PB02	VALLEY	2	1	
					Job Reference (optional)

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NOTES

- 6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1 and 31 lb uplift at joint 6.
- 8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard





Job		Truss		Truss Type		Qty	Ply	WOODMAN	PARK		
L25096	8	T02		CAL. POLYN	IESIAN	1	1				J1884250
								Job Reference	e (optional	I)	
Builder	rs FirstSource,	Lake City, F	FI 32055		6.300 s F	eb 15 2006	MiTek I	ndustries, Inc.	Fri Aug 2	4 09:39:19 2007	Page 1
	-2-0-0	7-9-7	9-0-Q	16-4-6	23-8-12	31-1-2		38-5-8	39-8-1	47-5-8	49-5-8

7-4-6

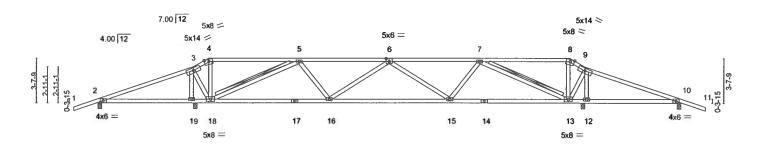
7-4-6

1-2-9

7-9-7

2-0-0 Scale = 1:90.2

7-4-6



		9-0-0				39-8-1		
	7-9-7	7-11-4	18-9-13	28-7-11	38-5-8	39-6-4	47-5-8	
100	7-9-7	0-1-13	9-9-13	9-9-13	9-9-13	1-0-12	7-9-7	34
		1-0-12				0-1-13		
Plate Offsets (X V)	. 16:0-3	3-0 0-3-01						

Plate Of	isets (X, Y): [6:0-3-0,0-3-0]				·						
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.16 1	IŠ-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.51	Vert(TL)	-0.35 1	15-16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 225 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

2-0-0

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

4-11-13 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 -

5-18, 7-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2/0-3-8, 19=1623/0-3-8, 12=1623/0-3-8, 10=2/0-3-8

Max Horz 2=63(load case 6)

Max Uplift 2=-226(load case 11), 19=-652(load case 4), 12=-657(load case 4),

10=-226(load case 10)

1-2-9

7-9-7

Max Grav 2=2(load case 1), 19=1629(load case 10), 12=1629(load case 11), 10=2(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-447/1075, 3-4=-104/362, 4-5=-117/326, 5-6=-1434/760,

6-7=-1434/760, 7-8=-117/326, 8-9=-109/362, 9-10=-447/1075, 10-11=0/33

BOT CHORD 2-19=-961/527, 18-19=-886/490, 17-18=-483/1220, 16-17=-483/1220,

15-16=-750/1718, 14-15=-483/1220, 13-14=-483/1220, 12-13=-886/490,

10-12=-961/527

3-19=-1528/671, 3-18=-354/1342, 4-18=-454/141, 5-18=-1685/899, 5-16=-102/433,

WEBS Continued on page 16=-356/250, 6-15=-356/250, 7-15=-102/433, 7-13=-1685/899, 8-13=-454/148,





Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T02	CAL. POLYNESIAN	1	1	J1884250
L250900	102	CAL. POLTNESIAN	'	,	Job Reference (optional)

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JOINT STRESS INDEX

2 = 0.28, 3 = 0.66, 4 = 0.64, 5 = 0.49, 6 = 0.54, 7 = 0.49, 8 = 0.64, 9 = 0.66, 10 = 0.28, 12 = 0.37, 13 = 0.67, 14 = 0.45, 15 = 0.39, 16 = 0.39, 17 = 0.45, 18 = 0.67 and 19 = 0.37

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2, 652 lb uplift at joint 19, 657 lb uplift at joint 12 and 226 lb uplift at joint 10.

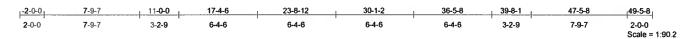
LOAD CASE(S) Standard

Julius Les Truss Coston Engineer Micros EE No. 34888 1100 Cassis Bay Bivd Goynton Gesch, FL 88495



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
			ĺ		J1884251
L250968	T03	CAL. POLYNESIAN	1	1	
					Job Reference (optional)

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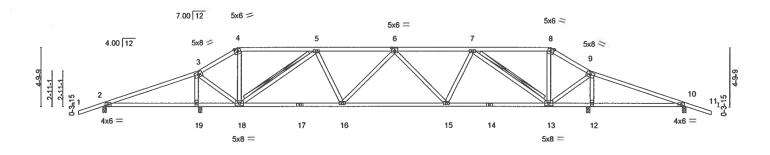




Plate Of	nsets (X, Y): [6:0-3-0,0-3-0]		-								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.17	10-12	>536	360	MT20	244/190
TCDL	7.0	Lumber increase	1.25	ВС	0.41	Vert(TL)	-0.22	16-18	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 236 lb	

LUMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or

5-4-1 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

5-18, 7-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=128/0-3-8, 19=1497/0-3-8, 12=1497/0-3-8, 10=128/0-3-8

Max Horz 2=77(load case 6)

Max Uplift 2=-242(load case 4), 19=-579(load case 5), 12=-579(load case 4),

10=-242(load case 5)

Max Grav 2=130(load case 10), 19=1497(load case 1), 12=1497(load case 1), 10=130(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-270/706, 3-4=-449/270, 4-5=-333/249, 5-6=-1305/744,

6-7=-1305/744, 7-8=-333/249, 8-9=-449/270, 9-10=-270/706, 10-11=0/33

BOT CHORD 2-19=-606/361, 18-19=-543/321, 17-18=-444/1185, 16-17=-444/1185,

15-16=-589/1462, 14-15=-444/1185, 13-14=-444/1185, 12-13=-543/321,

10-12=-606/361

WEBS 3-19=-1406/715, 3-18=-384/1096, 4-18=-26/93, 5-18=-1055/571, 5-16=-78/336,

Continued on page 16=-233/164, 6-15=-233/164, 7-15=-78/336, 7-13=-1055/571, 8-13=-25/93.

Julius Lee Truss Ceston Engineer Picrica Pa No. 341666 1100 Cestol Ray Blvd. Boynton Beach, FL 85465

August 24,2007



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Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling installing and Bracing Recommendation authorities (Mood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J18842	251
L250968	T03	CAL. POLYNESIAN	1	1		
					Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.28, 3 = 0.71, 4 = 0.29, 5 = 0.45, 6 = 0.41, 7 = 0.45, 8 = 0.29, 9 = 0.71, 10 = 0.28, 12 = 0.23, 13 = 0.49, 14 = 0.47, 15 = 0.45, 16 = 0.45, 17 = 0.47, 18 = 0.49 and 19 = 0.23

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 2, 579 lb uplift at joint 19, 579 lb uplift at joint 12 and 242 lb uplift at joint 10.

LOAD CASE(S) Standard

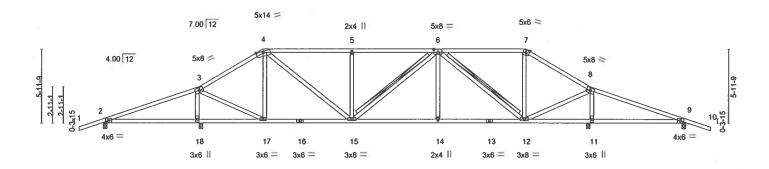
Julius Les Truse Design Engineer Horida PE No. 24889 1409 Caestal Bay Blvd Boynton Beach N. 68466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T04	CAL. POLYNESIAN	1	1	J18	884252
L230900	104	GAL. FOLTNESIAN	'	· ·	Job Reference (optional)	

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		7-9-7 0-1-13 5-0-1	2	1-2-1		7-0-11		1-2-1	5-0-12	0-1-13	7-9-7	
Plate Of	ffsets (X,Y): [6:0-4-0,0-3-0]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.18	2-18	>491	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.32	Vert(TL)	-0.14	14-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 246 lb	

27-3-1

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

7-9-7

13-0-0

7-11-4

BRACING

TOP CHORD Structural wood sheathing directly applied or

5-3-6 oc purlins. **BOT CHORD**

34-5-8

Rigid ceiling directly applied or 6-0-0 oc

39-8-1

47-5-8

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 -

6-15, 6-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

(lb/size) 2=230/0-3-8, 18=1392/0-3-8, 11=1410/0-3-8, 9=218/0-3-8 REACTIONS

Max Horz 2=90(load case 6)

Max Uplift 2=-294(load case 4), 18=-512(load case 5), 11=-514(load case 4),

20-2-7

9=-280(load case 5)

Max Grav 2=233(load case 10), 18=1392(load case 1), 11=1410(load case 1), 9=222(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-117/394, 3-4=-846/463, 4-5=-1245/751, 5-6=-1244/751,

6-7=-655/440, 7-8=-834/453, 8-9=-146/431, 9-10=0/33

BOT CHORD 2-18=-308/216, 17-18=-252/177, 16-17=-125/654, 15-16=-125/654,

14-15=-432/1241, 13-14=-432/1241, 12-13=-432/1241, 11-12=-286/204,

9-11=-343/243

WEBS 3-18=-1298/692, 3-17=-343/1001, 4-17=-391/226, 4-15=-392/777, 5-15=-399/282,

Continued on page 15=-26/23, 6-14=0/218, 6-12=-778/397, 7-12=-52/182, 8-12=-363/1027,

August 24,2007

🛦 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building occil. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884252
L250968	T04	CAL. POLYNESIAN	1	1	
,					Job Reference (optional)

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JOINT STRESS INDEX

2 = 0.27, 3 = 0.72, 4 = 0.93, 5 = 0.33, 6 = 0.37, 7 = 0.43, 8 = 0.72, 9 = 0.27, 11 = 0.22, 12 = 0.93, 13 = 0.46, 14 = 0.33, 15 = 0.73, 16 = 0.31, 17 = 0.56 and 18 = 0.22

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 2, 512 lb uplift at joint 18, 514 lb uplift at joint 11 and 280 lb uplift at joint 9.

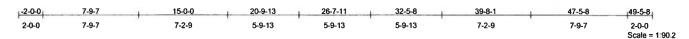
LOAD CASE(S) Standard

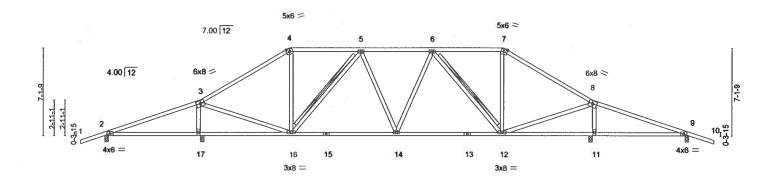
Julius Les Truss Coston Engineer Ploride FE No. 341009 1400 Costol Bay Blori Goynton Geson, FL 26466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884253
L250968	T05	CAL. POLYNESIAN	1	1	
					Job Reference (optional)

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	-	7-9-7	7-11-4	15-0-0	-	23-8-12		32-5-8	-	39-6-4	39-8-1	47-5-8	
		7-9-7	0-1-13	7-0-12		8-8-12		8-8-12	- 10	7-0-12	0-1-13	7-9-7	
LOADIN	G (psf)	SPAC	ING	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plates	Increase	1.25	TC	0.41	Vert(LL)	0.18	9-11	>489	360	MT20	244/190
TCDL	7.0	Lumbe	er Increas	e 1.25	BC	0.39	Vert(TL)	-0.19	12-14	>999	240		
BCLL	10.0	* Rep S	tress Incr	YES	WB	0.32	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code	FBC2004	I/TPI2002	(Mat	rix)						Weight: 249 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WERS	2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-10-2 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -5-16, 6-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=274/0-3-8, 17=1351/0-3-8, 11=1351/0-3-8, 9=274/0-3-8

Max Horz 2=104(load case 6)

Max Uplift 2=-318(load case 4), 17=-469(load case 5), 11=-456(load case 4),

9=-306(load case 5)

Max Grav 2=278(load case 10), 17=1351(load case 1), 11=1351(load case 1),

9=278(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-84/236, 3-4=-1017/528, 4-5=-790/528, 5-6=-1100/681, 6-7=-790/528

, 7-8=-1017/528, 8-9=-56/236, 9-10=0/33

BOT CHORD 2-17=-158/155, 16-17=-111/116, 15-16=-321/1081, 14-15=-321/1081,

13-14=-321/1081, 12-13=-321/1081, 11-12=-111/116, 9-11=-158/155

WEBS 3-17=-1255/693, 3-16=-292/927, 4-16=-100/229, 5-16=-519/267, 5-14=-4/146, 6-14=-4/146, 6-12=-519/267, 7-12=-91/229, 8-12=-292/927, 8-11=-1255/693

Julius Les Tross Cesign Engineer Piorids PE No. 34225 1400 Constal Bay Blyd

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J1884	4253
L250968	T05	CAL. POLYNESIAN	1	1		
					Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.22, 3 = 0.74, 4 = 0.55, 5 = 0.46, 6 = 0.46, 7 = 0.55, 8 = 0.74, 9 = 0.22, 11 = 0.22, 12 = 0.83, 13 = 0.41, 14 = 0.46, 15 = 0.41, 16 = 0.83 and 17 = 0.22

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 318 lb uplift at joint 2, 469 lb uplift at joint 17, 456 lb uplift at joint 11 and 306 lb uplift at joint 9.

LOAD CASE(S) Standard

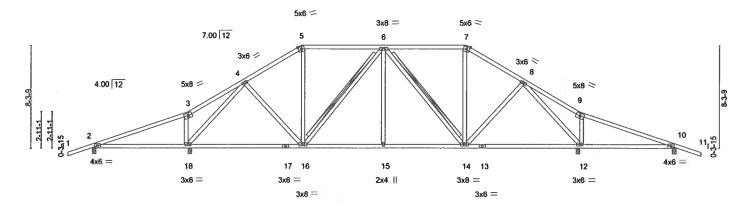
Julius Les Truse Cesian Engineer Floride FE No. 24889 1199 Gessial Bay Blvd Boynton Beach, FL bb+26



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884254
L250968	T06	CAL. POLYNESIAN	1	1	
	300				Job Reference (optional)

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1	- If		- 1		-				11		
	7-9-7 0-1-13	9-0-12		6-8-12	6-8-	12		9-0-12	0-1-13	7-9-7	
(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.20	10-12	>449	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.23	16-18	>999	240		
10.0	* Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.04	12	n/a	n/a		
5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 265 lb	
	20.0 7.0 10.0	7.9-7 0-1-13 (psf) SPACING 20.0 Plates Increase 7.0 Lumber Increase 10.0 * Rep Stress Incr	7-9-7 0-1-13 9-0-12 (psf) SPACING 2-0-0 20.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 10.0 * Rep Stress Incr YES	7-9-7 0-1-13 9-0-12 (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 7.0 Lumber Increase 1.25 BC 10.0 * Rep Stress Incr YES WB	7.9-7 0-1-13 9-0-12 6-8-12 (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 0.44 7.0 Lumber Increase 1.25 BC 0.46 10.0 * Rep Stress Incr YES WB 1.00	(psf) SPACING 2-0-0 CSI DEFL 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL)	(psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.20 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.23 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL) 0.04	(psf) SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.20 10-12 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.23 16-18 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL) 0.04 12	(psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.20 10-12 >449 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.23 16-18 >999 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL) 0.04 12 n/a	(psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.20 10-12 >449 360 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.23 16-18 >999 240 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL) 0.04 12 n/a n/a	7-9-7 0-1-13 9-0-12 6-8-12 9-0-12 0-1-13 7-9-7 (psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d PLATES 20.0 Plates Increase 1.25 TC 0.44 Vert(LL) 0.20 10-12 >449 360 MT20 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.23 16-18 >999 240 10.0 * Rep Stress Incr YES WB 1.00 Horz(TL) 0.04 12 n/a n/a

23.8.12

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.9.7

7-11-4

17.0.0

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD St

Structural wood sheathing directly applied or

5-11-14 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 6-16, 6-14

47-5-R

Fasten T and I braces to narrow edge of web

with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=302/0-3-8, 18=1323/0-3-8, 12=1323/0-3-8, 10=302/0-3-8

Max Horz 2=118(load case 6)

Max Uplift 2=-315(load case 4), 18=-416(load case 5), 12=-402(load case 4),

10=-300(load case 5)

Max Grav 2=306(load case 10), 18=1323(load case 1), 12=1323(load case 1), 10=306(load case 11)

,

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-86/159, 3-4=-24/136, 4-5=-996/570, 5-6=-821/552, 6-7=-821/552,

7-8=-996/570, 8-9=-24/136, 9-10=-72/159, 10-11=0/33

BOT CHORD 2-18=-85/172, 17-18=-141/687, 16-17=-141/687, 15-16=-236/999, 14-15=-236/999,

13-14=-141/687, 12-13=-141/687, 10-12=-85/172

WEBS 3-18=-375/370, 4-18=-1126/438, 4-16=-110/258, 5-16=-60/239, 6-16=-353/201,

6-15=0/162, 6-14=-353/201, 7-14=-60/239, 8-14=-121/258, 8-12=-1126/438,

9-12=-375/370

Trues Design Chaineer Phonds PE No. 34868 1160 Casstell Bay Blvd. Boynton Weach, Ft. 20406

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	٦
					J1884254	.
L250968	T06	CAL. POLYNESIAN	1	1	* Y	1
1					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:24 2007 Page 2

JOINT STRESS INDEX

2 = 0.27, 3 = 0.77, 4 = 0.40, 5 = 0.47, 6 = 0.56, 7 = 0.47, 8 = 0.40, 9 = 0.77, 10 = 0.27, 12 = 0.37, 13 = 0.22, 14 = 0.56, 15 = 0.33, 16 = 0.56, 17 = 0.22 and 18 = 0.37

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2, 416 lb uplift at joint 18, 402 lb uplift at joint 12 and 300 lb uplift at joint 10.

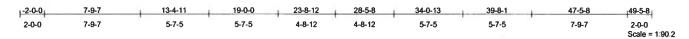
LOAD CASE(S) Standard

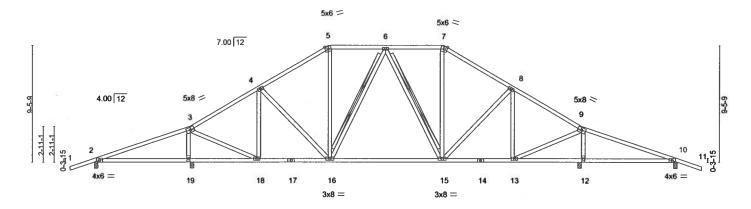
Julius Lee Truss Ceston Cnotneer Plands PE No. 34868 1498 Casstell Rey Blvd Boynton Beach, FL 95426



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
1						J1884255
L250968	T07	CAL. POLYNESIAN	1	1	*	
					Job Reference (optional)	

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		7-9-7 0-1-13 5-5-7	5-7-5	1	9-5-8	5-7-5	5-5-	- 11	7-9-7	
LOADIN	IG (psf)	SPACING 2-0	o csi		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase 1.3	5 TC	0.44	Vert(LL)	0.18 10-12	>492	360	MT20	244/190
TCDL	7.0	Lumber Increase 1.3	5 BC	0.42	Vert(TL)	-0.26 15-16	>999	240		
BCLL	10.0	* Rep Stress Incr YE	S WB	0.29	Horz(TL)	0.02 12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI200	2 (Matr	ix)					Weight: 275 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD Structural wood sheathing directly applied or

5-11-1 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc **BOT CHORD**

bracing.

WEBS T-Brace: 2 X 4 SYP No.3 -6-16, 6-15

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=280/0-3-8, 19=1345/0-3-8, 12=1345/0-3-8, 10=280/0-3-8

Max Horz 2=132(load case 5)

Max Uplift 2=-338(load case 4), 19=-391(load case 6), 12=-365(load case 7),

10=-313(load case 5)

Max Grav 2=290(load case 10), 19=1345(load case 1), 12=1345(load case 1),

10=290(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-93/231, 3-4=-932/480, 4-5=-969/582, 5-6=-775/566, 6-7=-775/566,

7-8=-969/582, 8-9=-932/480, 9-10=-48/231, 10-11=0/33

BOT CHORD 2-19=-154/150, 18-19=-90/114, 17-18=-139/729, 16-17=-139/729, 15-16=-133/831,

14-15=-139/729, 13-14=-139/729, 12-13=-90/114, 10-12=-154/150

WEBS 3-19=-1246/655, 3-18=-279/898, 4-18=-347/187, 4-16=-108/154, 5-16=-80/229,

6-16=-231/174, 6-15=-231/174, 7-15=-80/229, 8-15=-125/154, 8-13=-347/187,

9-13=-279/898, 9-12=-1246/655

Engineer

Continued on page 2

August 24,2007

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1.050000	T07	CAL DOLVALECIAN			J1884255
L250968	T07	CAL. POLYNESIAN]	1	Job Reference (optional)
l			L		Job Reference (optional)

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JOINT STRESS INDEX

2 = 0.26, 3 = 0.76, 4 = 0.40, 5 = 0.43, 6 = 0.45, 7 = 0.43, 8 = 0.40, 9 = 0.76, 10 = 0.26, 12 = 0.21, 13 = 0.50, 14 = 0.26, 15 = 0.60, 16 = 0.60, 17 = 0.26, 18 = 0.50 and 19 = 0.21

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 2, 391 lb uplift at joint 19, 365 lb uplift at joint 12 and 313 lb uplift at joint 10.

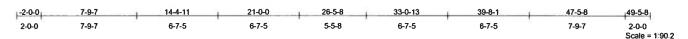
LOAD CASE(S) Standard

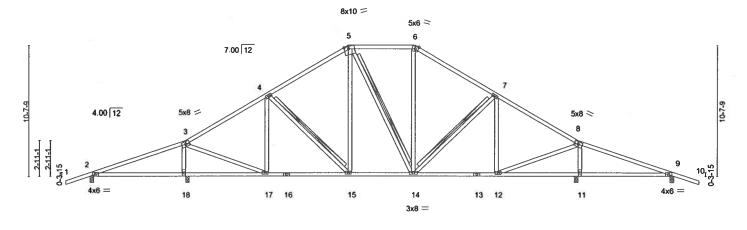
Julius Les Truss Design Chainear Plonds PE No. 31889 1100 Chastel Bay Blvd Boynton Besch, Ft. 86466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J.	1884256
L250968	T08	CAL. POLYNESIAN	1	1		
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:28 2007 Page 1





	7-9-7	7-11-4	14-4-11	21-0-0	26-5-8	33-0-13	39-6-4	39 ₁ 8-1	47-5-8	
1	7-9-7	0-1-13	6-5-7	6-7-5	5-5-8	6-7-5	6-5-7	0-1-13	7-9-7	

Plate Of	fsets (X,Y): [5:0-4-1,Edge]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.19	9-11	>485	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.12	9-11	>735	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 275 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2

WEBS

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 5-10-13 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc

bracing. T-Brace:

2 X 4 SYP No.3 -

4-15, 5-14, 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=291/0-3-8, 18=1335/0-3-8, 11=1332/0-3-8, 9=293/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-354(load case 4), 18=-406(load case 6), 11=-378(load case 7),

9=-319(load case 5)

Max Grav 2=300(load case 10), 18=1335(load case 1), 11=1332(load case 1), 9=304(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-104/194, 3-4=-1008/503, 4-5=-947/580, 5-6=-740/571, 6-7=-947/580

, 7-8=-1009/503, 8-9=-33/188, 9-10=0/33

BOT CHORD 2-18=-118/136, 17-18=-62/105, 16-17=-149/788, 15-16=-149/788, 14-15=-50/740,

13-14=-149/790, 12-13=-149/790, 11-12=-54/99, 9-11=-111/135 3-18=-1238/654, 3-17=-267/900, 4-17=-273/173, 4-15=-108/138, 5-15=-68/206,

5-10--1230/034, 3-17--207/300, 4-17--273/173, 4-13--100/130, 3-13--00/200,

5-14=-140/141, 6-14=-68/207, 7-14=-125/138, 7-12=-272/172, 8-12=-267/895, Continued on page 21=-1235/653

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▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building occie. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884256
L250968	T08	CAL. POLYNESIAN	1	1	
					Job Reference (optional)

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JOINT STRESS INDEX

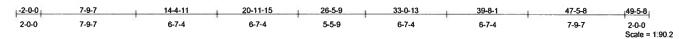
2 = 0.24, 3 = 0.81, 4 = 0.40, 5 = 0.53, 6 = 0.50, 7 = 0.40, 8 = 0.81, 9 = 0.24, 11 = 0.21, 12 = 0.49, 13 = 0.29, 14 = 0.61, 15 = 0.290.36, 16 = 0.28, 17 = 0.49 and 18 = 0.21

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 406 lb uplift at joint 18, 378 lb uplift at joint 11 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J1	884257
L250968	T09	CAL. POLYNESIAN	1	1	*.	
					Job Reference (optional)	
Builders FirstSou	irce, Lake City, Fl 3	2055 6.	300 s Feb 15 2006	MiTek I	Industries, Inc. Fri Aug 24 09:39:29 2007 Page 1	age 1



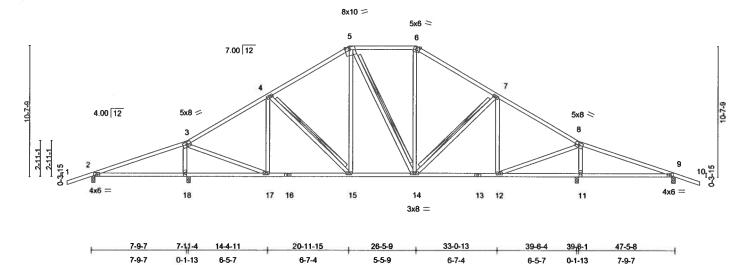


Plate Of	ffsets (X,Y): [5:0-4-1,Edge]		,							-	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.19	9-11	>485	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.26	Vert(TL)	-0.12	9-11	>735	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 275 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

5-10-13 oc purlins, except

2-0-0 oc purlins (6-0-0 max.): 5-6.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -4-15, 5-14, 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=291/0-3-8, 18=1335/0-3-8, 11=1332/0-3-8, 9=293/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-354(load case 4), 18=-406(load case 6), 11=-378(load case 7),

9=-319(load case 5)

Max Grav 2=300(load case 10), 18=1335(load case 1), 11=1332(load case 1), 9=304(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-104/194, 3-4=-1007/503, 4-5=-947/580, 5-6=-740/571, 6-7=-947/580

, 7-8=-1009/503, 8-9=-33/188, 9-10=0/33

BOT CHORD 2-18=-118/136, 17-18=-62/105, 16-17=-149/788, 15-16=-149/788, 14-15=-50/740,

13-14=-149/790, 12-13=-149/790, 11-12=-54/99, 9-11=-111/135

WEBS 3-18=-1238/654, 3-17=-267/900, 4-17=-273/173, 4-15=-108/138, 5-15=-67/206, Continued on page 24=-140/141, 6-14=-68/207, 7-14=-125/138, 7-12=-272/172, 8-12=-267/895.

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overald building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult 8CS-I or HIB-91 Handling Installing and Bracing Recommendation author brood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884257
L250968	250968 T09	CAL. POLYNESIAN	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:29 2007 Page 2

JOINT STRESS INDEX

2 = 0.24, 3 = 0.81, 4 = 0.40, 5 = 0.53, 6 = 0.50, 7 = 0.40, 8 = 0.81, 9 = 0.24, 11 = 0.21, 12 = 0.49, 13 = 0.29, 14 = 0.61, 15 = 0.49, 0.36, 16 = 0.28, 17 = 0.49 and 18 = 0.21

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 406 lb uplift at joint 18, 378 lb uplift at joint 11 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard



Job	Truss	Truss Type	-	Qty	Ply	WOODMAN PARK		,
L250968	T10	CAL. POLYNESIAI	١	1	1			J188425
						Job Reference (option		
Builders FirstSour	ce, Lake City, Fl 32	2055	6.300 s Feb 1	5 2006	Mileki	Industries, Inc. Fri Aug	24 09:39:30 2007	Page 1
-2-0-0	7-9-7	14-4-11 20-11-15	26-5-9	7	33-0-13	39-8-1	47-5-8	49-5-8

5-5-9

6-7-4

6-7-4

7-9-7

6-7-4

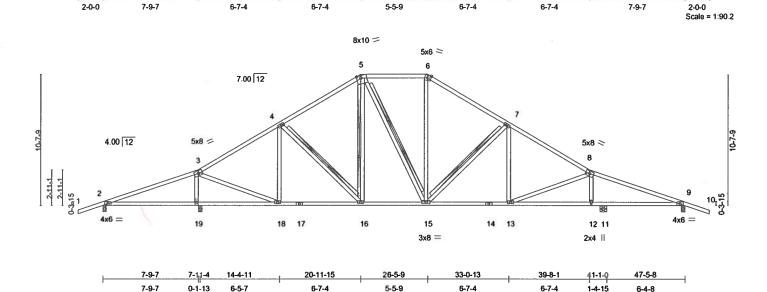


Plate Offsets (X,Y): [5:0-4-1,Edge]												
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.18	2-19	>491	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.36	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 275 lb	

LUMBER		BRACING	
	D 2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing
		TOP CHORD	9
BOT CHOR	D 2 X 4 SYP No.2		4-11-2 oc purlins, except
WEBS	2 X 4 SYP No.3		2-0-0 oc purlins (6-0-0 ma:
		BOT CHORD	Rigid ceiling directly applie

6-7-4

2-0-0

7-9-7

ourlins (6-0-0 max.): 5-6. Rigid ceiling directly applied or 2-2-0 oc bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -4-16, 5-16, 5-15, 7-15

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

wood sheathing directly applied or

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=208/0-3-8, 19=1550/0-3-8, 9=670/0-3-8, 11=822/0-6-7

Max Horz 2=164(load case 5)

Max Uplift 2=-350(load case 4), 19=-423(load case 6), 9=-285(load case 7),

11=-286(load case 7)

Max Grav 2=254(load case 10), 19=1550(load case 1), 9=670(load case 1), 11=822(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-130/446, 3-4=-1056/525, 4-5=-1078/635, 5-6=-917/643,

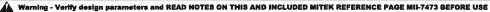
6-7=-1151/663, 7-8=-1516/716, 8-9=-1285/489, 9-10=0/33

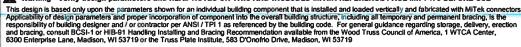
BOT CHORD 2-19=-356/227, 18-19=-275/186, 17-18=-168/830, 16-17=-168/830, 15-16=-97/852,

14-15=-336/1232, 13-14=-336/1232, 12-13=-361/1189, 11-12=-335/1149,

9-11=-335/1149

WFR Flued on page 19=-1452/738, 3-18=-381/1188, 4-18=-382/216, 4-16=-109/156, 5-16=-48/168,







Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	1
					J1884258	
L250968	T10	CAL. POLYNESIAN	1	1		
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:30 2007 Page 2

JOINT STRESS INDEX

2 = 0.24, 3 = 0.82, 4 = 0.40, 5 = 0.53, 6 = 0.50, 7 = 0.40, 8 = 0.70, 9 = 0.71, 12 = 0.33, 13 = 0.34, 14 = 0.50, 15 = 0.61, 16 = 0.36, 17 = 0.30, 18 = 0.65 and 19 = 0.23

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 2, 423 lb uplift at joint 19, 285 lb uplift at joint 9 and 286 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Les Truss Cesian Engineer Plofide PE No. 34269 1109 Chestel Bay Blvd Bovnjon Wesch, FL 28486



ob	Truss		Truss	Туре		Qty	Ply	WOODN	IAN PA	RK		J1884259
250968	T11		CAL.	POLYNESIAN		3	1	Job Refe	rence (optional)		J 1004258
uilders FirstS	Source, Lake (City, FI 320)55	(6.300 s Feb	15 2006	MiTek I	ndustries,	Inc. Fr	ri Aug 24 09	9:39:31 200	7 Page 1
-2-0-0	7-9-7	-	14-4-11	20-11-15	26-5-9	-	33-0-13	-	39-8-1		47-5-8	49-5-8
2-0-0	7-9-7		6-7-4	6-7-4	5-5-9		6-7-4		6-7-4		7-9-7	2-0-0 Scale = 1:90.
				8	8x10 =							
					5	5x6 ≃ 6						
			7.00 12		TAN TO THE TANK							
			7.00 12	4				1				
	4 00 12	5v8 <	7.00 12					1		Ev0 >>		
	4.00 12	5x8 =	7.00 12					7		5x8 \ 8		
	4.00 12	=	7.00 12					7				
241-4	4.00 12	=	7.00 12					7				9 10,5,6
2-11-1	4.00 12 4.00 =	=	7.00 12	18 17	16	15		7				9 10, 50 4x6 =
2-11-1		3	7.00 12	18 17						8		9 101.25
2-11-1		19	7.00 12	18 17		15	33-0-13	14 13	39-8-1	12 11	47.5-8	9 101.55

LOADIN	. ,	SPACING	2-0-0	CSI		DEFL	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.23 12	-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.79	Vert(TL)	-0.41 12	-13	>990	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 275 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.1D *Except*

14-17 2 X 4 SYP No.2

WEBS

BOT CHORD

2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

3-10-9 oc purlins, except

2-0-0 oc purlins (6-0-0 max.): 5-6.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -

4-16, 5-16, 5-15, 7-15

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=153/0-3-8, 19=1686/0-3-8, 9=949/0-3-8, 11=462/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-337(load case 4), 19=-449(load case 6), 9=-351(load case 7),

11=-174(load case 4)

Max Grav 2=224(load case 10), 19=1686(load case 1), 9=949(load case 1), 11=462(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-230/613, 3-4=-1079/537, 4-5=-1153/677, 5-6=-1023/703,

6-7=-1273/733, 7-8=-1805/883, 8-9=-2191/998, 9-10=0/33

2-19=-514/321, 18-19=-426/276, 17-18=-178/850, 16-17=-178/850, 15-16=-134/916

, 14-15=-481/1483, 13-14=-481/1483, 12-13=-828/2022, 11-12=-814/2004,

9-11=-814/2004

WFR Rued on page 19=-1585/815, 3-18=-489/1372, 4-18=-451/257, 4-16=-123/213, 5-16=-81/138,

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP11 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handfling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1			<u> </u>		J1884259
L250968	T11	CAL. POLYNESIAN	3	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:31 2007 Page 2

JOINT STRESS INDEX

2 = 0.27, 3 = 0.82, 4 = 0.40, 5 = 0.53, 6 = 0.51, 7 = 0.40, 8 = 0.64, 9 = 0.78, 12 = 0.33, 13 = 0.34, 14 = 0.56, 15 = 0.61, 16 = 0.36, 17 = 0.31, 18 = 0.75 and 19 = 0.26

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 337 lb uplift at joint 2, 449 lb uplift at joint 19, 351 lb uplift at joint 9 and 174 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Les Truss Design (Ingineer Floride PE No. 34869 1199 (Jacks) Fly Blyd Boynton Beach, FL 66466



Truss Type Ply **WOODMAN PARK** Job Truss Qty J1884260 L250968 T12 CAL. POLYNESIAN 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:33 2007 Page 1 -2-0-0 7-9-7 20-11-15 33-0-13 39-8-1 47-5-8 49-5-8 6-7-4 2-0-0 7-9-7 6-7-4 5-5-9 6-7-4 6-7-4 7-9-7 2-0-0 Scale = 1:90.2 8x10 = 5x6 = 5 7.00 12 4.00 12 5x8 = 5x14 < 4x6 = 19 15 12 11 18 17 16 14 13 3x8 = 2x4 || 2x4 \\ 7-9-7 20-11-15 33-0-13 47-5-8 40-7-0 6-5-7 6-7-4 7-9-7 0-1-13 5-5-9 6-7-4 6-7-4 0-10-15 6-10-8 Plate Offsets (X,Y): [5:0-4-1,Edge] LOADING (psf) **SPACING** 2-0-0 CSI **DEFL PLATES GRIP** L/d in (loc) I/defl **TCLL** 20.0 Plates Increase 1.25 TC 0.38 Vert(LL) 0.17 2-19 >533 360 MT20 244/190 **TCDL** 7.0 Lumber Increase 1.25 BC 0.26 Vert(TL) -0.11 2-19 >807 240 **BCLL** 10.0 * Rep Stress Incr YES **WB** 0.30 0.02 Horz(TL) 11 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 278 lb LUMBER **BRACING** TOP CHORD TOP CHORD 2 X 4 SYP No.2 Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.1D *Except* 5-7-15 oc purlins, except 14-17 2 X 4 SYP No.2 2-0-0 oc purlins (6-0-0 max.): 5-6. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** T-Brace: 2 X 4 SYP No.3 -4-16, 5-15, 7-15 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=288/0-3-8, 19=1358/0-3-8, 9=232/0-3-8, 11=1372/0-6-7

Max Horz 2=164(load case 5)

Max Uplift 2=-352(load case 4), 19=-409(load case 6), 9=-304(load case 5),

11=-376(load case 7)

Max Grav 2=298(load case 10), 19=1358(load case 1), 9=247(load case 11),

11=1372(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

BOT CHORD

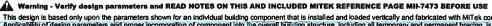
TOP CHORD 1-2=0/33, 2-3=-101/202, 3-4=-1028/511, 4-5=-976/593, 5-6=-775/586, 6-7=-988/598

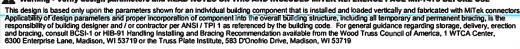
, 7-8=-1102/551, 8-9=-103/344, 9-10=0/33

2-19=-125/144, 18-19=-69/112, 17-18=-156/806, 16-17=-156/806, 15-16=-62/765, 14-15=-192/871, 13-14=-192/871, 12-13=0/179, 11-12=0/175, 9-11=-261/203

WEBS 3-19=-1260/665, 3-18=-283/926, 4-18=-284/180, 4-16=-106/132, 5-16=-67/203,

Continued on page 26=-139/161, 6-15=-77/221, 7-15=-169/177, 7-13=-202/133, 8-13=-250/747,







Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
	T12	0.1. 501.4.501.1.			J1884260
L250968	112	CAL. POLYNESIAN	1	1	
	27				Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:33 2007 Page 2

JOINT STRESS INDEX

2 = 0.26, 3 = 0.81, 4 = 0.40, 5 = 0.53, 6 = 0.50, 7 = 0.40, 8 = 0.42, 9 = 0.32, 11 = 0.48, 12 = 0.33, 13 = 0.41, 14 = 0.33, 15 = 0.61, 16 = 0.36, 17 = 0.33, 18 = 0.51 and 19 = 0.21

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 2, 409 lb uplift at joint 19, 304 lb uplift at joint 9 and 376 lb uplift at joint 11.

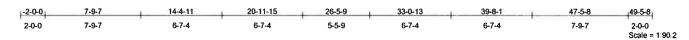
LOAD CASE(S) Standard

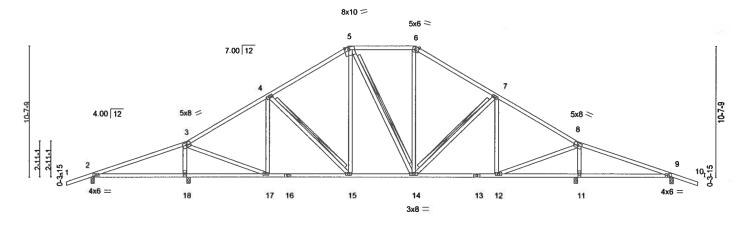
Julius Les Truss Ossign Engineer Platida PE No. 34535 1409 Crastal Say Slvd Boynton Beach, PL 55456



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
						J1884261
L250968	T13	CAL. POLYNESIAN	2	1		
					Job Reference (optional)	

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J	7-9-7	7-11-4	14-4-11	20-11-15	26-5-9	33-0-13	39-6-4	39 ₁ 8-1	47-5-8	-4
	7-9-7	0-1-13	6-5-7	6-7-4	5-5-9	6-7-4	6-5-7	0-1-13	7-9-7	

Plate Of	ffsets (X,Y	'): [5:0-4-1,Edge]				1						
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.19	9-11	>485	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.12	9-11	>735	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 275 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 5-10-13 oc purlins, except

2-0-0 oc purlins (6-0-0 max.): 5-6.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -4-15, 5-14, 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=291/0-3-8, 18=1335/0-3-8, 11=1332/0-3-8, 9=293/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-354(load case 4), 18=-406(load case 6), 11=-378(load case 7),

9=-319(load case 5)

Max Grav 2=300(load case 10), 18=1335(load case 1), 11=1332(load case 1), 9=304(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-104/194, 3-4=-1007/503, 4-5=-947/580, 5-6=-740/571, 6-7=-947/580

, 7-8=-1009/503, 8-9=-33/188, 9-10=0/33

BOT CHORD 2-18=-118/136, 17-18=-62/105, 16-17=-149/788, 15-16=-149/788, 14-15=-50/740,

13-14=-149/790, 12-13=-149/790, 11-12=-54/99, 9-11=-111/135

WEBS 3-18=-1238/654, 3-17=-267/900, 4-17=-273/173, 4-15=-108/138, 5-15=-67/206, Continued on page 24=-140/141, 6-14=-68/207, 7-14=-125/138, 7-12=-272/172, 8-12=-267/895,

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🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling installing and Bracing Recommendation authorities from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T13	CAL. POLYNESIAN	2	1		J1884261
					Job Reference (optional)	

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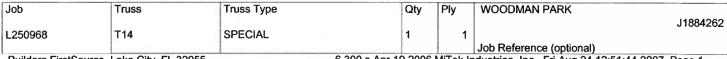
JOINT STRESS INDEX

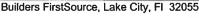
2 = 0.24, 3 = 0.81, 4 = 0.40, 5 = 0.53, 6 = 0.50, 7 = 0.40, 8 = 0.81, 9 = 0.24, 11 = 0.21, 12 = 0.49, 13 = 0.29, 14 = 0.61, 15 = 0.490.36, 16 = 0.28, 17 = 0.49 and 18 = 0.21

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 406 lb uplift at joint 18, 378 lb uplift at joint 11 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard







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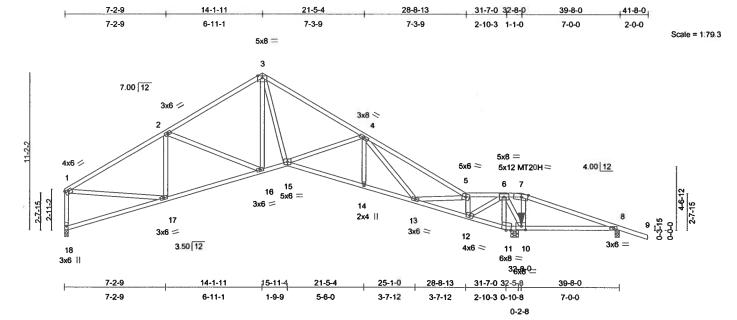


Plate Offsets (X,Y):	[1:0-3-0,0-1-8], [7:0-5	-7,Edge],	[10:0-3-	8,0-3-0]						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TPI2	2-0-0 1.25 1.25 NO 2002	CSI TC BC WB (Matr	0.75 0.36 0.49	DEFL Vert(LL) Vert(TL) Horz(TL)	in (loc) -0.12 14-15 -0.24 14-15 0.16 10	l/defi >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 MT20H Weight: 224 lb	GRIP 244/190 187/143

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-15

oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 4-11-13 oc bracing.

REACTIONS (lb/size) 18=932/0-3-8, 8=-214/0-3-8, 10=2421/0-6-7

Max Horz 18=-315(load case 3)

Max Uplift 18=-191(load case 5), 8=-335(load case 4), 10=-767(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1357/286, 2-3=-1290/263, 3-4=-1559/302, 4-5=-1525/296, 5-6=-351/180,

6-7=-166/1436, 7-8=-249/1648, 8-9=0/33, 1-18=-917/227

BOT CHORD 17-18=-134/352, 16-17=-220/1148, 15-16=-50/1071, 14-15=-151/1603, 13-14=-149/1609,

12-13=-107/468, 11-12=-997/244, 10-11=-943/234, 8-10=-1498/295

WEBS 2-17=-326/102, 2-16=-185/199, 3-16=-98/184, 3-15=-64/875, 4-15=-376/263, 4-14=0/166,

4-13=-493/76, 5-13=-63/799, 5-12=-1166/304, 6-12=-356/1515, 6-11=0/207, 6-10=-984/82,

7-10=-1000/325, 1-17=-136/1007

JOINT STRESS INDEX

1 = 0.75, 2 = 0.42, 3 = 0.70, 4 = 0.80, 5 = 0.56, 6 = 0.70, 7 = 0.81, 8 = 0.49, 10 = 0.18, 11 = 0.16, 12 = 0.61, 13 = 0.41, 14 = 0.34, 15 = 0.60, 16 = 0.38, 17 = 0.52 and 18 = 0.32

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

Piònda PE No. 3-1889 1 100 Chestal Bay Blvd Boynton Goach, PL 85486

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Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T14	SPECIAL	4		J1884262
L250900	1 14	SPECIAL		1	Job Reference (optional)

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NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 18, 335 lb uplift at joint 8 and 767 lb uplift at joint 10.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-6=-54, 6-7=-118(B=-64), 7-9=-54, 15-18=-10, 11-15=-10, 10-11=-22(B=-12), 8-10=-10

Concentrated Loads (lb) Vert: 10=-411(B)

> Julius Les Truss Design Engineer Plonde PE No. 24868 1199 Caestel Bay Blvd Boynton Wesch, FL 20426



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
						J1884263
L250968	T14A	SCISSORS	2	1		
					Job Reference (optional)	

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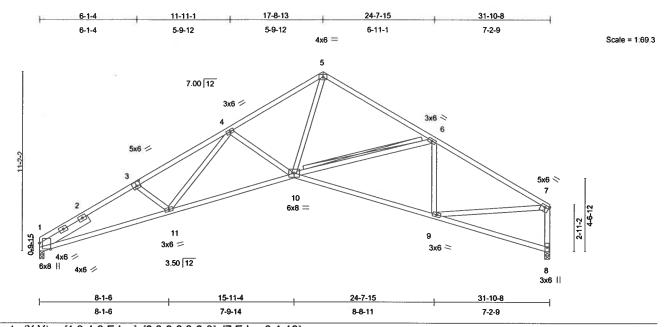


Plate Offsets (X,Y): [1:0-4-8,Edge], [3:0-3-0,0-3-0], [7:Edge,0-1-12]

TCLL 20.0 Plates increase 1.25 TC 0.67 Vert(LL) 0.21 10-11 >999 360 MT20 24	1444400
	244/190
TCDL 7.0 Lumber Increase 1.25 BC 0.61 Vert(TL) -0.39 10-11 >979 240	
BCLL 10.0 * Rep Stress Incr YES WB 0.45 Horz(TL) 0.27 8 n/a n/a	
BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 176 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER Left 2 X 6 SYP No.1D 3-7-2 **BRACING**

WEBS

TOP CHORD Structural wood sheathing directly applied or

3-1-2 oc purlins, except end verticals. **BOT CHORD**

Rigid ceiling directly applied or 6-1-14 oc

bracing.

T-Brace:

2 X 4 SYP No.3 -

6-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1013/0-3-8, 8=1013/0-3-8

Max Horz 1=300(load case 5)

Max Uplift 1=-219(load case 6), 8=-204(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2573/1192, 2-3=-2478/1206, 3-4=-2351/1170, 4-5=-1860/906, 5-6=-1465/706,

6-7=-1500/686, 7-8=-999/496

BOT CHORD 1-11=-1059/2141, 10-11=-821/1969, 9-10=-523/1280, 8-9=-65/99

WEBS 3-11=-110/179, 4-11=-157/266, 4-10=-456/357, 5-10=-603/1302, 6-10=-188/246,

6-9=-364/226, 7-9=-437/1135

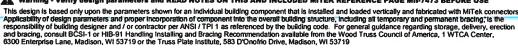
JOINT STRESS INDEX

1 = 0.84, 1 = 0.45, 1 = 0.45, 2 = 0.00, 3 = 0.29, 4 = 0.40, 5 = 0.65, 6 = 0.40, 7 = 0.71, 8 = 0.28, 9 = 0.57, 10 = 0.69 and 11 = 0.450.37

Continued on page 2

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🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1.050000	T440	colocopo		١.	J1884263
L250968	T14A	SCISSORS	2	1	Joh Defenses (anti-nell)
L					Job Reference (optional)

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 1 and 204 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truss Cesian Engineer Plonida PE No. 3-1869 1166 Cesstel Bay Blvn Govnton Gesch, FL 68466



Job	Truss	Truss Typ	oe .	Qty	Ply	WO	ODMAN I	PARK		14004004
L250968	T15	SPECIAL		1		1				J1884264
								e (optional)		
Builders FirstSou	rce, Lake City, FI	32055	6.30	0 s Feb 15 200	6 MiTek	Indust	ries, Inc.	Fri Aug 24	09:39:37 2007	Page 1
₁ -2-0-0 ₁	7-9-7	14-10-5	21-11-3	28-4-12	3-	4-10-5	38-5-8	39-8-1	47-5-8 4	9-5-8
2-0-0	7-9-7	7-0-14	7-0-14	6-5-9	1	6-5-9	- 1	1-2-9	1	2-0-0
			E.	t6 =						Scale = 1:93.5
I		7.00 12		5						
5			6 /		lx6 >					
		3,			6					
3							5x6 ==	5x14 <>		
4	.00 12 8	x14 =					7	5x6 ≈		
1.1.	3	3		16				9		21
2-11-1 3-7-9 15 15	1				15					6 46-12 3-7-9
2 5 2 5 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	,		17		3x6 <		14		1	0 11 2
3 ()							14	E.	1 13	~ I
4x	6 =		3=				3x8 =	13	12 4x6 :	= ~ 6
4x		18 ix6 =	3.50 12				3x8 <=	13 5x6 = 3x	12	= 7 6
4x		18					3x8 =		12	= 7 6
4x	5	18 ix8 =		, 28-4-12	. 34	4-10-5	3x8 ≈ 39.4+	5x6 = 3x	6 \	= ~ 6
4x	7-11-4 8-	18 ==	3.50 12	<u>28-4-12</u> 4-8-0		4-10-5 6-5-9		5x6 = 3x	12	= 7 0
Plate Offsets (X,	7-11-4 8- 7-11-4 0-1	18 ix8 =	3.50 12 23-8-12				39-4-	5x6 = 3x	47-5-8	=
⊢ Plate Offsets (X,	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3	18 ix6 = 14-10-5 i-12 6-9-5	3.50 12 23-8-12 8-10-7	4-8-0	' (8-5-9	39-4- 4-6-3	5x6 = 3x 8	47-5-8 5-8-4	
Plate Offsets (X,	7-11-4 8- 7-11-4 0-1	18 ix6 = 14-10-5 i-12 6-9-5 i-4]	3.50 12 23-8-12	4-8-0	' (6-5-9 (loc)	39-4-	5x6 = 3x	47-5-8	GRIP
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Incre	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48	4-8-0	in 0.22 -0.38	(loc) 2-18 16-17	39-4- 4-6-3	5x6 = 3x 8 41-9-4 3 2-4-12	47-5-8 5-8-4	GRIP
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Increa * Rep Stress I	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48 WB 0.54	DEFL Vert(LL)	in 0.22	(loc) 2-18	39.4- 4.6-3 I/defl >432	5x6 = 3x 8 41-9-4 3 2-4-12 L/d 360	47-5-8 5-8-4 PLATES MT20	GRIP 244/196
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Increa * Rep Stress I	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48	DEFL Vert(LL) Vert(TL)	in 0.22 -0.38	(loc) 2-18 16-17	39-4- 4-6-3 I/defl >432 >999	5x6 = 3x 8 41-9-4 3 2-4-12 L/d 360 240	47-5-8 5-8-4	GRIP 244/196
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Increa * Rep Stress I	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48 WB 0.54	DEFL Vert(LL) Vert(TL)	in 0.22 -0.38	(loc) 2-18 16-17	39-4- 4-6-3 I/defl >432 >999	5x6 = 3x 8 41-9-4 3 2-4-12 L/d 360 240	47-5-8 5-8-4 PLATES MT20	GRIP 244/196
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0 LUMBER TOP CHORD 2	7.11-4 8-7.11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Incre * Rep Stress I Code FBC2	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48 WB 0.54	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.22 -0.38 0.16	(loc) 2-18 16-17 12 Structu	1/defl >432 >999 n/a	5x6 = 3x 8	47-5-8 5-8-4 PLATES MT20	GRIP 244/190
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0 LUMBER TOP CHORD 2 BOT CHORD 2	7-11-4 8- 7-11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Incre * Rep Stress I Code FBC20 2 X 4 SYP No.2	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48 WB 0.54	DEFL Vert(LL) Vert(TL) Horz(TL) BRACING TOP CHOR	in 0.22 -0.38 0.16	(loc) 2-18 16-17 12 Structu 4-3-9 o	1/defi >432 >999 n/a	5x6 = 3x 8	47-5-8 5-8-4 PLATES MT20 Weight: 245	GRIP 244/19
Plate Offsets (X, LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0 LUMBER TOP CHORD 2 BOT CHORD 2	7.11-4 8-7.11-4 0-1 Y): [3:0-7-0,0-3 SPACING Plates Increa Lumber Incre * Rep Stress I Code FBC2	18 ix6 = 1-12 ix6 = 1-	23-8-12 8-10-7 CSI TC 0.65 BC 0.48 WB 0.54	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.22 -0.38 0.16	(loc) 2-18 16-17 12 Structu 4-3-9 o	I/defl >432 >999 n/a aral wood	5x6 = 3x 8	47-5-8 5-8-4 PLATES MT20 Weight: 245	GRIP 244/19 Ib

REACTIONS (lb/size) 2=66/0-3-8, 18=1625/0-3-8, 10=-248/0-3-8, 12=1807/0-3-8

Max Uplift 2=-384(load case 4), 18=-461(load case 6), 10=-303(load case 5),

12=-459(load case 7)

Max Grav 2=181(load case 10), 18=1625(load case 1), 12=1807(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

Max Horz 2=-179(load case 4)

TOP CHORD 1-2=0/33, 2-3=-262/885, 3-4=-948/398, 4-5=-1204/490, 5-6=-1491/592,

6-7=-1916/782, 7-8=-1533/787, 8-9=-50/237, 9-10=-693/1764, 10-11=0/33

BOT CHORD 2-18=-769/350, 17-18=-963/453, 16-17=-125/774, 15-16=-389/1658,

14-15=-645/1697, 13-14=0/142, 12-13=-183/112, 10-12=-1603/763

WEBS 3-18=-1268/634, 3-17=-476/1651, 4-17=-558/264, 4-16=-65/327, 5-16=-302/932,

6-16=-473/376, 6-15=-43/153, 7-15=-128/244, 7-14=-1231/604, 8-14=-730/1661,

8-13=-805/163, 9-13=-91/712, 9-12=-2195/1006

4-16

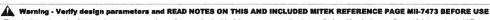
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

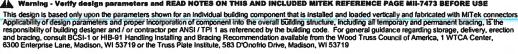
Brace must cover 90% of web length.

minimum end distance.

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Continued on page 2







Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T15	SPECIAL	1	1		J1884264
		0. 202			Job Reference (optional)	:

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:37 2007 Page 2

JOINT STRESS INDEX

2 = 0.24, 3 = 0.83, 4 = 0.40, 5 = 0.53, 6 = 0.40, 7 = 0.60, 8 = 0.54, 9 = 0.59, 10 = 0.44, 12 = 0.82, 13 = 0.29, 14 = 0.93, 15 = 0.37, 16 = 0.85, 17 = 0.83 and 18 = 0.67

NOTES

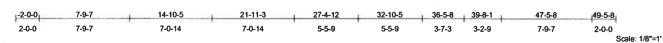
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 2, 461 lb uplift at joint 18, 303 lb uplift at joint 10 and 459 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plorida PE No. 3-1899 1199 Grassal Bay Blvd Govnton Besch, PL 88466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884265
L250968	T16	SPECIAL	1	1	
					Job Reference (optional)
Builders FirstSour	ce, Lake City, Fl 3	2055	6.300 s Feb 15 2006	MiTek I	ndustries, Inc. Fri Aug 24 09:39:38 2007 Page 1



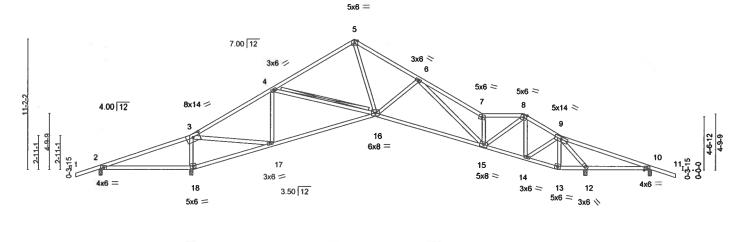


Plate Of	tsets (X,Y): [3:0-7-0,0-3-4]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.22	2-18	>432	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.50	Vert(TL)	-0.40	15-16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.15	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 247 lb	

23-8-12

8-10-7

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 4-2-2 oc purlins.

39-4-8 41-9-4

2-11-0 2-4-12

BOT CHORD

Rigid ceiling directly applied or 4-10-1 oc

bracing. T-Brace:

2 X 4 SYP No.3 -4-16

47-5-8

5-8-4

WEBS T

32-10-5

9-1-9

36-5-8

3-7-3

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=79/0-3-8, 18=1615/0-3-8, 10=-216/0-3-8, 12=1772/0-3-8

Max Horz 2=-179(load case 4)

Max Uplift 2=-384(load case 4), 18=-459(load case 6), 10=-295(load case 5),

12=-456(load case 7)

8-1-0

0-1-12

7-11-4

14-10-5

6-9-5

Max Grav 2=187(load case 10), 18=1615(load case 1), 12=1772(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-248/846, 3-4=-974/411, 4-5=-1228/493, 5-6=-1513/612,

6-7=-2105/1063, 7-8=-1688/804, 8-9=-742/416, 9-10=-677/1677, 10-11=0/33

BOT CHORD 2-18=-733/337, 17-18=-925/439, 16-17=-128/800, 15-16=-342/1606,

14-15=-110/627, 13-14=-158/120, 12-13=-152/111, 10-12=-1523/749

3-18=-1267/635, 3-17=-474/1637, 4-17=-559/260, 4-16=-57/321, 5-16=-332/988,

6-16=-415/355, 6-15=-369/412, 7-15=-1284/715, 8-15=-540/1373, 8-14=-590/174,

9-14=-254/829, 9-13=-24/70, 9-12=-2137/986

Julius Les Truss Design Engineer Florida Fis No. 3-1889 1199 Grastal Say Blvd Boviton Besch, FL 88486

August 24,2007

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T16	SPECIAL	1	1		J1884265
	770	Of EOI/IE		<u> </u>	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:39 2007 Page 2

JOINT STRESS INDEX

2 = 0.24, 3 = 0.82, 4 = 0.40, 5 = 0.48, 6 = 0.40, 7 = 0.62, 8 = 0.44, 9 = 0.52, 10 = 0.44, 12 = 0.80, 13 = 0.19, 14 = 0.50, 15 = 0.58, 16 = 0.83, 17 = 0.83 and 18 = 0.66

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 2, 459 lb uplift at joint 18, 295 lb uplift at joint 10 and 456 lb uplift at joint 12.

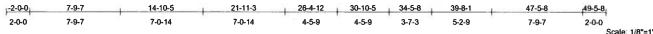
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Pionas PE No. 34000 1199 Chastel Buy Blvd Soviton Besch, FL 28448



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
		2	1.		J1884266
L250968	T17	SPECIAL	1	1	
					Job Reference (optional)

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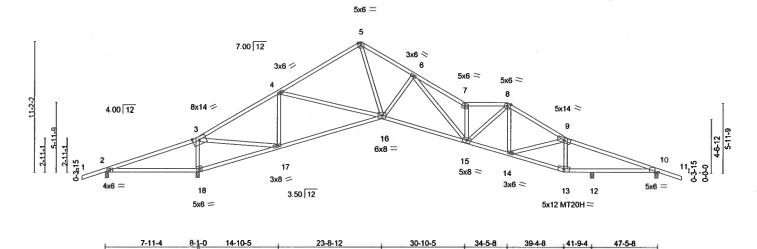


Plate Offsets (X,Y): [3:0-7-0,0-3-4], [10:0-3-3,Edge]

0-1-12

LOADIN	IC (==6)	CDACING	200	001		DEEL		(1)	N7-8	1.4	DIATEO	ODID
LOADIN	i G (psi)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.32	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.58	15-16	>692	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.21	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 246 lb	

8-10-7

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

2-18 2 X 4 SYP No.1D, 10-13 2 X 4 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or

4-11-0

3-3-8 oc purlins.

Rigid ceiling directly applied or 4-8-7 oc

2-4-12

5-8-4

bracing.

REACTIONS (lb/size) 2=-152/0-3-8, 18=2070/0-3-8, 10=862/0-3-8, 12=470/0-3-8

Max Horz 2=179(load case 5)

Max Uplift 2=-371(load case 4), 18=-517(load case 6), 10=-363(load case 7),

12=-169(load case 7)

Max Grav 2=77(load case 10), 18=2070(load case 1), 10=862(load case 1), 12=470(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-571/1551, 3-4=-900/378, 4-5=-1491/618, 5-6=-1869/800,

6-7=-3216/1558, 7-8=-2665/1246, 8-9=-2222/1046, 9-10=-1948/948, 10-11=0/33

BOT CHORD 2-18=-1399/642, 17-18=-1649/772, 16-17=-120/732, 15-16=-509/2060,

14-15=-653/1921, 13-14=-760/1776, 12-13=-765/1771, 10-12=-766/1772

3-18=-1517/749, 3-17=-766/2270, 4-17=-800/374, 4-16=-73/585, 5-16=-508/1342,

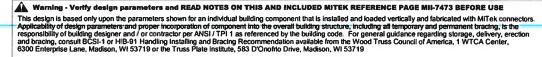
9-14=0/183, 9-13=-857/462

WEBS 6-16=-686/474, 6-15=-784/1258, 7-15=-1777/910, 8-15=-407/1115, 8-14=-88/105,

JOINT STRESS INDEX

2 = 0.38, 3 = 0.91, 4 = 0.40, 5 = 0.46, 6 = 0.82, 7 = 0.79, 8 = 0.44, 9 = 0.73, 10 = 0.72, 13 = 0.84, 14 = 0.37, 15 = 0.68, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 16 = 0.82, 17 = 0.84, 18 = 00.74, 17 = 0.95 and 18 = 0.86

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884266
L250968	T17	SPECIAL	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:40 2007 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2, 517 lb uplift at joint 18, 363 lb uplift at joint 10 and 169 lb uplift at joint 12.

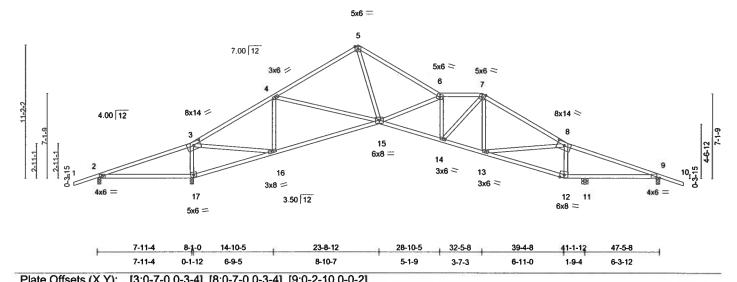
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884267
L250968	T18	SPECIAL	1	1	
		<u> </u>			Job Reference (optional)

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T late of	10010 [71, 1	j. [0.0-7-0,0-0-4], [0.	0 1 0,0 0	1, [0.0	2 10,0 0							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.28	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.84	Vert(TL)	-0.53	12-13	>750	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.20	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	` ′					Weight: 242 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

2-17 2 X 4 SYP No.1D, 9-12 2 X 4 SYP No.1D

WEBS

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

3-9-14 oc purlins.

Rigid ceiling directly applied or 5-1-2 oc

bracing.

REACTIONS (lb/size) 2=-78/0-3-8, 17=1962/0-3-8, 9=820/0-3-8, 11=546/0-6-7

Max Horz 2=179(load case 5)

Max Uplift 2=-381(load case 4), 17=-495(load case 6), 9=-350(load case 7),

11=-209(load case 7)

Max Grav 2=112(load case 10), 17=1962(load case 1), 9=820(load case 1),

11=546(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-445/1323, 3-4=-984/420, 4-5=-1505/626, 5-6=-1899/762,

6-7=-2302/1072, 7-8=-2109/932, 8-9=-1800/841, 9-10=0/33

BOT CHORD 2-17=-1184/523, 16-17=-1419/644, 15-16=-130/806, 14-15=-750/2443,

13-14=-528/1795, 12-13=-652/1627, 11-12=-667/1635, 9-11=-668/1636

3-17=-1475/724, 3-16=-678/2120, 4-16=-740/340, 4-15=-74/541, 5-15=-437/1267,

6-15=-847/567, 6-14=-824/318, 7-14=-291/850, 7-13=-51/171, 8-13=0/197,

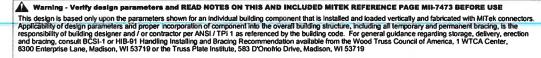
8-12=-872/488

JOINT STRESS INDEX

2 = 0.32, 3 = 0.85, 4 = 0.40, 5 = 0.63, 6 = 0.58, 7 = 0.57, 8 = 0.65, 9 = 0.78, 12 = 0.93, 13 = 0.37, 14 = 0.62, 15 = 0.71, 16 = 0.89 and 17 = 0.81

Continued on page 2

August 24,2007





lius Les Jes Cesian Engineer Mas Et No. 3-1885 80 Cesta No. 3-1885

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
		000000			J1884267
L250968	T18	SPECIAL	1	1	
					Job Reference (optional)

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 381 lb uplift at joint 2, 495 lb uplift at joint 17, 350 lb uplift at joint 9 and 209 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee Truss Cosion Engineer Fiction FE No. 31808 1195 Cassid Bay Sivil Boynton Beson, FL 55466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T19	SPECIAL	1	1		J1884268
				1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:42 2007 Page 1



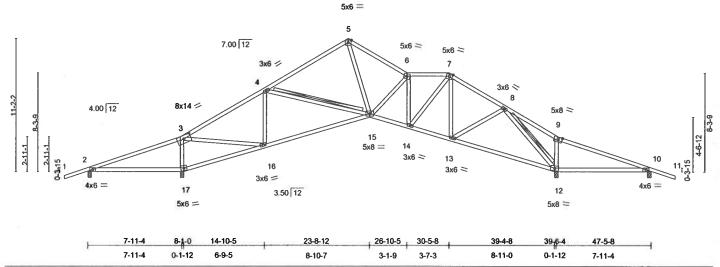


Plate Offsets	(X.Y):	[3:0-7-0	.0-3-41

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defi	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.22	10-12	>417	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.31	15-16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.13	12	n/a	n/a		
BCDL	BCDL 5.0 Code FBC2004/TPI2002		(Mat	rix)						Weight: 250 lb		

		84	8	_	_
_	u	m	-	_	ĸ

WEBS

LOMDEN	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-11-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

4-15, 8-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=161/0-3-8, 17=1472/0-3-8, 12=1454/0-3-8, 10=164/0-3-8

Max Horz 2=179(load case 5)

Max Uplift 2=-391(load case 4), 17=-437(load case 6), 12=-421(load case 7),

10=-319(load case 5)

Max Grav 2=227(load case 10), 17=1472(load case 1), 12=1454(load case 1), 10=206(load case 11)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-144/596, 3-4=-1034/439, 4-5=-1177/474, 5-6=-1419/581,

6-7=-1336/625, 7-8=-1103/498, 8-9=-43/625, 9-10=-176/587, 10-11=0/33

BOT CHORD 2-17=-497/222, 16-17=-670/316, 15-16=-134/853, 14-15=-221/1426, 13-14=-79/947 +

, 12-13=-51/569, 10-12=-490/270 3-17=-1198/601, 3-16=-379/1444, 4-16=-479/226, 4-15=-59/232, 5-15=-299/912,

6-15=-323/280, 6-14=-675/203, 7-14=-226/690, 7-13=-265/103, 8-13=-38/428,

6-15=-323/280, 6-14=-075/203, 7-14=-226/690, 7-13=-265/103, 8-13=-38/428, Continued on page 12=-1619/524, 9-12=-282/324

August 24,2007

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation authority Mood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
			١.		J1884268
L250968	T19	SPECIAL	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:42 2007 Page 2

JOINT STRESS INDEX

2 = 0.22, 3 = 0.83, 4 = 0.40, 5 = 0.49, 6 = 0.35, 7 = 0.32, 8 = 0.53, 9 = 0.59, 10 = 0.26, 12 = 0.59, 13 = 0.37, 14 = 0.53, 15 = 0.92, 16 = 0.73 and 17 = 0.60

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2, 437 lb uplift at joint 17, 421 lb uplift at joint 12 and 319 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truss Cesian Engineer Plonas PE No. 24569 1 109 Cesstel Bay Blvn Boynton Besch, FL 88438



Job	Truss	Truss Ty	ре	Qty	Ply	WOODMAN	PARK		14004000
250968	T20	SPECIA	-	1	1	Job Reference	a (antiona	ın.	J1884269
Builders FirstSour	ce, Lake City, FI	32055	6.3	300 s Feb 15 200					Page 1
-2-0-0	7-9-7	14-10-5	21-11-3	24-10-5 28-5-8	34-0-13			47-5-8 4	9-5-8
2-0-0	7- 9 -7	7-0-14	7-0-14	2-11-3 3-7-3	5-7-5	5-7-5	•	7-9-7 2	2-0-0 Scale = 1:93.5
				5x6 =					
		_		5 5x6 = 5	ix6 <				
t		7.00 12	. //	6	7				*
		3	x6 //			3x6 <>			
	_					8			
g 4.0	00 12	8x14 =					8x14 ≫		- S
1 1		3		15			9		
211 2			40		4 x8 ≈			11	5 46-12
4x6	_		16 6 ==			13 3x6 ≈		4x6 =	2,12,1
47.0		17 5x6 =	3.50 12				12 5x6 ≃	420 -	_
⊢		8-1-0 14-10-5	23-8-12	28-5-8	34-0-13	- 2001		47-5-8	
late Offsets (X,)		0-1-12 6-9-5 3-4], [9:0-7-0,0-3-	8-10-7 4 1	4-8-12	5-7-5	5-3-11	0-1-12	7-11-4	
iato Onocio (X,	<u> </u>								
	SPACING	2-0-0	CSI TC 0.46	DEFL Vert(LL)	in (ld 0.22 2-	oc) I/defl 17 >426	L/d 360	PLATES MT20	GRIP 244/19
OADING (psf)		1 25 مودو		V GIULL/				WITZU	277/13
CLL 20.0 CDL 7.0	Plates Incre Lumber Inc	rease 1.25	BC 0.44		-0.32 15-	16 >999	240		
	Plates Incre Lumber Inc * Rep Stress	rease 1.25				16 >999 12 n/a	n/a	Weight: 253	lb
CLL 20.0 CDL 7.0 CLL 10.0 CDL 5.0	Plates Incre Lumber Inc * Rep Stress Code FBC	rease 1.25 Incr YES	BC 0.44 WB 0.48	Vert(TL) Horz(TL)	0.10	12 n/a	n/a		
CLL 20.0 CDL 7.0 CLL 10.0 CDL 5.0	Plates Incre Lumber Inc * Rep Stress Code FBC	rease 1.25 Incr YES	BC 0.44 WB 0.48	Vert(TL) Horz(TL)	0.10 D Str	12 n/a	n/a sheathir	Weight: 253	

Brace must cover 90% of web length.

WEBS

bracing.

T-Brace:

minimum end distance.

REACTIONS (lb/size) 2=180/0-3-8, 17=1436/0-3-8, 12=1489/0-3-8, 10=145/0-3-8

Max Horz 2=179(load case 5)

Max Uplift 2=-391(load case 4), 17=-432(load case 6), 12=-423(load case 7),

10=-330(load case 5)

Max Grav 2=238(load case 10), 17=1436(load case 1), 12=1489(load case 1),

10=191(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-134/538, 3-4=-1045/447, 4-5=-1158/473, 5-6=-1345/589,

6-7=-926/513, 7-8=-1139/521, 8-9=-802/371, 9-10=-149/654, 10-11=0/33

BOT CHORD 2-17=-442/201, 16-17=-610/294, 15-16=-137/862, 14-15=-93/1278, 13-14=-54/651,

12-13=-729/340, 10-12=-553/245

3-17=-1179/596, 3-16=-365/1397, 4-16=-459/221, 4-15=-68/205, 5-15=-313/900,

6-15=-316/257, 6-14=-582/88, 7-14=-49/274, 8-14=-92/334, 8-13=-591/254,

Continued on page 13=-362/1328, 9-12=-1195/588

WEBS

August 24,2007

2 X 4 SYP No.3 -

ullus Les russ Design Engineer loride PE No. 34 ses 198 Crestel Say Slvd toynton Gesch. PL 66466

4-15

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

itors. Bu Firs

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T20	SPECIAL	1	4		J1884269
L230900	120	SPECIAL	'	6	Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.21, 3 = 0.83, 4 = 0.40, 5 = 0.47, 6 = 0.31, 7 = 0.43, 8 = 0.40, 9 = 0.73, 10 = 0.24, 12 = 0.63, 13 = 0.68, 14 = 0.63, 15 = 0.84, 16 = 0.71 and 17 = 0.59

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2, 432 lb uplift at joint 17, 423 lb uplift at joint 12 and 330 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plorias PE No. 3-1888 1499 Chastal Bay Blvd Bovnton Wesch, Nr. 56446



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T21	SPECIAL	1	1	J1884270
12200000		0. 202			Job Reference (optional)

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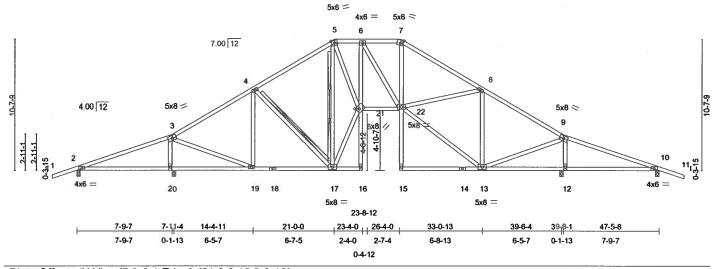


Plate Of	fsets (X,Y	'): [7:0-2-1,Edge], [2	1:0-2-12,0	-2-12]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.18	2-20	>490	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.31	Vert(TL)	-0.21	16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.19	12	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)						Weight: 306 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-16 2 X 4 SYP No.3, 7-15 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins, except

2-0-0 oc purlins (5-6-4 max.): 5-7.

Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -

4-17, 5-17

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=155/0-3-8, 20=1447/0-3-8, 12=1492/0-3-8, 10=124/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-370(load case 4), 20=-422(load case 6), 12=-373(load case 7),

10=-314(load case 5)

Max Grav 2=198(load case 10), 20=1447(load case 1), 12=1492(load case 1), 10=169(load case 11)

Julius Les Truse Design Engineer Plantas PE No. 3-1209 1100 Geestal Say Sivri Bovnton Wesch, FL 25466

August 24,2007

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J188427	70
L250968	T21	SPECIAL	1	1		
					Job Reference (optional)	

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-122/607, 3-4=-798/487, 4-5=-801/579, 5-6=-1136/603, 6-7=-1162/613, 7-8=-1441/637,

8-9=-770/483, 9-10=-109/704, 10-11=0/33

BOT CHORD 2-20=-508/189, 19-20=-433/151, 18-19=-135/607, 17-18=-135/607, 16-17=-1/13, 16-21=0/6, 6-21=-169/167,

15-22=0/93, 7-22=-87/357, 14-15=0/18, 13-14=0/18, 12-13=-521/168, 10-12=-599/206

WEBS 3-20=-1347/679, 3-19=-307/1118, 4-19=-360/189, 4-17=-92/121, 5-17=-1242/40, 17-21=-115/1431,

5-21=-81/1355, 6-22=-148/159, 13-22=-167/720, 8-22=-66/598, 8-13=-817/296, 9-13=-322/1187, 9-12=-1392/690,

21-22=-84/1149

JOINT STRESS INDEX

2 = 0.25, 3 = 0.82, 4 = 0.40, 5 = 0.63, 6 = 0.30, 7 = 0.61, 8 = 0.40, 9 = 0.83, 10 = 0.25, 12 = 0.22, 13 = 0.53, 14 = 0.15, 15 = 0.23, 16 = 0.15, 17 = 0.75, 18 = 0.23, 19 = 0.61, 20 = 0.21, 21 = 0.97 and 22 = 0.53

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2, 422 lb uplift at joint 20, 373 lb uplift at joint 12 and 314 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truse Cesign Engineer Flonda FE No. 3-1869 1-190 Cassisi Bay Bivd Goynton Beach, FL 55-196



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
						J1884271
L250968	T22	SPECIAL	1	1		
					Job Reference (optional)	

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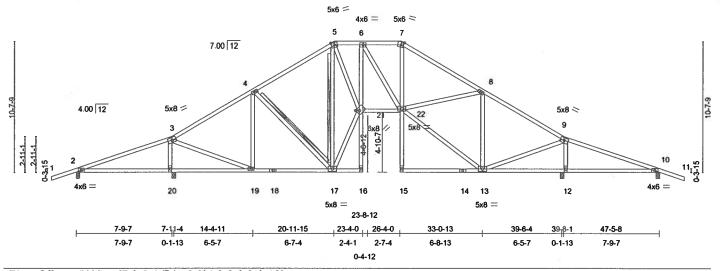


Plate Offs	sets (X,Y): [7:0-2-1,Edge], [21	1:0-2-8,0-2	2-12]		1					T	
LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.18	2-20	>490	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.21	16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.19	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 306 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-16 2 X 4 SYP No.3, 7-15 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or

5-0-0 oc purlins, except

2-0-0 oc purlins (5-6-3 max.): 5-7.

Rigid ceiling directly applied or 6-0-0 oc

rigid ceiling directly applied or o

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -4-17, 5-17

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=156/0-3-8, 20=1447/0-3-8, 12=1491/0-3-8, 10=124/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-370(load case 4), 20=-422(load case 6), 12=-373(load case 7),

10=-314(load case 5)

Max Grav 2=198(load case 10), 20=1447(load case 1), 12=1491(load case 1), 10=169(load case 11)

Julius Lew Truse Cesign Engineer Plánida ME No. 34868 1992 Coesial Sey Blyd Goynton Gesch, FL 55456

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884271
L250968	T22	SPECIAL	1	1	
					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-122/606, 3-4=-798/486, 4-5=-801/579, 5-6=-1137/603, 6-7=-1162/613, 7-8=-1441/637,

8-9=-770/483, 9-10=-109/703, 10-11=0/33

BOT CHORD 2-20=-507/189, 19-20=-432/151, 18-19=-135/607, 17-18=-135/607, 16-17=-1/13, 16-21=0/6, 6-21=-170/167,

15-22=0/93, 7-22=-87/357, 14-15=0/18, 13-14=0/18, 12-13=-521/168, 10-12=-599/207

WEBS 3-20=-1347/679, 3-19=-308/1118, 4-19=-360/189, 4-17=-92/121, 5-17=-1241/40, 17-21=-115/1430,

5-21=-82/1355, 6-22=-148/159, 13-22=-167/720, 8-22=-66/598, 8-13=-817/296, 9-13=-322/1186, 9-12=-1392/690,

21-22=-84/1150

JOINT STRESS INDEX

2 = 0.25, 3 = 0.82, 4 = 0.40, 5 = 0.63, 6 = 0.30, 7 = 0.61, 8 = 0.40, 9 = 0.83, 10 = 0.25, 12 = 0.22, 13 = 0.53, 14 = 0.15, 15 = 0.23, 16 = 0.15, 17 = 0.75, 18 = 0.23, 19 = 0.61, 20 = 0.21, 21 = 0.92 and 22 = 0.53

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2, 422 lb uplift at joint 20, 373 lb uplift at joint 12 and 314 lb uplift at joint 10.

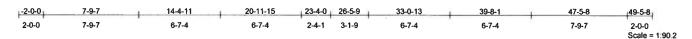
LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1 250968	T23	SPECIAL	2	4	J1884272
L230900	125	SPECIAL	_	'	Job Reference (optional)

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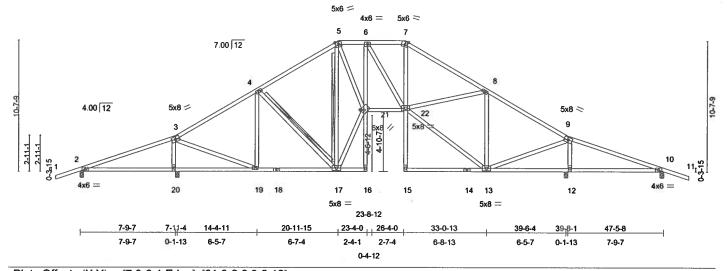


Plate Of	tsets (X,Y	'): [7:0-2-1,Edge], [2'	1:0-2-8,0-2	2-12]						****		
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	тс	0.47	Vert(LL)	0.18	2-20	>490	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.31	Vert(TL)	-0.21	16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.19	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 306 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-16 2 X 4 SYP No.3, 7-15 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

BRACING

BOT CHORD

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins, except

2-0-0 oc purlins (5-6-3 max.): 5-7.

Rigid ceiling directly applied or 6-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

4-17, 5-17

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=156/0-3-8, 20=1447/0-3-8, 12=1491/0-3-8, 10=124/0-3-8

Max Horz 2=164(load case 5)

Max Uplift 2=-370(load case 4), 20=-422(load case 6), 12=-373(load case 7),

10=-314(load case 5)

Max Grav 2=198(load case 10), 20=1447(load case 1), 12=1491(load case 1), 10=169(load case 11)

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
				'	J1884272
L250968	T23	SPECIAL	2	1	
					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-122/606, 3-4=-798/486, 4-5=-801/579, 5-6=-1137/603, 6-7=-1162/613, 7-8=-1441/637,

8-9=-770/483, 9-10=-109/703, 10-11=0/33

BOT CHORD 2-20=-507/189, 19-20=-432/151, 18-19=-135/607, 17-18=-135/607, 16-17=-1/13, 16-21=0/6, 6-21=-170/167,

15-22=0/93, 7-22=-87/357, 14-15=0/18, 13-14=0/18, 12-13=-521/168, 10-12=-599/207

WEBS 3-20=-1347/679, 3-19=-308/1118, 4-19=-360/189, 4-17=-92/121, 5-17=-1241/40, 17-21=-115/1430,

5-21=-82/1355, 6-22=-148/159, 13-22=-167/720, 8-22=-66/598, 8-13=-817/296, 9-13=-322/1186, 9-12=-1392/690,

21-22=-84/1150

JOINT STRESS INDEX

2 = 0.25, 3 = 0.82, 4 = 0.40, 5 = 0.63, 6 = 0.30, 7 = 0.61, 8 = 0.40, 9 = 0.83, 10 = 0.25, 12 = 0.22, 13 = 0.53, 14 = 0.15, 15 = 0.23, 16 = 0.15, 17 = 0.75, 18 = 0.23, 19 = 0.61, 20 = 0.21, 21 = 0.92 and 22 = 0.53

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 2, 422 lb uplift at joint 20, 373 lb uplift at joint 12 and 314 lb uplift at joint 10.

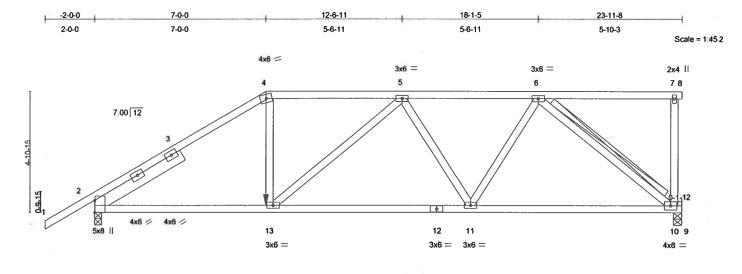
LOAD CASE(S) Standard

Julius Les Truss Cesion Engineer Plonas Es No. 2-1209 1-199 Cassial Bay Sival Bovinton Besch, FL 55-456



I	Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
	. 050000	TOE	MONOLUB			J1884273
	L250968	T25	MONO HIP	1	1	Joh Reference (entional)
d			L		L	Job Reference (optional)

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7-0-0	15-4-0	23-11-8
7-0-0	8-4-0	8-7-8
Dist. 0% -1- (V)0 - (0.0 5 44 5 t)		

Plate Off	sets (X,Y): [2:0-5-14,Edge]		T		T					T	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	тс	0.75	Vert(LL)	-0.14	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.80	Vert(TL)	-0.37	10-11	>766	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	, ,					Weight: 135 lb	

LU	MB	ER
----	----	----

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 6 SYP No.1D 4-1-13

BRACING

BOT CHORD

WEBS

TOP CHORD

Structural wood sheathing directly applied or

3-2-15 oc purlins.

Rigid ceiling directly applied or 6-2-0 oc

bracing.

T-Brace:

2 X 4 SYP No.3 -

6-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1597/0-3-8, 10=1690/0-4-0

Max Horz 2=182(load case 5)

Max Uplift 2=-627(load case 5), 10=-765(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-2392/955, 3-4=-2301/971, 4-5=-1954/862, 5-6=-2011/849, 6-7=0/0,

7-8=0/0

BOT CHORD 2-13=-853/1929, 12-13=-1019/2263, 11-12=-1019/2263, 10-11=-759/1631,

WEBS 4-13=-350/777, 5-13=-406/326, 5-11=-488/339, 6-11=-194/737, 6-10=-2121/987,

7-10=-262/177

JOINT STRESS INDEX

2 = 0.73, 2 = 0.43, 3 = 0.00, 4 = 0.54, 5 = 0.42, 6 = 0.62, 7 = 0.33, 10 = 0.46, 11 = 0.55, 12 = 0.80 and 13 = 0.49August 24,2007 Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.

Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-10 or III-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884273
L250968	T25	MONO HIP	1	1	
					Job Reference (optional)

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NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 2 and 765 lb uplift at joint 10.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-118(F=-64), 2-13=-10, 10-13=-22(F=-12)

Concentrated Loads (lb)

Vert: 13=-411(F)

Julius Les Trues Occion Engineer Piòrida PE No. 34500 1100 Casstel Bay Blvd Govnion Gesch. PL 85466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
						J1884274
L250968	T26	MONO HIP	1	1		
					Job Reference (optional)	

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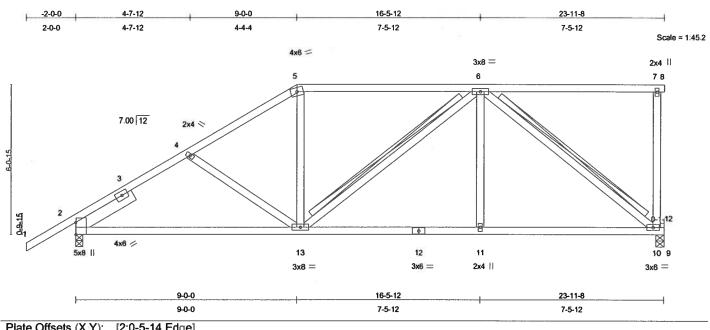


Plate Of	isets (X,Y): [2:0-5-14,Edge]									Y	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.10	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.40	Vert(TL)	-0.19	2-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 142 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D

Left 2 X 6 SYP No.1D 2-8-8

BRACING

TOP CHORD Structural wood sheathing directly applied or

5-11-2 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

> bracing, Except: 8-6-9 oc bracing: 2-13.

WEBS T-Brace: 2 X 4 SYP No.3 -

6-13, 6-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=870/0-3-8, 10=760/0-4-0

Max Horz 2=219(load case 6)

Max Uplift 2=-221(load case 6), 10=-211(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1101/460, 3-4=-1025/476, 4-5=-906/439, 5-6=-760/439, 6-7=0/0,

2-13=-546/840, 12-13=-392/711, 11-12=-392/711, 10-11=-392/711, 9-10=0/0

WEBS 4-13=-99/129, 5-13=0/221, 6-13=-68/102, 6-11=0/237, 6-10=-913/504,

7-10=-155/106

JOINT STRESS INDEX

2 = 0.62, 2 = 0.40, 3 = 0.00, 4 = 0.33, 5 = 0.66, 6 = 0.56, 7 = 0.33, 10 = 0.34, 11 = 0.33, 12 = 0.25 and 13 = 0.56

Continued on page 2

BOT CHORD



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
					J1884	274
L250968	T26	MONO HIP	1	1		
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 211 lb uplift at joint 10.

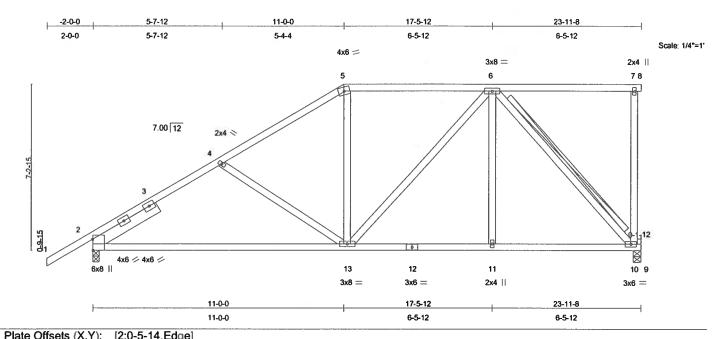
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plorida PE No. 34289 1109 Chestal Bay Blvd Soviton Beach, PL 88498



- 1	Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
			≅			J1884275
	L250968	T27	MONO HIP	1	1	
						Job Reference (optional)

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1 late Of	113613 (7, 1). [2.0-5-14,Luge]		1		1						
LOADIN	NG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.23	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.42	2-13	>683	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.03	10	n/a	n/a	:	
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	` ′					Weight: 151 lb	
											_	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2

WEBS

2 X 4 SYP No.2 2 X 4 SYP No.3

SLIDER Left 2 X 6 SYP

Left 2 X 6 SYP No.1D 3-3-7

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

8-3-13 oc bracing: 2-13.

WEBS

T-Brace:

2 X 4 SYP No.3 -

6-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=870/0-3-8, 10=760/0-4-0

Max Horz 2=256(load case 6)

Max Uplift 2=-225(load case 6), 10=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1076/431, 3-4=-1006/452, 4-5=-836/382, 5-6=-666/393, 6-7=0/0,

7-8=0/0

BOT CHORD 2-13=-571/840, 12-13=-302/530, 11-12=-302/530, 10-11=-302/530, 9-10=0/0

WEBS 4-13=-211/212, 5-13=0/196, 6-13=-136/202, 6-11=0/179, 6-10=-797/454,

7-10=-135/92

JOINT STRESS INDEX

2 = 0.65, 2 = 0.20, 2 = 0.20, 3 = 0.00, 4 = 0.33, 5 = 0.63, 6 = 0.56, 7 = 0.33, 10 = 0.37, 11 = 0.33, 12 = 0.17 and 13 = 0.56 August 24,2007 Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 8300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 833 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1.050000	T07	AAONO LUD			J1884275
L250968	T27	MONO HIP	11	1	
L					Job Reference (optional)

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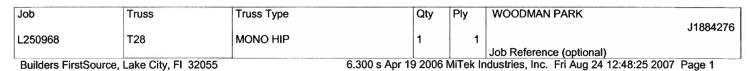
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 2 and 210 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Plande PE No. 34509 1400 Castal Bay Blyd Boynton Besch, FL, 180436





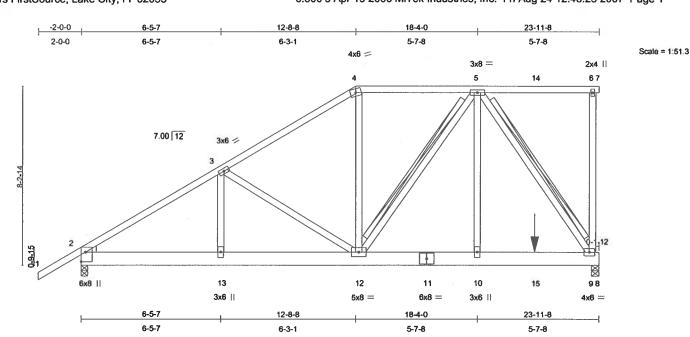


Plate Of	Plate Offsets (X,Y): [2:Edge,0-2-5]											
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.05	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.43	Vert(TL)	-0.09	9-10	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 192 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS

2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 5-2-8

oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 5-12,

5-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS

(lb/size) 2=1012/0-3-8, 9=1988/0-4-0

Max Horz 2=291(load case 5)

Max Uplift 2=-270(load case 5), 9=-593(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

BOT CHORD

1-2=0/41, 2-3=-1373/233, 3-4=-1004/215, 4-5=-794/223, 5-14=0/0, 6-14=0/0, 6-7=0/0 2-13=-367/1073, 12-13=-367/1073, 11-12=-222/805, 10-11=-222/805, 10-15=-222/805,

9-15=-222/805, 8-9=0/0

WEBS

3-13=0/209, 3-12=-336/170, 4-12=-18/246, 5-12=-45/190, 5-10=-232/792, 5-9=-1426/39

6-9=-287/122

JOINT STRESS INDEX

2 = 0.65, 2 = 0.00, 3 = 0.42, 4 = 0.52, 5 = 0.74, 6 = 0.34, 9 = 0.37, 10 = 0.26, 11 = 0.23, 12 = 0.28 and 13 = 0.16

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

2) Provide ad a guate drainage to prevent water ponding.

August 24,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult 8CSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



1	Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1	1.050000	T24	MONO LUD			J1884279
	L250968	T31	MONO HIP]1	1	Job Reference (optional)
ı			L			

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NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 545 lb uplift at joint 10.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-54, 5-14=-54, 7-14=-134(F=-80), 7-8=-80(F), 2-15=-10, 10-15=-25(F=-15), 9-10=-15(F)

Concentrated Loads (lb)

Vert: 15=-1098(F)

Julius Les Truss Design Engineer Florida PE No. 3-1869 1-199 Crestal Bay Blod 1-199 Crestal Bay Blod



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
1.050000	T00	MONOLUD			,	J1884280
L250968	T32	MONO HIP	1	1		
				İ	Job Reference (optional)	

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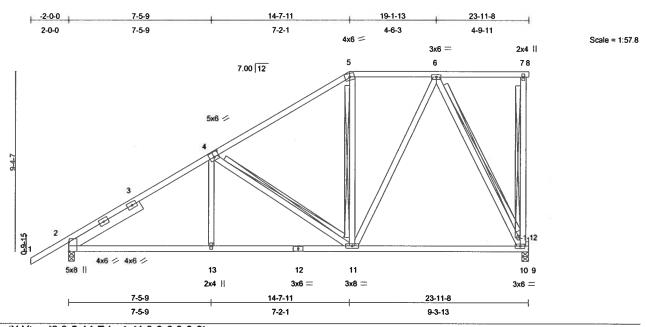


Plate Of	fsets (X,Y	'): [2:0-5-14,Edge], [4	4:0-3-0,0-3	3-0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.18	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.53	Vert(TL)	-0.33	10-11	>870	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.03	10	n/a	n/a	27	
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 163 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D 4-4-

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-3-8 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -4-11, 5-11, 6-10, 7-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=870/0-3-8, 10=760/0-4-0

Max Horz 2=324(load case 6)

Max Uplift 2=-222(load case 6), 10=-208(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1093/335, 3-4=-894/354, 4-5=-661/264, 5-6=-486/300, 6-7=0/0,

7-8=0/0

BOT CHORD 2-13=-565/826, 12-13=-566/825, 11-12=-566/825, 10-11=-193/306, 9-10=0/0

WEBS 4-13=0/199, 4-11=-409/319, 5-11=-50/112, 6-11=-247/415, 6-10=-686/434,

7-10=-106/72

Julius Les Trues Cesign Engineer Ponda PE No. 24988 1190 Chastel Bay Blvd Boynton Besch, FL 68465

JOINT STRESS INDEX

2 = 0.59, 2 = 0.20, 2 = 0.20, 3 = 0.00, 4 = 0.65, 5 = 0.71, 6 = 0.45, 7 = 0.33, 10 = 0.45, 11 = 0.61, 12 = 0.27 and 13 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T32	MONO HIP	4	4	J1884280
L250900	132	WONO HIP	'	'	Job Reference (optional)

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2 and 208 lb uplift at joint 10.

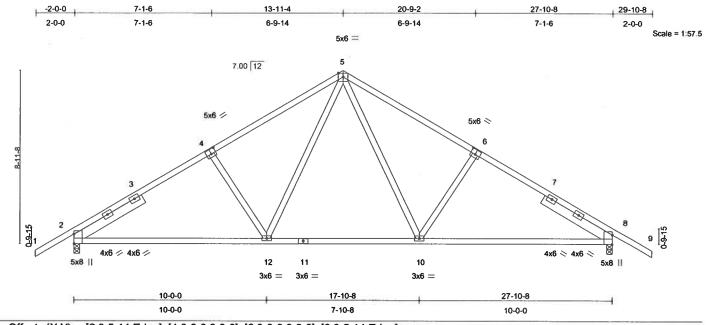
LOAD CASE(S) Standard

Julius Les Truse Ceston Engineer Philida Pit No. 34166 1100 Chastel Bay Blvd Boynton Esson, Pt. 55456





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Flate Of	fsets (X,Y)	: [2:0-5-14,Edge], [4:	0-5-0,0-5-0	η, <u>τ</u> υ.υ - υ-	0,0-5-0],	0.0-5-14,Euge	1					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.29 1	0-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.76	Vert(TL)	-0.53 1	0-12	>637	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mat	rix)	` ´					Weight: 159 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 6 SYP No.1D 4-1-12,

Right 2 X 6 SYP No.1D 4-1-12

BRACING

TOP CHORD

Structural wood sheathing directly applied or 4-5-12

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-8-5 oc bracing.

REACTIONS (lb/size) 2=1236/0-3-8, 8=1236/0-3-8

Max Horz 2=-239(load case 4)

Max Uplift 2=-342(load case 6), 8=-342(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1771/769, 3-4=-1661/797, 4-5=-1554/816, 5-6=-1554/816, 6-7=-1661/797,

7-8=-1771/769, 8-9=0/34

BOT CHORD 2-12=-498/1389, 11-12=-242/1030, 10-11=-242/1030, 8-10=-498/1389

WEBS 4-12=-251/244, 5-12=-300/614, 5-10=-300/614, 6-10=-251/244

JOINT STRESS INDEX

2 = 0.71, 2 = 0.33, 2 = 0.33, 3 = 0.00, 4 = 0.47, 5 = 0.65, 6 = 0.47, 7 = 0.00, 8 = 0.71, 8 = 0.33, 8 = 0.33, 10 = 0.53, 11 = 0.67 and 12 = 0.670.53

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live Confinued on page 2

August 24,2007

A Warning - Verify design para and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occie. For general guidance regarding storage, delivery, erection and bracing, consult 8CSI-1 or HIB-91 Handing Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	T35	COMMON	3	1	J1884	4281
					Job Reference (optional)	

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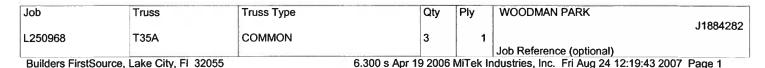
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 2 and 342 lb uplift at joint
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

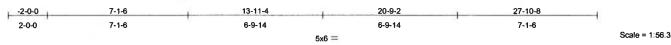
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-9=-54, 2-12=-10, 10-12=-70(F=-60), 8-10=-10







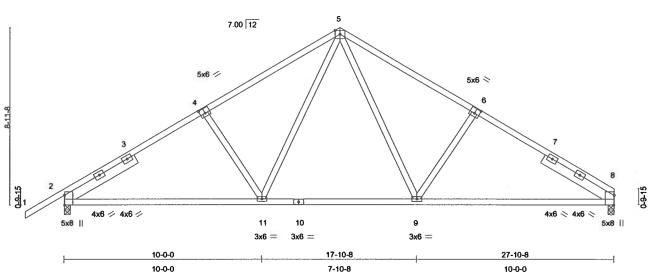


Plate Offsets (X,Y): [2:0-5-14,Edge], [2:0-0-0,0-0-0], [2:0-0-0,0-0-0], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-5-14,Edge], [8:0-0-0,0-0-0], [8:0-0-0,0-0-0]

		_	,0 0 0		_		1						
LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.42	Vert(LL)	0.29	9-11	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.52	9-11	>642	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 156 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

SLIDER Left 2 X 6 SYP No.1D 4-1-12, Right 2 X 6 SYP No.1D 4-1-12 **BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-5-9

oc purlins.

BOT CHORD Rigid ceiling directly applied or 8-2-0 oc bracing.

REACTIONS (lb/size) 2=1240/0-3-8, 8=1124/0-3-8

Max Horz 2=252(load case 5)

Max Uplift 2=-343(load case 6), 8=-255(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1778/778, 3-4=-1668/806, 4-5=-1561/825, 5-6=-1570/836, 6-7=-1678/817,

7-8=-1764/796

BOT CHORD 2-11=-548/1395, 10-11=-293/1036, 9-10=-293/1036, 8-9=-566/1410

WEBS 4-11=-251/244, 5-11=-299/614, 5-9=-319/630, 6-9=-265/261

JOINT STRESS INDEX

2 = 0.71, 2 = 0.33, 2 = 0.33, 3 = 0.00, 4 = 0.52, 5 = 0.64, 6 = 0.52, 7 = 0.00, 8 = 0.71, 8 = 0.33, 8 = 0.33, 9 = 0.54, 10 = 0.68 and 11 = 0.680.54

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T35A	COMMON	3	1	J18842
				<u></u>	Job Reference (optional)

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- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 2 and 255 lb uplift at joint
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-8=-54, 2-11=-10, 9-11=-70(F=-60), 8-9=-10



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
1.050000	T050	CARLE			J1884283
L250968	T35G	GABLE	1	1	
					Job Reference (optional)

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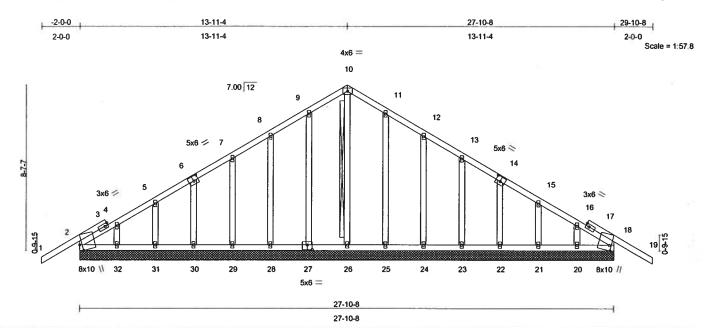


Plate Of	fsets (X,Y)	: [2:0-4-1,Edge], [6:	0-3-0,0-3-	0], [14:0	0-3-0,0-3	-0], [18:0-3-5,	Edge], [27:0-3-	0,0-3-0]		1	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	19	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.07	19	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.01	18	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 185 lb	

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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **OTHERS** 2 X 4 SYP No.3

Continued on page 2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

BOT CHORD WEBS

T-Brace:

2 X 4 SYP No.3 -

10-26

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=480/27-10-8, 18=482/27-10-8, 26=218/27-10-8, 27=248/27-10-8,

28=246/27-10-8, 29=256/27-10-8, 30=240/27-10-8, 31=265/27-10-8, 32=111/27-10-8, 25=247/27-10-8, 24=247/27-10-8, 23=255/27-10-8,

22=240/27-10-8, 21=265/27-10-8, 20=111/27-10-8

Max Horz 2=292(load case 5)

Max Uplift 2=-168(load case 4), 18=-199(load case 7), 26=-3(load case 5),

27=-116(load case 6), 28=-132(load case 6), 29=-131(load case 6), 30=-123(load case 6), 31=-139(load case 6), 32=-68(load case 5), 25=-112(load case 7), 24=-133(load case 7), 23=-131(load case 7),

22=-123(load case 7), 21=-139(load case 7), 20=-57(load case 4)

Max Grav 2=480(load case 1), 18=482(load case 1), 26=218(load case 1), 27=253(load case 10), 28=246(load case 1), 29=256(load case 10),

30=240(load case 10), 31=265(load case 1), 32=112(load case 10),

25=251(load case 11), 24=247(load case 1), 23=255(load case 11),

22=240(load case 11), 21=265(load case 1), 20=112(load case 11)

Engineer

August 24,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure; including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI/TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T35G	GABLE	1	1	J188428
					Job Reference (optional)

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FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-12/71, 2-3=-240/200, 3-4=-241/196, 4-5=-201/174, 5-6=-160/170, 6-7=-120/165, 7-8=-87/172, 8-9=-87/218,

9-10=-89/257, 10-11=-89/257, 11-12=-89/207, 12-13=-90/145, 13-14=-91/86, 14-15=-93/54, 15-16=-89/58,

16-17=-120/75, 17-18=-150/79, 18-19=-12/71

BOT CHORD 2-32=-47/185, 31-32=-47/185, 30-31=-47/185, 29-30=-45/187, 28-29=-45/187, 27-28=-45/187, 26-27=-46/186.

25-26=-46/186, 24-25=-46/186, 23-24=-46/186, 22-23=-46/186, 21-22=-45/183, 20-21=-45/183, 18-20=-45/183

WEBS 10-26=-197/16, 9-27=-232/137, 8-28=-227/160, 7-29=-235/158, 6-30=-222/151, 5-31=-236/161, 4-32=-126/85.

11-25=-231/137, 12-24=-227/160, 13-23=-235/158, 14-22=-222/151, 15-21=-237/161, 16-20=-126/81

JOINT STRESS INDEX

2 = 0.53, 3 = 0.00, 3 = 0.00, 3 = 0.26, 4 = 0.33, 5 = 0.33, 6 = 0.18, 7 = 0.33, 8 = 0.33, 9 = 0.33, 10 = 0.26, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.18, 15 = 0.33, 16 = 0.33, 17 = 0.00, 17 = 0.26, 17 = 0.00, 18 = 0.53, 20 = 0.33, 21 = 0.33,

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2, 199 lb uplift at joint 18, 3 lb uplift at joint 26, 116 lb uplift at joint 27, 132 lb uplift at joint 28, 131 lb uplift at joint 29, 123 lb uplift at joint 30, 139 lb uplift at joint 31, 68 lb uplift at joint 32, 112 lb uplift at joint 25, 133 lb uplift at joint 24, 131 lb uplift at joint 23, 123 lb uplift at joint 22, 139 lb uplift at joint 21 and 57 lb uplift at joint 20.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

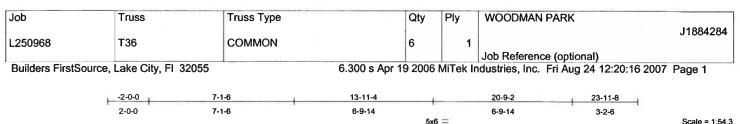
LOAD CASE(S) Standard

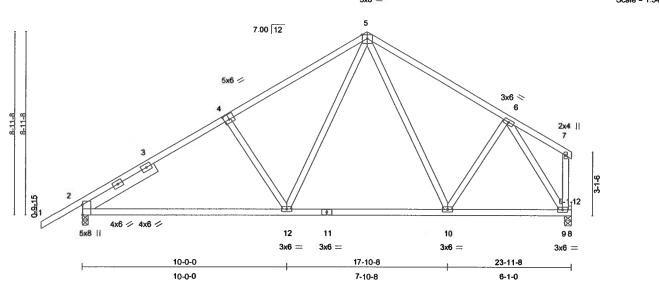
1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-10=-114(F=-60), 10-19=-114(F=-60), 2-18=-10

Julius Lew France Cesign Engineer Flance PE No. 24229 Flance PE No. 24229 Joynton Gesch. PL 25426







LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.24	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.45	10-12	>634	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 145 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 6 SYP No.1D 4-1-12

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-12

oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing,

Except:

8-4-3 oc bracing: 2-12.

REACTIONS (lb/size) 2=1064/0-3-8, 9=1031/0-4-0

Plate Offsets (X,Y): [2:0-5-14,Edge], [4:0-3-0,0-3-0]

Max Horz 2=248(load case 5)

Max Uplift 2=-305(load case 6), 9=-225(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1461/618, 3-4=-1351/646, 4-5=-1244/668, 5-6=-1003/551, 6-7=-8/70

2-12=-544/1130, 11-12=-281/751, 10-11=-281/751, 9-10=-296/625, 8-9=0/0 **BOT CHORD**

WEBS 4-12=-269/255, 5-12=-307/631, 5-10=-122/198, 6-10=-92/360, 6-9=-1198/566, 7-9=-25/21

JOINT STRESS INDEX

2 = 0.64, 2 = 0.27, 2 = 0.27, 3 = 0.00, 4 = 0.49, 5 = 0.63, 6 = 0.36, 7 = 0.34, 9 = 0.47, 10 = 0.47, 11 = 0.62 and 12 = 0.54

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at Continue and pagebuplift at joint 9.

August 24,2007

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
L250968	Т36	COMMON	6	1		J1884284
		- Comment			Job Reference (optional)	

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6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

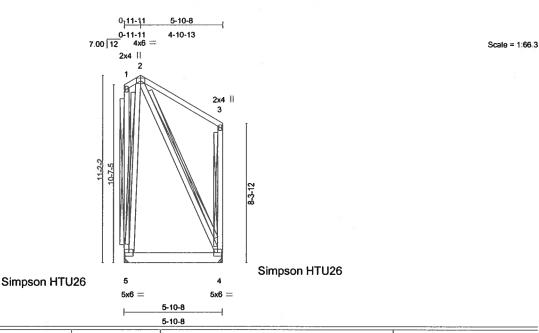
Uniform Loads (plf)

Vert: 1-5=-54, 5-7=-54, 2-12=-10, 10-12=-70(F=-60), 9-10=-10



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
Loronno	TOO	20141011				J1884285
L250968	T38	COMMON	1	1		
					Job Reference (optional)	

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LOADING TCLL TCDL	20.0 7.0	SPACING Plates Increase Lumber Increase	2-0-0 1.25 1.25	CSI TC BC	0.48 0.37	DEFL Vert(LL) Vert(TL)	in -0.04 -0.09	(loc) 4-5 4-5	l/defl >999 >781	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL	10.0	* Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 86 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 8 SYP 2400F 2.0E

2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 5-10-8 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 1-5, 2-5

, 3-4, 2-4

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 5=1098/Mechanical, 4=1098/Mechanical

Max Horz 5=-79(load case 6)

Max Uplift 5=-439(load case 6), 4=-243(load case 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-13/13, 2-3=-78/78, 1-5=-33/17, 3-4=-193/100 **BOT CHORD** 4-5=-14/64

WEBS 2-5=-193/198, 2-4=-158/34

JOINT STRESS INDEX

1 = 0.01, 2 = 0.15, 3 = 0.09, 4 = 0.04 and 5 = 0.06

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

August 24,2007

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
L250968	T38	COMMON	1	1	J188428
1230300	100	COMMON		'	Job Reference (optional)

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 5 and 243 lb uplift at joint
- 6) Girder carries tie-in span(s): 19-6-0 from 0-0-0 to 5-10-8
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-81(F=-27), 2-3=-81(F=-27), 4-5=-312(F=-302)

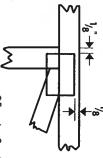


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

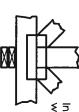
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



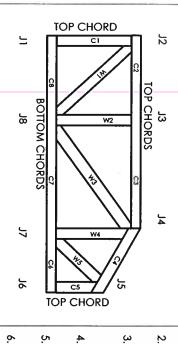
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA 96-31, 96-67
ICBO 3907, 4922

3907, 4922

SBCC! 9667, 9432A

561

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WISC/DILHR

960022-W, 970036-N



MiTek Engineering Reference Sheet: MII-7473

m 🔼 General Safety Notes

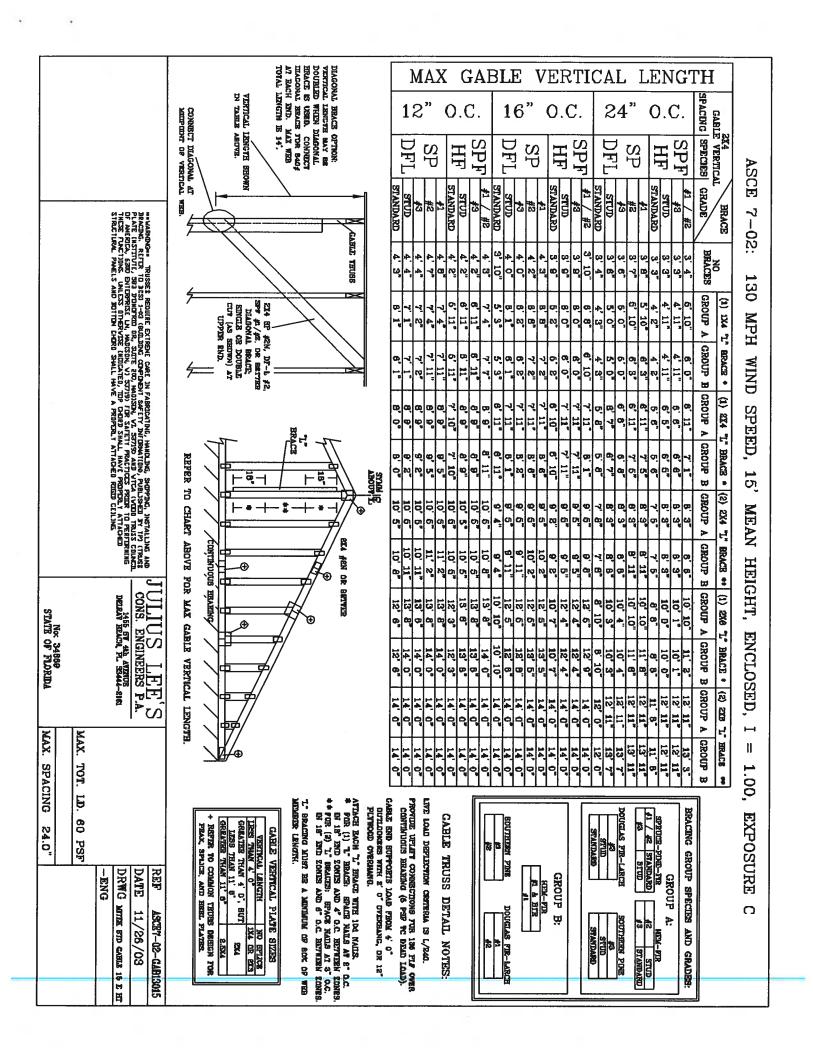
Failure to Follow Could Cause Property Damage or Personal Injury

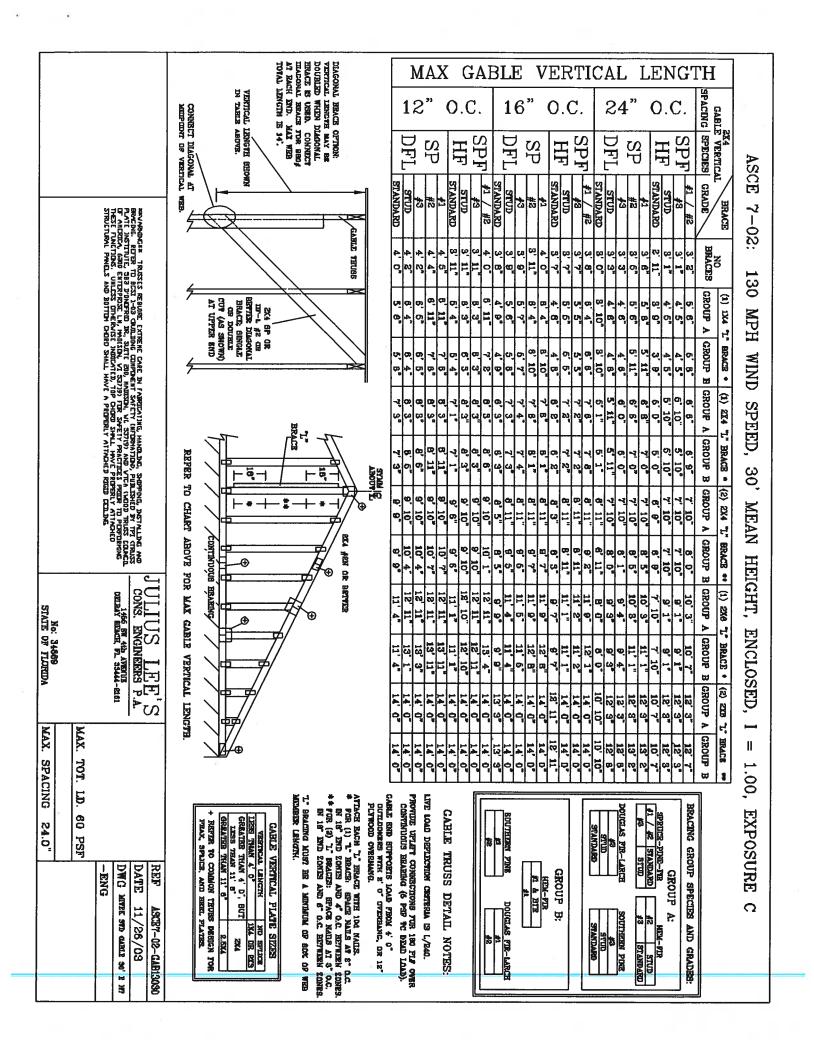
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)

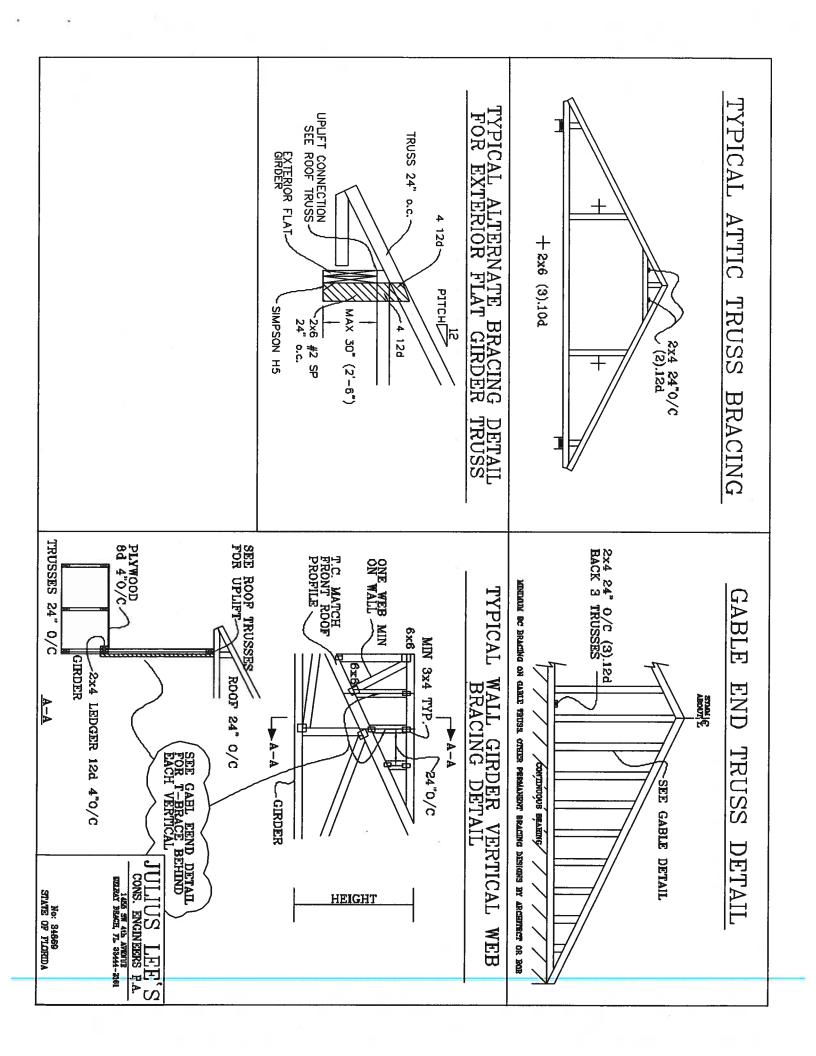
Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- . Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.

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BOT CHORD CHORD WEBS 224 424 444 444 444 ながぬ BETTER
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PIGGYBACK DETAIL

TAPE LIGHT

SPANS

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REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS

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ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PURCYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO BUCINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MBAN HGT, ASCE 7-93, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MBAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN BGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

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AXB OR SX6 TRULOX AT 4'
ROTATED VERTICALLY

B

C

1.5X3 **6X**4

1.6X4

1.004

1.5X4

6X6

8

923

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4×8

6X6

8

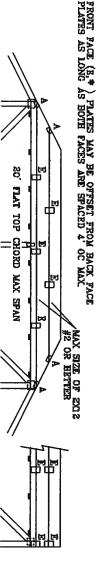
BX8

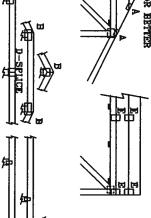
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2.5X4

2.6X4

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EITHER PLATE
LOCATION IS
ACCEPTABLE

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;	∄ │	0.1	3	0' TO 7'8"	ER)	
;	10, 10, 14,	OT O. R.	,	,,0,,	LENGTH	
MEMBER. ATTA	Zx4 T	MEMBER, OK HE	1x4 "I" BRACE. SAME GRADE, S	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

ATTACH TRUICX PLATES WITH (6) 0.120° X 1.575" NAILS, OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUICAX INFORMATION.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF PARKICATION. ATTACH TO SUPPORTING TRUSS THE (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

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B 1/4"

BAVARINISM TRUCKSEE REQUIRE CYTRINE EME IN FABRICATING, HANDLING, ENIPPING, INSTALLING AND BANCHON, BEFFET IN 2551 1-120 GRUTANG CORPORIDAT SAFET INFORMUTA, PILELICHED BY TRY CIRLISE PARTE DISTITUTE, 252 GROUPRED TR, SUITE 2011, MANCIN, V. 1. 35799 AND VICE, VICEN TREES CHILICUTE TRANSCRIP, LOUIN TREES CHILICUTE TO FAREICA, LOUIN CHIEFANES LM, MANCIN, VI. 35799 FIRE SAFETY PROFICES PROFILE TO PREFEDENCE TREES LINCOLUTE (TO PLOTE DANLE HAVE PROFILEY ATTACHED STRUCTUREAL PARTES AND BOTTON CHOOL SHALL HAVE A PROPIREY ATTACHED RIGHT CELLING.

¥¥ ∇

ATTACH

PIGGYBACK WITH 9X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

		ř	in o		
		1449 SW 4th AVENUE DYLKAY REACH FL. S3444 2161	CONS. ENGINEERS P.A.	1111 1112 1 EE, C	THIS DRAWIN
I.CO DOM. FAC.	50 PSF AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	THIS DRAWING REPLACES DRAWINGS 634,016 894,017 & 847,045
	-ENG JL	DRWGMITEK STD PIGGY	DATE 11/26/09	REF PIGGYBACK	34,016 694,017 & 647,
		(CG)			045

STATE OF FLORIDA					1455 SW 4th AVENUE	NS. ENGINEERS P.A.	מוווס דאה, מ
SPACING 24.0*	1.15 DUR. FAC.	47 PSF AT	L.ZD DUK. FAC.	50 PSF AT	1.33 DUR. FAC.	55 PSF AT	MAX LOADING
				-ENG JL	DRWGMITEK STD PI	DATE 11/26/09	REF PIGGYBACK
					D Pi	20	CK

VALLEYTRUSS DETAIL

B07 CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE)
- ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: SHC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

CUT FROM 2X6 OR LARGER AS REQ'D

12

4-0-0

XAM

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VAILEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

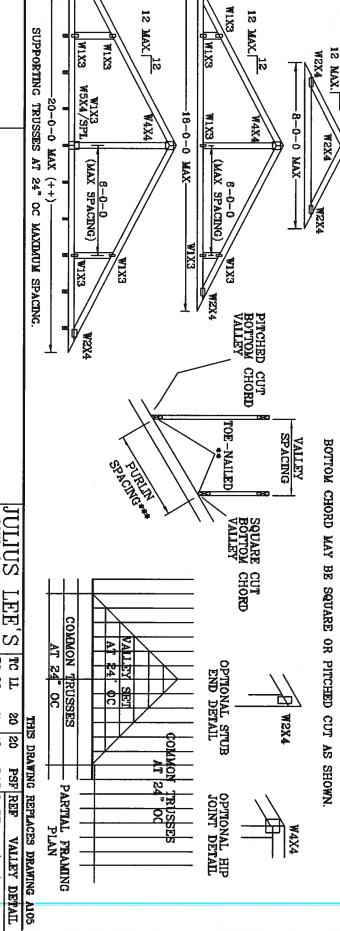
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN Engineers' Sealed Design. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

*

NOT EXCEED 12'0"



SEVERINIGEM TRUSSES FEBURE ECTRENE FARE IN FABRICATING, HANDING, ENIFONG, DISTALLING AND BRACING. REFER TO EXCI. FOR GUILLING CEPOPHOR SAFETY PERPENATION, PAULISMED BY THE (TRUSS PLANT INCITUTE, 820 DOGSTRO DR. SUITE 280, MADISON, VA. 53759 AND WITEA COURS THE SECRET PRICE. ASID ENTERPOSES LA MANISON, VI. 53759 FOR SAFETY PRACTICES PRICE TO PERFORM OF THESE DISCOUNTED, TOTO CHERO SHALL HAVE PROPERLY ATTACHED RIGHT OF THE PROPERTY
CONS. ENGINEERS P.A. DELBAT BEACH, I'L 150446-2001

BC II BC DI TC DL

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PSF

-ENG

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PSF DRWG VALTRUSS1103

PSF DATE

11/26/09

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No: 34869 STATE OF FLORIDA

SPACING DURFAC 1.25 TOT. LD.

24 1.25

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884276
L250968	T28	MONO HIP	1	1	
					Job Reference (optional)

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NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 2 and 593 lb uplift at joint 9.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-14=-54, 6-14=-134(F=-80), 6-7=-80(F), 2-15=-10, 9-15=-25(F=-15), 8-9=-15(F)

Concentrated Loads (lb)

Vert: 15=-1098(F)

Julius Les Trues Ceston Engineer Plonide PE No. 3-1808 1196 Espatal Bay Blvn Bovnton Besch, PL 95466



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK	
						J1884277
L250968	T29	MONO HIP	1	1		
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:51 2007 Page 1

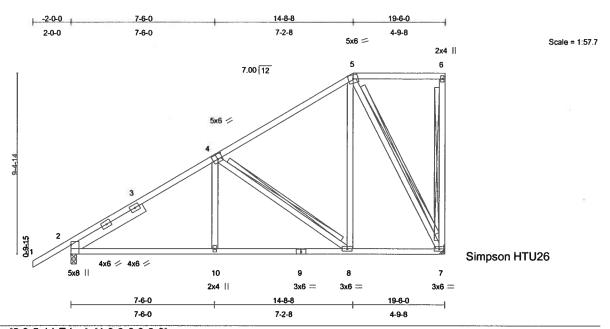


Plate Of	fsets (X,Y): [2:0-5-14,Edge], [4	1:0-3-0,0-3	3-0]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.04	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.09	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 135 lb	

LUMBER	
TOP CHORD	2
DOT OLIODO	_

X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

SLIDER

Left 2 X 6 SYP No.1D 4-4-5

BRACING

TOP CHORD

BOT CHORD

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 9-4-1 oc

Structural wood sheathing directly applied or

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 - 6-7,

4-8, 5-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=614/Mechanical, 2=733/0-3-8

Max Horz 2=325(load case 6)

Max Uplift 7=-187(load case 6), 2=-185(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-849/185, 3-4=-649/203, 4-5=-407/116, 5-6=-4/3, 6-7=-87/73

BOT CHORD 2-10=-440/620, 9-10=-440/619, 8-9=-440/619, 7-8=-172/262

4-10=0/225, 4-8=-431/325, 5-8=-143/363, 5-7=-567/376 **WEBS**

JOINT STRESS INDEX

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884277
L250968	T29	MONO HIP	1	1	
			<u></u>	l	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:51 2007 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 7 and 185 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truse Cesign Engineer Plonda PE No. 34889 1199 Chestel Bay Blvd Source March Bay Blvd



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884278
L250968	T30	MONO HIP	1	1	· .
L					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:52 2007 Page 1

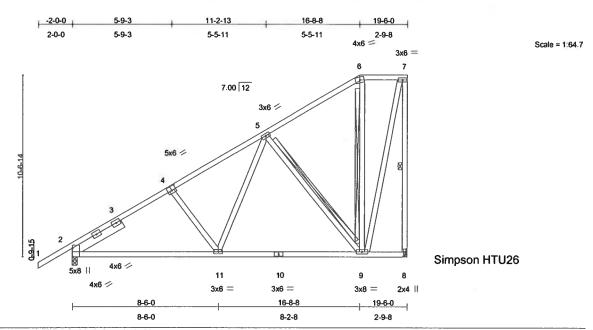


Plate Offsets (X,Y): [2:0-5-14, Edge], [4:0-3-0,0-3-0] LOADING (psf) **SPACING** 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** in (loc) 20.0 TCLL Plates Increase 1.25 TC 0.21 Vert(LL) -0.08 9-11 >999 360 MT20 244/190 TCDL Lumber Increase BC 0.33 Vert(TL) >999 7.0 1.25 -0.15 9-11 240 0.75 10.0 Rep Stress Incr YES **WB BCLL** Horz(TL) 0.02 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 149 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS**

Left 2 X 6 SYP No.1D 3-4-6 **SLIDER**

BRACING

BOT CHORD

WEBS

Structural wood sheathing directly applied or TOP CHORD

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-0-2 oc

bracing.

1 Row at midpt

7-8

T-Brace:

2 X 4 SYP No.3 - 5-9,

6-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=614/Mechanical, 2=733/0-3-8

Max Horz 2=363(load case 6)

Max Uplift 8=-225(load case 6), 2=-171(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-868/158, 3-4=-714/180, 4-5=-676/180, 5-6=-246/44, 6-7=-141/92,

7-8=-611/392

BOT CHORD 2-11=-491/648, 10-11=-307/425, 9-10=-307/425, 8-9=-2/2

WEBS 4-11=-209/225, 5-11=-141/331, 5-9=-454/342, 6-9=-155/155, 7-9=-383/588

JOINT STRESS INDEX

2 = 0.59, 2 = 0.16, 2 = 0.16, 3 = 0.00, 4 = 0.33, 5 = 0.41, 6 = 0.39, 7 = 0.41, 8 = 0.33, 9 = 0.66, 10 = 0.33 and 11 = 0.47

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
					J1884278
L250968	T30	MONO HIP	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Aug 24 09:39:52 2007 Page 2

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 8 and 171 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK
			1.		J1884279
L250968	T31	MONO HIP	1	1	
]		Job Reference (optional)

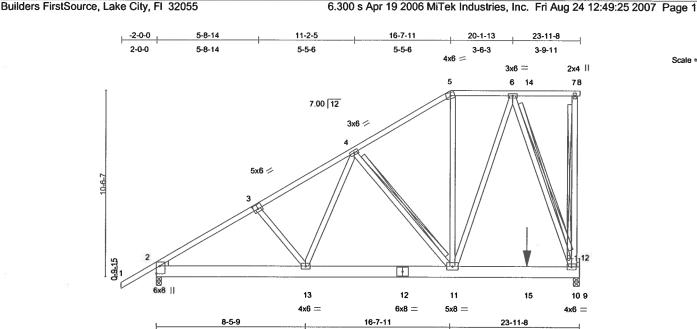


Plate Offsets (X,Y): [2:Edge,0-2-5], [3:0-3-0,0-3-0] LOADING (psf) **SPACING** 2-0-0 CSI **DEFL** in (loc) I/defl L/d **PLATES GRIP** 20.0 TCLL Plates Increase 1.25 TC 0.36 Vert(LL) -0.08 10-11 >999 360 **MT20** 244/190 **TCDL** 7.0 Lumber Increase 1.25 BC 0.51 Vert(TL) -0.15 10-11 >999 240 **BCLL** 10.0 Rep Stress Incr NO WB 0.75 0.01 Horz(TL) 10 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 206 lb

8-2-1

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 5-4-1

7-3-13

Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace:

2 X 4 SYP No.3 - 4-11, 6-10, 7-10

Scale = 1:62.8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1012/0-3-8, 10=1988/0-4-0

Max Horz 2=365(load case 5)

Max Uplift 2=-260(load case 5), 10=-545(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/41, 2-3=-1331/218, 3-4=-1148/226, 4-5=-839/193, 5-6=-658/205, 6-14=0/0, 7-14=0/0,

7-8=0/0

BOT CHORD 2-13=-434/1041, 12-13=-328/879, 11-12=-328/879, 11-15=-114/388, 10-15=-114/388,

9-10=0/0

WEBS 3-13=-186/155, 4-13=-34/230, 4-11=-355/195, 5-11=-38/217, 6-11=-285/853,

8-5-9

6-10=-1179/348, 7-10=-221/90

JOINT STRESS INDEX

2 = 0.65, 2 = 0.00, 3 = 0.31, 4 = 0.42, 5 = 0.33, 6 = 0.80, 7 = 0.34, 10 = 0.38, 11 = 0.50, 12 = 0.28 and 13 = 0.33

August 24,2007

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conn Applicability of design parameters and proper incorporation of component into the overall building structure, including ell temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, ere and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

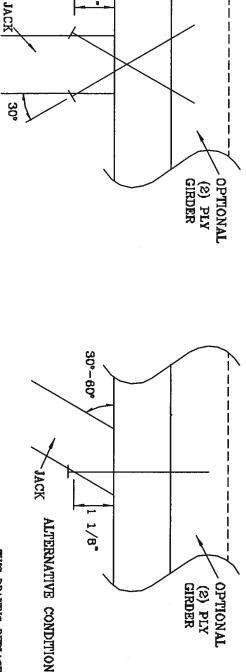
PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE PINE FIR	PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 PLY	1 PLY 2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	#881
3	296#	383#	271#	351#	234#	304#	230#	#862
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALU	ES MAY BI	ILIGITION :	ddy ab di	ROPRIATE 1	DURATION	ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	CTOR	



1/B"

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROED DEEDING.	INSTITUTE, 583 YUNGPRID JR., SUITE 200, MADISON, VI. 32719) AND VICA (MUID CICA, 6300 ENTERPRISE LH, MATERO, VI 33739) FOR SAFETY PRACTICES PRIJE TO	WARDER TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SUPPING, INSTALLING AND BRACING. PETER TO REX 1-13 CAN INC. CHINDER FOR SAFETY (WITHOUT INC. SUPPING, INSTALLING AND	
STATE OF FLORIDA	No: 34869			DELEAY BEACH, FL SHIFT- 2161	CONS. ENGINEERS P.A.	S, HHI SOUTOR
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF REF
			-ENG JL	DRWG	DATE	REF
			T	DRWG CNTONAIL1103	11/26/09	TOE-NAIL
				03		

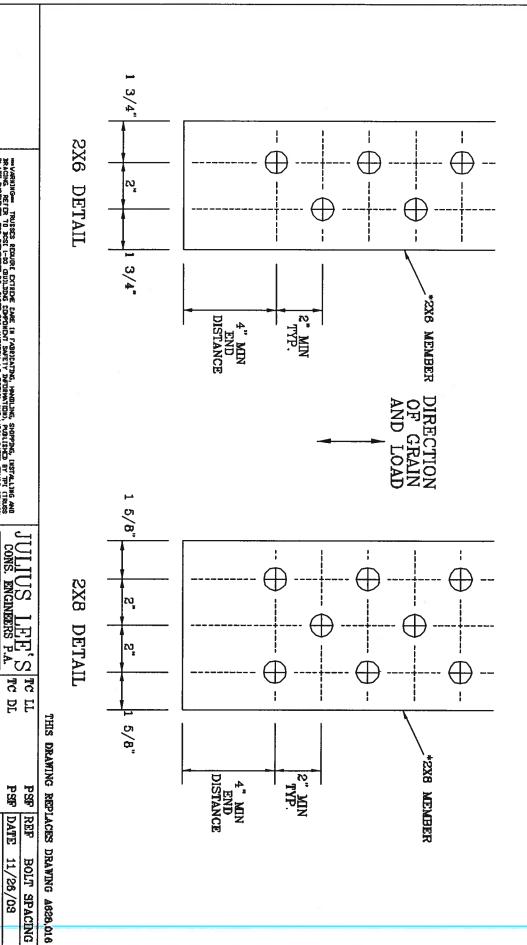
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL T0GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIANETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



DELRAY BEACH, FL 33444-2161

BC DL

PSS

DRWG DATE

CNBOLTSP1103 11/26/09

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No: 34869 STATE OF FLORIDA

SPACING DUR. FAC TOT. LD. BC LL

TRULOX CONNECTION

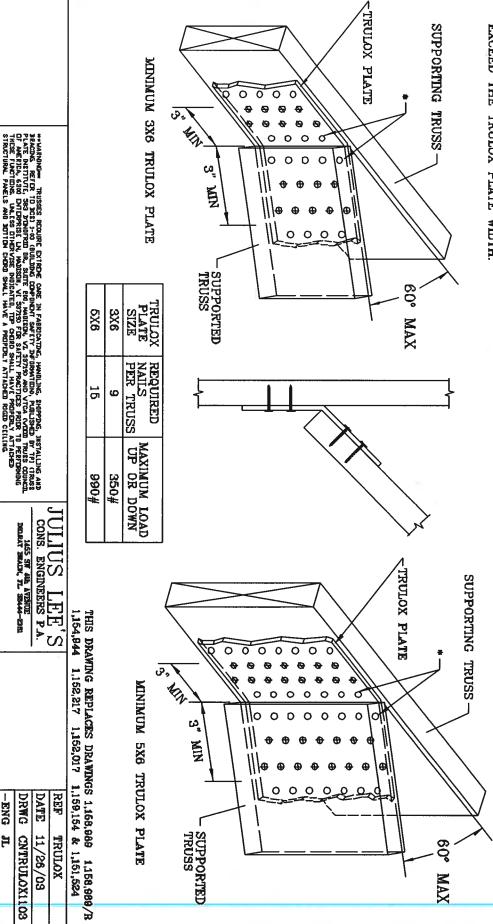
SHOWN (+). 11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN THIS DETAIL FOR LUMBER, PLATES, AND OTHER

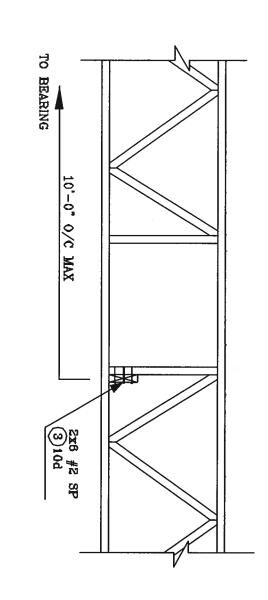


No: 34869 STATE OF FLORIDA

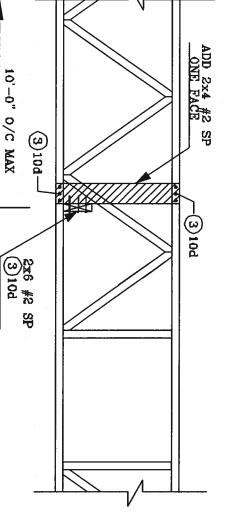
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STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

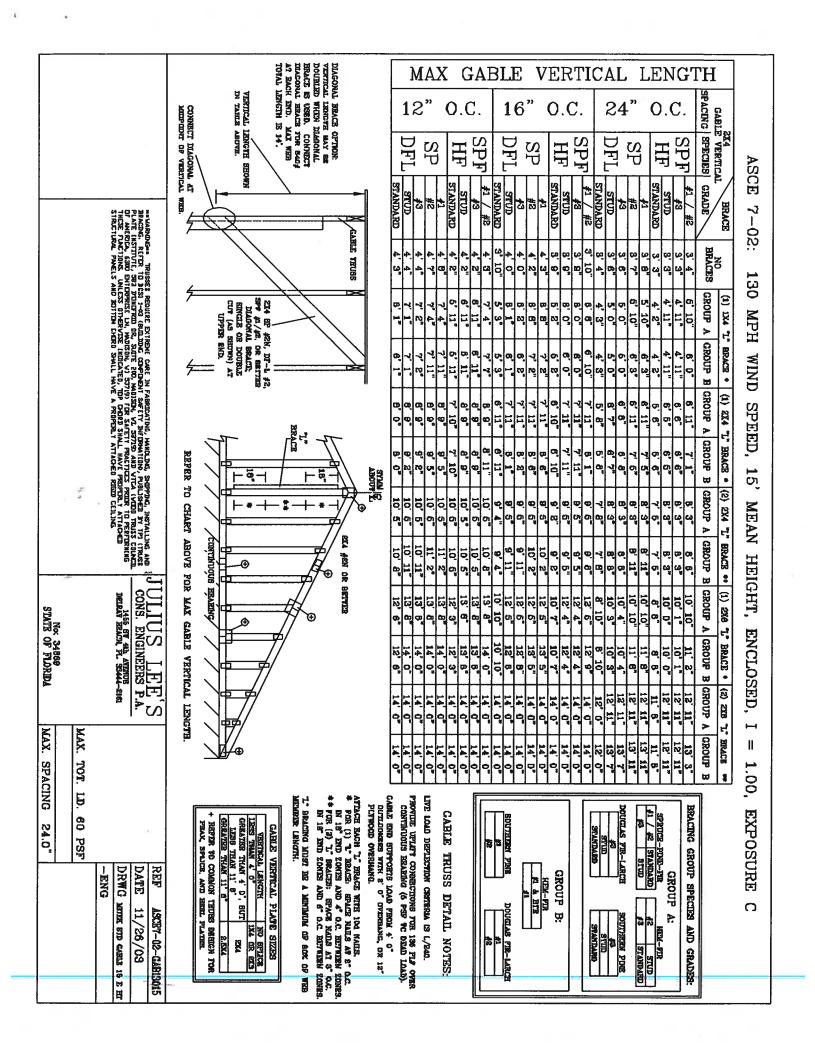


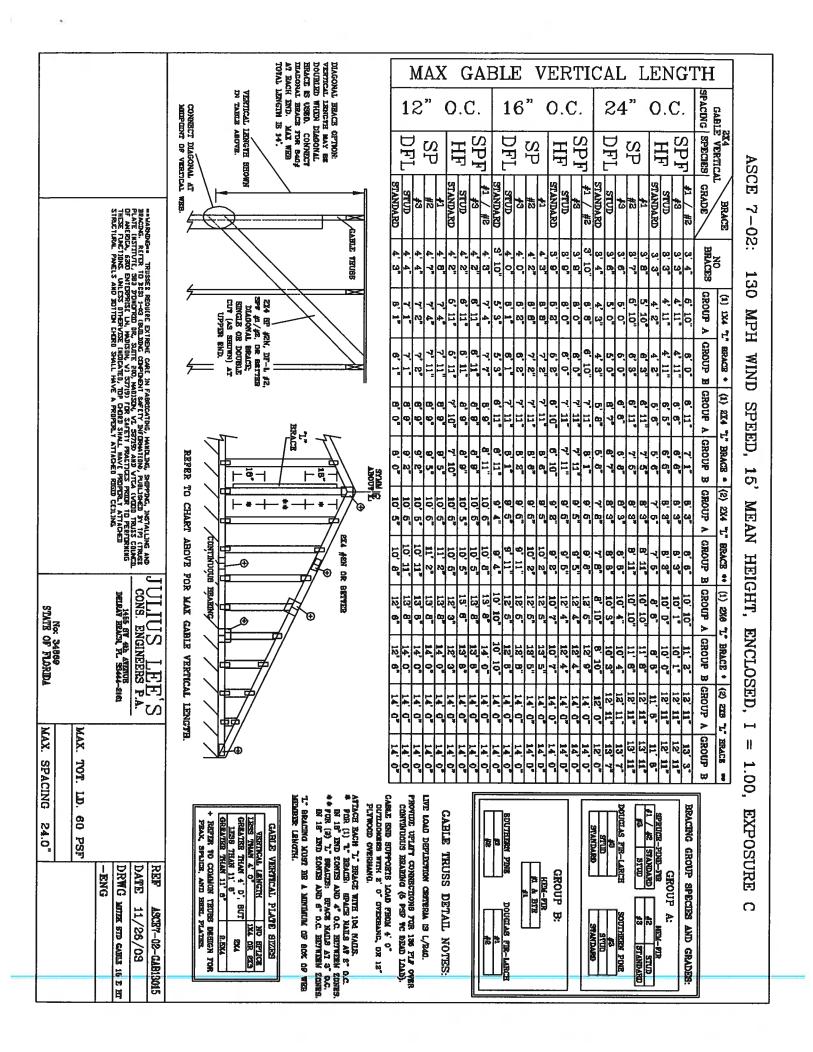
ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP

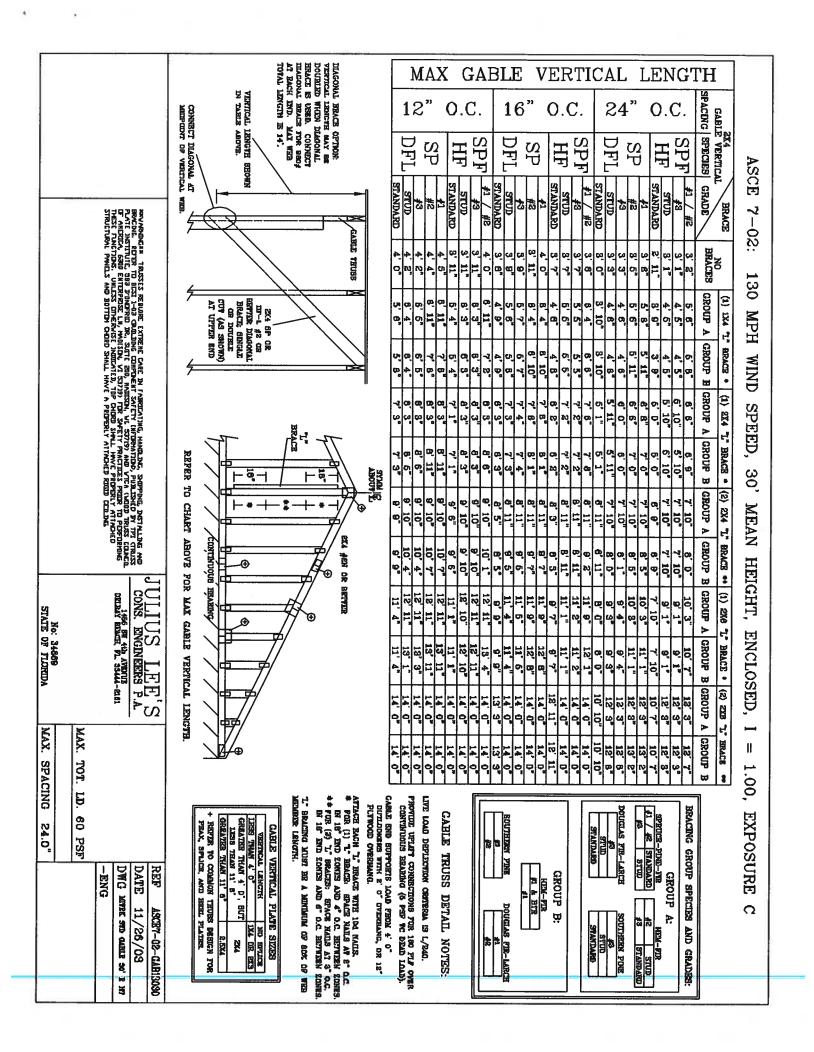


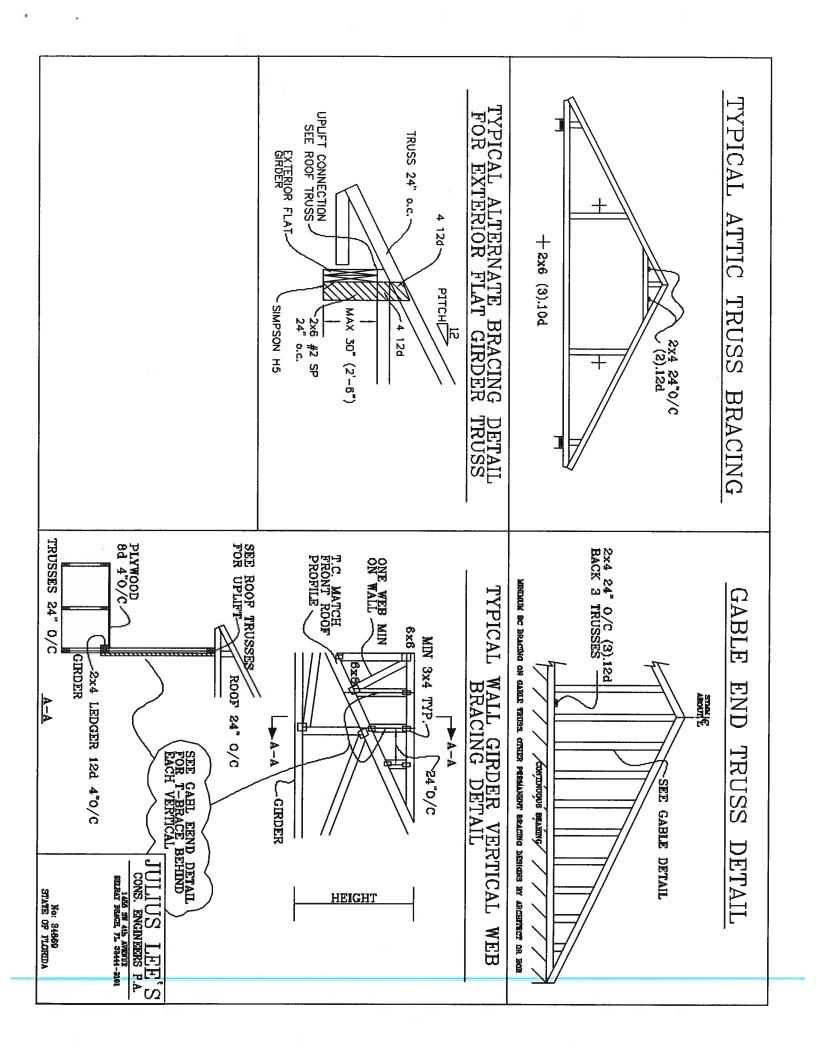
JULIUS LEE'S CONS. ENGINEERS P.A.

No: 84869 STATE OF FLORIDA TO BEARING









BOP CHORD CHORD WEDS 2002 BETTER BETTER

PIGGYBACK DETAIL

JON'S

SPANS

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REFER TO SEALED DESIGN FOR DASHED PLATES

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER. SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PICGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY HE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO BUGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:
110 MPH WIND, 30' MEAN HGT, ASCE 7-83, CLOSED BIDG,
LLOCATED ANYWHERE IN ROOF, 1 M FROM COAST
CAT L EXP C, WIND TC DL-6 PSF, WIND BC DL-6 PSF
110 MPH WIND, 30' MEAN HGT, SBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-6 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30° MEAN BCT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

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5X4

6%

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5X6

AXB OR SX6 TRULOX AT 4'
ROTATED VERTICALLY

В

C Ħ >

1.633

1.5X4

1.6X4

1.6X4

4×8

8X8

8

9XG

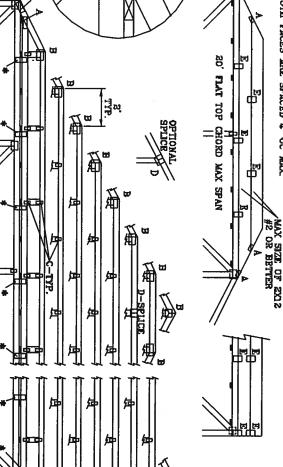
284

2.5X4

2.6X4

335

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. #2 OR BETTER



LOCATION IS
ACCEPTABLE

J XAX 12

ATTACH

PIGGYBACK WITH SX8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB ERACING CHART WEB LENGTH REQUIRED BRACING O' TO 7'9" NO BRACING 114 "T" BRACE. SAME GRADE, SPECIES AS WEB T'9" TO 10' MEMBER. OR HETTER, AND 80% LENGTH OF WEB 10' TO 14' MEMBER. OR HETTER, AND 80% LENGTH OF WEB 10' TO 14' MEMBER. OR HETTER, AND 80% LENGTH OF WEB 10' TO 14' MEMBER. ATTACH WITH 164 MAILS AT 4° OC.						
NO BRACING CHART REQUIRED BRACING NO BRACING NO BRACING NO BRACING NO BRACING NO BRACING REMBER, OR HETTER, AND 80% LENGTH OF MEMBER, ATTACH WITH 184 MAILS AT 4° O		10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
		I "T" BRACE. SAME GRADE, SPECIES AS MHER. OR HETTER, AND 80% LENGTH OF MHER. ATTACH WITH 16d NAMES AT 4° O	1x4 "I" BRACE. SAME GRADE, SPECIES AS I MEMBER, OR BETTER, AND 80% LENGTH OF I MEMBER. ATTACH WITH 8d NAMES AT 4" OC.	ľ	REQUIRED BRACING	
	- 1	B 6			_	Ш

8 1/4"		ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.	* PIGGYRACK SPRCIAL PLATE
	- 1		

THIS DRAWING REPLACES DRAWINGS 634,018 834,017 & 847,045

			STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.	OF AMERICA, 6200 CATERPRIZE LY, MARKEN, WI 187789 FOR SAFETY PROFITERS PROR TO PROFIDENCE TO THE CATERON TO THE CHORD SHALL HAVE PROPERTY ATTACHED.	PACE DEFINE TRUCKSES REQUIRE ENTREME CAME IN FARRICATING, HANDLING, SHEPPING, DISTALLING AND BACKING. REFER TO INCL. THE COULTING COMPONENT SAFETY INTERNATION, PILICIPIES BY THE COULTING CONFORMED SAFETY APPROXIMATION, PILICIPIES AND PACE CONTROLLED SAFETY APPROXIMATION OF THE CONTROLLED SAFETY APPROXIMATION OF		
NO: 3488B	:			1450 SW 4th AVENUE	CONS. ENGINEERS P.A.	N. H. H. L.	
SPACING 24.0°	47 PSF AT 1.15 DUR. FAC.	1.25 DUR. FAC.		1.33 DUR. FAC.		MAX LOADING	THE PERSON NAMED IN COLUMN PARTY OF THE PERSON NAMED IN COLUMN PAR
			-ENG JL	DRWGMITEK STD PIGGY	DATE 11/28/09	REF PIGGYBACK	ATTICATION OFFICE OF STATE OF STATES

VALLEYTRUSS DETAIL

TOP CHORD BOT CHORD WEBS 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: SHC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 18d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. ++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

CUT FROM 2X8 OR LARGER AS REQ'D

12 MAX.

W2X4

12

4-0-0

MAX

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN NOT EXCEED 12'0"

12 MAX 12 MAX SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING W1X3 W1X3 ₩1X3 WZX4 12 12 W1X3 W5X4/SPL 20-0-0 MAX (++)-₩4X4 16-0-0 MAX-₩1X3 8-0-0 MAX **W4X4** (MAX SPACING) (MAX SPACING 8-0-0 **72X4** W1X3 EXIM W1X3 WZX4 PITCHED CUT BOTTOM CHORD VALLEY W2X4 TOE-NAILED SPACING SPACING PURLIN SQUARE BOTTOM VALLEY CHORD COMMON TRUSSES VALLEY AT 24 OPTIONAL STUB 24" 8 00 W2X4 THIS DRAWING REPLACES DRAWING A105 PARTIAL FRAMING
PLAN OPTIONAL HIP JOINT DETAIL W4X4

₩1X3

SHAVAHINGS. TRUISES READE EXTREM ENGLE (I FABRITATIO, HANDING, SHAPP)
SHOULK REFER TO EXIL TO BUILDING COPPIENT SAFETY APPRIMATION, FIGHT,
PLATE DANTIFIER TO EXIL THE BUILDING COPPIENT SAFETY PROPERTY AND WITH A COPPIENT SAFETY PROPERTY AND WITH A COPPIENT SAFETY PROPERTY AND WITH A COPPIENT SAFETY PROPERTY AND A COPPIENT SAFETY PROPERTY AND A COPPIENT SAFETY PROPERTY AND A COPPIENT AND A COPP

			ERLY ATTACHED	RIDR TO PERFORMUL	PDIG DISTALLING AND		
STATE OF PLORIDA	No: 34869			DELEAT HEACH, IL 25444-2161	INEERS P.A.	S, HH'I SOI'IOI'	
SPA	DUR.	TOT	BC II	BC	IC.	TC	ı
SPACING	DURFAC 1.25	TOT. I.D. 32 40	F	Ħ	Œ	Ŧ	l
	E	32	0	U	~ Z	20	l
24"	1.25	40	0	Ç'n	15	92	
		PSF	PSF	PSF	PSF	PSF REF	
			PSF -ENG JL	PSF DRWG	PSF DATE	REF	
			л	VALTRUSS1103	11/26/09	VALLEY DETAIL	
				3	_	Ħ	ľ

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

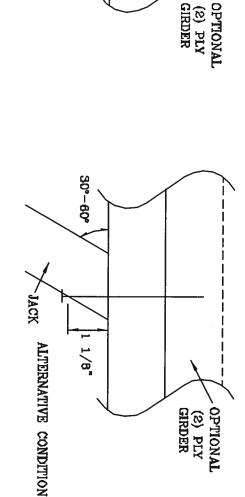
PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

	MAXIM
	IN
	LATERAL
	RESISTAN
	CE (
	E.
	ANCE OF 18d (
	0.1
ı	62
	62 X3.5
	MAXIMUM LATERAL RESISTANCE OF 18d (0.162"X3.5") COMMON
	(62"X3.5") COMMON TOE-NAILS

NUMBER OF	SOUTHERN PINE		DOUGLAS	DOUGLAS FIR-LARCH		HEM-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 PLY	1 PLY 2 PLIES	YIA I	2 PLIES
ນ	197#	256#	181#	234#	156#	203#	154#	#661
3	296#	383#	271#	351#	234#	304#	#052	#862
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	498#
ALL VALUE	ES MAY BE	MULTIPLIE	D BY APP	ROPRIATE	DURATION	ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	CTOR	



1/B

JACK

THIS DRAWING REPLACES DRAWING

	-							1
WARDIG* TRISSES REDURE EXTREME CARE IN FARICATING, HANDLING, SUPPING, DISTAILING AND BRACHIC. REPER TO BEST 14-33 CHALIDMIC COMPINENT SAFETY (HETWANTIDO), PUBLISHED BY TPY CIRLLSS PART INSTITUTE, 389 PHORPED BY, SUFFE MAN SAFETY PRACTICES POUR TO PERFORMED ASSET PARTITIONS. UNICLES OF THE EVISES DISCAILING OF SAFETY PARTITIONS POUR TO PERFORMED THESE CHACHED STRUCTURAL PANILS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHT OF THE SAFETY PANILS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHT CHARGES STRUCTURAL PANILS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHT CHARGES STRUCTURAL PANILS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGHT CHARGES.						•		
	STATE OF FLORIDA	No. 34869			DELPAY SEACH, FL SO444—2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
	SPACING	DUR. FAC.	TOT. LD.	вс ш	BC DL	TC DL	TC LL	3116
		1.00	PSF	PSF	PSF	PSF	PSF	TOTAL CAST WATER
				-ENG JL	DRWG	DATE	REF	
				Л	CNTONAIL 103	11/26/03	TOE-NAIL	DISCUSSION THE PROPERTY DISCUSSION OF CHOP
					33			10

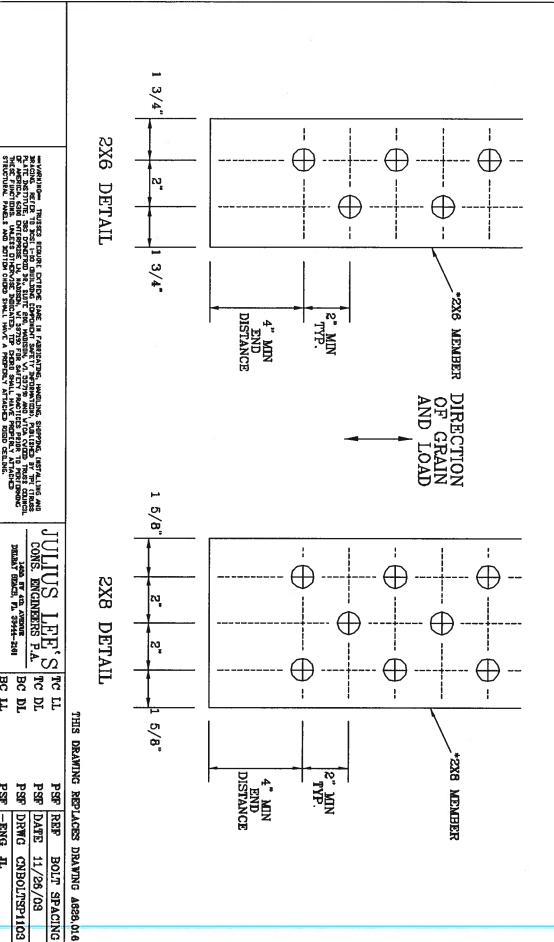
BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



No: 34869 STATE OF FLORIDA

SPACING DUR. FAC. TOT. LD. BC IL

PSH

-ENG

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TRULOX CONNECTION DETAIL

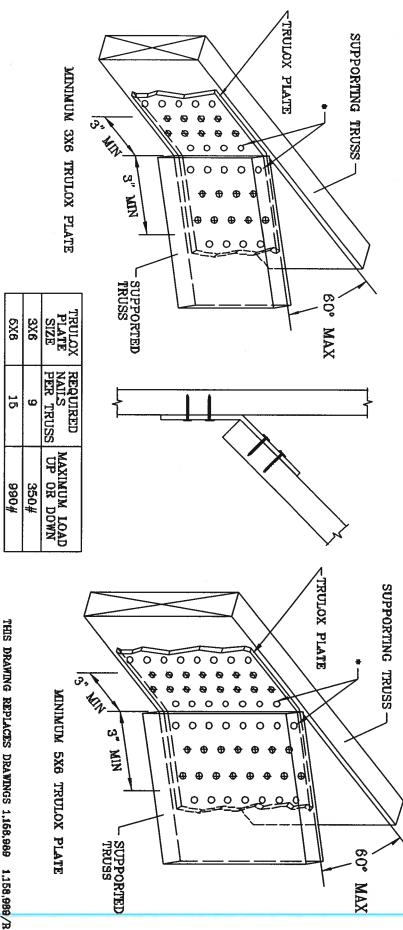
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\$\phi\$).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



* TRISSES ROWER EXTEDE CARE IN FABROATHG, HANDLING, SHOPPAG, HETALLING AND FIFT TO SELL THE GROWNERS ARE THE TRIBUTED OF THE LINES TO SELL THE MET AND THE PROPERTY OF THE SELL THE SEL

ULIUS LEE'S CONS. ENGINEERS P.A.

1455 997 48A AVENUE

1555 197 48A AVEN

1,154,944

1,152,217 1,152,017 1,159,154 & 1,151,524

DATE DRWG

CNTRULOX1103

11/26/03

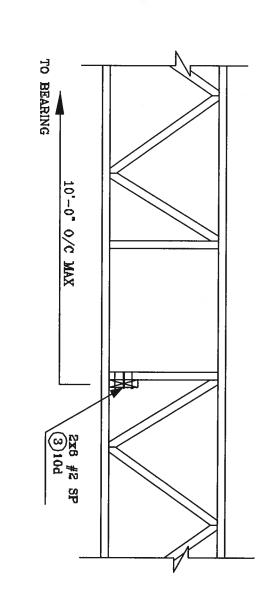
TRULOX

-ENG

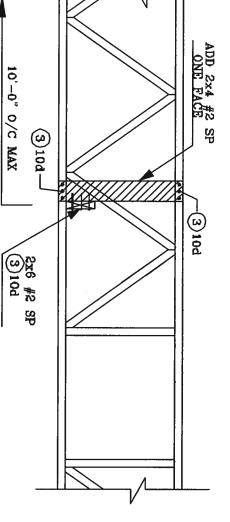
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No: 34869 STATE OF FLORIDA

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

TO BEARING

No: 34869 STATE OF FLORIDA

Load Short Form Entire House LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN
Date: Aug 21, 2007

Ву:

Project Information

For: MARK HADDOX, WOODMAN PARK BUILDERS

	Liter	Cla		Infiltration
	Htg	Clg		
Outside db (°F)	33	92	Method	Simplified
Inside db (°F)	70	75	Construction quality	Average
Design TD (°F)	37	17	Fireplaces	1 (Average)
Daily range `	-	М	•	` ',
nside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING	S EQUIPMENT			COOLING EQ	UIPMENT	
Make n/a			Make	n/a		
Trade n/a			Trade	n/a		
Model n/a			Cond	n/a		
	**		Coil	n/a		
Efficiency	n/a		Efficiency	/	n/a	
Heating input			Sensible		0	Btuh
Heating output	0	Btuh	Latent co	oling	0	Btuh
Temperature rise	0	°F	Total coo		0	Btuh
Actual air flow	0	cfm	Actual air	r flow	0	cfm
Air flow factor	0.000	cfm/Btuh	Air flow fa	actor	0.000	cfm/Btuh
Static pressure	0.00	in H2O	Static pre	essure	0.00	in H2O
Space thermostat	n/a			sible heat ratio	0.00	

ROOM NAME		Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
ZONE ONE ZONE 2	d d	1482 860	26621 15819	23969 9778	1153 613	1153 613
Entire House Other equip loads Equip. @ 0.97 RSM Latent cooling	d	2342	42440 0	33747 0 32734 5731	1766	1766
TOTALS		2342	42440	38465	1766	1766



Load Short Form ZONE ONE LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

By:

Project Information

For: MARK HADDOX, WOODMAN PARK BUILDERS

		Desigr	n Information	
	Htg	Clg		Infiltration
Outside db (°F)	33	92	Method	Simplified
Inside db (°F)	70	75	Construction quality	Average
Design TD (°F)	37	17	Fireplaces	1 (Average)
Daily range	-	M	•	,
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Trade Model	Ruud Ruud UPNE Series UPNE-036J*Z			Make Trade Cond Coil	Ruud Ruud UPNE Serie UPNE-036J*Z UBHK-21+RCHJ-		
Efficience	CV	8.2 HSPF		Efficiency		13 SEER	
Heating				Sensible c	ooling	24220	Btuh
Heating		36400	Btuh @ 47°F	Latent coo	ling	10380	Btuh
	ature rise	29	°F	Total cooli	ng	34600	Btuh
Actual a	ir flow	1153	cfm	Actual air f	low	1153	cfm
Air flow	factor	0.043	cfm/Btuh	Air flow fac	ctor	0.048	cfm/Btuh
Static pr	ressure	0.10	in H2O	Static pres	sure	0.10	in H2O
	hermostat		E	Load sens	ible heat ratio	0.84	

ROOM NAME		Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
FAMILY ROOM LAUNDRY ROOM MASTER BATH M/CLOSET M/BEDROOM DINING ROOM FOYER KITCHEN		404 104 160 65 238 187 116 209	4795 884 3809 2477 5540 4521 2044 2551	3616 4241 1788 605 2602 3169 1384 6564	208 38 165 107 240 196 89 110	174 204 86 29 125 152 67 316
ZONE ONE Other equip loads Equip. @ 0.97 Latent cooling	d RSM	1482	26621 1691	23969 777 24003 4781	1153	1153
TOTALS		1482	28312	28784	1153	1153



Load Short Form ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Ву:

Project Information

For: MARK HADDOX, WOODMAN PARK BUILDERS

《公司》,《公司》,《公司》		Desigr	Information			
Htg Clg Infiltration						
Outside db (°F)	33	92	Method	Simplified		
Inside db (°F)	70	75	Construction quality	Average		
Design TD (°F)	37	17	Fireplaces	1 (Average)		
Daily range	-	М	•	(1 1 2 2 7		
Inside humidity (%)	-	50				
Moisture difference (gr/lb)	-	52				

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Ruud				uud	
Trade Ruud UPNE Series			Trade R	uud UPNE Series	
Model UPNE-018J*Z			Cond U	PNE-018J*Z	
			Coil 17	7AHSA18HM+RCSA-H*24	17A*
Efficiency	8.5 HSPF		Efficiency	13 SEEF	
Heating input			Sensible cooli	ng 12880	Btuh
Heating output	17300	Btuh @ 47°F	Latent cooling	5520) Btuh
Temperature rise	26	°F	Total cooling	18400	Btuh
Actual air flow	613	cfm	Actual air flow	613	cfm
Air flow factor	0.039	cfm/Btuh	Air flow factor	0.063	cfm/Btuh
Static pressure	0.10	in H2O	Static pressure	e 0.10	in H2O
Space thermostat			Load sensible		i

ROOM NAME		Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
		88 230 206 206 65 65	131 3962 4752 5428 773 773	283 3315 2673 2811 348 348	5 154 184 210 30 30	18 208 168 176 22 22
ZONE 2 Other equip loads Equip. @ 0.97 RS Latent cooling	d SM	860	15819 1300	9778 597 10064 3527	613	613
TOTALS		860	17120	13590	613	613

Load Multizone Summary Report

Job: MORTENSEN Date: Aug 21, 2007

LARRY RESMONDO AIR CONDITIONING

Infiltration Summary

ZONE NAME	Volume ft ³	Hea ACH	ting AVF cfm	HTM Btuh/ft²	Volume	Coo ACH	oling AVF cfm	HTM Btuh/ft²
ZONE ONE ZONE 2	13932 7737	0.34	79 56	2.9	13932 7737	0.15 0.19	34 24	0.6 0.6
Entire House	21669	0.38	136	2.9	21669	0.16	58	0.6

Load and AVF Summary

ROOM NAME	Area ft²	Htg load Btuh	Clg load Btuh	Htg AVF cfm	Clg AVF cfm
FAMILY ROOM	404	4795	3616	208	174
LAUNDRY ROOM	104	884	4241	38	204
MASTER BATH	160	3809	1788	165	86
M/CLOSET	65	2477	605	107	29
M/BEDROOM	238	5540	2602	240	125
DINING ROOM	187	4521	3169	196	152
FOYER	116	2044	1384	89	67
KITCHEN	209	2551	6564	110	316
ZONE ONE	1482	28312	23969	1153	1153
HALL	88	131	283	5	18
BEDROOM 2	230	3962	3315	154	208
BEDROOM 3	206	4752	2673	184	168
BEDROOM 4	206	5428	2811	210	176
BATH 2	65	773	348	30	22
BATH 3	65	773	348	30	22
ZONE 2	860	17120	9778	613	613
Entire House	2342	42440	33747	1323	1323

Building Analysis Entire House LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

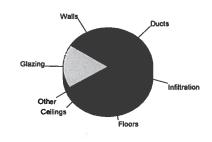
Project Information

MARK HADDOX, WOODMAN PARK BUILDERS For:

		Design Co	nditions		
Location: Gainesville, FL, US Elevation: 0 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 1 (Average)	Cooling 75 17 50 51.6

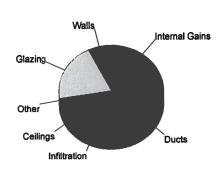
Heating

Walls 2.3 6665 15.7 Glazing 32.2 7693 18.1 Doors 14.4 606 1.4 Ceilings 1.2 2773 6.5 Floors 3.9 9195 21.7 Infiltration 2.9 5518 13.0 Ducts 9990 23.5 Piping 0 0.0 Humidification 0 0.0 Ventilation 0 0.0 Adjustments 0 0.0	Component	Btuh/ft²	Btuh	% of load
Total 42440 100.0	Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments	32.2 14.4 1.2 3.9	7693 606 2773 9195 5518 9990 0 0	18.1 1.4 6.5 21.7 13.0 23.5 0.0 0.0



Cooling

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	0.8 28.7 11.4 2.0 0.0 0.6	2434 6867 477 4695 0 1081 11272 0 6920 0 33747	7.2 20.3 1.4 13.9 0.0 3.2 33.4 0.0 20.5 0.0



Overall U-value = 0.122 Btuh/ft²-°F

Data entries checked.

Building Analysis **ZONE ONE** LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

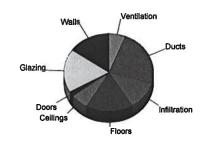
Project Information

MARK HADDOX, WOODMAN PARK BUILDERS For:

第 位于10年第四世间第	Design Conditions								
Location: Gainesville, FL, US Elevation: 0 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 1 (Average)	Cooling 75 17 50 51.6				

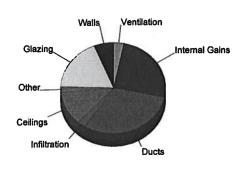
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation	2.5 32.2 14.4 1.2 3.3 2.9	4328 5118 606 1755 4874 3234 6705 0	15.3 18.1 2.1 6.2 17.2 11.4 23.7 0.0 0.0 6.0
Adjustments Total		0 28312	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower	0.9 28.7 11.4 2.0 0.0 0.6	1509 4569 477 2972 0 633 7579 777 6230	6.1 18.5 1.9 12.0 0.0 2.6 30.6 3.1 25.2 0.0
Adjustments Total		24746	100.0



Overall U-value = 0.115 Btuh/ft2-°F

Data entries checked.

Building Analysis ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

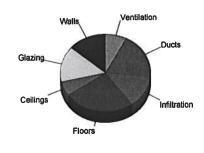
Project Information

MARK HADDOX, WOODMAN PARK BUILDERS For:

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 0 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 1 (Average)	Cooling 75 17 50 51.6

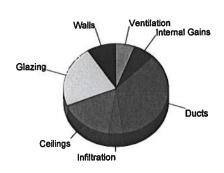
Heating

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	2.0 32.2 0.0 1.2 5.0 2.9	2337 2575 0 1018 4321 2283 3285 0 1300 17120	13.6 15.0 0.0 5.9 25.2 13.3 19.2 0.0 7.6



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	0.8 28.7 0.0 2.0 0.0 0.6	925 2299 0 1723 0 447 3693 597 690 0 10375	8.9 22.2 0.0 16.6 0.0 4.3 35.6 5.8 6.7 0.0



Overall U-value = 0.134 Btuh/ft2-°F

Data entries checked.

Project Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

Notes:

Design Information

Weather: Gainesville, FL, US **Winter Design Conditions Summer Design Conditions** 92 75 17 Outside db 33 °F 70 °F 37 °F Outside db Inside db Inside db °F ۴ Design TD Design TD Daily range М Relative humidity 50 % gr/lb Moisture difference **Heating Summary** Sensible Cooling Equipment Load Sizing 32450 Btuh Structure Structure Ducts 9990 Btuh **Ducts** 11272 Btuh Central vent (73 cfm) 0 Btuh Central vent (73 cfm) 0 Btuh Humidification 0 Btuh Blower 0 Btuh Btuh **Piping** 42440 Btuh Use manufacturer's data Equipment load 0.97 Rate/swing multiplier Equipment sensible load 32734 Btuh Infiltration Method Simplified Latent Cooling Equipment Load Sizing Construction quality Average 2826 **Fireplaces** 1 (Average) Structure Btuh 2906 Btuh Ducts Heating Cooling Central vent (73 cfm) 0 Btuh Area (ft²) Volume (ft³) 2342 21669 2342 Equipment latent load 5731 Btuh 21669 Air changes/hour Equiv. AVF (cfm) 0.38 Equipment total load 38465 Btuh 0.16 Reg. total capacity at 0.70 SHR 136 3.9 ton Cooling Equipment Summary **Heating Equipment Summary** Make Make n/a Trade n/a **Trade** n/a Model n/a Cond n/a Coil n/a Efficiency n/a **Efficiency** n/a Heating input Sensible cooling 0 Btuh Heating output Btuh Latent cooling 0 Btuh °F Total cooling **Btuh** 0 O Temperature rise Actual air flow 0 cfm Actual air flow O cfm 0.000 cfm/Btuh Air flow factor 0.000 cfm/Btuh Air flow factor Static pressure 0.00 in H2O Static pressure 0.00 in H2O Space thermostat Load sensible heat ratio 0.00 n/a



Project Summary ZONE ONE LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

Notes:

Design Information

	Weather: Gai	nesville, FL, US	
Winter Desig	n Conditions	Summer Design C	onditions
Outside db Inside db Design TD	33 °F 70 °F 37 °F	Outside db Inside db Design TD Daily range Relative humidity Moisture difference	92 °F 75 °F 17 °F M 50 % 52 gr/lb
Heating	Summary	Sensible Cooling Equipm	ent Load Sizing
Structure Ducts Central vent (42 cfm) Humidification Piping	19916 Btuh 6705 Btuh 1691 Btuh 0 Btuh 0 Btuh	Structure Ducts Central vent (42 cfm) Blower	16390 Btuh 7579 Btuh 777 Btuh 0 Btuh
Equipment load	28312 Btuh ration	Use manufacturer's data Rate/swing multiplier Equipment sensible load	n 0.97 24003 Btuh
Method Construction quality	Simplified Average	Latent Cooling Equipme	ent Load Sizing
Fireplaces Area (ft²)	1 (Average) Heating Cooling 1482 1482 13932 13932	Structure Ducts Central vent (42 cfm) Equipment latent load	1387 Btuh 1937 Btuh 1456 Btuh 4781 Btuh
Volume (ft³) Air changes/hour Equiv. AVF (cfm)	0.34 0.15 79 34	Equipment total load Req. total capacity at 0.70 SHR	28784 Btuh 2.9 ton
Heating Equip	ment Summary	Cooling Equipment	Summary
Make Ruud Trade Ruud UPNE S Model UPNE-036J*Z	eries	Make Ruud Trade Ruud UPNE Series Cond UPNE-036J*Z	

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Coil

Efficiency Sensible cooling

Latent cooling

Total cooling

Actual air flow

Air flow factor

Static pressure Load sensible heat ratio

UBHK-21+RCHJ-36A1



Efficiency

Heating input Heating output

Air flow factor Static pressure Space thermostat

Temperature rise Actual air flow

13 SEER

24220 Btuh 10380 Btuh

34600 Btuh

1153 cfm

0.84

0.048 cfm/Btuh

0.10 in H2O

8.2 HSPF

36400 Btuh @ 47°F 29 °F 1153 cfm

0.043 cfm/Btuh

0.10 in H2O

Project Summary ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

Notes:

Design Information

	Weather: Gainesvi	ille, FL, US	
Winter Desig	n Conditions	Summer Design C	onditions
Outside db Inside db Design TD	33 °F 70 °F 37 °F	Outside db Inside db Design TD Daily range Relative humidity Moisture difference	92 °F 75 °F 17 °F M 50 % 52 gr/lb
Heating S	Summary	Sensible Cooling Equipm	nent Load Sizing
Structure Ducts Central vent (32 cfm) Humidification	12534 Btuh 3285 Btuh 1300 Btuh 0 Btuh	Structure Ducts Central vent (32 cfm) Blower	6084 Btuh 3693 Btuh 597 Btuh 0 Btuh
Piping Equipment load Infiltr	0 Btuh 17120 Btuh ation	Use manufacturer's data Rate/swing multiplier Equipment sensible load	n 0.97 10064 Btuh
Method	Simplified	Latent Cooling Equipme	ent Load Sizing
Construction quality Fireplaces Area (ft²)	Average 1 (Average) Heating Cooling 860 860	Structure Ducts Central vent (32 cfm) Equipment latent load	1438 Btuh 968 Btuh 1120 Btuh 3527 Btuh
Volume (ft³) Air changes/hour Equiv. AVF (cfm)	7737 7737 0.44 0.19 56 24	Equipment total load Req. total capacity at 0.70 SHR	13590 Btuh
Heating Equipr	nent Summary	Cooling Equipment	Summary
Make Ruud Trade Ruud UPNE Se Model UPNE-018J*Z Efficiency Heating input Heating output Temperature rise Actual air flow Air flow factor Static pressure Space thermostat	8.5 HSPF 17300 Btuh @ 47°F 26 °F 613 cfm 0.039 cfm/Btuh 0.10 in H2O	Make Ruud Trade Ruud UPNE Series Cond UPNE-018J*Z Coil 17AHSA18HM+RCSA-H Efficiency Sensible cooling Latent cooling Total cooling Actual air flow Air flow factor Static pressure Load sensible heat ratio	1*2417A* 13 SEER 12880 Btuh 5520 Btuh 18400 Btuh 613 cfm 0.063 cfm/Btuh 0.10 in H2O 0.75



Duct System Summary ZONE ONE LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Ву:

Project Information

For: MARK HADDOX, WOODMAN PARK BUILDERS

Heating Cooling External static pressure 0.10 in H2O 0.10 in H2O Pressure losses 0.25 in H2O 0.25 in H2O Available static pressure -0.2 in H2O -0.2 in H2O Supply / return available pressure -0.08 / -0.07 in H2O -0.08 / -0.07 in H2O Lowest friction rate 0.100 in/100ft 0.100 in/100ft Actual air flow 1153 cfm 1153 cfm Total effective length (TEL)

180 ft

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
FAMILY ROOM	h	4795	208	174	0.100	9	12x6	VIFx	100.0	0.0	st1
LAUNDRY ROOM-A	C	2121	19	102	0.100	7	12×4	VIFx	100.0	0.0	st1
LAUNDRY ROOM	С	2121	19	102	0.100	7	12×4	VIFx	100.0	0.0	st1
MASTER BATH	h	3809	165	86	0.100	8	12x5	VIFx	100.0	0.0	st1
M/CLOSET	h	2477	107	29	0.100	7	12x4	VIFx	100.0	0.0	st1
M/BEDROOM	l h	5540	240	125	0.100	9	12x6	VIFx	100.0	0.0	st1
DINING ROOM	h	4521	196	152	0.100	9	12x6	VIFx	100.0	0.0	st1
FOYER	h	2044	89	67	0.100	6	12×3	VIFx	100.0	0.0	st1A
KITCHEN-A	c	3282	55	158	0.100	8	12x5	VIFx	100.0	0.0	st1A
KITCHEN	C	3282	55	158	0.100	8	12x5	VIFx	100.0	0.0	st1A

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	1153	1153	0.100	798	16	16 x 13	RectFbg	st1
st1A	Peak AVF	199	382	0.100	573	10	16 x 6	RectFbg	

Bold/Italic values have been manually overridden



Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb2	0×0	240	125	80.0	0.100	494	9	10x 7		VIFx	

Duct System Summary ZONE 2 LARRY RESMONDO AIR CONDITIONING

Job: MORTENSEN Date: Aug 21, 2007

Project Information

For:

MARK HADDOX, WOODMAN PARK BUILDERS

	Heating		Co	ooling
External static pressure	0.10 in H2O			in H2O
Pressure losses	0.25 in H2O		0.25	in H2O
Available static pressure	-0.2 in H2O		-0.2	in H2O
Supply / return available pressure	-0.08 / -0.07 in H2O		-0.08 / -0.07	in H2O
Lowest friction rate	0.100 in/100ft		0.100	in/100ft
Actual air flow	613 cfm		613	cfm
Total effective length (TEL)		135 f	ť	

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
HALL BEDROOM 2 BEDROOM 3 BEDROOM 4 BATH 2 BATH 3	c c h h h h	283 3315 4752 5428 773 773	5 154 184 210 30 30	18 208 168 176 22 22	0.100 0.100 0.100 0.100 0.100 0.100	9 8 9 4	12x1 12x6 12x5 12x6 12x1 12x1	VIFX VIFX VIFX VIFX VIFX VIFX	75.0 75.0 75.0 75.0 75.0 75.0	0.0 0.0 0.0 0.0 0.0 0.0	st1 st1 st1 st1A st1 st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	613	613	0.100	690	12	16 x 8	RectFbg	st1
st1A	Peak AVF	210	176	0.100	315	10	16 x 6	RectFbg	

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSi (in)	ze	Stud/Joist Opening (in)	Duct Mati	Trunk
rb2 rb3 rb4	0x0 0x0 0x0	154 184 210		60.0 60.0 60.0	0.100 0.100 0.100	530	9 8 9		7 5 7		VIFx VIFx VIFx	

Bold/italic values have been manually overridden



New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

26232

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise, Section 1: General Information (Treating Company Information) Company Name: Aspen Peet Control, Inc. Company Address: 231 N.W. Cole Terrace, Suite 107 City Lake City Company Business License No. Company Phone No. 388-755-3811 • 352-494-575 FHA/VA Case No. (if any) _____ Section 2: Builder Information Westman Park Builders Company Phone No. Section 3: Property Information Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) ____ Basement ☐ Crawl Other ____ Type of Fill Approximate Depth of Footing: Outside _____/ > Section 4: Treatment Information Date(s) of Treatment(s) _____ Brand Name of Product(s) Used _______ Approximate Final Mix Solution % 86 Linear ft. Approximate Size of Treatment Area: Sq. ft. 90 ZZZ Linear ft. of Masonry Voids ZZZ Approximate Total Gallons of Solution Applied ____ Z-No. Was treatment completed on exterior? Yes □ No Service Agreement Available? Yes Note: Some state laws require service agreements to be issued. This form does not preempt state law. Attachments (List) ___ Comments : 97-0- Brown an Certification No. (if required by State law) The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001449

DATE $09/$	13/2007	PARCEL ID # 2	27-6S-16-03951-105	
APPLICANT	MARK HADDO	x	PHONE	755-2411
ADDRESS _	P.O. BOX	1755	LAKE CITY	FL 32056
OWNER G	EORGE MORTEN	SEN	PHONE	
ADDRESS 2	250 SW VARGA	AS WAY	FT. WHITE	FL 32038
CONTRACTO	OR WOODMAN	PARK/WILLIAM WOOD	PHONE	755-2411
LOCATION O	F PROPERTY	47S, TL ON JEANLEA, TL O	ON VARGAS WAY, 3RD L	OT ON LEFT
		<u> </u>		
SUBDIVISION	N/LOT/BLOCK/	PHASE/IINIT FOXWOOD)	5
	VEO I/BEGGR			
SIGNATURE	Mu	M Khin		
OIOIWII OILD	00000	1.11		XIII
	<u>INSTALLAT</u>	TION REQUIREMENTS		
X	driving surface	ill be 18 inches in diameter e. Both ends will be mitered d concrete slab.	with a total lenght of 3 d 4 foot with a 4 : 1 slop	2 feet, leaving 24 feet of se and poured with a 4 inch
	a) a majority b) the drived Turnouts s concrete d	ON NOTE: Turnouts will be of the current and existing way to be served will be partial be concrete or paved or paved driveway, whicheved existing paved or concrete	g driveway turnouts are ved or formed with con a minimum of 12 feet v er is greater. The width	crete. wide or the width of the
	Culvert instal	ation shall conform to the	approved site plan star	ndards.
	Department o	f Transportation Permit ins	stallation approved star	ndards.
	Other			

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Building permit No. 000026232

25.68

Fire:

Waste: 67.00

Parcel Number 27-6S-16-03951-105

Use Classification SFD, UTILITY

Permit Holder WOODMAN PARK/WILLIAM WOOD

Owner of Building GEORGE MORTENSEN

250 SW VARGAS WAY, FT. WHITE, FL Location:

Date: 06/18/2008

92.68

Total:

Building Inspector

POST IN A CONSPICUOUS PLACE Business Places Only]

