ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1UZ6215-Z0128121608

Truss Fabricator W.B. Howland

Job Identification 8320-/SMITH RES.--LAKE CITY /S&S CONSTRUCTION -- LAKE CITY

Truss Count: 40

Model Code
Truss Criteria.
Florida Building Code 2010
FBC2010Res/TPI-2007(STD)

Engineering Software Alpine Software, Version 12.03.

Structural Engineer of Record The identity of the structural EOR did not exist as of

Address the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 130 MPH ASCE 7-10 -Closed

Notes

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1 William H Krick -Truss Design Engineer-

08/2/8/2013

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

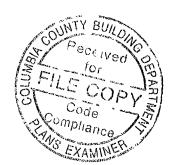
1950 Marley Drive Haines City, FL 33844

3. As shown on attached drawings; the drawing number is preceded by: HCUSR215

Details: 14030EC1-GBLLETIN-PB16010-PB16010-14015EC1-BRCLBSUB-CNNAILSP-

#	Ref Description	Drawing#	Date
1	62506A1	13240001	08/28/13
2	62507A2	13240038	08/28/13
3	62508A3	13240039	08/28/13
4	62509A4	13240036	08/28/13
5	62510~~A5	13240016	08/28/13
6	62511A6	13240017	08/28/13
7	62512A7	13240018	08/28/13
8	62513A8	13240019	08/28/13
9	62514A9	13240020	08/28/13
10	62515A10	13240021	08/28/13
11	62516A11	13240022	08/28/13
12	62517B1	13240023	08/28/13
13	62518B2	13240002	08/28/13
14	62519B3G	13240032	08/28/13
15	62520B4	13240003	
16	62521B5	13240004	
17	62522B6G	13240033	
18	62523C1	13240005	
19	62524C2	13240034	
20	62525D1	13240006	
21	62526D2	13240007	
22	62527D3	13240031	
23	62528D4	13240024	
24	62529D5	13240025	
25	62530D6	13240026	
26	62531D7	13240027	
27	62532D8	13240028	
28	62533~-D9	13240035	
29	62534D10	13240029	
30	62535D11	13240040	
31	62536D12	13240037	
32	62537J1	13240030	08/28/13
33	62538J2	13240008	08/28/13
34	62539J3	13240009	08/28/13
35	62540J4	13240010	
36	62541J5G	13240011	08/28/13

#	Ref	Description	Drawing#	Date
37	62542-	~J6G	13240012	08/28/13
38	62543-	-J7G	13240013	08/28/13
39	62544-	-J8	13240014	08/28/13
40	62545-	-J9G	13240015	08/28/13



130 mph wind, 24.16 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18 Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E

Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load factor for dead load is 1.50. Creep increase

design.

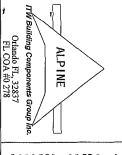
Special loads TC- From TC- From BC- From Dur.Fac.=1.25 / I 60 plf at -0.63 t 60 plf at 5.45 t 4 plf at -0.63 t / Plate Dur.Fac.=1.25)
i3 to 60 plf at 5.45
i5 to 60 plf at 11.53
i3 to 4 plf at 11.53

Gable end supports 8" max rake overhang

See DWGS A14030ENC100212 & GBLLETIN0212 for more requirements.

The overall height of this truss excluding overhang is 5-1-1. In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

See Detail PB160100212 for piggyback details. R=16 Rw=78 U=71 RL=107/-107 0-0-4 2X4(B1) ₩=5.167" (5.167)min) 5-5-7 10 \Box 12-2-0 Over 3 Supports 10-10-14-4X4 = 卣 10 2X4 (B1) VD-7-9



Design Crit: FBC2010Res/TPI-2007(STD)

(5 168" min.)

FL/-/5/-/-/R/-

Scale = .5"/Ft.

FT/RT=20%(0%)/10(0)

Note: All Plates

Are 2X4 Except As Shown. R=66 PLF U=18 PLF W=10-10-14

PLT TYP. Wave

WARNING REDO AND FOLLOW ALL NOTES ON THIS SHEET!

***UNDORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS (LOCATION INSTALLERS

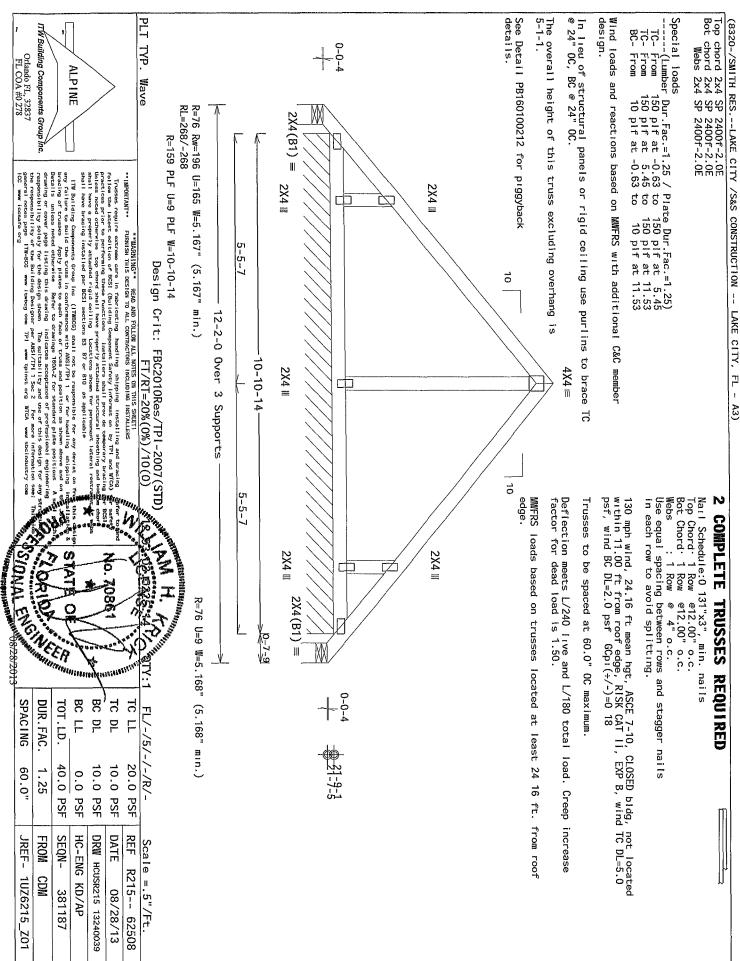
Trusses require extreme are in charlcating bandling shipping installing and bracing Report to follow the latest edition of BGS! (Building Component Safety Informat on by TPI and WTCA) tog safety practices prior to performing these functions installers shall prov de temporary bracing ped BGS!, the practices prior to performing these functions installers stall prov de temporary bracing ped BGS!, the practices of the practices of the property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have a property attached rigid ceiling Locations shown for permanent lateral restrains of the shall have been considered to the shall have been considered to the shall be a property attached rigid ceiling the shall be a prop practices prior to performing these functions Installers shall prove tem Unless noted otherwise top chord shall have properly attached structural six shall have a properly attached rigid celling Locations shown for permanent shall have bracing installed per BCSI sections 83 87 or 810 as applicable

bracing of trusses here the feet of the design the design responsibility solely for the design the responsibility of the Building by the responsibility of the design the responsibility of the Mulling by ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation my failure to build the truss in conformance with MSI/TPI 1 or for handling shipping racing of trusses Apply plates to each face of truss and position as shown above and o see com

SSONAL ENGAGE TOT BC LL BC DL TC LL SPACING H F DUR. FAC. TOT.LD. 1.25 10.0 PSF 20.0 PSF 40.0 PSF 10.0 PSF 24.0" 0.0 PSF SEQN-DATE REF FROM HC-ENG DRW HCUSR215 13240001 JREF-R215-- 62506 1UZ6215_Z01 KD/AP 380988 08/28/13

Deflection | factor for (130 mph wind, 24.16 ft mean hgt, ASCE 7-10, CLOSED bidg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18See Detail PB160100212 for piggyback details. Wind loads and reactions based on MWFRS with additional C&C member Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E PLT TYP. ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE Wave meets L/240 live and L/180 total load. Creep increase dead load is 1.50. RL=107/-107 R=30 Rw=79 U=66 W=5.167" 2X4(B1) =R=64 PLF U=16 PLF W=10-10-14 Trusses require extreme care in fabricating shaping shipping installing and bracing practices prior to performing these functions. Installers shall have properly attached rigid celling. Locations shown for permanent lateral restrict. When the shaping shaping shaping the stopping to performing these functions. Installers shall provide temporary bracing are BSIs builts now properly attached structural shaething and bottom shall have properly attached rigid celling. Locations shown for permanent lateral restricted shall have properly attached rigid celling. Locations shown for permanent lateral restricted shall have beauting installed per BSIs sections 88 87 or 810 as applicable.

I'll Building Components Group inc (I'll BGC) shall not be responsive value or reaching intruses Apply plates to research results unless noted or research results. 2X4 III 2X4 III 5-5-7 (5.167" min.) 70 12-2-0 Over 3 Supports 10-10-14 2X4 III 4X4≡ 10 5-5-7 Special loads
-----(Lumber
TC- From
TC- From
BC- From The overall height of this truss excluding overhang is 5-1-1. Design Dead Loads 1.00 PSF In lieu of rigid ceiling use purlins to brace BC @ 2X4 Ⅲ 2X4 Ⅲ AND THE PROPERTY OF THE PARTY O Dur.Fac.=1.25 60 plf at -0.66 60 plf at 5.46 4 plf at -0.66 0 plf at -0.63 t 0 plf at 5.45 t 4 plf at -0.63 t O CONTRACTOR OF THE PARTY OF TH R=30 U=10 W=5.168" (5.168" min.) 2X4(B1) =based on material weight adjusted for slope (TENTY:12 FL/-/5/-/-/R/-5 6 6 Plate BC LL BC DL 60 plf at 5 45 60 plf at 11.53 4 plf at 11.53 TC DL TC LL SPACING DUR.FAC. TOT.LD. Dur, Fac, =1, 25) 40.0 10.0 PSF 20.0 PSF 1.25 10.0 PSF 24.0" 0.0 PSF PSF 24" DATE FROM REF R215-- 62507 JREF-SEQN-HC-ENG DRW HCUSR215 13240038 Scale =.5"/Ft 1UZ6215_Z01 KD/AP 08/28/13 380990



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

In lieu of rigid ceiling use purlins to brace BC @ 24" 8

Truss passed check for 20 psf additional bottom chord live load areas with $42"\hbox{-high} \times 24"\hbox{-wide clearance.}$ Ξ.

The overall height of this truss excluding overhang is 7-6-6.

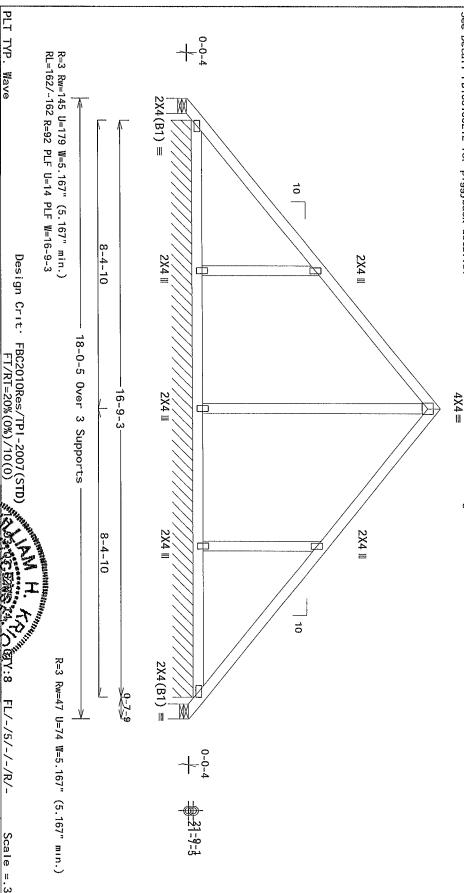
See Detail PB160100212 for piggyback details

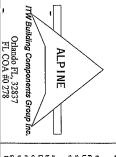
130 mph wind, 25.39 ft mean hgt, ASCE 7-10, within 9.00 ft from roof edge, RISK CAT II, wind BC DL=2.0 psf. GCpi(+/-)=0.18CLOSED bldg, not located EXP B, wind TC DL=5.0 psf.

design. Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

edge. MWFRS loads based on trusses located at least 12.69 ft. from roof





PLT TYP.

Wave

Trusses require extreme care in febricating being installers installers and prollow the latest edition of BESI (Building Component Safety Informating and bracing and bracing these controls the installers installers are performing these controls on the installers are performing these controls on the installers are performing these controls on the installers and bracing installers are performing these controls on the installers and better the section of BESI (Building Component Safety Information to be performed to the installers and performed stallers are performed to the installers are performed tor performed to the installers are performed to the installers are

TANOL

SPACING

JREF-

1UZ6215_Z01

DUR. FAC.

1.25 24.0"

TOT.LD.

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381282

E SOLD THE STATE OF THE STATE O

BC LL BC DL

0.0 PSF PSF

HC-ENG

KD/AP

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR215 13240016

TC DL

DATE REF

08/28/13

TC LL

FL/-/5/-

Scale = .375"/Ft. R215-- 62510

130 mph wind, 25.39 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psi wind BC DL=2.0 psf. $GCp_1(+/-)=0.18$ Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance. 4X4 \equiv Wind loads and reactions based on MWFRS with additional C&C member Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E The overall height of this truss excluding overhang is 7-6-6PLT TYP. See Detail PB160100212 for piggyback details. design. ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE Wave R=3 Rw=145 U=179 W=5.167" (5.167" min.) RL=18292182F U=14 PLF W=16-9-3 2X4(B1) =PARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!

IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLIONNE INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing rection of DESI (Building Component Safety Information by TP) and WTCA) registering precisions prior to performing these functions. Installiers shall provide temporary bracing peg BESI.

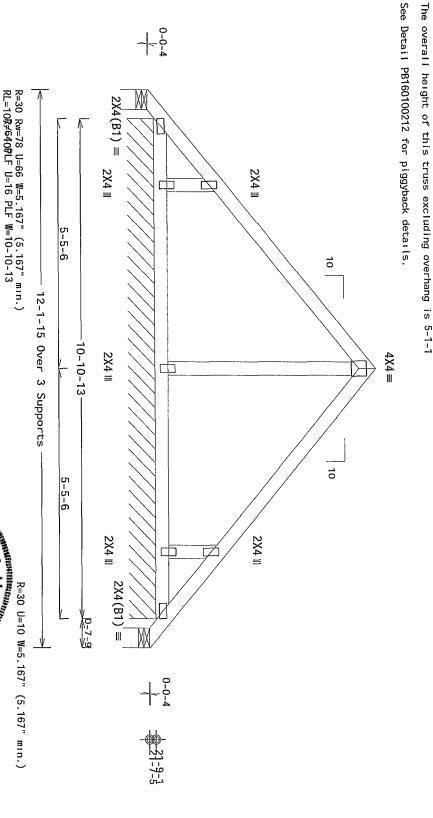
Unless noted otherwise to pended shall have properly attached structural shatching and bords against the properly attached structural shatching and bords against shall have a properly attached rigid ceiling. Locations shown for permanent lateral restrains shall have bracing installed per BESI sections 88 BF or 810 as applicable.

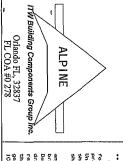
The Building Components Group Inc. (ITNESS) shall not be resource. 70 2X4 Ⅲ 2X4 Ⅲ Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0) 18-0-5 Over 3 Supports 2X4 III 16-9-3 psf Nail Schedule:0.131"x3", min. nails Top Chord 1 Row @12.00" o.c. Bot Chord 1 Row @12.00" o.c. Webs : 1 Row @ 4" o c. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. MWFRS loads based on trusses located at least 12.69 ft. In lieu of rigid ceiling use purlins to brace BC @ 24" in each row to avoid splitting Use equal spacing between rows and stagger nails COMPLETE 2X4 III 234 8-4-10 MAN BC BC **TRUSSES** 70 R 3 2X4(B1) REQUIRED Rw=47 U=74 W=5.167" (5.167" min.) V-7-9 BC DL BC LL TC DF SPACING DUR. FAC. TOT.LD. TC LL FL/-/5/-1.25 40.0 /-/R/-10.0 PSF 10.0 PSF 20.0 PSF 24.0" 0.0 PSF PSF from roof REF SEQN-DATE FROM HC-ENG DRW HCUSR215 13240017 JREF-Scale = .375"/Ft. R215-- 62511 1UZ6215_Z01 KD/AP 08/28/13 381300

130 mph wind, 24.17 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5 0 psf, wind BC DL=2.0 psf, GCpi(+/-)=0.18 Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E Special loads In lieu of rigid ceiling use purlins to 유수수 From (Lumber r Dur.Fac.=1.25 / Plate I 60 plf at -0.63 to 60 60 plf at 5.45 to 60 4 plf at -0.63 to 4 Te Dur.Fac =1.25)
60 plf at 5.45
60 plf at 11.53
4 plf at 11.53 brace BC @ 24" 8

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

는 기 TC LL

DATE

08/28/13

REF R215---

62512

Scale =.5"/Ft.

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR215 13240018

FL/-/5/-/-/R/-

I'W Building Components Group inc. (I'WECO) shall not be responsible for any deviation from any failure to build the trust in confinance with NBSI/TPI 1 or for handling shipping in beauting of trustees. Apply places to each feed of trusts and position as shown above and on the Desails unless moted otherwise. Before to drawings 160A-Z for standard plate positions. A position as necessary to the component of professional engineering responsibility soilty for the desain gone The suitability and use of this design for any the responsibility soilty for the Building Designer par MSI/TPI 1 Sec 2 for more information see

ORIONAL ENGINEERING BC BC DL SPACING BC LL DUR. FAC TOT.LD.

40.0

0.0 PSF PSF

HC-ENG

KD/AP

381010

1.25 24.0"

> FROM SEQN-

1UZ6215

JREF-

<u>|</u>201

130 mph wind, 20.12 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

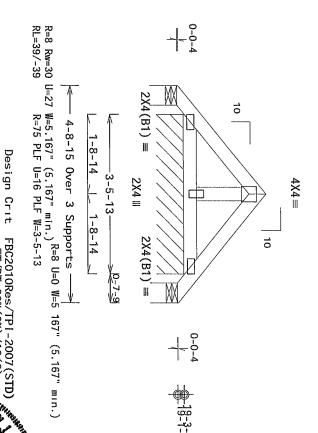
The overall height of this truss excluding overhang is 2-0-0.

See Detail PB160100212 for piggyback details.

Special loads -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC- From 60 plf at -0.63 to 60 plf at 1.74 TC- From 60 plf at 1.74 to 60 plf at 4.11 BC- From 4 plf at -0.63 to 4 plf at 4.11

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ALPINE

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

FT/RT=20%(0%)/10(0)

FL/-/5/-/-/R/-

Scale =.5"/Ft.

PLT TYP.

Wave

Trusses require extreme care in fabricating handling shipping installing and bracing follow the latest edition of ECSI (Euliding Component Sefety Information by IPI and WTCA) for practices prior to performing these functions. Installers shall provide temporary bracing page thiness noted otherwise top chord shall have properly attached structural sheathing and botter shall have a properly attached rigid colling locations shown for permanent lateral restrains shall have a properly attached rigid colling locations shall have a properly attached rigid ri

I'll Building Components Group Inc. (I'llBCD) shall not be responsible for any deviate on from any failure to build the transaction of the Massira in order to build the transaction of trusts and position as shown above and on the breains unless noted otherwise he extend of trusts and position as shown above and on the breains unless noted otherwise here to drawing 15004-Z for standard plates positions. A send drawing or converge listing this drawing indicates acceptance of orders soon against responsibility of the design shown. The suitability and use of this design for any structure responsibility of the duilding besigner per ARSJ/Pl | Sec 2 for more information can. The

MANDA E B BC DL BC LL 교 TC LL SPACING DUR. FAC. TOT.LD. 40.0 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 1.25 0.0 PSF PSF DATE FROM SEQN-REF HC-ENG DRW HCUSR215 13240019 JREF-R215-- 62513 1UZ6215_Z01 KD/AP 380808 08/28/13

Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E Special loads (Lumber

Wind loads and TC- From
TC- From
BC- From Dur.Fac.=1.25 / Plate Dur Fac.=1.25)
60 plf at -0.63 to 60 plf at 1.24
60 plf at 1.24 to 60 plf at 3.11
4 plf at -0.63 to 4 plf at 3.11 reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

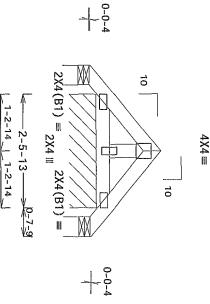
See Detail PB160100212 for piggyback details

COMPLETE TRUSSES REQUIRED

Nail Schedule.0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs: 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

130 mph wind, 19.91 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT iI, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $G_{\rm CP}(+/-)=0.18$

The overall height of this truss excluding overhang is 1-7-0.



1-2-14 1-2-14

<3-8-15 Over 3 Supports>

PLT TYP.

Wave

WARNING* READ AND FOLLOW ALL NOTES OR THIS SHEET!

IMPORTANT* FURNISH THIS DESIGN TO ALL CONFROCTIONS INCLUDING INSTALLERS

Trussees require extreme care in rabificating handling shipping installling and bracing sheet of the practices prior to perform on these functions installers shall provide temporary bracing sheet practices prior to perform on these functions installers shall provide temporary bracing besting the properly attached structural sheathing and bottom shall have properly attached structural sheathing and bottom shall have properly attached registeration of the certains shown for permanent lateral restrains shall have bracing installed per BCSI sections SB ET or B10 as applicable

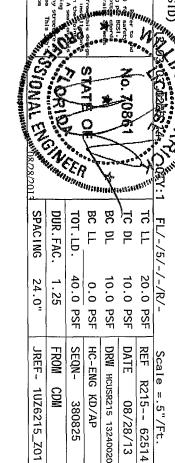
ITW Building Components Group Inc. (ITMBCC) shall now a applicable

ITW Building Components Group Inc. (ITWBCC) shall not be responsible for any deviation from any failure to build the truss in conformance with AMSI/TPI i or for handling shipping in bracing of trusses Apply places to each face of truss and position as shown above and on the betails unless noted otherwise Refer to drawings 160A-Z for standard plate positions. A

ITM Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278



130 mph wind, 19.91 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member

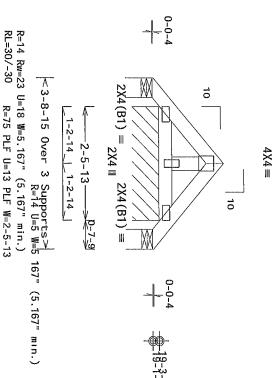
The overall height of this truss excluding overhang is 1-7-0

See Detail PB160100212 for piggyback details

Special loads TC- From TC- From BC- From ----(Lumber r Dur.Fac.=1.25 / Plate I 60 plf at -0.63 to 60 60 plf at 1.24 to 60 4 plf at -0.63 to 4 te Dur.Fac.=1.25)
60 plf at 1.24
60 plf at 3.11
4 plf at 3.11

ln lieu of rıgıd ceiling use purlıns to brace BC @ 24" 8

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ALP I NE

ITW Building Components Group

Orlando FL, 32837 FL COA #0 278

PLT TYP.

Wave

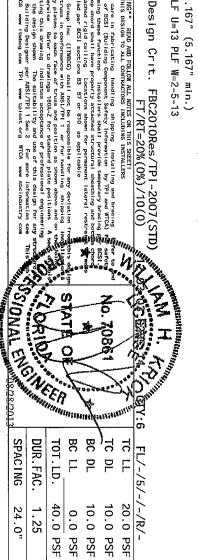
WARNING READ AND FOLLOW ALL WOITES ON THIS SHET!

IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLINION INSTALLERS

Trusses require excreme care in fabricating handling shipping installing and bracing follow the lacest edition of BCSI (Building Component Safety information by TPI and WTCA). ** safety practices prior to performing these functions interesting shall provide temporary bracing and BCSI (Building Component Safety information by TPI and WTCA). ** safety practices prior to performing these functions. Interesting shall have properly attached structural sheathing and bottom shall have properly attached structural sheathing and bottom shall have bracing installed per BCSI sections 88 87 or 810 as applicable

[TW BLII I have bracing installed per BCSI sections 88 87 or 810 as applicable.]

ITM Building Components Group Inc (ITMM) any failure to build the truss in conformate bracing of trusses. Apply places to each Details unless noted otherwise Refer to drawing or cover page listing this drawing responsibility solely for the design shown the responsibility of the building besigness. enents Group Inc (ITM806) shall not be responsible for any deviation from time truss in conformance with ANSI/TPI 1 or for handling shipping in Apply places to seak face of truss and position as shown above and on a lotherwise Refer to drawings 160A-Z for standard plate positions. A



DATE

08/28/13

REF R215-- 62515

Scale = .5"/Ft.

DRW HCUSR215 13240021

SEQN-

380820

HC-ENG

KD/AP

FROM

JREF-

1UZ6215_Z01

130 mph wind, 19.91 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=2.0 psf. GCpi(+/-)=0.18TC DL=5.0 psf, wind BC

Wind loads and reactions based on MWFRS with additional C&C member

Deflection factor for meets L/240 live and L/180 total load dead load is 1 50. Creep increase

See Detail PB160100212 for piggyback details

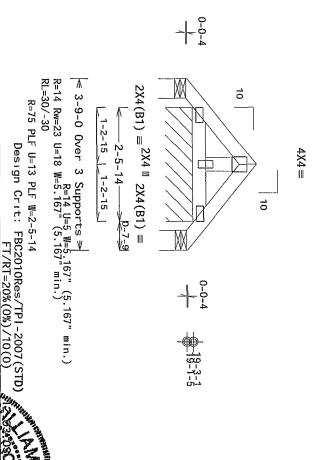
Special loads TC- From
TC- From
BC- From r Dur.Fac.=1.25 / Plate D 60 plf at -0.63 to 60 60 plf at 1.24 to 60 4 plf at -0.63 to 4 e Dur.Fac.=1.25)
60 plf at 1.24
60 plf at 3.12
4 plf at 3.12

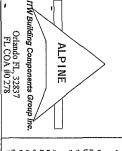
Gable end supports 8" max rake overhang

See DWGS A14030ENC100212 & GBLLETIN0212 for more requirements.

in lieu of rigid ceiling use purlins to brace BC @ 24"

The overall height of this truss excluding overhang is 1-7-0





PLT TYP.

Wave

Pesign Crit: FBC2010Res/TPI-2007(STD)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

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**WARNIN shall have a properly attached rigid ceiling Locations shown for permanent shall have bracing installed per BCSI sections 83 87 or B10 as applicable

I'll Building Components Group Inc (ITMBCG) shall not be responsibile for any deviction from any failure to build the truss in conformance that ANSI/TPI I or for handling shipping into bracing of trussess. Apply plates to each face of truss and position as shown above and on the Betails unless noted otherwise. Refer to drawings 180xPz for standard plate positions. As ser awaing or cover page listing this drawing indicates acceptance of professional engineering reaponsibility softsy for the design shown. The suitability and use of this design for any state the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see the professional page 17M-BCG www.tbiodeg.com. TPI www.tpinsto.org WTCA www.sbcindustry.com

O'IONAL ENGINEERS

SPACING

24.0" 1.25

JREF-

_Z01

DUR. FAC TOT.LD.

FROM SEQN-

일 1UZ6215_

380818

Some of the state of the state

40.0

BC DL

10.0 PSF 10.0 PSF 20.0 PSF

H H TC LL

DATE

08/28/13

REF R215-- 62516

Scale =.5"/Ft.

FL/-/5/-/-/R/-

BC LL

0.0 PSF PSF

> HC-ENG KD/AP DRW HCUSR215 13240022

PLT TYP. Wave Note: All Plates Are 2X4 Except As Shown. LOADS. OTHERS. In lieu of structural panels use purlins to brace all flat TC @ 24" 0C_{\cdot} See Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E + MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (a) Continuous lateral bracing equally spaced on member ITW Building Components Group Inc DWGS A14015ENC100212 & GBLLETIN0212 for more requirements Orlando FL, 32837 FL COA #0 278 R=80 PLF U=1 PLF W=24-9-0 RL=7/-7 PLF ALPINE ITW Building Components Group Inc (11WBCG) shall any failure to build the truss in conformance with A bracing of trusses Apply plates to each face of tr Details unless noted otherwise Refer to drawings Trusses require extreme care in febricating bandling shipping installing and bracing Refer to follow the latest edition of BCSI (Building Component Safety Information by TPI and WICA) register practices prior to performing these functions. Installing shall provide temporary bracing peg BCSI libiases noted otherwise top chord shall have properly attached structural sheathing and bottom captured the properly attached right celling. Locations shann for permanent lateral restraint shall have a properly attached right celling. **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET! 10-7-0 Design Crit. FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0) 70 24-9-0 Over Continuous Support (a) 2X4(**) Ⅲ 4 X 4 ≡ 5X5= 2X4(**) III (a) (a) Bottom chord checked for 10.00 psf non-concurrent live load. Wind loads and reactions based on MWFRS with additional C&C member 130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5 0 psf. 6Cpi(+/-)=0.18 (**) 2 plate(s) require special positioning. Refer to plot details for special positioning requirements. (a) ANDIS. By S 10 SPACING BC LL BC DL TC PL TC L DUR.FAC. 101.LD. FL/-/5/-/-/R/-40.0 10.0 PSF 20.0 PSF 1.25 10.0 PSF 24.0" $3X4(D1) \equiv$ 0.0 PSF PSF scaled plate FROM SEQN-DATE REF R215-- 62517 HC-ENG KD/AP DRW HCUSR215 13240023 JREF-Scale = .3125"/Ft 1UZ6215_Z01 380816 08/28/13

Top chord 2x4 Bot chord 2x4 Webs 2x4 4 SP 2400f-2.0E 4 SP 2400f-2.0E 4 SP 2400f-2.0E

in lieu of structural panels use purlins to brace all flat TC @ 24" $\,$ OC.

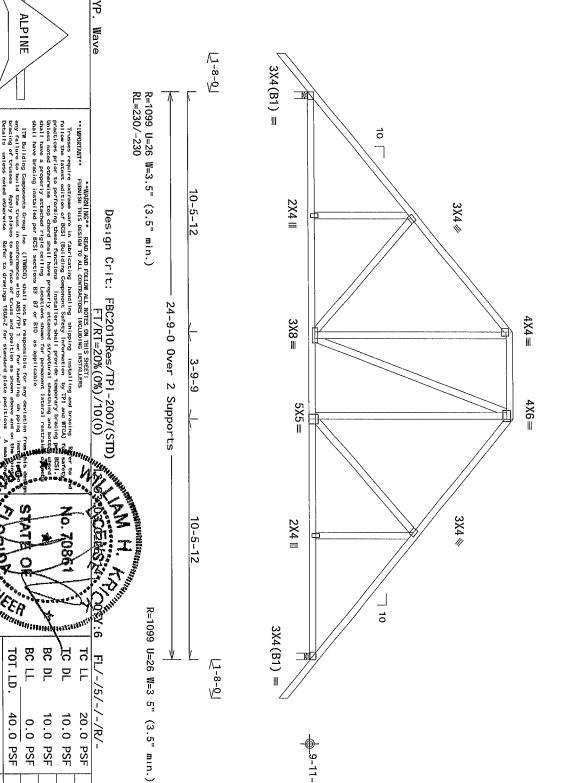
Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 9-1-9

130 mph wind, 15.00 ft mean anywhere in roof, RISK CAT | DL=5.0 psf. GCp:(+/-)=0.18 n hgt, ASCE 7-10, CLOSED bidg, Located II, EXP B, wind TC DL=5.0 psf, wind BC

design Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50 $\,$



PLT TYP. Wave

ITW Building Components Group Orlando FL, 32837 FL COA #0 278

ORIONAL ENGLISHMENT BC

BC DL BC LL

> 10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR215 13240002

0.0 PSF PSF

HC-ENG

KD/AP 380823

) C TC L

DATE REF

08/28/13

FL/-/5/-/-/R/-

Scale = .25"/Ft. R215-- 62518

SPACING

DUR. FAC. T01.LD.

1.25 24.0"

FROM

JREF-

1UZ6215_Z01

40.0

SEQN-

ALPINE

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Gable end supports 8" max rake overhang

See DWGS A14030ENC100212 & GBLLETIN0212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-9-0

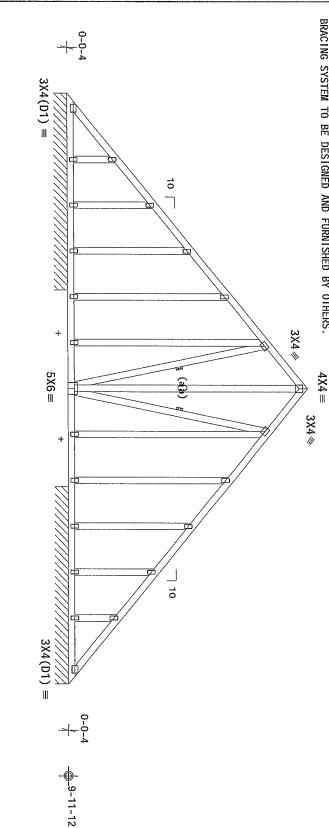
MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

130 mph wind, 15.50 ft mean anywhere in roof, RISK CAT | DL=5.0 psf. GCpi(+/-)=0.18 n hgt, ASCE 7-10, II, EXP B, wind TC DL=5.0 psf, wind BC

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member

Deflection meets $L/240\,$ live and $L/180\,$ total load. Creep increase factor for dead load is 1.50.





Note: All Plates Are 2X4 Except As Shown.

PLT TYP. Wave

ITM Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278

MANDI CINES . BC LL BC DL SPACING TC DL DUR. FAC. TOT.LD. 40.0 10.0 PSF 10.0 PSF 1.25 24.0" 0.0 PSF PSF

FROM

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JREF-

1UZ6215_Z01

SEQN-

380803

HC-ENG KD/AP DRW HCUSR215 13240003 TC LL

20.0 PSF

DATE REF

08/28/13

FL/-/5/-/-/R/-

Scale = .25"/Ft. R215-- 62520

In lieu of structural panels use purlins to brace all flat TC @ $24^{\prime\prime}$ OC

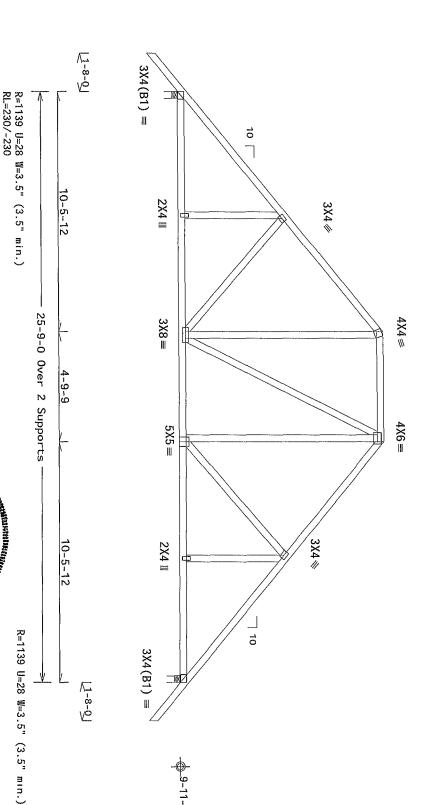
Bottom chord checked for 10.00 psf non-concurrent live load

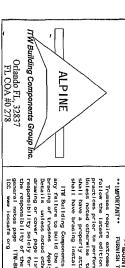
The overall height of this truss excluding overhang is 9-1-9

130 mph wind, 15.00 ft mean anywhere in roof, RISK CAT | DL=5.0 psf GCp:(+/-)=0.18 n hgt, AS ASCE 7-10, CLOSED bldg, Located (P B, wind TC DL=5.0 psf, wind BC

design. Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50 $\,$





PLT TYP. Wave

Design Crit: FBC2010Res/TP1-2007(STD) FT/RT=20%(0%)/10(0)

Trusses require extreme care in fabricating bandling shipping installing and bracing follow the latest edition of BCSI (Building Components Safety Information by Practices prior to performing these frontending Component Safety Information by Info

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from any failure to build the truss in conformance with ANSI/FP! 1 or for handling shipping in bracing of trusses Apply plates to each face of truss and position as shown above and on betails unless noted otherwise Refer to drawings 160A-Z for standard plate positions. A

SIONAL ENGINEERING BY

DUR. FAC.

TOT.LD.

40.0

SEQN-

380806

FROM

S

SPACING

24.0" 1.25

JREF-

1UZ6215_Z01

BC DL

10.0 PSF 10.0 PSF

DRW HCUSR215 13240004

TC DL TC LL

DATE

08/28/13

FL/-/5/-/-/R/-

20.0 PSF

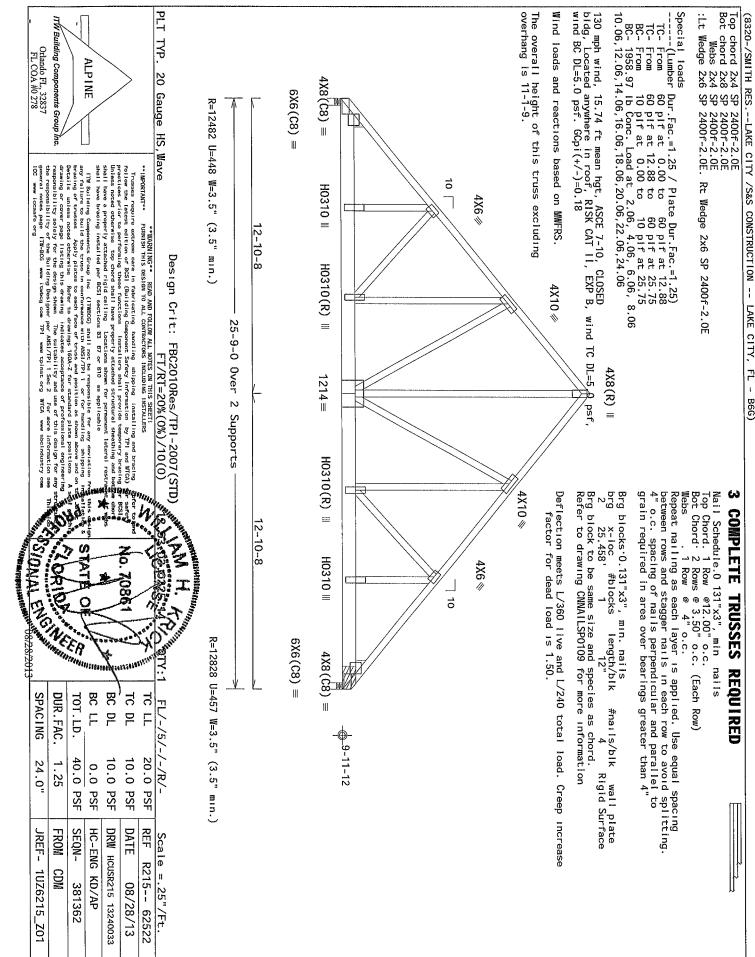
REF R215-- 62521

Scale = .25"/Ft.

BC LL

0.0 PSF PSF

HC-ENG KD/AP



DWGS A14015ENC100212 & GBLLETIN0212 for more requirements

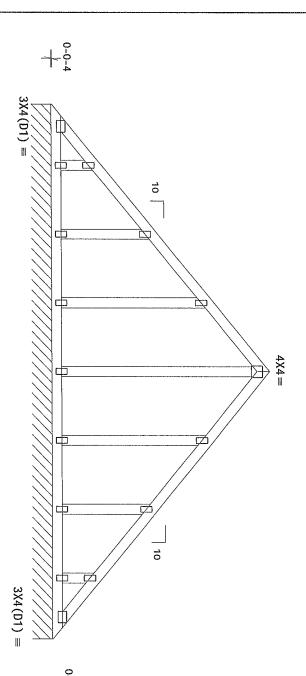
Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 6-5-15

130 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, RISK CAT II, EXP B, DL=5 0 psf. GCp:(+/-)=0.18 7-10, CLOSED bldg, Located wind TC DL=5.0 psf, wind BC

design. Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50.\,$



7-9-4 15-6-8 Over Continuous Support 7-9-4

R=80 PLF U=0 PLF W=15-6-8 RL=8/-8 PLF

Note: All Plates Are 2X4 Except As Shown. Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0)

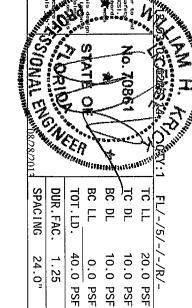
PLT TYP. Wave

Trusses require extreme care in fabricating handling shipting installing and bracing practices prior to performing these functions in Safety Information by Trustices prior to performing these functions in Safety Information by Trustices prior to performing these functions in Safety Information by Trustices are some properly stall in how properly attached structural sheathing and bottom shall have a properly attached rigid colling Locations shown for permanent interni restrains shall have a properly attached rigid colling Locations shown for permanent structures that in the properly attached rigid colling Locations shown for permanent structures that it is the properly attached rigid colling Locations shown for permanent structures that it is the properly should be properly attached that the properly should be properly attached that the properly should be presented to the properly should be properly attached to be presented to the properly attached to the prope

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278



DATE

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Scale = .375"/Ft. R215-- 62523

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380810

HC-ENG

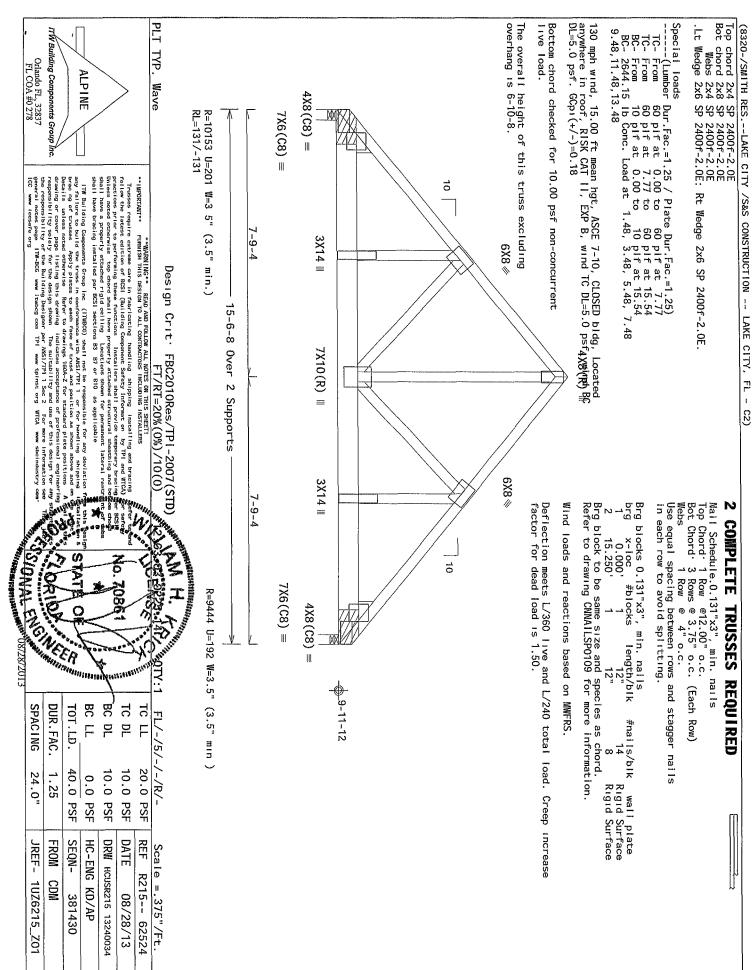
KD/AP

DRW HCUSR215 13240005

JREF-FROM

1UZ6215_Z01

CD.



Top chord Bot chord Webs 2x4 2x4 2x4 4 SP 2400f-2.0E 4 SP 2400f-2.0E 4 SP 2400f-2.0E

DWGS A14030ENC100212 & GBLLETIN0212 for more requirements

(a) Continuous lateral bracing equally spaced on member.

in fieu of structural panels use purlins to brace all flat TC @ 0C.

24"

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

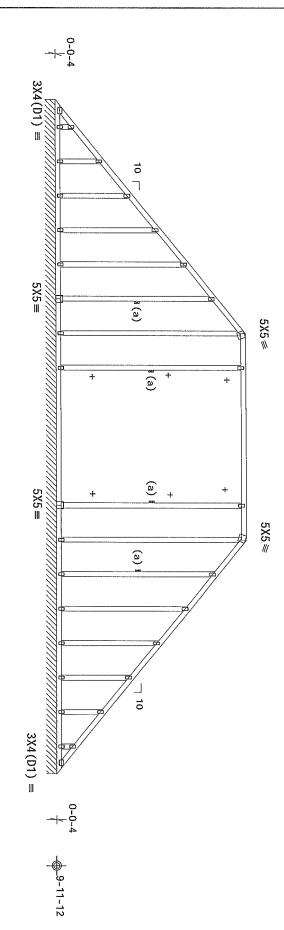
Design Dead Loads based on material weight adjusted for slope 1.00 PSF $\,$ TC.

130 mph wind, 15.79 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18 CLOSED bldg, l TC DL=5.0 psf, Located , wind BC

Wind loads and reactions based on MWFRS with additional C&C member

Bottom chord checked for 10.00 psf non-concurrent live load

The overall height of this truss excluding overhang is 11-4-1.



39-2-0 Over Continuous Support 13-7-0

PLT TYP. Wave Note: All Plates R=79 PLF U=2 PLF W=39-2-0 RL=6/-6 PLF Are 2X4 Except Trusses require extreme care in fabricating handling shipping installing and bracing follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) practices prior to performing these functions. Installers shall provide temporary bracing lineas noted otherwise top chord shall have properly attached structural sheathing and bot shall have a properly attached rigid celling. Locations shall have a properly attached rigid celling locations shall have a properly attached rigid celling. **IMPORTANT** **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS As Shown. Design Crit. FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0) STD) The state of the s BC DL BC LL TC DL TC LL FL/-/5/-/-/R/-10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF DATE REF HC-ENG DRW HCUSR215 13240006 Scale = .1875"/Ft R215-- 62525 KD/AP 08/28/13



ITW Building Components Group Inc (ITWEGS) shall not be responsible for any deviation from the first the state of the stat

08/28/2013

SPACING

24.0" 1.25

JREF-

1UZ6215_Z01

DUR. FAC. TOT.LD.

FROM

40.0

PSF

SEQN-

380986

Top chord 2x4 SP 2400f-2.0E T3 2x8 SP 2400f-2.0E:
Bot chord 2x10 SP 2400f-2.0E :B1, B6 2x6 SP 2400f-2.0E:
:B3 2x4 SP 2400f-2.0E:
Webs 2x4 SP 2400f-2.0E
:Lt Wedge 2x6 SP 2400f-2.0E
:Rt Wedge 2x6 SP 2400f-2.0E T3 2x8 SP 2400f-2.0E: :B1, B6 2x6 SP 2400f-2.0E:

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

In lieu of structural panels use purlins to brace all flat TC @ 0C.

Bottom chord checked for 10.00 psf non-concurrent live load

BC attic room floor 12-3-0 to 27-8-0. loading: LL = 40.00 psf; DL = 10.00 psf, from

The overall height of this truss excluding overhang is 11-7-9

Design Dead Loads 1.00 PSF based 9 material weight adjusted for slope ನ

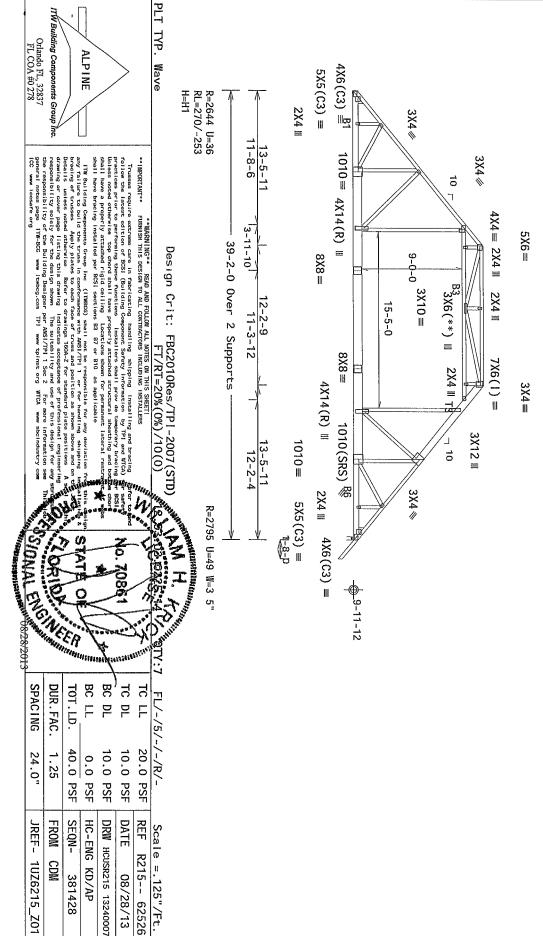
(l) - plates so marked were sized using a Fabrication Tolerance of and a Rotational Tolerance of θ degrees. 0%

130 mph wind, 15.30 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5 0 psf. GCpi(+/-)=0.18TC DL=5.0 psf, wind BC

Wind loads and reactions based on MWFRS with additional C&C member

Collar-tie braced with continuous lateral bracing at 24" ල . or rigid

Deflection meets $L/360~{\rm live}$ and $L/240~{\rm total}$ load. Creep increase factor for dead load is 1.50.



1UZ6215_Z01

CDM

KD/AP 381428

08/28/13

62526

Top chord 2x4 SP 2400f-2.0E Bot chord 2x10 SP 2400f-2.0E ·B3 2x4 SP 2400f-2.0E: Webs 2x4 SP 2400f-2.0E :B1, B6 2x6 SP 2400f-2.0E

130 mph wind, 15.99 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18 CLOSED bldg, Located TC DL=5 0 psf, wind BC

Calculated horizontal deflection is 0.12" due to dead load. due to live load and 0.22"

Bottom chord checked for 10.00 psf non-concurrent live load

BC attic room floor loading: LL = 40.00 psf; 12-3-0 to 27-8-0. DL = 10.00 psf; from

The overall height of this truss excluding overhang is 11-7-9

Design Dead Loads based on material weight adjusted for slope 1.00 PSF S

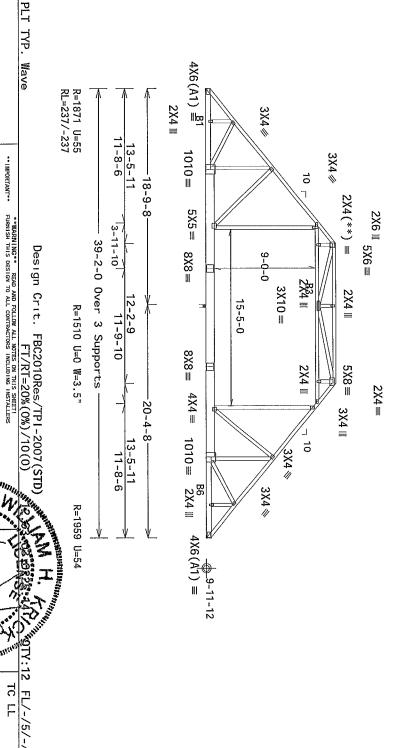
(**) 1 plate(s) require special positioning. Refer
plot details for special positioning requirements. 6 scaled plate

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" $\,$ OC.

Collar-tie braced with continuous celling. lateral bracing at 24" <u>ල</u> or rigid

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in rebricating handing shipping installing and bracing.

For two perform of RSSI (Sulfilling Component Safety Informat on by JPI) and WICA) are stood of the season of the performing these functions. Installings shall now the property attached rigid celling Component Safety Informat on by JPI and WICA) are stood of the performing these functions. Installings shall have bracing installing her RSSI sections \$18.00 per performance of the performance of th

I'll Bu Iding Components Group Inc. (I'll BCC) stall not be responsible for any deviation for this any fall une to be responsible for any deviation for the service of the control of the

OJONAL ENGINEERING

SPACING DUR. FAC. TOT.LD.

24.0"

JREF-

1UZ6215_Z01

1.25 40.0 PSF

FROM

S

BC DL BC LL

> 10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR215 13240031

0.0 PSF

HC-ENG

KD/AP

SEQN-

381353

TC DL

DATE

08/28/13

REF

R215-- 62527

/-/R/-

Scale = .125"/Ft.

Top chord 2x4 SP 2400f-2.0E Bot chord 2x10 SP 2400f-2.0E :B3, B7 2x4 SP 2400f-2.0E: Webs 2x4 SP 2400f-2.0E .B1, B6 2x6 SP 2400f-2.0E

130 mph wind, 15 94 ft mean hgt, ASCE 7-10, anywhere in roof, RISK CAT II, EXP B, wind DL=5.0 psf. GCpi(+/-)=0.18 TC DL=5.0 psf, wind BC

In lieu of structural panels use purlins $0\ensuremath{\text{C}}.$ 6 brace all flat TC @ 24"

BC attıc room floor loading: LL = 40.00 psf; 12-3-0 to 27-8-0. DL = 10.00psf,

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Design Dead Loads based on 1,00 PSF material weight adjusted for slope ਨ .

(**) 1 plate(s) require special positioning. Refer plot details for special positioning requirements. to scaled plate

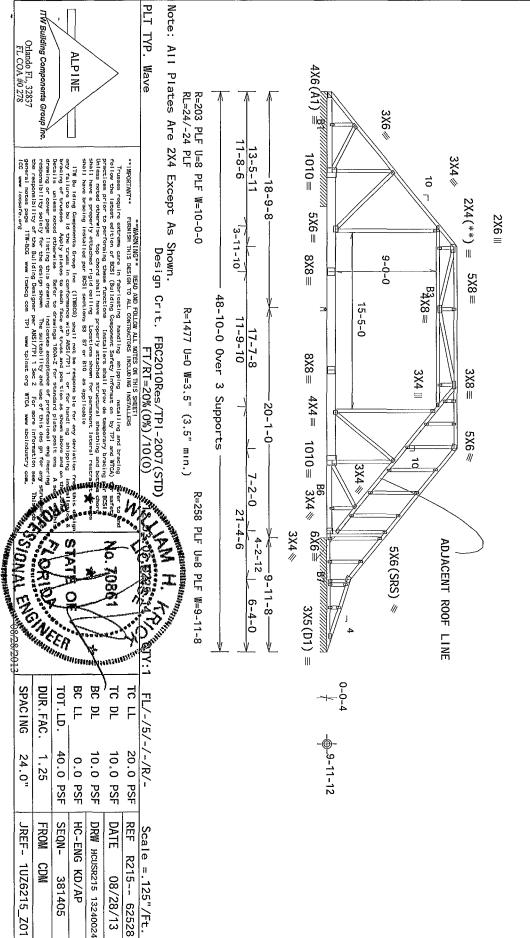
Bottom chord checked for 10.00 psf non-concurrent live load

Wind loads and reactions based on MWFRS with additional C&C member

Collar-tie braced with continuous lateral bracing at 24" OC or rigid

Deflection meets L/360 live and L/240 total factor for dead load is 1 50. load. Creep increase

The overall height of this truss excluding overhang is 11-7-9



ITW Building Components Group Orlando FL, 32837 FL COA #0 278

SIONAL

SPACING DUR. FAC. TOT.LD.

24.0" 1.25

JREF-

1UZ6215_Z01

40.0

PSF

381405

FROM SEQN-

(8320-/SMITH RES.--LAKE CITY

/S&S CONSTRUCTION --

LAKE CITY, FL -

D5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

PLT TYP. Design Dead Loads based on material weight adjusted for slope $1.00\ \mathsf{PSF}$ Top chord 2x4 SP 2400f-2 OE Bot chord 2x10 SP 2400f-2.0E Webs 2x4 SP 2400f-2 OE Deflection meets L/360 live and L/240 total load factor for dead load is 1.50. Collar-tie braced with continuous lateral bracing at 24" Trusses to be spaced at 37 0 In lieu of structural panels use purlins to brace Special loads ITW Building Components Group overall height of this truss excluding overhang is 11-7-9. July 25 / Pi.

July 25 / Pi.

July 27 / Pi.

July 27 / Pi.

From 133 pif at 10 25 to 1.

From 125 pif at 13 48 to 1.

From 125 pif at 22.52 to 12.

From 33 pif at 31 56 to 93 pif at 32.98 to 93 pif at 32.98 to 93 pif at 42 65 to 93 pif at 42 65 to 154 pif 154 pif at 12.46 to 154 pif 154 pif at 12.46 to 154 pif 154 pif at 23.46 to 154 pif at 93 pif at 23.46 to 154 pif at 93 pif at 23.46 to 154 pif at 93 pif at 23.48 to 31 pif at 31 pif at 29.80 to 31 pif at 31 pif at 29.80 to 31 pif at 7 pif at 48.83 to 7 pif at 7 pif at 48.83 to 7 pif at 7 pif at 48.83 to 7 pif at 25.27 6 Conc. Load at 12 25.27 6 Conc. Load at 11 02 20 conc. Load at 13 02.15 02.17 02

July 27 / Pif 28 / Pif 28 / Pif 29 / Pif 29 / Pif 29 / Pif 20 / Orlando FL, 32837 FL COA #0 278 chord checked for 10 00 psf non-concurrent live load Furnish a copy of this DWG to the installation contractor care must be taken during handling, shipping and installation es. See "MARNING" note below. **ALPINE** 20 Gauge HS, Wave Trusses require extreme care in fabricating handling shipping installing and bracing practices prior to perform a fuel shall have properly attached rigid celling. Loadins shall have a properly attached rigid celling. Loadins shall have properly attached rigid celling. Loadins shown for handling any principles of trusses and the three propers and the properly attached rigid celling. Loadins shown for personant lateral restrict the shall have properly attached rigid celling. Loadins shown for personant lateral restrict the shall have properly attached the properly attached rigid celling. Loadins shown for personant lateral restrict the shall have properly attached the properly attached rigid celling. Loadins shown for personant lateral restrict the shall have properly attached the properly attached rigid celling. Loadins shown for personant lateral restrict the shall have properly attached rigid celling. Loadins shown for properly attached rigid celling. Loadins shown for properly attached rigid celling. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shown above and on the shall have properly attached rigid. Loadins shall have properly attached rigid. Loadins shall have properly attached rigid the state of the shall have properly attached rigid the state of the shall have properly attached rigid the state of the shall have properly attached rigid the state of the shall have properly attached rigid the state of the shall have properly attached rigid that the sh OC maximum. В1, late Dur Fac. =1.25)
93 pif at 12 25
93 pif at 22 25
125 pif at 23 25
125 pif at 32 88
93 pif at 42 65
93 pif at 42 65
93 pif at 42 65
93 pif at 50 65
154 pif at 27.67
154 pif at 27.67
154 pif at 23 48
93 pif at 32 32
154 pif at 23 48
154 pif at 23 48
154 pif at 32 32
17 pif at 48 33 31 B6 2x6 SP 2400f-2 02 Creep increase TC @ 24" 0C ನ 4X5(A1) =1四年 10-0-0-R=3460 U=151 W=3 5" (3 5" min.)
RL=410/-434 R=2388 U=144 W=3.5" (3.5" min.)
RL=410/-434 R=2388 U=144 W=3.5" (3.5" min.) 2X4 Ⅲ 3X4 ∕ 四四 13-5-12 5X6 ∅ 1010= 6 3X4 ≡ 4X5 < ₩ 2X6 III 4X14(R) III 130 mph wind 15.30 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5 0 psf, wind BC DL=5 0 psf, GCpi(+/-)=0 18 (**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements. 2 COMPLETE TRUSSES REQUII

Nail Schedule 0 131"x3", min nails

Top Chord 1 Row @11 00" o.c.

Bot Chord 1 Row @12 00" o.c.

Webs 1 Row @ 4" o.c.

Use equal spacing between rows and stagger in each row to avoid splitting Wind loads and reactions based on NWFRS Negative reaction(s) of -206# MAX (See below) from a non-wind load case requires uplift connection 2X4 Ⅲ 5X8≡ MWFRS loads based on trusses located Left cantilever is exposed to wind 8X8 ≡ 3X4(**) ∅ 48-10-0 Over 4 Supports 4X5 = 9-0-8 SIONAL ENGINEER 5X8≡ 2X4 III 3X4≡ 8X8 4X5≡ 30-8-0-2X4 III H0510(R) Ⅲ 9-0-7 2X4 III 5X8≡ 1214 =196423 REQUIRED 6X10(R) || 2X4 || 3X4∥ √ 10 at least 30 BC LL BC DL TC DL 9-3-12 6X12 ∥ SPACING DUR.FAC. TC LL TOT.LD. FU-, R=5150 U=154 W=3.6" $4X5(R) \parallel 4X5(A1) =$ R=239/-207 U=60 W=3.5" (3.5" min.) 4X5(R) Ⅲ →--8-0-0-¥⁸50 စ္ပ 6-2-4 5X6(SRS) ≈ 20.0 40.0 10.0 PSF 10.0 PSF 37.0" 1.25 0.0 -/R/ ŧ PSF PSF PSF (3.5" min.) DATE REF FROM SEQN-HC-ENG DRW HCUSR215 13240026 JREF-Scale = .09375"/Ft R215---1UZ6215_Z01 KD/AP 08/28/13 381403 62530

Top chord 2x4 SP 2400f-2.0E Bot chord 2x10 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E :B1, B6 2x6 SP 2400f-2.0E

130 mph wind, within 13.00 f psf, wind BC D , 15.30 ft mean hgt, ASCE 7-10, CLOSED bldg, not located ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 DL=5.0 psf. GCpi (+/-)=0.18

Left cantilever is exposed to wind

28 lieu of structural panels use purlins to brace all flat C @ 24"

Collar-tie braced with continuous lateral bracing at 24° ceiling ၀ က or rigid

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

The overall height of this truss excluding overhang is 11-7-9 $2X6\ \ensuremath{\mathbb{H}}$

Design Dead Loads based on material weight adjusted 1.00 PSF for slope

(**) 1 plate(s) require special positioning. Refer plot details for special positioning requirements. 6 scaled plate

Wind loads and reactions based on MWFRS with additional C&C member

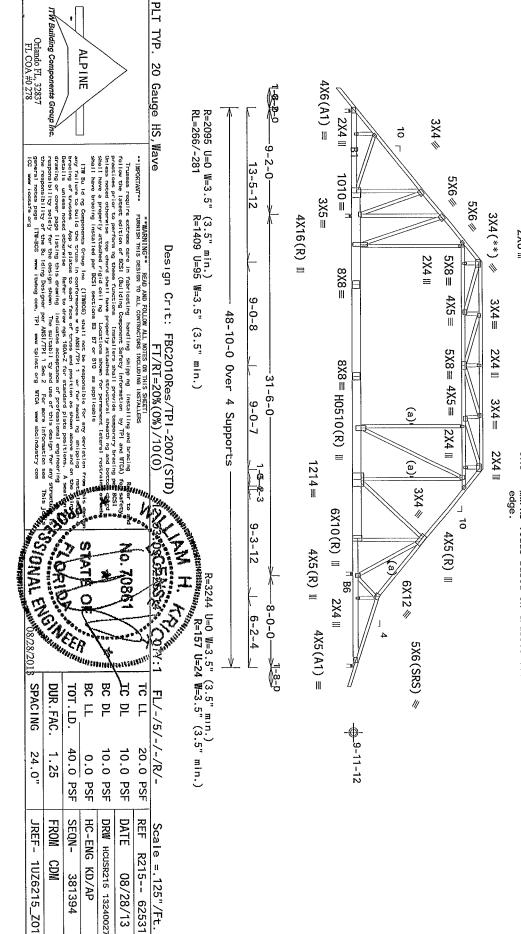
(a) Continuous lateral bracing equally spaced on

Bottom chord checked for 10 00 psf non-concurrent live load

BC attic room floor loading LL = 40.00 12-3-0 to 27-8-0. psf, DL = 10.00 psf, from

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

5X8≡ MWFRS loads based on trusses located at least 30.60 ft. from roof



ITW Building Components Group

Orlando FL, 32837 FL COA #0 278

SPACING

24.0" 1.25

JREF-

1UZ6215_Z01

DUR. FAC.

FROM

₹.

lop chord Bot chord 130 mph wind, 15.30 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18chord 2x4 SP 2400f-2.0E chord 2x10 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E . В1, **B6** 2x6 SP 2400f-2.0E Design Dead Loads based on material weight adjusted for slope 1 00 PSF g scaled plate

(**) 1 plate(s) require special positioning. Refer plot details for special positioning requirements.

Wind loads and reactions based on MWFRS with additional C&C member

(a) Continuous lateral bracing equally spaced on

Bottom chord checked for 10.00 psf non-concurrent live load

BC attic room floor loading LL = $40.00 \, \mathrm{psf}$; DL = $10.00 \, \mathrm{psf}$, from $12\text{-}3\text{-}0 \, \mathrm{to} \, 27\text{-}8\text{-}0$.

Collar-tie braced with continuous lateral bracing at 24° ceiling.

In lieu of structural panels use purlins to brace all flat TC @ 0C.

Left cantilever is exposed to wind

Deflection meets L/360 live and L/240 total load. factor for dead load is 1.50.

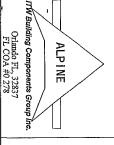
Creep increase

OC. or rigid

24"

WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 11-7-9. 4X5(A1) =福 R=1886 U=21 W=3.5" (3.5" mın.) RL=266/-281 R=1587 U=95 W=3.5" 2X4Bit 3X4 ∅ -7-2-0-13-5-12 1010≡ 4X16(R) III 5X6 ≪ 9 4X4 =5X6 🕢 2X6 III 5X8≡ 2X4 III 4X5 ≡ 3X4(**) ∅ 9-0-8 3X4≡ (3.5" min) 48-10-0 Over 4 Supports 2X4 III 5X8 = 4X5 =8X8 ≡ 33-6-0 4X14(R) 3X4≡ 9-0-7 <u>a</u> 2X4 ≡ 2X4 III 5X8≡ (a) 1214≡ -G-A-3 3X4 ∥ 6X10(R) Ⅲ 70 H. Tourney 9-3-12 4X5(R) Ⅲ 4X5(R) Ⅲ R=3228 U=76 W=3.5" В6 6X12// 8-0-0-2X4 III 6-2-4 5X6(SRS) ∥ 4X5(A1) =(3.5" mın.) ₩=3.5" (3.5" min.) ____9-11-12



PLT TYP.

Wave

"WARNING"* READ AND FOLLOW ALL NOTES ON THIS SHEET!

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in few-locating hand many shipping installing and bracing for the pre-close functions installers shall provide temporary bracing per gold in the property students as the provide temporary bracing per gold in the appropriate the provide temporary bracing per gold in the property students as the provide temporary bracing per gold in the appropriate the provide temporary bracing per gold in the property students as shall have properly students as supplicable.

If we build in the components from the cities of the per gold shall have bracing in restallers and provide the property students as the property students. The property students as the propert I'W Building Components Group Inc. (ITBEOS) shall not be respons ble for any deviation from any fall lure to build the truss in conformance we than NBI/TPI 1 or for handling shipping into breatis unless noted otherwise. Refer to drawings 160x2 for standard plate positions and on the Details unless noted otherwise. Refer to drawings 160x2 for standard plate positions. A drawing or cover page listly ghis drawing indicates acceptance of professional engineering responsibility polely for the design shown. The su tability and use of this design for any state responsibility of the Building Designer per NBI/TPI 1 Sec. 2 for more information see the responsibility of the Building Designer per NBI/TPI 1 seven the component of the personal policy of the design for any state proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the proposability of the Building Designer per NBI/TPI 1 seven the prop

OSIONAL ENGINEER

SPACING DUR.FAC. TOT.LD.

24.0" 1.25

JREF-

1UZ6215_Z01

Selection of the Select

40.0

PSF

SEQN-

381388

FROM

BC LL BC DL TC DL TC LL

0.0 PSF 10.0 PSF

HC-ENG

KD/AP

DRW HCUSR215 13240028

FL/-/5/-/-/R/-

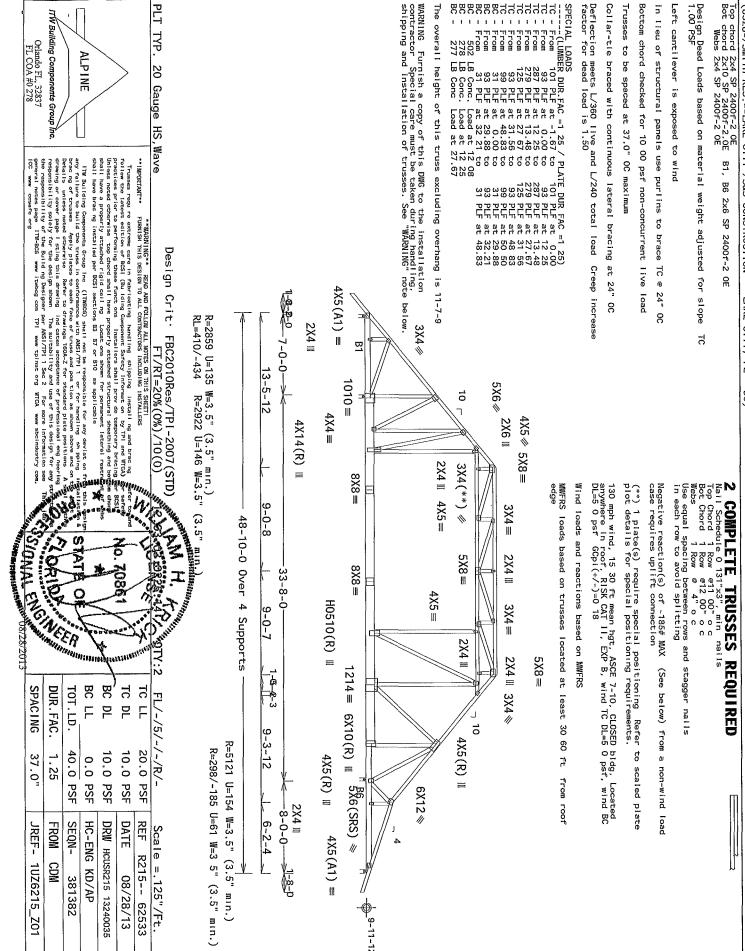
10.0 PSF 20.0 PSF

DATE

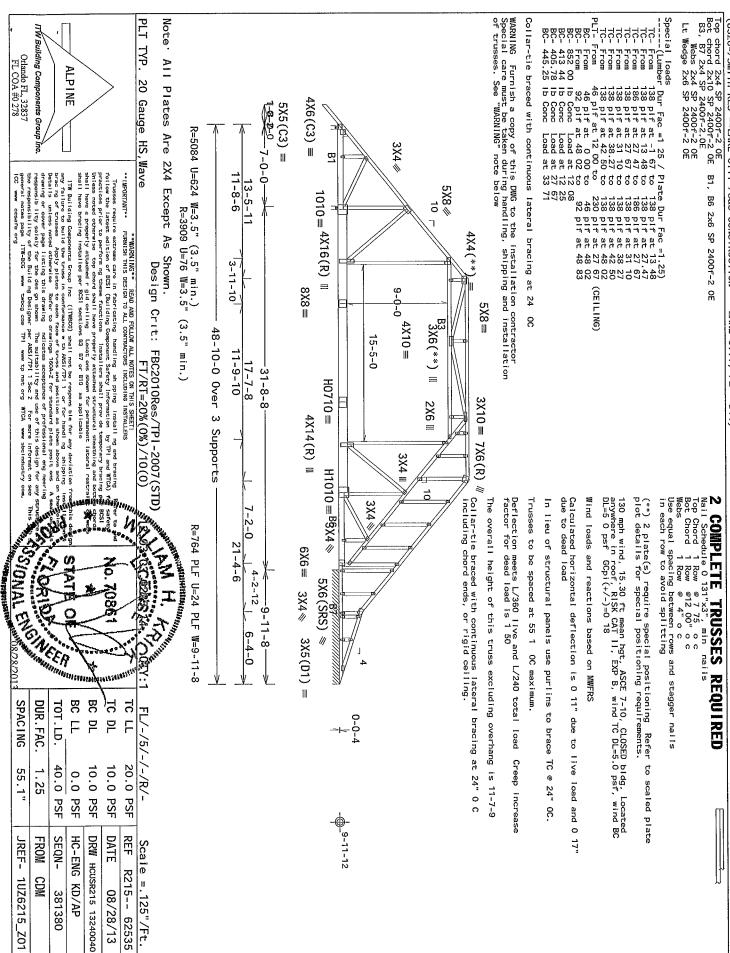
08/28/13

REF R215-- 62532

Scale =.125"/Ft.



Collar-tie ceiling. 130 mph wind, 15.30 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, (+/-)=0.18PLT TYP. Top chord 2x4 SP 2400f-2.0E Bot chord 2x10 SP 2400f-2.0E :B1, Webs 2x4 SP 2400f-2.0E The overall height of this truss excluding overhang is 11-7-9. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to 0C.Left cantilever is exposed to wind ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE Wave braced with continuous lateral bracing 4X5(A1) =7-0-0-R=1818 U=25 RL=266/-281 2X4Bil 3X4 ∅ I'll Building Components Group Inc. (I'llBECS) shall not be respons the for any deviation from the same of the form of the same of the sam <u>1</u>3-5-12 1010 = 4X14(R)W=3.5" (3.5" mın.) R=1698 U=102 W=3.5" 5X6 **∅** 6 В6 3X4(**) « 2x6 SP 2400f-2.0E 5X8≡ 2X6 Ⅲ 2X4 III brace =8X8 4X5≡ at all flat 9-0-8 3X4≡ (3.5" min.) 24" 48-10-0 Over 8 TC 2X4 III 5X8≡ 8X8≡ or rigid 33-8-0 4X5≡ 4 Supports 4X14(R) 3X4≡ <u>a</u> 2X4 Ⅲ/ 2X4 III 5X8≡ (a) 1214≡ G-12-3 * 19 E WARNING Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. BC attic room floor loading LL = 40.00 psf, DL = 10.00 psf; from 12-3-0 to 27-8-0. Bottom chord checked for 10.00 psf non-concurrent live load design. Wind loads and reactions based on MWFRS with additional C&C member (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. Design Dead Loads based 1.00 PSF (a) Continuous lateral bracing equally spaced on 3X4 ∕/ N. H. TOWN 6X10(R) Ⅲ SIONAL ENG NUMBER BL. 70 9-3-12 4X5(R) Ⅲ 4X5(R) III ⊓ B6 R=3263 U=75 W=3.5" R=153 U=28 6X12 ≈ 2X4 III -8-0-0-6-2-4 9 $4X5(A1) \equiv$ material weight adjusted for slope 5X6(SRS) ∥ BC LL BC DL (3.5" mın.) ₩=3.5" (3.5" mın.) SPACING JG PL DUR. FAC. TOT.LD. TC LL FL/-/5/-/-/R/-20.0 PSF 40.0 10.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF DATE SEQN-DRW HCUSR215 13240029 REF R215-- 62534 FROM HC-ENG JREF-Scale = .125"/Ft. T_C 1UZ6215_Z01 KD/AP 08/28/13 381355



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Left end vertical not exposed to wind pressure

Bottom chord checked for 10.00 psf non-concurrent live load

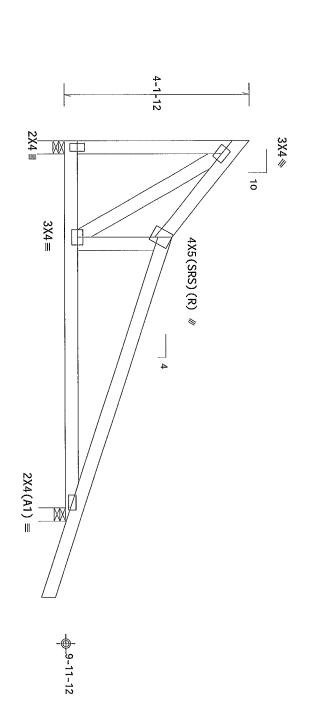
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

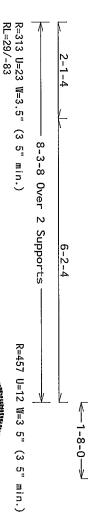
Design Dead Loads based on material weight adjusted for slope.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, within 4.50 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18CLOSED bldg, EXP B, wind TC DL=5.0 psf,

Wind loads and reactions based on MWFRS with additional C&C member

The overall height of this truss excluding overhang is 4-1-12





ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

Design Crit FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0)

OTY:6

FL/-/5/-/-/R/-

DATE

08/28/13

REF R215-- 62537

Scale =.5"/Ft.

DRW HCUSR215 13240030

HC-ENG KD/AP

PLT TYP.

Wave

Trusses require extreme care in fabricating handling shipping installing and bracing practices prior to performing these functions. Installing and bracing practices prior to performing these functions. Installing stability the same properly attached rigid colling Component Safety Information by TPI and WICA) are stability as properly attached rigid colling Component Safety Information by TPI and WICA) are stability as properly attached rigid colling Component Safety Information by TPI and WICA) are stability are properly attached rigid colling Component Safety Information by TPI and WICA) are stability are properly attached rigid colling Component Safety Information and Components for the personation of the safety of the Stability of the Stabil

SONAL ERGE TO LEASE T BC LL BC DL TC DL TC LL DUR. FAC. TOT.LD. 40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF

SPACING 24.0" 1.25 FROM JREF-1UZ6215_Z01

SEQN-

381275

Left cantilever is exposed to wind

(a) Continuous lateral bracing equally spaced on member

Truss passed check for 20 psf additional bottom chord live load areas with $42"\hbox{-high}\times24"\hbox{-wide clearance}.$ Ξ.

Deflection meets $L/240\,$ live and $L/180\,$ total load factor for dead load is 1.50. Creep increase

> 130 mph wind, 15.00 ft mean hgt, ASCE 7-10, within 9.00 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18 CLOSED bldg, not located EXP B, wind TC DL=5.0 ps psf

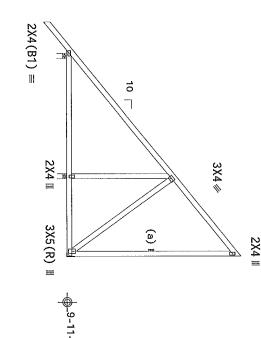
Wind loads and reactions based on MWFRS with additional C&C member design.

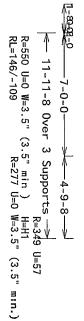
Right end vertical not exposed to wind pressure

Bottom chord checked for 10.00 psf non-concurrent live load

The overall height of this truss excluding overhang is 10-4-6.

MWFRS loads based on trusses located at least 30.00 ft. from roof





ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

PLT TYP.

Wave

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
***INFORMANT** FIRMING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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ITW Build ng Components Group Inc (ITMBCG) shall not be respons ble for any deviation for any failure to build the truss in conformance with AMSI/FPI so of for handling saighing in bracing of trussess Apply places to describe of truss afford to the truss and the saighing in Details unless noted otherwise. After to drawing indicates acceptance of professional originating the drawing indicates acceptance of professional originating the drawing indicates acceptance of professional originating the drawing indicates acceptance or professional origination or profession or professional origination or profession or profession

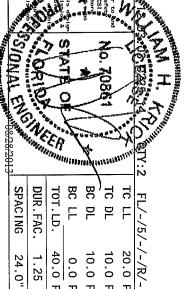
TANO

SPACING

24.0" 1.25 40.0 PSF

JREF-

1UZ6215_Z01



10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR215 13240008

DATE

08/28/13

REF R215-- 62538

Scale = .1875"/Ft

0.0 PSF

HC-ENG

KD/AP

SEQN-

381292

FROM

Top chord Bot chord 2x4 SP 2400f-2.0E 2x4 SP 2400f-2.0E 2x4 SP 2400f-2.0E

Left cantilever is exposed to ₩ınd

(a) Continuous lateral bracing equally spaced on member

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-4-6.

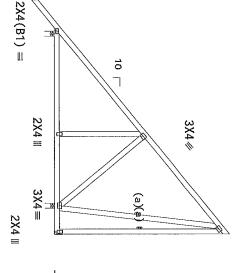
MWFRS loads based on trusses located at least $15\ 00\ \text{ft.}$ from roof edge.

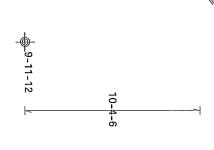
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, within 9.00 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCp:(+/-)=0.18 CLOSED bldg, not located EXP B, wind TC DL=5.0 psf,

Wind loads and reactions based on MWFRS with additional C&C member

Right end vertical not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





1-80-0 R=537 U=0 W=3.5" (3.5" minR}=55 U=24 RI=146/-109 H=HR=473 U=0 W=3.5" 11-11-8 Over 3 Supports -> 1-9-8

ITW Building Components Group Orlando FL, 32837 FL COA #0 278 **ALPINE**

PLT TYP.

Wave

Trusses require extreme care in fabricate in gamping structural sheathing and bracing practices prior to performing these functions installers shall have properly attached rigid colling locations shown for practice shall have properly attached rigid colling locations shown for practice shall have properly attached rigid colling locations shown for practice shall have properly attached rigid colling locations shown for persenting and between the shall have properly attached rigid colling locations shown for persental process the process of the practice of the practice

SIDNAL

SPACING

24.0" 1.25

JREF-

1UZ6215_Z01

40.0

SEQN-HC-ENG

381286

FROM

SE

0.0 PSF PSF

(3.5" min. CANE DE BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD.

FL/-/5/-/-/R/-

Scale = .1875"/Ft R215--

20.0 PSF

DATE REF

08/28/13

62539

10.0 PSF 10.0 PSF

DRW HCUSR215 13240009

KD/AP

Top chord Bot chord 2x4 SP 2400f-2.0E 2x4 SP 2400f-2.0E 2x4 SP 2400f-2.0E

(a) Continuous lateral bracing equally spaced on member

Truss passed check for 20 psf additional bottom chord live load areas with $42"\text{-high}\times\ 24"\text{-wide clearance.}$ Ξ

Bottom chord checked for 10.00 psf non-concurrent live load

The overall height of this truss excluding overhang is 10-4-6

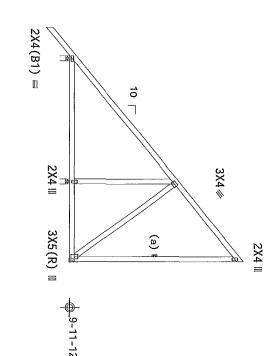
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, within 9.00 ft from roof edge, RISK CAT II, wind BC DL=5.0 psf. GCpi(+/-)=0.18 CLOSED bldg, not located EXP B, wind TC DL=5.0 ps

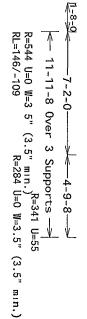
Wind loads and reactions based on MWFRS with additional C&C member

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.00 ft. from roof





WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0)

FL/-/5/-/-/R/-

REF

Scale = .1875"/Ft R215-- 62540

DATE

08/28/13

PLT TYP.

Wave

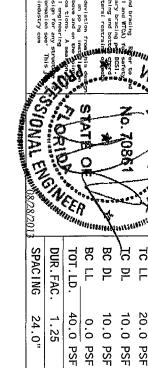
Trusses require extreme care in fabricating handling shipping installing and bracing Refer to a follow the latest edition of BSSI (Building Component Safety Information by TPI and MTCA). The safety from the property of the property and many these functions Installing installing per BSI. Whites noted otherwise top chord shall have properly attached structural sheathing and botto could be properly attached rigid coiling locations shown for permanent lateral restrains shall have bracing installed per BSI sections 83 BY or BYO as applicable.

I'M Bulliding Components Group Inc. (IYMSCA) have reported as applicable.

I'W Building Components Group inc. (ITREGO) shall not be responsible for any deviation from any failure to unlid the trues in conformance with ABSI/TPI 1 or for handling at high right bracking of trueses. Apply places to each fees of trues and position as shown above and on the Details unless moted otherwise. Before the drawing 1600-2 for standard place positions. A Details unless moted otherwise. Before the drawing 1600-2 for standard place positions and proposal in the case positions. A decaying or cover page 1 sting this drawing inclinates acceptance of professional engineering responsibility solely for the design shown. The substitute is the true of this decaying for any the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of the Building Des Sper per per (ANSI/TPI 1 Sec. 2 True more information seems the responsibility of th

TW Building Components Group Orlando FL, 32837 FL COA #0 278

ALP I NE



FROM

CDM 1UZ6215_

JREF-

Z01

SEQN-

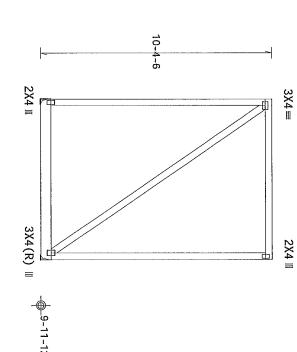
381132

HC-ENG KD/AP DRW HCUSR215 13240010

Top chord 2x4 SP 2400f-2.0E Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E

Special loads TC- From BC- From BC- 350.00 (Lumber Ib Conc. Load Dur.Fac.=1.25 / Plate Dur Fac.=1.25)
60 plf at 0.00 to 60 plf at 7.00
20 plf at 0.00 to 20 plf at 7.00
1b Conc. Load at 1.81, 3 81, 5.81

Wind loads and reactions based on MWFRS with additional C&C member



10-4-6

COMPLETE TRUSSES REQUIRED

Nail Schedule.0.131"x3", min. nails
Top Chord 1 Row @12.00" o.c.
Bot Chord 1 Row @10.00" o.c.
Webs ... 1 Row @ 4 o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

130 mph wind, 20.34 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 6Cpi(+/-)=0.18

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50 $\,$

Truss must be installed as shown with top chord ÷

lieu of structural sheathing The TC of this truss shall be braced with attached spans at 24" 8

Ξ

The overall height of this truss excluding overhang is 10-4-6

MWFRS loads based on trusses located at least 10.17 ft. from roof

These hangers and support conditions used 감 bearings indicated

(H1) = Simpson(H2) = (J) Hanger not calculated

<7-0-0 Over 2 Supports>

R=758 U=57 R=852 U=68 H=H2

ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

PLT TYP.

Wave

N. I. Co. O'IONAL ENGINEER L Silven manage BC DL BC LL 는 는 TC LL **SPACING** DUR. FAC. TOT.LD. FL/-/5/-/-/R/-

10.0 PSF

20.0 PSF

REF DATE

R215-- 62541

08/28/13

Scale = .25"/Ft.

10.0 PSF

DRW HCUSR215 13240011

KD/AP

381370

0.0 PSF

40.0

PSF

FROM SEQN-HC-ENG

24.0" 1.25

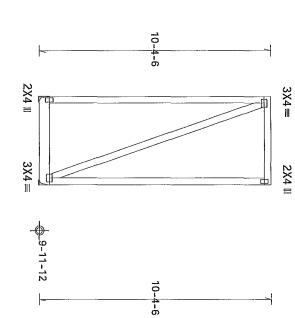
JREF-

1UZ6215_

Z01

Top chord 2x4 SP 2400f-2.0E Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E Special loads

Wind loads and reactions based on MWFRS TC- From 60 BC- From 20 BC- 348.54 lb (Dur.Fac.=1.25 / Plate Dur.Fac.=1. 60 plf at 0.00 to 60 plf at 3 20 plf at 0.00 to 20 plf at 3 lb Conc. Load at 1.81, 2.02 =1.25) 3.83 3.83



COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min nails Top Chord 1 Row @12.00" o.c. Bot Chord 1 Row @ 8.25" o.c. Webs 1 Row @ 4" o.c Nail Schedule:0.131"x3"

130 mph wind, 20.34 ft mean hgt, ASCE 7-10, CLOSED bldg, Locate anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind DL=5.0 psf. GCpi(+/-)=0.18 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Located F, wind BC

End verticals not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up

The TC of this truss shall be braced with attached lieu of structural sheathing. spans at 24" 8 Ξ

The overall height of this truss excluding overhang is 10-4-6

hangers and support conditions used at bearings indicated

(H1) = Simpson(H2) = (J) Hanger not calculated

ITW Building Components Group Orlando FL, 32837 FL COA #0 278 ALPINE

PLT TYP.

Wave

3-10-0 Over 2 Supports

R=502 U=75 H=H1 R=502 U=75

IT Building Components Group Inc (ITMECG) shall not be personal that the trusses Apply places to each face of functions of functions of functions of shall not proceed otherwise. I'll Building Components Group Inc. (I'll860s) shall not be responsible for any deviany failure to build the trues n conformance w th MSI/TPI 1 or for handling shi
browing of trueses. Apply plates to each face of trues and position as shown above
Details unless mored otherwise. Before to drawing 1604-2 for standard plates positi
developeration for cover page listing this drawing indicates acceptance of professional or
deaving or cover page listing this drawing indicates acceptance of professional
desponsibility soft the Building Designer par, MSI/TPI 1 Sec. 2

Tor more informathe responsibility of the Building Designer par, MSI/TPI 1 Sec. 2

Tor more informa-

PSIONAL ENGINEER BC DL 7d oĭ BC LL TC LL SPACING DUR. FAC. TOT.LD. FL/-/5/-/-/R/-

10.0 PSF 20.0 PSF

DATE

08/28/13

REF R215-- 62542

Scale = .25"/Ft.

10.0 PSF

DRW HCUSR215 13240012

40.0

PSF

SEQN-

0.0

PSF

HC-ENG

KD/AP 381295

1.25

FROM

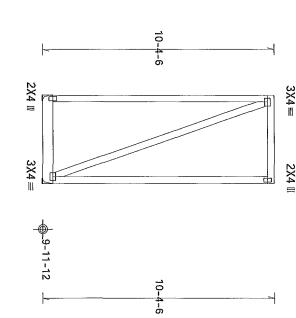
S

24.0"

JREF- 1UZ6215

Z01

Wind loads and reactions based on MWFRS Top chord 2x4 SP 2400f-2.0E Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E Special loads TC- From 60 plf at 0.00 BC- From 20 plf at 0.00 BC- 54.98 lb Conc. Load at ជ ជ Plate te Dur.Fac.=1. 60 plf at 3 20 plf at 3 1, 2.02



COMPLETE TRUSSES REQUIRED

Nail Schedule 0.131"x3", min. nails
Top Chord 1 Row @12.00" o.c.
Bot Chord 1 Row @12.00" o.c
Webs 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

130 mph wind, 20.34 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCp:(+/-)=0.18

End verticals not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up

The TC of this truss shall be braced with attached spans lieu of structural sheathing. at 24" 8 Ξ,

The overall height of this truss excluding overhang is 10-4-6

These hangers and support conditions used at bearings indicated

Hanger not langer not calculated calculated

H=H1 and H2 R=208 U=42 3-10-0 Over 2 Supports R=208 U=42

H=H1 and H2

PLT TYP.

Wave

Design Crit: FBC2010Res/TPI-2007(STD)

WARNING READ AND FOLLOW ALL MOTES ON THIS SMEET!

WARNING READ AND FOLLOW ALL MOTES ON THIS SMEET!

WARNING READ AND FOLLOW ALL MOTES ON THIS SMEET!

WARNING READ AND FOLLOW ALL MOTES ON THIS SMEET!

WARNING READ AND FOLLOW ALL MOTES ON THIS SMEET!

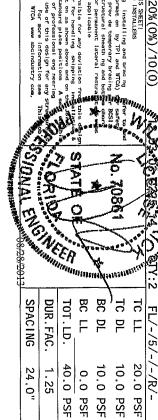
WARNING READ AND FOLLOW ALL MOTES ON THIS AND THE AND THE

ITW Build ng Components Group Inc (ITWBCO) shall not be responsible for any deviation fre any failure to build the truss n conformance with AMSI/IPH 1 or for handling shipping there is not provided by the state of truss and post on as shown above and on the brail is unless mored others so Refer to drawing 160A-Z for standard plate positions. Because of cover page listing this drawing indicates acceptance of professional eng neering responsibility so lely for the design stame. The suttability and use of this design for any the responsibility of the Building Designer per AMSI/IPI 1 Sec 2 For more information see

ITW Building Components Group Inc.

ALPINE

Orlando FL, 32837 FL COA #0 278



PSF

DATE

08/28/13

REF R215-- 62543

Scale = .25"/Ft.

PSF PSF

> HC-ENG KD/AP DRW HCUSR215 13240013

SEQN-

381289

FROM

JREF-

1UZ6215_Z01

Top chord 2x4 SP 2400f-2.0E Bot chord 2x4 SP 2400f-2.0E Webs 2x4 SP 2400f-2 0E

Left end vertical not exposed to wind pressure

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 4-10-2

MWFRS loads based on trusses located at least 15.00 ft, from roof

2X4 Ⅲ

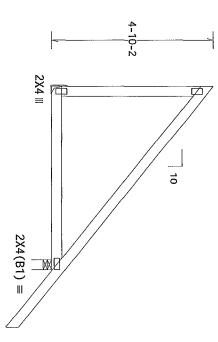
130 mph wind, 15.00 ft mean anywhere in roof, RISK CAT I DL=5.0 psf. GCpi(+/-)=0 18 hdt, ASCE 7-10, CLOSED bldg, Located II, EXP B, wind TC DL=5.0 psf, wind BC

Wind loads and reactions based on MWFRS with additional C&C member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

These hangers and support conditions used at bearings indicated

Hanger not calculated Hanger not calculated Hanger not calculated



← 5-4-0 Over 2 Supports → <u>1-8-0</u>≥

RL=43/-74 H=H1 and H2

R=187 U=10

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/10(0) R=349 U=0 W=3.5"

PLT TYP.

Mave

ITW Building Components Group Orlando FL, 32837 FL COA #0 278

ALP I NE

CONFER . BC LL SPACING DUR. FAC. TOT.LD. 40.0 1.25 24.0'

BC DL TC DL TC LL

10.0 PSF 10.0 PSF

DRW HCUSR215 13240014 HC-ENG KD/AP

08/28/13

0.0 PSF PSF

FROM SEQN-

381140

JREF-

1UZ6215_Z01

FL/-/5/-/-/R/-

20.0 PSF

留 DATE

Scal

le =.375"/Ft. R215-- 62544

rusalish This DESIGN TO ALL CONTRACTIONS INCLUDING INSTALLERS
Trusses require excreme care in fabricating handling shapping install ng and bracing follow the latest edition of BCSI (Suit Iding Component Safety) Information by TPI and WICA) server practices prior to performing those functions installers shall nove the emporary bracing fabricating the practices prior to performing those functions installers shall nove the emporary bracing fabricating the practices of the protection of the protec

Top chord 2x4 SP 2400f-2.0E Bot chord 2x6 SP 2400f-2.0E Webs 2x4 SP 2400f-2.0E Special loads ----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1 TC- From 30 plf at 0.00 to 30 plf at 3 BC- From 10 plf at 0.00 to 10 plf at 3 BC- 186.82 lb Conc. Load at 1.81, 3.81, 5.8) to 30 plf at 7) to 10 plf at 7. t 1.81, 3.81, 5.81 1 25) 7 00 7.00

3X4 =

End verticals not exposed to wind pressure Wind loads and reactions based on NWFRS.

> 2 COMPLETE **TRUSSES** REQUIRED

Nail Schedule:0.131"x3", min. nails Top Chord 1 Row @12.00" o.c. Bot Chord 1 Row @12.00" o.c. Webs 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5 0 psf. GCpi(+/-)=0.18

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

 $2X4 \,
Vert$ Truss must be installed as shown with top chord up

4-10-2 ф

The TC of this truss shall be braced with attached spans at $24\ensuremath{^{"}}\ 0\ensuremath{\text{C}}$ in lieu of structural sheathing

The overall height of this truss excluding overhang is 4-10-2

4-10 (H2) = (J) Hanger not calculated bearings indicated These hangers and support conditions used at bearings indicated (H1) = Simpson

R=395 U=20 7-0-0 Over 2 Supports R=445 U=23 H=H2

2X4 III

3X4 =

⊕9-11-12

PLT TYP. Wave

Trusses require excreme care in fabricating banding shipping install gand bracing free copied otherwise noted otherwise or copied shall have properly attended right and colling free copied shall have properly attended right colling installers.

Thus to latest edition of BCSI (Building Component Safety Information by IPI) and WIDA) are safety practices prior to performing these functions. Installers shall provide temporary brace notes that have properly attended structural sheathing and bottle control shall have properly attended structural sheathing and bottle control shall have properly attended structural sheathing installed prior deciling locations shown for permanent lateral restrict. The shall have properly attended structural shall have broken in components of control and shall have broken in components. Details unless noted otherwise & drawing or cover page listing this responsibility solely for the design the responsibility of the Building openion.

ITW Building Components Group

ALPINE

Orlando FL, 32837 FL COA #0 278

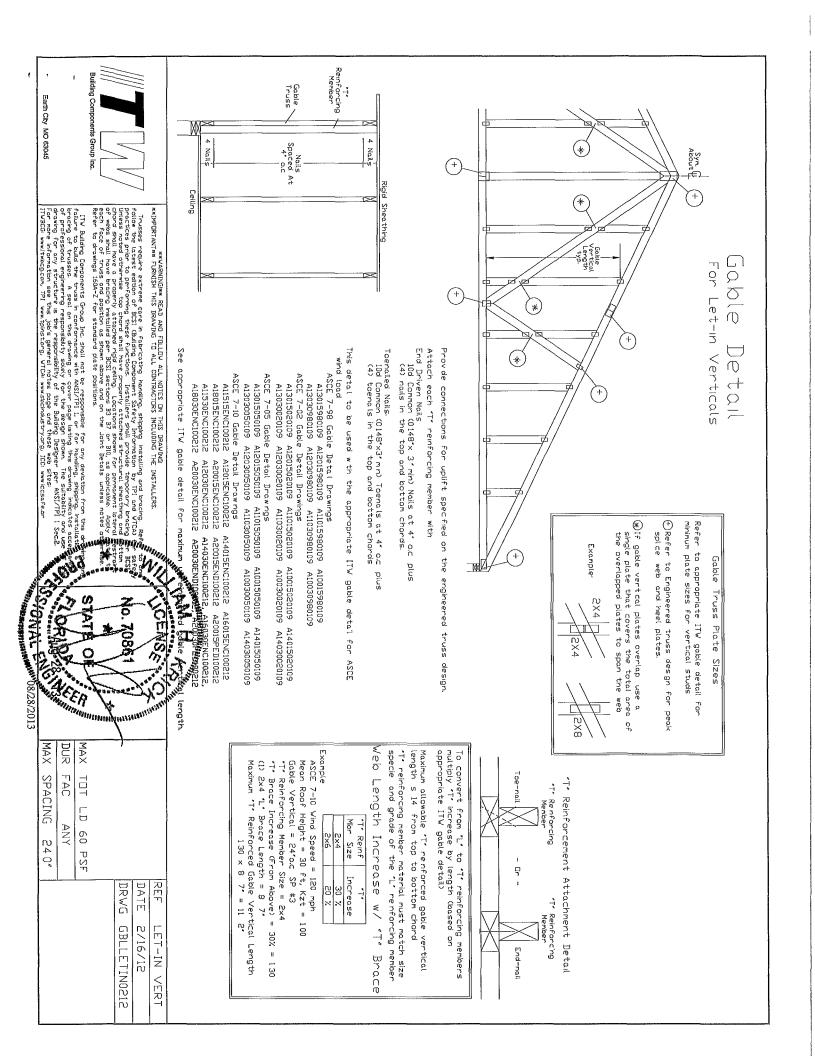
SIDNAL ELECTRICATION TOTAL BC LL BC DL TC DL TC LL SPACING DUR. FAC TOT.LD. 40.0 10.0 PSF 10.0 PSF 20.0 PSF 1.25 24.0" 0.0 PSF PSF REF SEQN-DATE HC-ENG KD/AP DRW HCUSR215 13240015 FROM REF-R215-- 62545 1UZ6215_Z01

381372

Scale = .5"/Ft.

08/28/13

brace is used. Connect diagonal brace for 525# at each end Max web total length is 14 Diagonal brace option vertical length may be doubled when diagonal Vertical Length Gable Max Earth City MO 63045 Vertical length shown in table above Connect diagonal at midpoint of vertical web Buinad 12" 16" 24 0.0 $O \cdot C$ \circ Ex4 Gable Vertical Species SPF SPF SPF DF! 9 干 8 Ü ASCE Grade Standard Standard #1 tandard Stud Stud tandard Stud Stud Stud Refer to chart above of the sections and position as shown above and on the Joint Deality and position as shown above and on the Joint Details unless noted otherwise to chord shall now practice. Refer to domains 160A-Z for standard date positions. # | # | #ω #; **#** Brace \ #2 ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing of failure to baild the trust in conformance with ANSI/TPI I. or for handling, shipping, installation beaching thrusses. A seal on this drawing or cover page listing this drawing, hallactes acceptanged for the design shown. The suitability and use of drawing for any structure is the responsibility of the Building Disjoner par ANSI/TPI I Sec2. For more information seen this job's general notices to these web sitted. #2 7 - 10Jable Truss 4 5 4 4 4 4 v v v v Braces 4 ഗ 4 4 4 4 N 0 ယုမြ 티아 ιń က် ထိ ထိ 낂턶 힌팅 ์รู้เ 140 2x6 DF-L #2 or better diagonal brace single or double cut (as shown) at upper end Group A (1) 1x4 'L' Brace * (1) 2x4 'L' Brace * (2) 2x4 'L' Brace ** (1) 2x6 'L' Brace * (2) 2x6 'L' Brace $\sqrt{|\alpha|}$ 900 d d 100 ű δ ą 6 6 0, 11, 6, 6, 6, 10 <u>ဝ</u> ကု δ 10, 777 group Ø WIND Speed, 30' Mean Height, Enclosed, Exposure C kzt = 100 120 mph Wind Speed 30 Mean Height Partially Enclosed Exposure C kzt = 100 120 mph Wind Speed 30 Mean Height Enclosed Exposure D kzt = 100 100 mph wind speed 30 Mean Height Partially Enclosed Exposure D kzt = 100 က် ကို ယုံယူ 2 5 N W υį Gable Group A 10 10 10 5 0, Q Stud Reinforgement ۲, Group B 인티 ą 9 5 $\tilde{\omega}$ <u>|</u> − <u>g</u> Group A Φ α ထ်ထ် ထိုထို ģ ģ ιń ڻ 0 0 0 0 0 0 0 0 ONA MANAGEMENT Group B ó ő 5 V Group A ī Detail ó 0 0 0 0 0 o o οź σ, 10, Ð The state of the s Group B 14 4 ü 14 14 41 41 20 22 22 10 o o o ď ထို 4 ណ៍ Group A 14 14 14 14 14 Exposur ط اط ď ď ď 읶 Group Æ⊕ 14 SPACING 101 w ή * For (1) 'L' braces space nails at 2' o.c. n 18' end zones and 4' o.c. between zones. **For (2) 'L' braces space nails at 3' o.c. So. Pine lumber design values based on the ALSC January 2012 rulin Gable end supports load from 4 0' outlookers with 2 0' overhang or 12' plywood overhang. Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load) $\widehat{\Box}$ Refer to the Building Designer 'L' bracing must be a minimum of 80% of web member length. Attach "L" braces with 10d (0.128"x3.0" min) nails Wind Load deflection criterion is L/240 not addressed by this detail ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group values may be used with these grades. 1x4 Braces shall be SRB (Stress-Rated Board) Bracing Group Species and Spruce-Pine-Fin #1 / #2 Standard \$1 Stud in 18" end zones and 6" o.c. between zones Douglas Fir-Larch Gable Truss Detail Notes Vertical Length Less than 4 0' Greater than 4 0' Douglas Fir-Larch 60 less than 11 6" Greather than 11 ス ス た れ Refer to common truss design peak splice and heel plates. 74 O Gable Vertical Plate Sizes Standard PXF **#** # H DATE DRWG dnoug group #1 & Btr 00 A14030ENC100212 t o 2/14/12 ASCE7-10-GAB14030 W ⊅ Southern Pine** #3 # # Standard for conditions 2.5X4 Stud 3.5X4 Splice 1.5X4 Grades. Standard ę W

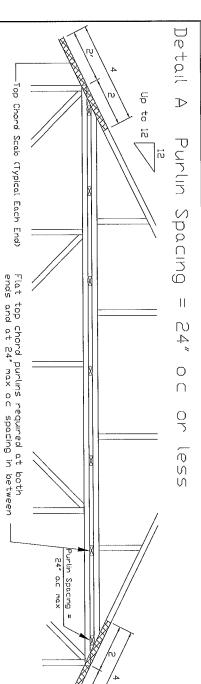


Piggyback 160 mph Wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Blag located anywhere in roof Exp C Dr 140 mph wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Blag located anywhere in roof Exp Detail 1 ASCE \vee ł 10 160 , N M M M 30 M Q Q C Wnd DL= 50 psf (min) D wind DL= 5.0 psf (m Height, (min) Kzt=10 Enclosed, Exposure \bigcap X N + 00

Note Top chonds of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing for out of plane loads over gable ends

Maximum truss spacing is 24° oc detail is not applicable if cap supports additional loads such as cupola steeple chimney on drag strut loads

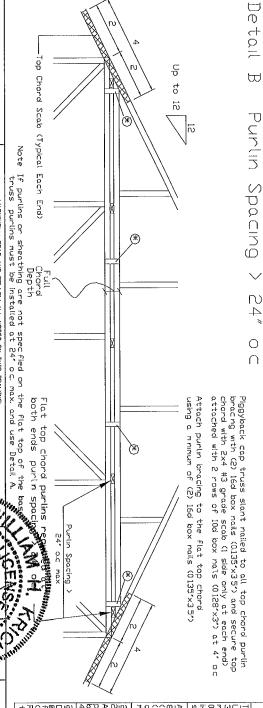
sealed truss design drawing for pggyback and base truss specfcations



purily bracing with (2) 16d box nails (0135*x35') and secure top chord with 2x4 #3 grade scab (1 sde only at each end) attached with 2 rows of 10d box nails (0.128*x3') at 4' oc Pggyback cap truss slant nailed to all top chord

Attach purlin bracing to the flat top chord using (2) 16d box nails (0135'x35')

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3x8 Trulox plate attached with (8) 0120°x1375′ nails (4) into cap TC & (4) into base truss TC or (1) 28P3 wave pgyback plate plated to the pigyback truss TC and attached to the base truss TC with (4) 0120°x1375′ nails Note Nailing thru holes of wave plate s acceptable



* In addition with one of the following methods

Use 3%8 Irulax plates for 2x4 charal member, an 3XID Trulax plates for 2x6 and larger chard members Aftach to each face & 8 or with (4) 0120°X1375° nails into cap bottom chard and (4) in base trusts top charal Trulax plates may be staggered 4 or front to back faces. ono

8"x8"x7/16' (min) APA rated shapthing gussets (each face) Attach & 8 or with (8) 5d common (10113"x2") rails per gusset (4) in cap bottom (Jussets to chord faces) and be staggered 4 or front to back faces PA Rated Gusset

| 2x4 Vertical Scabs | Partical Scabs | staggered

Ins 28PB wave pigpyback plate to each face B ac Attach teeth to pigpyback at time B according that to each face per ply such that B according the supporting truss with 120% 1375, nails per face per ply such that a constant provided the supporting that the provided that the supporting the supporting that the supporting that the supporting that t to back faces 2828 Wave Piggyback Plate o.c front 맠

DATE PB160100212 2/14/12 PIGGYBACK

Trusses require extreme care in Fabricating handling, shipping, installing and bracing. Reference and follow the latest edition of BCSI (Building Component Safety, Information, By PRI and Information and Information and By PRI and Information and Informat

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawf failure to build the truss in conformancie with ANSI/TPI I. or for handling, simplifying, installation tracing of truspens to set a seal on this building or cover page listing the drawing, brightest accepts of professional engineering responsibility body for the design shown. The suitability and use or graving for any structure; at the responsibility of the Building Designer per ANSI/TPI I Sec.2.

Service of the servic

SPACING

240"

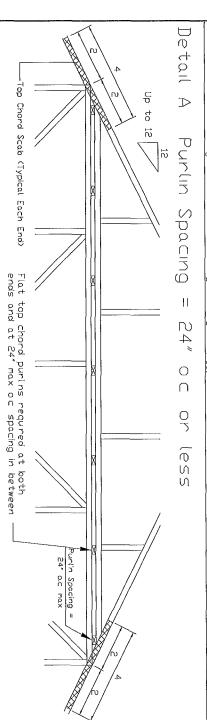
Earth City MO 63045

Piggyback mph Wind 3000 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp C Wind DL= 50 psf (min) Kzt=10 140 mph wind 3000 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp D, wind DL= 5.0 psf (min) Kzt=10 Detail ASC \Box V $^{'}-10$ 160 , 7 3 8 8 30 Mean Height, Enclos ў Ф ў: Exposure \bigcirc **ベスセー1** 00

Note Top chands of trusses supporting piggyback cap trusses must be adequately braced by sheathing on purlns. The building Engineer of Record s provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends Shall

Maximum truss spacing s 24° oc detal is not applicable if cap supports additional loads such as cupola steeple chimney or drag strut loads

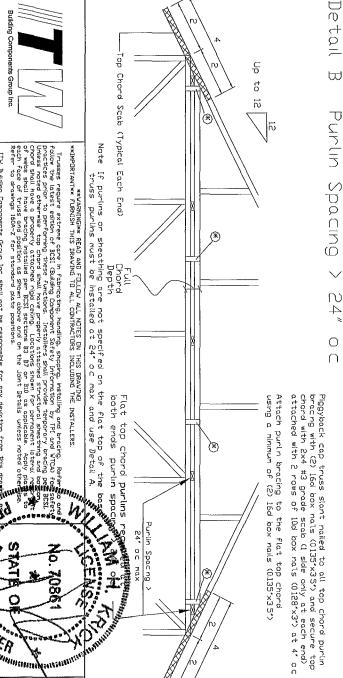
sealed truss design drawing for piggyback and base truss specifications



(1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0135"x35") and secure top chord with 2x4 #3 grade scab

Attach purlin bracing to the flat top chord using (2) 16d box nails (0) 135° x3 5°)

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3X8 Truibox plate attached with 68 0.1120 X1375 nails (4) into cap TC % (4) into base truss TC or (1) 28PB wave p ggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120 X1375 nails Note Naing thru holes of plate is acceptable



* In addition
with one of provide connection the following methods

Use 3X8 Trulox plates for 2x4 chand member and 3X10 Trulox plates for 2x6 and larger chand members Aftach to each face & 8 or with (4) 01207x1375 nals into cap bottom chand and (4) in base truss top chand Trulox plates may be staggered 4 or front to back faces 000

8'x8'x7/16' (min) APA rated sheathing gussets (each face) Attach @ 8' or with (8) 6d common (0113'x2') nails per gusset, (4) in cap bottom chord and (4) in base truss top chord Gussets may be staggered 4 or front to back faces APA Rated Gusset

28PB Wave Piggyback Plate nails (0128"x3") d and (3) in y be staggered

Dine 28PB wave piggyback plate to each face 8 oc. Attach teeth to piggyback at time of fabrication Attach to supporting truss with (4) 0120'x1375' nails per face per ply Piggyback plates may be staggered 4' oc frowack faces DATE PB160100212 2/14/12 PIGGYBACK

on front

of.

SALER HOLDEN

SPACING

24 O,

Earth City MO 63045

ITV Bullish Components (Iroup Inc. shell not be responsible for any deviction from this drawing in failure to blid the they trues in conformatice with MNI/IPI I, or for moding, shipping, installation & bracing of trueses A real to mother brawing or cover page listing two brawing locates acceptant of professional engineering responsibility solely for the design grown. The stability and use of trawing for any structure, is the responsibility of the Building Designer per ANSI/IPI I secci.

Diagonal brace option vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web MaxVertical Length Gable Vertical length shown in table above Earth City MO 63045 Buinad Connect diagonal at midpoint of ventical web. 12" 16" 24" \circ \subset $O \subset \iota$ 0 Eable Vertcal Species SPT SPF SPF DFI DFL H ૂ 8 SP 9 ASCE Grade Standard tandard tandard Stud Stud Stud tandard Stud Stud Refer to Chart above and on the John Details unless noted of store as store and on the John Details unless noted of store of store of store of store of the store # # # # # ## #ω #ω ITV Building Components Group Inc. shall not be responsible for any deviation from this drappy we follow to build the trues in conferenance with ANSI/TP1 L or for handling, shapping, installations for professional engineering responsibility solely for the design shown. The suitability and use of professional engineering responsibility solely for the design shown. The suitability and use of drawing for any structure is the responsibility of the Building Diseigner per ANSI/TP1 I Sec2. For more information see this job's general notes page and these was stress from the suitability of the suitability of the Building Diseigner per ANSI/TP1 I Sec2. For more information see this job's general notes page and these was stress from the suitability of the suitability of the Building Diseigner per ANSI/TP1 I Sec2. 7-10 # #:0 Jable Truss Braces ณ์ ณ์ No ណ៍ ω ထ်ထ် Ħ 11. င် ထိ ထိ ထိ 140 Group A brace single or double cut (as shown) at 8 better diagonal 2×4 DF-L #2 or ω Z Z Z 1×4 'L' ယူယ္ 4 4. က် က် က် 🗗 4 ;|;**+** ιń **ų**ω 999 Group B Brace * ∑ ∑ ∑ 120 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt = 100 120 mph Wind Speed 15 Mean Height Enclosed Exposure D Kzt = 100 mph Wind Speed 15 Mean Height Partially Enclosed Exposure D Kzt = 100 4, 4. ď တ် က် က် ထံ ထို လ် လ် ထို ထ် က် 4 4 á 7. Gable Group A (1) 2x4 "L" Brace * (2) 2x4 "L" Brace ** (1) 2x6 "L" Brace * (2) 2x6 "L" Brace Speed ω Ø Ø n 0 0 0 ထ့ ≓ | ∞ | ∞ | ∞ وٰاوَ ထ္ ထု Stud Group B ú $\underline{\underline{\omega}}$ <u>1</u>0, ωį Reinforcement group Mean Height, ထု D Group B 10 S. A. A. σ ω ď ιń ωį Group A 14 14 14 4 13 14 14 芯 ŭ 딦 Enclosed, ő ó Q 0 0 0 00 ó | ó Detail of l ó ó σ oʻ oʻ ญ ດ໌ເ September 1 Group B 14 14 14 Ω 13 ó Q 익숙 Q ď drosp Grond 14 4 4 14 4 4 4 14 Exposure ó ó Q ď ó ó MA X groons U/O 14 14 14 14 14 14 4 4 4 14 **∺** 14 14 14 14 0" 14 SPACING ó ó 익 **€** | € ď ď ó ď Q W \Box Refer ** For (1) 'L' brace space nails at 2' o.c. n 18' end zones and 4' o.c. between zones.
*****For (2) 'L' braces space nails at 3' o.c. So. Pine lumber design values based on the ALSC January 2012 rulin Gable end supports load from 4 0' autlookers with 2 0' overhang or 12' plywood overhang. Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load) Attach "L" braces with 10d (0.128"x3.0" min) nails Wind Load deflection criterion is L/240 not addressed by this detail. "L" bracing must be a minimum of 80% of member length. ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group values may be used with these grades. 1x4 Braces shall be SRB (Stress-Rated Board) Bracing Douglas Fir-Larch
#1
#2 Spruce-Pine-Fin n 18' end zones and 6' o.c between zones Gable Truss Detail Notes Douglas Fir-Larch Vertical Length
Less than 4 0'
Greater than 4 0'
less than 11 6' 60 ス N t Refer to common truss design peak splice and heel plates. Greather than 11 υ 4 to the Building Designer Gable Vertical Plate Sizes Standard PSF Stud ė Group Species П DRVG DATE gnoan dnoug ___ 00 c C A1401SENC100212 2/14/12 Southern Pine*** ASCE7-10-GAB14015 ш Southern Pine*** D gnd Standard 1X4 or 2X3 for conditions 2.5×4 3×4 Grades Standard ĝ

H W W $\overline{\mathcal{J}}$ $\bigcap_{i=1}^{\infty}$ SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

NUTES

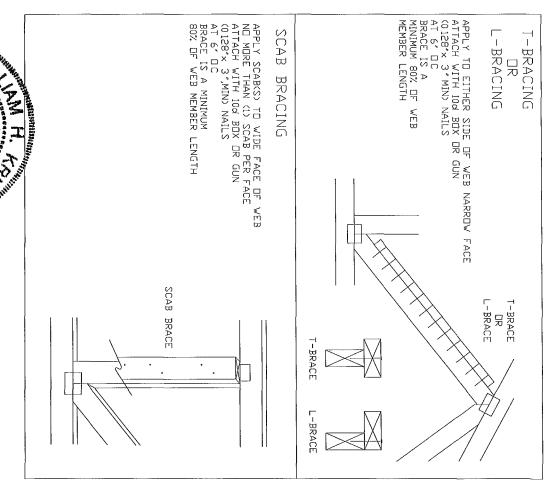
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING BRACING RE-RUN DESIGN WITH APPROPRIATE

3-5X6(*)	2X6	2 ROWS	8X2
1-2X8	2X6	1 ROV	8X5
2-2×4(米)	5X6	2 ROWS	2X6
1-2X6	2×4	1 ROW	2X6
2-2X4	2X6	2 ROWS	2X3 OR 2X4
1-2X4	2X4	1 ROW	뮸
SCAB BRACE	T DR L-BRACE	BRACING	SIZE
ALTERNATIVE BRACING	AL TERNATI	SPECIFIED CLB	WEB MEMBER

T-BRACE L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN

 $\widehat{*}$ FACE OF WEB CENTER SCAB ON WIDE FACE OF VEB APPLY (1) SCAB TO EACH





**VARNINGEX READ AND FOLLOY ALL NOTES ON THIS SHEET

In usees require extreme care in fabricating, hondling shipping installing and bracing. Refer follow DCS (Balliang Component's Safety Information, by IPI and VICA) for safety practices compered these functions, brotallers shall provide temporary bracing per BCSL. Unless need otherwise to chord should have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have properly attached rigid ceiling. Locations shown for permanent lateral notes page for more should be used to be component to the properly of this DESIGN TO INSTALLATION CONTRACTION.

***IMPORTANTEX** CIPNUTS** CIPNUTS** STORE IN THIS DESIGN TO INSTALLATION CONTRACTION.

***IMPORTANTEX** CIPNUTS** CIPNUTS*

on this drawing on cover page indicates acceptance and professional engineering respi for the truss component design shown. The suitability and use of this component for g. is the responsibility of the Building Besigner per MST/TPI I. Sec. 2. Su www.itwbcg.com, TPI www.tpinst.com, VTCA: www.sbcindustry.com, ICC: www.ccsafe.org

Earth City MO 63045

STONAL STORY Marin is BC LL SPACING TOT LD DUR FAC P 닏 PSF PSF PSF PSF DRWG DATE

C

PSF

RET

SUBST

1/1/09 CLB

BRCLBSUB0109



MINIMUM SPACING FOR SINGLE BLOCK IS AND STAGGER NAILING FOR TWO BLOCKS REQUIRED TO AVOID SPLITTING NADHS SHOWN DOUBLE NAIL SPACINGS GREATER SPACING MAY BE

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN

DF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL

C - END DISTANCE (15 NAIL DIAMETERS) STAGGERED ROWS

DIAMETERS

LOAD PARALLEL TO GRAIN

EDGE DISTANCE (6 NAIL DIAMETERS)
SPACING OF NAILS IN A ROW AND END :
SPACING BETWEEN STAGGERED ROWS OF DISTANCE (15 NAIL DIAMETERS)
NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN —BELOW ** SPACING MAY BE REDUCED BY 50% BY 33%

C** | A A BLOCK LENGTH DIRECTION F LOAD ANI ROWS ROWS C * * LINE ₽ D B/2/ MEMBER ₩ Ж BLOCK LENGTH \Box

> MINIMUM NAIL SPACING DISTANCES

	1	DIS	DISTANCES			
	NAIL TYPE	D	₩ *	C **	D	
<u>-</u>	8d BOX (0113"X 25",MIN)	3/4"	1 3/8"	1 3/4"	7/8"	
ļ	10d B□X (0128"X 3",MIN)	7/8″	1 5/8"	νį	1"	
	12d BOX (0128"X 325",MIN)	7/8″	1 5/8"	ινį	1"	
	16d BOX (0135"X 35",MIN)	7/8"	1 5/8"	2 1/8" 1 1/8"	1 1/8"	
	20d BOX (0148 X 4 ",MIN)	1"	1 7/8"	2 1/4"	1 1/8"	
	8d COMMON (0131"X 25",MIN)	7/8″	1 5/8"	ινį	1"	
	10d COMMON (0148"X 3",MIN)	1"	1 7/8"	2 1/4" 1 1/8"	1 1/8"	
RS)	12d COMMON (0148"X 325",MIN)	F."	1 7/8"	2 1/4"	1 1/8"	
ERS	ERS) 16d COMMON (0162"X 35",MIN)	1'	ινį	2 1/2"	1 1/4"	
	GUN (0120"X 25",MIN)	3/4"	1 1/2"	1 7/8"	1"	
	GUN (0131"X 25",MIN)	7/8"	1 5/8"	νį	1"	
 *	C** GUN (0120"X 3",MIN)	3/4"	1 1/2"	1 7/8"		
	GUN (0131"X 3",MIN)	7/8"	1 5/8"	ιν	1"	
C/2**	*					-

_OAD APPLIED PERPENDICULAR TO GRAIN

A. H. A. H.

ENDICULAR II GRAIN

LIAD APPLIED PARALLEL II

ANVARNINGS READ AND FULLDY ALL NOTES ON THIS SHEET

Trusses require extreme cure in februcating, handling shipping, installing and bracing. Refer follow BCSI Galding Component Safety Information, by IFP and VICA) for safety practices per february bearing to be short shall have properly attached structural panels and botton chard shall have properly attached reportant lateral restraint of webs sold have properly attached right calling. Locations shown for permanent lateral restraint of webs sold have properly attached right calling. Locations shown for permanent lateral restraint of webs sold have properly attached right calling. Locations shown for permanent lateral restraint of webs sold have properly attached right calling. Locations and the sections of the short of

on this drawing or cover page indicates acceptance and professional engineering respons for the truss component design shown. The suitability and use of this component for g. is the responsibility of the Building Designer per ANST/TPI 1 Sec. 3.

ICC: www.iccsafe.org

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DATE CNNAILSP0109 1/1/09

NAIL

SPACE

DRWG

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