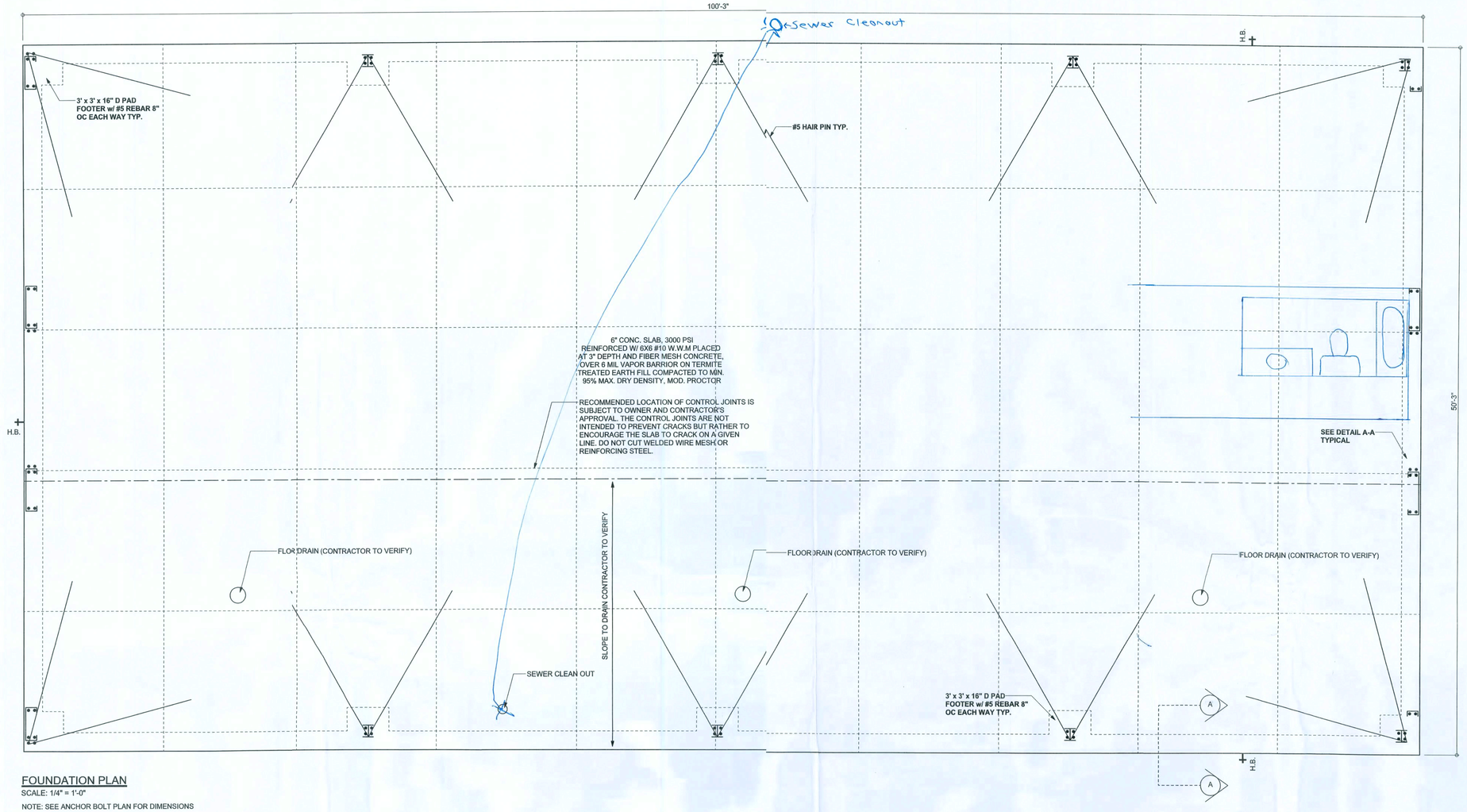


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

NOTE: SEE ANCHOR BOLT PLAN FOR DIMENSIONS

SITE PREPARATION: SITE ANALYSIS AND PREPAR/ION ARE NOT PART OF THIS PLAN AND ARE RESPONSIBILITY OF THE OWNER. SITE INSPECTION BY BUILDER OR BUILDING OFFICIAL SHALL DETERMIR IF THERE IS ANY EVIDENCE OF UNSUITABLE BEARING MATERIALS. IF THERE IS ANY QUESTION, CALL A GEOTECHNICAL ENGINEER TO ASSURE ThT EXPANDING CLAYS AND OTHER PROBLEMATIC SOIL CONDITIONS DO NOT EXIST OR TO ALLOW MITIGATION SHOULD THEY EXIST. ALL FILL UNDERSTRUCTURAL ELEMENTS SHALL BE CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED TO 95% OF MAXIMUM DF BEARING CAPACITY, IN LIFTS OF NOT MORE THAN 6 INCHES. IT IS THE OWNER'S/BUILDER'S RESPONSIBILITY TO VERIFY EXISTING SOIL AND CEAN FILL ARE COMPACTED STABLE SOIL CONDITIONS WITH 1000 PSF BEARING CAPACITY OR TO REQUEST FOUNDATION DESIGN BASED ON ACTU. SITE CONDITIONS.

FOUNDATION:THE OWNER HAS NOT YET PROVIDE A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 2000 PSF SHALL BE APPROVED BY THE OWNER. FOOTINGSND SLAB ARE TO BEAR ON FIRM UNDISTURBED EARTH OR CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED ILIFTS OF NOT MORE THAN 6 INCHES. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. FOUNDATION WORK MUST BE COORDINATED WITH UNDERGROUND UTILITIES. FOOTINGS SHALL BE LOWERED WHERE REQUIRED TO AVOIUTILITIES. TO MINIMIZE WEATHERING, THE LAST 6 INCHES OF EXCAVATION FOR ALL FOOTINGS SHALL BE MADE IMMEDIATELY PRIOR TO PLACEMNT OF FOOTINGS.

CONCRETE:MINIMUM COMPRESSIVE STRENGTH C CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICEABILITY IS DEGADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OFCONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER, OR HIS REPRESENTATIVE.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GRUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE ½ INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL E FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL OMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUBTED BY THE BUILDING OFFICIAL.

WELDED WIRE REINFORCED SLAB:6"x6" W1.4xW1. FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN THE MIDDLE OF THE SLAB: SUPPORTD WITH APPROVED MATERIALS OR SUPPORTS AT SPACING NOT TO EXCEED 3'.

REBAR: ASTM A 615, GRADE 60, REINFORCED BAR FY = 60 KSI. ALL LAP SPLICES 40" DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH AC315-95 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 INCHES.

CONCRETE CONTROL JOINTS: WHERE SPECIFIED;AWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302.JOINTS SHALL BE CUT WITH IN 12 HOURS OF SLAB PLACEENT. LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT \. DO NOT CUT W.W.M. OR RNFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

16" x 16" x 25' x 100 = 4444
6' x 6" x 25' x 100 = 7500

VERT. 8.60 + 3.2 = 11.8 (K)

BASED ON COLUMN REACTIONS BY WHIRLWIND STEEL BUILDINGS
SEALED ENGINEERING FOR PROJECT 14180, DATED 1/15/2008

M=48013
.4410 x 3 =1.323
1.323 x 60,000 x .6 x 1.083 =51581

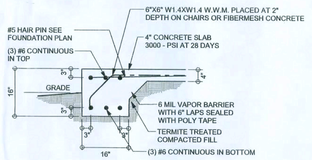
NOTE: THIS FOUNDATION DESIGN MEETS ALL REQUIREMENTS FOR WIND LOAD'S PER FBC2004, SECTION 1609, 110 MPH BASIC WIND SPEED, EXPOSURE B, 1.0 USE FACTOR; COLUMN PAD LOCATIONS ARE TYPICAL, EXACT ANCHOR BOLT LOCATIONS AND SIZES ARE PER METAL BUILDING SEALED ENGINEERING ANCHOR BOLT PLAN.

- ANCHOR BOLTS AND REINFORCEMENT - 16" A-307 ANCHOR BOLTS, BOLT DIAMETER, AND LOCATION PER METAL BUILDING SEALED ENGINEERING DESIGN DRAWINGS. TIE ANCHOR BOLTS TO BOTTOM REINFORCING STEEL. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI DETAILING MANUAL, SP-66, AND ACI318. REINFORCING SHALL NOT BE HEATED OR WELDED. REINFORCING SHALL BE APPROVED BY ENGINEER OR HIS REPRESENTATIVE BEFORE CONCRETE IS PLACED. PROVIDE 3" COVER FOR EXPOSED FOOTING SURFACES, 2" COVER FOR FORMED EXPOSED SURFACES, 3/4" COVER FOR NOT EXPOSED SURFACES. LAP SPLICES SHALL BE 48 BAR DIAMETERS. TOP STEEL LAPS SHALL OCCUR AT MID SPAN; BOTTOM LAPSAT COLUMNS.
- CONCRETE - MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

- CONTROL JOINTS - SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

- FOUNDATION - THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 2000 PSF SHALL BE CONFIRMED IN THE FIELD BY A REGISTERED GEOTECHNICAL ENGINEER OR SHALL BE APPROVED BY THE OWNER. FOOTINGS AND SLABS ARE TO BEAR ON FIRM UNDISTURBED EARTH OR APPROVED CONTROLLED FILL. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL.

- UNLESS OTHERWISE SPECIFIED ALL MATERIALS AND CONSTRUCTION ARE TO MEET LOCAL BUILDING CODES.



A/A MONOLITHIC FOOTING

NTS SCALE: 1/2" = 1'-0"

FILE COPY

FILE COPY

REVISIONS	
7/25/2017	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB868, Lake City, FL 32056, 386-754-549

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer al questions to Mark Disosway, P.E for resolution. Do not proceed without clarification.

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Mark Disosway, P.E hereby expressly reserves its common law coprights and property right in these instruments oservice. This document is not to be reproduces, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I ereby certify that I have examined this plan, ind that the applicable portions of the plan, vlelting to wind engineering comply with section1609, florida building code 2004, to the best of ny knowledge.

LIMITATION: This design is valid for one building, at specifiedlocation.

MARK DISOSWAY
P.E. 53915

18 JAN 08

SEAL

Sparks Construction

Taylor, ohn & Julie
Meta Building
Founcation Plan

ADDRESS:
Columbia County, Florida

Mark Disosway P.E.
P.O.Box 868
Lake City,Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 754 - 6749
Email: mdpe@bellsouth.net

PRINTED DATE: Januay 18, 2008

DRAWN BY: Ben Sparks

CHECKED BY:

FINALS DATE: 01/ Jan / 08

JOB NUMBER: 801162

DRAWING NUMBER F1

OF · SHEET

GENERAL NOTES

1. Design and/or fabrication shall be in accordance with Whirlwind's standard practices and interpretations of the following codes:
   Ninth Edition AISC Steel Construction Manual ASD,
   2001 North American Specification For The Design Of Cold-Formed Steel Structural Members,
   2004 AWS D1.1 Structural Welding Code-Steel and any other code listed within the Design Information.

2. Materials

	ASTM Designation	Minimum Yield and Tensile
Hot Rolled Mill Shapes	A992 or A572	Fy= 50 ksi, Fu= 65 ksi
Steel Plate/Flat Bar	A572 or A529	Fy= 50 ksi, Fu= 60 ksi
Structural Steel Strip	A1011 or A529	Fy= 50 ksi, Fu= 60 ksi
Structural Steel Sheet	A1011, A529 or A572	Fy= 50 ksi, Fu= 60 ksi
Cold-Formed Light Gauge Shapes	A1011	Fy= 57 ksi, Fu= 65 ksi
Cable Bracing	A475	Extra High Strength
Round Bar Bracing	A36	Fy= 36 ksi, Fu= 58 ksi
Roof and Wall Cladding	A792	Fy= 50, 60 ksi, Fu= 65 ksi
Machine Bolts	A307	
High Strength Bolts	A325	
Anchor Rods (not by Whirlwind)	A307	

3. Finish:

a. All cold-formed structural framing members shall be shot blasted, given one coat (0.5 mils) of premium polyester-based red primer, then oven baked prior to any fabrication. The primer contains a "wax" type lubricant to facilitate roll forming and deter marring during these operations. Hair line crazing which may occur during forming operations is considered normal.

b. All other structural framing members shall be cleaned in accordance with the AISC Code of Standard Practice and given one shop coat (1.0 mils) of Whirlwind's standard red-oxide primer designed for short term field protection during the erection process. Whirlwind's standard red-oxide primer meets the performance specifications of Federal Specification TT-P-636D and TT-P-664.

4. All bolts are ½" x 0'-1¼" A307 except as noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.

5. A325 Bolt Tightening Requirements
   All high strength bolts are A325 unless specifically noted otherwise. All structural A325 bolts with heavy hex nuts for the Rigid Frame are to be installed using the turn-of-the-nut method specified in the "Specification for Structural Joints Using ASTM A325 or A490 Bolts" in the AISC Manual. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.

   Closure Strips Furnished for Roof Applications
6. INSIDE – under roof panels at eave
   OUTSIDE – between endwall panels and rake trim and at the high side eave trim

ERECTION NOTES

1. All bracing shown and provided by Whirlwind for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.9).

2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.9).

3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, or cutting and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to Whirlwind by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.12).

4. Erection tolerances are set forth in AISC Code of Standard Practice 7.11 except that individual members are considered plumb, level and aligned if the deviation does not exceed 1:300. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.

   When crane support systems are part of the metal building system erection tolerances Section 9, Common Industry Practices, 1996 MBMA Low Rise Building Systems Manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.

5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met:

   a. Welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code – Steel or AWS D1.3 Structural Welding Code – Sheet Steel as applicable, for the processes, positions, and materials involved.

   b. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.

   All documentation and records shall be the responsibility of the customer.

6. Any claims or shortages by buyer must be made to Whirlwind within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to Whirlwind Steel Buildings Customer Service Department.

7. Claims for correction of alleged misfits will be disallowed unless Whirlwind shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from Whirlwind.

8. Neither Whirlwind nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.13).

RESPONSIBILITIES

1. The Whirlwind Customer, hereafter referred to as the "customer", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. It is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.

2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system.

   It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to Whirlwind.

   It is the responsibility of Whirlwind, through Whirlwind's Engineer, to design the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. Whirlwind is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.

   Whirlwind is responsible only for the structural design of the metal building system it provides. Whirlwind or Whirlwind's Engineer is not the Design Professional or Engineer of Record for the Construction Project. Whirlwind is not responsible for the design of any components or materials not provided by it or their interface and connection with the metal building system.

3. Whirlwind Steel Buildings' standard specifications apply unless stipulated otherwise in the Contract Documents. Whirlwind design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary notwithstanding. It is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans and/or specifications.

4. In case of discrepancies between Whirlwind's structural steel plans and plans for other trades, Whirlwind's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 3.3)

5. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by Whirlwind and Whirlwind's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or Whirlwind's assumptions will govern.

6. Anchor bolts and foundation bolts are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5.1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual.

   All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.

7. Once the Contractor has signed Whirlwind's Approval Package and the project is released for fabrication, changes shall be billed to the Contractor including material, engineering, and other costs. An additional fee may be charged if the project must be moved from fabrication and shipping schedule.

8. Whirlwind does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of he presence of the metal building system.

APPROVAL NOTES

1. It is imperative that any changes to these drawings:
   A) Be made in contrasting ink
   B) Be legible and unambiguous
   C) Have all instances of change clearly indicated.

2. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Whirlwind customer.

3. Whirlwind reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.

4. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between Whirlwind and its customer are not binding on Whirlwind unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.

5. Approval of Whirlwind drawings and/or calculations indicate that Whirlwind has correctly interpreted the contract requirements. This approval constitutes the customer acceptance of the Whirlwind design, concepts, assumptions, and loadings. Once the Whirlwind customer has signed and returned the Whirlwind drawings and/or calculations and the project is released for fabrication, changes shall be billed to the Whirlwind customer including material, engineering, and other cost. An additional fee may be charged if the project must be moved from the fabrication and/or the shipping schedule.

6. The customer approves of all notes and conditions on the drawings and/or calculations by signing an Approval Drawing Waiver Form.

DRAWING SCHEDULE

DRAWING NUMBER	ISSUE	DATE	DESCRIPTION
C1	1	01.21.08	COVER SHEET
E1	1	01.21.08	ROOF FRAMING PLAN
E2	1	01.21.08	ELEVATION FRAMING
E3	1	01.21.08	ELEVATION FRAMING
E4	1	01.21.08	ELEVATION FRAMING
D1	1	01.21.08	DETAIL PAGE
D2	1	01.21.08	DETAIL PAGE
P1	1	01.21.08	RIGID FRAME ELEVATION
F1	1	01.21.08	ANCHOR BOLT PLAN
F2	1	01.21.08	REACTIONS SHEET

ERECTOR NOTE
ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

0801-121

STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

FEB 14 2008

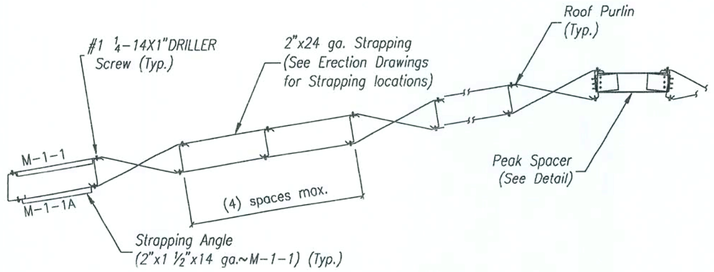
WHIRLWIND STEEL BUILDINGS
P.O. BOX 75280
HOUSTON, TX 77234
PH: 800-324-9992
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

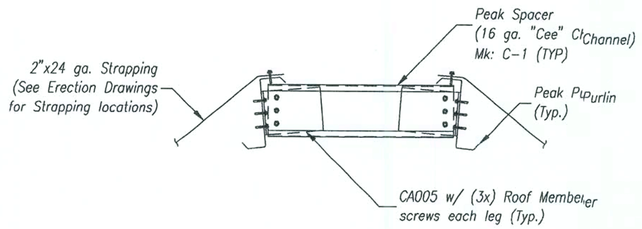
DESC: COVER SHEET
BLDG. SIZE: 50'-0" x 100'-0" x 16'-0"
CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32056
REFERENCE: TAYLOR BUILDING
JOBSITE: LAKE CITY, FL. 32056
COUNTY: COLUMBIA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
MBS	.	1/ 8/08	TM	14180	C1	1

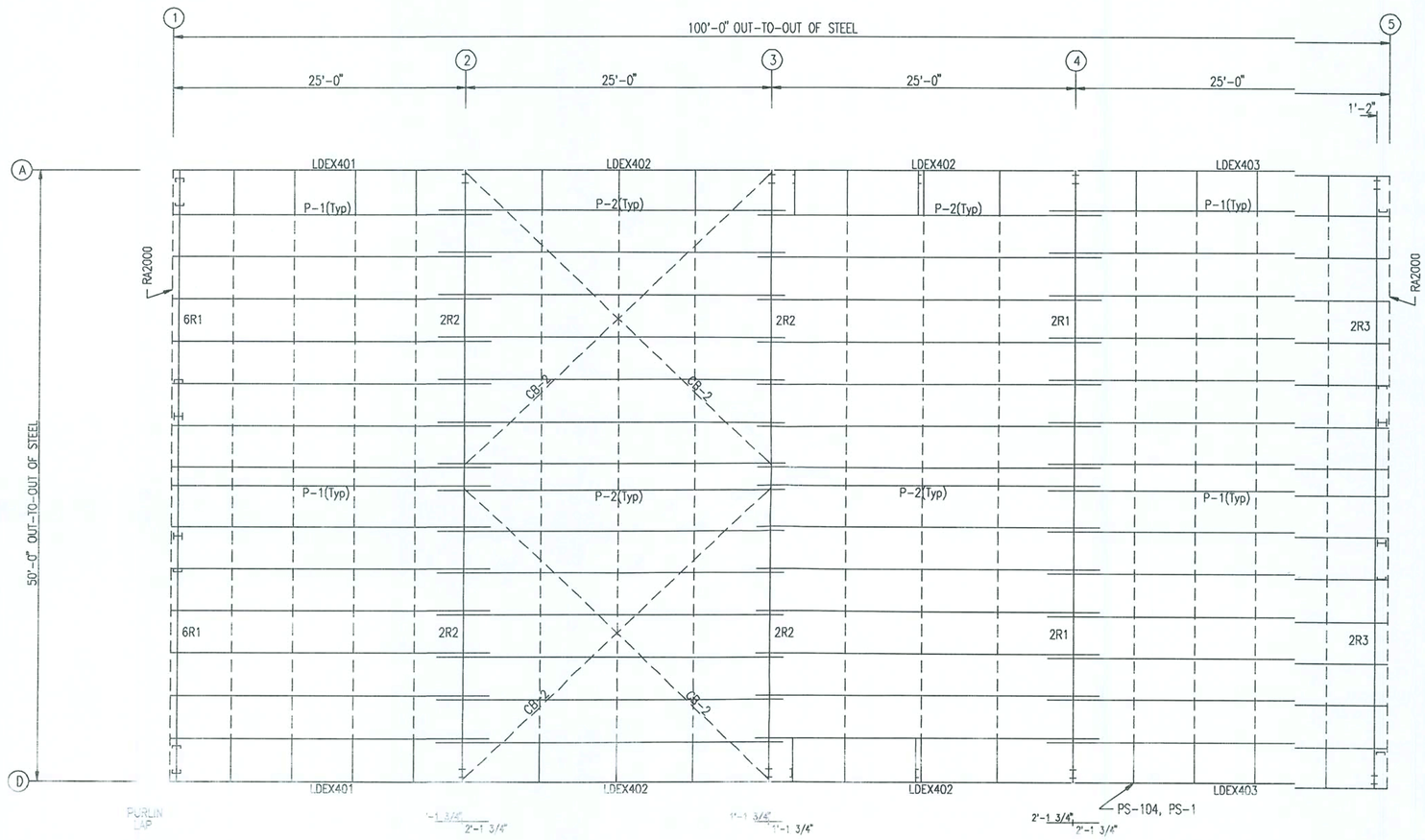
MEMBER TABLE ROOF PLAN	
MARK	PART
P-1	8X25Z14
P-2	8X25Z14
C-1	6X25C16
LDEX401	8E14
LDEX402	8E14
LDEX403	8E14
CB-2	0.25_CBL



ROOF STRAPPING DETAIL
(Standing Seam Roof)



PEAK SPACER ATTACHMENT DETAIL



ROOF FRAMING PLAN

NOTE:
GUTTER & DOWNSPOUTS ARE BY OTHERS

DESIGN INFORMATION					
BUILDING CODE:	FBC 04	Ground/Roof Snow Load:	0.00	SNOW IMPORTANCE:	N/A
DEAD LOAD:	Metal Building Only	SNOW EXPOSURE:	N/A	SNOW THERMAL:	N/A
COLLATERAL LOAD: (psf)	2	SEISMIC USE GROUP:	I	SEISMIC IMPORTANCE :	.
SPRINKLER LOAD: (psf)	0	SPECTRAL RESP. ACCEL Ss:	12.2	SPECTRAL RESP. ACCEL S1:	5.4
ROOF LIVE LOAD: (psf)	20	SPECTRAL RESP. SDs:	.	SPECTRAL RESP. SD1:	.
FRAME LIVE LOAD:	Reducible	SITE CLASS:	D	SEISMIC DESIGN CATEGORY:	.
WIND LOAD: (mph)	110	DIRECTION:	LATERAL / LONGITUDINAL		
WIND IMPORTANCE:	1.00	STRUCTURAL SYSTEM: STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.			
WIND EXPOSURE:	B	RESPONSE MODIFICATION (R):	.	.	
WIND COND.:	Enclosed	DEFLECTION AMPLIFICATION (Cd):	.	.	NO SKYLIGHT
INTERNAL PRES. COEFF.:	+/-0.18	SEISMIC RESPONSE COEFF. CS:	.	.	
EDGE ZONE WIDTH: (feet)	5.00	DESING BASE SHEAR V:	.		NO VENTILATORS
		ANALYSIS:	EQUIVALENT LATERAL FORCE PROCEDURE		

24GA ROOF – SUPER SEAM PLUS – GALVALUME PLU
26GA WALL – SUPER SPAN X – SAHARA TAN
26GA TRIM – EVERGREEN
GUTTER & DOWNSPOUTS – BY OTHERS
UL580, CLASS 90 ROOF – CONST. No. 538

WALKDOORS AND WINDOWS				FRAMED OPENINGS				
MK.	QTY.	FINISH	DESCRIPTION	MK.	QTY.	WIDTH	HEIGHT	OPENING FOR:
A	2	WHITE	3070 (M) WALDOOR W/ KEY ALIKE, DEAD BOLT & WEATHERSTRIP	B	4	14'-0"	14'-0"	RUD
				C	2	10'-0	10'-0"	RUD

WHIRLWIND STEEL BUILDINGS

P.O. BOX 75280
HOUSTON, TX 77234

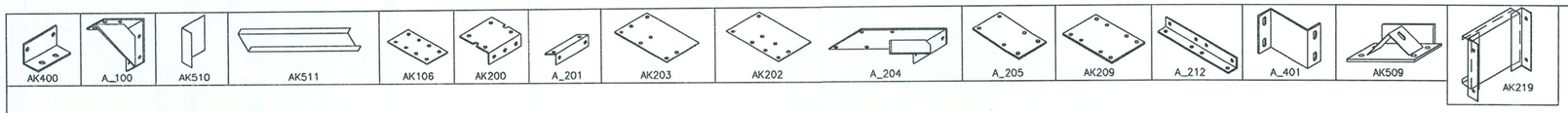
PH: 800-324-9992
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
1	01.21.08	FOR ERECTION	SLV	VA

DESC: ROOF FRAMING
CUSTOMER:
REFERENCE:
JOBSITE: LAKE CITY,
DWR: | CHK: | DATE: 1/ 8/08 | ENG:

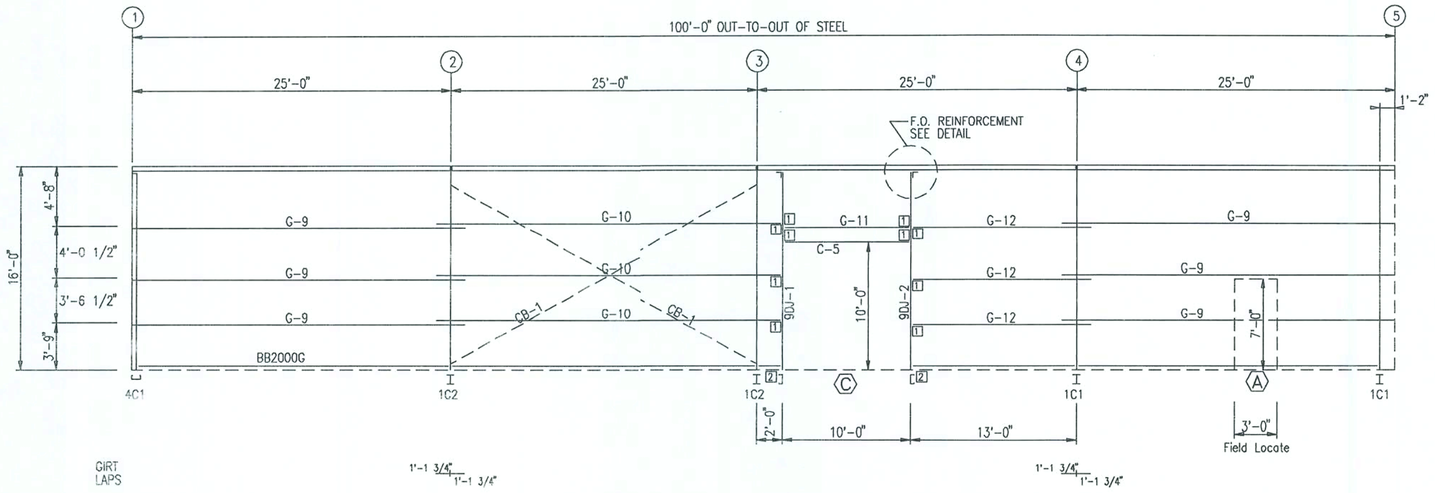
STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

FEB 14 2008

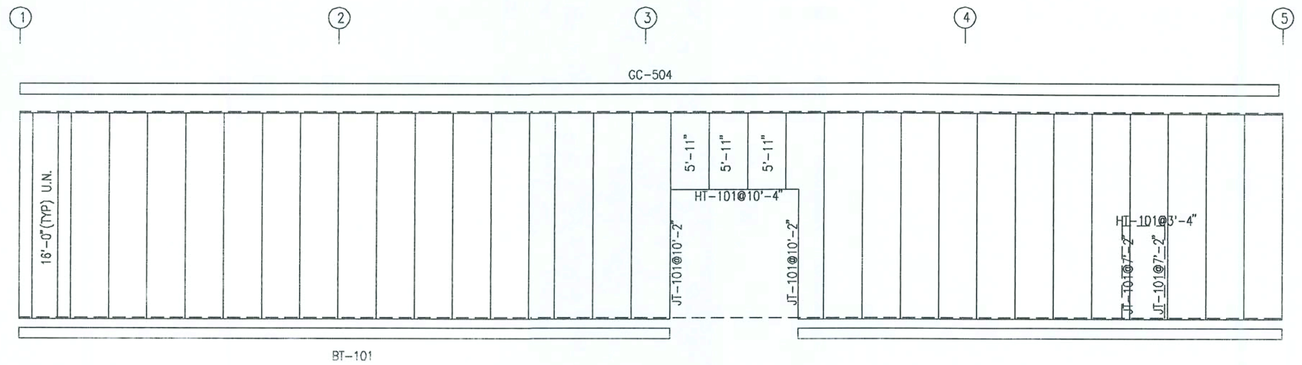


CONNECTION PLATES FRAME LINE D	
☐ ID	MARK/PART
1	AK200
2	AK400

MEMBER TABLE FRAME LINE D	
MARK	PART
9DJ-1	8X25C14
9DJ-2	8X25C14
C-5	8X25C16
G-9	8X35Z12
G-10	8X35Z12
G-11	8X35Z16
G-12	8X35Z16
CB-1	0.31_CBL

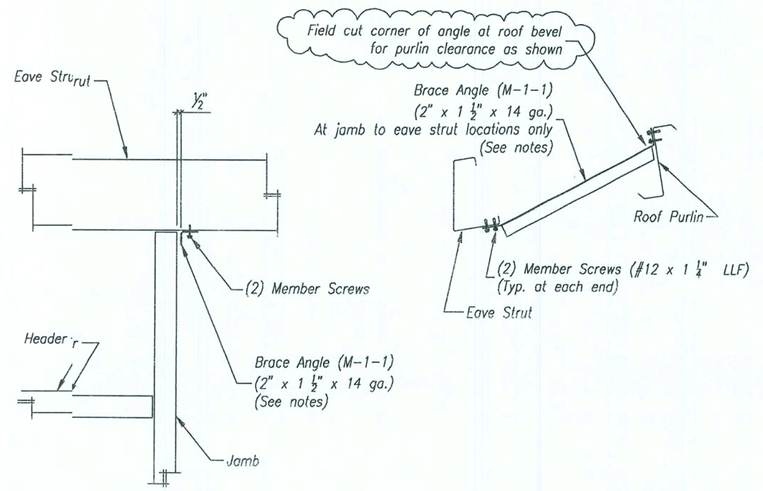


SIDEWALL FRAMING: FRAME LINE D



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Ga. SSX - SAHARA TAN



FRAMED OPENING REINFORCEMENT

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 2'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'-0 o.c. maximum, larger downspouts 5'-0 o.c maximum.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as requir to fit.
3. Flange braces are marked by the length in decimal inches.
4. Outside flange of girt turns downless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at w doors.
7. Field slot girts for brace rods orables.
8. Field locate windows and walk do.
9. Field weld all splices at 14 gauge alley gutters.
10. Field bolt AK400 base clip to each columns:
    (2) 5/8" x 1-1/2" A325 bolts (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts (2) AK400 req'd
11. Locate top of roof framed opens flush with the pan of the roof panel.
12. Some field drilling at framed opngs may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closure etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

"SEE DRAWING D1 FOR BUILT-UP ? SECTION LEGEND"

ERECTOR NOTE
ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

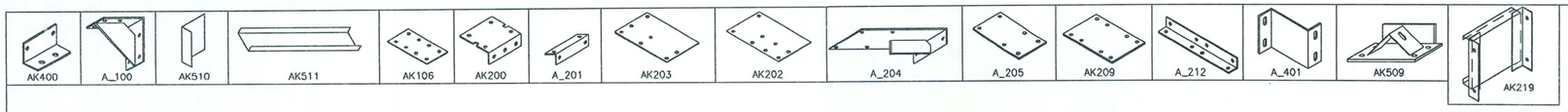
FEB 14 2008

WHIRLWIND STEEL BUILDINGS
P.O. BOX 75280
HOUSTON, TX 77234
PH: 800-324-9992
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

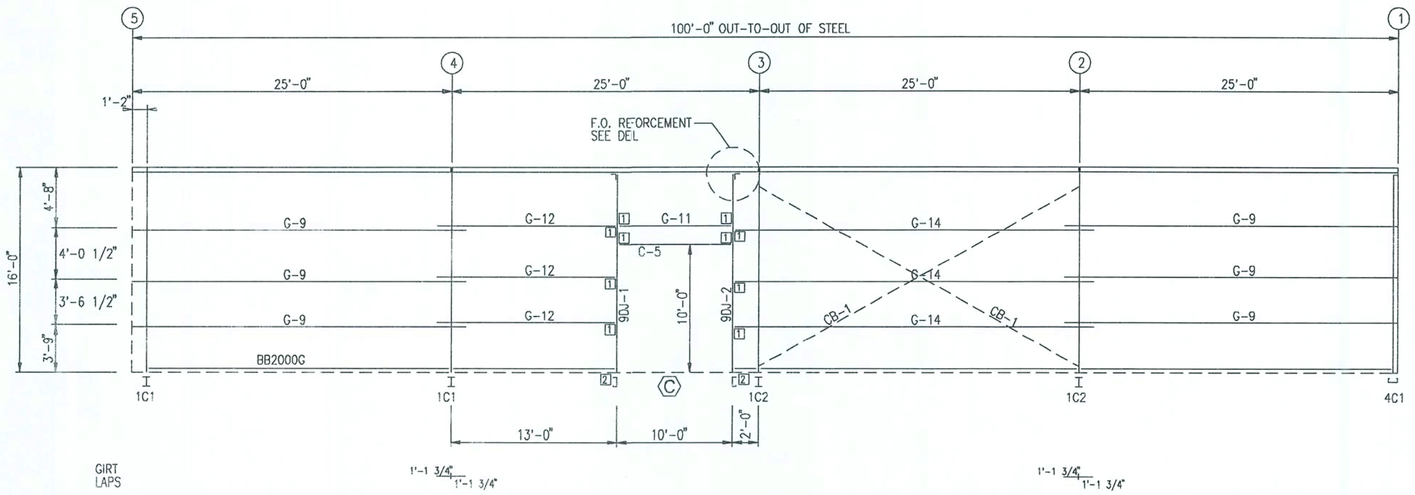
DESC: SIDEWALL FRAMING
BLDG. SIZE: 50'-0" x 100'-0" x 16'-0"
CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32055
REFERENCE: TAYLOR BUILDING
JOBSITE: LAKE CITY, FL. 32056
COUNTY: COLUMBIA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
MBS	.	1/ 8/08	TM	14180	E2	1

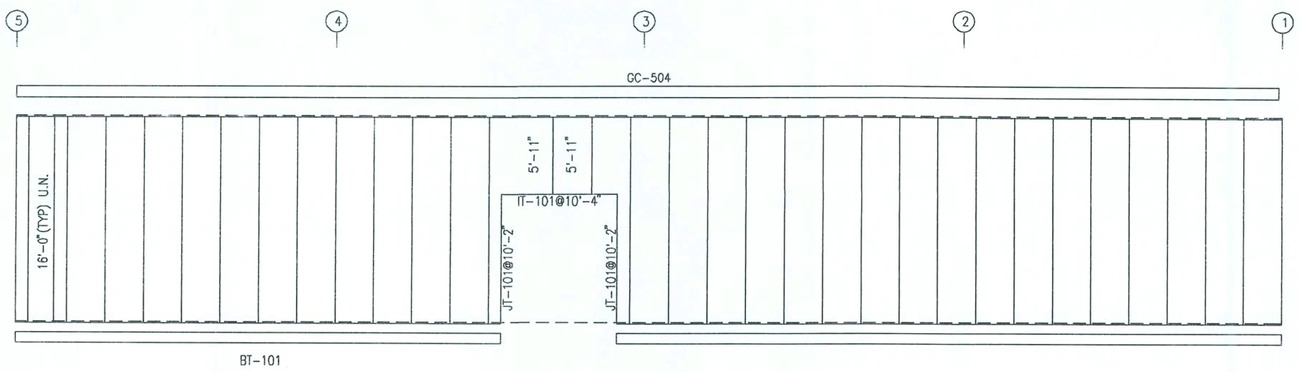


CONNECTION PLATES FRAME LINE A	
☐ ID	MARK/PART
1	AK200
2	AK400

MEMBER TABLE FRAME LINE A	
MARK	PART
9DJ-1	8X25C14
9DJ-2	8X25C14
C-5	8X25C16
G-9	8X35Z12
G-11	8X35Z16
G-12	8X35Z16
G-14	8X35Z12
CB-1	0.31_CBL

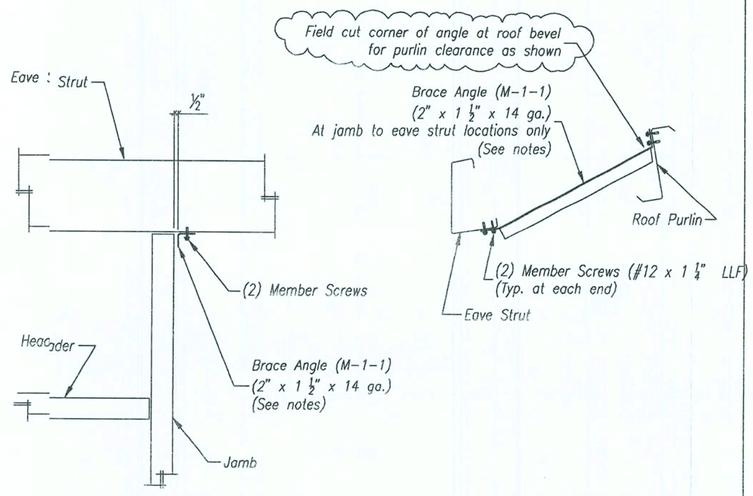


SIDEWALL FRAMING: FRAME LINE A



SIDEWAL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. SSX - SAHARA TAN



FRAMED OPENING REINFORCEMENT

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2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
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4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
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7. Start endwall panels at centerline of bldg, unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
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11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 2'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'-0 o.c. maximum, larger downspouts 5'-0 o.c maximum.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

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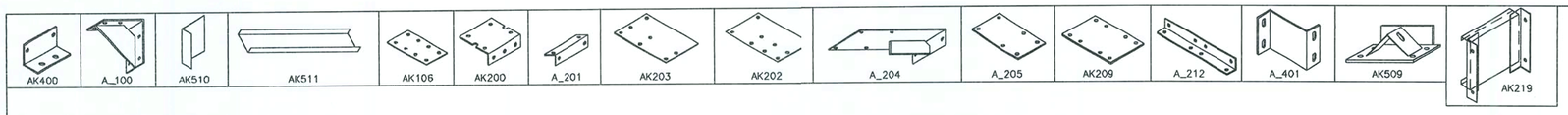
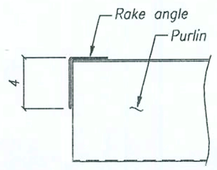
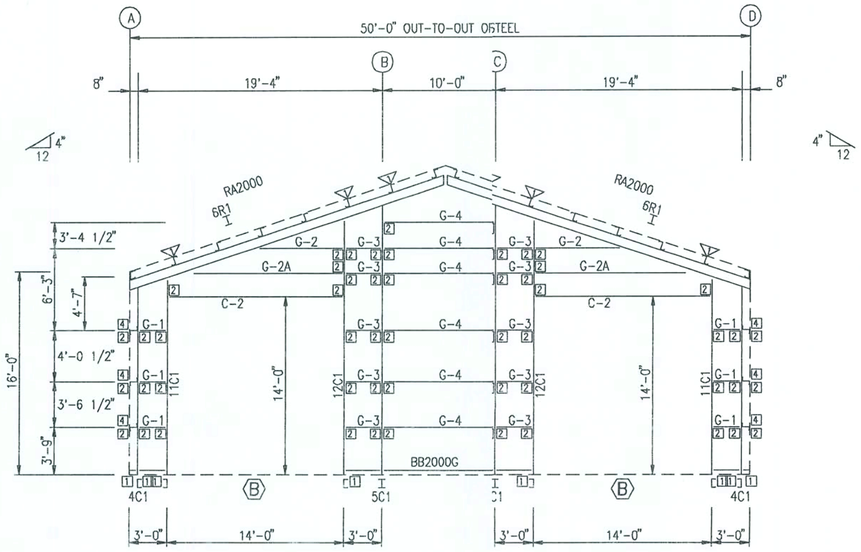
1	01.21.08	FOR ERECTION	SLV	VA

DESC: SIDEWALL FRAMING						BLDG SIZE: 50'-0" x 100'-0" x 16'-0"	
CUSTOMER: SPARKS CONSTRUCTION						LOCATION: LAKE CITY, FL. 32056	
REFERENCE: TAYLOR BUILDING							
JOBSITE: LAKE CITY, FL. 32056						COUNTY: COLUMBIA	
DWN: MBS	CHK: .	DATE: 1/ 8/08	ENG: TM	JOB NO: 14180	DWG NO: E3	ISSUE 1	



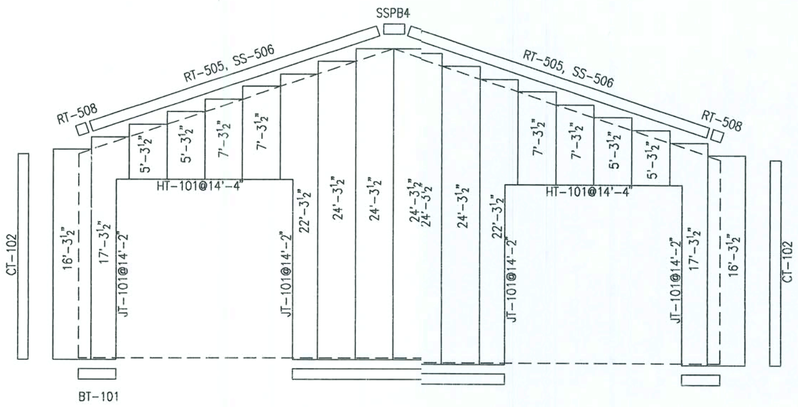


Detail at Rake Angle



ENDWALL FRAMING: FRAM LINE 1

FIELD BEVEL CUT
END PANELS



ENDWALL SHEETING & TRRIM: FRAME LINE 1

PANELS: 26 Ga. SSX – – SAHARA TAN

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
6R1/6R1	8	A325	5/8"	2"
Columns	2	A325	5/8"	1 1/2"
Jamb	2	A325	1/2"	1 1/2"

FLANGE BRACE TABLE
FRAME LINE 1

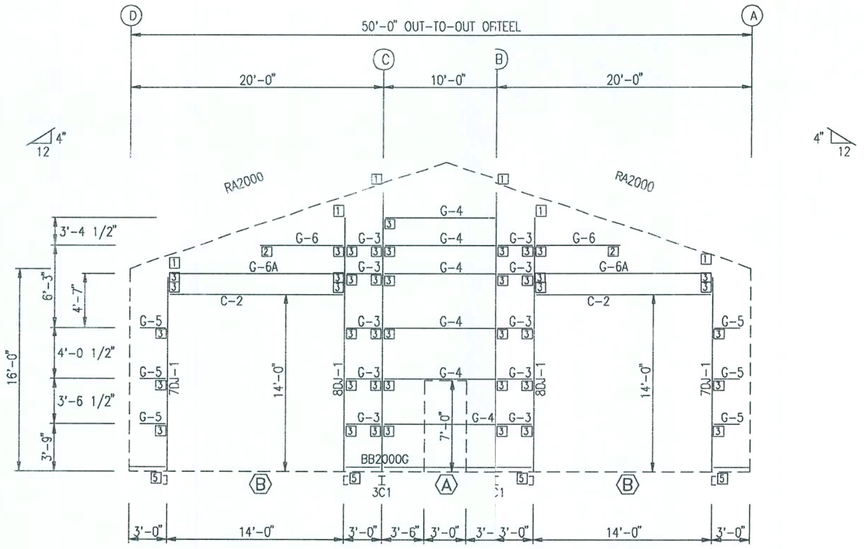
▽ID	MARK	LENGTH
1	FB28.88A	2'-4 7/8"

CONNECTION PLATES
FRAME LINE 1

□ ID	MARK/PART
1	AK400
2	AK200
4	SA005

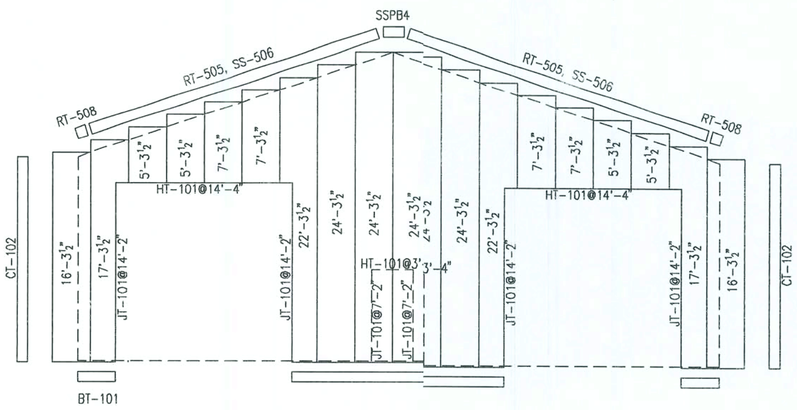
MEMBER TABLE
FRAME LINE 1

MARK	PART
4C1	8X35C14
5C1	W8X10
6R1	W8X15
C-2	8X25C14
11C1	8X35C12
12C1	8X35C12
G-1	8X25Z16
G-2	8X25Z16
G-2A	8X25Z16
G-3	8X25Z16
G-4	8X25Z16



ENDWALL FRAMING: FRAM LINE 5

FIELD BEVEL CUT
END PANELS



ENDWALL SHEETING & TRIRIM: FRAME LINE 5

PANELS: 26 Ga. SSX – – SAHARA TAN

"SEE DRAWING D1 FOR BUILT-L-UP SECTION LEGEND"

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns	4	A325	5/8"	1 1/2"
Jamb	4	A325	5/8"	1 1/2"

CONNECTION PLATES
FRAME LINE 5

□ ID	MARK/PART
1	AC411
2	10SC1
3	AK200
5	AK400

MEMBER TABLE
FRAME LINE 5

MARK	PART
3C1	W8X10
7DJ-1	8X35C12
8DJ-1	8X35C12
C-2	8X25C14
G-3	8X25Z16
G-4	8X25Z16
G-5	8X25Z16
G-6	8X25Z16
G-6A	8X25Z16

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10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
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FEB 14 2008

WHIRLWIND STEEL BUILDINGS

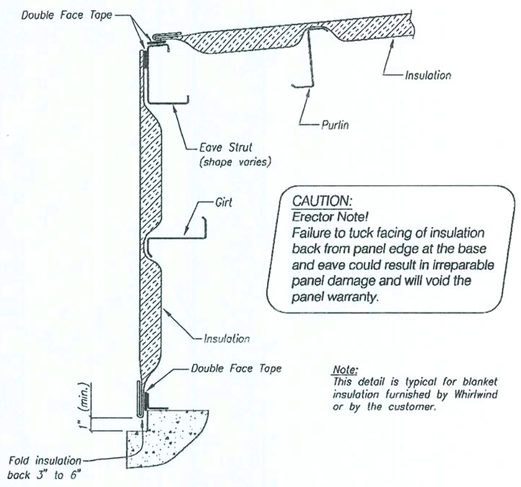
P.O. BOX 75280
HOUSTON, TX 77234

PH: 800-324-9958
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

DESC: ENDWALL FRAMING
BLDG SIZE: 50'-0" x 100'-0" x 16'-0"
CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32056
REFERENCE: TAYLOR BUILDING
JOBSITE: LAKE CITY, FL. 32056
COUNTY: COLUMBIA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
MBS	.	1/ 8/08	TM	14180	E4	1



CAUTION:
Erector Note!
Failure to tuck facing of insulation back from panel edge at the base and eave could result in irreparable panel damage and will void the panel warranty.

Note:
This detail is typical for blanket insulation furnished by Whirlwind or by the customer.

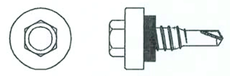
STANDARD DETAIL @ BLANKET INSULATION

Description: ¼–14 x ⅞ Hex Head Underc (#14 x ⅞ Long–Life
#1 Point Self–Drilling Lap Lap–Tek S.D.S.)
Long–Life Zinc Die Cast Hd

Seating Torque: 30 to 60 in–lbs

Recommended Driving Tool: 1800 RPM ettric screw gun with depth sensing nosepiece to prent overdriving and stripout

Suggested Pre–Drill: None



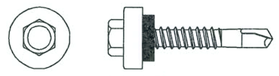
Actual Size

Description: 12–14 x 1¼ Hex Head Undercut (#12 x 1¼ Long–
#3 Long Pilot Point Self–Drilling Life S.D.S.)
Long–Life Zinc Die Cast Head

Seating Torque: 30 to 60 in–lbs

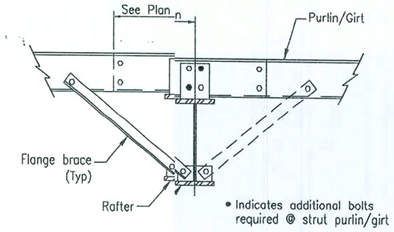
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout

Suggested Pre–Drill: None



Actual Size

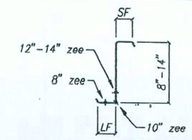
Note: Flange braces may be on one or both sides.
See frame cross–section for r locations.



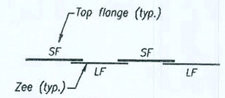
(6) ½"ø A–307 machine bolts lts required at standard laps.
(8) ½"ø A–307 machine bolts lts required at strut members.
(A strut member is a purlin or girt at the bracing points.
The struts continue across th the length of the building.)
See E–drawing for exceptions ns to size and quantity of bolts.

LAP DETAIL

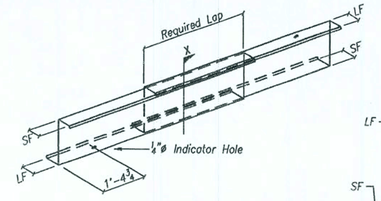
Notes:
1. Rotate each zee as required to alternate between long and short flanges.
2. The ¼"ø hole 1'–4¾ from each end is located nearest the long flange.
3. Lap connection bolts are omitted for clarity.



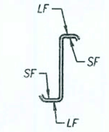
1/4" INDICATOR HOLE LOCATION



LAP LAYOUT

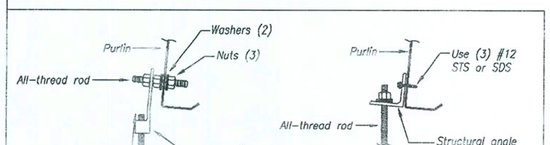


LF = Long Flange
SF = Short Flange

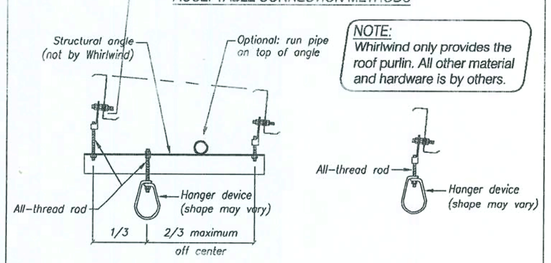


SECTION "X"

ZEE LAP DETAIL @ THE LONG AND SHORT FLANGES



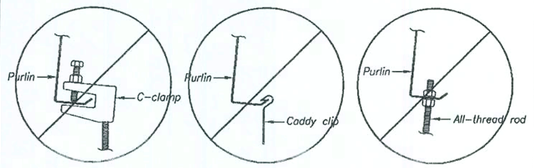
ACCEPTABLE CONNECTION METHODS



NOTE:
Whirlwind only provides the roof purlin. All other material and hardware is by others.

TRAPEZE HANGER ~ 4" MAX. PIPE

HANGER ~ 2-1/2" MAX. PIPE



Flange C-Clamp is not an acceptable connection

Caddy clip attached to lip is not an acceptable connection

Connection through the flange is not acceptable

ACCEPTABLE CONNECTIONS FOR ALL COLLATERAL LOADS FOR HANGER ATTACHMENT

BUILT-UP SECTION LEGEND

5 = 5	3 = 3/16	8 = ½	1= 10ga.
6 = 6	4 = ¼	0 = 5/8	2= 8ga.
8 = 8	5 = 5/16	2 = ¾	3= 3/16
0 = 10	6 = ⅜	1 = 1	4= ¼
2 = 12			6= ⅜

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CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32056
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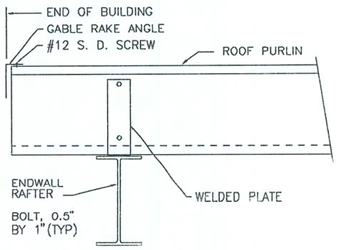
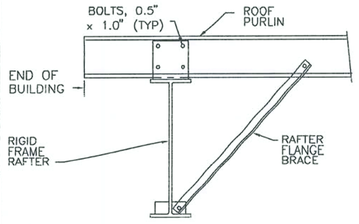
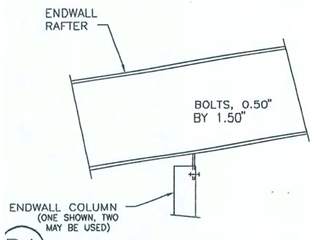


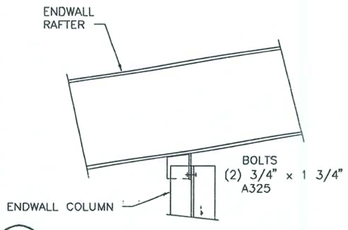
A7 SECTION THRU ENDWALL RAFTER



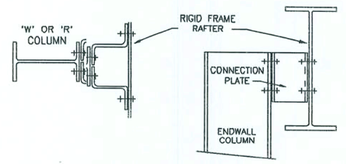
A10 ROOF PURLIN TO EXPANDABLE ENDWALL RIGID FRAME



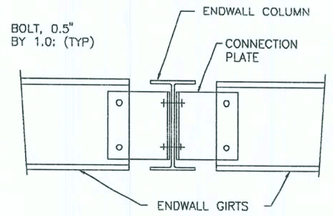
B4 ENDWALL RAFTER TO COLUMN



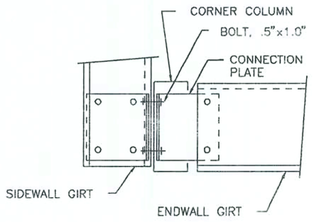
B6 ENDWALL RAFTER TO COLUMN



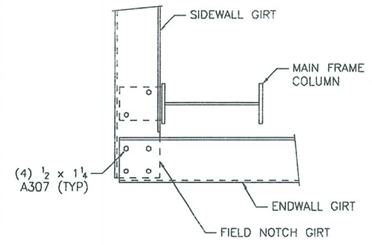
B34 ENDWALL RAFTER TO COLUMN



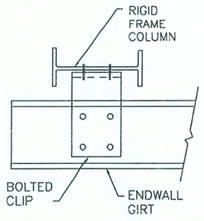
C3 ENDWALL COLUMN TO WALL GIRT



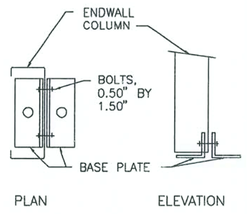
D1 CORNER COLUMN TO WALL GIRT



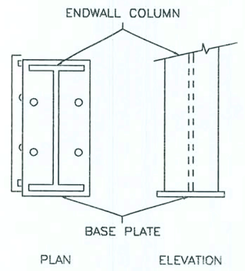
D16 GIRT TO GIRT CONNECTION AT CORNER



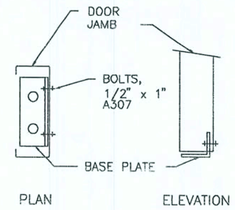
D27 CORNER COLUMN TO ENDWALL GIRT



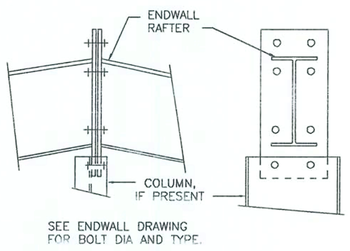
E1 BASE PLATE FOR ENDWALL COLUMN OR DOOR JAMB



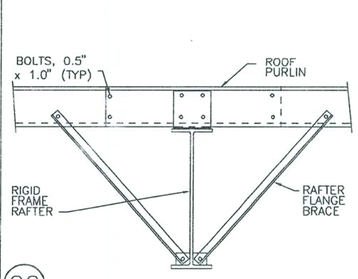
E3 BASE PLATE FOR ENDWALL COLUMN



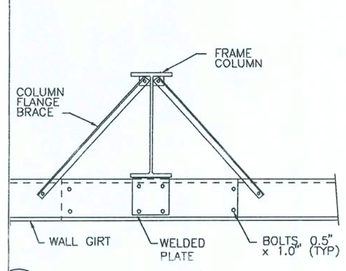
E6 BASE PLATE FOR DOOR JAMB



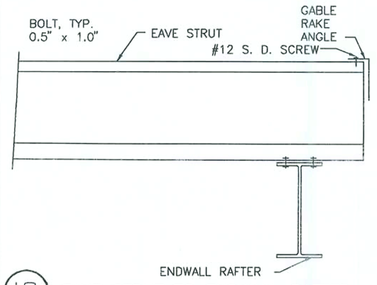
F12 RAFTER SPLICE AT SURFACE CHANGE



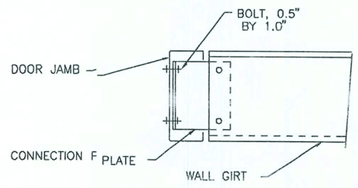
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



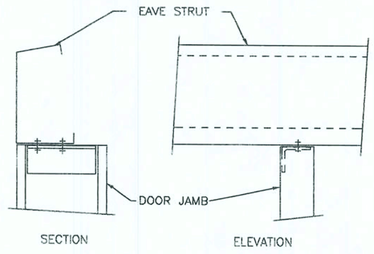
H2 WALL GIRT TO FRAME COLUMN



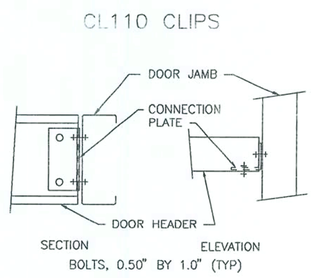
I8 EAVE STRUT TO ENDWALL RAFTER



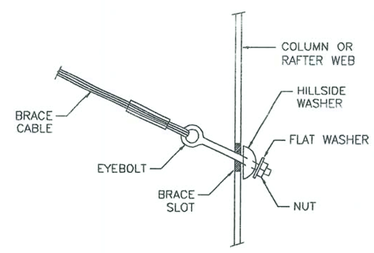
K2 WALL GIRT TO DOOR JAMB



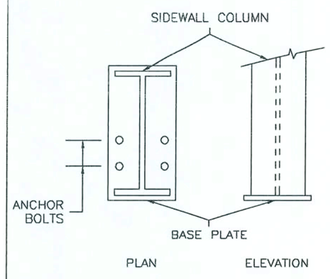
L3 DOOR JAMB TO EAVE STRUT



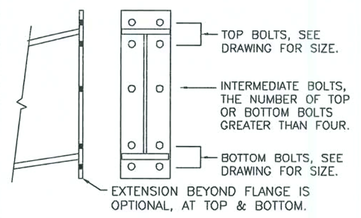
M1 DOOR HEADER TO DOOR JAMB



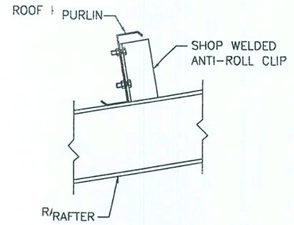
Q2 DETAIL AT BRACE CABLE CONNECTION



R2 ANCHOR BOLTS AT SIDEWALL COLUMN



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK



X1 DETAIL AT ANTI-ROLL CLIP

STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

FEB 1 4 2008

ERECTOR NOTE
ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

WHIRLWIND STEEL BUILDINGS
P.O. BOX 75280
HOUSTON, TX 77234
PH: 800-324-9992
FAX: 832-553-4600

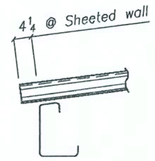
REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

DESC: DETAIL PAGE					BLDG SIZE: 50'-0" x 100'-0" x 16'-0"	
CUSTOMER: SPARKS CONSTRUCTION					LOCATION: LAKE CITY, FL. 32056	
REFERENCE: TAYLOR BUILDING						
JOBSITE: LAKE CITY, FL. 32056					COUNTY: COLUMBIA	
DWN: MBS	CHK: .	DATE: 1/ 8/08	ENG: TM	JOB NO: 14180	DWG NO: D2	ISSUE 1

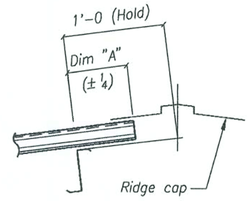
SPLICE BOLTS							
Splice Mark	Quan Top/Bot/Int			Type	Dia	-----Bolt----- Len	
SP- 1	4	4	0	A325	3/4"	2 1/2"	
SP- 2	4	4	0	A325	3/4"	2"	

BASE PLATES			
Col Id	Plate Size Wid	Thick	Length
BP- 1	8"	3/8"	1'-4 1/2"

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X15X12



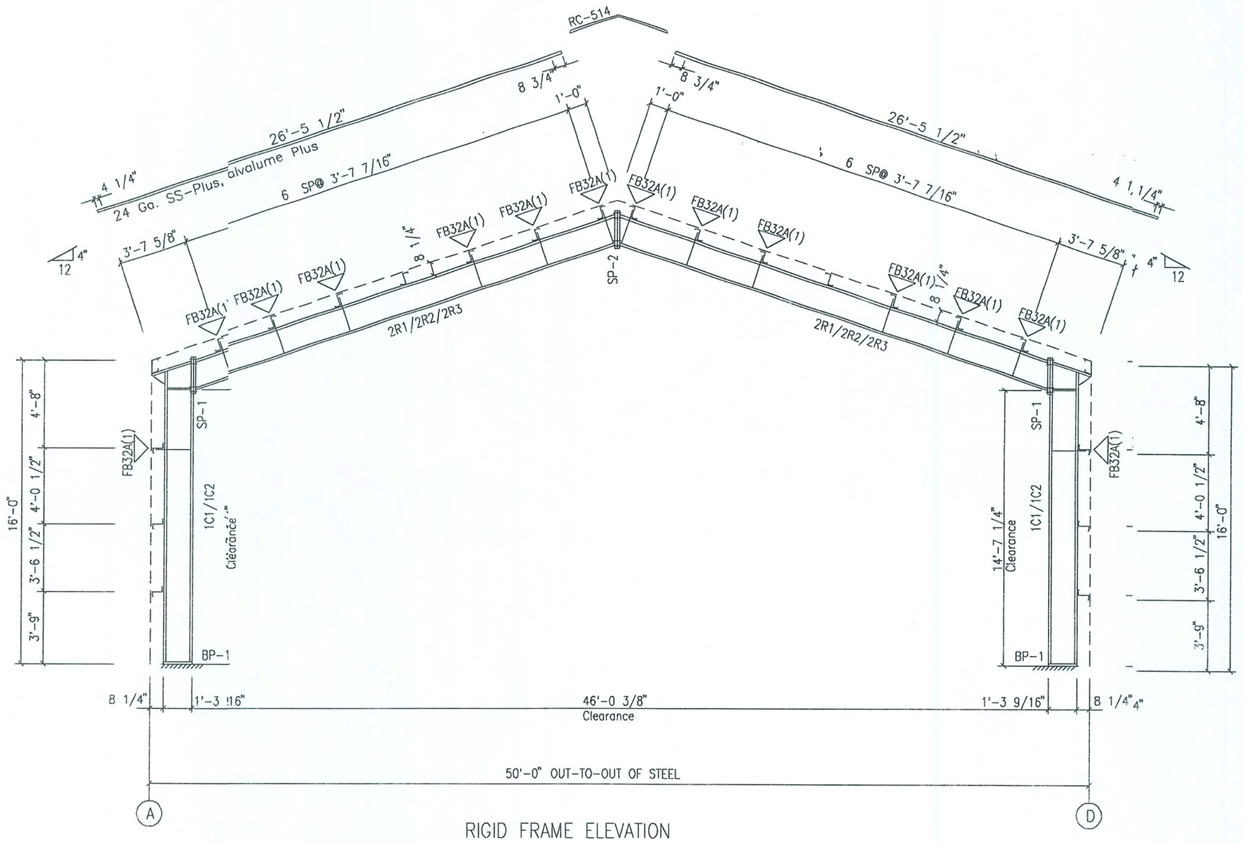
DETAIL @ PANEL END



Roof Slope	Dim "A"
0 to 1½ : 12	0'-7¾"
1 9/16 to 2½ : 12	0'-8¼"
2 9/16 to 3½ : 12	0'-8½"
3 9/16 to 4 : 12	0'-8¾"
@ Ridge Vent	4½" min.

DETAIL @ RIDGE CAP

MEMBER SIZE TABLE			
PIECE	WEB DEPTH START/END	WEB PLATE THICK	OUTSIDE FLANGE W x T x LEN	INSIDE FLANGE W x T x LEN
1C1	15.0/15.0	0.164	6x1/4	6x5/16"
2R1	15.0/15.0	0.134	6x1/4"	6x1/4"
	15.0/15.0	0.134		



RIGID FRAME ELEVATION
FOR FRAME LINE 2 3 4 5

TURN-OF-NUT TIGHTENING
Connections for rigid frames must be properly pre-tensioned. The Specification for Structural Joints Using ASTM A325 or A490 Bolts dated November 13, 1985 (future reference to this section is to be called the Code) recognizes four methods to properly tighten the bolts; 1) "Turn-of-Nut", 2) calibrated wrench, 3) alternate design bolts and 4) direct tension indicator. All of these methods require special bolts and/or equipment to install, except the Turn-of-Nut Method. This is why Whirlwind specifies this method for bolt installation.

According to paragraph 7(c) of the Code, washers are not required to be installed.

Excerpts from the Code for installation come from paragraph 8(d): "Bolts shall be installed in all holes of the connection and brought to a snug-tight condition. Snug-tight is defined as the tightness that exist when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following this initial operation, all bolts in the connection shall be tightened further by the applicable amount of rotation specified in table 5..."

Table 5. Nut Rotation From Snug-Tight Condition.

Bolt Length (from under side of head to end of bolt)
Up to and including 4 diameters ..... 1/3 turn
Over 4 diameters, but not exceeding 8 diameters ..... 1/2 turn
Over 8 diameters, but not exceeding 12 diameters ..... 2/3 turn

ERECTOR NOTE
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STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

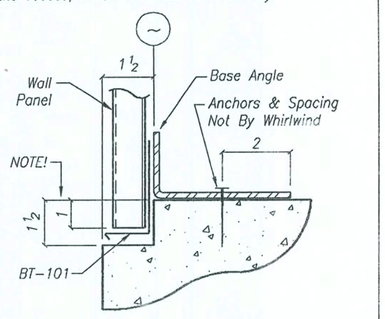
FEB 14 2008

WHIRLWIND STEEL BUILDINGS
P.O. BOX 75280
HOUSTON, TX 77234
PH: 800-324-9992
FAX: 832-553-4600

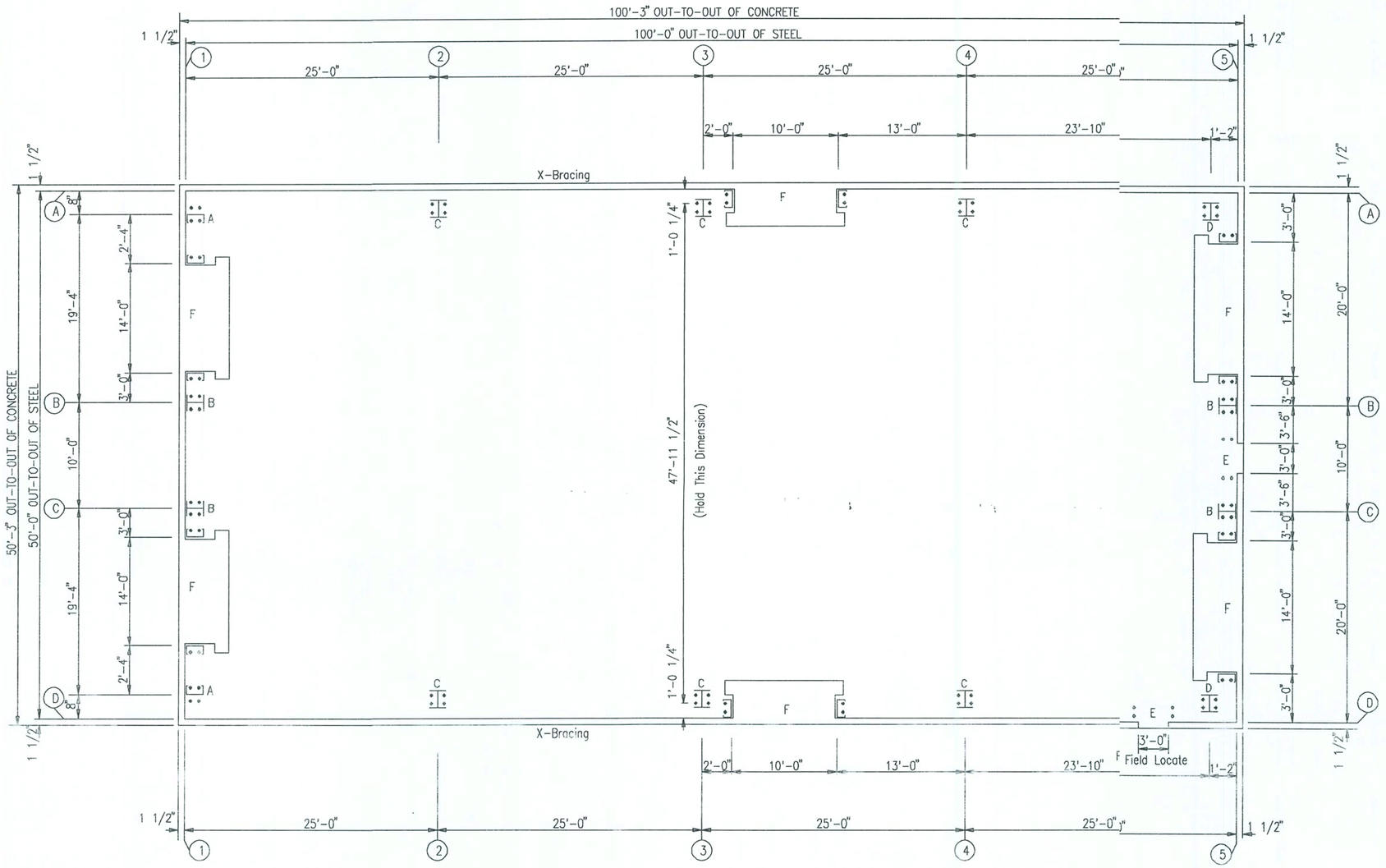
REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

DESC: RIGID FRAME ELEVATION						BLDG SIZE: 50'-0" x 100'-0" x 16'-0"	
CUSTOMER: SPARKS CONSTRUCTION						LOCATION: LAKE CITY, FL. 32056	
REFERENCE: TAYLOR BUILDING							
JOBSITE: LAKE CITY, FL. 32056						COUNTY: COLUMBIA	
DWN: MBS	CHK: .	DATE: 1/ 8/08	ENG: TM	JOB NO: 14180		DWG NO: P1	ISSUE: 1

Horizontal leg of recess must remain flat or slope away from the building. Notch area indicates a recess for metal wall panels. Panels must not touch the bottom of the recess, which will void the warranty.



DETAIL @ SHEETING RECESS NTS



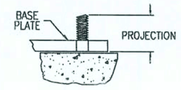
ANCHOR BOLT PLAN

NOTE: All Base Plates @ Finish Floor (U.N.)

ANCHOR BOLT SCHEDULE

SYMBOL	QUANTITY	SIZE	PROJECTION
○	8	$0\frac{1}{2}$	2
●	48	$0\frac{5}{8}$	$2\frac{1}{2}$
⊕	32	$0\frac{3}{4}$	3
✳	.	1	$3\frac{1}{2}$
◐	.	$1\frac{1}{4}$	4
◕	.	$1\frac{1}{2}$	4

THREADED ANCHOR BOLT

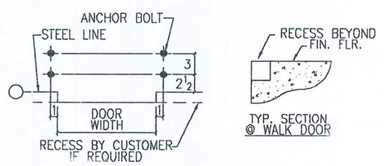


NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

REFERENCE NOTES

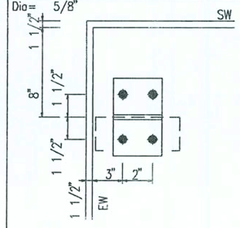
1. ALL ANCHOR BOLTS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
2. ALL ANCHOR BOLTS ARE DESIGNED TO FULL S.A.E. DIAMETERS WITH CUT THREADS. NO SUBSTITUTIONS ARE ALLOWED.
3. WHIRLWIND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIALS AND WORKMANSHIP OF THE FOUNDATION. ANCHOR BOLT PLANS PREPARED BY WHIRLWIND ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER, AND PROJECTION OF ANCHOR BOLTS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO THE FOUNDATION. WHIRLWIND IS RESPONSIBLE FOR PROVIDING TO THE BUILDER THE LOADS IMPOSED BY THE METAL BUILDING SYSTEM ON THE FOUNDATION. IT IS THE RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING BOLT EMBEDMENT, BEARING ANGLES, TIE RODS, AND/OR OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY THE METAL BUILDING SYSTEM, OTHER IMPOSED LOADS, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. (1996 LOW RISE BUILDING SYSTEMS MANUAL, SECTION IV; 3.2.2)

Dia= 1/2"



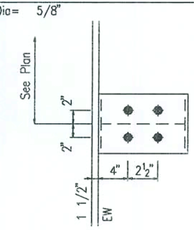
DETAIL E

Dia= 5/8"



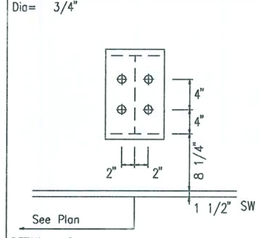
DETAIL A

Dia= 5/8"



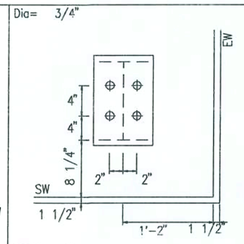
DETAIL B

Dia= 3/4"



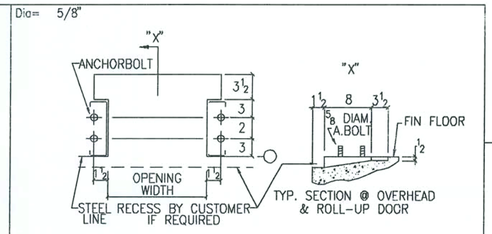
DETAIL C

Dia= 3/4"



DETAIL D

Dia= 5/8"



DETAIL F

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FEB 14 2008

WHIRLWIND STEEL BUILDINGS

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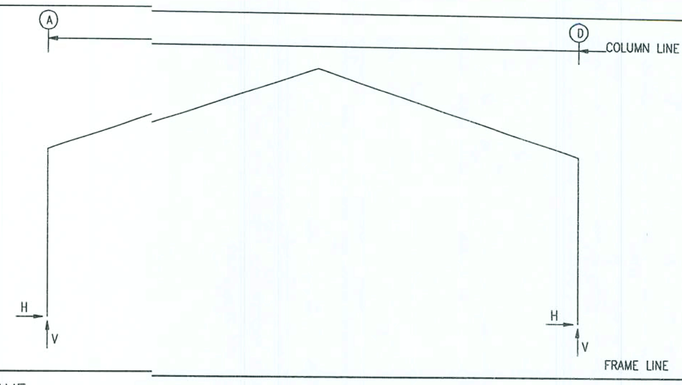
HOUSTON, TX 77234

PH: 800-324-9992
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

DESC: ANCHOR BOLT PLAN
BLDG SIZE: 50'-0" x 100'-0" x 16'-0"
CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32056
REFERENCE: TAYLOR BUILDING
JOBSITE: LAKE CITY, FL. 32056
COUNTY: COLUMBIA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
MBS	.	1/ 8/08	TM	14180	F1	1



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt No	Anc. Bolt D(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Elev. (in)
2 *	A	4	0.750	8.000	16.50	0.375	0.0
2 *	D	4	0.750	8.000	16.50	0.375	0.0

2 * Frame lines: 2 3 4 5

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_L1 Horiz	Wind_L1 Vert	Wind_R1 Horiz	Wind_R1 Vert
2 *	A	0.78	2.38	0.50	1.32	2.85	7.50	0.00	0.00	-5.60	-10.19	0.53	-7.19
2 *	D	-0.78	2.38	-0.50	1.32	-2.85	7.50	0.00	0.00	-0.53	-7.19	5.60	-10.19

Frame Line	Column Line	Wind_L2 Horiz	Wind_L2 Vert	Wind_R2 Horiz	Wind_R2 Vert	LnWind_1 Horiz	LnWind_1 Vert	LnWind_2 Horiz	LnWind_2 Vert	Seismic_L Horiz	Seismic_L Vert	Seismic_R Horiz	Seismic_R Vert
2 *	A	-5.62	-6.05	0.51	-3.04	0.32	-10.79	-0.91	-9.89	-0.19	-0.12	0.19	0.12
2 *	D	-0.51	-3.04	5.62	-6.05	0.91	-9.89	-0.32	-10.79	-0.19	0.12	0.19	-0.12

Frame Line	Column Line	LnSeis Horiz	LnSeis Vert
2 *	A	0.00	-0.83
2 *	D	0.00	-0.83

2 * Frame lines: 2 3 4 5

ENDWALL COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions (k) Dead Vert	Coll Vert	Live Vert	Wind-Left Horiz	Wind-Left Vert	Wind-Right Horiz	Wind-Right Vert	Out-Of-Plane Wd P Horiz	Out-Of-Plane Wd S Horiz	Anc. Bolt No	Anc. Bolt D(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Elev. (in)
1	A	0.5	0.2	1.7	0.0	-2.3	0.0	-1.8	-1.3	1.6	4	0.625	5.500	5.438	0.375	0.0
1	B	1.0	0.4	3.3	0.0	-4.3	0.0	-2.5	-2.7	3.0	4	0.625	8.000	8.500	0.375	0.0
1	C	1.0	0.4	3.3	0.0	-2.5	0.0	-4.3	-2.7	3.0	4	0.625	8.000	8.500	0.375	0.0
1	D	0.5	0.2	1.7	0.0	-1.8	0.0	-2.3	-1.3	1.6	4	0.625	5.500	5.438	0.375	0.0
5	C	0.2	0.0	0.0	0.0	0.0	0.0	0.0	-2.8	3.1	4	0.625	8.000	8.500	0.375	0.0
5	B	0.2	0.0	0.0	0.0	0.0	0.0	0.0	-2.8	3.1	4	0.625	8.000	8.500	0.375	0.0

BRACING REACTIONS, PANEL SHEAR

Wall Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft)
L_EW	1						
F_SW	D	2 ,3	3.8	2.2	1.4	0.8	93
R_EW	5	Rigid Frame At Endwall					
B_SW	A	3 ,2	3.8	2.2	1.4	0.8	

ERECTOR NOTE
ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

STEEL DESIGN ONLY NO OTHER CODE REQUIREMENTS ARE INCLUDED

FEB 14 2008

WHIRLWIND STEEL BUILDINGS
P.O. BOX 75280
HOUSTON, TX 77234
PH: 800-324-9992
FAX: 832-553-4600

REV	DATE	DESCRIPTION	BY	CHK
0	01.09.08	FOR CONSTRUCTION	AM	JM
1	01.21.08	FOR ERECTION	SLV	VA

DESC: REACTIONS SHEET
BLDG SIZE: 50'-0" x 100'-0" x 16'-0"
CUSTOMER: SPARKS CONSTRUCTION
LOCATION: LAKE CITY, FL. 32056
REFERENCE: TAYLOR BUILDING
JOBSITE: LAKE CITY, FL. 32056
COUNTY: COLUMBIA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE
MBS	.	1/ 8/08	TM	14180	F2	1