

Date: July 06, 2022



B+T Group
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630

Subject: Structural Analysis Report

Carrier Designation: Verizon Wireless Co-Locate
Site Number: 712924
Site Name: CC Lake City Airport

Crown Castle Designation: BU Number: 825272
Site Name: Lake City Airport
JDE Job Number: 706810
Work Order Number: 2133059
Order Number: 605523 Rev. 1

Engineering Firm Designation: B+T Group Project Number: 121641.005.01

Site Data: 336 SE Newell DR, Lake City, Columbia County, FL
Latitude 30° 10' 13.5", Longitude -82° 35' 9.7"
132 Foot - Monopole Tower

B+T Group is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

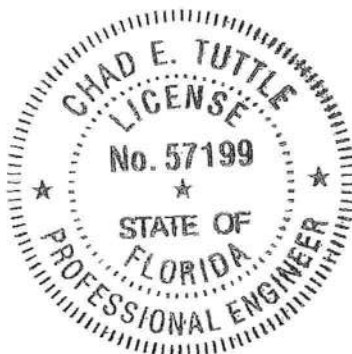
LC5: Proposed Equipment Configuration

Sufficient Capacity

This analysis utilizes an ultimate 3-second gust wind speed of 118 mph as required by the 2020 Florida Building Code 7th Edition. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Massood Sattari

Respectfully submitted by: B+T Engineering, Inc.



CHAD TUTTLE, P.E.
B&T ENGINEERING, INC.
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Chad E. Tuttle, P.E.

tnxTower Report - version 8.1.1.0

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1) INTRODUCTION

This tower is a 132 ft. Monopole tower designed by Sabre Communications.

2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H
Risk Category: II
Wind Speed: 118 mph
Exposure Category: C
Topographic Factor: 1
Service Wind Speed: 60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
123.0	123.0	2	Commscope	RCMDC-6627-PF-48	2	2
		6	JMA Wireless	MX06FIT865-02		
		3	Ericsson	4449		
		3	Ericsson	8843		
		3	Ericsson	AIR 6449 B77		
		1	Site Pro 1	HRK12		
		1	Site Pro 1	RMQP-NP Low Profile Platform		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
133.0	133.0	1	--	Platform Mount [LP 701-1_HR-1]	3	1-5/8
	129.0	4	Commscope	FFVV-65C-R3-V1_TMO		
		4	Ericsson	AIR6449 B41_T-MOBILE		
		4	Ericsson	RADIO 4460 B2/B25 B66_TMO		
		4	Ericsson	RADIO 4480 B71_TMO		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
Tower Manufacturer Drawing	3553565	CCI Sites
Foundation Drawing	3877066	CCI Sites
Geotech Report	3553564	CCI Sites
Crown CAD Package	Date: 06/30/2022	CCI Sites

3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) The tower and structures were maintained in accordance with the - TIA-222 standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P _{allow} (K)	% Capacity	Pass / Fail
L1	132 - 99.75	Pole	TP21.66x13.5x0.188	1	-10.303	759.503	76.9	Pass
L2	99.75 - 49	Pole	TP34.12x20.589x0.313	2	-17.127	1994.265	58.5	Pass
L3	49 - 0	Pole	TP45.9x32.42x0.313	3	-27.698	2777.460	65.0	Pass
							Summary	
						Pole (L1)	76.9	Pass
						Rating =	76.9	Pass

Table 5 - Tower Component Stresses vs. Capacity –LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Anchor Rods	Base	54.5	Pass
1,2	Base Plate	Base	47.4	Pass
1,2	Base Foundation (Structure)	Base	57.0	Pass
1,2	Base Foundation (Soil Interaction)	Base	50.7	Pass

Structure Rating (max from all components) =	76.9%
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Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H Section 15.5.

4.1) Recommendations

The tower and its foundations have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

TOWER DESIGN NOTES


1. Tower is located in Columbia County, Florida.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 118 mph basic wind in accordance with the TIA-222-H Standard.
4. Deflections are based upon a 60 mph wind.
5. Tower Risk Category II.
6. Topographic Category 1 with Crest Height of 0.000 ft
7. TIA-222-H Annex S
8. TOWER RATING: 76.9%

ALL REACTIONS
ARE FACTORED

Diagram illustrating the internal forces and reactions for a cantilever beam under a 118 mph wind load. The beam is fixed at the left end and has a length of 100 ft. The internal forces at the free end (right) are:

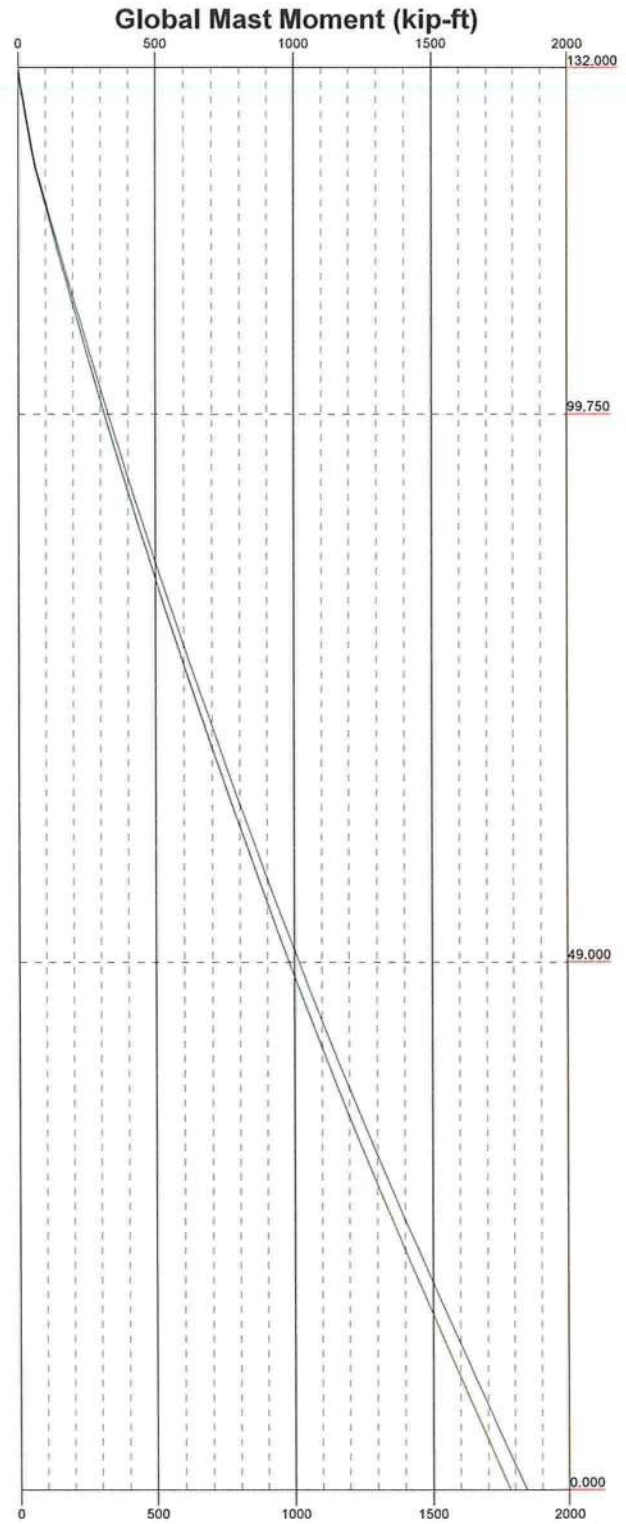
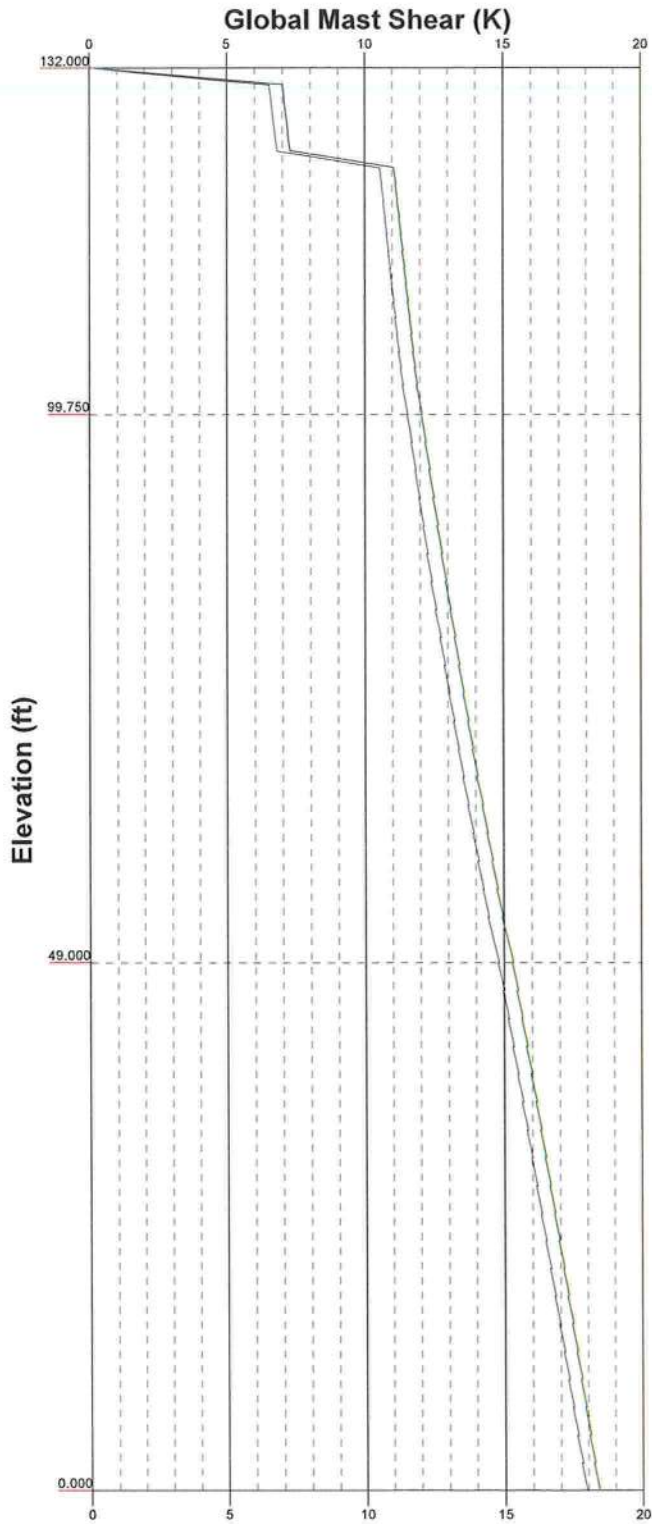
- AXIAL: 28 K
- SHEAR: 18 K
- MOMENT: 1846 kip-ft
- TORQUE: 3 kip-ft

REACTIONS - 118 mph WIND

 <p>B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job: 121641.005.01 - Lake City Airport, FL (BU# 82527)</p> <p>Project:</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Client: Crown Castle</td> <td style="width: 33%;">Drawn by: R AITHAL</td> <td style="width: 33%;">App'd:</td> </tr> <tr> <td>Code: TIA-222-H</td> <td>Date: 07/06/22</td> <td>Scale: NTS</td> </tr> <tr> <td colspan="2">Path:</td> <td>Dwg No. E-1</td> </tr> </table>	Client: Crown Castle	Drawn by: R AITHAL	App'd:	Code: TIA-222-H	Date: 07/06/22	Scale: NTS	Path:		Dwg No. E-1
Client: Crown Castle	Drawn by: R AITHAL	App'd:								
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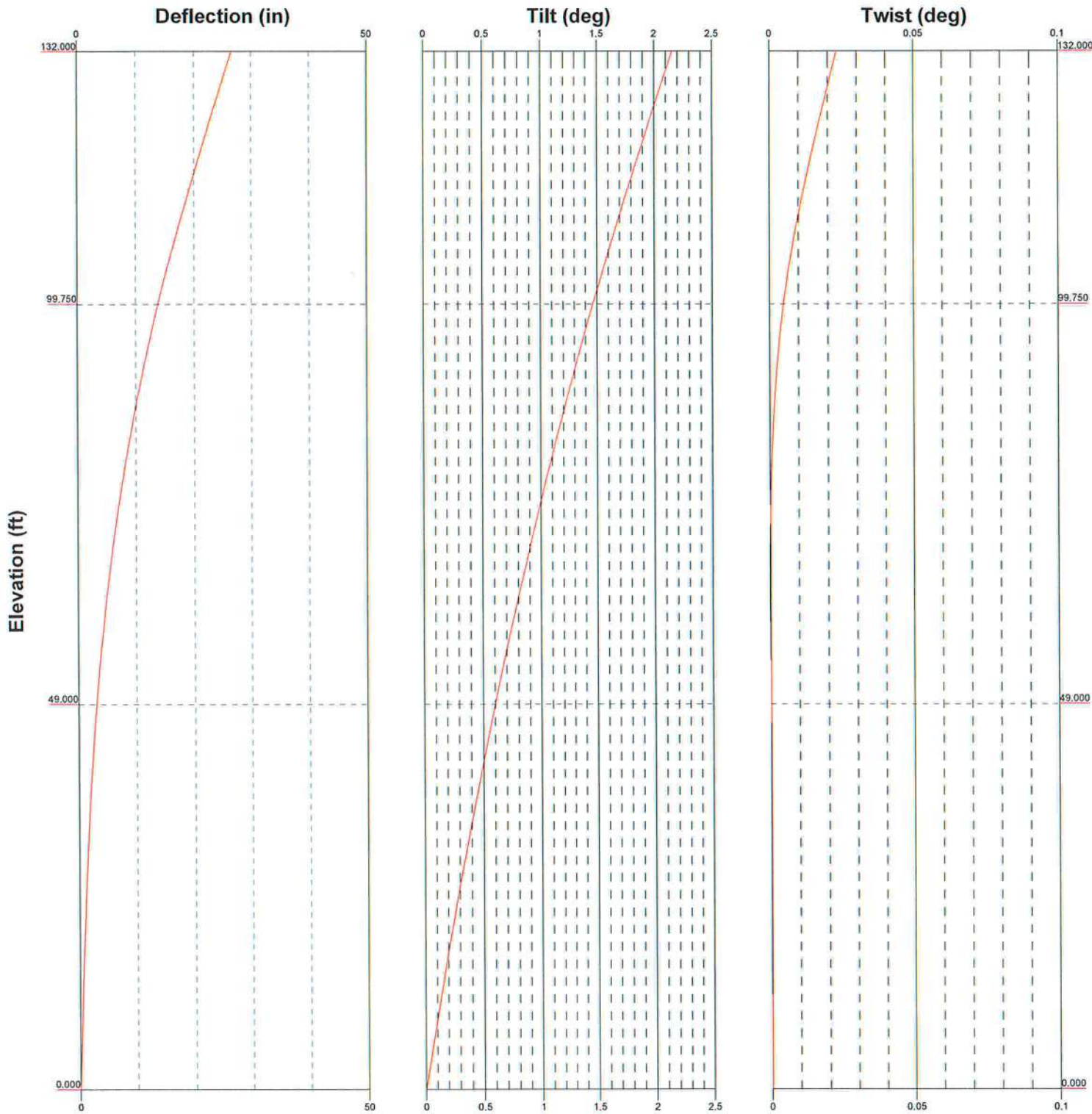
— Vx — Vz

— Mx — Mz



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 Project:
 Client: **Crown Castle** Drawn by: **R. AITHAL** App'd:
 Code: **TIA-222-H** Date: **07/06/22** Scale: **NTS**
 Path: Dwg No. **E-4**



Feed Line Distribution Chart

0' - 132'

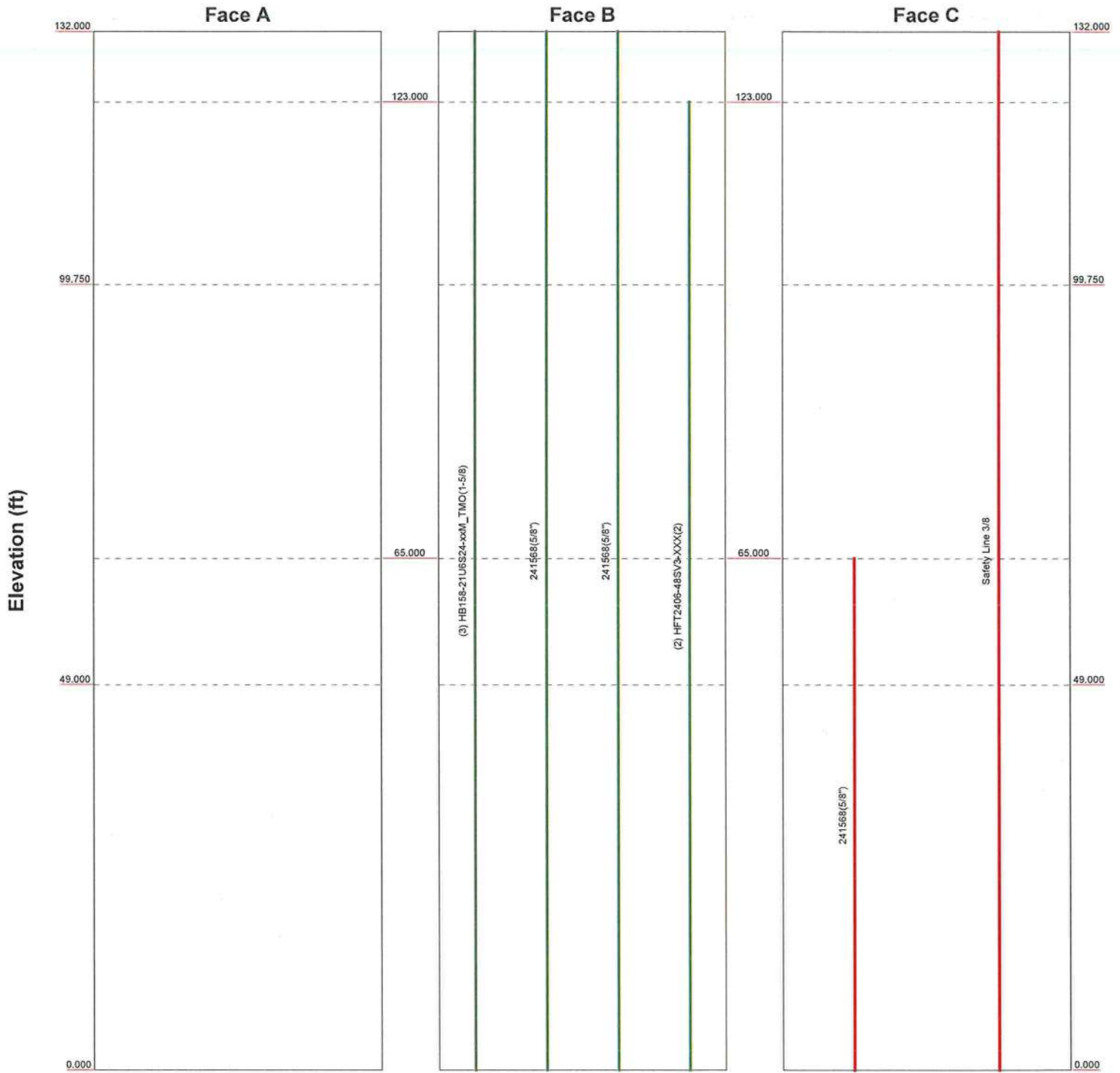
Round

Flat

App In Face

App Out Face

Truss Leg



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Client: Crown Castle	Drawn by: R AITHAL	App'd:
Code: TIA-222-H	Date: 07/06/22	Scale: NTS
Path:	Dwg No. E-7	

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	Client	Crown Castle	Designed by	R AITHAL

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Columbia County, Florida.

Tower base elevation above sea level: 167.000 ft.

Basic wind speed of 118 mph.

Risk Category II.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.000 ft.

Deflections calculated using a wind speed of 60 mph.

TIA-222-H Annex S.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Tower analysis based on target reliabilities in accordance with Annex S.

Load Modification Factors used: $K_{es}(F_w) = 0.95$.

Maximum demand-capacity ratio is: 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	√ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	√ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
√ Use Code Stress Ratios	Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
√ Use Code Safety Factors - Guys	Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	√ Bypass Mast Stability Checks	√ Consider Feed Line Torque
Always Use Max Kz	√ Use Azimuth Dish Coefficients	Include Angle-Block Shear Check
Use Special Wind Profile	√ Project Wind Area of Appurt.	Use TIA-222-H Bracing Resist. Exemption
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Use TIA-222-H Tension Splice Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Poles
Secondary Horizontal Braces Leg	Sort Capacity Reports By Component	√ Include Shear-Torsion Interaction
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
SR Members Are Concentric	Ignore KL/ry For 60 Deg. Angle Legs	Pole Without Linear Attachments
		Pole With Shroud Or No Appurtenances
		Outside and Inside Corner Radii Are
		Known

Tapered Pole Section Geometry

Section	Elevation	Section	Splice	Number	Top	Bottom	Wall	Bend	Pole Grade
	ft	Length	Length	of	Diameter	Diameter	Thickness	Radius	
		ft	ft	Sides	in	in	in	in	

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Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	132.000-99.750	32.250	2.750	18	13.500	21.660	0.188	0.750	A572-65 (65 ksi)
L2	99.750-49.000	53.500	4.250	18	20.589	34.120	0.313	1.250	A572-65 (65 ksi)
L3	49.000-0.000	53.250		18	32.420	45.900	0.313	1.250	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	Iu/Q in ²	w in	w/t
L1	13.679	7.923	177.388	4.726	6.858	25.866	355.010	3.962	2.046	10.912
	21.965	12.779	744.378	7.623	11.003	67.651	1489.735	6.391	3.482	18.572
L2	21.565	20.112	1044.685	7.198	10.459	99.881	2090.744	10.058	3.074	9.836
	34.598	33.533	4842.098	12.002	17.333	279.358	9690.567	16.770	5.455	17.456
L3	33.965	31.847	4147.810	11.398	16.469	251.849	8301.077	15.926	5.156	16.499
	46.560	45.217	11872.214	16.184	23.317	509.161	23760.050	22.613	7.528	24.091

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 132.000-99.75 0				1	1	1			
L2 99.750-49.000				1	1	1			
L3 49.000-0.000				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight klf
* 241568(5/8")	C	No	Surface Ar (CaAa)	65.000 - 0.000	1	1	0.200 0.210	0.630		0.000
* Safety Line 3/8	C	No	Surface Ar (CaAa)	132.000 - 0.000	1	1	0.150 0.160	0.375		0.000
* *										

Feed Line/Linear Appurtenances - Entered As Area

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Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		$C_A A_A$ ft ² /ft	Weight klf
HB158-21U6S24-xx M_TMO(1-5/8) *	B	No	No	Inside Pole	132.000 - 0.000	3	No Ice	0.000	0.003
241568(5/8")	B	No	No	Inside Pole	132.000 - 0.000	1	No Ice	0.000	0.000
241568(5/8") *	B	No	No	Inside Pole	132.000 - 0.000	1	No Ice	0.000	0.000
HFT2406-48SV3-X XX(2) * *	B	No	No	Inside Pole	123.000 - 0.000	2	No Ice	0.000	0.003

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	$C_A A_A$ In Face ft ²	$C_A A_A$ Out Face ft ²	Weight K
L1	132.000-99.750	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.398
		C	0.000	0.000	1.209	0.000	0.007
L2	99.750-49.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.715
		C	0.000	0.000	2.911	0.000	0.014
L3	49.000-0.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.690
		C	0.000	0.000	4.925	0.000	0.020

Feed Line Center of Pressure

Section	Elevation ft	CP_X in	CP_Z in	CP_X Ice in	CP_Z Ice in
L1	132.000-99.750	-0.096	0.284	-0.059	0.175
L2	99.750-49.000	-0.172	0.450	-0.106	0.277
L3	49.000-0.000	-0.301	0.732	-0.186	0.452

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
L1	10	Safety Line 3/8	99.75 - 132.00	1.0000	1.0000
L2	8	241568(5/8")	49.00 - 65.00	1.0000	1.0000
L2	10	Safety Line 3/8	49.00 - 99.75	1.0000	1.0000

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	Crown Castle	R AITHAL

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L3	8	241568(5/8")	0.00 - 49.00	1.0000	1.0000
L3	10	Safety Line 3/8	0.00 - 49.00	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
Lightning Rod 3/4" x 4'	A	From Leg	4.000 0.000 0.000	0.000	136.000	No Ice 0.300	0.300	0.030
Strobe	C	None		0.000	137.000	No Ice 4.500	3.000	0.020
4' x 3" Pipe Mount	C	None		0.000	135.000	No Ice 1.000	1.000	0.029
Side Lighting	A	From Leg	0.500 0.000 0.000	0.000	65.000	No Ice 0.112	0.112	0.005
Side Lighting	B	From Leg	0.500 0.000 0.000	0.000	65.000	No Ice 0.112	0.112	0.005
*								
AIR6449 B41_T-MOBILE w/ Mount Pipe	A	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 5.187	2.705	0.128
AIR6449 B41_T-MOBILE w/ Mount Pipe	B	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 5.187	2.705	0.128
AIR6449 B41_T-MOBILE w/ Mount Pipe	C	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 5.187	2.705	0.128
AIR6449 B41_T-MOBILE w/ Mount Pipe	C	From Face	4.000 0.000 -4.000	0.000	133.000	No Ice 5.187	2.705	0.128
FFVV-65C-R3-V1_TMO w/ Mount Pipe	A	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 12.969	6.196	0.157
FFVV-65C-R3-V1_TMO w/ Mount Pipe	B	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 12.969	6.196	0.157
FFVV-65C-R3-V1_TMO w/ Mount Pipe	C	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 12.969	6.196	0.157
FFVV-65C-R3-V1_TMO w/ Mount Pipe	C	From Face	4.000 0.000 -4.000	0.000	133.000	No Ice 12.969	6.196	0.157
RADIO 4460 B2/B25 B66_TMO	A	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 2.139	1.686	0.109
RADIO 4460 B2/B25 B66_TMO	B	From Leg	4.000 0.000 -4.000	0.000	133.000	No Ice 2.139	1.686	0.109
RADIO 4460 B2/B25	C	From Leg	4.000	0.000	133.000	No Ice 2.139	1.686	0.109

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
B66_TMO			0.000						
RADIO 4460 B2/B25 B66_TMO	C	From Face	-4.000 4.000 0.000	0.000	133.000	No Ice	2.139	1.686	0.109
RADIO 4480 B71_TMO	A	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	2.852	1.383	0.093
RADIO 4480 B71_TMO	B	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	2.852	1.383	0.093
RADIO 4480 B71_TMO	C	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	2.852	1.383	0.093
RADIO 4480 B71_TMO	C	From Face	-4.000 4.000 0.000	0.000	133.000	No Ice	2.852	1.383	0.093
8' x 2" Mount Pipe	A	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	1.900	1.900	0.029
8' x 2" Mount Pipe	B	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	1.900	1.900	0.029
8' x 2" Mount Pipe	C	From Leg	-4.000 4.000 0.000	0.000	133.000	No Ice	1.900	1.900	0.029
8' x 2" Mount Pipe	C	From Face	-4.000 4.000 0.000	0.000	133.000	No Ice	1.900	1.900	0.029
Platform Mount [LP 701-1_HR-1] *	C	None	0.000	0.000	133.000	No Ice	55.580	55.580	3.082
AIR 6449 B77 w/ Mount Pipe	A	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	3.650	2.715	0.108
AIR 6449 B77 w/ Mount Pipe	B	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	3.650	2.715	0.108
AIR 6449 B77 w/ Mount Pipe	C	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	3.650	2.715	0.108
(2) MX06FIT865-02 w/ Mount Pipe	A	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	4.540	4.710	0.104
(2) MX06FIT865-02 w/ Mount Pipe	B	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	4.540	4.710	0.104
(2) MX06FIT865-02 w/ Mount Pipe	C	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	4.540	4.710	0.104
RCMDC-6627-PF-48	A	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	4.056	3.098	0.032
RCMDC-6627-PF-48	B	From Leg	4.000 0.000 0.000	0.000	123.000	No Ice	4.056	3.098	0.032
4449	A	From Leg	4.000	0.000	123.000	No Ice	1.969	1.402	0.071

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
			0.000						
			0.000						
(2) 4449	B	From Leg	4.000	0.000	123.000	No Ice	1.969	1.402	0.071
			0.000						
			0.000						
8843	A	From Leg	4.000	0.000	123.000	No Ice	1.650	1.363	0.072
			0.000						
			0.000						
8843	B	From Leg	4.000	0.000	123.000	No Ice	1.650	1.363	0.072
			0.000						
			0.000						
8843	C	From Leg	4.000	0.000	123.000	No Ice	1.650	1.363	0.072
			0.000						
			0.000						
8' x 2" Mount Pipe	A	From Leg	4.000	0.000	123.000	No Ice	1.900	1.900	0.029
			0.000						
			0.000						
8' x 2" Mount Pipe	B	From Leg	4.000	0.000	123.000	No Ice	1.900	1.900	0.029
			0.000						
			0.000						
8' x 2" Mount Pipe	C	From Leg	4.000	0.000	123.000	No Ice	1.900	1.900	0.029
			0.000						
			0.000						
RMQP-NP+HRK12	C	None		0.000	123.000	No Ice	21.170	19.650	1.485
*									
*									

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice

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Comb. No.	Description
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	Dead+Wind 0 deg - Service
27	Dead+Wind 30 deg - Service
28	Dead+Wind 60 deg - Service
29	Dead+Wind 90 deg - Service
30	Dead+Wind 120 deg - Service
31	Dead+Wind 150 deg - Service
32	Dead+Wind 180 deg - Service
33	Dead+Wind 210 deg - Service
34	Dead+Wind 240 deg - Service
35	Dead+Wind 270 deg - Service
36	Dead+Wind 300 deg - Service
37	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	132 - 99.75	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	20	-10.407	281.101	-1.874
			Max. Mx	8	-10.403	-282.893	-3.440
			Max. My	14	-10.303	-1.627	-297.447
			Max. Vy	8	11.424	-282.893	-3.440
			Max. Vx	14	11.906	-1.627	-297.447
			Max. Torque	8			-2.682
			Max Tension	1	0.000	0.000	0.000
L2	99.75 - 49	Pole	Max. Compression	20	-17.175	915.494	-0.341
			Max. Mx	8	-17.174	-917.400	-5.502
			Max. My	14	-17.127	-3.518	-955.400
			Max. Vy	8	14.501	-917.400	-5.502
			Max. Vx	14	14.969	-3.518	-955.400
			Max. Torque	6			-2.586
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	20	-27.699	1782.113	1.419
L3	49 - 0	Pole	Max. Mx	8	-27.699	-1784.052	-7.461
			Max. My	14	-27.697	-5.396	-1846.313
			Max. Vy	8	17.976	-1784.052	-7.461
			Max. Vx	14	18.415	-5.396	-1846.313
			Max. Torque	6			-2.567

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	14	27.716	-0.034	-18.387
	Max. H _x	20	27.716	17.949	0.034
	Max. H _z	2	27.716	0.034	18.387
	Max. M _x	2	1840.231	0.034	18.387
	Max. M _z	8	1784.052	-17.949	-0.034

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. Torsion	18	2.419	15.528	-9.164
	Min. Vert	11	20.787	-15.561	-9.223
	Min. H _x	8	27.716	-17.949	-0.034
	Min. H _z	14	27.716	-0.034	-18.387
	Min. M _x	14	-1846.313	-0.034	-18.387
	Min. M _z	20	-1782.113	17.949	0.034
	Min. Torsion	6	-2.562	-15.528	9.164

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturing Moment, M _x kip-ft	Overturing Moment, M _z kip-ft	Torque kip-ft
Dead Only	23.097	0.000	0.000	2.467	-0.793	0.001
1.2 Dead+1.0 Wind 0 deg - No Ice	27.716	-0.034	-18.387	-1840.231	3.482	0.871
0.9 Dead+1.0 Wind 0 deg - No Ice	20.787	-0.034	-18.387	-1809.817	3.667	0.871
1.2 Dead+1.0 Wind 30 deg - No Ice	27.716	8.945	-15.907	-1591.181	-888.516	2.009
0.9 Dead+1.0 Wind 30 deg - No Ice	20.787	8.945	-15.907	-1564.980	-873.268	1.957
1.2 Dead+1.0 Wind 60 deg - No Ice	27.716	15.528	-9.164	-914.935	-1542.871	2.562
0.9 Dead+1.0 Wind 60 deg - No Ice	20.787	15.528	-9.164	-900.200	-1516.557	2.472
1.2 Dead+1.0 Wind 90 deg - No Ice	27.716	17.949	0.034	7.460	-1784.052	2.375
0.9 Dead+1.0 Wind 90 deg - No Ice	20.787	17.949	0.034	6.536	-1753.654	2.274
1.2 Dead+1.0 Wind 120 deg - No Ice	27.716	15.561	9.223	928.652	-1547.295	1.549
0.9 Dead+1.0 Wind 120 deg - No Ice	20.787	15.561	9.223	912.090	-1520.896	1.463
1.2 Dead+1.0 Wind 150 deg - No Ice	27.716	9.004	15.940	1601.652	-896.205	0.359
0.9 Dead+1.0 Wind 150 deg - No Ice	20.787	9.004	15.940	1573.678	-880.812	0.310
1.2 Dead+1.0 Wind 180 deg - No Ice	27.716	0.034	18.387	1846.313	-5.396	-0.873
0.9 Dead+1.0 Wind 180 deg - No Ice	20.787	0.034	18.387	1814.201	-5.048	-0.874
1.2 Dead+1.0 Wind 210 deg - No Ice	27.716	-8.945	15.907	1597.244	886.618	-1.870
0.9 Dead+1.0 Wind 210 deg - No Ice	20.787	-8.945	15.907	1569.350	871.898	-1.823
1.2 Dead+1.0 Wind 240 deg - No Ice	27.716	-15.528	9.164	920.977	1540.961	-2.419
0.9 Dead+1.0 Wind 240 deg - No Ice	20.787	-15.528	9.164	904.555	1515.177	-2.334
1.2 Dead+1.0 Wind 270 deg - No Ice	27.716	-17.949	-0.034	-1.420	1782.113	-2.371
0.9 Dead+1.0 Wind 270 deg - No Ice	20.787	-17.949	-0.034	-2.181	1752.254	-2.270
1.2 Dead+1.0 Wind 300 deg - No Ice	27.716	-15.561	-9.223	-922.593	1545.339	-1.686
0.9 Dead+1.0 Wind 300 deg - No Ice	20.787	-15.561	-9.223	-907.721	1519.485	-1.596

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Load Combination	Vertical K	Shear _x K	Shear _y K	Overturning Moment, M _x kip-ft	Overturning Moment, M _y kip-ft	Torque kip-ft
No Ice						
1.2 Dead+1.0 Wind 330 deg - No Ice	27.716	-9.004	-15.940	-1595.572	894.262	-0.499
0.9 Dead+1.0 Wind 330 deg - No Ice	20.787	-9.004	-15.940	-1569.295	879.411	-0.447
Dead+Wind 0 deg - Service	23.097	-0.008	-4.477	-442.353	0.266	0.218
Dead+Wind 30 deg - Service	23.097	2.178	-3.873	-382.229	-215.010	0.481
Dead+Wind 60 deg - Service	23.097	3.781	-2.232	-219.005	-372.892	0.611
Dead+Wind 90 deg - Service	23.097	4.371	0.008	3.575	-431.064	0.575
Dead+Wind 120 deg - Service	23.097	3.789	2.246	225.868	-373.963	0.385
Dead+Wind 150 deg - Service	23.097	2.193	3.882	388.309	-216.865	0.095
Dead+Wind 180 deg - Service	23.097	0.008	4.477	447.363	-1.875	-0.217
Dead+Wind 210 deg - Service	23.097	-2.178	3.873	387.239	213.401	-0.471
Dead+Wind 240 deg - Service	23.097	-3.781	2.232	224.014	371.283	-0.602
Dead+Wind 270 deg - Service	23.097	-4.371	-0.008	1.433	429.453	-0.574
Dead+Wind 300 deg - Service	23.097	-3.789	-2.246	-220.859	372.352	-0.392
Dead+Wind 330 deg - Service	23.097	-2.193	-3.882	-383.299	215.254	-0.102

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-23.097	0.000	-0.000	23.097	-0.000	0.000%
2	-0.034	-27.716	-18.387	0.034	27.716	18.387	0.000%
3	-0.034	-20.787	-18.387	0.034	20.787	18.387	0.000%
4	8.945	-27.716	-15.907	-8.945	27.716	15.907	0.000%
5	8.945	-20.787	-15.907	-8.945	20.787	15.907	0.000%
6	15.528	-27.716	-9.164	-15.528	27.716	9.164	0.000%
7	15.528	-20.787	-9.164	-15.528	20.787	9.164	0.000%
8	17.949	-27.716	0.034	-17.949	27.716	-0.034	0.000%
9	17.949	-20.787	0.034	-17.949	20.787	-0.034	0.000%
10	15.561	-27.716	9.223	-15.561	27.716	-9.223	0.000%
11	15.561	-20.787	9.223	-15.561	20.787	-9.223	0.000%
12	9.004	-27.716	15.940	-9.004	27.716	-15.940	0.000%
13	9.004	-20.787	15.940	-9.004	20.787	-15.940	0.000%
14	0.034	-27.716	18.387	-0.034	27.716	-18.387	0.000%
15	0.034	-20.787	18.387	-0.034	20.787	-18.387	0.000%
16	-8.945	-27.716	15.907	8.945	27.716	-15.907	0.000%
17	-8.945	-20.787	15.907	8.945	20.787	-15.907	0.000%
18	-15.528	-27.716	9.164	15.528	27.716	-9.164	0.000%
19	-15.528	-20.787	9.164	15.528	20.787	-9.164	0.000%
20	-17.949	-27.716	-0.034	17.949	27.716	0.034	0.000%
21	-17.949	-20.787	-0.034	17.949	20.787	0.034	0.000%
22	-15.561	-27.716	-9.223	15.561	27.716	9.223	0.000%
23	-15.561	-20.787	-9.223	15.561	20.787	9.223	0.000%
24	-9.004	-27.716	-15.940	9.004	27.716	15.940	0.000%
25	-9.004	-20.787	-15.940	9.004	20.787	15.940	0.000%
26	-0.008	-23.097	-4.477	0.008	23.097	4.477	0.000%
27	2.178	-23.097	-3.873	-2.178	23.097	3.873	0.000%
28	3.781	-23.097	-2.232	-3.781	23.097	2.232	0.000%
29	4.371	-23.097	0.008	-4.371	23.097	-0.008	0.000%
30	3.789	-23.097	2.246	-3.789	23.097	-2.246	0.000%
31	2.193	-23.097	3.882	-2.193	23.097	-3.882	0.000%
32	0.008	-23.097	4.477	-0.008	23.097	-4.477	0.000%
33	-2.178	-23.097	3.873	2.178	23.097	-3.873	0.000%
34	-3.781	-23.097	2.232	3.781	23.097	-2.232	0.000%
35	-4.371	-23.097	-0.008	4.371	23.097	0.008	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
36	-3.789	-23.097	-2.246	3.789	23.097	2.246	0.000%
37	-2.193	-23.097	-3.882	2.193	23.097	3.882	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00026565
3	Yes	5	0.00000001	0.00012369
4	Yes	6	0.00000001	0.00057022
5	Yes	6	0.00000001	0.00017866
6	Yes	6	0.00000001	0.00048687
7	Yes	6	0.00000001	0.00014948
8	Yes	5	0.00000001	0.00095393
9	Yes	5	0.00000001	0.00042823
10	Yes	6	0.00000001	0.00056849
11	Yes	6	0.00000001	0.00017637
12	Yes	6	0.00000001	0.00053698
13	Yes	6	0.00000001	0.00016480
14	Yes	5	0.00000001	0.00035844
15	Yes	5	0.00000001	0.00016486
16	Yes	6	0.00000001	0.00050417
17	Yes	6	0.00000001	0.00015391
18	Yes	6	0.00000001	0.00057829
19	Yes	6	0.00000001	0.00018125
20	Yes	5	0.00000001	0.00086710
21	Yes	5	0.00000001	0.00038954
22	Yes	6	0.00000001	0.00050105
23	Yes	6	0.00000001	0.00015437
24	Yes	6	0.00000001	0.00054283
25	Yes	6	0.00000001	0.00016827
26	Yes	4	0.00000001	0.00037729
27	Yes	5	0.00000001	0.00013151
28	Yes	5	0.00000001	0.00008894
29	Yes	4	0.00000001	0.00075256
30	Yes	5	0.00000001	0.00013335
31	Yes	5	0.00000001	0.00011055
32	Yes	4	0.00000001	0.00040958
33	Yes	5	0.00000001	0.00009417
34	Yes	5	0.00000001	0.00014103
35	Yes	4	0.00000001	0.00072719
36	Yes	5	0.00000001	0.00008995
37	Yes	5	0.00000001	0.00010935

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	132 - 99.75	26.494	32	2.155	0.024
L2	102.5 - 49	14.720	32	1.518	0.007

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L3	53.25 - 0	3.607	32	0.661	0.002

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
137.000	Strobe	32	26.494	2.155	0.024	12706
136.000	Lightning Rod 3/4" x 4'	32	26.494	2.155	0.024	12706
135.000	4' x 3" Pipe Mount	32	26.494	2.155	0.024	12706
133.000	AIR6449 B41 T-MOBILE w/ Mount Pipe	32	26.494	2.155	0.024	12706
123.000	AIR 6449 B77 w/ Mount Pipe	32	22.677	1.956	0.018	7059
65.000	Side Lighting	32	5.326	0.839	0.001	3134

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	132 - 99.75	108.763	14	8.744	0.102
L2	102.5 - 49	60.689	14	6.254	0.028
L3	53.25 - 0	14.890	14	2.728	0.007

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
137.000	Strobe	14	108.763	8.744	0.102	3261
136.000	Lightning Rod 3/4" x 4'	14	108.763	8.744	0.102	3261
135.000	4' x 3" Pipe Mount	14	108.763	8.744	0.102	3261
133.000	AIR6449 B41 T-MOBILE w/ Mount Pipe	14	108.763	8.744	0.102	3261
123.000	AIR 6449 B77 w/ Mount Pipe	14	93.198	7.971	0.076	1811
65.000	Side Lighting	14	22.014	3.472	0.006	766

Compression Checks

Pole Design Data

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	132 - 99.75 (1)	TP21.66x13.5x0.188	32.250	0.000	0.0	12.365	-10.303	723.336	0.014
L2	99.75 - 49 (2)	TP34.12x20.589x0.313	53.500	0.000	0.0	32.467	-17.127	1899.300	0.009
L3	49 - 0 (3)	TP45.9x32.42x0.313	53.250	0.000	0.0	45.217	-27.698	2645.200	0.010

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{ux} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M _{uy} kip-ft	φM _{uy} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
L1	132 - 99.75 (1)	TP21.66x13.5x0.188	297.451	376.543	0.790	0.000	376.543	0.000
L2	99.75 - 49 (2)	TP34.12x20.589x0.313	955.408	1581.500	0.604	0.000	1581.500	0.000
L3	49 - 0 (3)	TP45.9x32.42x0.313	1846.325	2750.642	0.671	0.000	2750.642	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T _u kip-ft	φT _n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	132 - 99.75 (1)	TP21.66x13.5x0.188	11.906	217.001	0.055	0.876	394.837	0.002
L2	99.75 - 49 (2)	TP34.12x20.589x0.313	14.969	569.790	0.026	0.875	1633.333	0.001
L3	49 - 0 (3)	TP45.9x32.42x0.313	18.415	793.560	0.023	0.873	3168.150	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	Ratio $\frac{M_{uy}}{\phi M_{uy}}$	Ratio $\frac{V_u}{\phi V_n}$	Ratio $\frac{T_u}{\phi T_n}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	132 - 99.75 (1)	0.014	0.790	0.000	0.055	0.002	0.807	1.050	4.8.2 ✓
L2	99.75 - 49 (2)	0.009	0.604	0.000	0.026	0.001	0.614	1.050	4.8.2 ✓
L3	49 - 0 (3)	0.010	0.671	0.000	0.023	0.000	0.682	1.050	4.8.2 ✓

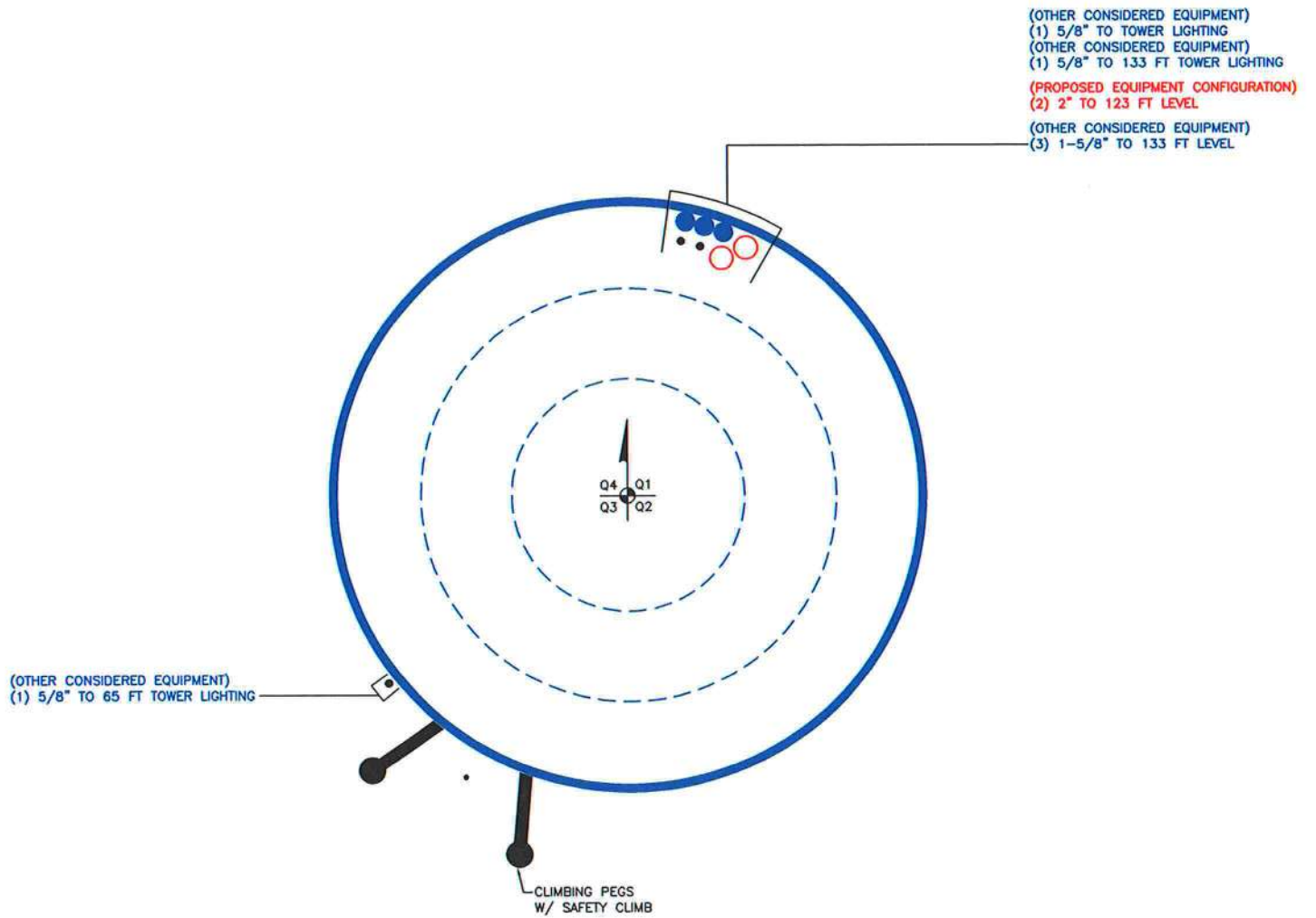
Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP _{allow} K	% Capacity	Pass Fail
-------------	-----------------	-------------------	------	---------------------	--------	--------------------------	---------------	--------------

<i>inxTower</i> B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job	121641.005.01 - Lake City Airport, FL (BU# 825272)	Page	13 of 13
	Project		Date	18:58:19 07/06/22
	Client	Crown Castle	Designed by	R AITHAL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	132 - 99.75	Pole	TP21.66x13.5x0.188	1	-10.303	759.503	76.9	Pass
L2	99.75 - 49	Pole	TP34.12x20.589x0.313	2	-17.127	1994.265	58.5	Pass
L3	49 - 0	Pole	TP45.9x32.42x0.313	3	-27.698	2777.460	65.0	Pass
							Summary	
							Pole (L1)	Pass
							RATING =	Pass

APPENDIX B
BASE LEVEL DRAWING



BUSINESS UNIT: 825272

APPENDIX C
ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

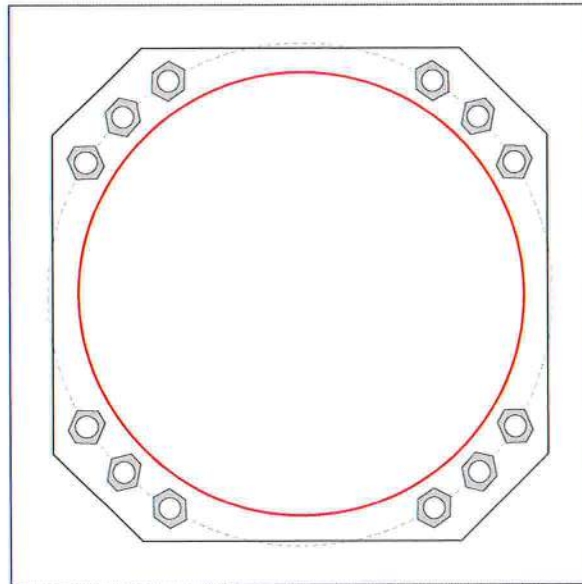


Site Info	
BU #	825272
Site Name	Lake City Airport, FL
Order #	605523, Rev# 1

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{br} (in)	0.625

Applied Loads	
Moment (kip-ft)	1846.32
Axial Force (kips)	27.70
Shear Force (kips)	18.41

*TIA-222-H Section 15.5 Applied



Connection Properties		Analysis Results	
Anchor Rod Data		Anchor Rod Summary (units of kips, kip-in)	
(12) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 52" BC		$P_{u,t} = 139.61$	$\phi P_{n,t} = 243.75$ Stress Rating
Anchor Spacing: 6 in		$V_u = 1.53$	$\phi V_n = 149.1$ 54.5%
		$M_u = n/a$	$\phi M_n = n/a$ Pass
Base Plate Data		Base Plate Summary	
51" W x 2.5" Plate (A572-60; $F_y=60$ ksi, $F_u=75$ ksi); Clip: 9 in		Max Stress (ksi):	26.9 (Flexural)
Stiffener Data		Allowable Stress (ksi):	54
N/A		Stress Rating:	47.4% Pass
Pole Data			
45.9" x 0.3125" 18-sided pole (A572-65; $F_y=65$ ksi, $F_u=80$ ksi)			

Drilled Pier Foundation

BU #: 1825272
 Site Name: Lake City Airport, FL
 Order Number: 605523, Rev# 1
 TIA-222 Revision: H
 Tower Type: Monopole



Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>
Additional Longitudinal Rebar	
Input Effective Depths (else Actual):	<input type="checkbox"/>
Shear Design Options	<input type="checkbox"/>
Check Shear along Depth of Pier:	<input checked="" type="checkbox"/>
Utilize Shear-Friction Methodology:	<input type="checkbox"/>
Override Critical Depth:	<input type="checkbox"/>

Go to Soil Calculations

Analysis Results			
Soil Lateral Check		Compression	Uplift
D _{red} (ft. from TOC)		6.67	-
Soil Safety Factor		2.50	-
Max Moment (kip-ft)		1960.67	-
Rating*		50.7%	-
Soil Vertical Check		Compression	Uplift
Skin Friction (kips)		248.81	-
End Bearing (kips)		95.43	-
Weight of Concrete (kips)		100.11	-
Total Capacity (kips)		344.24	-
Axial (kips)		127.83	-
Rating*		35.4%	-
Reinforced Concrete Flexure		Compression	Uplift
Critical Depth (ft. from TOC)		6.73	-
Critical Moment (kip-ft)		1960.64	-
Critical Moment Capacity		4232.08	-
Rating*		44.1%	-
Reinforced Concrete Shear		Compression	Uplift
Critical Depth (ft. from TOC)		15.47	-
Critical Shear (kip)		244.38	-
Critical Shear Capacity		408.28	-
Rating*		57.0%	-

Structural Foundation Rating*	57.0%
Soil Interaction Rating*	50.7%

*Rating per TIA-222-H Section 15.5

Applied Loads	
Comp.	Uplift
Moment (kip-ft)	1846.32
Axial Force (kips)	27.72
Shear Force (kips)	18.39

Material Properties	
Concrete Strength, f _c :	3 ksi
Rebar Strength, F _y :	60 ksi
Tie Yield Strength, F _y :	40 ksi

Rebar 2, F _y Override (ksi)	
--	--

Rebar & Pier Options

Embedded Pole Inputs

Belled Pier Inputs

Pier Design Data	
Depth	25 ft
Ext. Above Grade	0.5 ft
Pier Section 1	
From 0.5' above grade to 25' below grade	
Pier Diameter	6 ft
Rebar Quantity	32
Rebar Size	9
Rebar Cage Diameter	64 in
Tie Size	4
Tie Spacing	12 in

Soil Profile

# of Layers	7
-------------	---

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y _{soil} (pcf)	Y _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	5	5	115	150	0	0	0.000	0.000	0.00			Cohesionless
2	5	6	1	115	150	0	33	0.000	0.000	0.60	0.45		Cohesionless
3	6	10	4	115	150	0	35	0.000	0.000	1.00	0.75		Cohesionless
4	10	11	1	110	150	0	32	0.000	0.000	1.30	0.98		Cohesionless
5	11	16	5	47.6	87.6	0	32	0.000	0.000	1.30	0.98		Cohesionless
6	16	21	5	47.6	87.6	0	30	0.000	0.000	0.80	0.60		Cohesionless
7	21	25	4	37.6	87.6	0.5	0	0.28	0.28	0.30	4.5		Cohesive

Groundwater Depth	11
-------------------	----

ASCE 7 Hazards Report

Address:
No Address at This
Location

Standard: ASCE/SEI 7-16
Risk Category: II
Soil Class: D - Default (see
Section 11.4.3)

Elevation: 167.28 ft (NAVD 88)
Latitude: 30.170417
Longitude: -82.586028



Wind

Results:

Wind Speed	118 Vmph
10-year MRI	75 Vmph
25-year MRI	84 Vmph
50-year MRI	90 Vmph
100-year MRI	98 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2
Date Accessed: Fri May 20 2022

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

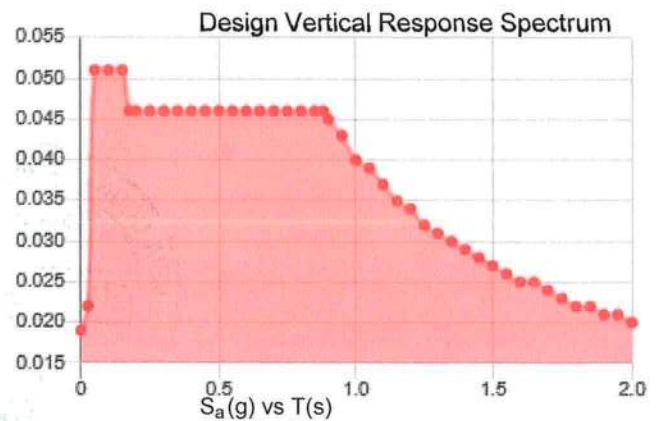
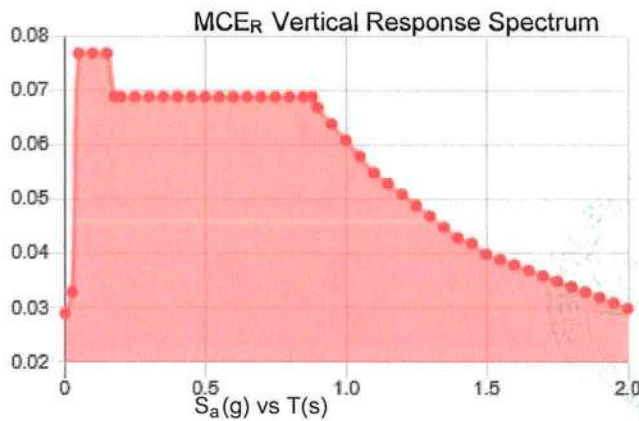
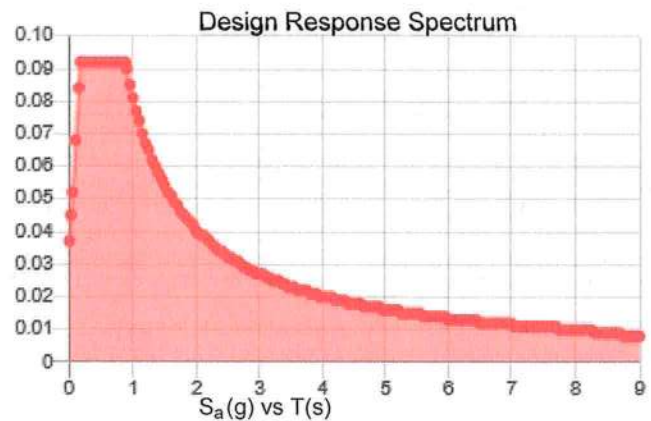
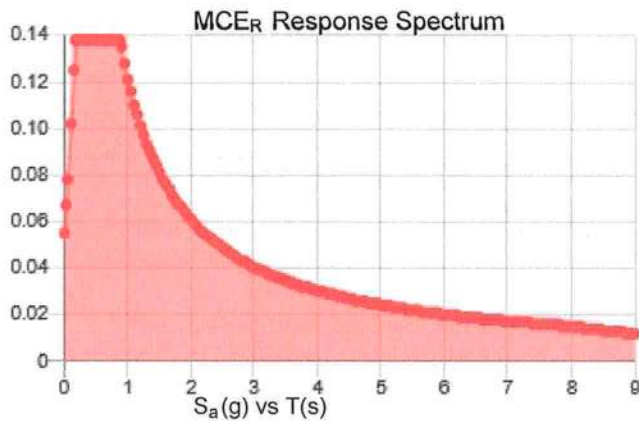
Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Site Soil Class: D - Default (see Section 11.4.3)

Results:

S_S :	0.086	S_{D1} :	0.081
S_1 :	0.051	T_L :	8
F_a :	1.6	PGA :	0.041
F_v :	2.4	PGA _M :	0.066
S_{MS} :	0.138	F_{PGA} :	1.6
S_{M1} :	0.121	I_e :	1
S_{DS} :	0.092	C_v :	0.7

Seismic Design Category B



Data Accessed: Fri May 20 2022

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.

Ice

Results:

Ice Thickness: 0.25 in.
Concurrent Temperature: 25 F
Gust Speed 30 mph

Data Source: Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

Date Accessed: Fri May 20 2022

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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VERIZON WIRELESS SITE NUMBER: 712924
VERIZON WIRELESS SITE NAME:
SITE TYPE:
TOWER HEIGHT:

BUSINESS UNIT #:825272
CC LAKE CITY AIRPORT
MONOPOLE
133'-0"

SITE ADDRESS:
COUNTY:
JURISDICTION:

VERIZON: NEW SITE BUILD

SITE INFORMATION

CROWN CASTLE USA INC. LAKE CITY AIRPORT
SITE NAME:
SITE ADDRESS: 336 SE NEWELL DR
LAKE CITY, FL 32025
COUNTY: COLUMBIA
MAP/PARCEL #: 02-4S-17-0779-104
AREA OF CONSTRUCTION: EXISTING
LATITUDE: 30° 10' 13.50"
LONGITUDE: -82° 35' 09.70"
LAT/LONG TYPE: NAD83
GROUND ELEVATION: 168 FT.
CURRENT ZONING: COLUMBIA COUNTY
JURISDICTION: U
OCCUPANCY CLASSIFICATION: U
TYPE OF CONSTRUCTION: IIB
A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION
PROPERTY OWNER: LJOY USA LLC
336 PENNINGTON CT
ROYAL PALM BEACH, FL 33411
TOWER OWNER: CROWN CASTLE
6420 CONGRESS AVENUE, STE 2000
BOCA RATON, FL 33487
CARRIER/APPLICANT: VERIZON WIRELESS
7701 TELECOM PARKWAY
TEMPLE TERRACE, FL 33637
ELECTRIC PROVIDER: FPL
TELCO PROVIDER: AT&T

DRAWING INDEX

SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1.1	SITE PLAN
C-1.2	PROPOSED EQUIPMENT PLAN
C-2	ANTENNA LAYOUT & TOWER ELEVATION
C-3	FINAL ANTENNA & CABLE CONFIGURATION
C-4	EQUIPMENT SPECS
C-5	SITE DETAILS
C-6	SITE DETAILS
C-7	SITE DETAILS
C-8	SITE DETAILS
C-9	GENERATOR SPECS
C-10	PROPANE TANK DETAILS
G-1	PROPOSED UTILITY ROUTING
G-2	PROPOSED GROUNDING PLAN
G-3	PANEL SCHEDULE & ONE-LINE DIAGRAM
G-4	ELECTRICAL & GROUNDING DETAILS
G-5	ELECTRICAL & GROUNDING DETAILS

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 11X17. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

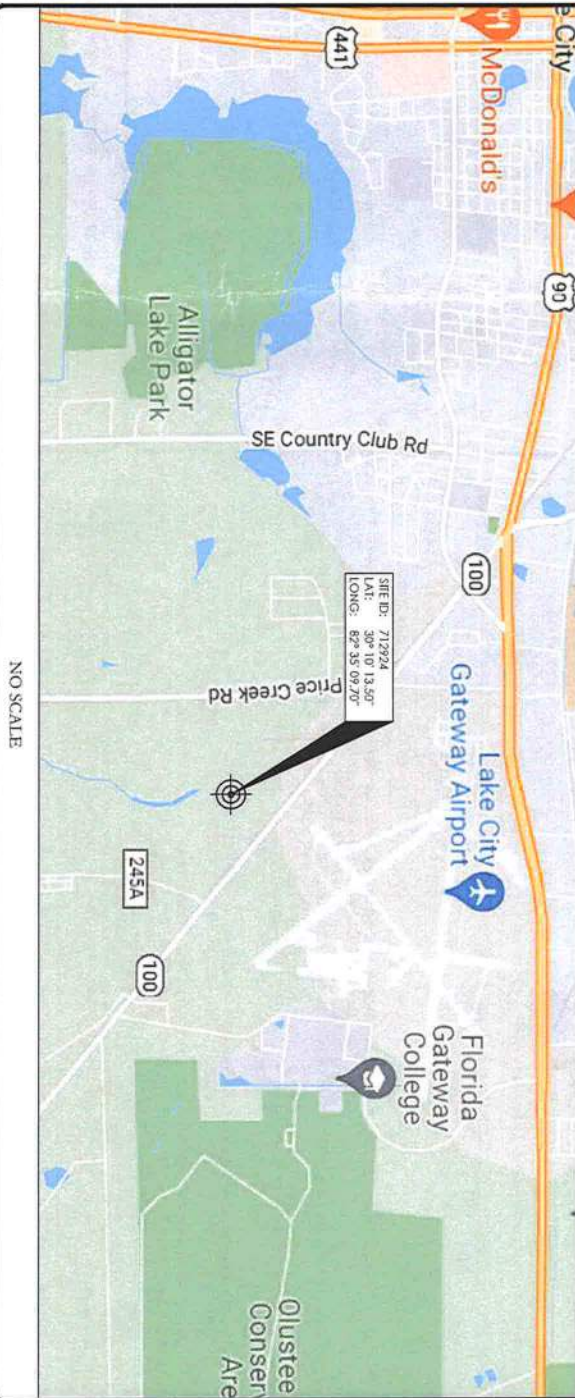
PROJECT TEAM

AKE FIRM: TOWERSOURCE, INC.
1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
CROWN CASTLE
USA INC. DISTRICT
CONTACTS: 6420 CONGRESS AVE SUITE 2000
BOCA RATON, FL 33487
PETER VERDECCHIA - PROJECT MANAGER
PETER.VERDECCHIA@CROWNCASTLE.COM

PROJECT DESCRIPTION

THE PURPOSE OF THIS PROJECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY.
TOWER SCOPE OF WORK:
• INSTALL (9) ANTENNAS
• INSTALL (2) OVPs
• INSTALL (6) RHHs
• INSTALL (2) 2" HYBRID CABLES
• INSTALL (1) PLATFORM ANTENNA MOUNT
GROUND SCOPE OF WORK:
• INSTALL (1) 4' x 10' CONCRETE EQUIPMENT PAD
• INSTALL (1) 4' x 10' CONCRETE PAD FOR LP TANK
• INSTALL (1) 4' x 10' CONCRETE GENERATOR PAD
• INSTALL (1) EQUIPMENT H-FRAME

LOCATION MAP



APPLICABLE CODES/REFERENCE DOCUMENTS

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:
CODE TYPE CODE
2020 FLORIDA BUILDING CODE 7TH EDITION/2015 IRC
MECHANICAL 2020 FLORIDA BUILDING CODE 7TH EDITION/2015 IMC
ELECTRICAL 2020 FLORIDA BUILDING CODE 7TH EDITION/2017 NEC

REFERENCE DOCUMENTS:

STRUCTURAL ANALYSIS: PROVIDED WITH APP
DATED:
MOUNT ANALYSIS: DATED:
ORDER ID: 605523
REVISION: 0
RFDS REVISION: ---
DATED: 04/29/2022

ANALYSIS CRITERIA:

APPLICABLE CODES: TIA 222-H / ASCE 7.16
WIND SPEED: V = 118 MPH (ULTIMATE 3 SECOND GUST)
EXPOSURE CATEGORY: C
RISK CATEGORY: II
TOPOGRAPHIC CATEGORY: I
CREST HEIGHT: 0
K_z = 1.0
SEISMIC RESPONSE: NA



verizon
7701 E. TELECOM PARKWAY
TEMPLE TERRACE, FL 33637

CROWN CASTLE
8000 AVALON BLVD, SUITE 700
ALPHARETTA, GA 30009

towersource
A NextEdge Company
1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
TEL: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924
BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025
EXISTING 133'-0" MONOPOLE

ISSUED FOR:

REV	DATE	DRAWN	DESCRIPTION	DES/QA
0	08/05/22	FS	ISSUED FOR FINALS	MAIT

JAVAD K. PARSA
No 87002
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Digitally signed by javad k parsa
Date: 2022.08.05 11:58:14 -0400
This item has been digitally signed and sealed by JAVAD K. PARSA, P.E. ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.
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SHEET NUMBER: T-1
REVISION: 0

NOTE:
PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 786-7011 & CROWN CONSTRUCTION MANAGER

FLOOD NOTE:
THE SUBJECT PROPERTY APPEARS TO BE
WITHIN FLOOD ZONE "X" PER FIRM MAP
NUMBER 12023C0315C WITH AN EFFECTIVE
DATE OF 02/04/2009.

7701 E. TELECOM PARKWAY
TEMPLE TERRACE, FL 33637

8000 AVALON BLVD, SUITE 700
ALPHARETTA, GA 30009

 **towersource**
A NextEdge Company
1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
TEL: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924

BU #: 825272

LAKE CITY AIRPORT

336 SE NEWELL DR
LAKE CITY, FL 32025

EXISTING 135'-0" MONOPOLE

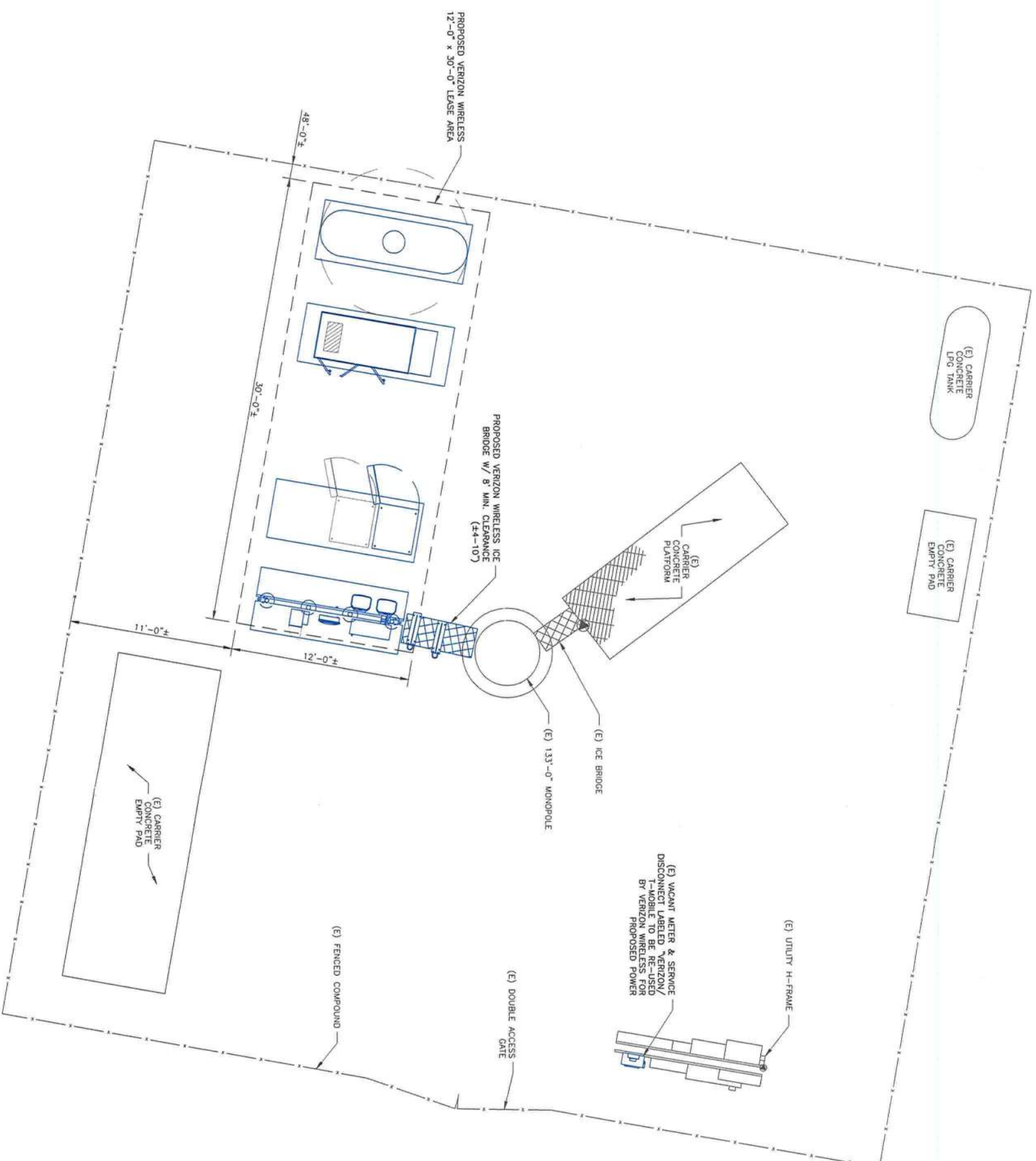
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SHEET NUMBER: REVISION

C-1.1
0

1
SITE PLAN
SCALE: 1/4"=1'-0" (FULL SIZE)
1/8"=1'-0" (1"x1")



verizon

7701 E. TELECOM PARKWAY
TEMPLE TERRACE, FL 33637

CROWN
CASTLE

8000 AVALON BLVD, SUITE 700
ALPHARETTA, GA 30009

towersource
A NextEdge Company

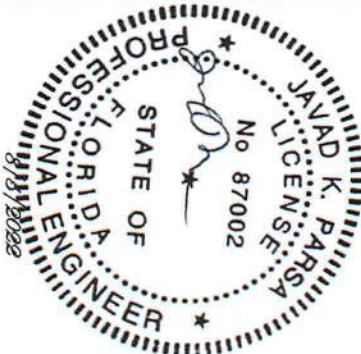
1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
TEL: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924

BU #: 825272
336 SE NEWELL DR
LAKE CITY, FL 32025

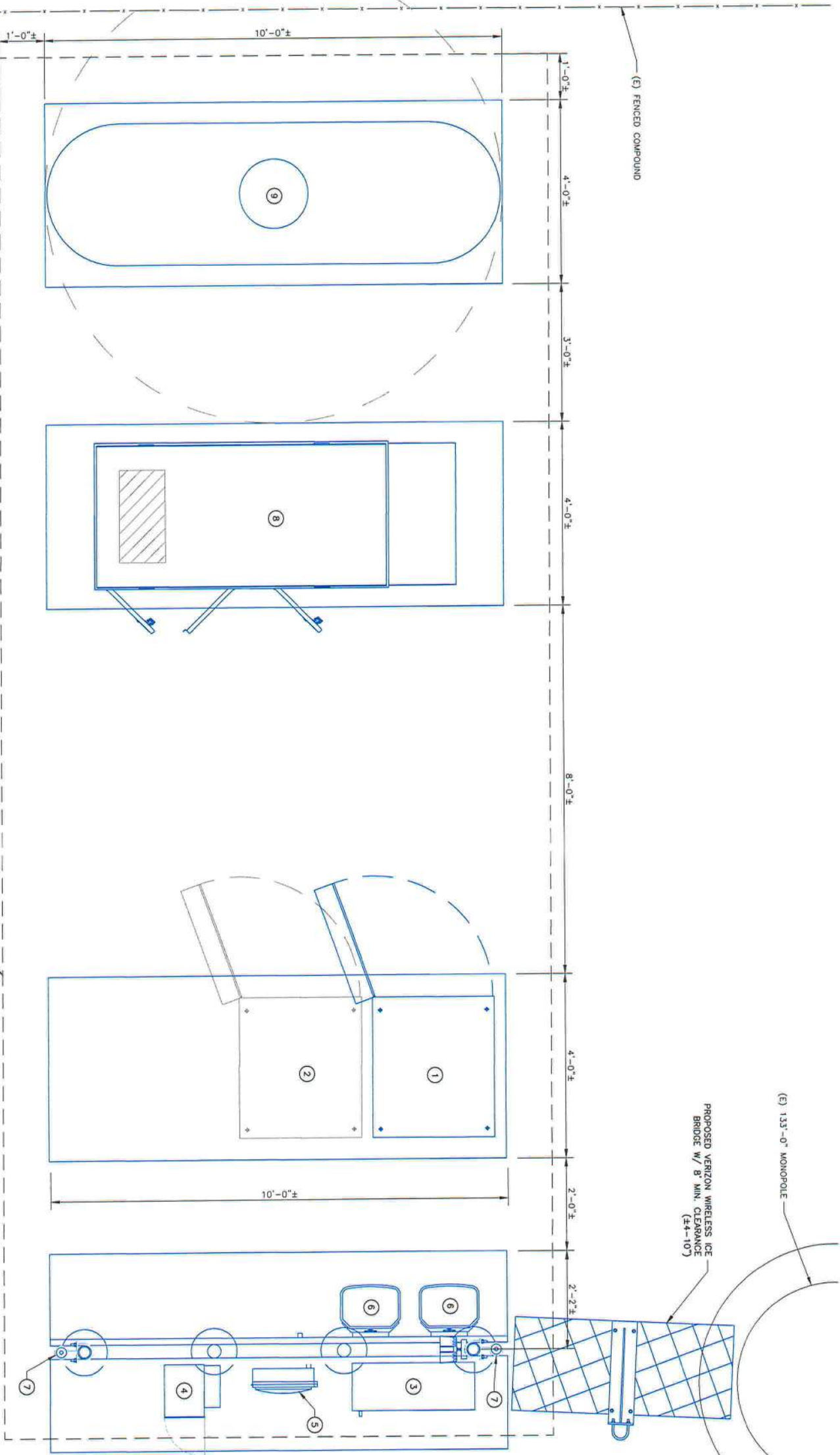
EXISTING 133'-0" MONOPOLE

ISSUED FOR:			
REV	DATE	DRWN	DESCRIPTION
0	06/05/23	FS	ISSUED FOR FINALS
			SLAT



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SHEET NUMBER: C-1.2
REVISION: 0



PROPOSED VERIZON WIRELESS EQUIPMENT	
1	PROPOSED VERIZON WIRELESS EQUIPMENT CABINET ON PROPOSED 4'-0" X 10'-0" SLAB
2	FUTURE VERIZON WIRELESS EQUIPMENT CABINET
3	PROPOSED VERIZON WIRELESS 200A, 120/240V 1φ ILC
4	PROPOSED VERIZON WIRELESS FIBER CABINET
5	PROPOSED VERIZON WIRELESS CIENNA BOX
6	PROPOSED VERIZON WIRELESS OVP
7	PROPOSED VERIZON WIRELESS GPS ANTENNA, TYP. OF 2
8	PROPOSED VERIZON WIRELESS 50kW LP GENERATOR ON PROPOSED 4'-0" X 10'-0" SLAB
9	PROPOSED VERIZON WIRELESS 500 GALLON LP TANK W/ INTEGRATED SHUT-OFF VALVE ON PROPOSED 4'-0" X 10'-0" SLAB

PROPOSED EQUIPMENT PLAN

2 SCALE: 3/4"=1'-0" (FULL SIZE)
3/8"=1'-0" (11x17)



7701 E. TELECOM PARKWAY
TEMPLE TERRACE, FL 33637

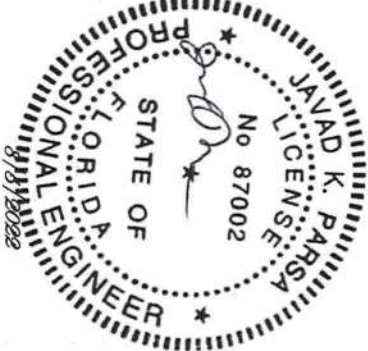
8000 AVALON BLVD, SUITE 700
ALPHARETTA, GA 30009

1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
TEL: 678-990-2358

VERIZON WIRELESS
SITE NUMBER: 712924
BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025

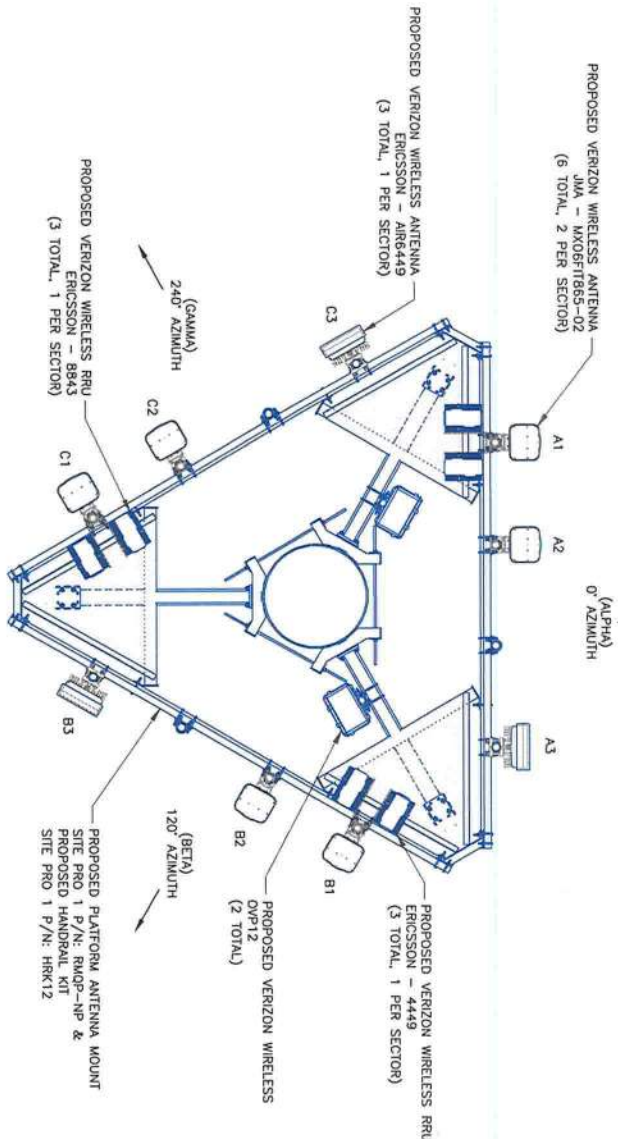
EXISTING 133'-0" MONOPOLE

ISSUED FOR:				
REV	DATE	DRAWN	DESCRIPTION	DRAWN/QA
0	08/05/22	JFS	ISSUED FOR FINALS	MAIT

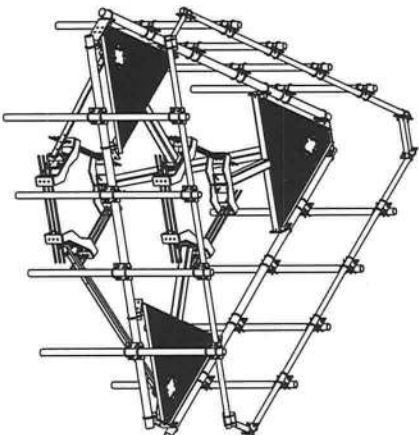


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SHEET NUMBER: C-2
REVISION: 0



2 FINAL ANTENNA PLAN
SCALE: NOT TO SCALE



3 SITE PRO 1 - RMOP-NP & HRK12
SCALE: NOT TO SCALE

VERIZON WIRELESS EQUIPMENT
ANTENNA CL: 123'-0"
MOUNT CL: 123'-0"

ANY AND ALL TOWER
MOUNTED EQUIPMENT MUST
NOT TRAP OR INTERFERE W/
EXISTING SAFETY CLIMB

1 TOWER ELEVATION
SCALE: NOT TO SCALE

7701 E. TELECOM PARKWAY
TEMPLE TERRACE, FL 33637



8000 AVALON BLVD, SUITE 700
ALPHARETTA, GA 30009



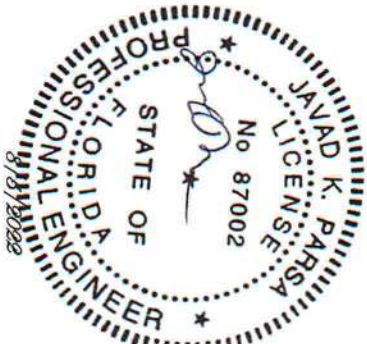
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1355 WINDWARD CONCOURSE
SUITE 410
ALPHARETTA, GA 30005
TEL: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924
BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025

EXISTING 133'-0" MONOPOLE

ISSUED FOR:				
REV	DATE	DRAWN	DESCRIPTION	DES/QA
0	06/05/22	RS	ISSUED FOR PERMITS	JAT



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C-3

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FINAL CABLE SCHEDULE				
STATUS	CABLE TYPE	SIZE	LENGTH	QTY
NEW	HYBRID	2"	175'-0"±	2
TOTAL CABLE QTY:				2

NOTE:
REFER TO RFDS FOR ELECTRICAL
& MECHANICAL DOWNTILTS

FINAL EQUIPMENT SCHEDULE						
SECTOR	STATUS	ANTENNA MANUFACTURER	ANTENNA MODEL	ANTENNA CENTERLINE	AZIMUTH	TOWER EQUIPMENT MANUFACTURER
A1	NEW	JMA	MX06FTB65-02	123'-0"	0°	ERICSSON
A2	NEW	JMA	MX06FTB65-02	123'-0"	0°	RAYCAP
A3	NEW	ERICSSON	AIR6449	123'-0"	0°	-
B1	NEW	JMA	MX06FTB65-02	123'-0"	120°	ERICSSON
B2	NEW	JMA	MX06FTB65-02	123'-0"	120°	RAYCAP
B3	NEW	ERICSSON	AIR6449	123'-0"	120°	-
C1	NEW	JMA	MX06FTB65-02	123'-0"	240°	ERICSSON
C2	NEW	JMA	MX06FTB65-02	123'-0"	240°	-
C3	NEW	ERICSSON	AIR6449	123'-0"	240°	-

1

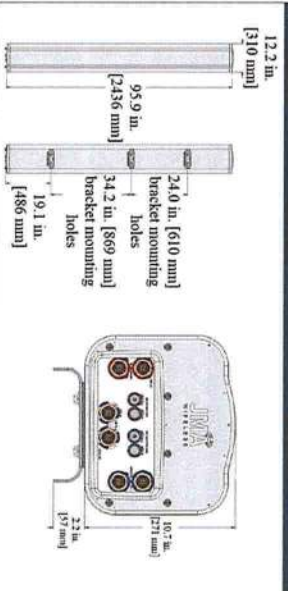
FINAL EQUIPMENT SCHEDULE

SCALE: NOT TO SCALE

JMA
MWAVE™ X-Pol Flex-Port Antenna

Model: MX06FT1865-02

Dimensions (height/width/depth, inches (mm))	56.9 x 22.1 x 3.7 (2448 x 510 x 273)
Shipping dimensions (height/width/depth, inches (mm))	106 x 20 x 15 (2692 x 508 x 381)
No. of RF input ports, connector type, and location	6 x 4.3-10 female, bottom
RF connector torque	96 in-lb (10.85 N-m or 8 in-lb-ft)
Net antenna weight, lb (kg)	7.1 (3.2)
Shipping weight, lb (kg)	120 (54.4)
Antenna mounting and downlink is included with antenna	915003116, 915003119 (resin bracket)
Net weight of the mounting and downlink, lb (kg)	26 (11.8)
Range of mechanical up/down (ft)	-2 to 12"
Range of electrical up/down (ft)	150 (45.7)
Frontal, lateral, and rear wind loading @ 150 km/h, lb (N)	118 (517), 91 (405), 147 (654)
Equivalent flat plate @ 100 mph and Cat-2, sq ft	2.17



Ordering information	
Antenna model	Description
MX06FT1865-02	6P X-Pol MWAVE FT 6S - 2.17' 10" NET, 4.3' 10.8" SPT
Optional accessories	
MSD cables	MSD cables for MSD connections
PCU-1000 NET controller	Stand-alone controller for NET control and configuration

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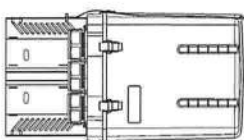
02/22/19
Page 2

1 JMA - MX06FT1865-02
SCALE: NOT TO SCALE



ERICSSON - 4449 B12 B71
WEIGHT: 62.7 LBS
DIMENSIONS (HxWxD): 17.9x13.2x9.3 IN.

3 ERICSSON 4449 B12 B71
SCALE: NOT TO SCALE



RAYCAP - OVP12/6627
WEIGHT: 32.0 LBS
SIZE (HxWxD): 28.93x15.73x10.31 IN.

2 RAYCAP - OVP12/6627
SCALE: NOT TO SCALE



ERICSSON - RADIO 8843
WEIGHT: 71.9 LBS
SIZE (HxWxD): 18.0x13.2x11.1 IN.

4 ERICSSON - RADIO 8843
SCALE: NOT TO SCALE

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ALPHARETTA, GA 30005
TEL.: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924
BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025
EXISTING 133'-0" MONOPOLE

ISSUED FOR:			
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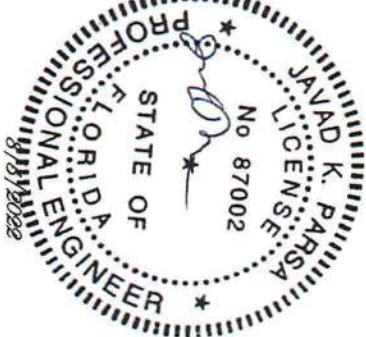
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VERIZON WIRELESS
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EXISTING 133'-0" MONOPOLE

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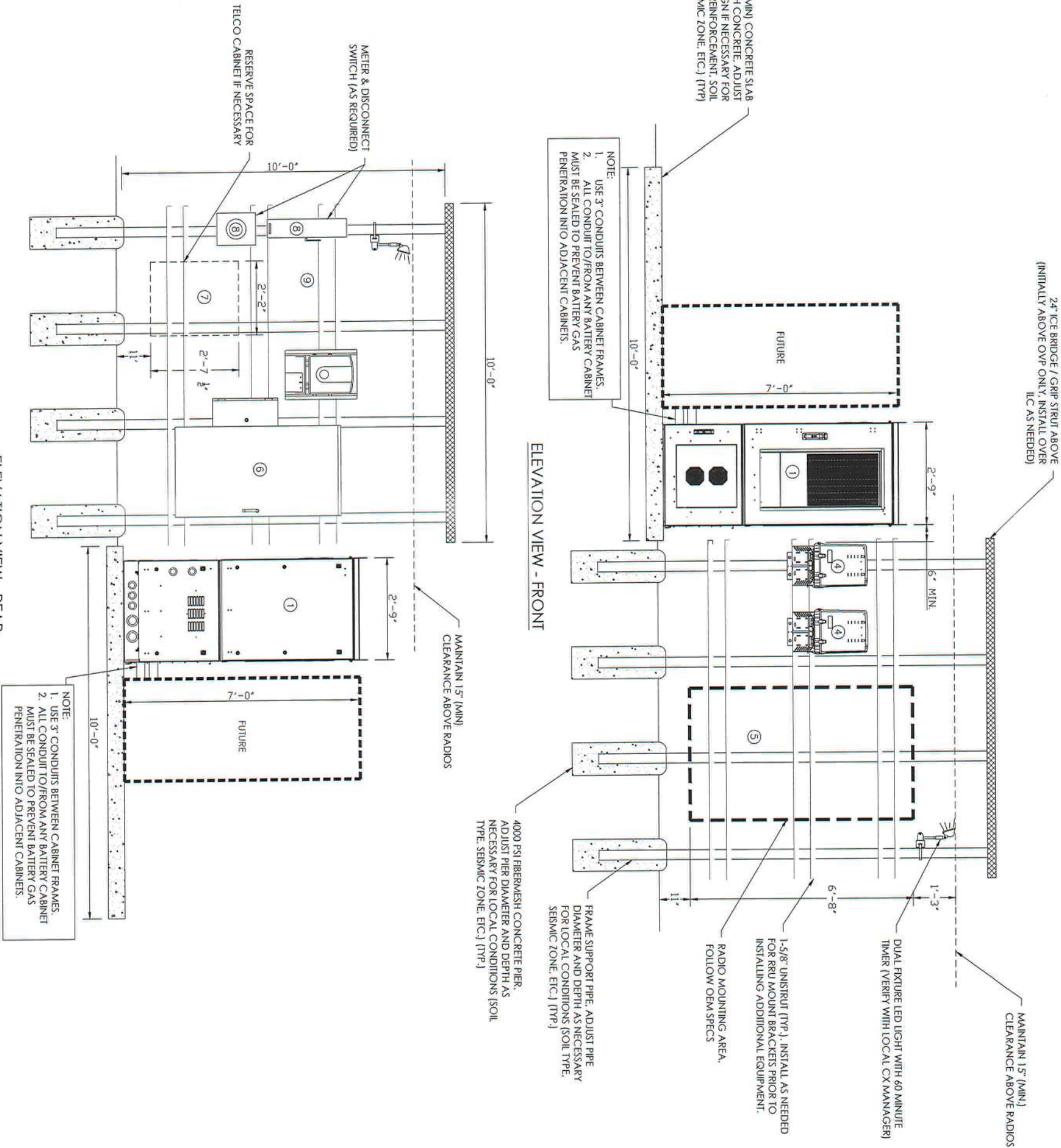


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EQUIPMENT ELEVATIONS
1 SCALE: NOT TO SCALE

ELEVATION VIEW - REAR



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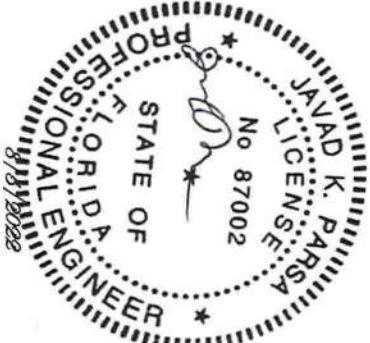
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VERIZON WIRELESS
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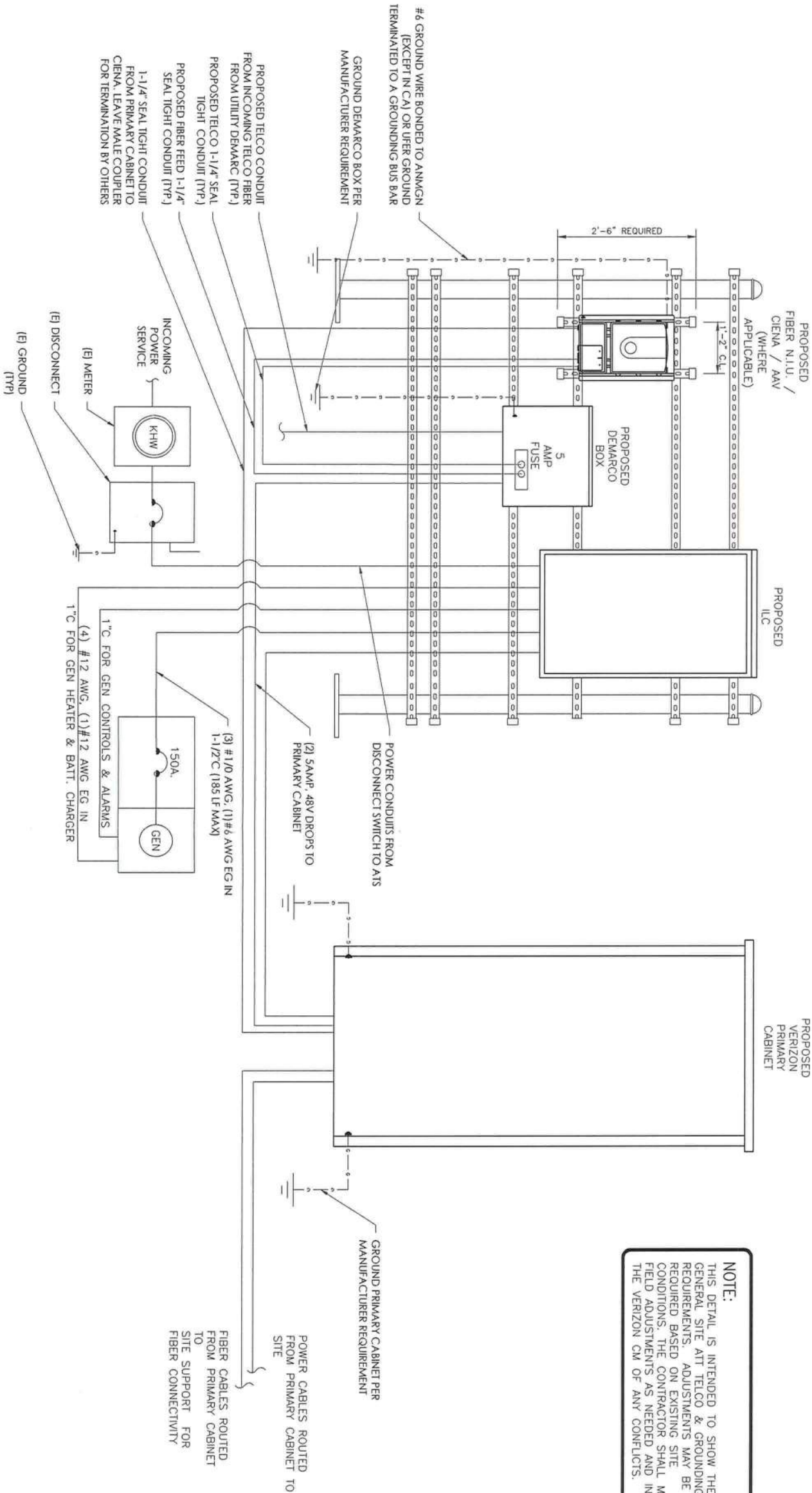


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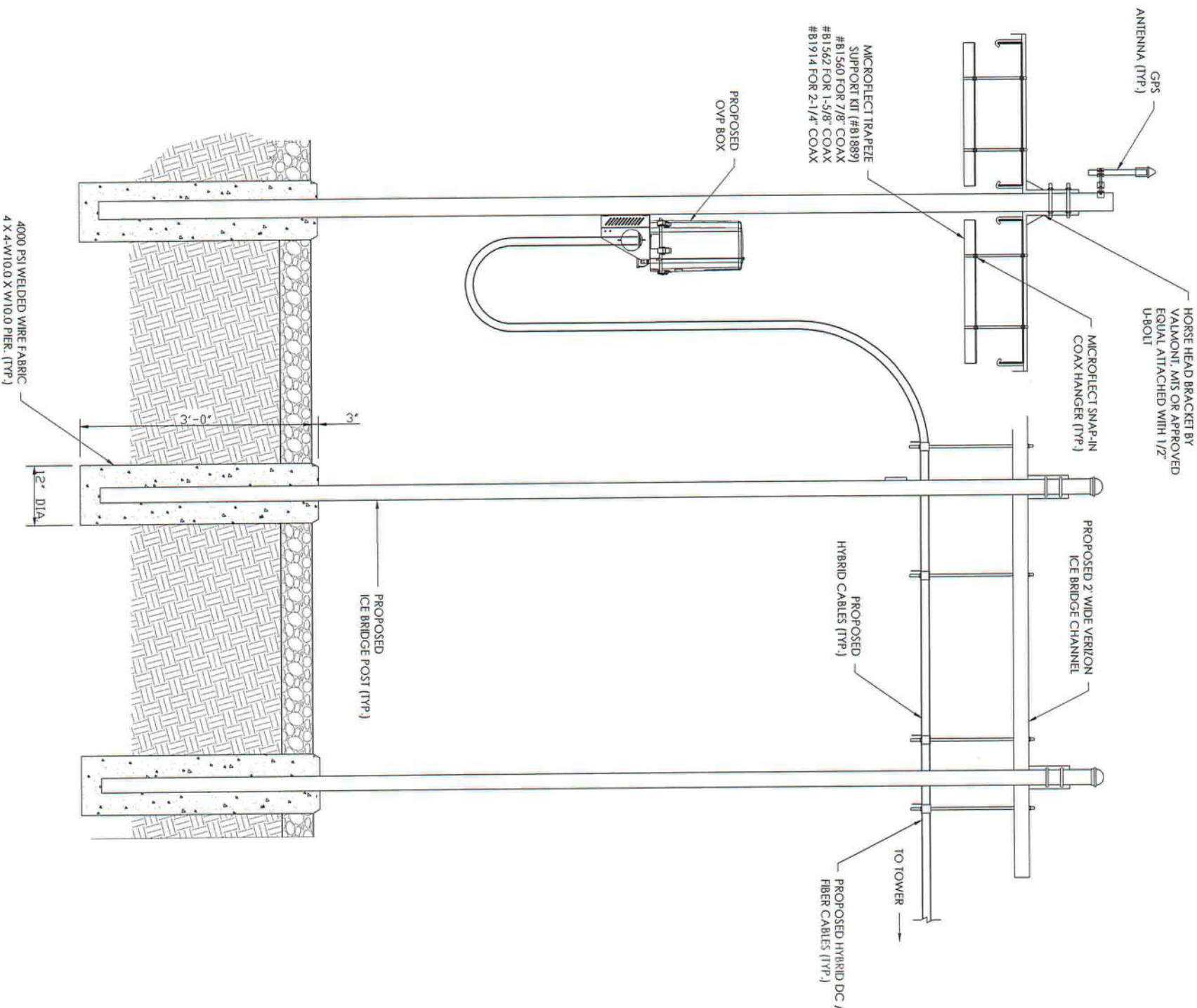
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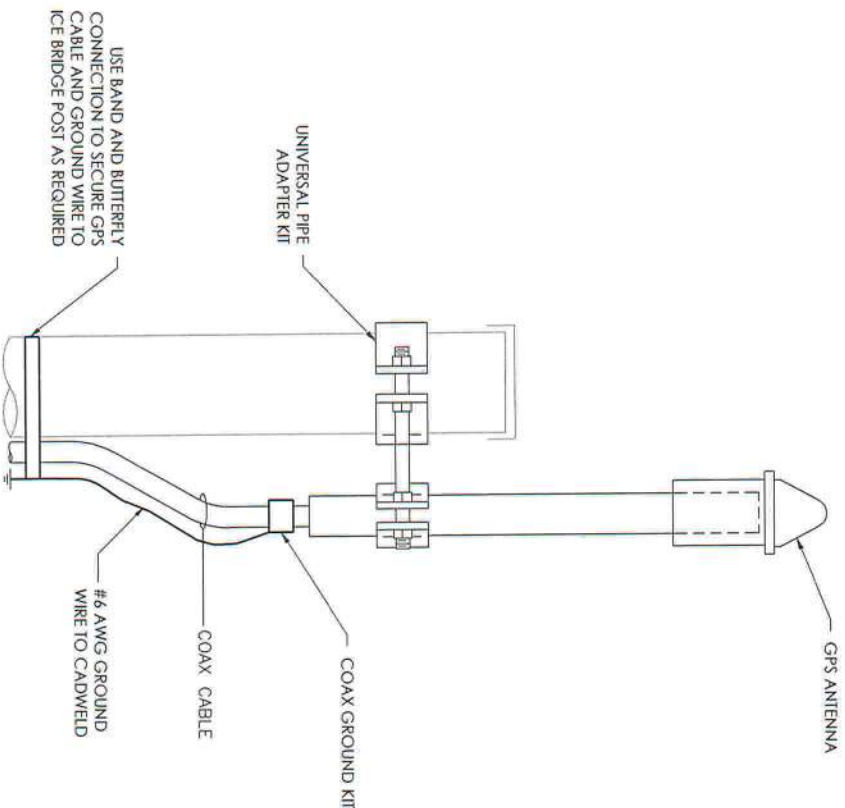
NOTE:
THIS DETAIL IS INTENDED TO SHOW THE GENERAL SITE ATT TELCO & GROUNDING REQUIREMENTS. ADJUSTMENTS MAY BE REQUIRED BASED ON EXISTING SITE CONDITIONS. THE CONTRACTOR SHALL MAKE FIELD ADJUSTMENTS AS NEEDED AND INFORM THE VERIZON CM OF ANY CONFLICTS.



1 EQUIPMENT POWER, TELCO, & GROUNDING DIAGRAM
SCALE: NOT TO SCALE



2 GPS ANTENNA MOUNTING DETAILS
SCALE: NOT TO SCALE



1 ICE BRIDGE DETAILS
SCALE: NOT TO SCALE

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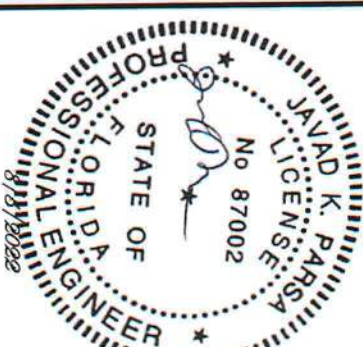
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VERIZON WIRELESS
SITE NUMBER: 712924

BU #: 825272
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336 SE NEWELL DR.
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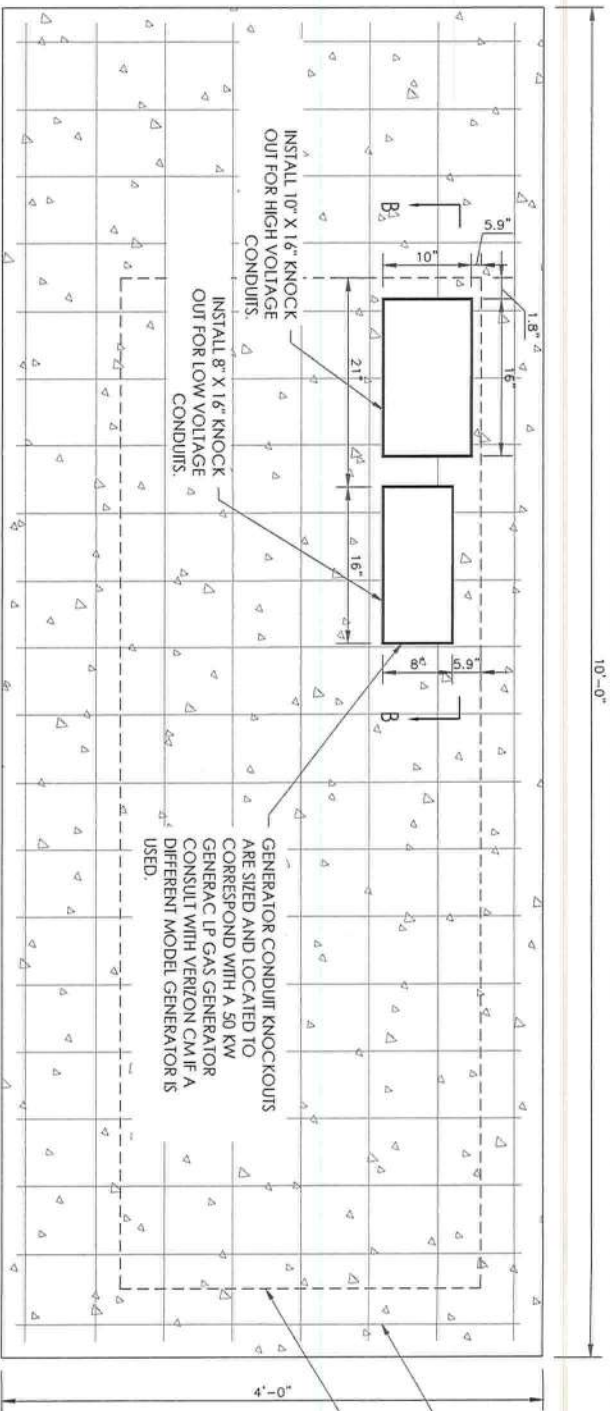
EXISTING 133'-0" MONOPOLE

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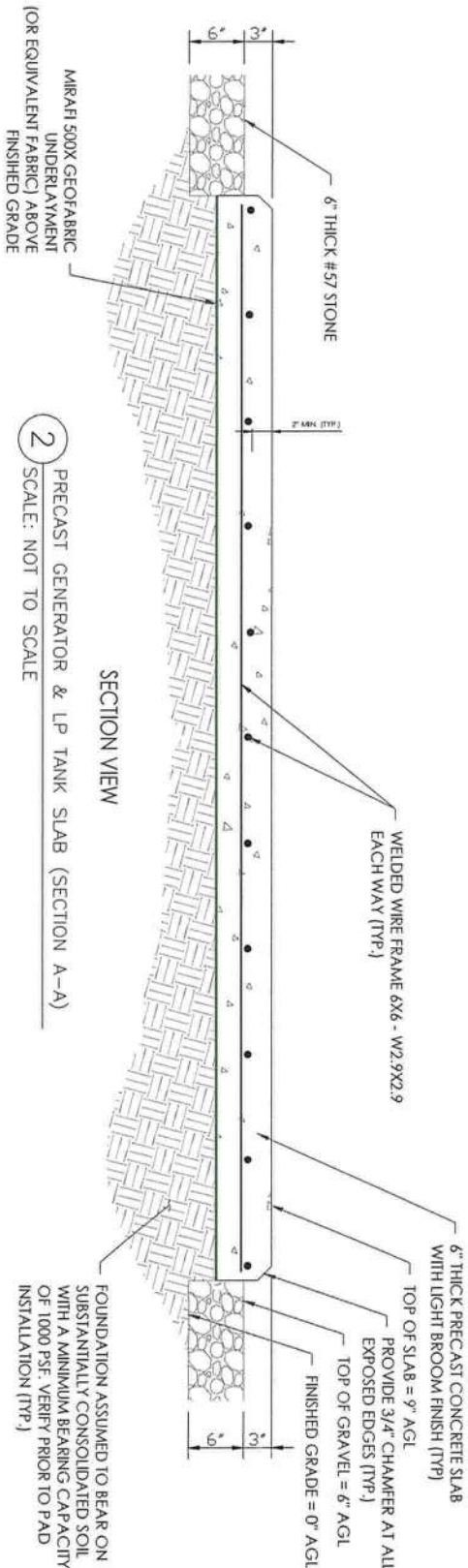
SHEET NUMBER: **C-7** REVISION: **0**



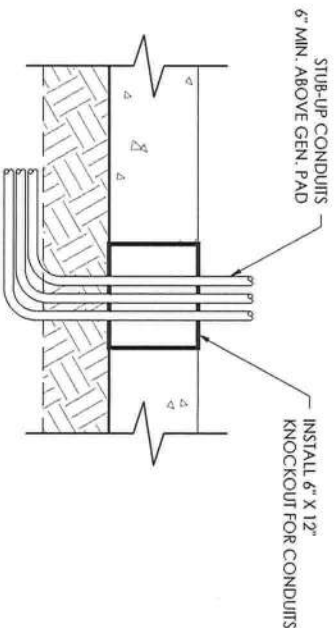
4" MIN. DISTANCE NEEDED FROM CONCRETE EDGES BEFORE DRILLING ANCHORS TO SUPPORT CABINETS OR ICE PROTECTION

1 CONCRETE SLAB DETAILS
SCALE: NOT TO SCALE

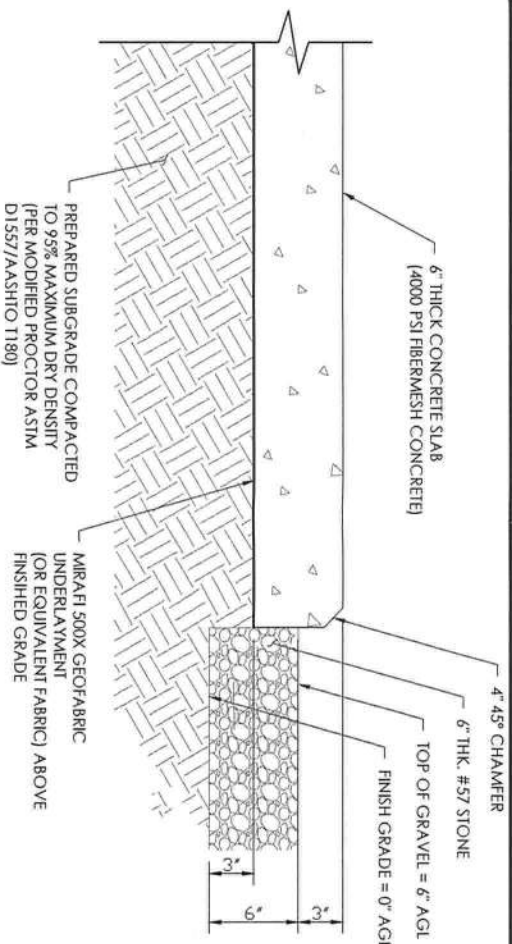
LIFTING NOTE:
THE CONTRACTOR SHALL COORDINATE THE INSTALLATION OF EMBEDDED HOISTING HOOKS OR REBAR AS REQUIRED PRIOR TO FABRICATION.



2 PRECAST GENERATOR & LP TANK SLAB (SECTION A-A)
SCALE: NOT TO SCALE



3 CONDUIT STUB-UP FOR GENERATOR SLAB (SECTION B-B)
SCALE: NOT TO SCALE



4 EQUIPMENT CONCRETE SLAB SECTION
SCALE: NOT TO SCALE

REINFORCED CONCRETE NOTES:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185, IBC 2006.
- PRECAST CONCRETE FOR SLABS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONCRETE TESTING IS NOT REQUIRED UNLESS NOTED OTHERWISE OR REQUIRED BY THE JURISDICTION HAVING AUTHORITY.
- SUMP - 4" MIN. / 6" MAX. AIR ENTRAINMENT - 2% TO 3% BY VOLUME
- CLASSES OF CONCRETE

CLASS	28 DAY STRENGTH (PSI)	WATER/CEMENT RATIO	MAX PLACEMENT LOCATION	NOTES
TYPE I	3000	0.55	PRECAST SLABS	NORMAL WEIGHT
TYPE III *	5000	0.45	SITE CAST SLABS & POST FOOTINGS	HIGH EARLY STRENGTH

* IF REQUIRED BY THE CONSTRUCTION SCHEDULE THE CONTRACTOR MAY SUBSTITUTE TYPE III HIGH EARLY STRENGTH CONCRETE WITH THE APPROVAL OF THE CONSTRUCTION MANAGER.

- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPICES FOR REBAR SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD. UNO, LAP'S FOR WELDED WIRE FABRIC SHALL BE AT LEAST 8 INCHES, UNO.
- THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
CONCRETE CAST AGAINST EARTH 3"
CONCRETE EXPOSED TO EARTH OR WEATHER 2"
#6 AND LARGER 1-1/2"
#5 AND SMALLER & W.W.F. 1-1/2"
- MAXIMUM COARSE AGGREGATE SIZE SHALL BE 3/4".
- MAINTAIN THE TEMPERATURE OF CAST IN PLACE CONCRETE AT BETWEEN 50 AND 90 DEGREES FAHRENHEIT. IF COLDER OR HOTTER CONDITIONS EXIST, THE CONCRETE MIX DESIGN SHALL BE ADJUSTED ACCORDINGLY.
- DO NOT USE RETEMPERED CONCRETE.
- INSTALLATION OF CONCRETE ANCHORS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN SPECIFICATIONS. THE ANCHOR BOLT, DOWEL, OR ROD SHALL CONFORM TO THE ANCHOR MANUFACTURER'S SPECIFICATIONS FOR MATERIAL STRENGTH, EMBEDMENT DEPTH, SPACING, AND EDGE DISTANCE OR AS DETAILED ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR ENGINEERING APPROVAL. WHEN DRILLING HOLES IN CONCRETE, EXPANSION BOLTS SHALL BE PROVIDED BY RAMSET/REDHEAD, HITL, OR APPROVED EQUAL. IF THE MANUFACTURER'S SPECIFICATIONS AND DETAILS ARE FOUND TO CONFLICT WITH THAT SHOWN HEREIN, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.
- THE CONTRACTOR SHALL VERIFY FROST LINE AND FOOTING DEPTH TO REQUIREMENTS WITH THE JURISDICTION HAVING AUTHORITY PRIOR TO CONSTRUCTION.

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VERIZON WIRELESS
SITE NUMBER: 712924

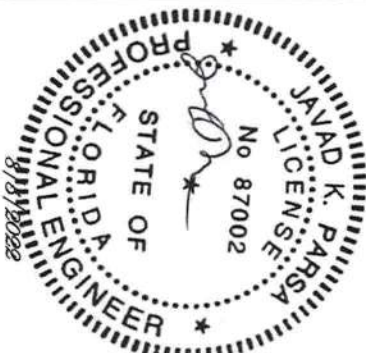
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EXISTING 133'-0" MONOPOLE

ISSUED FOR:

REV	DATE	DRAWN	DESCRIPTION	DWG. QTY
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VERIZON WIRELESS
SITE NUMBER: 712924

BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025

EXISTING 133'-0" MONOPOLE

ISSUED FOR:				
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0	08/05/22	FS	ISSUED FOR FINALS	MAT



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SHEET NUMBER: REVISION:

C-9
O[illegible][illegible][illegible]

5x/4 vs. Voltage Dip			
	30%	208-210 VAC	30%
277-480 VAC	98		75
K0263112-8721	98	600/650/12-1471	75
K0263112-8721	72.4	600/640/12-1471	95

[illegible]

GENERATOR SET

- Special Testing
- Factory Box

 SGS OILS INDUSTRIAL SPARK-IGNITION GENERATOR SET		EPR, Certified Stationary OPERATING DATA	
FUEL CONSUMPTION RATES*			
Heavy Gas – 50% (m/h)		Propane Vapor – 50% (m/h)	
Preheated	Stock	Preheated	Stock
50% 12.0 (0.5)	50% 12.0 (0.5)	50% 12.0 (0.5)	50% 12.0 (0.5)
65% 13.0 (0.5)	65% 13.0 (0.5)	65% 13.0 (0.5)	65% 13.0 (0.5)
85% 14.0 (0.5)	85% 14.0 (0.5)	85% 14.0 (0.5)	85% 14.0 (0.5)
100% 15.0 (0.5)	100% 15.0 (0.5)	100% 15.0 (0.5)	100% 15.0 (0.5)
* Fuel inputs available with maximum rated generator rates at 100% load			
At 75% (m/h) (New Stationary)		At 75% (m/h) (m/h)	
Generator	Stock	Generator	Stock
50% 12.0 (0.5)	50% 12.0 (0.5)	50% 12.0 (0.5)	50% 12.0 (0.5)
65% 13.0 (0.5)	65% 13.0 (0.5)	65% 13.0 (0.5)	65% 13.0 (0.5)
85% 14.0 (0.5)	85% 14.0 (0.5)	85% 14.0 (0.5)	85% 14.0 (0.5)
100% 15.0 (0.5)	100% 15.0 (0.5)	100% 15.0 (0.5)	100% 15.0 (0.5)
Maximum Generator Load = 75% (m/h) (New Stationary)			
Maximum Stationary Rating = 100% (m/h)			

[illegible]

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SPEC SHEET

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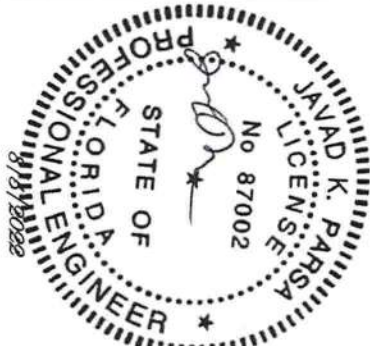
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LAKE CITY AIRPORT

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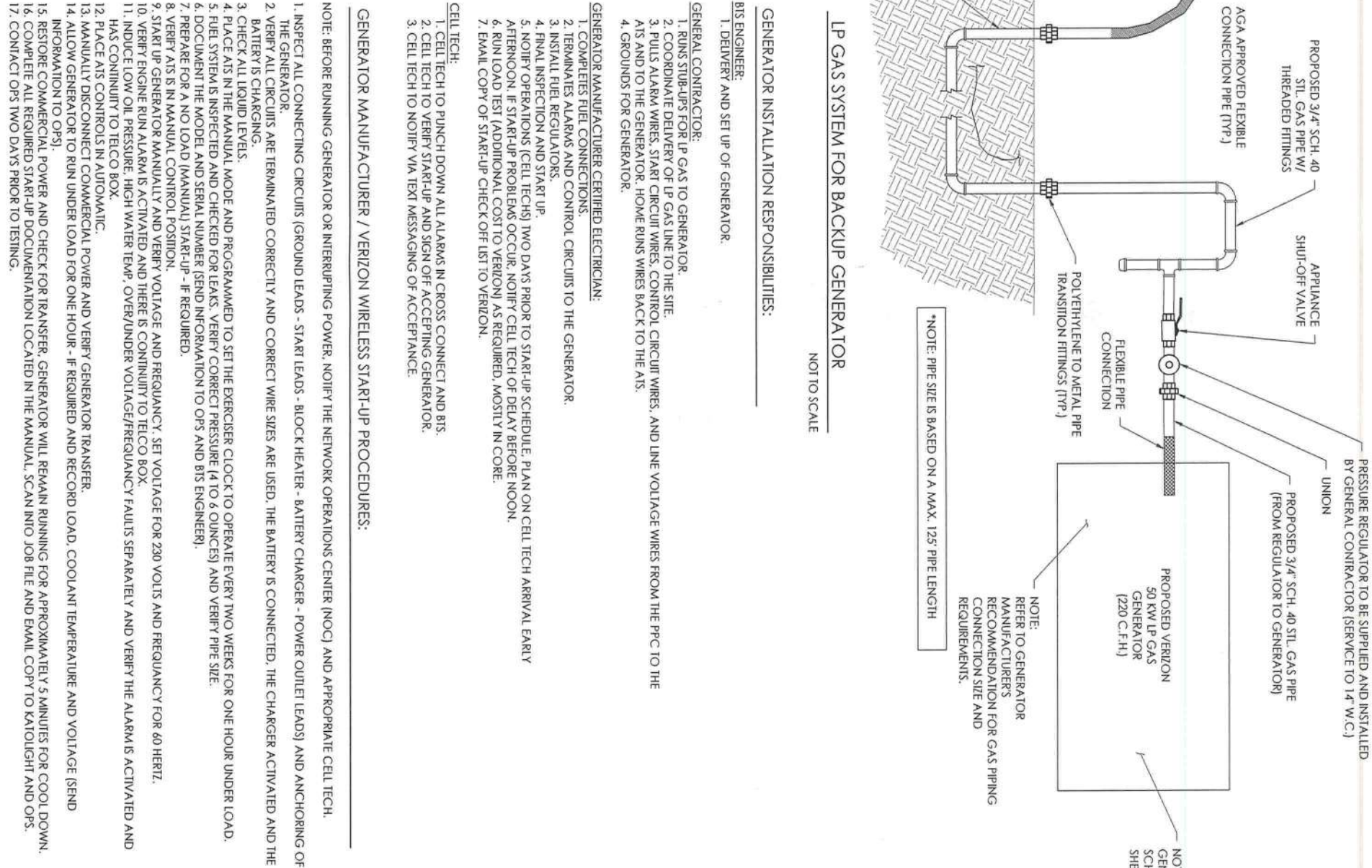
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0	06/05/22	FS	ISSUED FOR FINALS	MAV



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SHEET NUMBER: C-10
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GENERATOR INSTALLATION RESPONSIBILITIES:

- BIS ENGINEER:
1. DELIVERY AND SET UP OF GENERATOR.

GENERAL CONTRACTOR:

1. RUNS STUB-UPS FOR LP GAS TO GENERATOR.
2. COORDINATE DELIVERY OF LP GAS LINE TO THE SITE.
3. PULS ALARM WIRES, START CIRCUIT WIRES, CONTROL CIRCUIT WIRES, AND LINE VOLTAGE WIRES FROM THE PPC TO THE AIS AND TO THE GENERATOR. HOME RUNS WIRES BACK TO THE AIS.
4. GROUNDS FOR GENERATOR.

GENERATOR MANUFACTURER CERTIFIED ELECTRICIAN:

1. COMPLETES FUEL CONNECTIONS.
2. TERMINATES ALARMS AND CONTROL CIRCUITS TO THE GENERATOR.
3. INSTALL FUEL REGULATORS.
4. FINAL INSPECTION AND START UP.
5. NOTIFY OPERATIONS (CELL TECHS) TWO DAYS PRIOR TO STARTUP SCHEDULE. PLAN ON CELL TECH ARRIVAL EARLY AFTERNOON. IF START-UP PROBLEMS OCCUR, NOTIFY CELL TECH OF DELAY BEFORE NOON.
6. RUN LOAD TEST (ADDITIONAL COST TO VERIZON) AS REQUIRED, MOSTLY IN CORE.
7. EMAIL COPY OF START-UP CHECK OFF LIST TO VERIZON.

CELL TECH:

1. CELL TECH TO PUNCH DOWN ALL ALARMS IN CROSS CONNECT AND BIS.
2. CELL TECH TO VERIFY START-UP AND SIGN OFF ACCEPTING GENERATOR.
3. CELL TECH TO NOTIFY VIA TEXT MESSAGING OF ACCEPTANCE.

GENERATOR MANUFACTURER / VERIZON WIRELESS START-UP PROCEDURES:

- NOTE: BEFORE RUNNING GENERATOR OR INTERRUPTING POWER, NOTIFY THE NETWORK OPERATIONS CENTER (NOC) AND APPROPRIATE CELL TECH.
1. INSPECT ALL CONNECTING CIRCUITS (GROUND LEADS - START LEADS - BLOCK HEATER - BATTERY CHARGER - POWER OUTLET LEADS) AND ANCHORING OF THE GENERATOR.
 2. VERIFY ALL CIRCUITS ARE TERMINATED CORRECTLY AND CORRECT WIRE SIZES ARE USED. THE BATTERY IS CONNECTED, THE CHARGER ACTIVATED AND THE BATTERY IS CHARGING.
 3. CHECK ALL LIQUID LEVELS.
 4. PLACE AIS IN THE MANUAL MODE AND PROGRAMMED TO SET THE EXERCISER CLOCK TO OPERATE EVERY TWO WEEKS FOR ONE HOUR UNDER LOAD.
 5. FUEL SYSTEM IS INSPECTED AND CHECKED FOR LEAKS. VERIFY CORRECT PRESSURE (4 TO 6 OUNCES) AND VERIFY PIPE SIZE.
 6. DOCUMENT THE MODEL AND SERIAL NUMBER (SEND INFORMATION TO OPS AND BIS ENGINEER).
 7. PREPARE FOR A NO LOAD (MANUAL) START-UP - IF REQUIRED.
 8. VERIFY AIS IS IN MANUAL CONTROL POSITION.
 9. START UP GENERATOR MANUALLY AND VERIFY VOLTAGE AND FREQUENCY. SET VOLTAGE FOR 220 VOLTS AND FREQUENCY FOR 60 HERTZ.
 10. VERIFY ENGINE RUN ALARM IS ACTIVATED AND THERE IS CONTINUITY TO TELCO BOX.
 11. INDUCE LOW OIL PRESSURE. HIGH WATER TEMP, OVER/UNDER VOLTAGE/FREQUENCY FAULTS SEPARATELY AND VERIFY THE ALARM IS ACTIVATED AND HAS CONTINUITY TO TELCO BOX.
 12. PLACE AIS CONTROLS IN AUTOMATIC.
 13. MANUALLY DISCONNECT COMMERCIAL POWER AND VERIFY GENERATOR TRANSFER.
 14. ALLOW GENERATOR TO RUN UNDER LOAD FOR ONE HOUR - IF REQUIRED AND RECORD LOAD, COOLANT TEMPERATURE AND VOLTAGE (SEND INFORMATION TO OPS).
 15. RESTORE COMMERCIAL POWER AND CHECK FOR TRANSFER. GENERATOR WILL REMAIN RUNNING FOR APPROXIMATELY 5 MINUTES FOR COOL DOWN.
 16. COMPLETE ALL REQUIRED START-UP DOCUMENTATION LOCATED IN THE MANUAL. SCAN INTO JOB FILE AND EMAIL COPY TO KATOLIGHT AND OPS.
 17. CONTACT OPS TWO DAYS PRIOR TO TESTING.

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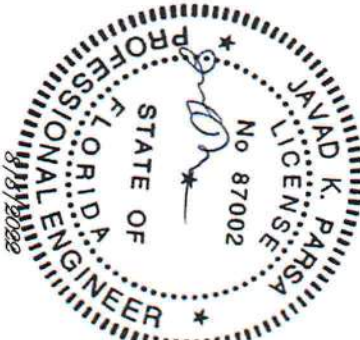
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TEL: 678-990-2338

VERIZON WIRELESS
SITE NUMBER: 712924
BU #: 825272
LAKE CITY AIRPORT
336 SE NEWELL DR
LAKE CITY, FL 32025

EXISTING 133'-0" MONOPOLE

ISSUED FOR:				
REV	DATE	DRAWN	DESCRIPTION	DES./QA
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1 PROPOSED UTILITY ROUTING
SCALE: 1"=40'-0" (FULL SIZE)
1"=80'-0" (1x17)

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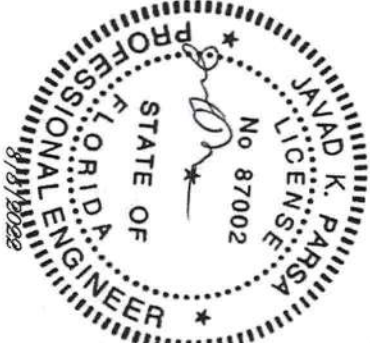
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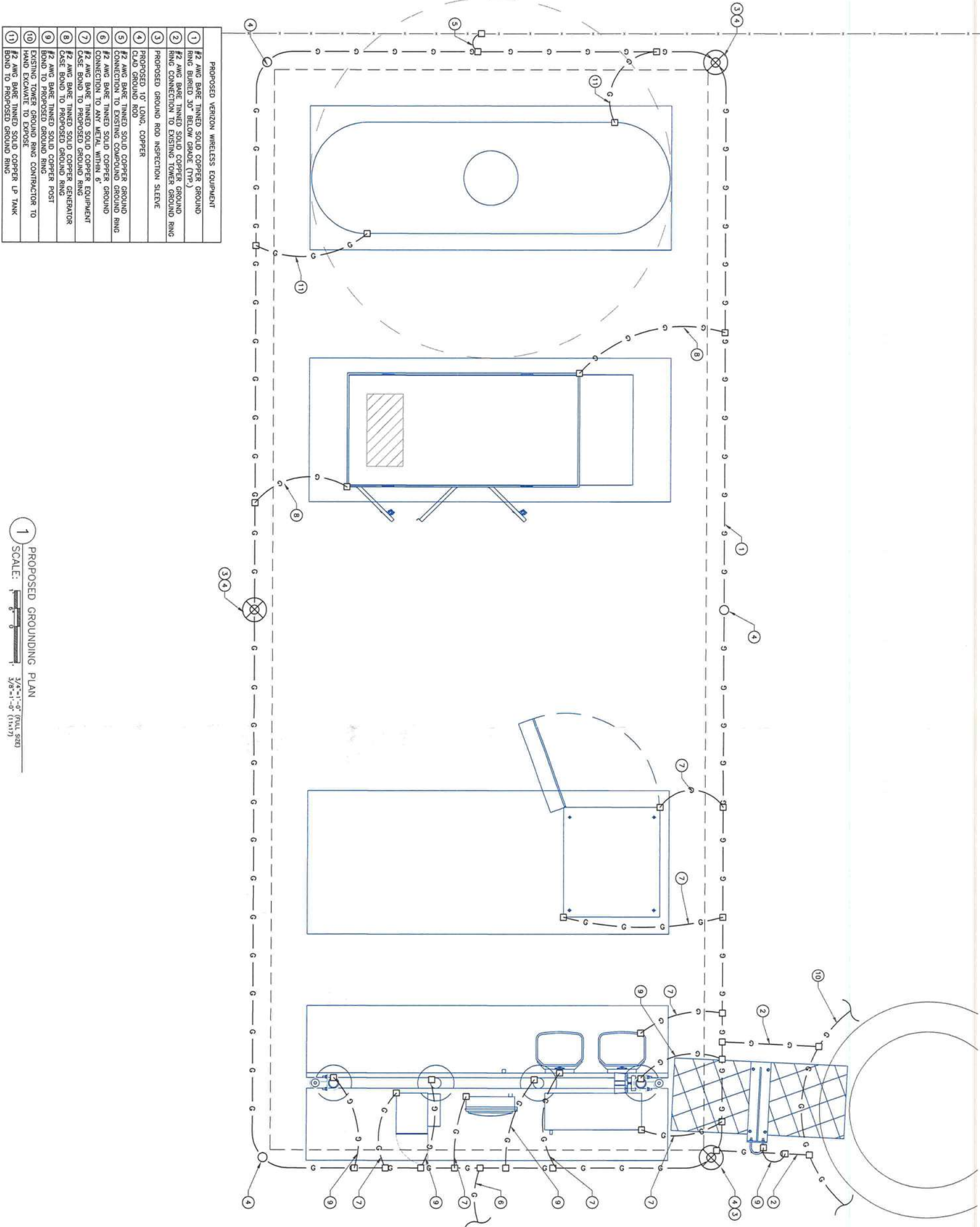
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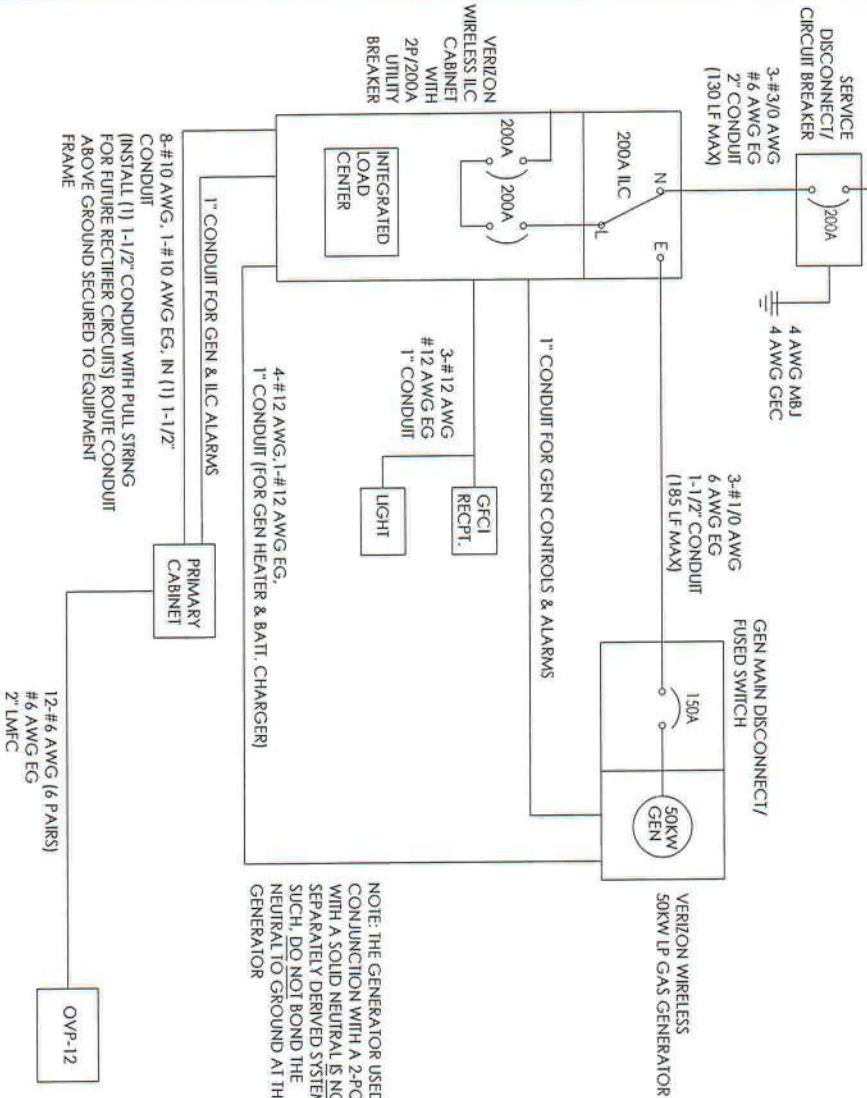
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1 PROPOSED GROUNDING PLAN
SCALE: 3/4"=1'-0" (FULL SIZE)
3/8"=1'-0" (11x17)

COMMON VZW DC PLANT RECTIFIER REQUIREMENTS			
RECTIFIER	INPUT FLA CURRENT AT 240 VAC (EACH RECTIFIER)	2 RECT / BRANCH CIRCUIT (PREFERRED APPROACH)	1 RECT / BRANCH CIRCUIT (ALT APPROACH)
GE 75A (NEC055AC48500A or sim)	22 AMPS (MAX)	60A/2P #3 THHN	30A/2P #10 THHN

LOAD CALCULATION	
LOAD	AMPS
PROPOSED LOAD:	123.0
TOTAL DEMAND:	129.0
VOLTAGE: 120/240V SINGLE PHASE 3W 200A	



ELECTRICAL SINGLE LINE DIAGRAM

- NOTES:
- ALL EQUIPMENT SHALL BE NEMA 3R RATED.
 - ALL EQUIPMENT SHALL BE LIGHTNING PROTECTED IN ACCORDANCE WITH TIA-222-G AND VERIZON WIRELESS STANDARDS.
 - CONDUCTOR SIZES AND DISTANCES HAVE BEEN SIZED FOR 35% MAX VOLTAGE DROP (TOTAL SYSTEM VOLTAGE DROP ON BOTH FEEDERS AND BRANCH CIRCUITS TO THE FARTHEST DEMAND SHALL NOT EXCEED 5%).
 - WIRE SIZING AND MAXIMUM DISTANCE FROM GENERATOR TO ILC ASSUMES POWER FACTOR OF 0.9.
 - BELOW GRADE CONDUIT SHALL BE SCHEDULE 80 PVC. ABOVE GRADE CONDUIT SHALL BE GALVANIZED RIGID CONDUIT. BELOW GRADE PVC CONDUIT SHALL TRANSITION TO GRC PRIOR TO RISING ABOVE GRADE. ALL BENDS SHALL HAVE A MINIMUM 24" RADIUS. ALL FITTINGS SHALL BE SUITABLE FOR USE WITH THREADED RIGID CONDUIT. VERIFY CONDUIT TYPE WITH LOCAL CONSTRUCTION MANAGER AND ADJUST IF NECESSARY. ALL CONDUIT SHALL MEET NEC, STATE, AND LOCAL CODE REQUIREMENTS AS REQUIRED.

PROPOSED VERIZON WIRELESS PANEL SCHEDULE												
PANEL NAME	VZW ILC			MODEL NUMBER		INTERSECT AA300S-1PH-3R						
RATED VOLTAGE	120	240	VOLTS		PHASE / WIRE	1	3					
MAIN BREAKER	200	AMPS	BUS RATING		200	KEY DOOR LATCH		YES				
MOUNT	SURFACE			NEUTRAL BAR		YES	HINGED DOOR		YES			
ENCLOSURE TYPE NEMA 3R				AIC		65K						
POS	USAGE FACTOR	BUS AMPS L1 L2		LOAD	POLES	AMPS L1 L2	POLE	LOAD	BUS AMPS L1 L2 USAGE FACTOR POS			
1	1	18		RECTIFIER	2	30A.		RECTIFIER	18		1	2
3	1		18	RECTIFIER	2	30A.		RECTIFIER	18		1	4
5	1	18		RECTIFIER	2	30A.		RECTIFIER	18		1	6
7	1		18	RECTIFIER	2	30A.		RECTIFIER	18		1	8
9	1	18		RECTIFIER	2	30A.		RECTIFIER	18		1	10
11	1		18	RECTIFIER	2	30A.		RECTIFIER	18		1	12
13	1	18		RECTIFIER	2	30A.		RECTIFIER	18		1	14
15	1		18	RECTIFIER	2	30A.		RECTIFIER	18		1	16
17	1.25	12		GFI RECPT. / LIGHT	1	20A.						18
19	1.25		12	BLOCK HEATER	1	20A.						20
21	1.25	5		BATT. CHARGER	1	20A.						22
23												24
25												26
27												28
29												30
		99.25	87	SUB TOTAL AMPS								
				SUB TOTAL AMPS:		72	72					
				FACTORED TOTAL		165.5	155					

NOTES

1. ALL CONDUCTIONS ARE TYPE THWN (75°C) COPPER

2. MAXIMUM LENGTH OF RUN FOR RECIPER CIRCUITS IS 50FT.

3. INTERSECT / GENBAC INTEGRATED LOAD CENTER INCLUDES 200A, MAIN DISCONNECT AND TRANSFER SWITCH OR PORTABLE OR PERMANENT GENERATOR.

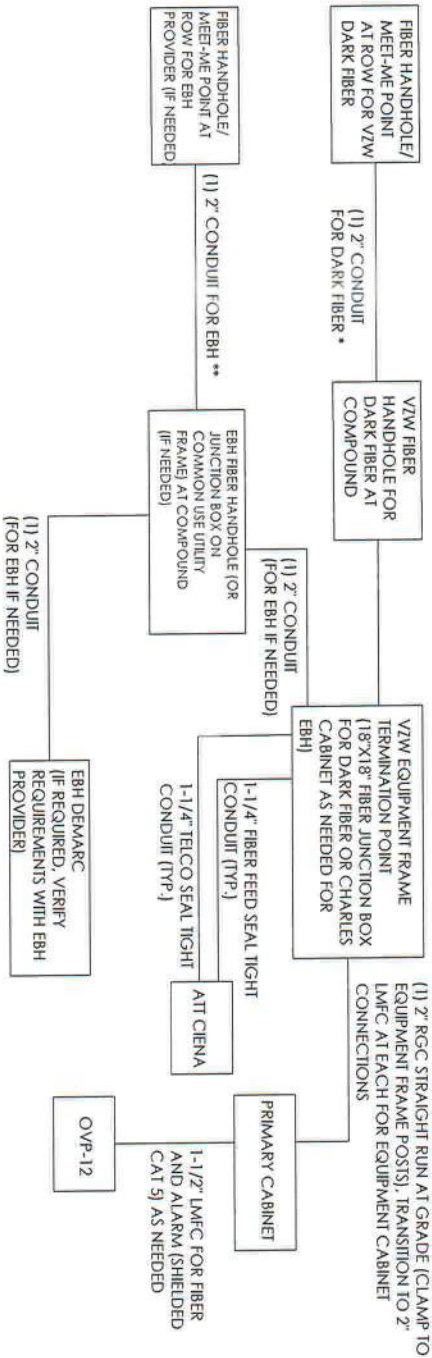
4. RECIPER LOADS ARE CONSIDERED TO BE NON-CONTINUOUS.

5. IF ADDITIONAL FUTURE LOADS ARE ADDED WHICH CAUSE TOTAL DEMAND TO EXCEED GENERATOR BREAKER SIZE, BACKUP POWER SYSTEM SHALL BE EVALUATED AND IMPROVED AS NECESSARY

- NOTES:
- ALL CONDUCTORS ARE TYPE THWN (75°C) COPPER
 - MAXIMUM LENGTH OF RUN FOR RECTIFIER CIRCUITS IS 50FT.
 - INTERSECT/GENERAC INTEGRATED LOAD CENTER INCLUDES 200A. MAIN DISCONNECT AND TRANSFER SWITCH FOR PORTABLE OR PERMANENT GENERATOR.
 - RECTIFIER LOADS ARE CONSIDERED TO BE NON-CONTINUOUS.
 - IF ADDITIONAL FUTURE LOADS ARE ADDED WHICH CAUSE TOTAL DEMAND TO EXCEED GENERATOR BREAKER SIZE, BACKUP POWER SYSTEM SHALL BE EVALUATED AND UPGRADED AS NECESSARY

PANEL SCHEDULE

PANEL SCHEDULE



FIBER SINGLE LINE DIAGRAM

- NOTES:
- ADD (1) ADDITIONAL 2" CONDUIT FOR DARK FIBER (2 TOTAL) IF REQUIRED BY LOCAL MARKET FACILITIES. VERIFY PRIOR TO CONSTRUCTION. (ADD 2 PULL STRINGS TO EACH CONDUIT)
 - VERIFY EBH REQUIREMENTS WITH TELCO PROVIDER PRIOR TO CONSTRUCTION. (ADD 2 PULL STRINGS TO EACH CONDUIT)

1 PANEL SCHEDULE & ONE-LINE DIAGRAM
SCALE: NOTE TO SCALE

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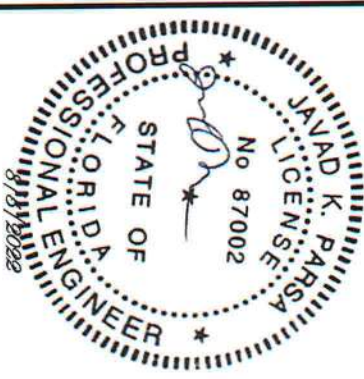
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BU #: 825272
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EXISTING 133'-0" MONOPOLE

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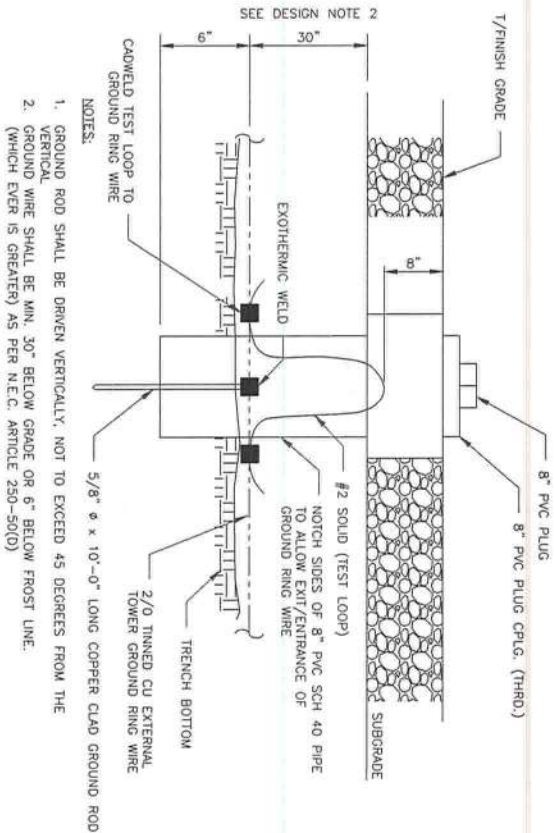
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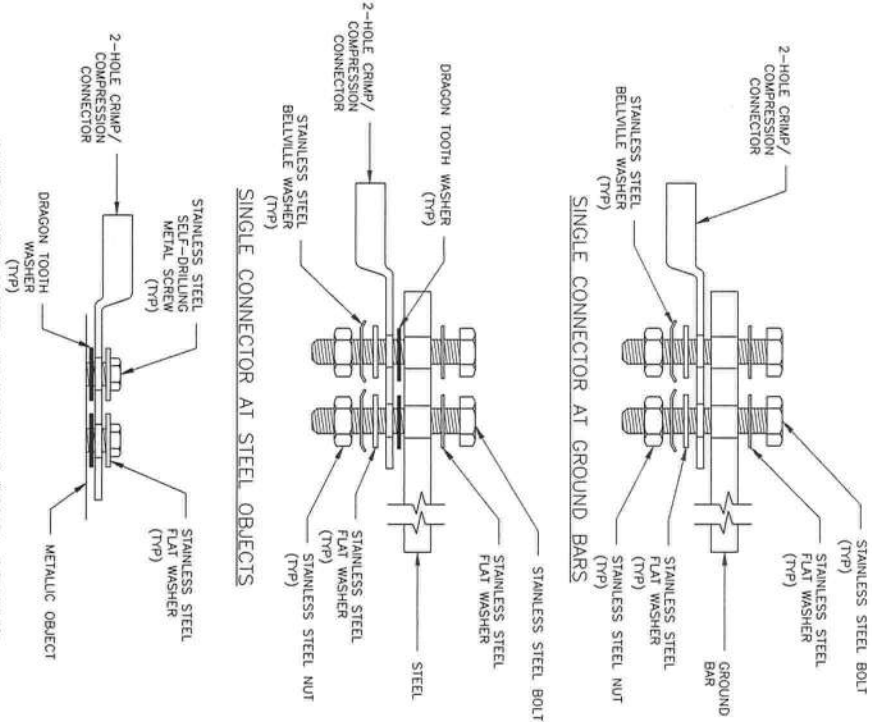
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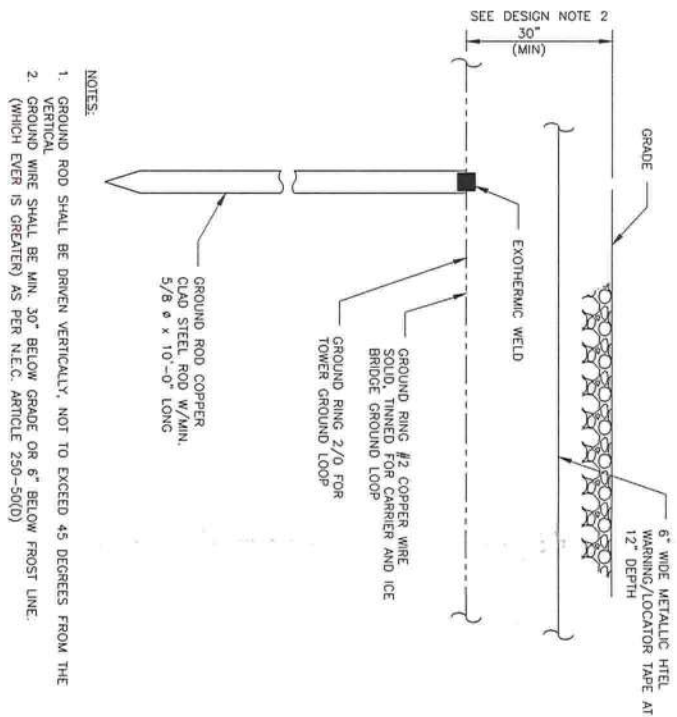
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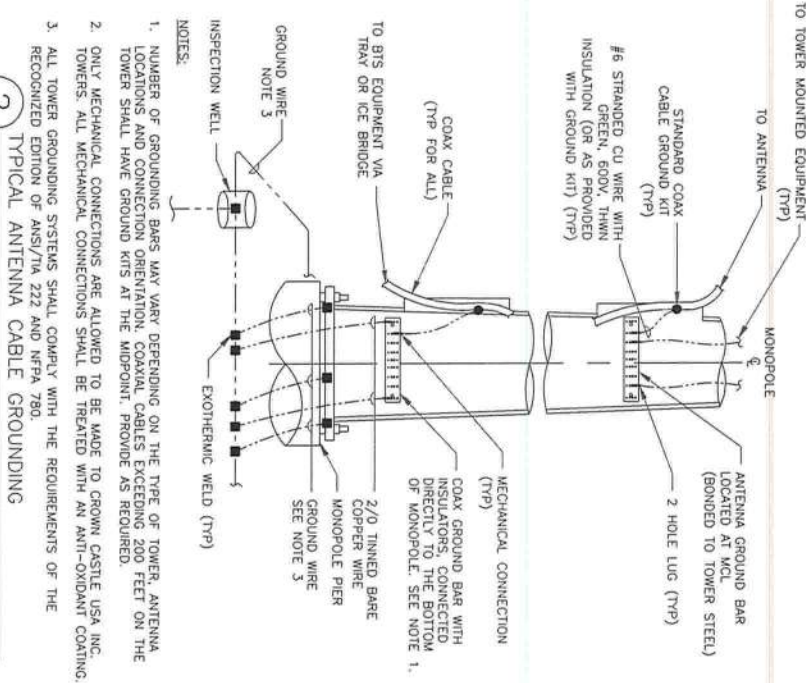
1 INSPECTION WELL DETAIL
SCALE: NOT TO SCALE



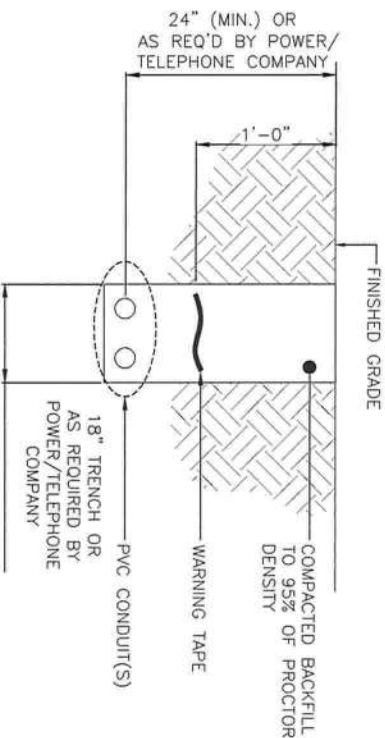
3 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS
SCALE: NOT TO SCALE



4 GROUND ROD DETAIL
SCALE: NOT TO SCALE



2 TYPICAL ANTENNA CABLE GROUNDING
SCALE: NOT TO SCALE



5 UTILITY TRENCH DETAIL
SCALE: NOT TO SCALE

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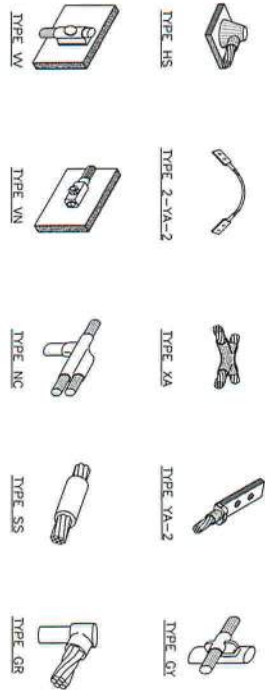
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No 87002
STATE OF FLORIDA
PROFESSIONAL ENGINEER
8/8/2022

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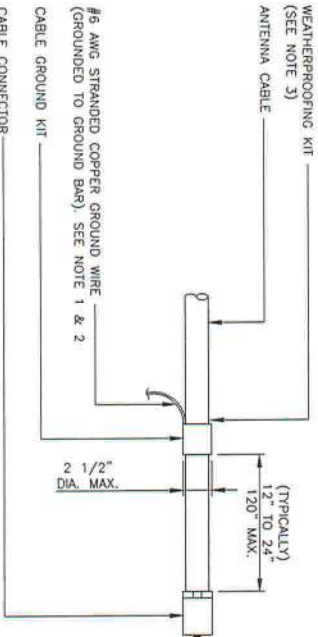
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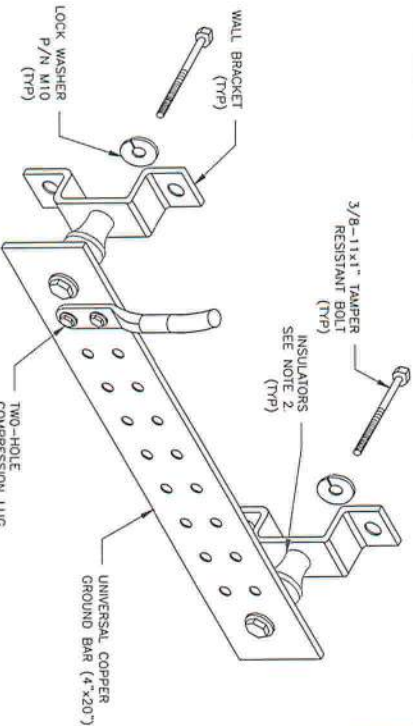
- NOTE:
1. ERICO EXOTHERMIC "MOLD TYPES" SHOWN HERE ARE EXAMPLES. CONSULT WITH CONSTRUCTION MANAGER FOR SPECIFIC MOLDS TO BE USED FOR THIS PROJECT.
 2. MOLD TYPE ONLY TO BE USED BELOW GRADE WHEN CONNECTING GROUND RING TO GROUND ROD.

1 CADWELD GROUNDING CONNECTIONS SCALE: NOT TO SCALE



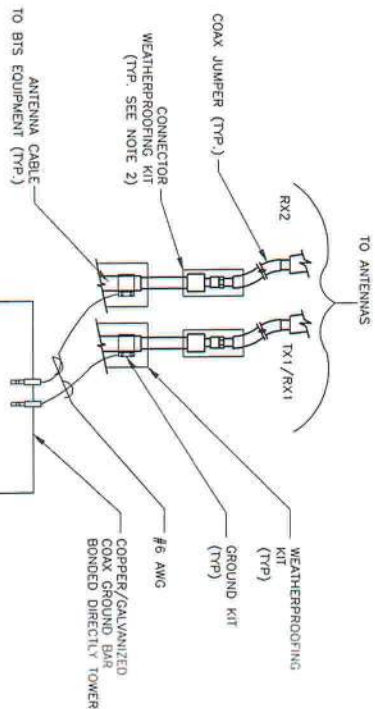
- WEATHERPROOFING KIT _____ (SEE NOTE 3)
- ANTENNA CABLE _____
- #6 AWG STRANDED COPPER GROUND WIRE _____ (GROUNDED TO GROUND BAR). SEE NOTE 1 & 2
- CABLE GROUND KIT _____
- CABLE CONNECTOR _____
- NOTES:
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
 2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
 3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

3 CABLE GROUND KIT CONNECTION SCALE: NOT TO SCALE



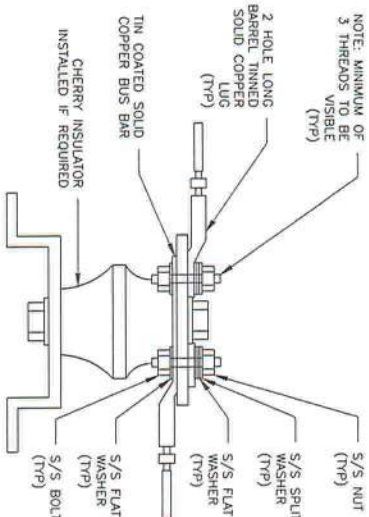
- NOTES:
1. DOWN LEAD (HORN RUN) CONDUCTORS ARE NOT TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER. PER THE GROUNDING DOWN CONDUCTOR POLICY QAS-STD-10091, NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION. CAD-WELDING ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
 2. Omit INSULATORS WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL. USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

6 GROUND BAR DETAIL SCALE: NOT TO SCALE



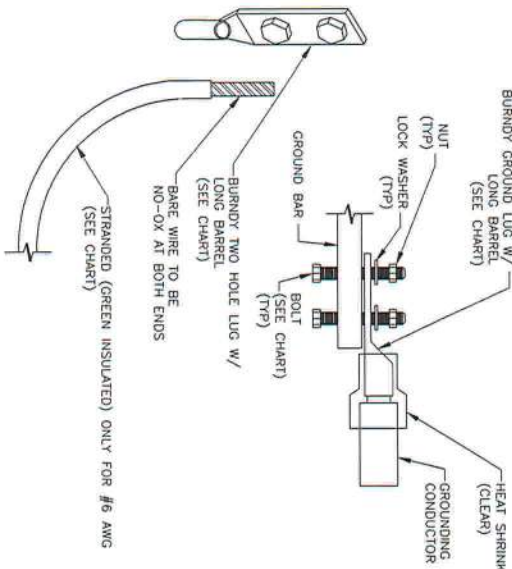
- NOTES:
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO ANTENNA GROUND BAR.
 2. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

4 GROUND CABLE CONNECTION SCALE: NOT TO SCALE



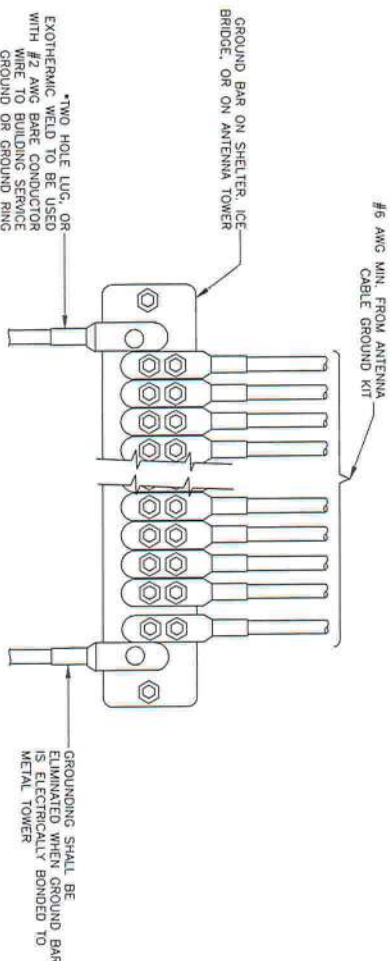
7 LUG DETAIL SCALE: NOT TO SCALE

WIRE SIZE	BURNDY LUG	BOLT SIZE
#6 AWG GREEN INSULATED	YA6C-21C38	3/8" - 16 NC S 2 BOLT
#2 AWG SOLID TINNED	YA3C-21C38	3/8" - 16 NC S 2 BOLT
#2 AWG STRANDED	YA2C-21C38	3/8" - 16 NC S 2 BOLT
#2/0 AWG STRANDED	YA2B-21C38	3/8" - 16 NC S 2 BOLT
#4/0 AWG STRANDED	YA2B-2N	1/2" - 16 NC S 2 BOLT

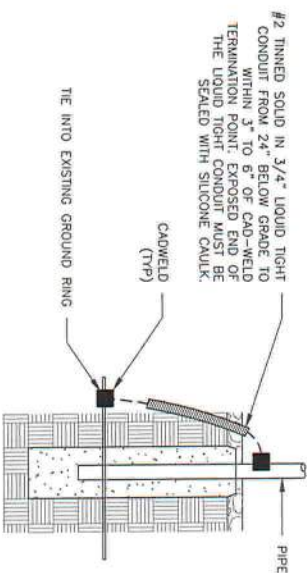


- NOTES:
1. ALL GROUNDING LUGS ARE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. BURNDY LUGS ARE TO BE USED. BURNDY LUGS SHALL BE STAINLESS STEEL. ALL HARDWARE ARE TO BE AS FOLLOWS: BOLT, FLAT WASHER, GROUND BAR, GROUND LUG, FLAT WASHER AND NUT.

2 MECHANICAL LUG CONNECTION SCALE: NOT TO SCALE



5 GROUNDWIRE INSTALLATION SCALE: NOT TO SCALE



8 TRANSITIONING GROUND DETAIL SCALE: NOT TO SCALE

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