

DATE 11/07/2005

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000023827

APPLICANT HUGO ESCALANTE

PHONE 386-288-8666

ADDRESS 6210 SW CR 18

FORT WHITE

FL 32038

OWNER JAY & CATHLEEN HART

PHONE 209-743-0434

FORT WHITE

FL 32038

ADDRESS 675 SW MORNING STAR GLEN

FORT WHITE

PHONE 386-288-8666

CONTRACTOR HUGO ESCALANTE

PHONE

386-288-8666

LOCATION OF PROPERTY

47 S, L SW MORNING STAR GLEN, 1/2 MILE ON THE LEFT

TYPE DEVELOPMENT

SFD, UTILITY

ESTIMATED COST OF CONSTRUCTION

110350.00

HEATED FLOOR AREA

2207.00

TOTAL AREA

3566.00

HEIGHT

22.10

STORIES 1

FOUNDATION

CONCRETE

WALLS FRAMED

ROOF PITCH

6/12

FLOOR SLAB

LAND USE & ZONING

A-3

MAX. HEIGHT

35

Minimum Set Back Requirements:

STREET-FRONT

30.00

REAR

25.00

SIDE

25.00

NO. EX.D.U.

0

FLOOD ZONE

XPP

DEVELOPMENT PERMIT NO.

PARCEL ID

34-5S-16-03752-109

SUBDIVISION

SHANNA MEADOWS

LOT 9

BLOCK

PHASE

UNIT

TOTAL ACRES

5.00

000000881

Culvert Permit No.

Culvert Waiver

Contractor's License Number

CRC1326967

Applicant/Owner/Contractor

JH

N

Driveway Connection

Septic Tank Number

05-0981-N

BK

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: NOC ON FILE, FLOOR 1 FOOT ABOVE THE ROAD

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power

Foundation

Monolithic

(Footer/Slab)

Under slab rough-in plumbing

date/app. by

Slab

date/app. by

Sheathing/Nailing

date/app. by

Framing

date/app. by

Rough-in plumbing above slab and below wood floor

Electrical rough-in

date/app. by

Heat & Air Duct

date/app. by

Peri. beam (Lintel)

date/app. by

Permanent power

date/app. by

C.O. Final

date/app. by

Culvert

date/app. by

M/H tie downs, blocking, electricity and plumbing

date/app. by

Pump pole

date/app. by

Utility Pole

date/app. by

Re-roof

date/app. by

M/H Pole

date/app. by

MISC. FEES \$

.00

ZONING CERT. FEE \$

50.00

FIRE FEE \$

.00

WASTE FEE \$

17.83

BUILDING PERMIT FEE \$

555.00

CERTIFICATION FEE \$

17.83

SURCHARGE FEE \$

17.83

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$

25.00

CULVERT FEE \$

25.00

TOTAL FEE

690.66

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

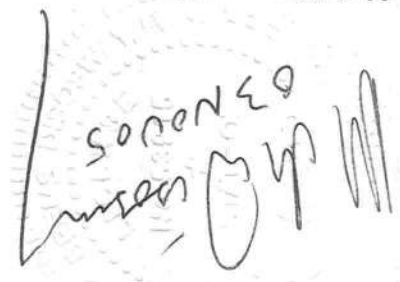
"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Mark Disosway, PE
Florida Registered Professional Engineer
Cc Ewpl Inc / Hugo Escalate (Builder)



- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.
 - The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

Building Department
Re: Permit 0510-83, Ewpl Inc / Hugo Escalate, Hart Residence, Lot #9, Shanna Meadows S/D, Columbia County, Florida
Dear Building Official:

November 3, 2005

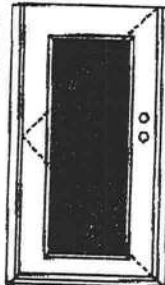
Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

COP-WL EN4141-02

X
Glazed Inswing Unit

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Test Data Review Certificate #3028447C
and COP-7100 Report Validation Matrix
#3028447C-001 provides additional
information - available from the ITSC/WH
website (www.woodedge.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.



Single Door
Maximum unit size = 3'0" x 6'8"
Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.
Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 133 Series



138 Series



650 Series



622 Series

1/2 GLASS:



105 Series



108, 100 Series



120 Series



200 Series



12 R/L, 23 R/L, 34 R/L Series



107 Series



108 Series



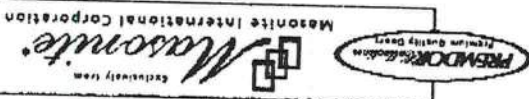
304 Series

This glass kit may also be used in the following door styles: 5-panel; 6-panel; 6-panel with scroll; Eyebrow 5-panel; Eyebrow 6-panel with scroll.

Entry Systems

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

June 17, 2002



X
Glazed Inswing Unit

COP WL FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt D. Portney

State of Florida, Professional Engineer
Kurt Portney, P.E. - License Number 56533

That Data Review Certificate #029447C and CO-01 Report additional information - available from the ITSMW website (www.itsmw.com). The Masonite website (www.masonite.com) or the Masonite technical center.

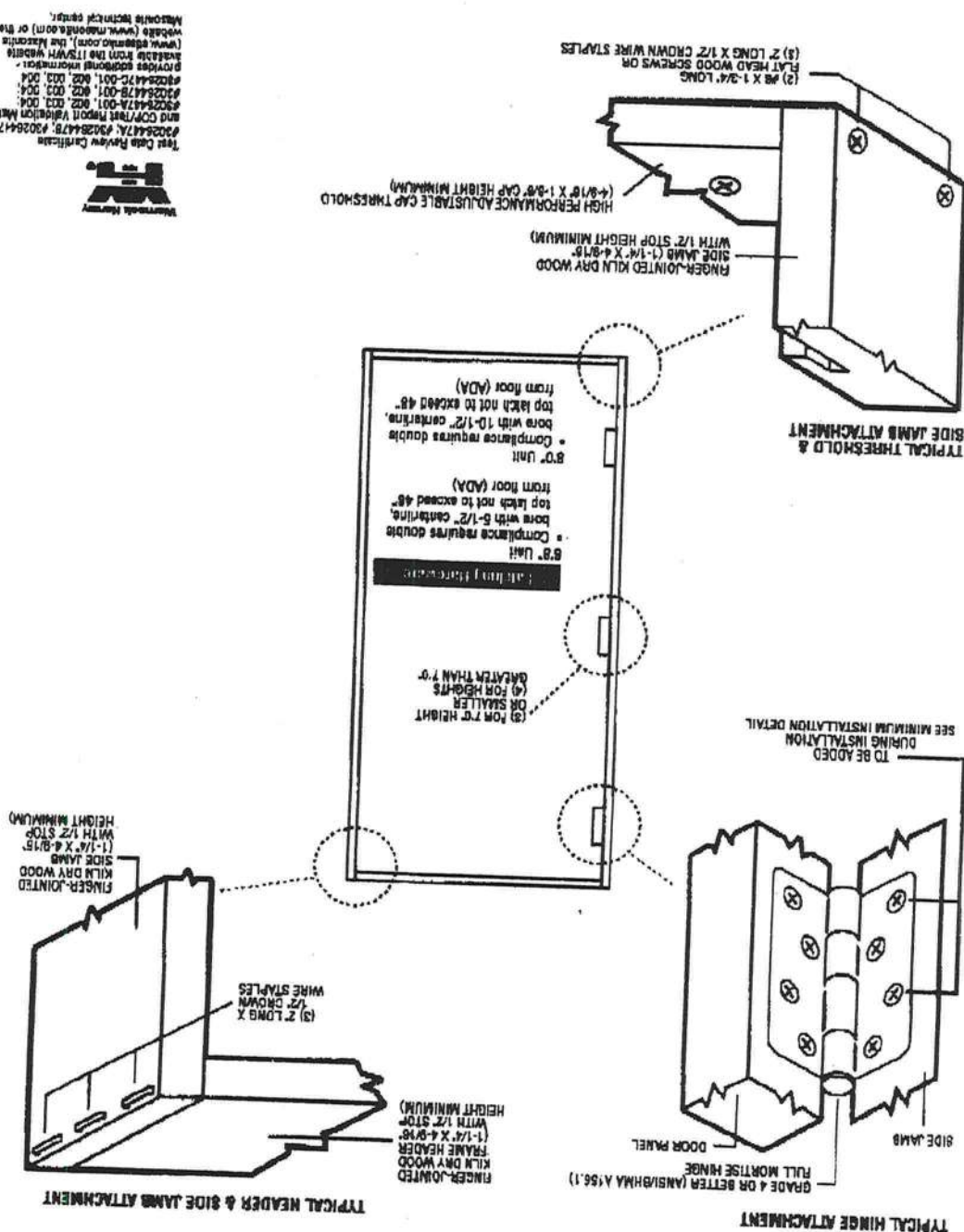


Entry Systems
June 17, 2002
Our labeling program of product improvements makes specifications, design and product more subject to change without notice.

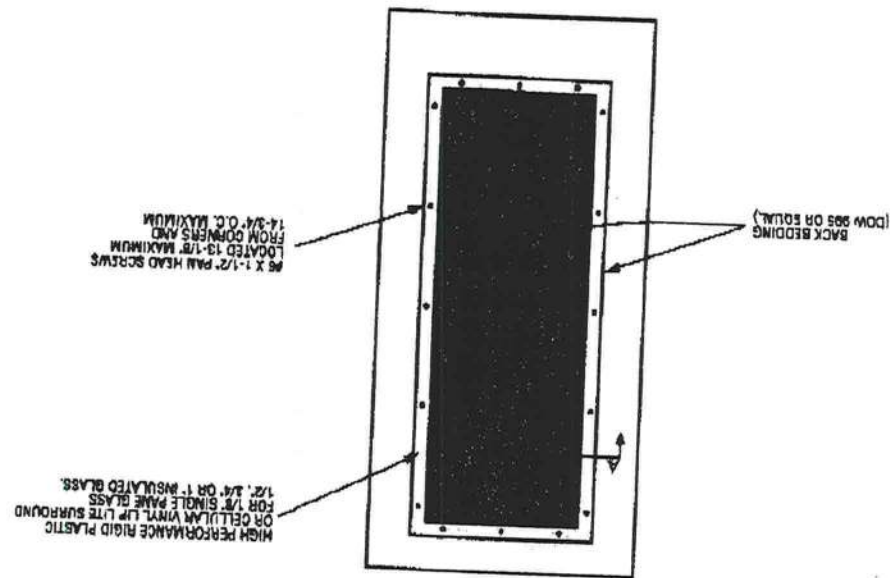
INSWING UNIT WITH SINGLE DOOR

X
Unit

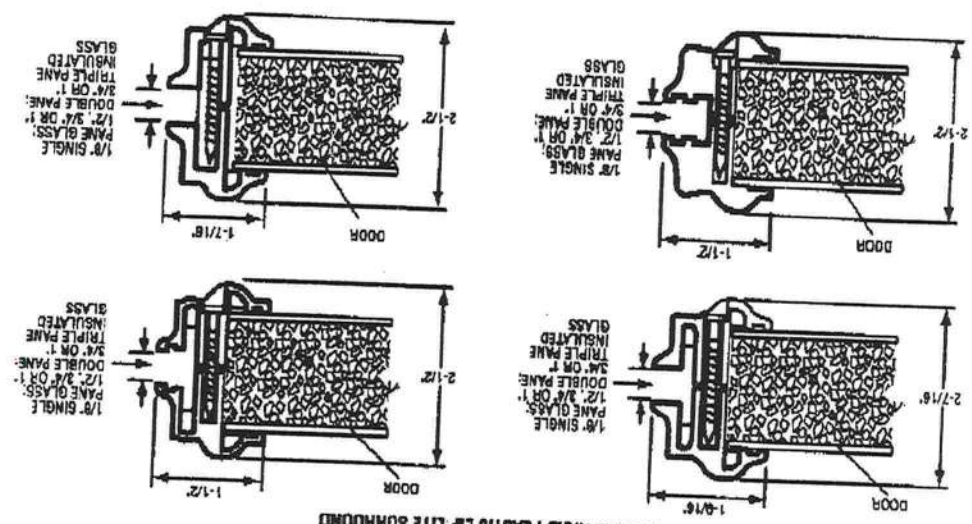
WAD-WI-MA0001-02



MAD-WI-MA0041-02
GLASS INSERT IN DOOR
OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP SURROUND

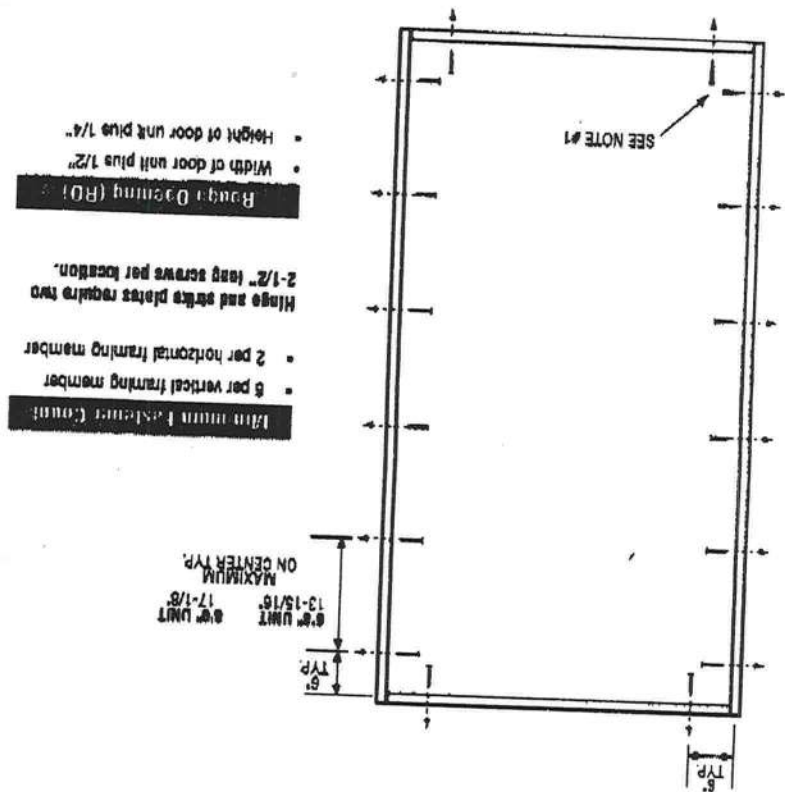


Glass inserts to be submitted by Intertek Testing Services/ETL Sample or approved validation service.

Intertek
Test Data Review Certificate #3026447A, #3026447B, #3026447C and CDP/TEST RATION VALIDATION
Marking #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides
additional information - available from the ITS/WH website (www.itswh.com), the Masonite
website (www.masonite.com) or the Masonite technical center.

MID-WL-MA0001-02

SINGLE DOOR



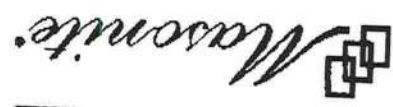
Watershed Technology
The Data Review Certificate #2026447A, #2026447B, #2026447C and COP/Part Report Validation Matrix
Additional information - available from the ITSMW website (www.watershed.com), the Masonite website
(www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3246*, 3247* or 3266
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #6 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.



March 10, 2002
Our continuing program of product improvement makes specifications design and product detail subject to change without notice.

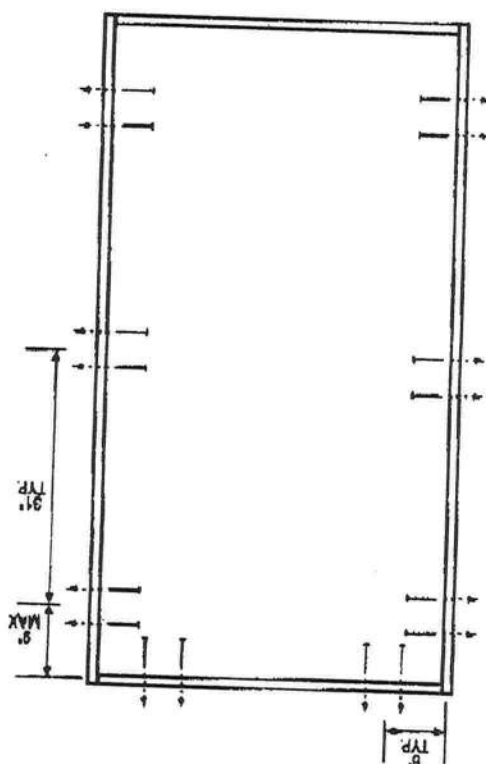
1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use, (Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Nailite Builders Choices 490 (or equal structural adhesives).
2. The common nail single shear design values come from ANSI/AP & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

:SB10N

Based on required Design Pressure - see COP sheet for details.

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed. UNITS COVERED BY COP DOCUMENT 0246, 3247, 3248, 3285, 3287, 3288 (ANSI/BHMA A156.18) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

Latching Hardware:

[illegible]

Bought Quantity (RO)

- Weight of door unit plus 1/2"
• Height of door unit plus 1/4"

Hinge and screw plates require the 2-1/2" long screws per location.

- 8 per vertical framing member for 7'0" height and smaller
- 6 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Information Systems Council

SINGLE DOOR

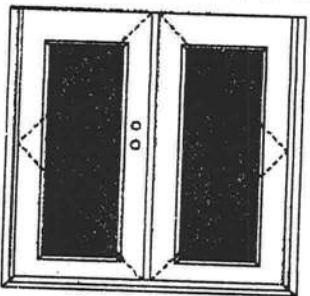
20-1000W-7M-DIW

X
Unit

COP-WI-FN4162 02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

This Data Review Certificate A20264ATC and COP/Ref Report Validation Matrix A20264ATC-DOT provides additional information - available from the IBC/WH website (www.intertek.com). The website (www.intertek.com) or the MacIntosh technical center.



Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure
+50.5/-50.5

Large Missile Impact Resistance
Limited water under special threshold design is used.

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

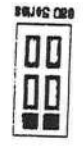
Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

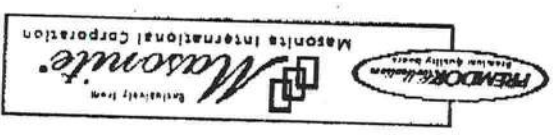
Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



1/2 GLASS:



Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, model and product names subject to change without notice.

*The glass kit may also be used in the following door styles: 5-panel, 5-panel with scroll, Eyebrow 5-panel, Eyebrow 5-panel with scroll.

COP-WI-FN416Z-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

2/4 GLASS:



FULL GLASS:

XX
Glazed Outswing Unit

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Pomey, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip like surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt A. Bathy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



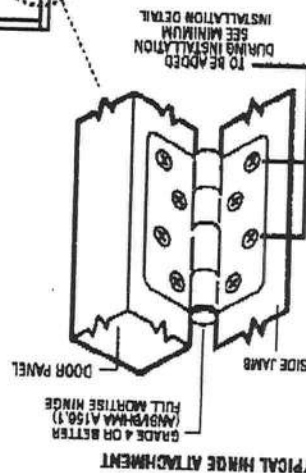
Test Data Review Certificate #0284470
and COP/WI Report Inspection Matrix
#0284470-001 (PDSM) is available from the ITSMH
website (www.elsa.com). The
website (www.elsa.com) is
located at the Harsco Technical Center.



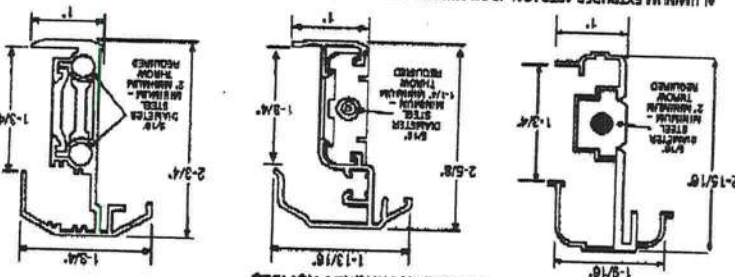
Entry Systems
Entergy

Our ongoing program of product improvement makes specifications, design and product
data subject to change without notice.

June 17, 2002

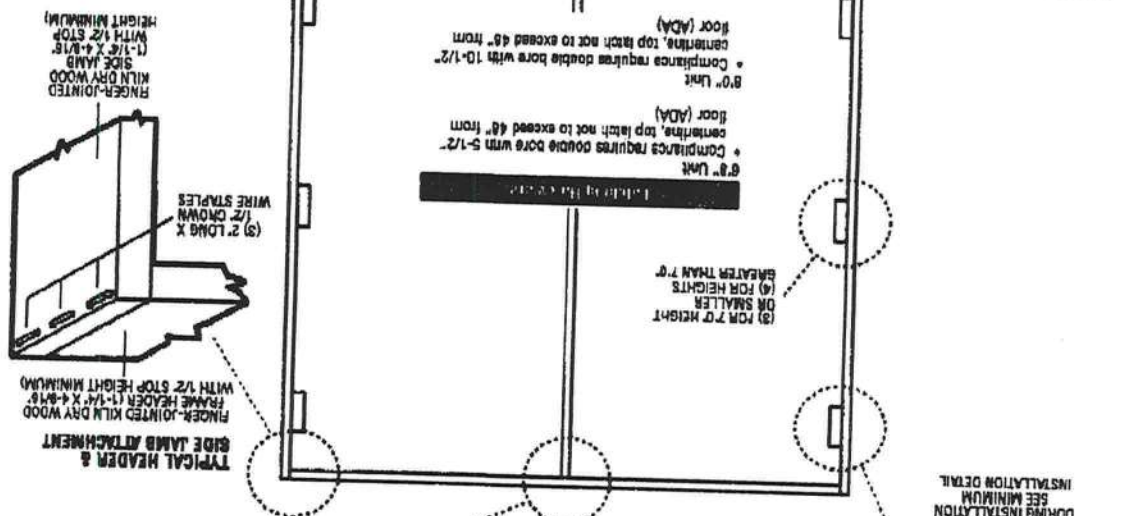
XX
UnitXX
Unit

— TO BE ADDED
DURING INSTALLATION
SEE MINIMUM
INSTALLATION DETAIL

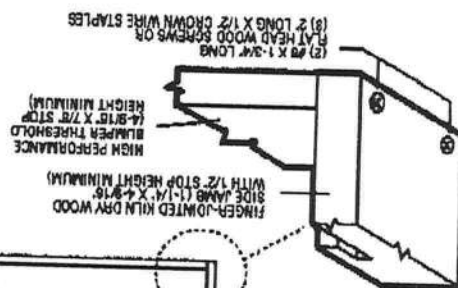


TYPICAL ASTRAGAL PROFILES

ALUMINUM EXTRUDED ASTRAVAL (DO NOT MINIMIZE WALL THICKNESS) WITH ADDED REINFORCEMENT INSETS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDRICAL/DEADEND LATCHING LOCATIONS. ATTACH WITH #4 X 1" HEAD SCREWS - LOCATE 1" FROM EACH END MINIMUM AND 22" O.C. MAXIMUM.



TYPICAL THRESHOLD &
SIDE JAMB ATTACHMENT

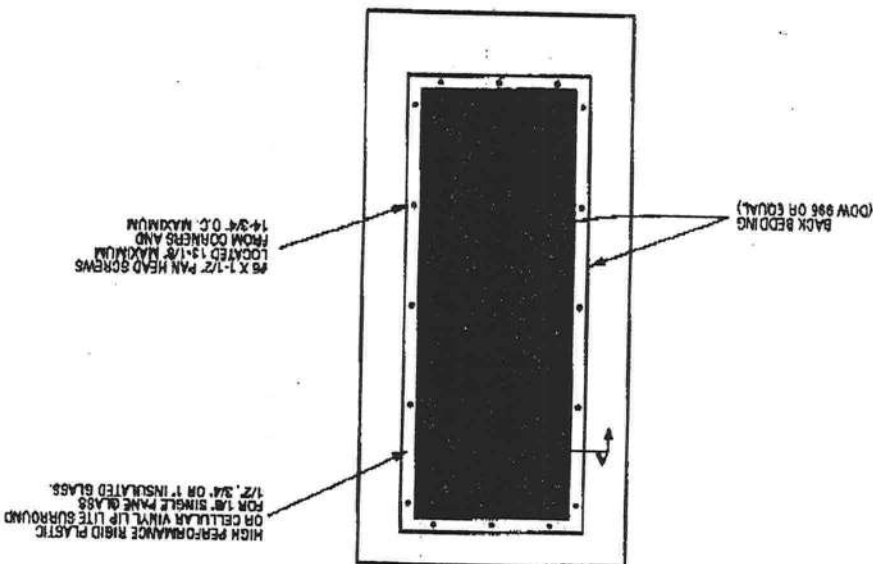


TYPICAL THRESHOLD &
SIDE JAMB ATTACHMENT

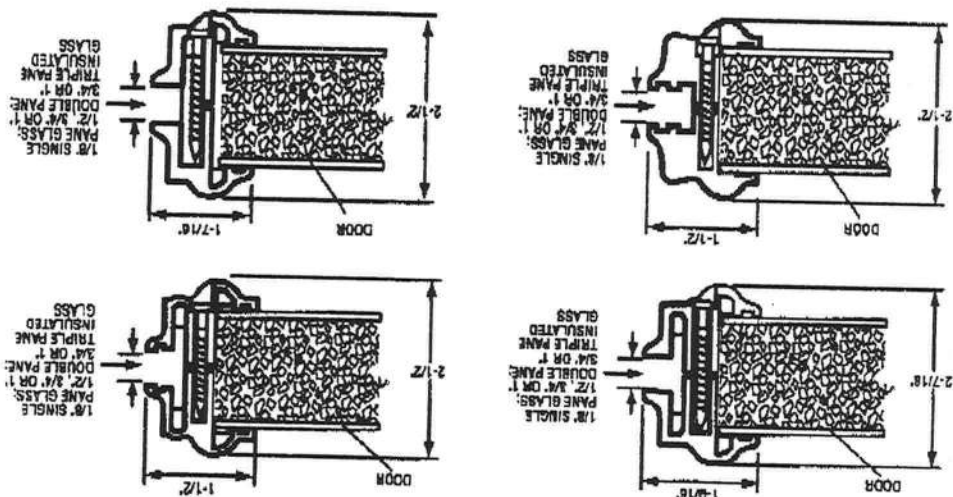
Over continuing program of product improvements, design and product details subject to change without notice.

GLASS INSERT IN DOOR OR SIDELITE PANEL

MA0041-02



SECTION A-A TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Samko or approved validation service.

Test Data Review Certificate #30294474, #30294475, #30294476 and COP/Retest Report Validation Reports (www.masonite.com) or the Masonite Technical Center. Additional information - available from the IFS/WH website (www.pdsamko.com), the Masonite website (www.masonite.com) or the Masonite Technical Center.

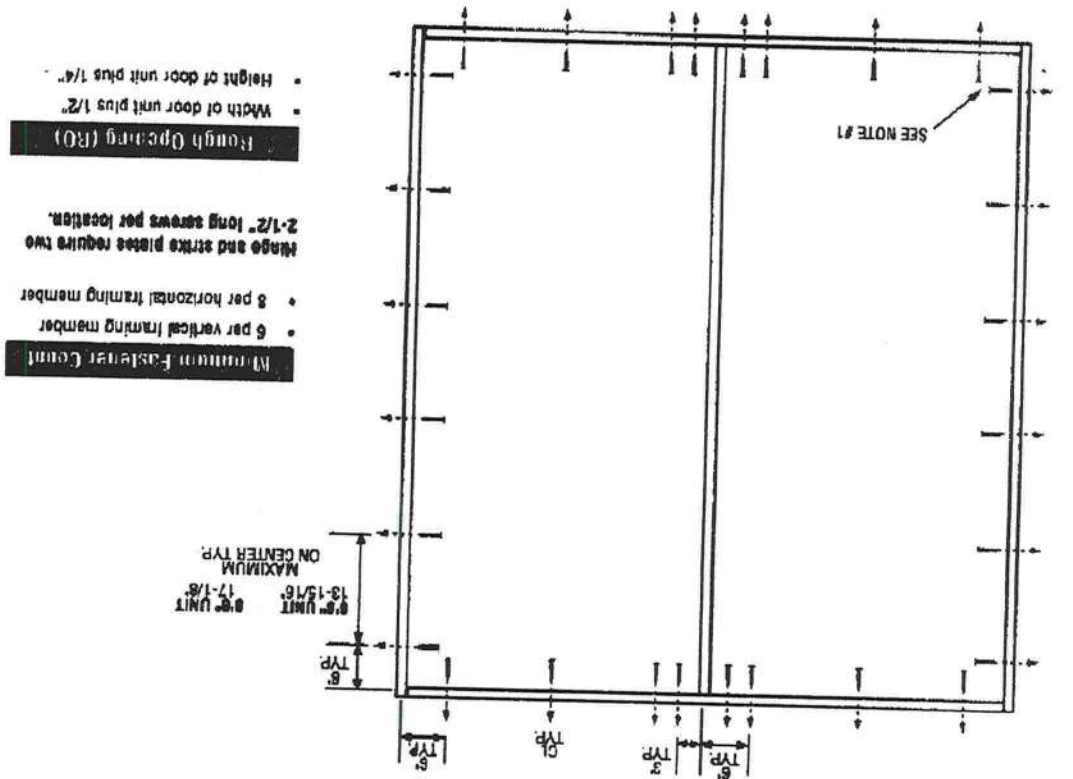
1. Anchor fasteners have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 450 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSVAF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Notes:

- Based on required Design Pressure - see COP sheet for details.
- and (1) at bottom.
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top
- UNITS COVERED BY COP DOCUMENT 8247, 8287, 8242, 8247, 8282 or 8287
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Latching Hardware:

For CDR Review Comments #302847A, #302847B, #302847C and COP/Ret Report Validation Matrix #302847A-001, 002, 003, 004, #302847B-001, 002, 003, 004, #302847C-001, 002, 003, 004 provide additional information - available from the ITB/WH website (www.masonite.com), the Masonite website (www.masonite.com) or the Masonite technical center.



DOUBLE DOOR

MID-WI-MA0002-02

Unit XX

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/APA & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Notes:

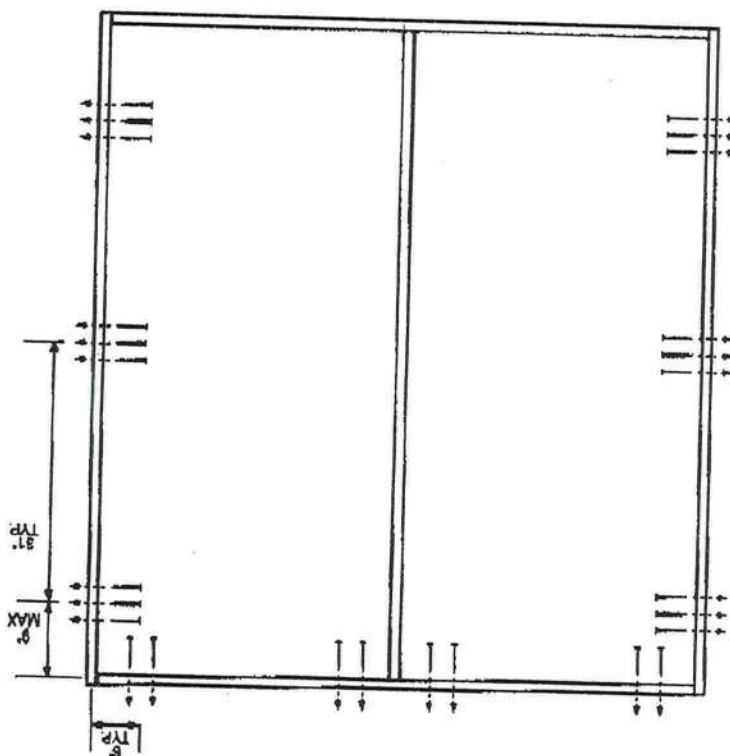
- Based on required Design Pressure - see COP sheet for details.
- Compliance requires that 8" GRADE 1 (ANSI/BHMA A158.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.
- UNITS COVERED BY COP DOCUMENT 0247, 0267, 3242, 3247, 3262 or 3267
- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Latching Hardware:

THIS DATA HAS BEEN CHECKED BY THE FOLLOWING: 020547A-001, 002, 003, 004; 020547B-001, 002, 003, 004 provides additional information - available from the TSCA website (www.epa.gov/tscatxt). The Masonite website (www.masonite.com) or the Masonite technical center.



- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member
- Hinge and strike plates require two 2-1/2" long screws per location.
- Rough Opening (RO)
- Width of door unit plus 1/2"
- Height of door unit plus 1/4"



DOUBLE DOOR

MID-WL MA0002 02

Unit
XX

- Test Reports
- 165 Single Hung
- #CTLA-787W (Fin)
- #CTLA-787W-1 (Flange)
- 740/744 Single Hung
- #01-40351.03 (Fin)
- #01-40351.04 (Flange)
- 165/740/744 Fixed
- #NCTL-310-0005-2.1 (Fin)
- # NCTL-310-0005-5.1 (Flange)
- #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

740/744 SINGLE HUNG (FIN & FLANGE)
165 SINGLE HUNG (FIN & FLANGE)
BB165/740/744 FIXED (FIN & FLANGE)



MI Home Products, Inc.
 650 West Market St.
 P.O. Box 370
 Gratz, PA 17030-0370

(717) 365-3300
 (717) 362-7025 Fax



Allen N. Reeves
15 FEBRUARY 2002

MAH:baw

Mark A. Hess, Technician

For ARCHITECTURAL TESTING, INC.

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft ²
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

TYPE: Aluminum Single Hung Window with Nail Fin

SERIES/MODEL: 740/744

MI HOME PRODUCTS, INC.

Rendered to:

AAMA/NWDA 101/L.S-2-97
TEST REPORT SUMMARY



THIS FENESTRATION PRODUCT COMPLIES * WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS,
EXPOSURE "B" (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST),
AND WALL ZONE "S" (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER ASTM E1300, THE CORRECT GLASS THICKNESS, BASED ON THE NEGATIVE
DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT.
THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

Series 470HP SLIDING GLASS DOOR - all 6'-8" High Panels

- 2'-6" WIDE DP + 40.0 / - 55.4
- 3'-0" WIDE DP + 40.0 / - 48.5
- 4'-0" WIDE DP + 40.0 / - 40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND
AIR INFILTRATION PER ATTACHED AAMA PERFORMANCE LABEL. BE ADVISED THAT
IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT
MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO
MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686

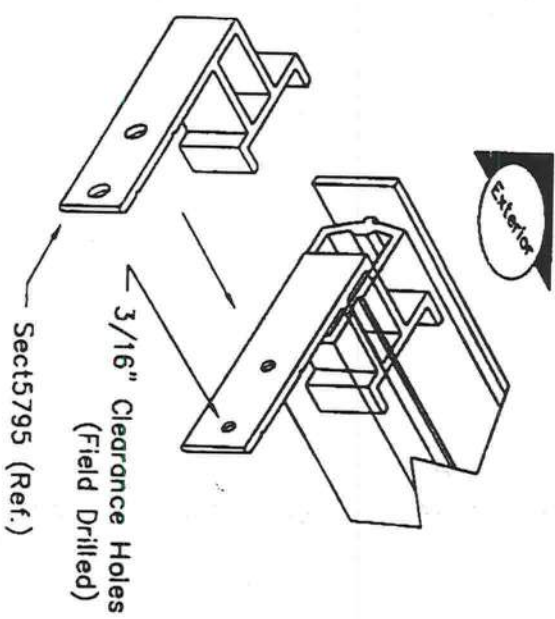
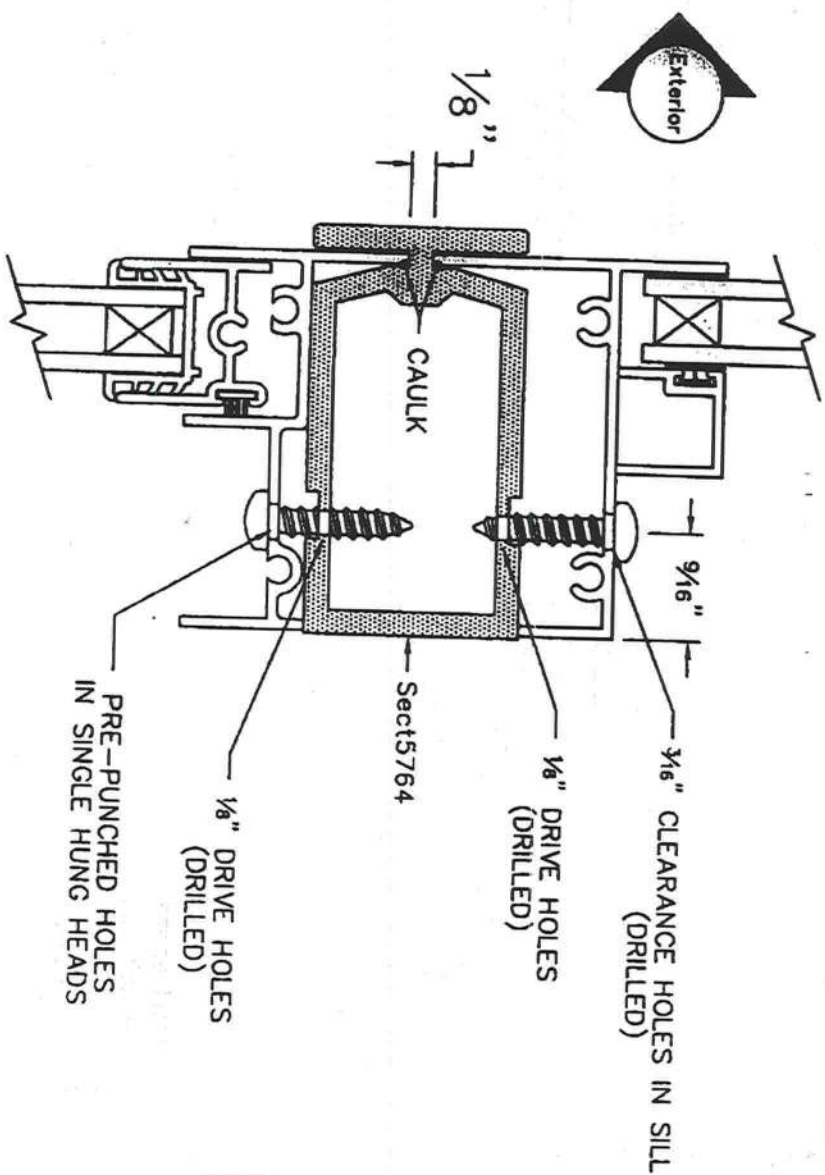
Series V83 HORIZONTAL MULLION for SINGLE UNITS - Florida Flange



165 & 740/744

NOTE: LENGTHS FOR STANDARD WIDTH UNITS ARE 19 1/8", 26 1/2", 37", AND 53 1/8".

- Step 1.** Position horizontal mull on top of lower unit as shown below. With 1/8" drill, drill up through pre-punched holes in the single hung heads into the mull. Before attaching with #8 x 3/4" screws (not included), run a full length bead of caulk in area shown.
- Step 2.** Position top unit on top of mull and drill 1/8" holes, in position shown, on same centers as lower unit. With 3/16" drill, re-drill holes in sill only and fasten with screws.
- Step 3.** Before lifting into rough opening, Drill two holes in each clip #SECT5795 and insert into each end of mull as shown below with tab pointing to inside. Fasten each clip tab to construction with two #10 x 1 1/2" screws for structural integrity.



DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.



Allen M. Riser
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A		
	Grade: 10		

Lock Manipulation Test	No entry	No entry
Test A7	No entry	No entry
Test A1 thru A5	No entry	No entry
Lock Manipulation Test	No entry	No entry

Optional Performance

4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds) @ 45.0 psf (positive) @ 45.0 psf (negative)	0.91"*	0.29" max.
		0.97"*	0.29" max.
	* Exceeds L/175 for deflection, but meets all other test requirements.		

4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds) @ 67.5 psf (positive) @ 67.5 psf (negative)	0.14"	0.20" max.
		0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002



Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft ²	0.30 cfm/ft ² max.
Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/IS-2-97 for air infiltration.			

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.86" 0.81"	0.29" max. 0.29" max.
Note: * Exceeds L/175 for deflection, but meets all other test requirements.			

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" <0.01"	0.20" max. 0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs Top rail Bottom rail In remaining direction at 50 lbs	0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100%

Left stile
Right stile
0.03"/6%
0.03"/6%



Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypropylene with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypropylene with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypropylene with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypropylene	1 Row	Active sash stiles

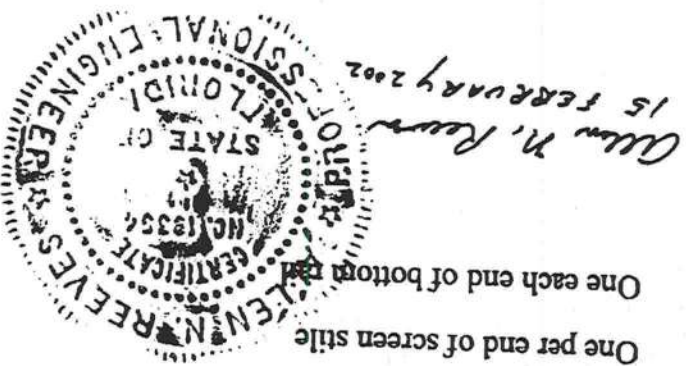
Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

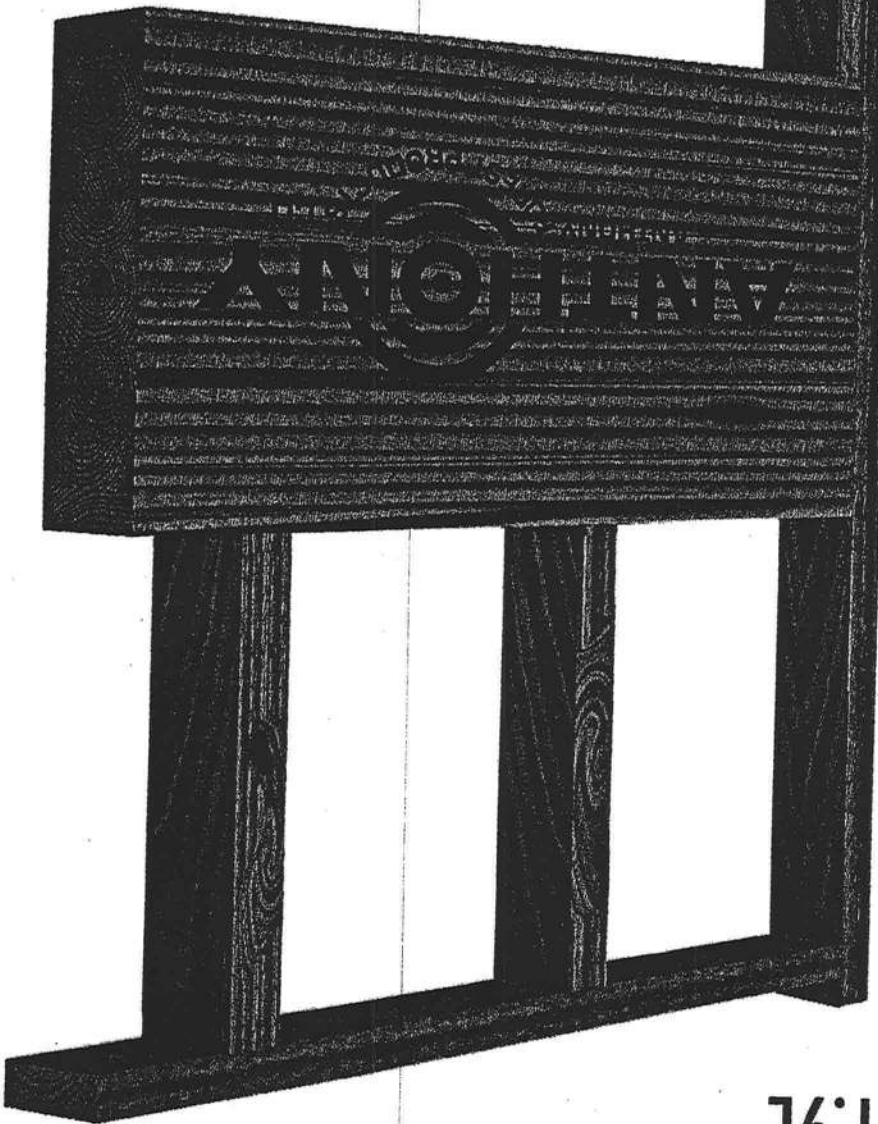
Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail



Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- ◆ Less Expensive than LVL or PSL
- ◆ Lighter than Steel, LVL or PSL
- ◆ Pre-Cut Lengths
- ◆ Renewable Resource
- ◆ Cambered or Non-cambered
- ◆ 3-1/2" Width to Match Framing
- ◆ One Piece - No Nail Laminating
- ◆ Lifetime Warranty

Garage Header Sizing Tables

26F_b - 1.9E

1/2 ROOF SPAN

NOV 18 1963

NON-SNOW LOAD AREA USING LOAD DURATION FACTOR = 1.25

1. Table assumes a simple span header supporting a uniform load transferred from $1/2$ the roof span plus a 2' soffit

1. Table assumes a simple span header supporting a uniform load transferred from $1/2$ the roof span plus a 2' soffit



4. Headers are assumed to have continuous lateral support along top edge

5. Bearing length based on full width bearing is indicated as follows:

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

Anthony Power Header®

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% SNOW LOAD AREA									
844	896	1216	1573	510	552	669	752	824	789
161	207	254	390	510	552	669	752	824	789
114	145	180	231	277	359	391	510	534	653

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% NON-SNOW LOAD AREA									
844	975	1322	390	510	552	724	752	897	693
161	207	254	390	510	552	724	752	897	693
114	145	180	231	277	359	391	510	534	699

GARAGE HEADER SUPPORTING ROOF, WALL AND FLOOR LOADS - 100% FLOOR DURATION									
562	778	888	1056	1363	1367	1582	540	501	715
107	153	169	245	260	380	368	540	501	715
76	107	120	171	185	267	261	380	356	521

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

26F_b - 1.9E

2600	FLEXURAL STRESS (F_b) =
740	COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =
225	HORIZONTAL SHEAR (F_v) =
1.9×10^6	MODULUS OF ELASTICITY (MOE) =

Standard Deviation (in %)	5.12	10.07	9.83	11.64	12.51	11.57	11.57
Minimum (in %)	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Maximum (in %)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
Weighted Average	326	514	789	1115	1521	2014	2604
Mean (in %)	8865	12015	15996	20145	24772	29877	35460
Mode (in %)	3908	4550	5250	5892	6533	7175	7817

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a
3. Flexural Stress, F_b , shall be modified
4. Allowable design properties and loa
5. The AISC NER 466 was used in cal

2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b , shall be modified by the Volume Factor, C_v as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

9000 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	3-1/2" x 16"
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"

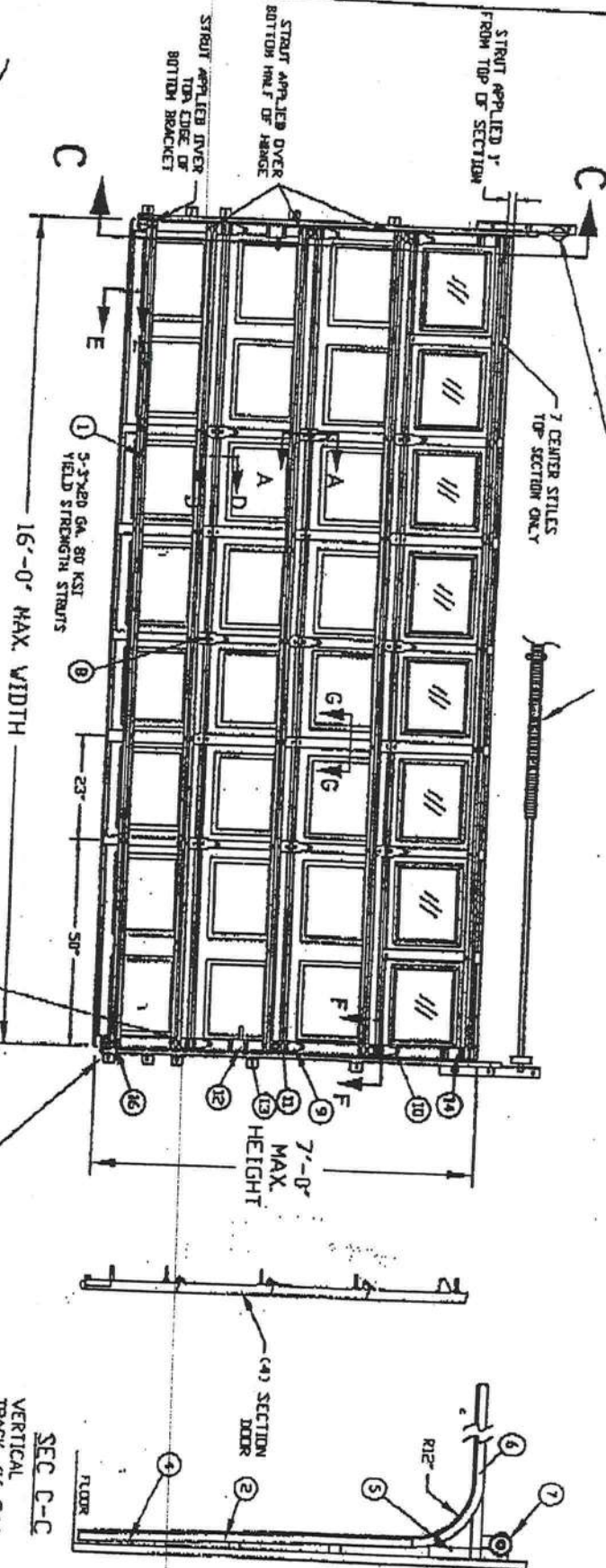
For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

© 2001 Anthony Forest Products Company

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-130
2. MAXIMUM SECTION HEIGHT- 27'
3. SECTION HEIGHTS OF 210" AND 195" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS
4. VARIOUS MAY BE INSTALLED IN THE TOP SECTION, OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION
5. MINIMUM LENGTH OF ROLLER STUD IS 5'-0" AS TESTED
6. THE STRUT PLACEMENT IN DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN
7. STRUTS SECURED AT ALL LOCATIONS WITH HEX SCREWS
8. QUANTITY OF SIDE LOCKS CAN BE 0, 1, OR 2 AS TESTED
9. DROP IN TYPE OF INSTALLATION IS OPTIONAL

NOT PART OF WIND LOAD SYSTEM
EXTENSION SPRING COUNTERBALANCE
TORSION SPRING COUNTERBALANCE



INSIDE ELEVATION

ALL ROLLER CARRIERS
AND HINGES ARE 14 GA

SEC C-C
VERTICAL
TRACK, 06 GA.)

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH
LOWEST BRACKET APPROX. 3" FROM FLOOR, 2ND BRACKET
NEAR THE HORIZONTAL C OF THE BOTTOM SECTION, AND 3RD
BRACKET NEAR THE TOP OF THE BOTTOM SECTION

The seal on this drawing only represents the product(s) illustrated and described herein (s) and does not represent the door as tested.



TEST REPORTS IN FILE VIDEO 10/19/00 400993

GATED DOORS

SERIES 7400, EXTERIOR STEEL = 0.07 MIN AS TESTED
SERIES 7025, EXTERIOR STEEL = 0.07 MIN
SERIES 7024, EXTERIOR STEEL = 0.07 MIN
(TESTED WITH VARIOUS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS BR KSI	STRUTS SIZE	STRUTS QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.	



GENERAL AMERICAN DOOR COMPANY
5050 BASEL AVE. ROAD
MONTGOMERY, IL 60738

SCALE 1/8" = 1'-0"	APPROVED BY	DATE 11-20-00	REVISION (A) 11-20-00
16' x 7' MAX. GATED PANEL STEEL DOOR - VERTICAL TRACK - 20 PSF			

PAGE 1 OF 2

DRAWING NUMBER V13220-1

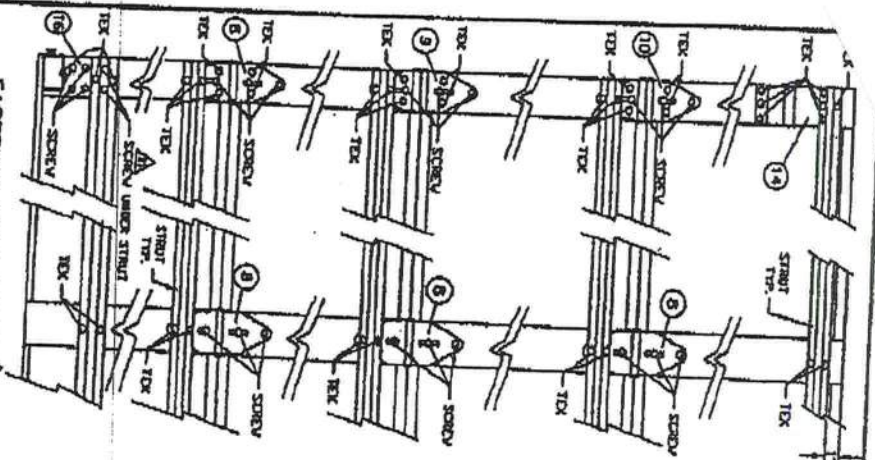
REV.	DATE	BY	DESCRIPTION
A-1	11-20-00	DM	SEE E.C.M. 611

The seal on this drawing only illustrates that the product(s) represented and described herein dimensions and installation(s) of the door as tested.

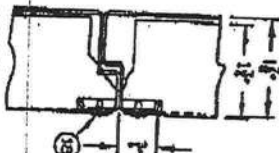
SEAL
PE No. 024280

NORTH CAROLINA
PROFESSIONAL

FASTENER ARRANGEMENT A



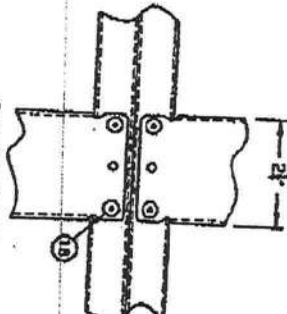
SEC. D-D
PAN ATTACHMENT
TO STILE
AS TESTED



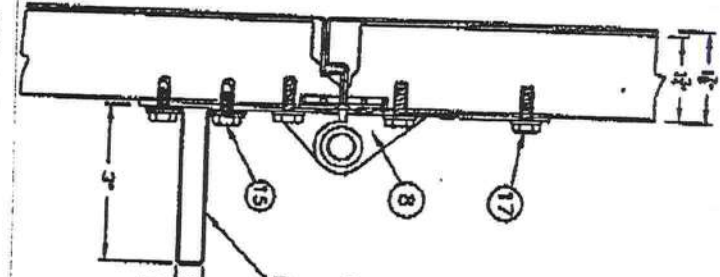
SEC. D-D
PAN ATTACHMENT
TO STILE
(OPTIONAL)



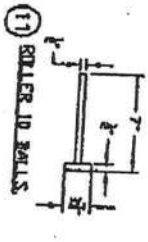
SEC. G-G
CENTER STILE
20 GA. GALV. VAMIZED



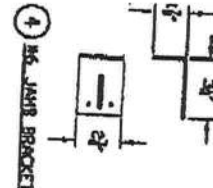
SEC. A-A



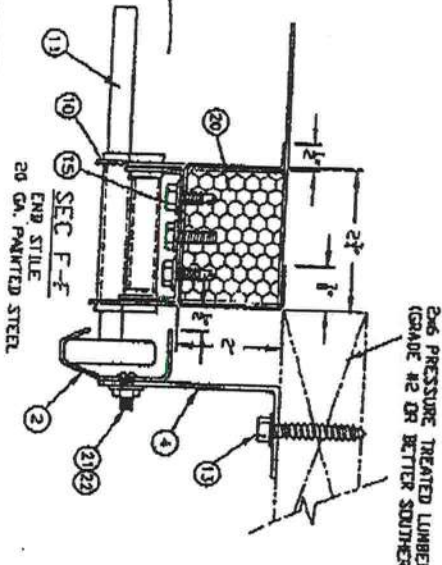
5-3"x20 GA. 80 KSI YIELD
STRENGTH REDUCED
STRENGTH APPLIED WITH
2 TEX SCREWS PER HINGE
OR STILE LOCATION
Q4 PER STILE, MINIMUM



TRACK
1/8 GA. GROSS MIN.



1-20 x 3"
HARD TEX SCREW
WITH #2
REDUCED POINT



SEC. F-F
END STILE
20 GA. PAINTED STEEL



SEC. E-E



ITEM	DESCRIPTION	QTY	UNIT
1	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
2	20 GA. PAINTED STEEL	1	EA
3	20 GA. GALV. VAMIZED	1	EA
4	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
5	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
6	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
7	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
8	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
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47	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
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69	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
70	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
71	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
72	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
73	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
74	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
75	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
76	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
77	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
78	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
79	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
80	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
81	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
82	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
83	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
84	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
85	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
86	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
87	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
88	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
89	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
90	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
91	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
92	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
93	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
94	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
95	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
96	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
97	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
98	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
99	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA
100	1/2" x 20" x 3" HARD TEX SCREW WITH #2 REDUCED POINT	1	EA

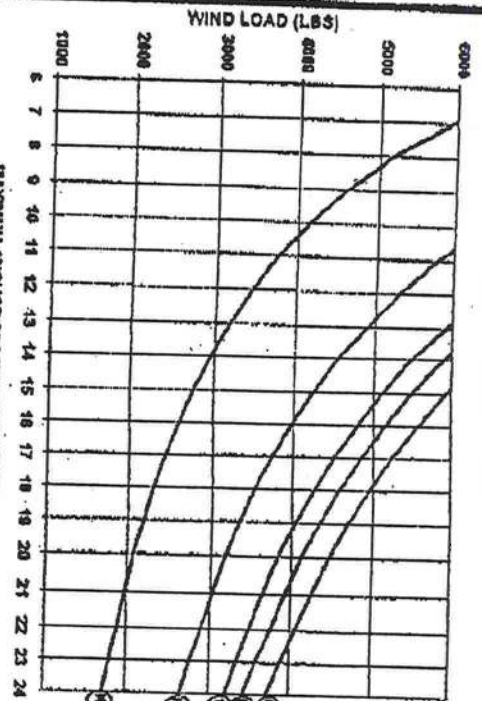


GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

REVISIONS
REV. 11-2-00
REVISED (3) 12-1-00

PAGE 2 OF 2
DRAWING NUMBER
V13220-2

WIND LOAD VS ANCHOR SPACING



DESIGN QBSY X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)

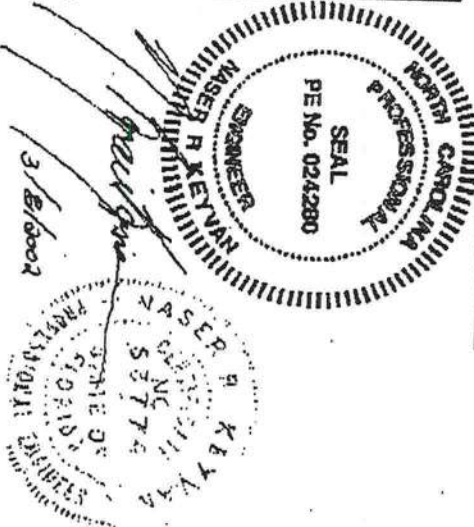
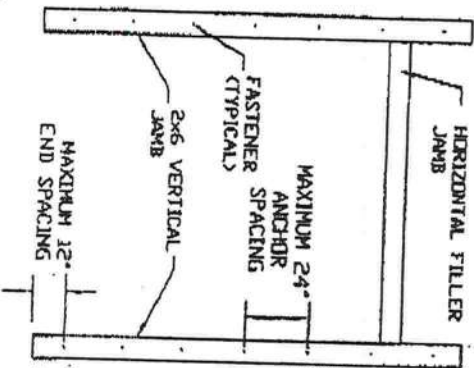
LOAD FT²

MAXIMUM ANCHOR SPACING (INCHES) PER EACH JAMB

EXAMPLE

- 30 LBS X 66 FT WIDE X 8 FT HIGH = 3960 LBS
- FT²
- (1) USE 22" SPACING
 - (2) USE 21" SPACING
 - (3) USE 19" SPACING
 - (4) USE 16" SPACING
 - (5) USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2X6 PRESSURE TREATED GRADE #2 OR BETTER SOUTHERN PINE WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SDOCA STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTD 10, CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS, STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (S2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FLOTTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE 2X6 WOOD JAMB SHALL BE ANCHORED TO STUDS, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS, ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2500 PSI (GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI) REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY	
5000 BASELINE ROAD	
MONTGOMERY, IL 60538	
MODEL NONE DATE 8-30-99 REVISIONS JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	DRAWN BY: DJV CHECKED MASTER DRAWING NUMBER: A10560 DESIGN NUMBER



**PRESTIQUE[®]
HIGH DEFINITION[™]**



RAISED PROFILE[™]

**Prestitque Plus High Definition[™]
and Prestitque Gallery Collection[™]**

Raised Profile

Product size 13"x 39" Exposure 5" Pieces/Bundle 16 Bundles/Square 4/98.5 sq. ft. Squares/Pallet 11 50-year limited warranty period: non-protected coverage for shingles and application labor for the initial 5 years, plus an option for transferability [®] ; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty [®] .	Product size 13"x 38" Exposure 5" Pieces/Bundle 22 Bundles/Square 3/100 sq. ft. Squares/Pallet 16 30-year limited warranty period: non-protected coverage for shingles and application labor for the initial 5 years, plus an option for transferability [®] ; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty [®] .
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Prestitque I High Definition

Product size 13"x 39" Exposure 5" Pieces/Bundle 16 Bundles/Square 4/98.5 sq. ft. Squares/Pallet 14 40-year limited warranty period: non-protected coverage for shingles and application labor for the initial 5 years, plus an option for transferability [®] ; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty [®] .

**HIP AND RIDGE SHINGLES
Seal-A-Ridge[®] w/FLX[™]**

Size: 12" x 12"
Exposure: 6"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Product size 13"x 38" Exposure 5" Pieces/Bundle 22 Bundles/Square 3/100 sq. ft. Squares/Pallet 16 30-year limited warranty period: non-protected coverage for shingles and application labor for the initial 5 years, plus an option for transferability [®] ; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty [®] .
--

Elk Starter Strip
52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

SPECIFICATIONS

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment for steep slopes.

WARRANTIES: Warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Available Colors: Antique Slate, Weatheredwood, Shakedown, Sienna Sunset[™].
Gallery Collection: Balsam Forest[™], Weathered Sage[™], Sienna Sunset[™].
All Prestitque, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard[®] sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.
Check for availability with built-in StainGuard[®] treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.
All Prestitque and Raised Profile shingles meet UL[®] Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.
All Prestitque and Raised Profile shingles meet the latest Metro Dade building code requirements.
*See actual limited warranty for conditions and limitations.
**Check for product availability.

Residential System Sizing Calculation

Summary

H-ART
Project Title:
H-ART Residence - Lot 9 Shannon

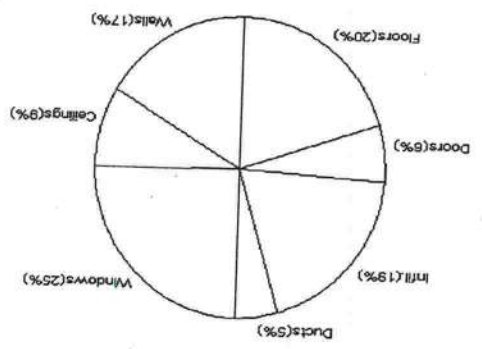
EWPL INC
Fort White, FL

Code Only
Professional Version
Climate: North
9/12/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature	31 F	Summer design temperature	93 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	39 F	Summer temperature difference	18 F
Total heating load calculation	33471 Btuh	Total cooling load calculation	32647 Btuh
Submitted heating capacity	36000 Btuh	Submitted cooling capacity	36000 Btuh
Submitted as % of calculated	107.6 %	Submitted as % of calculated	110.3 %

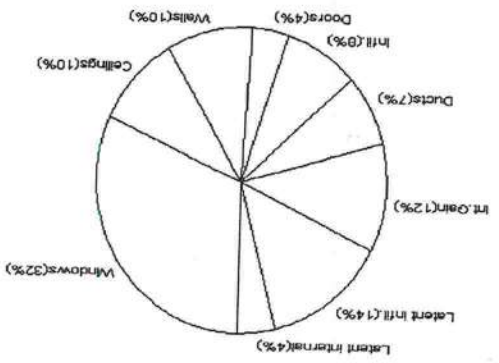
WINTER CALCULATIONS

Winter Heating Load (for 2207 sqft)	
Load component	
Window total	296 sqft
Wall total	1890 sqft
Door total	134 sqft
Ceiling total	2207 sqft
Floor total	210 ft
Infiltration	147 cfm
Subtotal	31877 Btuh
Duct loss	1594 Btuh
TOTAL HEAT LOSS	33471 Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2207 sqft)	
Load component	
Window total	296 sqft
Wall total	1890 sqft
Door total	134 sqft
Ceiling total	2207 sqft
Floor total	2207 sqft
Infiltration	129 cfm
Internal gain	3800 Btuh
Subtotal(sensible)	24357 Btuh
Duct gain	2436 Btuh
Total sensible gain	26793 Btuh
Latent gain(infiltration)	4474 Btuh
Latent gain(internal)	1380 Btuh
Total latent gain	5854 Btuh
TOTAL HEAT GAIN	32647 Btuh



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: _____
DATE: _____

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
Fort White, FL
Project Title: Hunt Residence - Lot 9 Shannon
Code Only
Professional Version
Climate: North

9/12/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - Manual J Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

EWPL INC

Fort White, FL

Project Title:
Hunt Residence - Lot 9 Shannon

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

9/12/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
2	2, Clear, Metal, DEF	S	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	S	9.3	28.3	264 Btuh
4	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	N	17.5	28.3	495 Btuh
6	2, Clear, Metal, DEF	N	60.0	28.3	1698 Btuh
7	2, Clear, Metal, DEF	N	42.0	28.3	1189 Btuh
8	2, Clear, Metal, DEF	W	21.3	28.3	604 Btuh
9	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
10	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
11	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
Window Total					
296					
Area X					
HTM=					
28.3					
Load					
1019 Btuh					
377 Btuh					
264 Btuh					
849 Btuh					
495 Btuh					
1698 Btuh					
1189 Btuh					
604 Btuh					
849 Btuh					
566 Btuh					
453 Btuh					
8363 Btuh					
Load					
5313 Btuh					
282 Btuh					
5595 Btuh					
Load					
718 Btuh					
323 Btuh					
331 Btuh					
718 Btuh					
2089Btuh					
Load					
2869Btuh					
2869 Btuh					
Ceilings					
1					
Under Attic					
Type					
R-Value					
30.0					
2207					
Area X					
HTM=					
1.3					
2869 Btuh					
Load					
2869Btuh					
Ceiling Total					
2207					
Floors					
1					
Slab-On-Grade Edge Insul					
Type					
R-Value					
0					
210.0 ft(p)					
Size X					
HTM=					
31.6					
6636 Btuh					
Load					
6636 Btuh					
Floor Total					
210					
ACH X					
0.40					
Building Volume					
22070(sqft)					
CFM=					
147					
6325 Btuh					
Load					
6325 Btuh					
Infiltration					
Type					
Natural					
Mechanical					
Infiltration Total					
0					
147					
6325 Btuh					
Load					
6325 Btuh					
Infiltration Total					
147					
6325 Btuh					
Load					
6325 Btuh					
Infiltration					
Type					
Natural					
Mechanical					
Infiltration Total					
0					
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Load					
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Infiltration					
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Type					
Natural					
Mechanical					
Infiltration Total					
0					
147					
6325 Btuh					
Load					
6325 Btuh					
Infiltration					
Type					
Natural					
Mechanical					
Infiltration Total					
0					
147					
6325 Btuh					
Load					
6325 Btuh					
Infiltration					

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC

Fort White, FL

Project Title:
Hunt Residence - Lot 9 Shannon

Code Only
Professional Version
Climate: North

9/12/2005

Totals for Cooling	Subtotal	24357 Btuh
	Duct gain(using duct multiplier of 0.10)	2436 Btuh
	Total sensible gain	26793 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	4474 Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380 Btuh
	Latent other gain	0 Btuh
TOTAL GAIN		32647 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC

Fort White, FL

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 18.0 F

9/12/2005

Project Title:

Hunt Residence - Lot 9 Shannon

Code Only

Professional Version

Climate: North

Window		Type	Overhang	Window Area(sqft)	HTM	Load
1	2, Clear, DEF, N, N	S	1.5	7.5	36.0	37
2	2, Clear, DEF, N, N	S	10.5	6.66	13.3	37
3	2, Clear, DEF, N, N	S	10	4	9.3	37
4	2, Clear, DEF, N, N	E	1.5	5.5	30.0	72
5	2, Clear, DEF, N, N	N	1.5	5.5	17.5	22
6	2, Clear, DEF, N, N	N	9	7	60.0	22
7	2, Clear, DEF, N, N	N	1.5	6.5	42.0	22
8	2, Clear, DEF, N, N	W	1.5	6.5	21.3	72
9	2, Clear, DEF, N, N	N	1.5	5.5	30.0	22
10	2, Clear, DEF, N, N	W	1.5	5.5	20.0	72
11	2, Clear, DEF, N, N	W	1.5	5	16.0	22
Window Total				296		10366 Btuh
Type		Len	Overhang	Window Area(sqft)	HTM	Load
Panes/SHGC/U/Insh/Exsh Omt		Hgt	Gross	Shaded	Unshaded	
1		7.5	36.0	36.0	0.0	792 Btuh
2		6.66	13.3	6.7	6.7	393 Btuh
3		4	9.3	9.3	0.0	205 Btuh
4		5.5	30.0	2.2	27.8	2048 Btuh
5		5.5	17.5	0.0	17.5	385 Btuh
6		7	60.0	0.0	60.0	1320 Btuh
7		6.5	42.0	0.0	42.0	924 Btuh
8		6.5	21.3	7.3	14.0	1170 Btuh
9		5.5	30.0	0.0	30.0	660 Btuh
10		5.5	20.0	1.5	18.5	1366 Btuh
11		5	16.0	1.0	15.0	1103 Btuh
Doors		Type	Area	HTM	Load	
1	Wood - Exter	40.0	10.0	10.0	399 Btuh	
2	Wood - Exter	18.0	10.0	10.0	180 Btuh	
3	Wood - Adjac	36.0	10.0	10.0	359 Btuh	
4	Wood - Exter	40.0	10.0	10.0	399 Btuh	
Door Total			134.0		1337 Btuh	
Type/Color		R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	2207.0	1.4	3134 Btuh	
Ceiling Total			2207.0		3134 Btuh	
Type		R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	210.0 ft(p)	0.0	0 Btuh	
Floor Total			210.0		0 Btuh	
Type		ACH	Volume	CFM=	Load	
	Natural	0.35	22070	129.0	2554 Btuh	
	Mechanical			0	0 Btuh	
Infiltration Total					2554 Btuh	
Internal gain						
Occupants			Bluh/occupant		Appliance	Load
6			X 300 +		2000	3800 Btuh

Project Information for:				Truss Design Load Information (UNO)			
Builder:	HUGO ESCALANTE	Date:	10/21/2005	Truss Page Count:	45	Design Program:	Mitek 5.2 / 6.2
Lot:	LOT 9 SHANNA MEADOWS	Start Number:	1100	County or City:	COLUMBIA COUNTY	Wind	
Subdivision:	N/A			Roof (psf):	42	Wind Standard:	ASCE 7-02
				Floor (psf):	55	Wind Speed (mph):	120
Note: See individual truss drawings for special loading conditions							
Building Designer, responsible for Structural Engineering: (See attached)				Address: P.O. BOX 280 FORT WHITE, FL. 32038			
Designer:				26			
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987 Company: Structural Engineering and Inspections, Inc. EB 9196 Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549							
Notes: 1. Truss Design Engineer is responsible for the individual trusses as components only. 2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet. 4. Trusses designed for vertical loads only, unless noted otherwise.							

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1021051100	10/21/2005	41	T33	1021051140	10/21/2005
2	CJ3	1021051101	10/21/2005	42	T34	1021051141	10/21/2005
3	CJ5	1021051102	10/21/2005	43	T35	1021051142	10/21/2005
4	EJ3	1021051103	10/21/2005	44	T36	1021051143	10/21/2005
5	EJ7	1021051104	10/21/2005	45	T36G	1021051144	10/21/2005
6	EJ7A	1021051105	10/21/2005				
7	EJ7B	1021051106	10/21/2005				
8	EJ7C	1021051107	10/21/2005				
9	HJ3	1021051108	10/21/2005				
10	HJ7	1021051109	10/21/2005				
11	PB01	1021051110	10/21/2005				
12	PB02	1021051111	10/21/2005				
13	PB03	1021051112	10/21/2005				
14	T01	1021051113	10/21/2005				
15	T02	1021051114	10/21/2005				
16	T03	1021051115	10/21/2005				
17	T04	1021051116	10/21/2005				
18	T05	1021051117	10/21/2005				
19	T06	1021051118	10/21/2005				
20	T07	1021051119	10/21/2005				
21	T08	1021051120	10/21/2005				
22	T09	1021051121	10/21/2005				
23	T10	1021051122	10/21/2005				
24	T11	1021051123	10/21/2005				
25	T12	1021051124	10/21/2005				
26	T17	1021051125	10/21/2005				
27	T17G	1021051126	10/21/2005				
28	T18	1021051127	10/21/2005				
29	T19	1021051128	10/21/2005				
30	T20	1021051129	10/21/2005				
31	T21	1021051130	10/21/2005				
32	T23	1021051131	10/21/2005				
33	T24	1021051132	10/21/2005				
34	T25	1021051133	10/21/2005				
35	T26	1021051134	10/21/2005				
36	T27	1021051135	10/21/2005				
37	T29	1021051136	10/21/2005				
38	T30	1021051137	10/21/2005				
39	T31	1021051138	10/21/2005				
40	T32	1021051139	10/21/2005				

OCT 21 2005

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Qualified Business License Required 11/24/2003

Special Qualifications
Effective Date

Expires:

08/31/2006

Licensure Date:

11/24/2003

Status:

Current, Active

License Number:

CRC1326967

Rank:

Cert Residential

License type:

Certified Residential Contractor

License Information

Main Address:

FORT WHITE, Florida 32038

P.O. BOX 280

EWPL INC (DBA Name)

ESCALANTE, HUGO (Primary Name)

Licensed Information

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Dwg.#1021051099

02:00:39 PM 10/6/2004

LOAD CASE(S) Standard

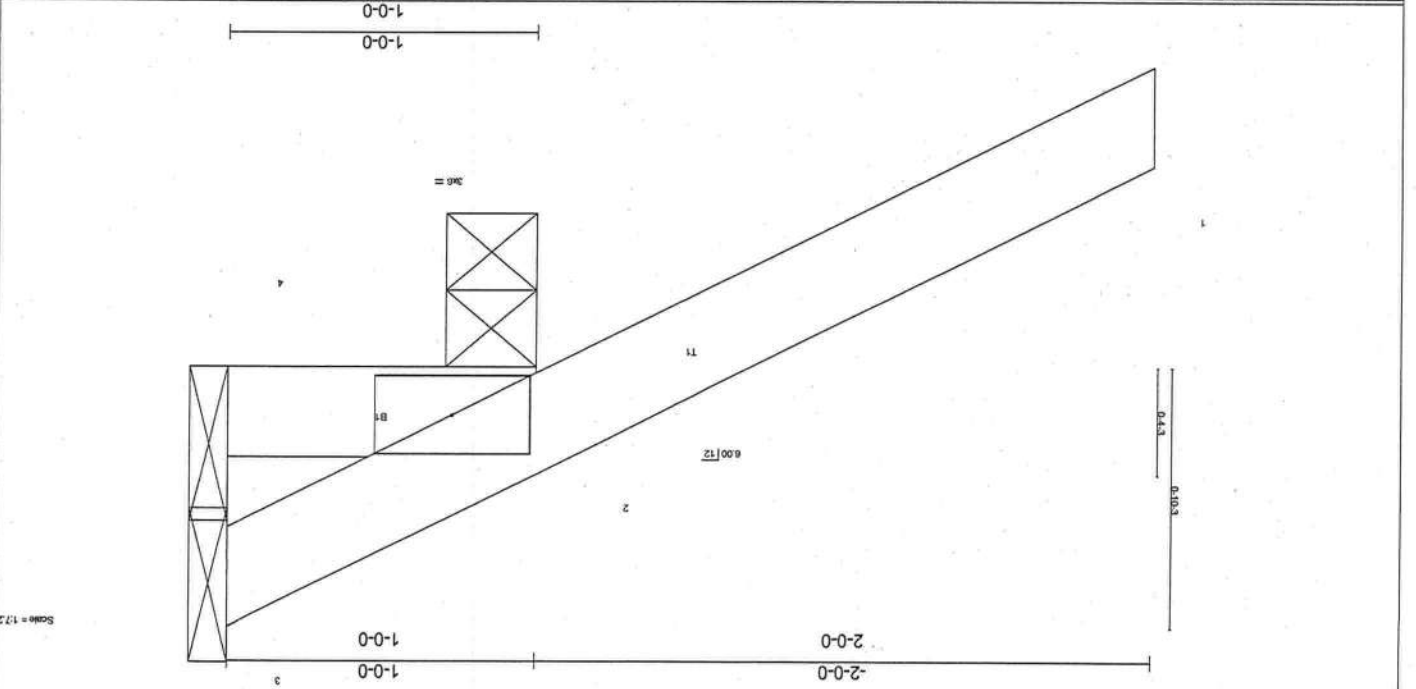
- NOTES
- 1) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDF=3.0psf; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for Extension(2) zone; porch left and right exposed; MWFRS for reactions specified.
 - 2) Refer to girder(s) for truss to truss connections.
 - 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 2, 11 lb uplift at joint 4 and 90 lb uplift at joint 3.

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-69/91
 BOT CHORD 2-4=0/0

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=90/Mechanical
 Max Horiz 2=103(load case 5)
 Max Uplift 2=-349(load case 5), 4=-11(load case 3), 3=-90(load case 1)
 Max Grav 2=266(load case 1), 4=14(load case 1), 3=154(load case 5)

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling wood sheathing directly applied or 1-0-0 oc bracing.

LOADING (psf)	SPACING	CSI	DEFL	Vert(LL)	Vert(TL)	Horz(TL)	Weight: 7 lb
TCDL 20.0	2-0-0	TC 0.33	in (loc)	-0.00	-0.00	0.00	PLATES
TCDF 7.0		BC 0.01	l/d	2 >999	2 >999	3 n/a	MT20
BCDF 10.0		WB 0.00	L/d	240	180	n/a	GRIP
BCDL 5.0							244/190



[illegible]

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-6, 4=42/Mechanical
Max Horiz Z=157(load case 5)
Max Uplift3=-34(load case 6), 2=-292(load case 5), 4=-33(load case 3)

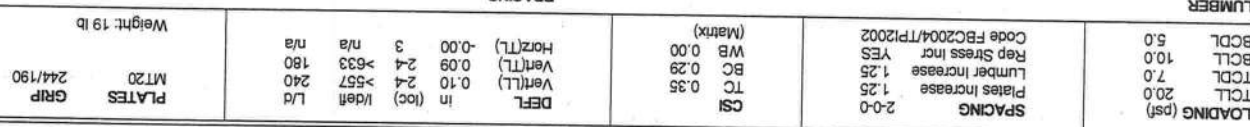
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-577
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Eave/roof(2) zone; porch left and right exposed; Lumber DOL=1.60 plate gnp DOL=1.60
MWFRS for reactions specified.
2) Refer to girders for truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3, 292 lb uplift at joint 2 and 33 lb uplift at joint 4.
Standard (S) CASES

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-6, 4=42/Mechanical
Max Horiz Z=157(load case 5)
Max Uplift3=-34(load case 6), 2=-292(load case 5), 4=-33(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-577
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Eave/roof(2) zone; porch left and right exposed; Lumber DOL=1.60 plate gnp DOL=1.60
MWFRS for reactions specified.
2) Refer to girders for truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3, 292 lb uplift at joint 2 and 33 lb uplift at joint 4.
Standard (S) CASES

[illegible]

BRACING
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horiz 2=21 (load case 5)

Max Uplift=107(load case 5), 2=320(load case 5), 4=57(load case 3)

NOTES

LOT CHORD 2-4=0/0

1-2=0/0, 2-3=10/20

Refer to attached for instructions for use in this connection.
 MWFRS for reactions specified:
 Exterior(2) zone: porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60; TCCL=4.26ft; BCDL=3.06ft; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCDL=3.06ft; DOL=1.60. This truss is designed for C-C for members and forces, and for

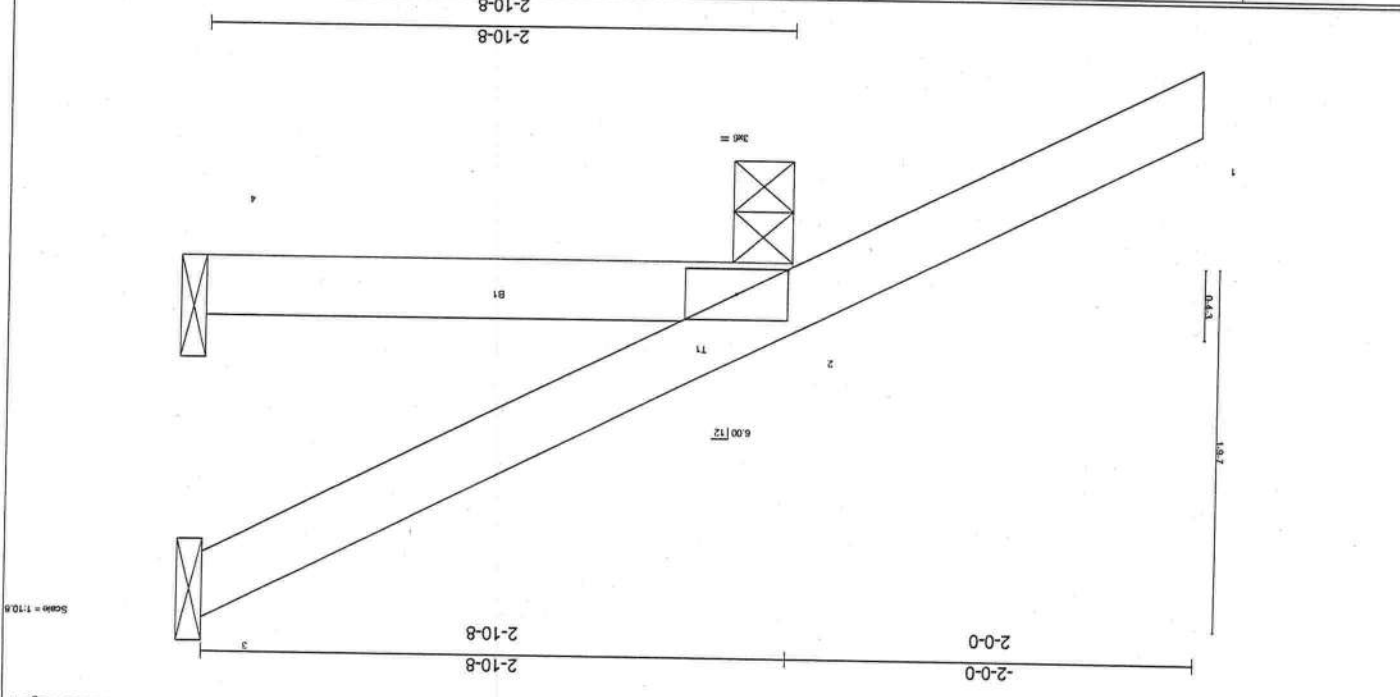
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 3, 320 lb uplift at joint 2 and 57 lb uplift at joint 4.

ROAD CASES) Standard

OCTOBER 21, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
15102 N. E.

18103 N. FLORIDA AVE., STE B, LOT 2, FL 33549

Job	Truss	Truss Type	MONO TRUSS	Qty	2	Ply	1	Job Reference (optional)	6.200 s Jul 13 2005 Miller Industries, Inc. Thu Oct 13 12:01:28 2005 Page 1
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LOADING (psf)	SPACING	CS	DEFL	BRACING	Weight: 13 lb
BCDL 5.0	Code FBC2004/TP12002	WB 0.00	Horz(TL) -0.00	TOP CHORD	PLATES
BCLL 10.0	Rep Stress Incr YES	BC 0.05	Vert(TL) -0.00	BOT CHORD	MT20
TCCL 7.0	Lumber Increase 1.25	TC 0.35	Vert(TL) -0.00	Structural wood sheathing directly applied or 2'-10-8 oc purlins.	GRIP
TCLL 20.0	Plates Increase 1.25		Defl In (loc) 2-4	Rigid ceiling directly applied or 1'-0-0 oc bracing.	244/190

LUMBER	REACTIONS	FORCES	NOTES
TOP CHORD 2 X 4 SYP No.2	(lb/size) 3=25/Mechanical, 2=27/4/0-3-8, 4=40/Mechanical	(lb) - Maximum Compression/Maximum Tension	(1) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
BOT CHORD 2 X 4 SYP No.2	Max Horz 2=153(load case 5)	TOP CHORD 1-2=0/47, 2-3=56/6	(2) Refer to girder(s) for truss to truss connections.
	Max Uplift 3=31(load case 6), 2=252(load case 5)	BOT CHORD 2-4=0/0	(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3 and 252 lb uplift at joint 2.

Job Reference (optional)

Scale = 1:16.9



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

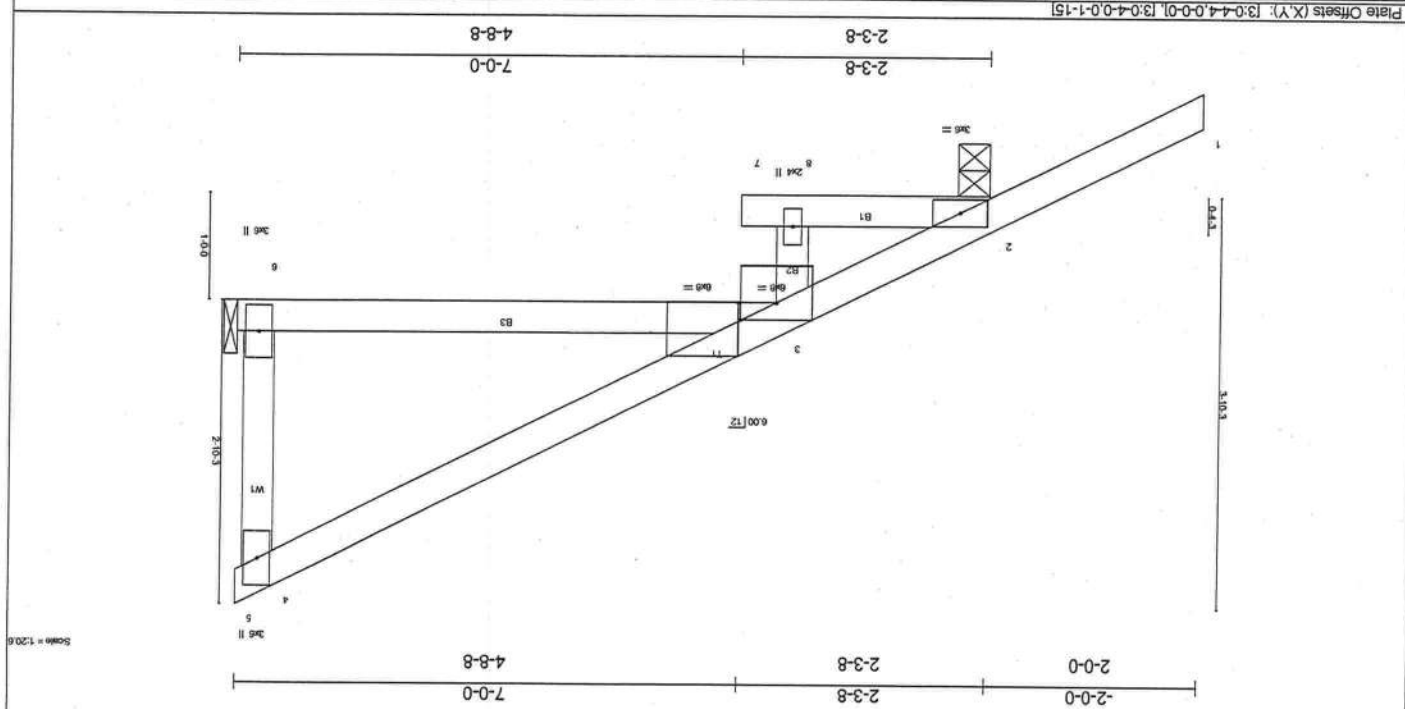
REACTIONS	(lb/size)	(lb/size)	Max Horiz Z=266/ (load case 5)	Max Uplift=164/ (load case 5), Z=263/ (load case 5)	- Maximum Compression/Maximum Tension	TOP CHORD 1-2=0/47, 2-3=-139/58	BOT CHORD 2-4=0/0
FORCES (lb)							

LOAD CASE(S) Standard

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 1 and 263 lb uplift at joint 2.

OCTOBER 21, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Truss	EJ7A	SPECIAL	Qty	6	Ply	1
Job Reference (optional)						



LOADING (psf)		SPACING		CSI		DEFL		BRACING	
TCL	20.0	Plates Increase	1.25	TC	0.59	Vert (LL)	0.12	TOP CHORD	Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BCDL	10.0	Lumber Increase	1.25	BC	0.60	Vert (TL)	-0.15	BOT CHORD	Rigid ceiling directly applied or 10'-0" oc bracing.
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz (TL)	0.07		
BCDL	5.0	Code FBC2004/TFP2002		(Matrix)					

LUMBER		TOP CHORD		BOT CHORD	
2 X 4 SYP No. 2	2 X 4 SYP No. 2	2 X 4 SYP No. 3	2 X 4 SYP No. 3	2 X 4 SYP No. 3	2 X 4 SYP No. 3

REACTIONS (lb/size) 6=266/Mechanical, 2=419/0-3-8
Max Horiz 2=267/load case 5)
Max Uplift 6=164/load case 5), 2=258/load case 5)

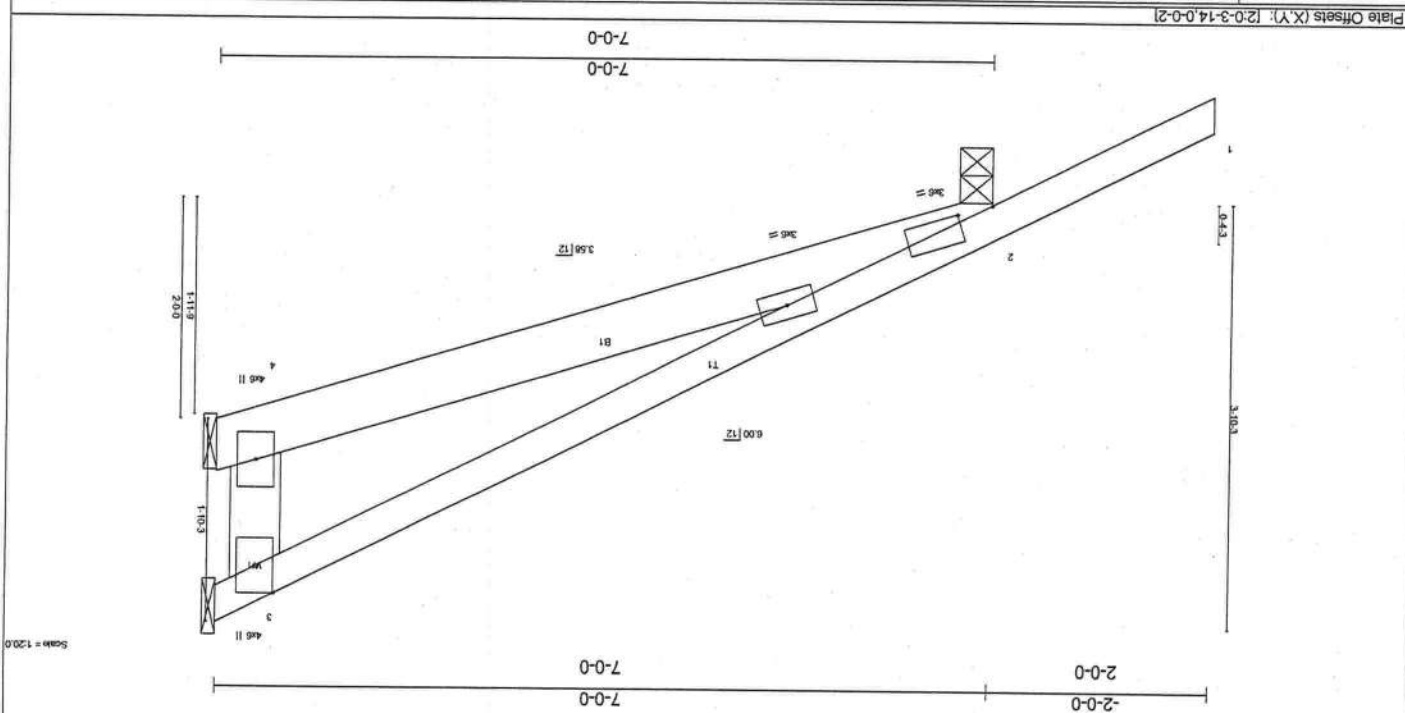
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=311/0, 3-4=159/14, 4-5=1/0, 4-6=-126/186
BOT CHORD 2-5=-92/198, 7-8=0/0, 3-8=-7/85, 3-6=-110/123

NOTES
1) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
Exterior(2) zone: Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 6 and 258 lb uplift at joint 2.

LOAD CASE(S) Standard

Truss	EJTB	MONO SCISSOR	Qty	10	Ply	1
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Builders FirstSource, Lake City, FL 32055
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LOADING (psf)	SPACING	CS	DEFL	BRACING	LUMBER
20.0	2'-0"	0.46	0.13	TOP CHORD	2 X 4 SYP No. 2
7.0			0.12	BOT CHORD	2 X 6 SYP No. 1D
10.0			0.12		
5.0			0.12		

WEBS 2 X 6 SYP No. 1D
 BOT CHORD 2 X 6 SYP No. 1D
 TOP CHORD 2 X 4 SYP No. 2

REACTIONS (lb/size) 2=108/0-3-8, 3=140/Mechanical, 4=114/Mechanical
 Max Horiz 2=317(load case 5)
 Max Uplift 2=351(load case 5), 3=-155(load case 5), 4=-101(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/49, 2-3=-133/48
 BOT CHORD 2-4=-106/27
 WEBS 3-4=0/0

NOTES
 1) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Extent(2) zone: porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
 MWFRS for reactions specified.
 2) Refer to girder(s) for truss connections.
 3) Bearing at joint(s) 2 considers parallel to grain value using ANSITP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 2, 155 lb uplift at joint 3 and 101 lb uplift at joint 4.
 5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

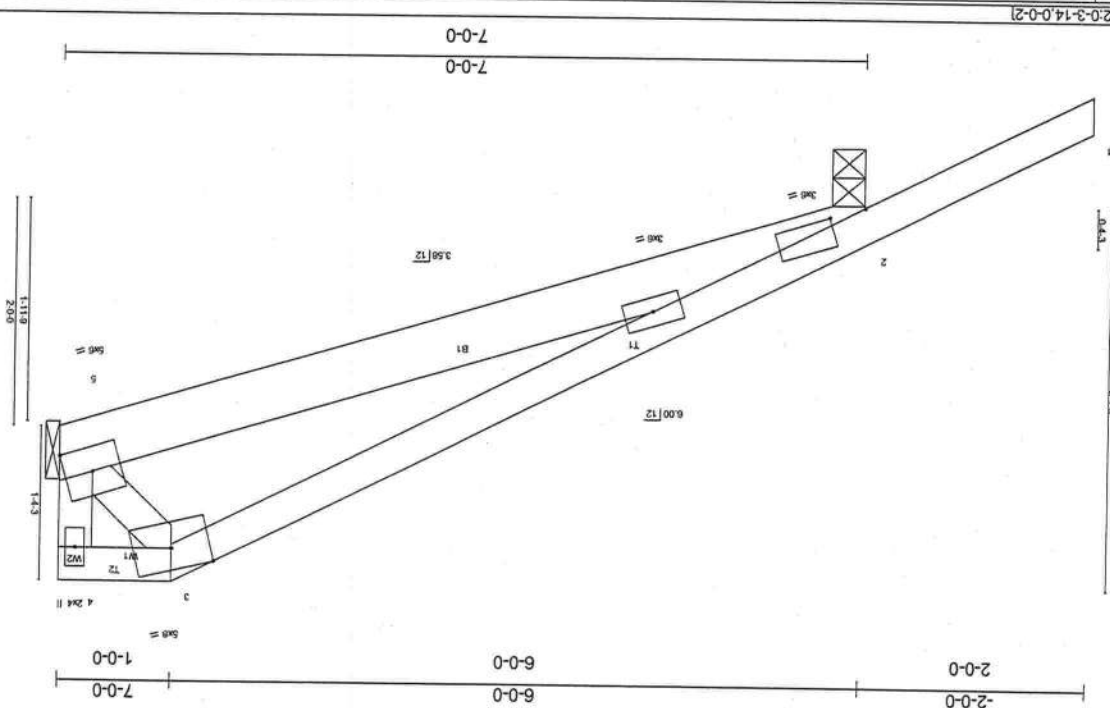
LOAD CASE(S) Standard

Truss	EJ7C	Truss Type	SPECIAL	Qty	2	Ply	1
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

6.200 s Jul 13 2005 Miller Industries, Inc. Thu Oct 13 12:01:31 2005 Page 1

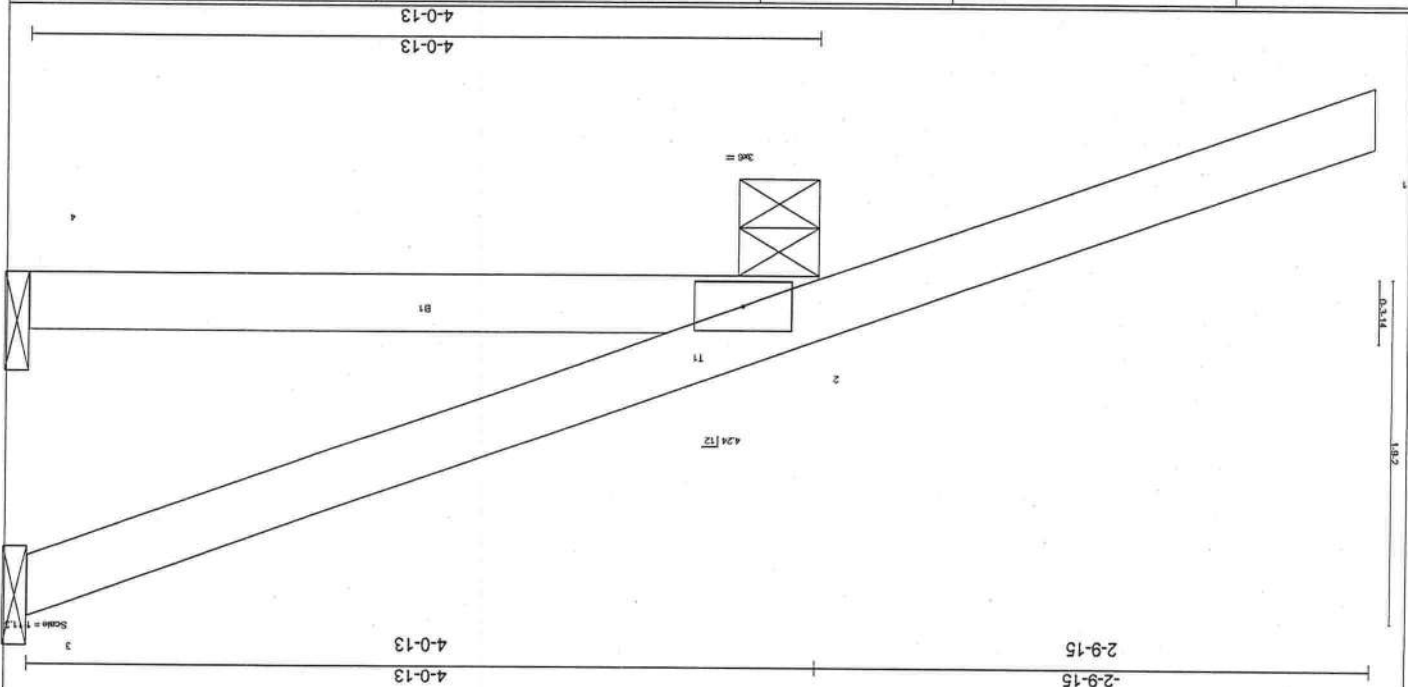


LOADING (psf)	SPACING	CSI	DEFLECTION	BRACING	LUMBER
TCLL 20.0	2-0-0	TC 0.73	in (loc)	TOP CHORD	TOP CHORD 2 X 4 SYP No.2
TCDL 7.0	Lumber Increase 1.25	BC 0.14	Vert(TL) 0.08	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	BOT CHORD 2 X 6 SYP No.1D
BCLL 10.0	Rep Stress Incr YES	WB 0.09	Horiz(TL) -0.00	Rigid ceiling directly applied or 9-4-14 oc bracing.	WEBS 2 X 4 SYP No.3
BCDL 5.0	Code FBC2004/TP2002	(Matrix)			

REACTIONS	FORCES	NOTES
(b) - Maximum Compression/Maximum Tension	(b) - Maximum Compression/Maximum Tension	1) Wind: ASCE 7-02, 120mph (3-second gust): h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
Max Horiz Z=302(load case 5)	Max Uplift=-239(load case 5), Z=-365(load case 5)	MWFRS for reactions specified.
5=263/Mechanical, 2=416/0-3-8		2) Provide adequate drainage to prevent water ponding.
		3) Refer to girder(s) for truss connections.
		4) Bearing at joint(s) 2 considers parallel to grain value parallel to grain value using ANSITP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
		5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 239 lb uplift at joint 5 and 365 lb uplift at joint 2.

OCTOBER 21, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

LOADING	(psf)	TCLL	20.0	TCDL	7.0	BCLL	10.0	BCDL	5.0
SPACING	2-0-0	Plates Increase	1.25	Lumber Increase	1.25	Rep Stress Incr	NO	Code FBC2004/FP12002	
CSI	TC	0.58	WB	0.06	Horz(T/L)	-0.00	i/d defl	n/a	
DEFL	Vert(LL)	-0.01	Vert(UL)	-0.01	Weight	17 lb	M/T20	GRIP	244/190



COMBLOC
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 BRACING
 TOP CHORD
 BOT CHORD
 Structural wood sheathing directly applied or 4-0-13 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS
 (b) Size = 3=9/Mechanical
 Max Horiz = 2=11(1load case 2)
 Max Uprift = 2=31(2load case 2)
 Max GYrf = 2=42(2load case 6), 2=289(load case 1), 4=40(load case 1)
 FORCES
 (b) - Maximum Compression/Maximum Tension
 TOP CHORD
 1-2=0/50, 2-3=37/14
 2-4=0/0
 BOT CHORD

NOTES

(1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

(2) Refer to girder(s) for truss connections.

(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 3 and 312 lb uplift at joint 2.

(4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASES) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=54
Trapezoidal Loads (plf)
Vert: 2=3(F=26, B=26)-to-3=-57(F=2, B=2), 2

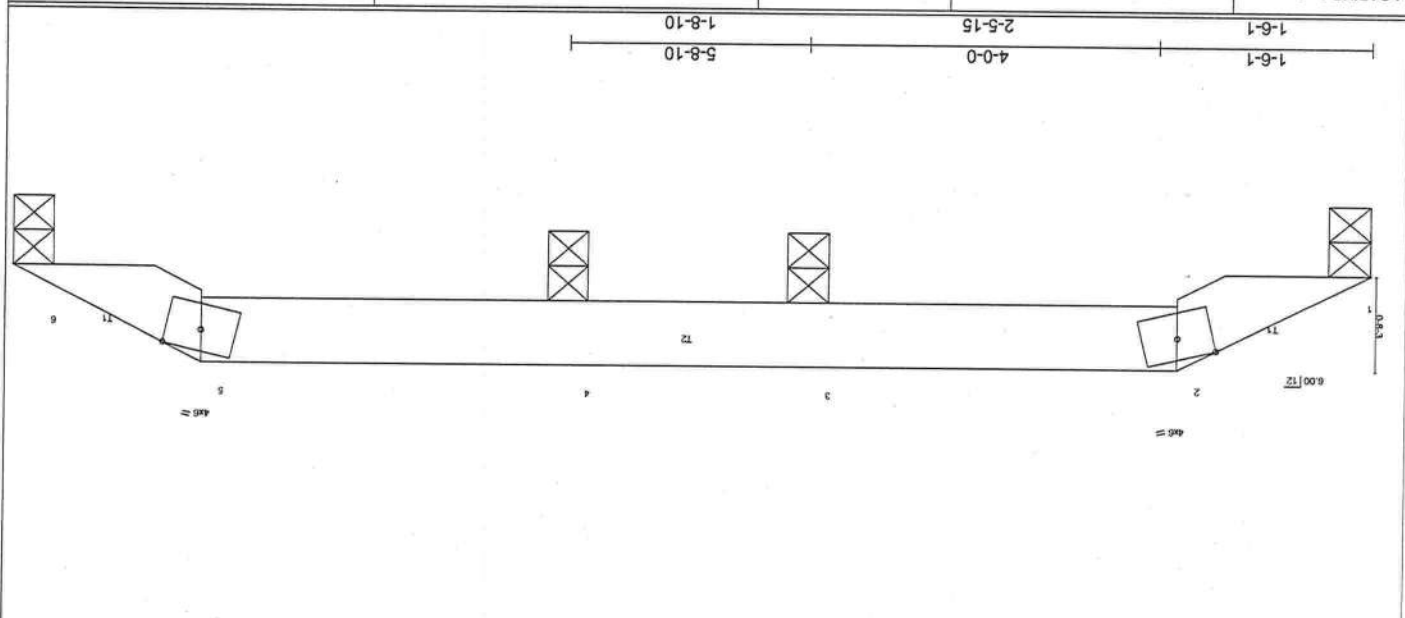
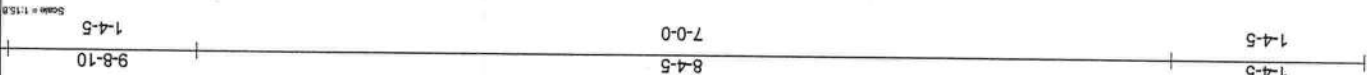
Trapezoidal Loads (plf)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



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Truss	P801	HIP PLIGGYBACK	2	Qty	1	Ply	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055							6,200 s Jul 13 2005 Mittek Industries, Inc. Thu Oct 13 12:01:33 2005 Page 1



LOADING (psf)		SPACING		CSI		DEFLECTION		BRACING		PLATES		Weight: 23 lb	
BCDL	5.0	Code FBC2004/TP12002	2-0-0	TC	0.04	Vert(TL)	0.01	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.	MT20	GRIP	244/190	
BCLL	10.0	Rep Stress Incr	YES	BC	0.00	Horz(TL)	-0.00	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.				
TCCL	20.0	Plates Increase	1.25										
TCCL	7.0	Lumber Increase	1.25										
BCDL	5.0												

REACTIONS (lb/size) 1=87/0-3-8, 6=87/0-3-8, 3=168/0-3-8, 4=168/0-3-8
Max Horiz 1=11 (load case 3)
Max Uplift 1=65 (load case 5), 6=67 (load case 6), 3=143 (load case 4), 4=141 (load case 10)
Max Grav 1=87 (load case 1), 6=87 (load case 9), 4=175 (load case 9), 4=175 (load case 10)

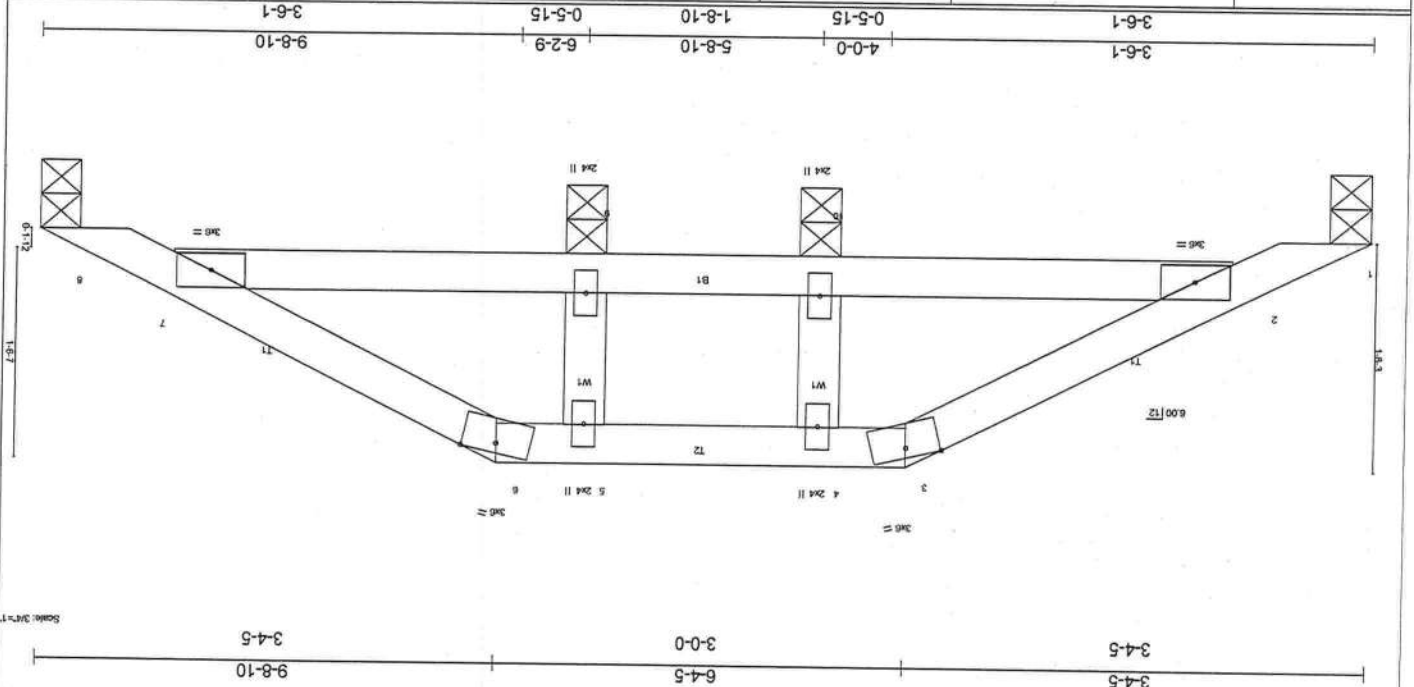
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=37/54, 2-3=0/45, 3-4=0/45, 4-5=0/45, 5-6=37/54
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 120mph (3-second gust): h=20ft: TCCL=4.2psf: BCDL=4.2psf: TCDL=4.2psf: DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSIT/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 1, 67 lb uplift at joint 6, 143 lb uplift at joint 3 and 141 lb uplift at joint 4.

LOAD CASE(S) Standard
3 and 141 lb uplift at joint 4.
3 and 141 lb uplift at joint 4.

Job#	Truss	Truss Type	Qty	Ply
L135117	P802	HIP PLIGGYBACK	2	1

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	BRACING	RECTIONS	FORCES	NOTES
TCLL 20.0	2'-0"	TC 0.14	0.01	TOP CHORD	(lb/size) 1=87/0-3-8, 8=87/0-3-8, 9=293/0-3-8	Max Horiz 1=28(load case 3)	1) Unbalanced roof live loads have been considered for this design.
TCDL 7.0		BC 0.11	-0.02	BOT CHORD	Max Uplift 1=50(load case 5), 8=58(load case 6), 10=133(load case 9), 9=125(load case 10)	Max Grav 1=92(load case 9), 8=92(load case 10), 10=322(load case 9), 9=322(load case 10)	2) Wind: ASCE 7-02: 120mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B: enclosed; MWFRS gable end zone and C-C
BCDL 10.0		WB 0.05	0.01				3) Provide adequate drainage to prevent water ponding.
BCDL 5.0							4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Structural wood sheathing directly applied or 6'-0" oc purlins.
Rigid ceiling directly applied or 6'-0" oc bracing.

TOP CHORD
BOT CHORD

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.3
WEBS

WEBS
BOT CHORD 2-10=103/119, 9-10=103/119, 7-9=103/119
TOP CHORD 1-2=36/47, 2-3=47/138, 3-4=0/103, 4-5=0/103, 5-6=0/103, 6-7=47/138, 7-8=36/40

LOAD CASE(S) Standard
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 120mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B: enclosed; MWFRS gable end zone and C-C
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 58 lb uplift at joint 8, 133 lb uplift at joint 10 and 125 lb uplift at joint 9.

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 120mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
3) Bearing at joint(s) 1, 7 considers parallel to grain value using $ANSI/TP1$ 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1, 29 lb uplift at joint 7, 188 lb uplift at joint 9 and 175 lb uplift at joint 8.

NOTES

- FORCES (lb)
TOP CHORD 1-2=-35/48, 2-3=-196/253, 3-4=-71/164, 4-5=-71/164, 5-6=-196/253, 6-7=-25/19
BOT CHORD 2-9=-183/255, 8-9=-183/255, 6-8=-183/255
WEBS 3-9=-242/252, 5-8=-242/252

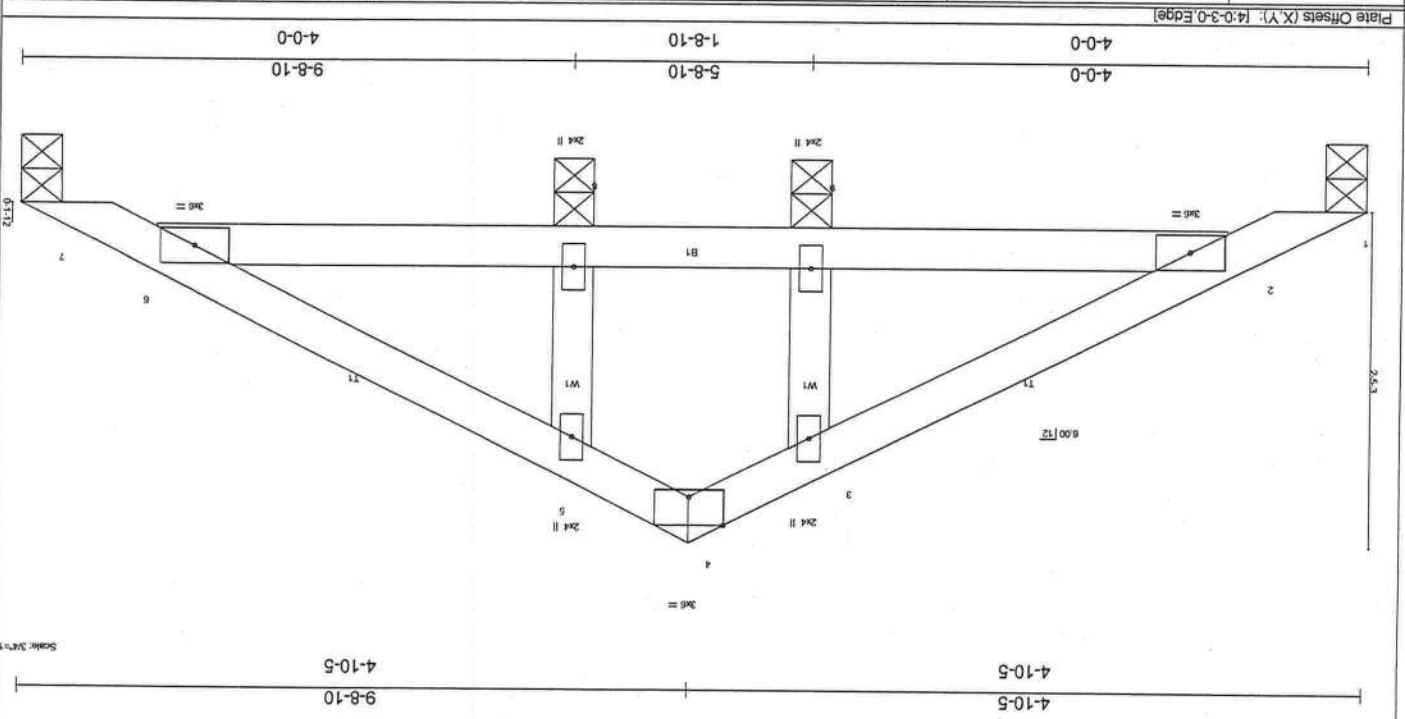
REACTIONS

- (lb/size) 1=50/0-3-8, 7=50/0-3-8, 9=330/0-3-8, 8=330/0-3-8
Max Horiz 1=41(load case 4)
Max Uplift 1=-17(load case 6), 7=-29(load case 6), 9=-188(load case 6), 8=-175(load case 6)
Max Grav 1=66(load case 9), 7=66(load case 10), 9=341(load case 9), 8=341(load case 10)

- LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

- BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling wood sheathing directly applied or 6-0-0 oc bracing.

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	PLATES	GRIP
20.0	2-0-0	TC 0.15	Vert(TL) 0.01	6-8 >999	240	MT20	244/190
7.0	Lumber increase 1.25	BC 0.10	Horz(TL) 0.00	7 n/a	n/a		
10.0	Rep Stress Incr YES	WB 0.06					
5.0	Code FBC2004/TP12002	(Matrix)					



[illegible]

OCTOBER 21, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

[illegible]

LOAD CASE(S) Standard

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-02; 1.20mph (3-second gust); $h=20'$; $TCDL=4.2psf$; $BCDL=3.0psf$; $DOL=1.60$; $TCDL=4.2psf$; $BCDL=3.0psf$; $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Provide adequate drainage to prevent water ponding.
- (4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2, 1394 lb uplift at joint 20 and 741 lb uplift at joint 12.

NOTES

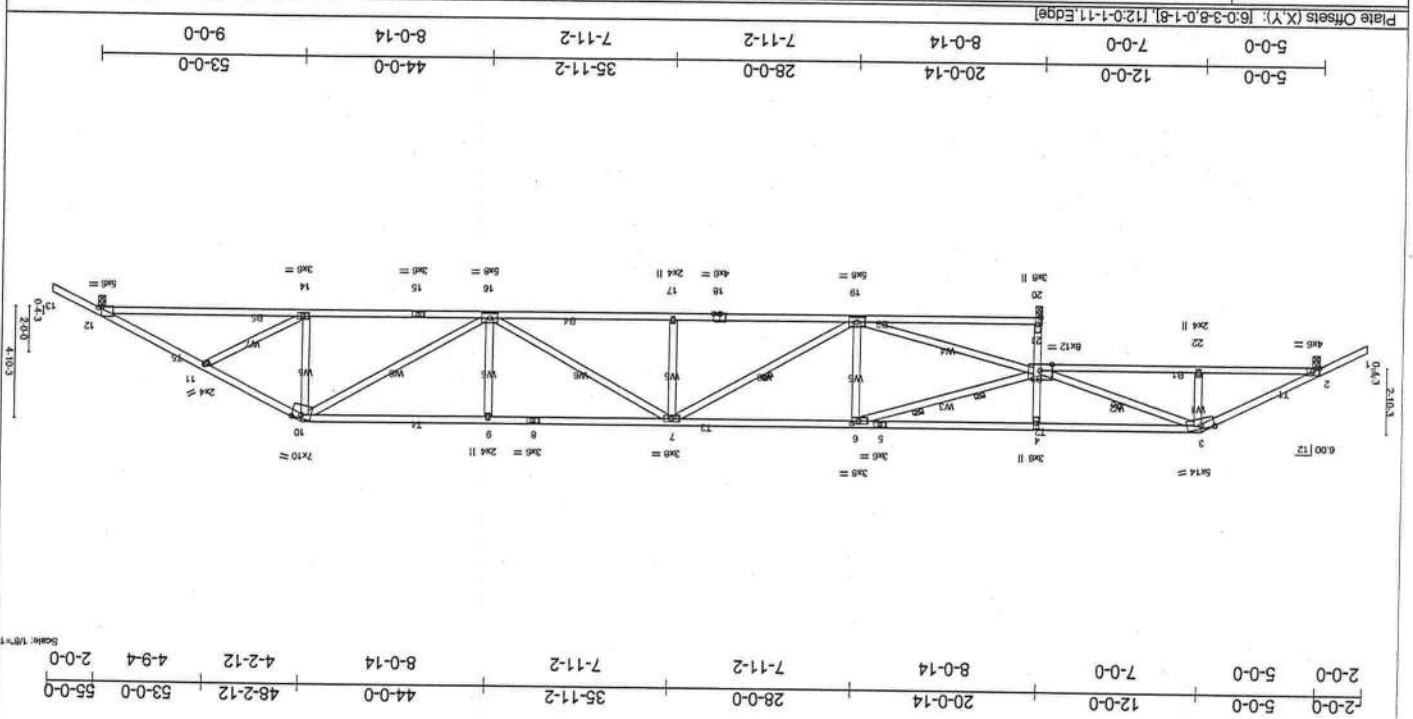
FORCES (lb)	1 - Maximum Compression/Maximum Tension	2 - 20/47, 2.3 = 56/375, 3.4 = 610/1466, 4.5 = 634/1484, 5.6 = 634/1484, 6.7 = 717/1618, 7.8 = 3394/1644, 8.9 = 3394/1644, 10.1 = 7289/1437, 11.1 = 2964/1538, 12.1 = 13 = 47	9 - 0 = 3394/1644, 10.1 = 7289/1437, 11.1 = 2964/1538, 12.1 = 13 = 47	10 - 1 = 2789/1437, 11.1 = 2964/1538, 12.1 = 13 = 47	11 - 2 = 2964/1538, 12.1 = 13 = 47	12 - 2 = 2964/1538, 13.1 = 131/44, 14.1 = 22 = 2538/1678, 15.1 = 22 = 2538/1678, 16.1 = 22 = 2538/1678, 17.1 = 17 = 1352/3135, 18.1 = 19 = 1352/3135, 19.20 = 0/81, 20.1 = 1352/3135, 21.1 = 1352/3135, 22.1 = 1352/3135, 23.1 = 1352/3135, 24.1 = 1352/3135, 25.1 = 1352/3135, 26.1 = 1352/3135, 27.1 = 1352/3135, 28.1 = 1352/3135, 29.1 = 1352/3135, 30.1 = 1352/3135, 31.1 = 1352/3135, 32.1 = 1352/3135, 33.1 = 1352/3135, 34.1 = 1352/3135, 35.1 = 1352/3135, 36.1 = 1352/3135, 37.1 = 1352/3135, 38.1 = 1352/3135, 39.1 = 1352/3135, 40.1 = 1352/3135, 41.1 = 1352/3135, 42.1 = 1352/3135, 43.1 = 1352/3135, 44.1 = 1352/3135, 45.1 = 1352/3135, 46.1 = 1352/3135, 47.1 = 1352/3135, 48.1 = 1352/3135, 49.1 = 1352/3135, 50.1 = 1352/3135, 51.1 = 1352/3135, 52.1 = 1352/3135, 53.1 = 1352/3135, 54.1 = 1352/3135, 55.1 = 1352/3135, 56.1 = 1352/3135, 57.1 = 1352/3135, 58.1 = 1352/3135, 59.1 = 1352/3135, 60.1 = 1352/3135, 61.1 = 1352/3135, 62.1 = 1352/3135, 63.1 = 1352/3135, 64.1 = 1352/3135, 65.1 = 1352/3135, 66.1 = 1352/3135, 67.1 = 1352/3135, 68.1 = 1352/3135, 69.1 = 1352/3135, 70.1 = 1352/3135, 71.1 = 1352/3135, 72.1 = 1352/3135, 73.1 = 1352/3135, 74.1 = 1352/3135, 75.1 = 1352/3135, 76.1 = 1352/3135, 77.1 = 1352/3135, 78.1 = 1352/3135, 79.1 = 1352/3135, 80.1 = 1352/3135, 81.1 = 1352/3135, 82.1 = 1352/3135, 83.1 = 1352/3135, 84.1 = 1352/3135, 85.1 = 1352/3135, 86.1 = 1352/3135, 87.1 = 1352/3135, 88.1 = 1352/3135, 89.1 = 1352/3135, 90.1 = 1352/3135, 91.1 = 1352/3135, 92.1 = 1352/3135, 93.1 = 1352/3135, 94.1 = 1352/3135, 95.1 = 1352/3135, 96.1 = 1352/3135, 97.1 = 1352/3135, 98.1 = 1352/3135, 99.1 = 1352/3135, 100.1 = 1352/3135, 101.1 = 1352/3135, 102.1 = 1352/3135, 103.1 = 1352/3135, 104.1 = 1352/3135, 105.1 = 1352/3135, 106.1 = 1352/3135, 107.1 = 1352/3135, 108.1 = 1352/3135, 109.1 = 1352/3135, 110.1 = 1352/3135, 111.1 = 1352/3135, 112.1 = 1352/3135, 113.1 = 1352/3135, 114.1 = 1352/3135, 115.1 = 1352/3135, 116.1 = 1352/3135, 117.1 = 1352/3135, 118.1 = 1352/3135, 119.1 = 1352/3135, 120.1 = 1352/3135, 121.1 = 1352/3135, 122.1 = 1352/3135, 123.1 = 1352/3135, 124.1 = 1352/3135, 125.1 = 1352/3135, 126.1 = 1352/3135, 127.1 = 1352/3135, 128.1 = 1352/3135, 129.1 = 1352/3135, 130.1 = 1352/3135, 131.1 = 1352/3135, 132.1 = 1352/3135, 133.1 = 1352/3135, 134.1 = 1352/3135, 135.1 = 1352/3135, 136.1 = 1352/3135, 137.1 = 1352/3135, 138.1 = 1352/3135, 139.1 = 1352/3135, 140.1 = 1352/3135, 141.1 = 1352/3135, 142.1 = 1352/3135, 143.1 = 1352/3135, 144.1 = 1352/3135, 145.1 = 1352/3135, 146.1 = 1352/3135, 147.1 = 1352/3135, 148.1 = 1352/3135, 149.1 = 1352/3135, 150.1 = 1352/3135, 151.1 = 1352/3135, 152.1 = 1352/3135, 153.1 = 1352/3135, 154.1 = 1352/3135, 155.1 = 1352/3135, 156.1 = 1352/3135, 157.1 = 1352/3135, 158.1 = 1352/3135, 159.1 = 1352/3135, 160.1 = 1352/3135, 161.1 = 1352/3135, 162.1 = 1352/3135, 163.1 = 1352/3135, 164.1 = 1352/3135, 165.1 = 1352/3135, 166.1 = 1352/3135, 167.1 = 1352/3135, 168.1 = 1352/3135, 169.1 = 1352/3135, 170.1 = 1352/3135, 171.1 = 1352/3135, 172.1 = 1352/3135, 173.1 = 1352/3135, 174.1 = 1352/3135, 175.1 = 1352/3135, 176.1 = 1352/3135, 177.1 = 1352/3135, 178.1 = 1352/3135, 179.1 = 1352/3135, 180.1 = 1352/3135, 181.1 = 1352/3135, 182.1 = 1352/3135, 183.1 = 1352/3135, 184.1 = 1352/3135, 185.1 = 1352/3135, 186.1 = 1352/3135, 187.1 = 1352/3135, 188.1 = 1352/3135, 189.1 = 1352/3135, 190.1 = 1352/3135, 191.1 = 1352/3135, 192.1 = 1352/3135, 193.1 = 1352/3135, 194.1 = 1352/3135, 195.1 = 1352/3135, 196.1 = 1352/3135, 197.1 = 1352/3135, 198.1 = 1352/3135, 199.1 = 1352/3135, 200.1 = 1352/3135, 201.1 = 1352/3135, 202.1 = 1352/3135, 203.1 = 1352/3135, 204.1 = 1352/3135, 205.1 = 1352/3135, 206.1 = 1352/3135, 207.1 = 1352/3135, 208.1 = 1352/3135, 209.1 = 1352/3135, 210.1 = 1352/3135, 211.1 = 1352/3135, 212.1 = 1352/3135, 213.1 = 1352/3135, 214.1 = 1352/3135, 215.1 = 1352/3135, 216.1 = 1352/3135, 217.1 = 1352/3135, 218.1 = 1352/3135, 219.1 = 1352/3135, 220.1 = 1352/3135, 221.1 = 1352/3135, 222.1 = 1352/3135, 223.1 = 13
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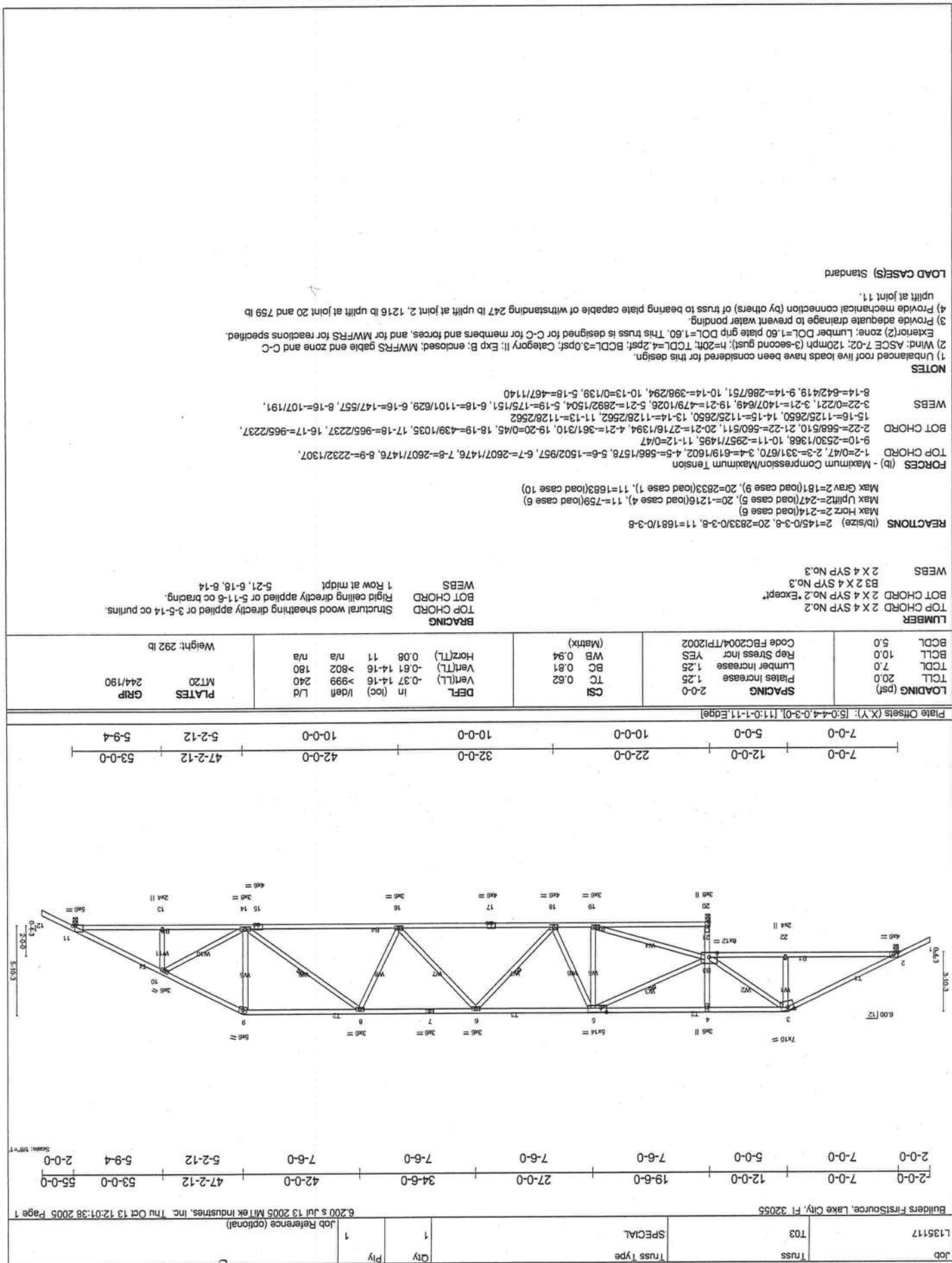
REACTIONS

Max Horiz = 1.97 (load case 6)
Max UprH = 4.39 (load case 5), 20 = 1.394 (load case 4), 12 = 1.741 (load case 6)
Max Gray 2 = 300 (load case 9), 20 = 2.653 (load case 1), 12 = 1.723 (load case 10)

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS
2 X 4 SYP No.3

BRACING	TOP CHORD	WEBS
Rigid ceiling directly applied or 5-5-10 cc bracing.	1 Row at midpt	2 Rows at 1/3 pts
Structural wood sheathing directly applied or 2-10-0 cc purlins.	3-21, 7-19	6-21





[illegible]

Structural wood sheathing directly applied or 3-5-8 oc purlins,
Rigid ceiling directly applied or 6-0-0 oc bracing,
1 Row at midpt 6-22, 8-20, 9-16

Max Horiz Z = -230 (load case 6)
Max Uplift Z = 231 (load case 5), 21 = 1170 (load case 4), 13 = 776 (load case 6)
Max Grav Z = 150 (load case 9), 21 = 2898 (load case 1), 13 = 1674 (load case 10)

[illegible]

LOAD CASE(S) Standard

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-02, 120mph (3-second gust), $h=20ft$; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Provide adequate drainage to prevent water ponding.
- (4) All plates are 3x6 M720 unless otherwise indicated.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 2, 1170 lb uplift at joint 21 and 776 lb uplift at joint 13.

LOAD CASE(S) Standard

[illegible]

REACTIONS (lb/size) 2=56/0-3-8, 21=2955/0-3-8, 13=1649/0-3-8

FORCES (b) - MAXIMUM Compression/Maximum Tension	TOP CHORD	BOT CHORD	WEBS
1-2=0/47, 2-3=938/20, 3-4=-533/21256, 4-5=-492/1232, 5-6=-470/1203, 7-8=-503/437, 8-9=-1743/1059, 9-10=-184/1159, 10-11=-2051/1168, 11-12=-2714/1161, 12-13=-2848/1430, 13-14=0/47	2-23=2707/373, 2-24=2707/373, 2-25=2707/373, 2-26=2707/373, 2-27=2707/373, 2-28=2707/373, 2-29=2707/373, 2-30=2707/373, 2-31=2707/373, 2-32=2707/373, 2-33=2707/373, 2-34=2707/373, 2-35=2707/373, 2-36=2707/373, 2-37=2707/373, 2-38=2707/373, 2-39=2707/373, 2-40=2707/373, 2-41=2707/373, 2-42=2707/373, 2-43=2707/373, 2-44=2707/373, 2-45=2707/373, 2-46=2707/373, 2-47=2707/373, 2-48=2707/373, 2-49=2707/373, 2-50=2707/373, 2-51=2707/373, 2-52=2707/373, 2-53=2707/373, 2-54=2707/373, 2-55=2707/373, 2-56=2707/373, 2-57=2707/373, 2-58=2707/373, 2-59=2707/373, 2-60=2707/373, 2-61=2707/373, 2-62=2707/373, 2-63=2707/373, 2-64=2707/373, 2-65=2707/373, 2-66=2707/373, 2-67=2707/373, 2-68=2707/373, 2-69=2707/373, 2-70=2707/373, 2-71=2707/373, 2-72=2707/373, 2-73=2707/373, 2-74=2707/373, 2-75=2707/373, 2-76=2707/373, 2-77=2707/373, 2-78=2707/373, 2-79=2707/373, 2-80=2707/373, 2-81=2707/373, 2-82=2707/373, 2-83=2707/373, 2-84=2707/373, 2-85=2707/373, 2-86=2707/373, 2-87=2707/373, 2-88=2707/373, 2-89=2707/373, 2-90=2707/373, 2-91=2707/373, 2-92=2707/373, 2-93=2707/373, 2-94=2707/373, 2-95=2707/373, 2-96=2707/373, 2-97=2707/373, 2-98=2707/373, 2-99=2707/373, 2-100=2707/373, 2-101=2707/373, 2-102=2707/373, 2-103=2707/373, 2-104=2707/373, 2-105=2707/373, 2-106=2707/373, 2-107=2707/373, 2-108=2707/373, 2-109=2707/373, 2-110=2707/373, 2-111=2707/373, 2-112=2707/373, 2-113=2707/373, 2-114=2707/373, 2-115=2707/373, 2-116=2707/373, 2-117=2707/373, 2-118=2707/373, 2-119=2707/373, 2-120=2707/373, 2-121=2707/373, 2-122=2707/373, 2-123=2707/373, 2-124=2707/373, 2-125=2707/373, 2-126=2707/373, 2-127=2707/373, 2-128=2707/373, 2-129=2707/373, 2-130=2707/373, 2-131=2707/373, 2-132=2707/373, 2-133=2707/373, 2-134=2707/373, 2-135=2707/373, 2-136=2707/373, 2-137=2707/373, 2-138=2707/373, 2-139=2707/373, 2-140=2707/373, 2-141=2707/373, 2-142=2707/373, 2-143=2707/373, 2-144=2707/373, 2-145=2707/373, 2-146=2707/373, 2-147=2707/373, 2-148=2707/373, 2-149=2707/373, 2-150=2707/373, 2-151=2707/373, 2-152=2707/373, 2-153=2707/373, 2-154=2707/373, 2-155=2707/373, 2-156=2707/373, 2-157=2707/373, 2-158=2707/373, 2-159=2707/373, 2-160=2707/373, 2-161=2707/373, 2-162=2707/373, 2-163=2707/373, 2-164=2707/373, 2-165=2707/373, 2-166=2707/373, 2-167=2707/373, 2-168=2707/373, 2-169=2707/373, 2-170=2707/373, 2-171=2707/373, 2-172=2707/373, 2-173=2707/373, 2-174=2707/373, 2-175=2707/373, 2-176=2707/373, 2-177=2707/373, 2-178=2707/373, 2-179=2707/373, 2-180=2707/373, 2-181=2707/373, 2-182=2707/373, 2-183=2707/373, 2-184=2707/373, 2-185=2707/373, 2-186=2707/373, 2-187=2707/373, 2-188=2707/373, 2-189=2707/373, 2-190=2707/373, 2-191=2707/373, 2-192=2707/373, 2-193=2707/373, 2-194=2707/373, 2-195=2707/373, 2-196=2707/373, 2-197=2707/373, 2-198=2707/373, 2-199=2707/373, 2-200=2707/373, 2-201=2707/373, 2-202=2707/373, 2-203=2707/373, 2-204=2707/373, 2-205=2707/373, 2-206=2707/373, 2-207=2707/373, 2-208=2707/373, 2-209=2707/373, 2-210=2707/373, 2-211=2707/373, 2-212=2707/373, 2-213=2707/373, 2-214=2707/373, 2-215=2707/373, 2-216=2707/373, 2-217=2707/373, 2-218=2707/373, 2-219=2707/373, 2-220=2707/373, 2-221=2707/373, 2-222=2707/373, 2-223=2707/373, 2-224=2707/373, 2-225=2707/373, 2-226=2707/373, 2-227=2707/373, 2-228=2707/373, 2-229=2707/373, 2-230=2707/373, 2-231=2707/373, 2-232=2707/373, 2-233=2707/373, 2-234=2707/373, 2-235=2707/373, 2-236=2707/373, 2-237=2707/373, 2-238=2707/373, 2-239=2707/373, 2-240=2707/373, 2-241=2707/373, 2-242=2707/373, 2-243=2707/373, 2-244=2707/373, 2-245=2707/373, 2-246=2707/373, 2-247=2707/373, 2-248=2707/373, 2-249=2707/373, 2-250=2707/373, 2-251=2707/373, 2-252=2707/373, 2-253=2707/373, 2-254=2707/373, 2-255=2707/373, 2-256=2707/373, 2-257=2707/373, 2-258=2707/373, 2-259=2707/373, 2-260=2707/373, 2-261=2707/373, 2-262=2707/373, 2-263=2707/373, 2-264=2707/373, 2-265=2707/373, 2-266=2707/373, 2-267=2707/373, 2-268		

LOAD CASE(S) Standard

LUMBER	TOP CHORD	2 X 4 SYP No.2
	BOT CHORD	2 X 4 SYP No.2
WEBS		2 X 4 SYP No.3
	Right	2 X 4 SYP No.3 1'-10-6
BRACING	TOP CHORD	
	BOT CHORD	
WEBS		
	1 Row at midpt	5-17, 6-17, 6-15
	Rigid ceiling directly applied or 6-0-0 oc bracing.	
	Structural wood sheathing directly applied or 5-9-13 oc purlins.	

REACTIONS	TOP CHORD	BOT CHORD	WEBS
(lb/size)	17=2364/0-3-8, 12=1048/0-3-8, 2=742/0-3-8		
Max Horiz 2=183/ (load case 5)			
Max Uplift 17=-1279/ (load case 4), 12=475/ (load case 6), 2=711/ (load case 5)			
Max Gray 17=2364/ (load case 1), 12=1080/ (load case 10), 2=764/ (load case 9)			
FORCES (lb) - Maximum Compression/Maximum Tension			
	1-2=0/47, 2-3=-943/1067, 3-4=-796/1082, 4-5=-334/532, 5-6=-315/568, 6-7=-567/324, 7-8=-567/324, 8-9=-861/534, 9-10=-1020/523,	10-11=-212/74	12-13=-355/1042,
		2-20=-848/768, 19-20=-848/768, 18-19=-848/768, 16-17=-215/243, 15-16=-658/712, 14-15=-104/567, 13-14=-355/1042,	4-20=-278/214, 4-18=-610/778, 5-18=-678/482, 5-17=-1138/1181, 6-17=-1374/882, 6-15=-798/1479, 8-15=-716/586, 8-14=-313/437,
			9-14=0/94, 10-14=-237/266, 10-13=0/140, 10-12=-1161/558

LOAD CASES) Standard

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 120 mph (3-second gust); $z=20$ ft; TCEDL=4 gpsf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior (2) zone; cantilever right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) All plates are 3x5 h/20 unless otherwise indicated.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1279 lb uplift at joint 17, 475 lb uplift at joint 12 and 711 lb uplift at joint 2.

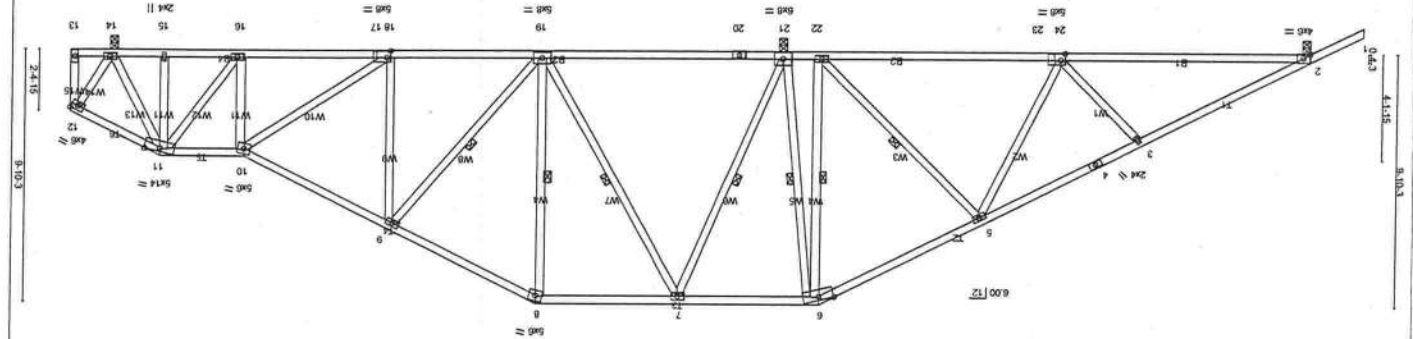
LOAD CASE(S) Standard

Dwg. #1021051121

Job	Truss	Truss Type	SPECIAL	Qty	1	Ply	1
Job	L135117	Truss	T09	1	Truss	1	Truss

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2-0-0	6-8-5	12-10-3	6-1-13	19-0-0	24-6-0	30-0-0	35-8-4	41-4-8	44-8-8	48-2-8	3-6-0
2-0-0	6-8-5	12-10-3	6-1-13	19-0-0	24-6-0	30-0-0	35-8-4	41-4-8	44-8-8	48-2-8	3-6-0



LOADING (psf)	20.0	7.0	10.0	5.0
SPACING	2-0-0	1-25	1-25	1-25
Code	FBC2004/TP12002	Rep Stress Incr	YES	YES
CSL	0.37	TC	0.48	WB
DEFL	in (loc)	0.39	2-24	>777
BRACING	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	5-22, 6-22, 6-21, 7-21, 7-19, 8-19, 9-19	
WEBS	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	5-22, 6-22, 6-21, 7-21, 7-19, 8-19, 9-19	
WEBS	1 Row at midpt			
Weight	317 lb			
PLATES	MT20	244/190	GRP	

REACTIONS	(lb/size) 21=251/10-3-8, 14=978/0-3-8, 2=652/0-3-8
FORCES	(b) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/47, 2-3=-496/640, 3-4=-496/640, 4-5=-496/640, 5-6=-275/631, 6-7=-342/655, 7-8=-405/268, 8-9=-525/230, 9-10=-931/354
BOT CHORD	10-11=-947/404, 11-12=-976, 12-13=-34/8
WEBS	9-19=-576/474, 9-18=-105/300, 10-18=-253/235, 10-16=-506/258, 11-16=-291/859, 11-15=-0/42, 11-14=-977/388, 12-14=-95/149

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 120mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 Extent(2) zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and
 forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are 3x6 MT20 unless otherwise indicated.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1365 lb uplift at joint 21, 454 lb uplift at joint 14 and 628 lb
 uplift at joint 2.
 LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
L135117	T10	SPECIAL	1		6.200 S Jul 13 2005 Milltek Industries, Inc. Thu Oct 13 12:01:45 2005 Page 1

Builders FirstSource, Lake City, FL 32055

Scale = 1/8" = 1'-0"

LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2004/TP12002	CS1	DEFL	BRACING
20.0	2-0	1.25	1.25	YES	YES	0.46	0.43	TOP CHORD
7.0						0.59	0.34	BOT CHORD
10.0						0.84	0.03	WEBS
BCDL	5.0							

PLATES	MT20	GRP
244/190		

Weight: 339 lb

1 Row at midpt

Rigid ceiling directly applied or 5-4-3 oc bracing

Structural wood sheathing directly applied or 4-4-1 oc purlins

REACTONS (lb/size) 22=3008/0-3-9 (0-3-8 + bearing block), 13=1130/0-3-8, 2=450/0-3-8

Max Horiz 2=210/(load case 5)

Max Uplift 22=1473/(load case 5), 13=617/(load case 6), 2=605/(load case 9)

Max Grav 22=3008/(load case 1), 13=1181/(load case 10), 2=606/(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 10-11=1378/682, 11-12=1541/677, 12-13=1821/838, 13-14=0/47

2-26=469/413, 25-26=539/389, 23-24=914/951, 21-22=427/734, 20-21=427/734, 19-20=427/734

BOT CHORD 18-19=292/776, 17-18=324/1378, 16-17=302/1333, 15-16=302/1333, 13-15=553/1585

WEBS 3-26=368/446, 5-26=870/664, 6-24=1034/432, 7-22=1517/877, 7-19=674/1277, 8-19=926/639, 9-18=343/734, 10-18=840/517, 10-17=15/83, 11-17=45/81, 11-15=77/392, 12-15=300/306, 6-22=1189/1522

NOTES

(1) 2 X 4 SYP No. 2 bearing block 12" long at Jt. 22 attached to front face with 2 rows of 0.131" X 3" Nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.

(2) Unbalanced roof live loads have been considered for this design.

(3) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(4) Provide adequate drainage to prevent water ponding.

(5) All plates are 3x6 MT20 unless otherwise indicated.

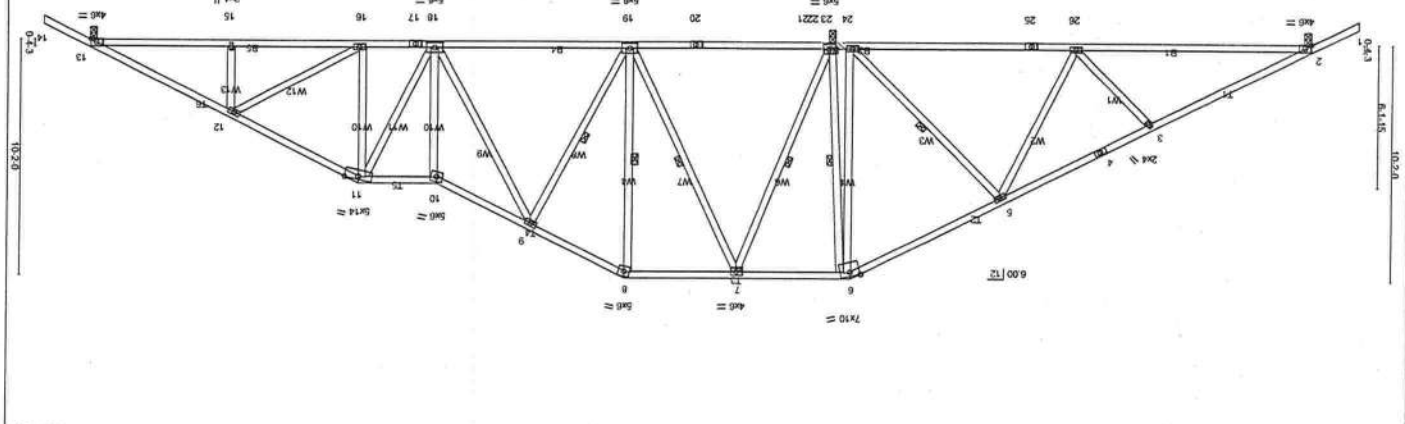
(6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1473 lb uplift at joint 22, 617 lb uplift at joint 13 and 605 lb uplift at joint 2.

LOAD CASE(S) Standard

Dwg. #1021051123

Job	Truss	Truss Type	SPECIAL	Qty	1	Ply	1
L135117	T11						
Builders FirstSource, Lake City, FL 32055							
6/2005 JUN 13 2005 Miller Industries, Inc. Thu Oct 13 12:01:47 2005 Page 1							
Job Reference (optional)							

2-0-0	6-10-14	13-3-5	19-7-11	24-6-0	29-4-5	33-4-7	37-4-8	40-8-8	46-3-0	52-4-0	54-4-0
2-0-0	6-10-14	13-3-5	19-7-11	24-6-0	29-4-5	33-4-7	37-4-8	40-8-8	46-3-0	52-4-0	54-4-0



LOADING (psf)	SPACING	CS	DEF	BRACING	TOP CHORD	BOT CHORD	WEBS	PLATES	GRP	Weight
BCDL 5.0	Code FBC2004/TP12002	WB 0.87	Horz(TL) 0.03	Structural wood sheathing directly applied or 4-5-0 oc purlins	1 Row at midpt	5-24, 7-19, 8-19, 9-19, 6-22, 7-22	5-24, 7-19, 8-19, 9-19, 6-22, 7-22	MT20	244/190	340 lb
BCLL 10.0	Rep Stress Incr YES	BC 0.48	Vert(TL) -0.34							
TCCL 7.0	Lumber Increase 1.25	TC 0.46	DEF -0.20							
TCL 20.0	Plates Increase 1.25									
2-0-0	2-0-0									

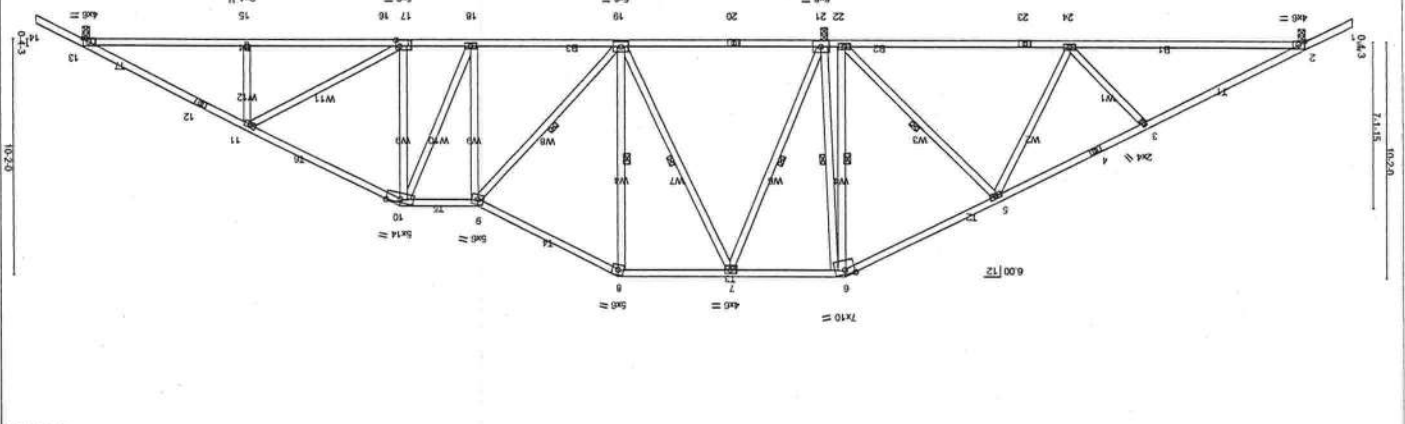
REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS	NOTES
(lb/size) 13=1120/0-3-8, 22=3012/0-3-9 (0-3-8 + bearing block), 2=471/0-3-8	Max Horiz 2=210(load case 5) Max Uplift 13=647(load case 6), 22=1085(load case 5) Max Grav 13=1171(load case 10), 22=3012(load case 9), 2=612(load case 9)	12=0/47, 23=529/314, 34=276/354, 45=135/391, 56=292/1093, 67=218/1021, 78=223/360, 89=301/345, 910=1290/855, 1011=1101/693, 1112=1322/738, 1213=1863/896, 1314=0/47, 1415=248/425, 1516=534/481, 1617=287/1128, 1718=287/1128, 1819=616/17, 1920=453/601, 2122=453/601, 2223=901/845, 2324=901/845, 2425=595/1592, 2526=595/1592, 2627=179/74, 2728=839/630, 2829=628/1073, 2930=368/404, 3132=111/87, 3233=156/351, 3435=541/355, 3637=12-15=0/196, 3839=1295/698, 4042=1532/633	10-18=-710/517, 11-18=-111/87, 12-18=-156/351, 13-18=-541/355, 14-18=-12-15=0/196, 15-18=-1295/698, 16-18=-1532/633, 17-18=-710/517, 18-18=-111/87, 19-18=-156/351, 20-18=-541/355, 21-18=-12-15=0/196, 22-18=-1295/698, 23-18=-1532/633, 24-18=-710/517, 25-18=-111/87, 26-18=-156/351, 27-18=-541/355, 28-18=-12-15=0/196, 29-18=-1295/698, 30-18=-1532/633	10-18=-710/517, 11-18=-111/87, 12-18=-156/351, 13-18=-541/355, 14-18=-12-15=0/196, 15-18=-1295/698, 16-18=-1532/633, 17-18=-710/517, 18-18=-111/87, 19-18=-156/351, 20-18=-541/355, 21-18=-12-15=0/196, 22-18=-1295/698, 23-18=-1532/633, 24-18=-710/517, 25-18=-111/87, 26-18=-156/351, 27-18=-541/355, 28-18=-12-15=0/196, 29-18=-1295/698, 30-18=-1532/633	1) 2 X 4 SYP No. 2 bearing block 12" long at JL 22 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP. 2) Unbalanced roof live loads have been considered for this design. 3) Wind: ASCE 7-02, 120mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 4) Provide adequate drainage to prevent water ponding. 5) All plates are 3x6 MT20 unless otherwise indicated. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 647 lb uplift at joint 13, 1085 lb uplift at joint 22 and 358 lb uplift at joint 2.

LOAD CASE(S)	Standard
uplift at joint 2.	

Dwg #1021051124

Job	L135117	Truss	T12	32055	Builders FirstSource, Lake City, FL
Truss Type	SPECIAL	Qty	1	Job Reference (optional)	
Ply	1				

2'-0"	6'-10-14	13'-3-5	19'-7-11	24'-6-0	29'-4-5	35'-4-8	38'-8-8	45'-3-0	54'-4-0	2'-0"
2'-0"	6'-10-14	13'-3-5	19'-7-11	24'-6-0	29'-4-5	35'-4-8	38'-8-8	45'-3-0	54'-4-0	2'-0"

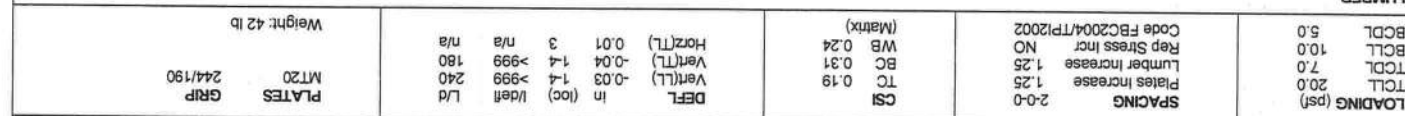


10'-1-1	19'-7-11	20'-5-12	8'-10-9	35'-4-8	38'-8-8	45'-3-0	54'-4-0	7'-1-0
10'-1-1	19'-7-11	20'-5-12	8'-10-9	35'-4-8	38'-8-8	45'-3-0	54'-4-0	7'-1-0

LOADING (psf)	SPACING	Code FBC2004/TP1202	Rep Stress Incr	Plates Increase	TC	BC	WB	CSI (Matrx)	DEFL	in (loc)	l/dell	L/d	MT20	GRIP
20.0	2.0-0	1.25	1.25	1.25	0.45	0.50	0.84	0.50	0.35	2.24	>564	240	244/190	244/190
TCLL									Vert(TL)					
TCDL									Horz(TL)					
BCLL														
BCDL														

LUMBER	TOP CHORD 2 X 4 SYP No.2	BRACING	TOP CHORD	WEBS	1 Row at midpt	5-22, 6-22, 7-19, 8-19, 9-19, 6-21, 7-21
BOT CHORD	2 X 4 SYP No.2		BOT CHORD			
WEBS	2 X 4 SYP No.3					

REACTIONS	(lb/size) 21=2948/0-3-8, 13=1146/0-3-8, 2=510/0-3-8	Max Horiz 2=210(load case 5) Max Uplift1=1452(load case 5), 13=628(load case 6), 2=624(load case 5) Max Grav 21=2948(load case 1), 13=1198(load case 10), 2=632(load case 9)	FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD 1-2=0/47, 2-3=572/27, 3-4=319/601, 4-5=179/623, 5-6=353/1006, 6-7=336/938, 7-8=287/298, 8-9=394/252, 9-10=931/529, 10-11=1220/583, 11-12=1718/801, 12-13=1872/786, 13-14=0/47	BOT CHORD 2-24=506/463, 23-24=455/336, 22-23=455/336, 21-22=823/886, 20-21=385/708, 19-20=385/708, 18-19=74/924, 17-18=137/1025, 16-17=137/1025, 15-16=496/1593, 13-15=496/1593, 6-22=-1060/519, 7-19=684/1277, 8-19=950/640, 9-18=157/374, 10-18=277/186, 11-16=183/451, 11-16=659/455, 11-15=0/231, 6-21=-1242/1523, 7-21=-1493/859	WEBS	NOTES 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B: enclosed; MWFRS gable end zone and C-C reactions specified. 3) Provide adequate drainage to prevent water ponding. 4) All plates are 3x6 MT20 unless otherwise indicated. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1452 lb uplift at joint 21, 628 lb uplift at joint 13 and 624 lb uplift at joint 2.
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LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3	BRACING TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 5-8-3 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS

(lb/inch) 1.563/0-3.8, 3.863/0-3.8
Max Horiz 1.44 (load case 3)
Max Uplift=1.387 (load case 4), 3.387 (load case 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1.42=1183/524, 2.33=1183/524
BOT CHORD 1.44=420/012, 3.44=420/012
WEBS 2.44=266/37

NOTES

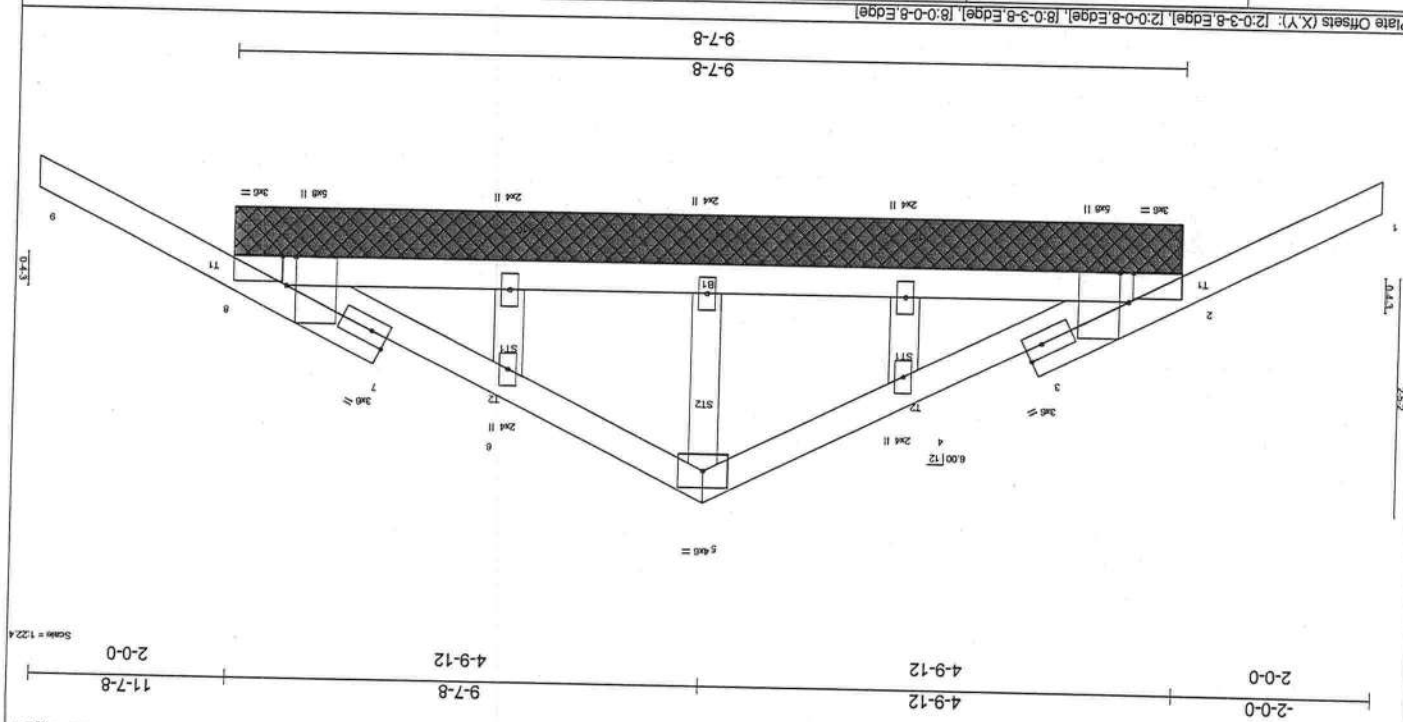
1) Unbraced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120 mph (3-second gust); n=20ft; TCDL=4.2psf.
DOL=1.60 plate dnp DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate
4) Girders carries full span(s): 7.0-0 from 0.0-0 to 9.7-8
5) In the LOAD CASE(S) section, loads applied to the face of the truss

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Load (psf)
Vert: 1-2=-54, 2-3=-54, 1-3=-129 (F=-99)

OCTOBER 21, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Job	Truss Type	Qty	Plg
L135117	COMMON	1	1
Builders FirstSource, Lake City, FL 32055			
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LOADING (psf)		SPACING		2-0-0		CS		DEFL		in (loc)		l/defl		L/4		MT20		GRIP		244/190		Weight: 46 lb	
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(TL)	-0.03	9	n/r	120	n/a	90	n/a	8	0.00	Horz(TL)	0.04	WB	0.04	(Matrix)			
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.05	9	n/r	90	n/a	90	n/a	8	0.00	Horz(TL)	0.04	WB	0.04	(Matrix)			
BCDL	10.0	Rep Stress Incr	NO	TC	0.06	Vert(TL)	-0.05	9	n/r	90	n/a	90	n/a	8	0.00	Horz(TL)	0.04	WB	0.04	(Matrix)			
BCDL	5.0	Code FBC2004/TP12002	NO	TC	0.06	Vert(TL)	-0.05	9	n/r	90	n/a	90	n/a	8	0.00	Horz(TL)	0.04	WB	0.04	(Matrix)			
LUMBER																							
TOP CHORD 2 X 4 SYP No.2																							
BOT CHORD 2 X 4 SYP No.2																							
OTHERS 2 X 4 SYP No.3																							
BRACING																							
TOP CHORD																							
BOT CHORD																							
Structural wood sheathing directly applied or 6-0-0 oc purlins.																							
Rigid ceiling directly applied or 10-0-0 oc bracing.																							

REACTIONS (lb/size) 2=392/9-7-8, 8=392/9-7-8, 11=221/9-7-8, 12=234/9-7-8, 10=234/9-7-8
 Max Horiz 2=80(load case 5)
 Max Uplift 2=308(load case 5), 8=320(load case 6), 11=49(load case 5), 12=79(load case 6), 10=76(load case 6)
 Max Gray 2=392(load case 1), 8=392(load case 1), 11=221(load case 1), 12=239(load case 9), 10=239(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=8/75, 2-3=64/85, 3-4=53/151, 5-6=53/151, 6-7=14/88, 7-8=64/85, 8-9=8/75
 BOT CHORD 2-12=0/72, 11-12=0/72, 10-11=0/72, 8-10=0/72
 WEBS 5-11=163/51, 4-12=174/153, 6-10=174/153

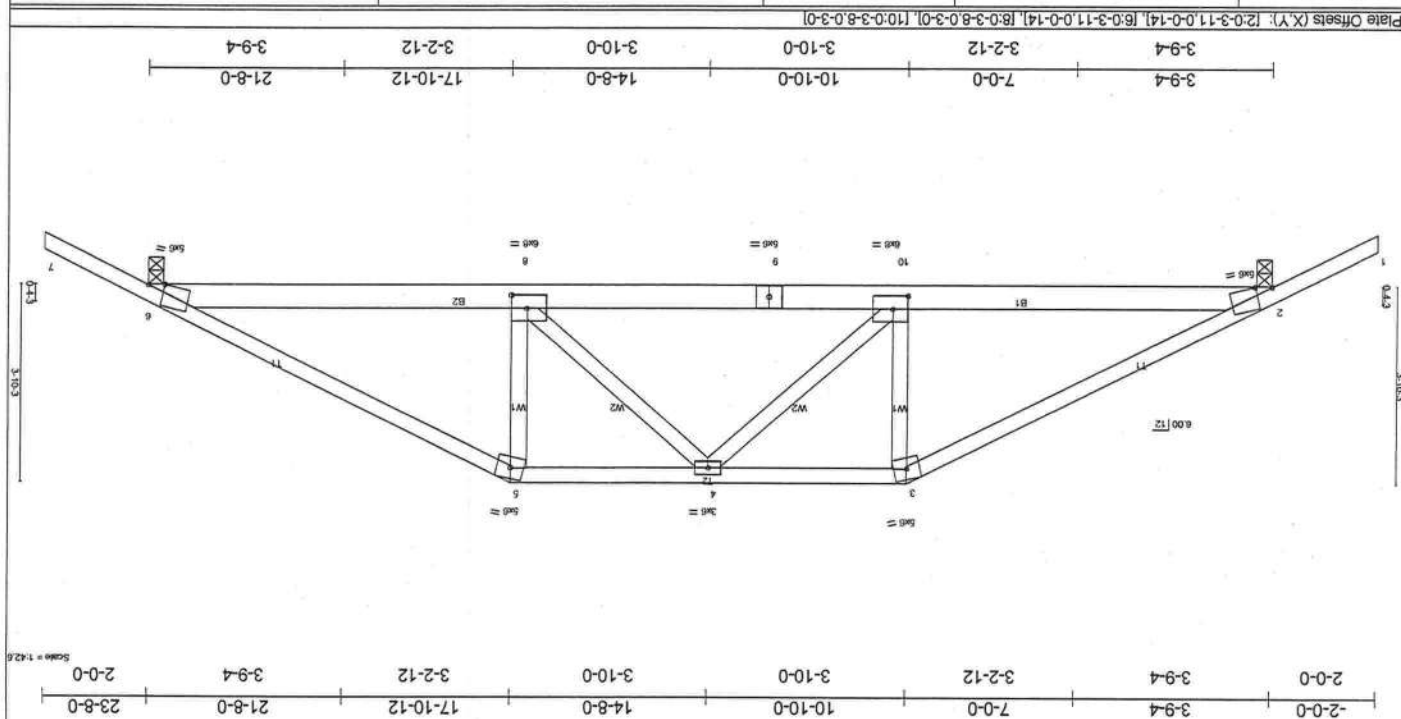
NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02, 120mph (3-second gust); n=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mittek "Standard Gable End Detail".
 4) Gable requires continuous bottom chord bearing.
 5) Gable studs spaced at 2-0-0 oc.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2, 320 lb uplift at joint 5, 49 lb uplift at joint 11, 79 lb uplift at joint 12 and 76 lb uplift at joint 10.
 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vent: 1-5=87(F=-33), 5-9=87(F=-33), 9-12=87(F=-33), 2-8=30

1135117	Truss	Truss type	HIP	Qty	1	Ply	1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 3-0-9 oc purlins, Rigid ceiling directly applied or 6-3-15 oc bracing.

REACTIONS

(lb/size) 2=1926/0-3-8, 6=1926/0-3-8
Max Horz 2=-106(load case 5)
Max Uplift 2=-1092(load case 4), 6=-1092(load case 5)

FORCES (15)

[illegible]

NOTES

Unbraced roof live loads have been considered for this design.
Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCDF=4; zpsf; BCDL=3.0psf; Category II; Exp. B; enclosed; MWFRS gable end zone; lumber DCL=1.60 plate gip DCL=1.60.
Provide adequate drainage to prevent water ponding.
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1092 lb uplift at joint 2 and 1092 lb uplift at joint 6.
Girder carries hip and with 7'-0" end seabck.
Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). 539 lb down and 348 lb up at 14'-8"-0", and 539 lb down and 348 lb up at 7'-0" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
Standard
Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (psf)
Vert: 1'-3"=54, 3'-5"=118(F=64), 5'-7"=54, 2'-10"=30, 8'-10"=65(F=35), 6'-8"=30
Concentrated Loads (lb)
Vert: 10"=539(F) 8"=539(F)

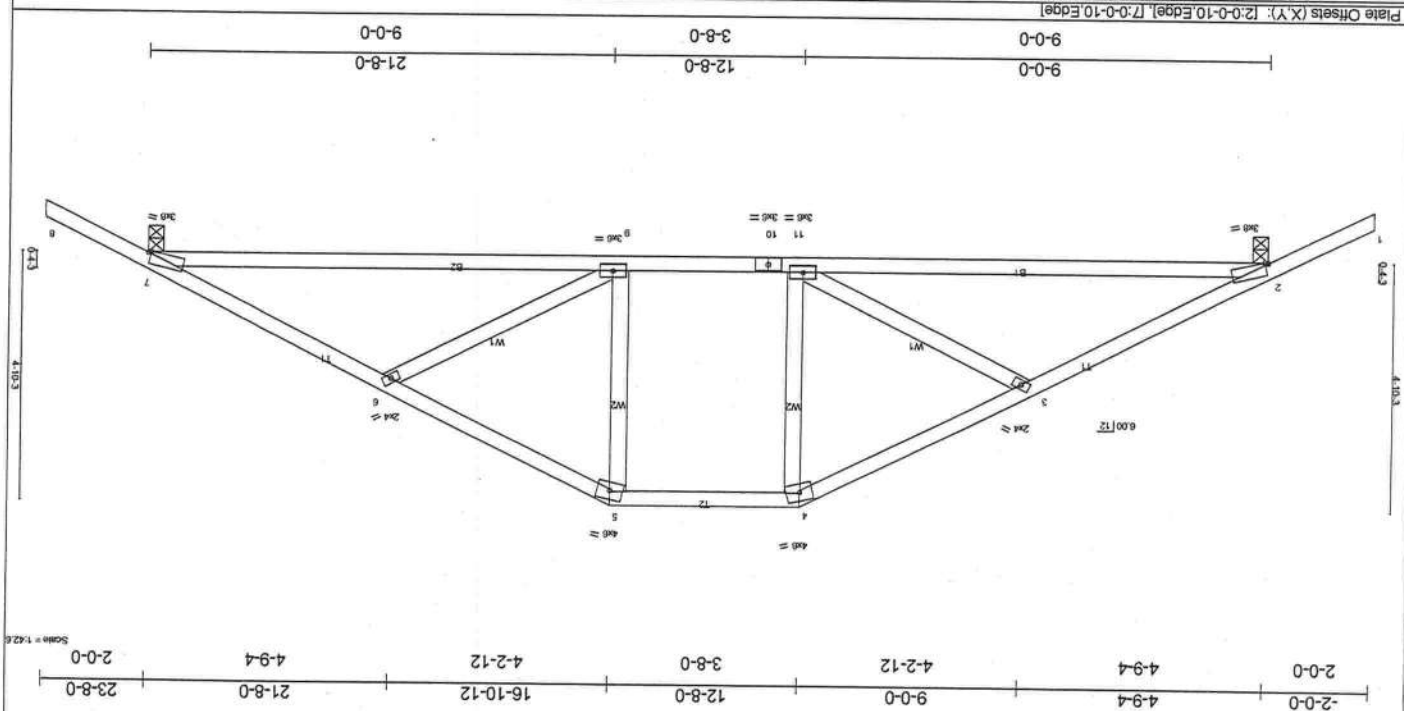
LOAD CASE(S) Standard

Uniform Loads (pH)
Regular: Lumber increase=1.25, Plate increase=1.25
Ver: 1-3=54, 3-5=54, 5-7=64, 7-10=64, 10-12=64, 12-15=64, 15-18=64, 18-21=64, 21-24=64, 24-27=64, 27-30=64, 30-33=64, 33-36=64, 36-39=64, 39-42=64, 42-45=64, 45-48=64, 48-51=64, 51-54=64, 54-57=64, 57-60=64, 60-63=64, 63-66=64, 66-69=64, 69-72=64, 72-75=64, 75-78=64, 78-81=64, 81-84=64, 84-87=64, 87-90=64, 90-93=64, 93-96=64, 96-99=64, 99-102=64, 102-105=64, 105-108=64, 108-111=64, 111-114=64, 114-117=64, 117-120=64, 120-123=64, 123-126=64, 126-129=64, 129-132=64, 132-135=64, 135-138=64, 138-141=64, 141-144=64, 144-147=64, 147-150=64, 150-153=64, 153-156=64, 156-159=64, 159-162=64, 162-165=64, 165-168=64, 168-171=64, 171-174=64, 174-177=64, 177-180=64, 180-183=64, 183-186=64, 186-189=64, 189-192=64, 192-195=64, 195-198=64, 198-201=64, 201-204=64, 204-207=64, 207-210=64, 210-213=64, 213-216=64, 216-219=64, 219-222=64, 222-225=64, 225-228=64, 228-231=64, 231-234=64, 234-237=64, 237-240=64, 240-243=64, 243-246=64, 246-249=64, 249-252=64, 252-255=64, 255-258=64, 258-261=64, 261-264=64, 264-267=64, 267-270=64, 270-273=64, 273-276=64, 276-279=64, 279-282=64, 282-285=64, 285-288=64, 288-291=64, 291-294=64, 294-297=64, 297-300=64, 300-303=64, 303-306=64, 306-309=64, 309-312=64, 312-315=64, 315-318=64, 318-321=64, 321-324=64, 324-327=64, 327-330=64, 330-333=64, 333-336=64, 336-339=64, 339-342=64, 342-345=64, 345-348=64, 348-351=64, 351-354=64, 354-357=64, 357-360=64, 360-363=64, 363-366=64, 366-369=64, 369-372=64, 372-375=64, 375-378=64, 378-381=64, 381-384=64, 384-387=64, 387-390=64, 390-393=64, 393-396=64, 396-399=64, 399-402=64, 402-405=64, 405-408=64, 408-411=64, 411-414=64, 414-417=64, 417-420=64, 420-423=64, 423-426=64, 426-429=64, 429-432=64, 432-435=64, 435-438=64, 438-441=64, 441-444=64, 444-447=64, 447-450=64, 450-453=64, 453-456=64, 456-459=64, 459-462=64, 462-465=64, 465-468=64, 468-471=64, 471-474=64, 474-477=64, 477-480=64, 480-483=64, 483-486=64, 486-489=64, 489-492=64, 492-495=64, 495-498=64, 498-501=64, 501-504=64, 504-507=64, 507-510=64, 510-513=64, 513-516=64, 516-519=64, 519-522=64, 522-525=64, 525-528=64, 528-531=64, 531-534=64, 534-537=64, 537-540=64, 540-543=64, 543-546=64, 546-549=64, 549-552=64, 552-555=64, 555-558=64, 558-561=64, 561-564=64, 564-567=64, 567-570=64, 570-573=64, 573-576=64, 576-579=64, 579-582=64, 582-585=64, 585-588=64, 588-591=64, 591-594=64, 594-597=64, 597-600=64, 600-603=64, 603-606=64, 606-609=64, 609-612=64, 612-615=64, 615-618=64, 618-621=64, 621-624=64, 624-627=64, 627-630=64, 630-633=64, 633-636=64, 636-639=64, 639-642=64, 642-645=64, 645-648=64, 648-651=64, 651-654=64, 654-657=64, 657-660=64, 660-663=64, 663-666=64, 666-669=64, 669-672=64, 672-675=64, 675-678=64, 678-681=64, 681-684=64, 684-687=64, 687-690=64, 690-693=64, 693-696=64, 696-699=64, 699-702=64, 702-705=64, 705-708=64, 708-711=64, 711-714=64, 714-717=64, 717-720=64, 720-723=64, 723-726=64, 726-729=64, 729-732=64, 732-735=64, 735-738=64, 738-741=64, 741-744=64, 744-747=64, 747-750=64, 750-753=64, 753-756=64, 756-759=64, 759-762=64, 762-765=64, 765-768=64, 768-771=64, 771-774=64, 774-777=64, 777-780=64, 780-783=64, 783-786=64, 786-789=64, 789-792=64, 792-795=64, 795-798=64, 798-801=64, 801-804=64, 804-807=64, 807-810=64, 810-813=64, 813-816=64, 816-819=64, 819-822=64, 822-825=64, 825-828=64, 828-831=64, 831-834=64, 834-837=64, 837-840=64, 840-843=64, 843-846=64, 846-849=64, 849-852=64, 852-855=64, 855-858=64, 858-861=64, 861-864=64, 864-867=64, 867-870=64, 870-873=64, 873-876=64, 876-879=64, 879-882=64, 882-885=64, 885-888=64, 888-891=64, 891-894=64, 894-897=64, 897-900=64, 900-903=64, 903-906=64, 906-909=64, 909-912=64, 912-915=64, 915-918=64, 918-921=64, 921-924=64, 924-927=64, 927-930=64, 930-933=64, 933-936=64, 936-939=64, 939-942=64, 942-945=64, 945-948=64, 948-951=64, 951-954=64, 954-957=64, 957-960=64, 960-963=64, 963-966=64, 966-969=64, 969-972=64, 972-975=64, 975-978=64, 978-981=64, 981-984=64, 984-987=64, 987-990=64, 990-993=64, 993-996=64, 996-999=64, 1000-1003=64, 1003-1006=64, 1006-1009=64, 1009-1012=64, 1012-1015=64, 1015-1018=64, 1018-1021=64, 1021-1024=64, 1024-1027=64, 1027-

Job	Truss	Truss Type	City	Ply	1
L135117	T19	HIP	1		

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	PLATES	GRIP
BCDL 5.0	Code FBC2004/TP12002	WB 0.14	Horz(TL) 0.04	7	n/a	MT20	244/190
BCLL 10.0	Rep Stress Incr YES	BC 0.55	Vert(TL) -0.35	7-9 > 728	180		
TCCL 7.0	Lumber Increase 1.25	TC 0.35	Vert(TL) -0.23	7-9 > 999	240		
TCCL 20.0	Plates Increase 1.25						

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD
WEBS 2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-10-12 oc purlins.

REACTIONS (lb/size) 2=1014/0-3-8, 7=1014/0-3-8
Max Horiz Z=121 (load case 6)
Max Uplift Z=516 (load case 5), 7=516 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1476/787, 3-4=-1215/644, 4-5=-1045/638, 5-6=-1215/644, 6-7=-1476/787, 7-8=0/47
BOT CHORD 2-1=-511/1280, 10-11=-270/1045, 9-10=-270/1045, 7-9=-511/1280
WEBS 3-11=-316/274, 4-11=-81/311, 5-9=-81/311, 6-9=-316/274

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 120mph (3-second gust); h=20ft, TCFL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 516 lb uplift at joint 2 and 516 lb uplift at joint 7.
LOAD CASE(S) Standard

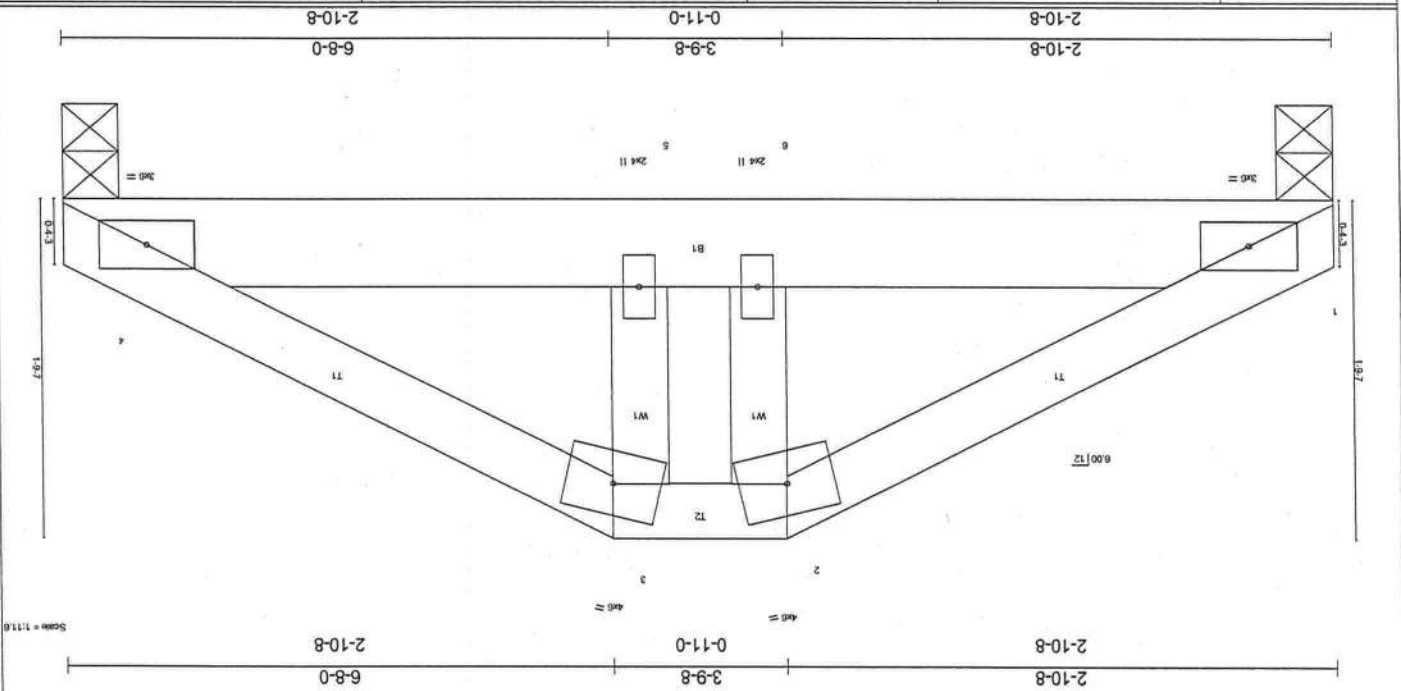
Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
L135117	T20	COMMON	4		6,200 s Jul 13 2005 Mitek Industries, Inc. Thu Oct 13 12:01:52 2005 Page 1

Builders FirstSource, Lake City, FL 32055

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Job	L135117	Truss	T21	Truss type	HIP	Qty	1	Ply	1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055



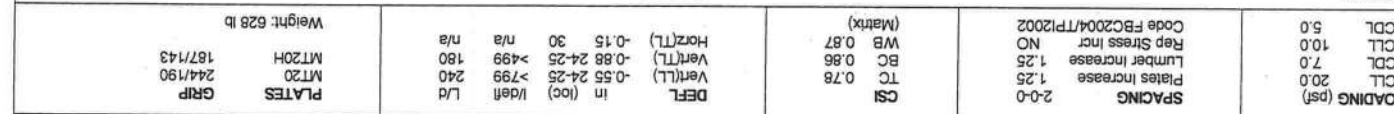
LOADING (psf)	SPACING	CSI	DEF.	VERT (LL)	VERT (TL)	HORIZ (TL)	TOP CHORD	BOT CHORD	BRACING
TCLL 20.0	2-0-0	TC 0.10	0.01	0.02	0.00	0.00	Structural wood sheathing directly applied or 6-0-0 oc purlins.	Rigid ceiling directly applied or 10-0-0 oc bracing.	Weight: 30 lb
PLATES GRIP									
MT20									
244/190									

TOP CHORD 2 X 4 SYP No.2	TOP CHORD 2 X 4 SYP No.3	WEBS	WEBS

REACTIONS (lb/size)	1=64/3-0-3-8, 4=64/3-0-3-8	Max Horiz 1=24/(load case 3)	Max Uplift 1=300/(load case 4), 4=300/(load case 5)
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FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD 1-2=-905/417, 2-3=-807/407, 3-4=-905/416	BOT CHORD 1-6=-343/776, 5-6=-353/807, 4-5=-338/776	WEBS 2-6=-140/332, 3-5=-140/332
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NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDF=3.0psf; Category III; Exp. B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grp DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 1 and 300 lb uplift at joint 4.
5) Girder carries tie-in spans(s): 7-0-0 from 0-0-0 to 6-8-0
6) Girder carries hip end with 2-10-8 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 55 lb down and 36 lb up at 3-9-8, and 55 lb down and 36 lb up at 2-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (psf)
Vert. 1-2=-54, 2-3=-62(F=-8), 3-4=-54, 1-6=-129(F=-99), 5-6=-133(F=-103), 4-5=-129(F=-99)
Concentrated Loads (lb)
Vert. 6=-55(F) 5=-55(F)



FACTORS (lb/size) 2=388/0-3-8, 30=5473/0-3-8, 19=3993/0-3-8
Max Horiz 2=847(load case 4)
Max Uplift2=-395(load case 4), 30=-3101(load case 2), 19=-2332(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension	CHORD	CHORD
1-2=0/47, 2-3=-90/1134, 3-4=-964/1836, 5-6=-3333/5850, 6-7=-3333/5850, 7-8=-6594/3131, 8-9=-8734/4268,	16-17=-352/464, 17-18=-45/138	16-17=-352/464, 17-18=-45/138
9-10=-8734/4268, 10-11=-8734/4268, 11-12=-9195/4643, 12-13=-9019/4654, 13-14=-5411/2827, 14-15=-5411/2827, 15-16=-352/464,	2-3=-822/50, 32-33=-848/95, 31-32=-5956/2787, 30-31=-5317/17313, 5-6=-1927/11318, 29-30=-183/201, 28-29=-133/387, 27-28=0/1174,	2-3=-822/50, 32-33=-848/95, 31-32=-5956/2787, 30-31=-5317/17313, 5-6=-1927/11318, 29-30=-183/201, 28-29=-133/387, 27-28=0/1174,

NOTES

2-ply truss to be connected together with 0.131"x3" Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0c.

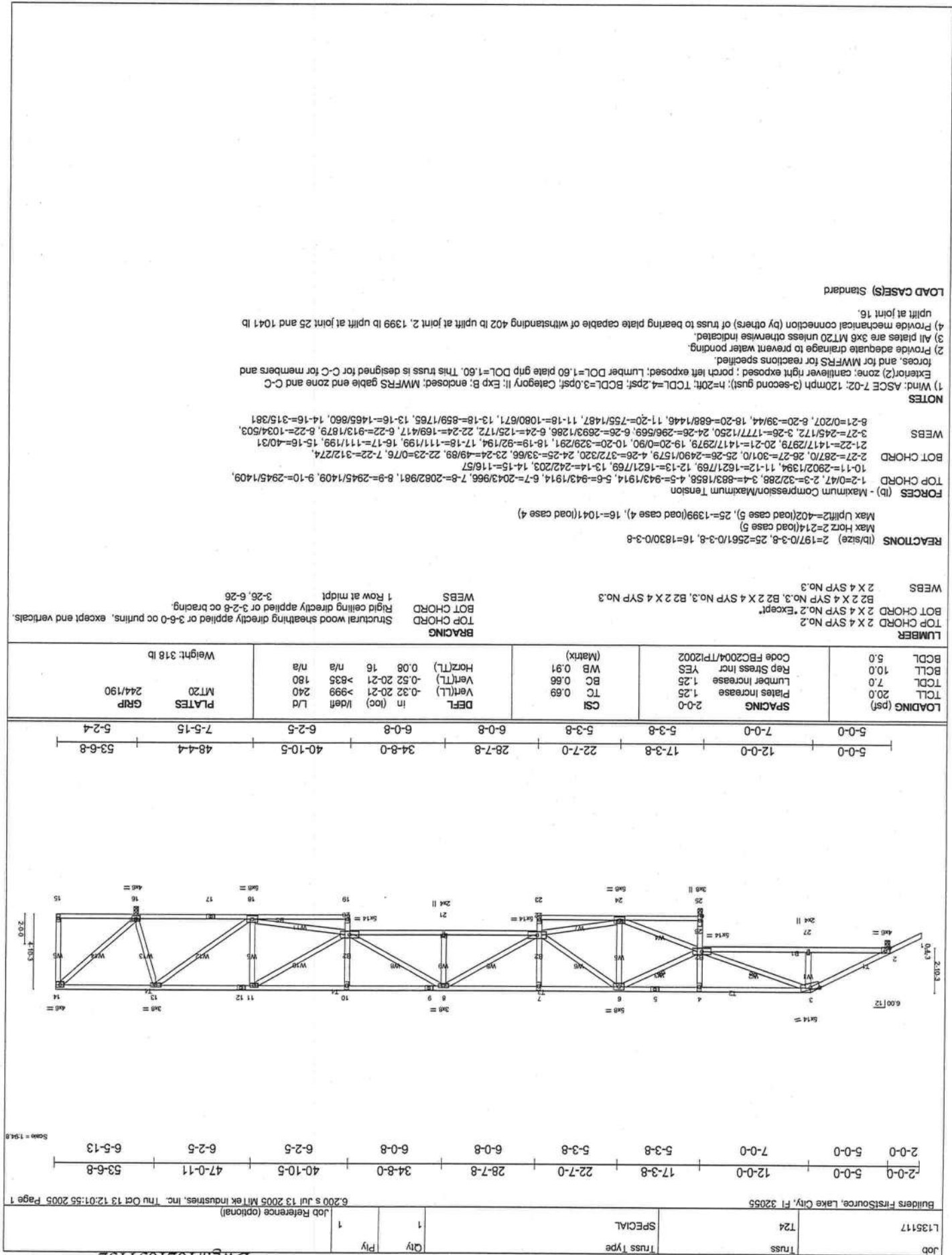
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0c.

Webbs connected as follows: 2 X 4 - 1 row at 0-9-0c.

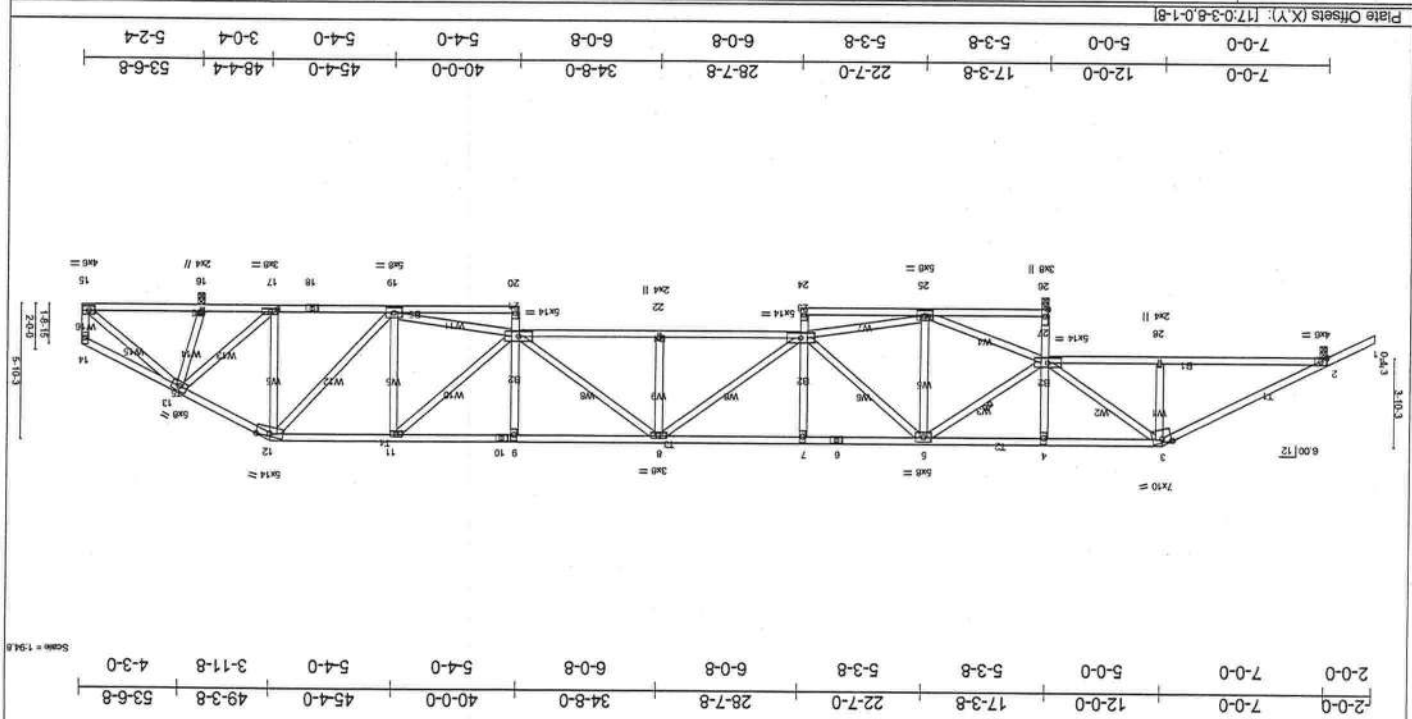
All nails are considered equally applied to all where required.

(B) or back (B) face in the (A) or front (F) face.

It is the contractor's option.



Job	Truss	Truss Type	Qty	Ply
L735117	T25	SPECIAL	1	
Builders FirstSource, Lake City, Fl 32055				
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Job Reference (optional)				



LOADING (psf)	SPACING	CSI	DEFL	BRACING	LUMBER	TOP CHORD	BOT CHORD	WEBS	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
BCDL 5.0	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
BCLL 10.0	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
TCDL 7.0	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
TCLL 20.0	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
Plates Increase 1.25	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
Lumber Increase 1.25	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
Rep Stress Incr YES	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
Code FBC2004/TP12002	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
(Matrix)	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
WEBS	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
2 X 4 SYP No.3	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
2 X 4 SYP No.3, B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
2 X 4 SYP No.3	2'-0"	2'-0"	0.05	0.05	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"

REACTIONS	FORCES (lb) - Maximum Compression/Maximum Tension	TOP CHORD	BOT CHORD	WEBS	LOAD CASES
(lb/size) 2=144/0-3-8, 26=263/0-3-8, 16=1815/0-3-8	Max Horiz 2=144(load case 5) Max Uplift2=1403(load case 5), 26=-812(load case 6) Max Grav 2=144(load case 1), 26=2643(load case 9), 16=1817(load case 10)	1-2=47, 2-3=-222/671, 3-4=-756/1603, 4-5=-752/1595, 5-6=-1514/814, 6-7=-1514/814, 7-8=-1537/823, 8-9=-2280/1169, 9-10=-2251/1157, 10-11=-2251/1157, 11-12=-1440/777, 12-13=-657/381, 13-14=-108/167, 14-15=-128/168, 15-16=-168/228, 16-17=-650/533, 17-18=-223/561, 18-19=-223/561, 19-20=-71/133, 20-21=302/268, 21-22=-299/255, 22-23=-32/52, 23-24=0/75, 24-25=-307/269, 25-26=-5/36, 26-27=-299/255, 27-28=560/284, 28-29=-2570/1610, 29-30=1033/2275, 30-31=1033/2275, 31-32=-1033/2275, 32-33=-293/215, 33-34=-293/215, 34-35=-1408/1075, 35-36=-139/350, 36-37=-2322/1195, 37-38=-513/1089, 38-39=-1010/592, 39-40=-653/1279, 40-41=-906/575, 41-42=-788/1556, 42-43=-15/26, 43-44=-580/1330, 44-45=-11-21=-513/1089, 45-46=-1010/592, 46-47=-653/1279, 47-48=-906/575, 48-49=-788/1556, 49-50=-13-16=-1738/1103, 50-51=-415/298	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 120mph (3-second gust); n=20; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C 3) Provide adequate drainage to prevent water ponding. 4) All plates are 3x6 MT20 unless otherwise indicated. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 403 lb uplift at joint 2, 1426 lb uplift at joint 26 and 812 lb uplift at joint 16.	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 120mph (3-second gust); n=20; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C 3) Provide adequate drainage to prevent water ponding. 4) All plates are 3x6 MT20 unless otherwise indicated. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 403 lb uplift at joint 2, 1426 lb uplift at joint 26 and 812 lb uplift at joint 16.	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 120mph (3-second gust); n=20; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C 3) Provide adequate drainage to prevent water ponding. 4) All plates are 3x6 MT20 unless otherwise indicated. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 403 lb uplift at joint 2, 1426 lb uplift at joint 26 and 812 lb uplift at joint 16.

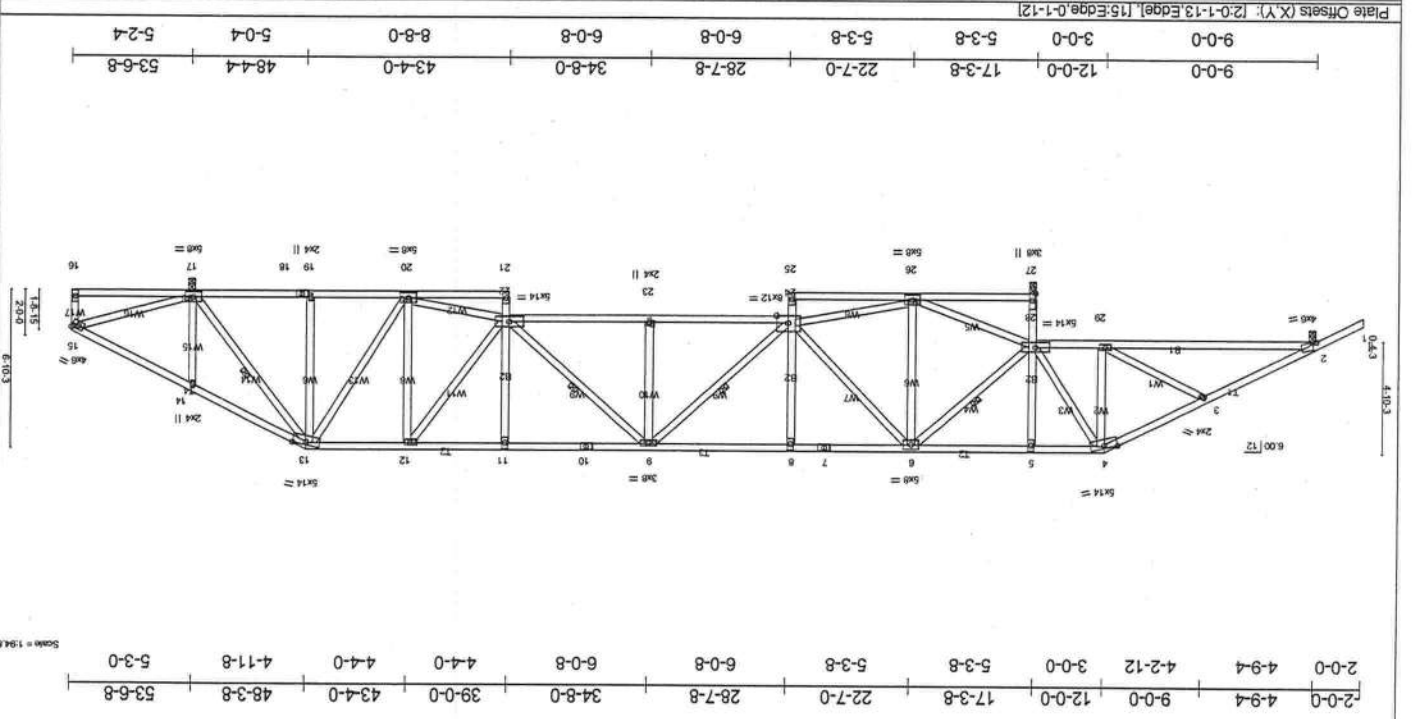
LOAD CASE(S) Standard

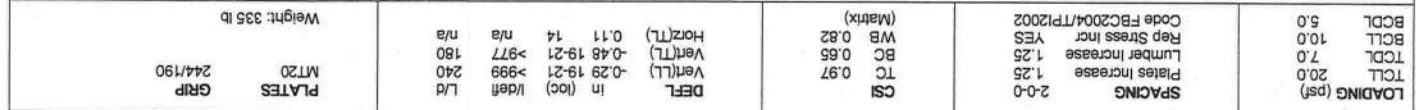
- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=0.3psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; cantilever right exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 M/T20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 2, 1388 lb uplift at joint 27 and 840 lb uplift at joint 17.

NOTES

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=1152/986, 11-12=1831/977, 12-13=1275/731, 13-14=812/294, 14-15=261/304, 15-16=115/67
BOT CHORD 2-3=240/669, 3-4=291/781, 4-5=577/1260, 5-6=567/1248, 6-7=1273/711, 7-8=1273/711, 8-9=1288/716, 9-10=1852/986, 10-11=1652/986, 11-12=1831/977, 12-13=1275/731, 13-14=812/294, 14-15=261/304, 15-16=115/67
WEBS 3-29=337/390, 4-29=518/399, 4-28=1084/906, 26-28=92/304, 6-28=5/80, 24-26=54/273, 6-24=673/1423, 9-24=799/383, 9-23=0/202, 9-22=26/20, 20-22=498/1226, 12-22=424/923, 12-20=954/535, 13-19=0/133, 13-17=1598/769, 14-17=292/361, 15-17=310/449, 13-20=474/945
REACTIONS (lb/size) 2=145/0-3-8, 17=1816/0-3-8, 27=2628/0-3-8, 17=1816/0-3-8
Max Horiz 2=131(load case 5)
Max Uplift 2=396(load case 5), 27=1388(load case 4), 17=840(load case 5)
Max Grav 2=147(load case 9), 27=2628(load case 1), 17=1825(load case 10)

LUMBER		TOP CHORD 2 X 4 SYP No.2		BOT CHORD 2 X 4 SYP No.2 "Except"		WEBS 2 X 4 SYP No.3, B2 2 X 4 SYP No.3, B2 2 X 4 SYP No.3	
LOADING (psi)	BCDL 5.0	Code FBC2004/TP12002	Rep Stress Incr YES	Plates Increase 1.25	Lumber Increase 1.25	TC 0.39	WB 0.73
CS	BC 0.46	CS	WB 0.73	DEFL (in (loc)) 0.05	Vert(TL) 0.22	Horz(TL) 0.05	Vert(TL) 0.22
SPACING	2-0-0	Plates Increase 1.25	Lumber Increase 1.25	DEFL (in (loc)) 0.05	Vert(TL) 0.22	Horz(TL) 0.05	Vert(TL) 0.22
PLATES	244/190	GRIP	244/190	Weight: 363 lb	BRACING		
TOP CHORD			BOT CHORD			WEBS	
Structural wood sheathing directly applied or 4-5-9 oc purlins, except end verticals.			Rigid ceiling directly applied or 3-5-2 oc bracing.			1 Row at midpt	
6-26, 9-24, 9-22, 13-17			6-26, 9-24, 9-22, 13-17				





REACTIONS	(lb/size) 2=69/0-3-8, 26=2848/0-3-8, 14=1681/0-3-8 Max Hot 2=164/(bad case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

NOTES

uplift at joint 14.

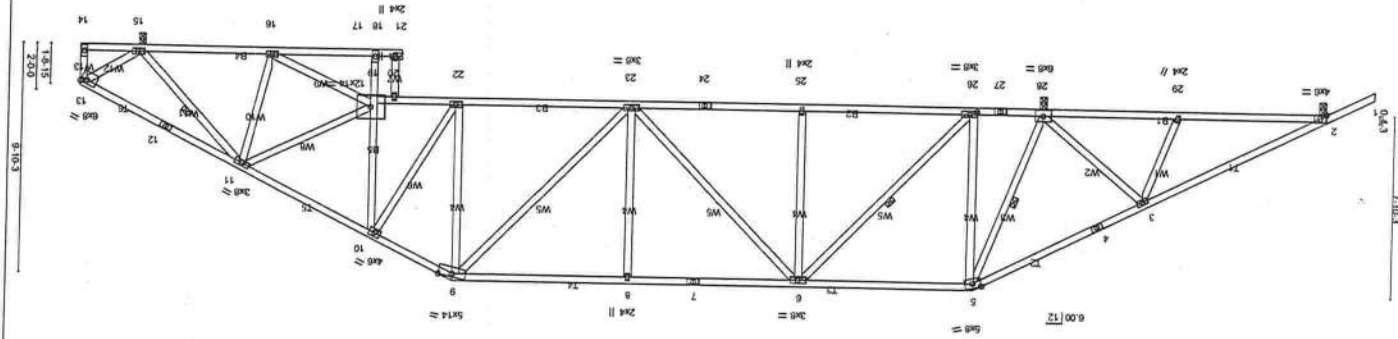
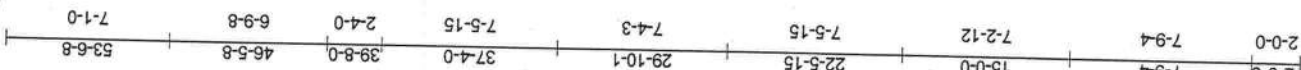
LOAD CASE(S) Standard

Job	Truss	Truss Type	City	Qty	PLY
L135117	T30	SPECIAL	1	1	

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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LOADING (psf)	SPACING	CS	DEFL	BRACING	REACTIONS
BCDL 5.0	Code FBC2004/TP12002	WB 0.88	Horz(TL) 0.11	TOP CHORD	(b) - Maximum Compression/Maximum Tension
BCLL 10.0	Rep Stress Incr YES	BC 0.72	Vert(TL) -0.32	BOT CHORD	Max Horiz 2=161(100 case 5)
TCDD 7.0	Lumber Increase 1.25	TC 0.66	Vert(BL) -0.19	TOP CHORD	Max Uplift 2=377(100 case 5), 28=2689(100 case 9), 15=1721(100 case 10)
TCCL 20.0	Plates Increase 1.25		In (loc) 15	WEBS	
			l/defl 240		
			L/D 180		
			n/a >999		
			n/a 15		
			n/a		

REACTIONS	TOP CHORD	BOT CHORD	WEBS	BRACING	LOADING (psf)
(b) - Maximum Compression/Maximum Tension	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.	BCDL 5.0
Max Horiz 2=161(100 case 5)	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	BCLL 10.0
Max Uplift 2=377(100 case 5), 28=2689(100 case 9), 15=1721(100 case 10)	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCDD 7.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCCL 20.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	5-26, 6-26, 11-15	

FORCES	TOP CHORD	BOT CHORD	WEBS	BRACING	LOADING (psf)
(b) - Maximum Compression/Maximum Tension	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.	BCDL 5.0
Max Horiz 2=161(100 case 5)	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	BCLL 10.0
Max Uplift 2=377(100 case 5), 28=2689(100 case 9), 15=1721(100 case 10)	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCDD 7.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCCL 20.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	5-26, 6-26, 11-15	

NOTES	TOP CHORD	BOT CHORD	WEBS	BRACING	LOADING (psf)
1) Unbalanced roof live loads have been considered for this design.	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.	BCDL 5.0
2) Wind: ASCE 7-02: 120mph (3-second gust); h=20ft; TCDD=4.2psf; BCDL=3.0psf; Category II: Exp B; enclosed; MWFRS gable end zone and C-C	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	BCLL 10.0
3) Provide adequate drainage to prevent water ponding.	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCDD 7.0
4) All plates are 3x6 MT20 unless otherwise indicated.	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCCL 20.0
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 2, 1182 lb uplift at joint 28 and 765 lb	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	5-26, 6-26, 11-15	

LOAD CASES	TOP CHORD	BOT CHORD	WEBS	BRACING	LOADING (psf)
Standard	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.	BCDL 5.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	BCLL 10.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCDD 7.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCCL 20.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	5-26, 6-26, 11-15	

THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987	TOP CHORD	BOT CHORD	WEBS	BRACING	LOADING (psf)
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.	BCDL 5.0
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:	BCLL 10.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCDD 7.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	1 Row at midpt	TCCL 20.0
	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	5-26, 6-26, 11-15	

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
L135117	T31	SPECIAL	1	1	

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Dwg.#1021051138

Builders FirstSource, Lake City, FL 32055

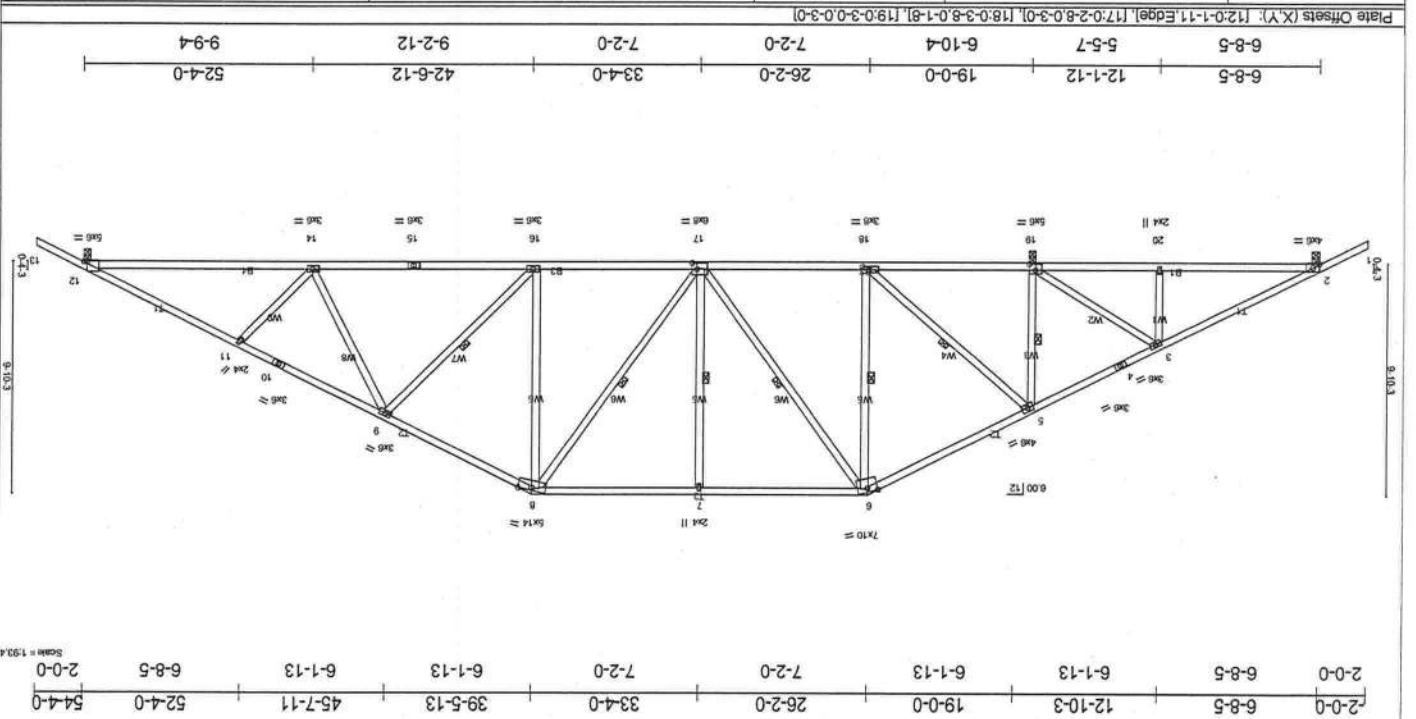
LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02: 120mph (3-second gust): h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1223 lb uplift at joint 19, 813 lb uplift at joint 12 and 388 lb uplift at joint 2.

NOTES

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=0/465, 3-4=368/748, 4-5=350/867, 5-6=804/555, 6-7=1317/938, 7-8=1317/938, 8-9=1804/1106, 9-10=2553/1425, 10-11=2633/1403, 11-12=2867/1508, 12-13=0/47
BOT CHORD 2-20=389/187, 19-20=389/187, 18-19=718/700, 17-18=131/633, 16-17=406/1561, 15-16=778/2028, 14-15=778/2028, 12-14=-1130/2501
WEBS 3-20=295/195, 3-19=564/736, 6-18=822/1813, 6-17=976/596, 6-17=594/1172, 7-17=407/357, 8-17=415/235, 8-16=339/736, 9-16=685/540, 9-14=-225/596, 11-14=-315/370, 5-19=-2196/1262
REACTIONS (lb/size) 19=2677/0-3-8, 12=1684/0-3-8, 2=242/0-3-8
Max Horiz 2-20(4load case 6), 2=388(4load case 5)
Max Uplift 19=-1223(4load case 5), 12=-813(4load case 5), 2=326(4load case 1), 2=326(4load case 9)
Max Grav 19=2677(4load case 1), 12=1684(4load case 1), 2=326(4load case 9)

LUMBER		BRACING		WEBS		PLATES		GRIP	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-4-1 cc purlins.	TOP CHORD	Structural wood sheathing directly applied or 3-4-1 cc purlins.	PLATES	MT20	GRIP	244/190
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 5-1-4 cc bracing.	BOT CHORD	Rigid ceiling directly applied or 5-1-4 cc bracing.	PLATES	MT20	GRIP	244/190
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt	WEBS	1 Row at midpt	PLATES	MT20	GRIP	244/190
SPACING	2-0-0	DEFL	in (loc)	DEFL	in (loc)	PLATES	MT20	GRIP	244/190
Plates Increase	1.25	Ver(TL)	-0.27	Ver(TL)	-0.27	PLATES	MT20	GRIP	244/190
Lumber Increase	1.25	Ver(BL)	14.16	Ver(BL)	14.16	PLATES	MT20	GRIP	244/190
Rep Stress Incr	YES	Horz(TL)	0.08	Horz(TL)	0.08	PLATES	MT20	GRIP	244/190
Code FBC2004/TP12002		Horz(BL)	12	Horz(BL)	12	PLATES	MT20	GRIP	244/190
WB	0.58	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
BC	0.76	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
TC	0.47	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
CSI		L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
WB	0.58	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
BC	0.76	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
TC	0.47	L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190
CSI		L/d	n/a	L/d	n/a	PLATES	MT20	GRIP	244/190

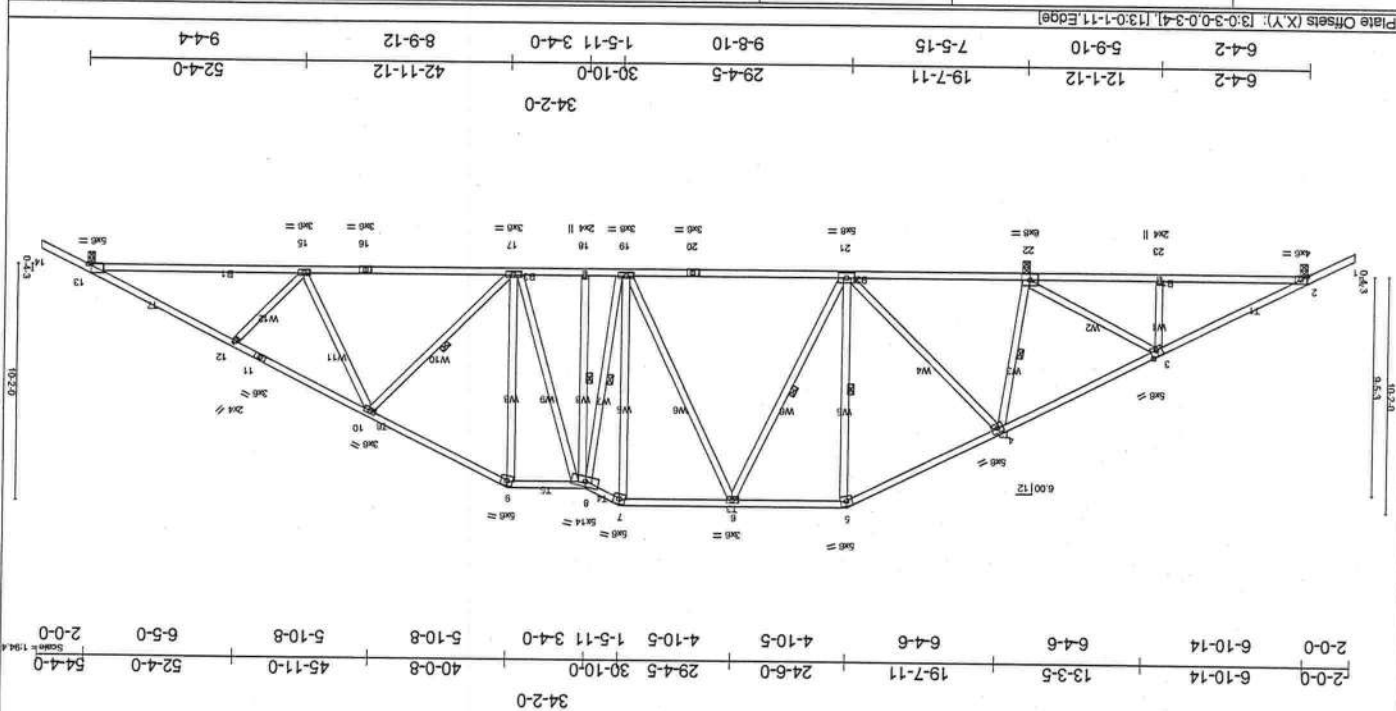


100	Truss	Truss Type	Qty	1	1
L135117	T33	SPECIAL			
Builders FirstSource, Lake City, FL 32055					

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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LUMBER	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	TOP CHORD	TOP CHORD	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17	
BRACING	2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	TOP CHORD	TOP CHORD	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17	
LOADING (psf)	20.0	20.0	20.0	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	TCLL	TCLL	TCLL	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
SPACING	2-0-0	2-0-0	2-0-0	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	Plates Increase	Plates Increase	Plates Increase	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
Code	FBC2004/TP12002	FBC2004/TP12002	FBC2004/TP12002	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	Rep Stress Incr	Rep Stress Incr	Rep Stress Incr	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
Lumber Increase	1.25	1.25	1.25	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	TC	TC	TC	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
CS	0.46	0.46	0.46	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	WB	WB	WB	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
(Matrx)	0.99	0.99	0.99	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	Horz(TL)	Horz(TL)	Horz(TL)	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
DEFL	in (loc)	in (loc)	in (loc)	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	0.25	0.25	0.25	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
Vert(TL)	15-17	15-17	15-17	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	>99	>99	>99	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
L/d	180	180	180	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	n/a	n/a	n/a	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
Weight: 348 lb	MT20	MT20	MT20	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17
	GRIP	GRIP	GRIP	WEBS	1 Row at midpt	4-22-5-21, 6-21, 8-18, 10-17

REACTIONS

(lb/size) 22=2785/0-3-8, 13=1667/0-3-8, 2=151/0-3-8

Max Horiz 2=-209(load case 6)
Max Uplift2=-1044(load case 5), 13=-845(load case 6), 2=-211(load case 5)
Max Grav 2=-2785(load case 1), 13=-1667(load case 1), 2=-275(load case 9)

TOP CHORD

- Maximum Compression/Maximum Tension
1-2=0/47, 2-3=-187/620, 3-4=-372/1090, 4-5=-806/634, 5-6=-647/649, 6-7=-1384/1022, 7-8=-1502/1067, 8-9=-1585/1113, 9-10=-1836/1156

WEBS

10-11-2=2549/14454, 11-12-2=2633/14134, 12-13-2=2845/15133, 13-14-2=2271/14040, 14-15-2=2232/13883, 15-16-2=529/3862, 16-17-2=517/3499, 17-18-2=2271/14072, 18-19-2=427/1504, 17-18-4=427/1502, 3-2=871/85, 4-5=670/420, 5-6=332/31271, 6-7=432/43856, 7-8=31-43856, 8-9=31-43856, 9-10=31-43856, 10-11=31-43856, 11-12=31-43856, 12-13=31-43856, 13-14=31-43856, 14-15=31-43856, 15-16=31-43856, 16-17=31-43856, 17-18=31-43856, 18-19=31-43856, 19-20=31-43856, 20-21=31-43856, 21-22=31-43856, 22-23=31-43856, 23-24=31-43856, 24-25=31-43856, 25-26=31-43856, 26-27=31-43856, 27-28=31-43856, 28-29=31-43856, 29-30=31-43856, 30-31=31-43856, 31-32=31-43856, 32-33=31-43856, 33-34=31-43856, 34-35=31-43856, 35-36=31-43856, 36-37=31-43856, 37-38=31-43856, 38-39=31-43856, 39-40=31-43856, 40-41=31-43856, 41-42=31-43856, 42-43=31-43856, 43-44=31-43856, 44-45=31-43856, 45-46=31-43856, 46-47=31-43856, 47-48=31-43856, 48-49=31-43856, 49-50=31-43856, 50-51=31-43856, 51-52=31-43856, 52-53=31-43856, 53-54=31-43856, 54-55=31-43856, 55-56=31-43856, 56-57=31-43856, 57-58=31-43856, 58-59=31-43856, 59-60=31-43856, 60-61=31-43856, 61-62=31-43856, 62-63=31-43856, 63-64=31-43856, 64-65=31-43856, 65-66=31-43856, 66-67=31-43856, 67-68=31-43856, 68-69=31-43856, 69-70=31-43856, 70-71=31-43856, 71-72=31-43856, 72-73=31-43856, 73-74=31-43856, 74-75=31-43856, 75-76=31-43856, 76-77=31-43856, 77-78=31-43856, 78-79=31-43856, 79-80=31-43856, 80-81=31-43856, 81-82=31-43856, 82-83=31-43856, 83-84=31-43856, 84-85=31-43856, 85-86=31-43856, 86-87=31-43856, 87-88=31-43856, 88-89=31-43856, 89-90=31-43856, 90-91=31-43856, 91-92=31-43856, 92-93=31-43856, 93-94=31-43856, 94-95=31-43856, 95-96=31-43856, 96-97=31-43856, 97-98=31-43856, 98-99=31-43856, 99-100=31-43856, 100-101=31-43856, 101-102=31-43856, 102-103=31-43856, 103-104=31-43856, 104-105=31-43856, 105-106=31-43856, 106-107=31-43856, 107-108=31-43856, 108-109=31-43856, 109-110=31-43856, 110-111=31-43856, 111-112=31-43856, 112-113=31-43856, 113-114=31-43856, 114-115=31-43856, 115-116=31-43856, 116-117=31-43856, 117-118=31-43856, 118-119=31-43856, 119-120=31-43856, 120-121=31-43856, 121-122=31-43856, 122-123=31-43856, 123-124=31-43856, 124-125=31-43856, 125-126=31-43856, 126-127=31-43856, 127-128=31-43856, 128-129=31-43856, 129-130=31-43856, 130-131=31-43856, 131-132=31-43856, 132-133=31-43856, 133-134=31-43856, 134-135=31-43856, 135-136=31-43856, 136-137=31-43856, 137-138=31-43856, 138-139=31-43856, 139-140=31-43856, 140-141=31-43856, 141-142=31-43856, 142-143=31-43856, 143-144=31-43856, 144-145=31-43856, 145-146=31-43856, 146-147=31-43856, 147-148=31-43856, 148-149=31-43856, 149-150=31-43856, 150-151=31-43856, 151-152=31-43856, 152-153=31-43856, 153-154=31-43856, 154-155=31-43856, 155-156=31-43856, 156-157=31-43856, 157-158=31-43856, 158-159=31-43856, 159-160=31-43856, 160-161=31-43856, 161-162=31-43856, 162-163=31-43856, 163-164=31-43856, 164-165=31-43856, 165-166=31-43856, 166-167=31-43856, 167-168=31-43856, 168-169=31-43856, 169-170=31-43856, 170-171=31-43856, 171-172=31-43856, 172-173=31-43856, 173-174=31-43856, 174-175=31-43856, 175-176=31-43856, 176-177=31-43856, 177-178=31-43856, 178-179=31-43856, 179-180=31-43856, 180-181=31-43856, 181-182=31-43856, 182-183=31-43856, 183-184=31-43856, 184-185=31-43856, 185-186=31-43856, 186-187=31-43856, 187-188=31-43856, 188-189=31-43856, 189-190=31-43856, 190-191=31-43856, 191-192=31-43856, 192-193=31-43856, 193-194=31-43856, 194-195=31-43856, 195-196=31-43856, 196-197=31-43856, 197-198=31-43856, 198-199=31-43856, 199-200=31-43856, 200-201=31-43856, 201-202=31-43856, 202-203=31-43856, 203-204=31-43856, 204-205=31-43856, 205-206=31-43856, 206-207=31-43856, 207-208=31-43856, 208-209=31-43856, 209-210=31-43856, 210-211=31-43856, 211-212=31-43856, 212-213=31-43856, 213-214=31-43856, 214-215=31-43856, 215-216=31-43856, 216-217=31-43856, 217-218=31-43856, 218-219=31-43856, 219-220=31-43856, 220-221=31-43856, 221-222=31-43856, 222-223=31-43856, 223-224=31-43856, 224-225=31-43856, 225-226=31-43856, 226-227=31-43856, 227-228=31-43856, 228-229=31-43856, 229-230=31-43856, 230-231=31-43856, 231-2

NOTES

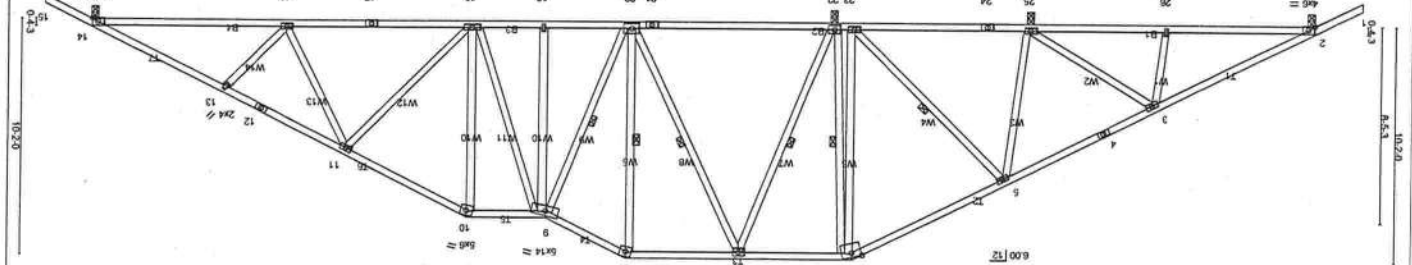
8-19=-728/570, 8-18=-156/0, 8-17=-154/267, 9-17=-222/479, 10-17=-662/521, 10-15=-206/562, 12-15=-293/345

3) Provide adequate

1.00 live loads have been considered for this design.
C-02, 1.20mph (3-second gust); $T_{CDL}=20\%$; $T_{CDL}=4.2\%$; $B_{CDL}=3.0\%$; Exp B: enclosed; MWFRS gable end zone and C-C zone; Lumber DOL=1.60 plate gip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. equire drainage to prevent water ponding.

LOAD CASES

Products (continued)

$$\begin{array}{r} 14.0 \\ -0.0 \\ \hline 14.0 \end{array}$$


LOADING (psf)	20.0	TCLD	7.0	BCLL	10.0	BCDL	5.0
SPACING	2-0-0	CSCI	0.43	TCB	0.82	WB	0.78
	Plates Increase	Ver(LL)	-0.14	Horz(TL)	0.03	n/a	n/a
	Lumber Increase	Ver(TL)	-0.22	Horz(TL)	0.03	14	14
	Rep Stress Incr	Yes				>999	>999
	Code FBC2004/TP12002					180	180
						L/d	I/d _{eff}
						240	240
						MT20	MT20
						GRIP	GRIP
						Weight: 354 lb	Weight: 354 lb

WEBS
2 X 4 SYP No.3
B3 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 "except"
TOP CHORD 2 X 4 SYP No.2
COMBRI

BRACING
TOP CHORD
BOT CHORD
WEBS
1 Row at midpt
5-23, 7-22, 7-20, 8-20, 9-20, 6-22

Structural wood sheathing directly applied or 4-4-3 oc purlins.
Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 25=559/0-3-8, 22=2460/0-3-8, 14=1202/0-3-8, 2=342/0-3-8

Max Uplift=2-2=0 (load case 5), 22=-912 (load case 6), 14=-672 (load case 6), 14=-672 (load case 6), 2=-267 (load case 5)
Max Grav=25=767 (load case 9), 22=2460 (load case 1), 14=1214 (load case 10), 2=417 (load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
1-2=20/47, 2-3=-20/32/20, 3-4=-190/537, 4-5=-169/675, 5-6=-153/868, 6-7=-68/770, 7-8=-317/473, 8-9=-390/468, 9-10=-842/698, 10-11=-1003/706, 11-12=-1637/964, 12-13=-1703/950, 13-14=-1914/1029, 14-15=-1047

2-26 = 190/267, 25-26 = 212/31/263, 24-25 = 525/513, 23-24 = 525/513, 22-23 = 705/720, 21-22 = 227/1465, 20-21 = 227/1465, 19-20 = 53/662, 18-19 = 83/662, 17-18 = 434/1251, 14-16 = 117/1644, 13-16 = 131/83, 12-15 = 257/232, 11-14 = 340/1251, 10-13 = 273/239, 9-12 = 262/302, 8-11 = 175/499, 7-10 = 574/1201, 6-9 = 94/1604, 5-8 = 94/1604, 4-7 = 64/1604, 3-6 = 66/84, 2-5 = 66/84, 1-4 = 66/84, 0-3 = 66/84, 9-19 = 214/1, 11-18 = 79/171, 10-18 = 79/171, 9-18 = 593/463, 11-16 = 175/499, 13-16 = 262/302, 6-22 = 766/467

NOTES

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-02, 120mph (3-second gust); h=20ft; TCDF=4.2psf; BCLF=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DCL=1.60, plate grip DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) Provide adequate drainage to prevent water ponding.
- (4) All plates are 3x6 M20 unless otherwise indicated.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 383 lb uplift at joint 25, 912 lb uplift at joint 22, 672 lb uplift at joint 14 and 267 lb uplift at joint 2.

LOAD CASE(S) Standard

LOAD CASE(S) Standard

LOADING (psf)	20.0	TCLL	7.0	BCLL	10.0	BCDL	5.0
SPACING							
2-0-0	1.25	Plates Increase	0.46	TC	0.52	BC	0.80
		Lumber Increase		Ver(TL)		WB	
		Rep Stress Incr		Ver(TL)			
		YES		Horz(TL)			
		Code FBC2004/TP12002					
CSI							
		in (loc)		Defl			
		l/d		l/d			
		240		240			
		>999		>999			
		180		180			
		>713		>713			
		0.34		0.04			
		13		13			
		n/a		n/a			
PLATES							
		MT20		GRIP			
		244/190		Weight: 335 lb			

LOAD CASE(S) Standard

LOAD CASE(S) Standard

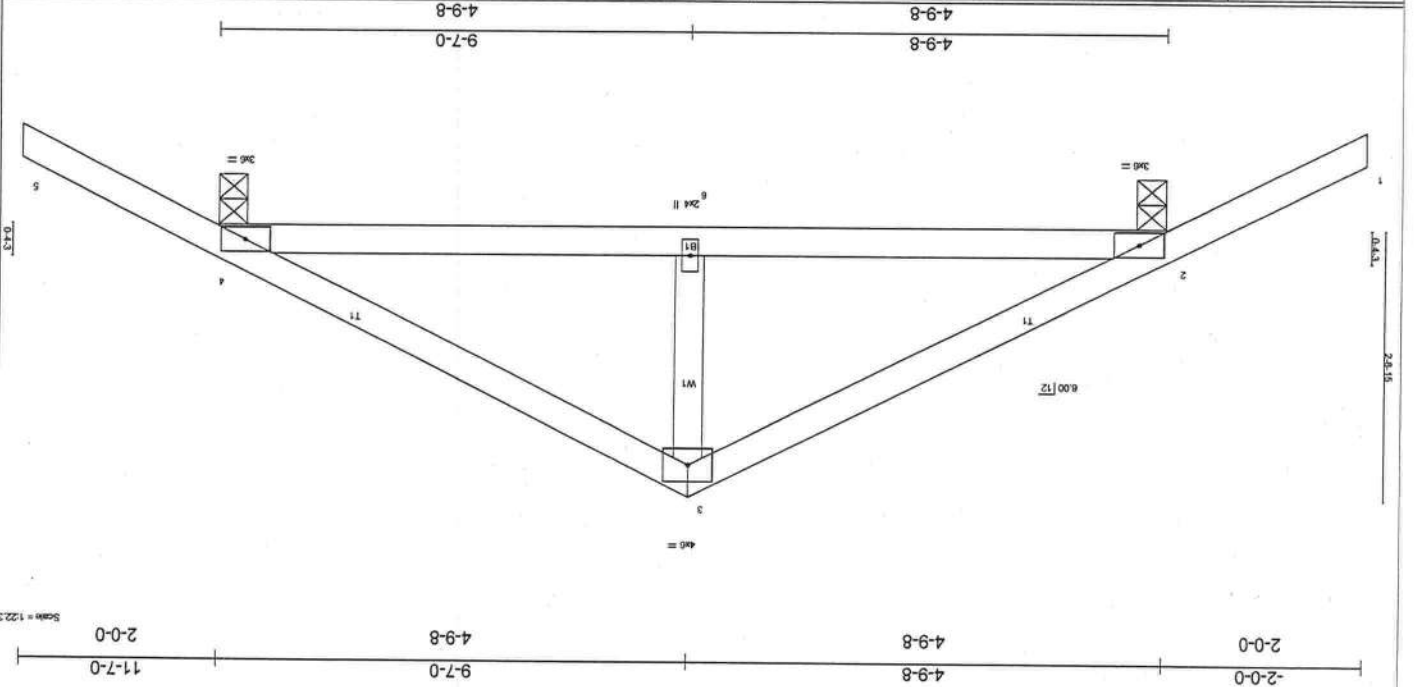
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 120mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extension(2) zone; porch left and right exposed; Lumber $DOL=1.60$ plate gnp $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 463 lb uplift at joint 2 and 463 lb uplift at joint 4.

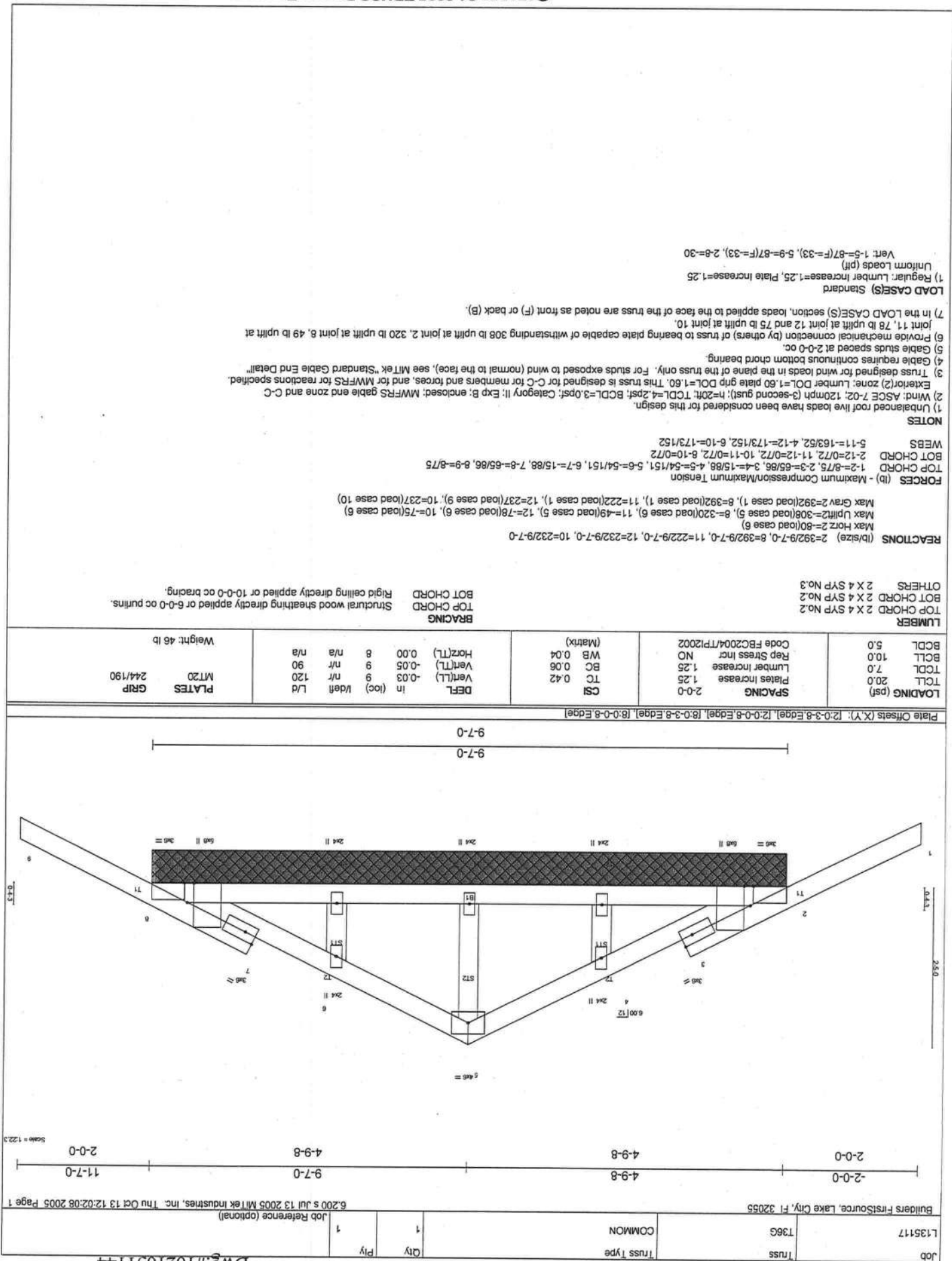
NOTES

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=489/664, 3-4=489/664, 4-5=0/47
 BOT CHORD 2-6=401/383, 4-6=401/383
 WEBS 3-6=310/148
REACTIONS (lb/size) 2=506/0-3-8, 4=506/0-3-8
 Max Horiz 2=85(load case 6)
 Max Uplift 2=463(load case 5), 4=463(load case 6)

LUMBER TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3
BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-6-6 oc bracing.

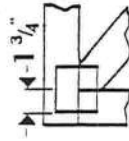
LOADING (psf)	SPACING	CSI	DEFL	VERT (LL)	VERT (TL)	HORIZ (TL)
TOLL 20.0	Plates Increase 1.25	TC 0.35	in (loc)	0.03	0.03	-0.01
TCDL 7.0	Lumber Increase 1.25	BC 0.13	in (loc)	2-6 >999	2-6 >999	4 n/a
BCLL 10.0	Rep Stress Incr YES	WB 0.05	L/d	180	180	n/a
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Weight: 40 lb			



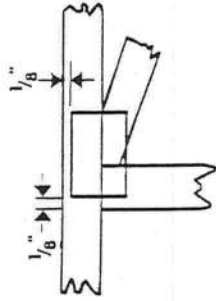


Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.



PLATE SIZE

4 X 4

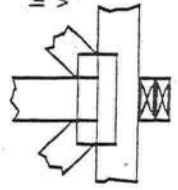
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



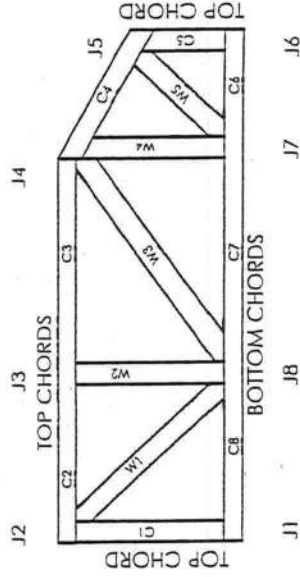
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR

**FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

- WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.
1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ----- 110 MPH
 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

- Applicant ☐ Plans Examiner ☐
- ☐ All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
 - ☐ Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.

Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC

- a. Basic wind speed (MPH)
- b. Wind importance factor (I) and building category
- c. Wind exposure - if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- d. The applicable internal pressure coefficient
- e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional

Elevations including:

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories

522
NITE

Floor Plan including:

- Rooms labeled and dimensioned
- Shear walls
- Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- Location of all load-bearing wall with required footings indicated as standard
- Or monolithic and dimensions and reinforcing
- All posts and/or column footing including size and reinforcing
- Any special support required by soil analysis such as piling
- Location of any vertical steel

Roof System:

- Truss layout and truss details signed and sealed by F.I. Pro. Eng.
- Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- Conventional Framing Layout including:

Wall Sections including:

- Masonry wall
- All materials making up wall
- Block size and mortar type with size and spacing of reinforcement
- Lintel, tie-beam sizes and reinforcement
- Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
- Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
- Fire resistant construction (if required)
- Fireproofing requirements
- Shoe type of termite treatment (termicide or alternative method)
- Slab on grade
- Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Must show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and supports
- Indicate where pressure treated wood will be placed
- Provide insulation R value for the following:
 - Attic space
 - Exterior wall cavity
 - Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
- a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
- b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

Floor Framing System:

- c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)
- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

- Electrical layout including:**
- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
 - b) Ceiling fans
 - c) Smoke detectors
 - d) Service panel and sub-panel size and location(s)
 - e) Meter location with type of service entrance (overhead or underground)
 - f) Appliances and HVAC equipment
 - g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom
- Energy Calculations** (dimensions shall match plans)
- Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done

Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.

(386) 758-1058 (Toilett facilities shall be provided for construction workers)

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**

A development permit will also be required. Development permit cost is \$50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE - TIME WILL NOT ALLOW THIS - PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE

COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR

AN APPOINTMENT TIME AND DATE:

YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE

TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY

EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

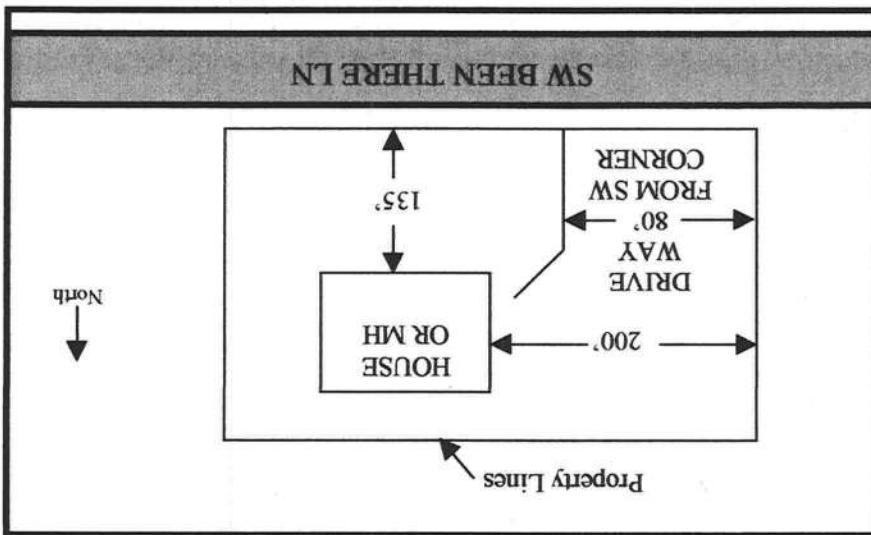
1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123") FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.

- a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).

- b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).

- c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Columbia County Building Permit Application

For Office Use Only	Application # 0510-93	Date Received 10/27/05	By G	Permit # 881/23827
Application Approved by - Zoning Official BSK		Date 03.11.05	Plans Examiner OK JTH	Date 11-4-05
Flood Zone	Development Permit	Zoning A-3	Land Use Plan Map Category A-3	Comments
STH n3e de d				

Applicants Name	Hugo Escalante	Address	6210 S.W. CR 18, Fort White, FL 32038
Owners Name	Jay B. Cashleen Hart	Address	675 S.W. Morning Star Glen, Ft White, FL 32038
Contractors Name	Hugo Escalante (EURL INC)	Phone	386-288-8666
Address	6210 S.W. CR 18, Fort White, FL 32038		
Fee Simple Owner Name & Address	None		
Bonding Co. Name & Address	None		
Architect/Engineer Name & Address	Donnel Sheehan, Lake City, Florida		
Mortgage Lenders Name & Address	None		

Circle the correct power company - FL Power & Light - <u>Clay Elec.</u> - Suwannee Valley Elec. - Progressive Energy	
Property ID Number	34-55-16-03752-109
Estimated Cost of Construction	180,000.00
Subdivision Name	Shanna Meadows
Lot	9
Block	9
Unit	
Phase	
Driving Directions	South on 47 from Lake City to S.W. Morningside Glen, make left 1/2 mile on left.
Type of Construction	New Single Family Residence
Number of Existing Dwellings on Property	0
Total Acreage	5 Acres
Do you need a - <u>Culvert Permit</u> or Culvert Waiver or Have an Existing Drive	
Actual Distance of Structure from Property Lines - Front	285'
Side	135'
Side	135'
Rear	285'
Total Building Height	29'-10"
Number of Stories	1
Heated Floor Area	2907 Sq Ft
Roof Pitch	6-12
Porches	888
Garage	471
TOTAL	3566

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

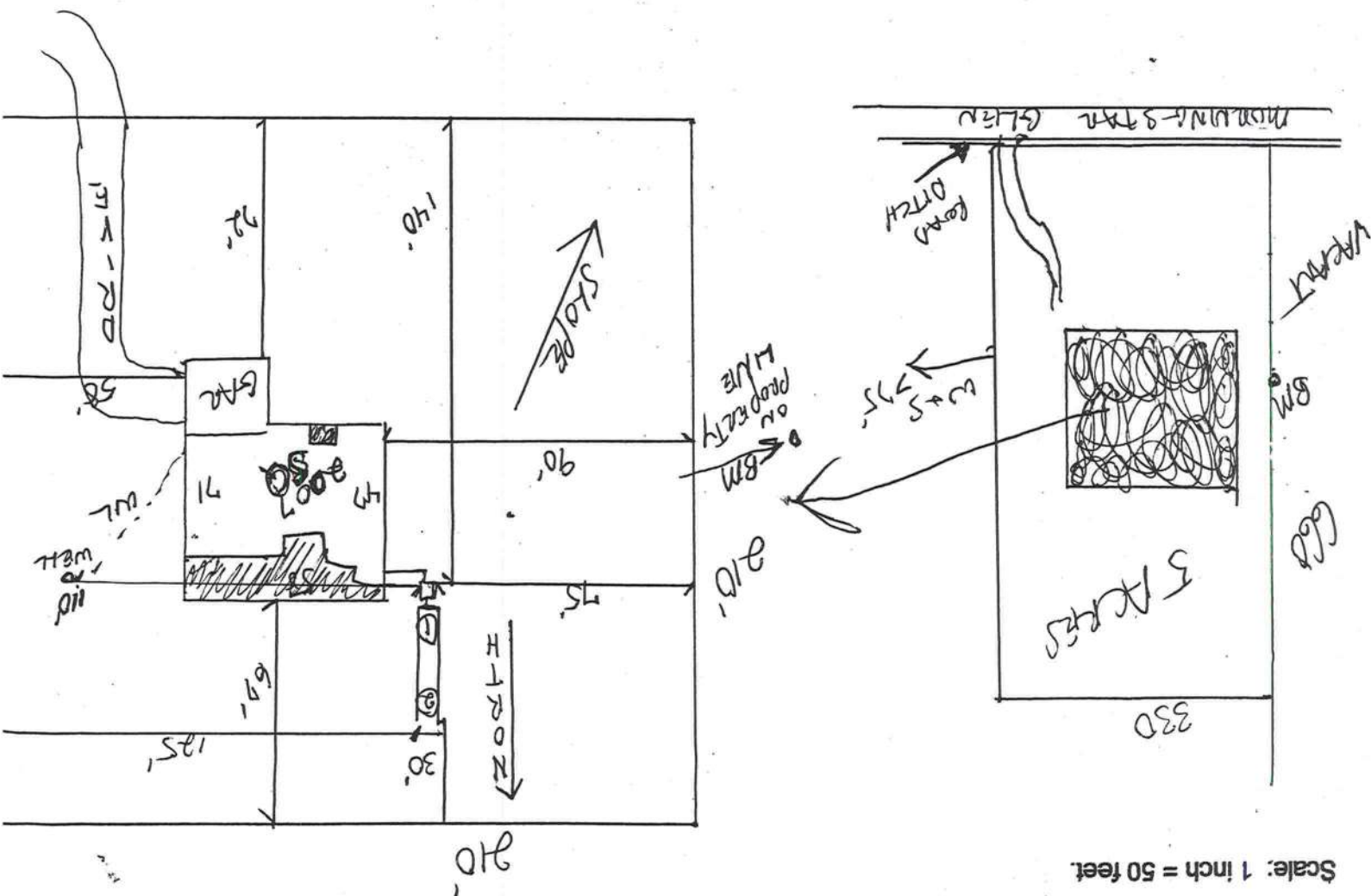
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner/Builder or Agent (Including Contractor)	Hugo Escalante
STATE OF FLORIDA	
COUNTY OF COLUMBIA	
Sworn to (or affirmed) and subscribed before me	
this 27th day of October	2005
Personally known ☒ or Produced Identification	
Notary Signature	Hugo Escalante
Notary Seal	
Contractor's License Number	CR1326967
Competency Card Number	
Notary Signature	

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Site Plan submitted by: Rock D. [Signature]
Plan Approved [Signature] Not Approved _____
By Mr. D. [Signature] Columb. Co.
County Health Department
Date 9-25-05
MASTER CONTRACTOR

Notes: 1 of 5 Marks



STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT
Permit Application Number 05-0

Permit Application Number.

COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 34-55-16-03759-109

1. Description of property: (legal description of the property and street address or 911 address)

Lod 9 Shanna Meadows S/D WD 1038-1855,
WD 1038-1856, WD 1045-2209, OC 1048-858

2. General description of improvement: New Single Family Residence

3. Owner Name & Address Jay Hard

Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner): N/A

5. Contractor Name Hugo Escobar Phone Number 386-288-8666

Address 6210 S.W. CR 18, Fort White, FL 32038

6. Surety Holders Name N/A Phone Number

Address N/A

Amount of Bond N/A

7. Lender Name N/A Phone Number

Address N/A

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Hugo Escobar Phone Number 386-288-8666

Address 6210 S.W. CR 18, Fort White, FL 32038

9. In addition to himself/herself the owner designates Hugo Escobar of

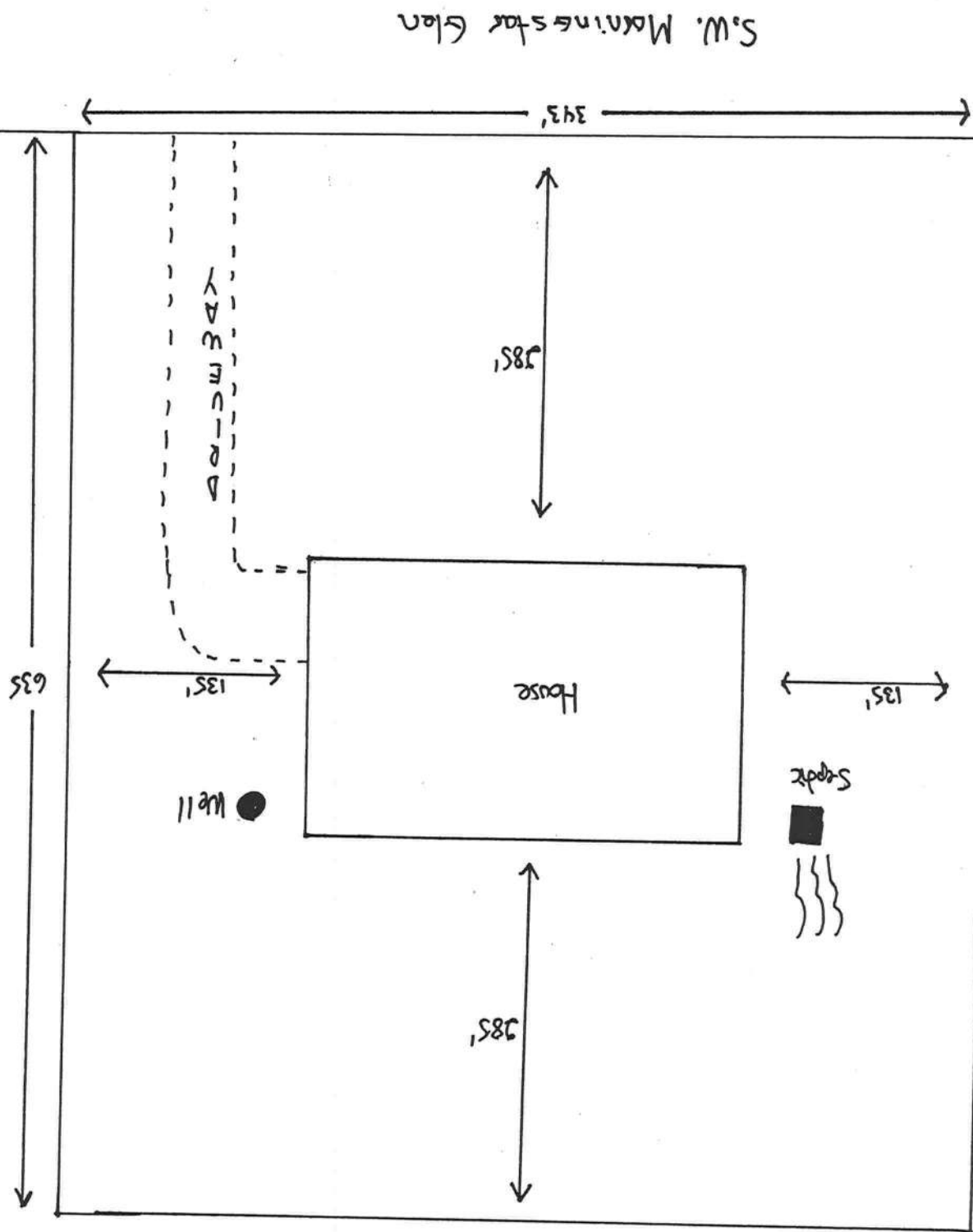
Fort White to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee 386-288-8666

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,

N

Lot 9 Shanna Meadows
Parcel # 34-55-16-03752-109



S.W. Main Street Glen

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
 PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: September 27, 2005

ENHANCED 9-1-1 ADDRESS:

675 SW MORNING STAR GLN (FORT WHITE, FL 32038)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 34-55-16-03752-109

Other Contact Phone Number (If any):

Building Permit Number (If known):

Remarks: LOT 9 SHANNA MEADOWS S/D

Address Issued By:

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
 9-1-1 ADDRESSING
 APPROVED

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave

Lake City, FL 32025

Phone 386-752-6677

Fax 386-752-1477

Gay Hart
Shanna Meadows

9

Building Permit #

Owner's Name

CWPA, Inc

Well Depth

Ft.

Casing Depth

Ft.

Water Level

Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Red Jacket Pump Model 150F31130511 HP 1 1/2

System Pressure (PSI) On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 80 (GPM)

Tank Installation: Bladder/Galvanized Make Challenger Model PG244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERIFY THAT THIS WATER WELL SYSTEM HAS BEEN
INSTALLED AS PER THE ABOVE INFORMATION.

Signature
Linda Newcomb

Print Name
Linda Newcomb

License Number
2609

Date

10-26-05

Columbia County Property

Appraiser

DB Last Updated: 8/3/2005

Parcel: 34-55-16-03752-109

Tax Record

Property Card

Interactive GIS Map

Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	HART JAY W & CATHLEEN SUE
Site Address	SHANNA MEADOWS
Mailing Address	19660 IVY DR SONORA, CA 95370
Brief Legal	LOT 9 SHANNA MEADOWS S/D, WD 1038-1855, WD 1038-1856, WD 1045-2209, QC 1048-858.

Use Desc. (code)	VACANT (000000)
Neighborhood	34516.00
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	5.000 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	\$24,000.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$24,000.00

Just Value	\$24,000.00
Class Value	\$0.00
Assessed Value	\$24,000.00
Exempt Value	\$0.00
Total Taxable Value	\$24,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
6/1/2005	1048/858	QC	V	U	01	\$100.00
4/1/2005	1045/2209	QC	V	U	01	\$100.00
2/17/2005	1038/1856	WD	V	Q		\$33,500.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (5.000AC)	1.00/1.00/1.00/1.00	\$24,000.00	\$24,000.00

Columbia County Property Appraiser

DB Last Updated: 8/3/2005

1 of 1

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0510-83

Hugo Escalate Owner Jay Hart Lot 9 of Shanna Meadows

On the date of November 2, 2005 application 0510-83 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0510-83 when making reference to this application.

1. Application 0510-83 which was filed with the building department on the date of October 27, 2005 will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Mark Disosway was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load design code requirements.

2. Please show compliance with sections R309 of the FRC-2004 R309.1 Opening

protection. Openings from a private garage directly into a room used for sleeping

purposes shall not be permitted. Other openings between the garage and residence shall

be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid

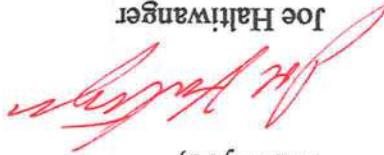
or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-

rated doors. R309.1.1 Duct penetration: Ducts in the garage and ducts penetrating the

walls or ceilings separating the dwelling from the garage shall be constructed of a

minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than 1/2-inch (12.7 mm) gypsum board or equivalent.

Thank you,



Joe Haltiwanger
Plan Examiner

Columbia County Building Department

**FLORIDA ENERGY EFFICIENCY CODE
FOR BUILDING CONSTRUCTION**

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	Hunt Residence - Lot 9 Shannon
Address:	Lot: 9, Sub: Shannon Meadows, Plat:
City, State:	Fort White, FL
Owner:	EWPL INC
Climate Zone:	North
Builder:	EWPL INC
Permitting Office:	Collier County
Permit Number:	2827
Jurisdiction Number:	221000

1. New construction or existing		2. Single family or multi-family		3. Number of units, if multi-family		4. Number of Bedrooms		5. Is this a worst case?		6. Conditioned floor area (ft ²)		7. Glass area & type		a. Clear - single pane		b. Clear - double pane		c. Tint/other SHGC - single pane		d. Tint/other SHGC - double pane		8. Floor types		a. Slab-On-Grade Edge Insulation		b. N/A		c. N/A		9. Wall types		a. Frame, Wood, Adjacent		b. Frame, Wood, Exterior		c. N/A		d. N/A		e. N/A		10. Ceiling types		a. Under Attic		b. N/A		c. N/A		11. Ducts		a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=128.0, 6.0 ft	
12. Cooling systems	a. Central Unit	b. N/A	c. N/A	13. Heating systems	a. Electric Heat Pump	b. N/A	c. N/A	14. Hot water systems	a. Electric Resistance	b. N/A	c. Conservation credits	(HR-Heat recovery, Solar DHP-Dedicated heat pump)	15. HVAC credits	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 36.0 kBtu/hr	SEER: 12.00	Cap: 36.0 kBtu/hr	HSPF: 6.80	Cap: 40.0 gallons	EF: 0.90	CF,																																		

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: 
DATE: 9-12-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

The Great Seal of the State of Florida is a circular emblem. It features a central shield depicting a landscape with a palm tree, a sun, and a body of water. Above the shield is a banner with the text "IN GOD WE TRUST". The outer ring of the seal contains the text "GREAT SEAL OF THE STATE OF FLORIDA" and is flanked by two stars.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; penetrations; between wall panels & top/bottom plates; between walls and floor.	
Floors	606.1.ABC.1.2.2	EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
		Penetrations/openings > 1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors; dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL, PERMIT #:

BASE		AS-BUILT					
WATER HEATING		As-Built Total:					
Number of Bedrooms		Bedrooms					
X Multiplier = Total		Number of X Tank X Multiplier X Credit = Total					
3		3					
2746.00		1.00					
8238.0		2684.98					
40.0		8054.9					
0.90		8054.9					

BASE		AS-BUILT			
Cooling Points + Heating Points + Hot Water Points = Total Points		Cooling Points + Heating Points + Hot Water Points = Total Points			
12911		8436			
11762		10760			
8238		8055			
32911		27251			

CODE COMPLIANCE STATUS

PASS



WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL, PERMIT #:

BASE					AS-BUILT				
INFILTRATION Area X BWPM = Points					Area X WPM = Points				
2207.0 -0.59 -1302.1					2207.0 -0.59 -1302.1				
Winter Base Points: 18747.1					Winter As-Built Points: 18743.7				
Total Winter X System = Heating Points					Total X Cap X Duct X System X Credit = Heating Points				
(DM x DSM x AHU)									
Component Ratio Multiplier Multiplier Multiplier					Component Ratio Multiplier Multiplier Multiplier				
18747.1 0.6274 11761.9					18743.7 1.00 1.145 0.501 1.000 10760.4				

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL, PERMIT #:

BASE										AS-BUILT									
GLASS TYPES										Type/SC									
.18 X Conditioned X BWPM = Points										Overhang									
Floor Area										Omt Len Hgt Area X WPM X WOF = Points									
.18										2207.0									
12.74										5061.1									
Double, Clear										S 1.5 7.5 36.0 4.03 1.06									
Double, Clear										S 10.5 6.7 13.3 4.03 3.33									
Double, Clear										S 10.0 4.0 9.3 4.03 3.61									
Double, Clear										E 1.5 5.5 30.0 9.09 1.04									
Double, Clear										N 1.5 5.5 17.5 14.30 1.00									
Double, Clear										N 9.0 7.0 60.0 14.30 1.02									
Double, Clear										N 1.5 6.5 42.0 14.30 1.00									
Double, Clear										W 1.5 6.5 21.3 10.77 1.02									
Double, Clear										N 1.5 5.5 30.0 14.30 1.00									
Double, Clear										W 1.5 5.5 20.0 10.77 1.03									
Double, Clear										W 1.5 5.0 16.0 10.77 1.03									
As-Built Total:										295.5									
Area X BWPM = Points										R-Value Area X WPM = Points									
Adjacent										Frame, Wood, Adjacent									
176.0										13.0 176.0 3.30									
3.60										580.8									
Frame, Wood, Exterior										13.0 1714.0 3.40									
1714.0										5827.6									
3.70										6341.8									
Exterior										As-Built Total:									
1890.0										1890.0									
Area X BWPM = Points										Area X WPM = Points									
DOOR TYPES										Type									
Area X BWPM = Points										Area X WPM = Points									
Adjacent										Exterior Wood									
36.0										40.0									
11.50										12.30									
414.0										492.0									
Exterior Wood										Exterior Wood									
12.30										18.0									
98.0										12.30									
1205.4										221.4									
Exterior										Adjacent Wood									
36.0										36.0									
11.50										11.50									
414.0										414.0									
Base Total:										As-Built Total:									
134.0										134.0									
Area X BWPM = Points										R-Value Area X WCM = Points									
CEILING TYPES										Type									
Area X BWPM = Points										Area X WPM = Points									
Under Attic										Under Attic									
2207.0										30.0 2207.0 2.05 X 1.00									
2.05										4524.4									
4524.4										As-Built Total:									
2207.0										2207.0									
Area X BWPM = Points										R-Value Area X WPM = Points									
FLOOR TYPES										Type									
Area X BWPM = Points										Area X WPM = Points									
Slab										Slab-On-Grade Edge Insulation									
210.0(p)										0.0 210.0(p)									
8.9										18.80									
1869.0										3948.0									
0.0										As-Built Total:									
1869.0										210.0									
Base Total:										3948.0									

SUMMER CALCULATIONS
Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL,
PERMIT #:

BASE					AS-BUILT				
INFILTRATION Area X BSPM = Points					Area X SPM = Points				
2207.0 10.21 22533.5					2207.0 10.21 22533.5				
Summer Base Points: 30263.9					Summer As-Built Points: 27929.3				
Total Summer X System = Cooling Points					Total X Cap X Duct X System X Credit = Cooling Points				
Multiplier					Ratio Multiplier Multiplier Multiplier Multiplier				
Points					(DM x DSM x AHU)				
12910.6					27929.3				
0.4266					1.000 (1.071 x 1.147 x 0.91) 0.284				
30263.9					27929.3				
1.118					0.950				
0.284					0.950				
0.950					8435.9				
8435.9					8435.9				

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL,

PERMIT #:

BASE										AS-BUILT									
GLASS TYPES																			
.18 X Conditioned X BSPM = Points										Overhang									
Floor Area										Type/SC									
.18										Omt Len Hgt Area X SPM X SOF = Points									
2207.0										295.5									
20.04										As-Built Total:									
7961.1										Double, Clear									
										S									
										1.5									
										7.5									
										36.0									
										34.50									
										0.91									
										1130.0									
										Double, Clear									
										S									
										10.5									
										6.7									
										13.3									
										34.50									
										0.47									
										215.4									
										Double, Clear									
										S									
										10.0									
										4.0									
										9.3									
										34.50									
										0.44									
										141.1									
										Double, Clear									
										E									
										1.5									
										5.5									
										30.0									
										19.22									
										0.93									
										731.5									
										Double, Clear									
										W									
										1.5									
										6.5									
										21.3									
										36.99									
										0.93									
										535.2									
										Double, Clear									
										N									
										1.5									
										30.0									
										19.22									
										0.90									
										663.5									
										Double, Clear									
										W									
										1.5									
										5.5									
										20.0									
										36.99									
										0.88									
										518.1									
										As-Built Total:									
										295.5									
										R-Value									
										Area X SPM = Points									
WALL TYPES										Type									
Area X BSPM = Points										R-Value									
Base Total:										As-Built Total:									
Adjacent										Frame, Wood, Adjacent									
176.0										13.0									
0.70										176.0									
123.2										0.60									
105.6										2571.0									
Exterior										Frame, Wood, Exterior									
1714.0										13.0									
1.70										1714.0									
2913.8										1.50									
3037.0										1890.0									
Area X BSPM = Points										Area X SPM = Points									
Door TYPES										Type									
Adjacent										Exterior Wood									
36.0										40.0									
2.40										6.10									
86.4										244.0									
597.8										109.8									
Exterior										Exterior Wood									
98.0										18.0									
6.10										6.10									
684.2										244.0									
134.0										40.0									
684.2										86.4									
										24.0									
										6.10									
										597.8									
										109.8									
										244.0									
										6.10									
										40.0									
										18.0									
										36.0									
										2.40									
										6.10									
										40.0									
										134.0									
										684.2									
CEILING TYPES										Type									
Area X BSPM = Points										R-Value									
Under Attic										Under Attic									
2207.0										30.0									
1.73										2207.0									
3818.1										1.73 X 1.00									
3818.1										3818.1									
Base Total:										As-Built Total:									
2207.0										2207.0									
Area X BSPM = Points										Area X SPM = Points									
FLOOR TYPES										Type									
Slab										Slab-On-Grade Edge Insulation									
210.0(p)										0.0									
-37.0										210.0(p)									
-7770.0										-41.20									
0.0										-8652.0									
										-8652.0									
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										-865									

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.7
The higher the score, the more efficient the home.

EWPL INC, Lot: 9, Sub: Shannon Meadows, Plat: , Fort White, FL,

1. New construction or existing
2. Single family or multi-family
3. Number of units, if multi-family
4. Number of Bedrooms
5. Is this a worst case?
6. Conditioned floor area (ft²)
7. Glass area & type
8. a. Clear - single pane
b. Clear - double pane
c. Tint/other SHGC - single pane
d. Tint/other SHGC - double pane
9. Floor types
- a. Slab-On-Grade Edge Insulation
- b. N/A
- c. N/A
10. Wall types
- a. Frame, Wood, Adjacent
- b. Frame, Wood, Exterior
- c. N/A
- d. N/A
- e. N/A
11. Ceiling types
- a. Under Attic
- b. N/A
- c. N/A
- d. N/A
- e. Sup: Unc. Ret: Unc. AH: Interior
- b. N/A

- [illegible]

based on installed Code compliant features.

Date:

City/FL Zip:



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs, Energy Code (800) version: FLRCPB v3.2)

contact the Department of Community Affairs, Emergency Management Division, at (800) 455-6824 or (904) 498-1234. For more information, visit www.flrcpb.com.

Columbia County Building Department

Culvert Permit

Culvert Permit No. 000000881

DATE 11/07/2005 PARCEL ID # 34-5S-16-03725-109

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER JAY & CATHLEEN HART PHONE 209-743-0434

ADDRESS 675 SW MORNING STAR GLEN FORT WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386-288-8666

LOCATION OF PROPERTY S 47, L SW MORNING STAR GLEN, 1/2 MILE ON THE LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT SHANNA MEADOWS 9

SIGNATURE

INSTALLATION REQUIREMENTS

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concrete turnouts.

Culvert installation shall conform to the approved site plan standards.

Department of Transportation Permit installation approved standards.

Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-5S-16-03752-109

Building permit No. 000023827

Use Classification SFD, UTILITY

Fire: 23.68

Permit Holder HUGO ESCALANTE

Waste: 49.00

Owner of Building JAY & CATHLEEN HART

Total: 72.68

Location: 675 SW MORNING STAR GLEN

Date: 06/12/2006

Harry Dickson Jr.
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



0-8

ROOF
PITCH

6/12

OVERHANG

1'-4"

HANGER SCHEDULE
(48) P.B. PLATE;

1) REFER TO HD 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING REQUIRED. REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

- 2) ALL TH055655 (INCLUDING TH055555 VALLEY FRAMING) MUST BE CONSIDERED DECAYED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS,
- 3) ALL VALLEY'S ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TH055655 ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON A FLOOR PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) S/442 TH05555 MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TH05555 HANKEYS TO BE SAMPSON H5526 UNLESS OTHERWISE NOTED. ALL FLOOR TH05555 HANKEYS TO BE SAMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BEYOND ABOVE FLOOR, ROOF TO BE FURNISHED BY BUILDER.

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION TRUSSES AND VOIDS. ALL PREVIOUS ARCHITECTURAL OR ENGINEERING TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VENDOR CONDITIONS TO INSURE AGAINST CHANGES THAT WILL BE IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____



Dunhill
PHONE: 904-437-3349 FAX: 904-437-

Jacksonville

Lake City

Confidential

PHONE: 401-322-0039 FAX: 401-322-

PHYSIOLOGICAL ANTI-

COLIMARIA CC

CELL: (REVISION:

DATE:	10/1/01
DEAN BY:	10/1/01

012102 | 61.0111 | 5122

Notice of Treatment

11802

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: BAYA AVE

City: LAKE CITY Phone: 7521703

Site Location: Subdivision Shannon

Lot # 9 Block# Permit # 23827

Address 675 SW Morning Star GLN

Product used

Active Ingredient

% Concentration

☐ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☒ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☐ Soil

☒ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

Dwelling

3566

822

5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

2/2/06

Date

1215

Time

F254

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

