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COA #0 278
04/20/2021

Alpine, an ITW Company
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Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B53428a
Job Description: ODUM RESIDENCE	
Address: 437 SE PEACOCK TERR., LAKE CITY, FL 32025	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res	IntelliVIEW Version: 20.02.00A JRef #: 1X4P8570011
Wind Standard: ASCE710 Wind Speed (mph): 0	Design Loading (psf): 55.00
Building Type:	

This package contains general notes pages, 8 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	110.21.0700.10227	F1 16'8"4 System 42 Gable	2	110.21.0700.06700	F2 16'11"12 Floor Truss
3	110.21.0700.03417	F3 13'3"4 Floor Truss Girder	4	110.21.0659.55667	F4 13'3"4 Floor Truss
5	110.21.0659.54147	F5 14'4"14 Floor Truss	6	110.21.0659.52767	F6 13'5"14 Floor Truss
7	110.21.0659.51047	F7 13'2"8 Floor Truss	8	110.21.0659.49257	F8 12'0"14 Floor Truss
9	PB160160118		10	PB180160118	
11	REPCHRD1014		12	STRBRIBR1014	
13	DEFLCAMB1014				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

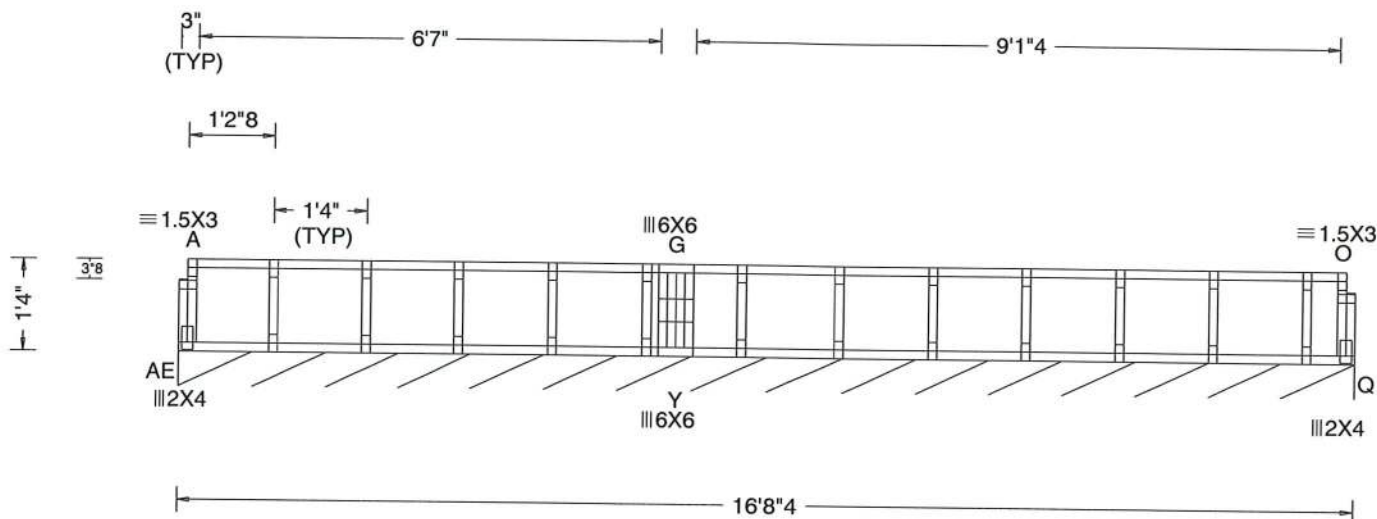
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 44635 FROM: RJL	SY42 Qty: 2	Ply: 1	Job Number: B53428a ODUM RESIDENCE Truss Label: F1 16'8"4 System 42 Gable	Cust: R 857 JRef: 1X4P8570011 T12 DrwNo: 110.21.0700.10227 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * =PLF
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.000 E 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.000 E 999 240	Q* 87 /- /- /- /- /-
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.000 B - -	Q Brg Width = 200 Min Req = -
Des Ld: 55.00	EXP: NA Kzt: NA		HORZ(TL): 0.001 B - -	Bearing AE is a rigid surface.
NCBCLL: 10.00	Mean Height: NA ft	Building Code:	Creep Factor: 2.0	Members not listed have forces less than 375#
Soffit: 0.00	TCDL: NA psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.058	
Load Duration: 1.00	BCDL: NA psf	TPI Std: 2014	Max BC CSI: 0.013	
Spacing: 19.2 "	MWFRS Parallel Dist: NA	Rep Fac: No	Max Web CSI: 0.025	
	C&C Dist a: NA ft	FT/RT:20(0)/10(0)		
	Loc. from endwall: NA	Plate Type(s):		
	l: NA GCpi: NA			
	Wind Duration: NA	WAVE	VIEW Ver: 20.02.00A.1020.20	

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3:

Bracing

Sheathing is required for any longitudinal (drag) forces. All connections to be designed by the building designer.

Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 1.5X3 except as noted.

Additional Notes

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Truss must be installed as shown with top chord up.



COA #0 278

04/20/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

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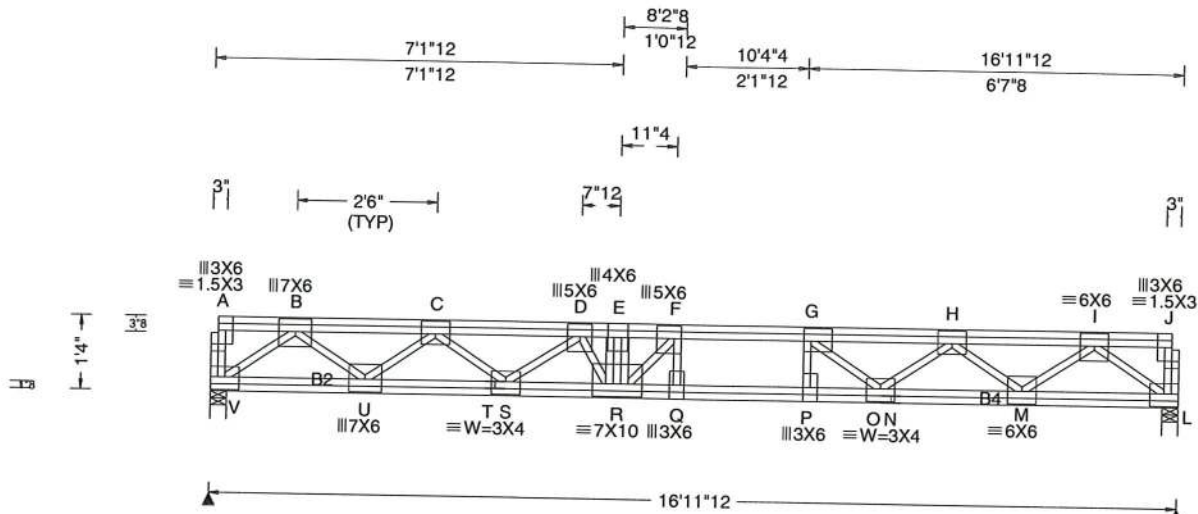
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org



6750 Forum Drive
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SEQN: 44657 FROM: RJL	SY42	Ply: 1 Qty: 8	Job Number: B53428a ODUM RESIDENCE Truss Label: F2 16'11"12 Floor Truss	Cust: R 857 JRef: 1X4P8570011 T7 DrwNo: 110.21.0700.06700 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.253 F 786 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.386 F 515 240	V 1263 /- /- /- /- /-
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.038 B - -	V 1110 /- /- /- /- /-
Des Ld: 55.00	EXP: NA Kzt: NA		HORZ(TL): 0.058 B - -	V Brg Width = 3.4 Min Req = 1.5
NCBCLL: 10.00	Mean Height: NA ft		Creep Factor: 2.0	L Brg Width = 3.4 Min Req = 1.5
Soffit: 0.00	TCDL: NA psf	Building Code:	Max TC CSI: 0.822	Bearings V & L are a rigid surface.
Load Duration: 1.00	BCDL: NA psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.737	Members not listed have forces less than 375#
Spacing: 19.2 *	MWFRS Parallel Dist: NA	TPI Std: 2014	Max Web CSI: 0.759	Maximum Top Chord Forces Per Ply (lbs)
	C&C Dist a: NA ft	Rep Fac: No		Chords Tens.Comp. Chords Tens. Comp.
	Loc. from endwall: NA	FT/RT: 12(0)/10(0)		B - C 0 -2672 F - G 0 -5286
	I: NA GCpi: NA	Plate Type(s):		C - D 0 -4786 G - H 0 -4110
	Wind Duration: NA	WAVE	VIEW Ver: 20.02.00A.1020.20	D - E 0 -6043 H - I 0 -2306
Lumber				E - F 0 -6043
Top chord: 4x2 SP #1:				

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP SS Dense; B2,B4 4x2 SP #1;
Webs: 4x2 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	80 plf at	0.12 to	80 plf at	16.85
BC: From	8 plf at	0.00 to	8 plf at	16.98
TC:	900 lb Conc. Load at	7.08		

Plating Notes

All plates are 5X6 except as noted.

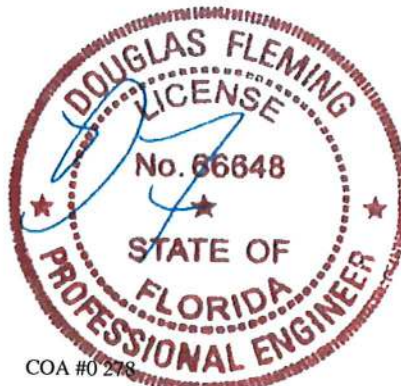
Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Truss must be installed as shown with top chord up.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0 278

04/20/2021

▲ Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V	1263	/-	/-	/-	/-	/-
L	1110	/-	/-	/-	/-	/-
V	Brg Width = 3.4			Min Req = 1.5		
L	Brg Width = 3.4			Min Req = 1.5		

Bearings V & L are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	0	-2672	F - G	0	-5286
C - D	0	-4786	G - H	0	-4110
D - E	0	-6043	H - I	0	-2306
E - F	0	-6043			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
V - U	1468	Q - P	5286
U - T	3850	P - O	5256
T - S	3850	O - N	3260
S - R	5711	N - M	3260
R - Q	5319	M - L	1301

Maximum Web Forces Per Ply (lbs)

Webbs	Tens.Comp.	Webbs	Tens. Comp.
V - B	0 - 1904	F - Q	95 - 606
B - U	1594 0	P - G	567 - 59
U - C	0 - 1558	G - O	0 - 1517
C - S	1239 0	O - H	1124 0
S - D	0 - 1224	H - M	0 - 1262
D - R	648 0	M - I	1331 0
E - R	76 - 1067	I - L	0 - 1687
R - F	1369 - 365		

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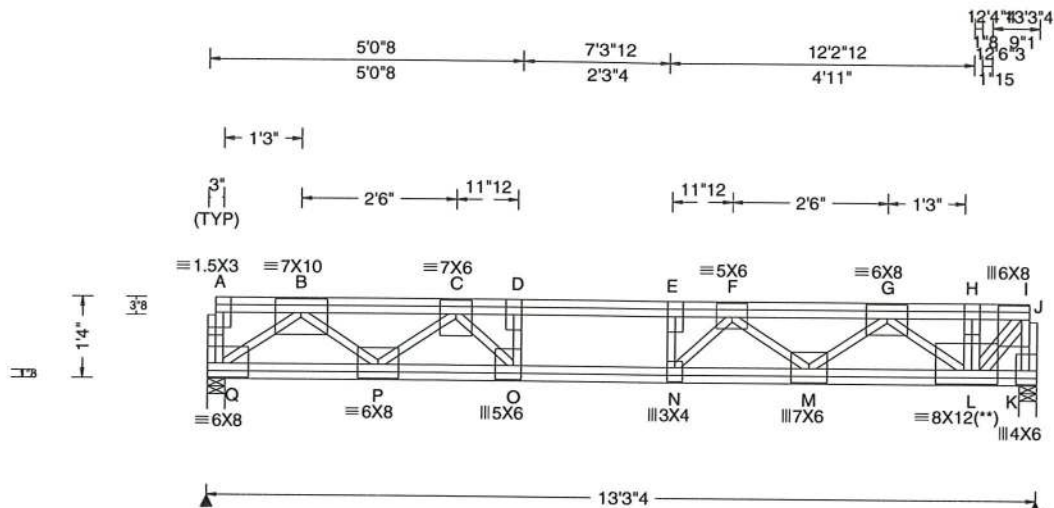
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SEQN: 44655 FROM: RJL	SY42 Qty: 1	Ply: 1 Qty: 1	Job Number: B53428a ODUM RESIDENCE Truss Label: F3 13'3"4 Floor Truss Girder	Cust: R 857 JRef: 1X4P8570011 T4 DrwNo: 110.21.0700.03417 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Def/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.072 E 999 360 VERT(CL): 0.284 E 544 240 HORZ(LL): 0.011 L - - HORZ(TL): 0.042 L - - Creep Factor: 2.0 Max TC CSI: 0.769 Max BC CSI: 0.908 Max Web CSI: 0.942 VIEW Ver: 20.02.00A.1020.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Q 2163 -/- -/- -/- -/- K 2944 -/- -/- -/- -/- Q Brg Width = 3.4 Min Req = 1.5 K Brg Width = 3.4 Min Req = 1.5 Bearings Q & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -3932 F - G 0 -5288 C - D 0 -6299 G - H 0 -2191 D - E 0 -6347 H - I 0 -2191 E - F 0 -6335

Lumber

Top chord: 4x2 SP SS Dense;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 315 plf at 0.12 to 315 plf at 13.15
BC: From 8 plf at 0.00 to 8 plf at 13.27
TC: 900 lb Conc. Load at 12.23

Plating Notes

All plates are 3X6 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Deflection

Max JT VERT DEFL: LL: 0.07" DL: 0.22". See detail DEFLCMB1014 for camber recommendations.

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

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COA #0 278

04/20/2021

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.		
Q - P	2437	0	N - M	6252	0
P - O	5474	0	M - L	4222	0
O - N	6347	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
Q - B	0 -3161	M - G	1410 0
B - P	1978 0	G - L	0 -2586
P - C	0 -2042	H - L	0 -1208
C - O	1424 0	L - I	3540 0
O - D	0 -913	I - J	0 -2891
N - F	432 -33	J - K	0 -2907
F - M	0 -1276		

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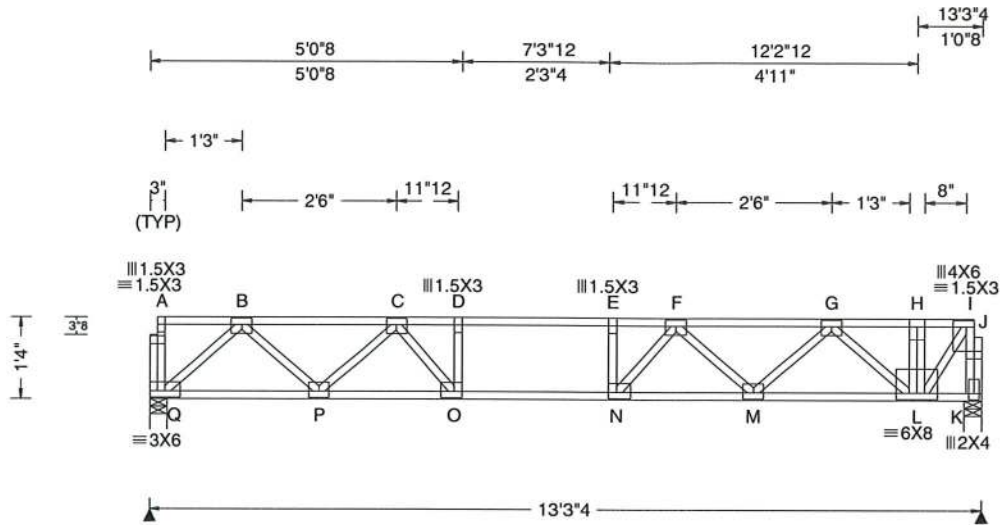
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SEQN: 44652 FROM: RJL	SY42	Ply: 1 Qty: 8	Job Number: B53428a ODUM RESIDENCE Truss Label: F4 13'3"4 Floor Truss	Cust: R 857 JRef: 1X4P8570011 T6 DrwNo: 110.21.0659.55667 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.110 E 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.190 E 815 240	Q	633	-/-	-/-	-/-	-/-	-/-
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.016 L - -	K	1414	-/-	-/-	-/-	-/-	-/-
	EXP: NA Kzt: NA		HORZ(TL): 0.024 I - -	Q	Brg Width = 3.4			Min Req = 1.5		
Des Ld: 55.00	Mean Height: NA ft		Creep Factor: 2.0	K	Brg Width = 3.4			Min Req = 1.5		
NCBCLL: 10.00	TCDL: NA psf	Building Code:	Max TC CSI: 0.648	Bearings Q & K are a rigid surface.						
Soffit: 0.00	BCDL: NA psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.692	Members not listed have forces less than 375#						
Load Duration: 1.00	MWFRS Parallel Dist: NA	TPI Std: 2014	Max Web CSI: 0.781	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 19.2 "	C&C Dist a: NA ft	Rep Fac: No		Chords	Tens.Comp.	Chords	Tens. Comp.			
	Loc. from endwall: NA	FT/RT:12(0)/10(0)		B - C	0 -1054	F - G	0 -1644			
	I: NA GCpi: NA	Plate Type(s):		C - D	0 -1753	G - H	0 -946			
	Wind Duration: NA	WAVE	VIEW Ver: 20.02.00A.1020.20	D - E	0 -1767	H - I	0 -946			

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
 TC: From 80 plf at 0.12 to 80 plf at 13.15
 BC: From 8 plf at 0.00 to 8 plf at 13.27
 TC: 900 lb Conc. Load at 12.23

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

See detail STBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Truss must be installed as shown with top chord up.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



COA #0 278
04/20/2021

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	639 0	N - M	1813 0
P - O	1480 0	M - L	1412 0
O - N	1767 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
Q - B	0 -868	H - L	0 -946
B - P	577 0	L - I	1640 0
P - C	0 -593	I - J	0 -1386
C - O	595 0	J - K	0 -1390
G - L	0 -621		

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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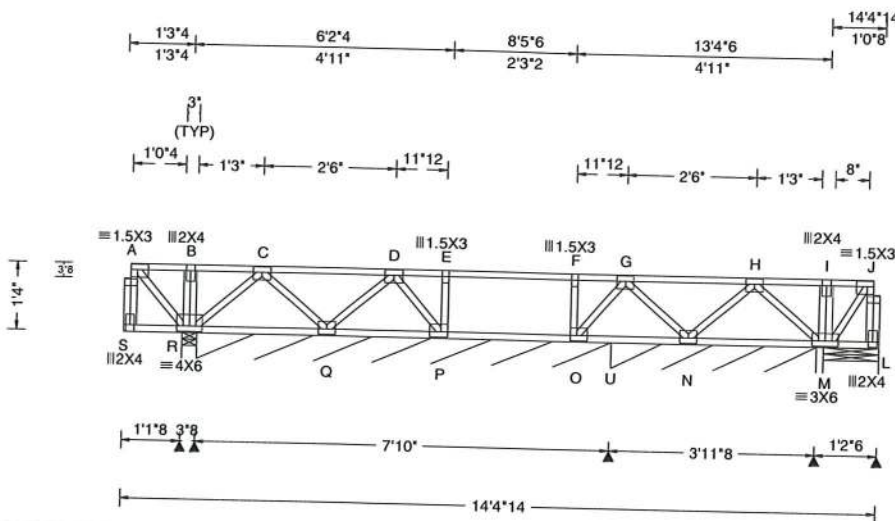
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 44648 FROM: RJL	SY42 Qty: 1	Ply: 1 Qty: 1	Job Number: B53428a ODUM RESIDENCE Truss Label: F5 14'4"x14' Floor Truss	Cust: R 857 JRef: 1X4P8570011 T5 DrwNo: 110.21.0659.54147 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *PLF
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 S 999 360 VERT(CL): 0.014 S 999 240 HORZ(LL): -0.003 A - - HORZ(TL): 0.006 A - - Creep Factor: 2.0 Max TC CSI: 0.267 Max BC CSI: 0.053 Max Web CSI: 0.224 VIEW Ver: 20.02.00A.1020.20	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL R 1097 /- /- /- /- /- R* 68 /-4 /- /- /- /- U* 45 /- /- /- /- /- L 211 /- /- /- /- /- R Brg Width = 3.5 Min Req = 1.5 R Brg Width = 94.0 Min Req = - U Brg Width = 47.5 Min Req = - L Brg Width = 12.9 Min Req = 1.5 Bearings R, R, U, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 660 0 B - C 660 0

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

See detail STBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

This truss spaced @ 19.2" oc supports additional concentrated load at left end from 8.09' stud wall (13.75 PSF) supporting 13-6-0 roof spans with 1-4-0 overhang. Roof load: 20.00 psf LL and 17.00 psf DL.

Truss must be installed as shown with top chord up.



COA #0 248

04/20/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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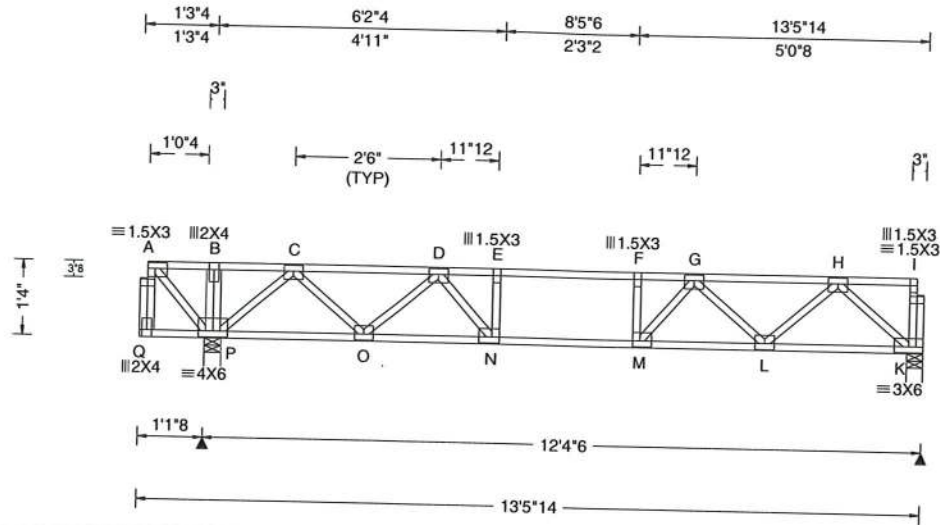
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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 44643 FROM: RJL	SY42	Ply: 1 Qty: 2	Job Number: B53428a ODUM RESIDENCE Truss Label: F6 13'5"14 Floor Truss	Cust: R 857 JRef: 1X4P8570011 T8 Dwno: 110.21.0659.52767 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)							
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity					
TCDL: 10.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.057 F 999 360	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.109 F 999 240	P	1359	/-	/-	/-	/-	/-	
BCDL: 5.00	Category: NA	Snow Duration: NA	HORZ(LL): -0.012 H - -	K	506	/-	/-	/-	/-	/-	
Des Ld: 55.00	EXP: NA Kzt: NA		HORZ(TL): 0.022 H - -	P Brg Width = 3.5 Min Req = 1.5							
NCBCLL: 10.00	Mean Height: NA ft		Creep Factor: 2.0	K Brg Width = 3.4 Min Req = 1.5							
Soffit: 0.00	TCDL: NA psf	Building Code:	Max TC CSI: 0.771	Bearings P & K are a rigid surface.							
Load Duration: 1.00	BCDL: NA psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.455	Members not listed have forces less than 375#							
Spacing: 19.2 "	MWFRS Parallel Dist: NA	TPI Std: 2014	Max Web CSI: 0.249	Maximum Top Chord Forces Per Ply (lbs)							
	C&C Dist a: NA ft	Rep Fac: No		Chords		Tens.Comp.		Chords		Tens. Comp.	
	Loc. from endwall: NA	FT/RT: 12(0)/10(0)		A - B	659	0	E - F	0	- 1089		
	I: NA GCpi: NA	Plate Type(s):		B - C	659	0	F - G	0	- 1089		
	Wind Duration: NA	WAVE		C - D	300	-559	G - H	0	- 777		
				D - E	0	- 1079					

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

See detail STBRIBR1014 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

This truss spaced @ 19.2" oc supports additional concentrated load at left end from 8.09' stud wall (13.75 PSF) supporting 13-6-0 roof spans with 1-4-0 overhang. Roof load: 20.00 psf LL and 17.00 psf DL.

Truss must be installed as shown with top chord up.



COA #0 218
04/20/2021

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

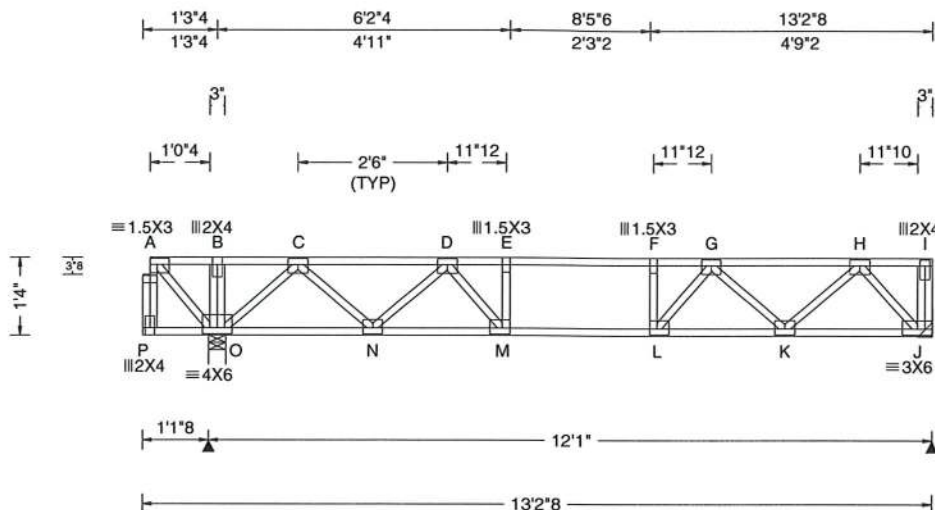
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 44641 FROM: RJL	SY42 Qty: 7	Ply: 1 Qty: 7	Job Number: B53428a ODUM RESIDENCE Truss Label: F7 13'2"8 Floor Truss	Cust: R 857 JRef: 1X4P8570011 T1 DrwNo: 110.21.0659.51047 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2"	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.049 F 999 360 VERT(CL): 0.094 F 999 240 HORZ(LL): -0.011 H - - HORZ(TL): 0.021 H - - Creep Factor: 2.0 Max TC CSI: 0.717 Max BC CSI: 0.414 Max Web CSI: 0.241 VIEW Ver: 20.02.00A.1020.20	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL O 1345 -/- /- /- /- /- J 495 -/- /- /- /- /- O Brg Width = 3.5 Min Req = 1.5 J Brg Width = - Min Req = - Bearing O is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 659 0 E - F 0 -1018 B - C 659 0 F - G 0 -1013 C - D 305 -530 G - H 0 -666 D - E 5 -1010

Lumber
Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Plating Notes
All plates are 3X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Additional Notes
See detail STBRIBR1014 for bracing and bridging recommendations.
Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.
This truss spaced @ 19.2" oc supports additional concentrated load at left end from 8.09' stud wall (13.75 PSF) supporting 13'-6" roof spans with 1'-4" overhang. Roof load: 20.00 psf LL and 17.00 psf DL.
Truss must be installed as shown with top chord up.



COA #0218
04/20/2021

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	210 -459	M - L	1018 0
N - M	860 -144	L - K	936 0

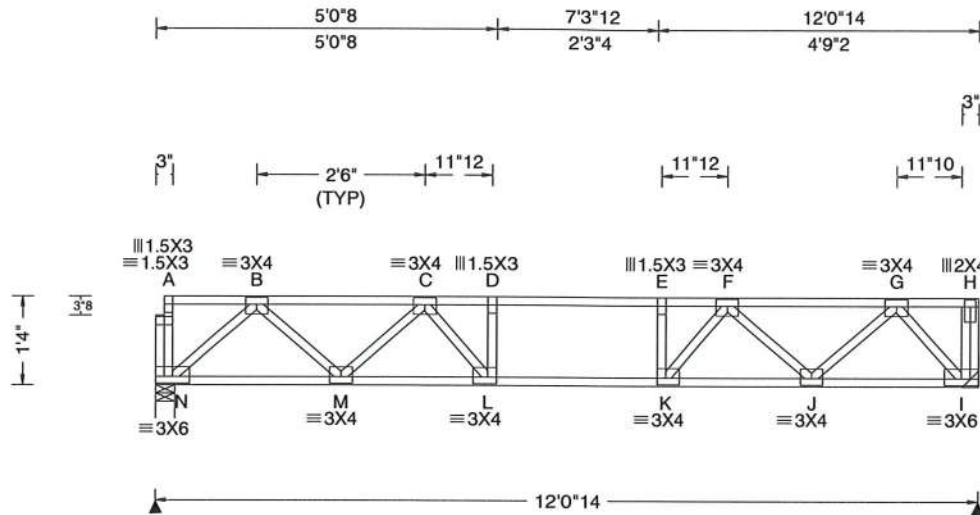
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - O	0 -958	D - M	490 0
O - C	0 -785	G - K	12 -376
C - N	506 0	K - H	414 0
N - D	0 -512	H - J	0 -592

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6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 44646 FROM: RJL	SY42 Ply: 1 Qty: 3	Job Number: B53428a ODUM RESIDENCE Truss Label: F8 12'0"14 Floor Truss	Cust: R 857 JRef: 1X4P8570011 T2 DrwNo: 110.21.0659.49257 SSB / DF 04/20/2021
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 40.00 TCCL: 10.00 BCCL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 19.2 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCCL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.053 D 999 360 VERT(CL): 0.086 D 999 240 HORZ(LL): 0.013 B - - HORZ(TL): 0.022 B - - Creep Factor: 2.0 Max TC CSI: 0.481 Max BC CSI: 0.359 Max Web CSI: 0.221 VIEW Ver: 20.02.00A.1020.20	Gravity Loc R+ / R- / Rh Non-Gravity / Rw / U / RL N 518 /- /- /- /- /- I 534 /- /- /- /- /- N Brg Width = 3.4 Min Req = 1.5 I Brg Width = - Min Req = - Bearing N is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 0 -823 E - F 0 -1190 C - D 0 -1192 F - G 0 -732 D - E 0 -1198

Lumber

Top chord: 4x2 SP #1;
Bot chord: 4x2 SP #1;
Webs: 4x2 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.
Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.
Truss must be installed as shown with top chord up.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
N - M	516 0	K - J	1051 0
M - L	1103 0	J - I	398 0
L - K	1198 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
N - B	0 -702	F - J	0 -444
B - M	426 0	J - G	464 0
M - C	0 -389	G - I	0 -641



COA #0 278
04/20/2021

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Suite 305
Orlando FL, 32821

Piggyback Detail	- ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00
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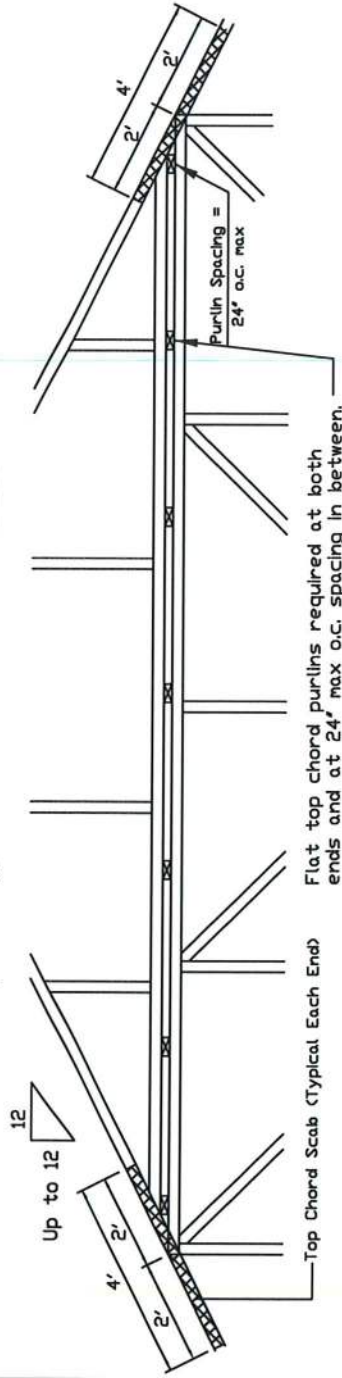
160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (nlr), Kzt=1.0, Dr 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (nlr), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

*** Refer to Engineer's sealed truss design drawing for piggypack and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

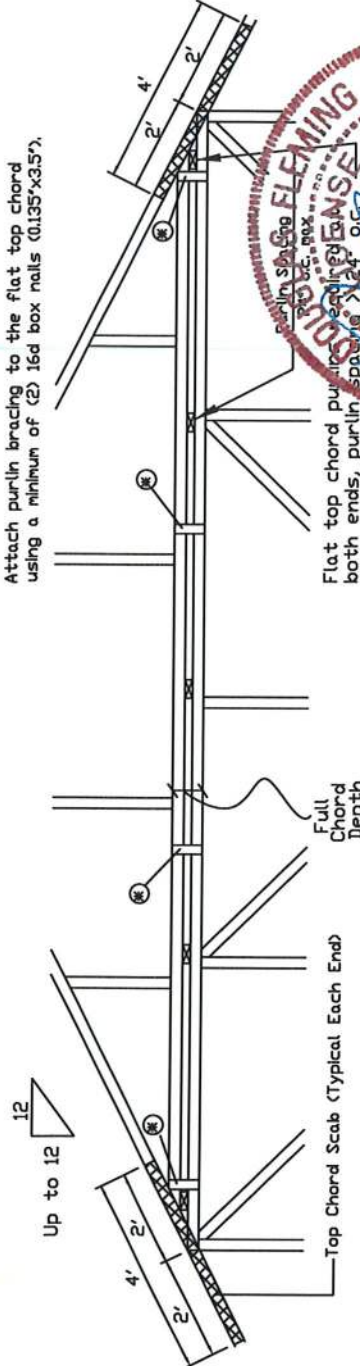


Piggyback cap truss slant nailed to all top chord
purlin bracing with (2) 16d box nails (0.135"x3.5")
and secure top chord with 2x4 #3 grade scab
(1 side only at each end) attached with
2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following (3) 3x8 Trulox plate attached with (8) 0.120"x1.375" nails; (4) into cap TC & (4) into base truss TC or (1) 28P3 wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Plggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Top Chord Scab (Typical Each End)

Note: If purlins or sheathing are not specified on the flat top of the roof, the following shall be used:

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IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following edition of BCSI Guiding Component Safety Information, by TPI and SBCIA for safe practices prior to installation. This information will provide temporary bracing of trusses. Unless noted otherwise, top chord shall have properly attached temporary bracing. Trusses shall have a properly attached R-D ceiling. Locations shown for permanent lateral bracing shall have bracing installed per BCSI sections 33, 37 or B10, as applicable. Apply plates to end of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings ISM-2 for standard plate positions.

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\\13723\Riverport\ Drive
\\ Suite\ 200
\\ Maryland\ Heights,\ MO\ 63043

※ In addition, provide connection with one of the following methods:

Trulox

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4" o.c. front to back faces.

APA Rated Gusset

8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Plogyback Plate

One 2PRB wave piggyback plate to each face @ 8" o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4" o.c. front to back faces.

REF	PIGGYBACK
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DATE 01/02/2018

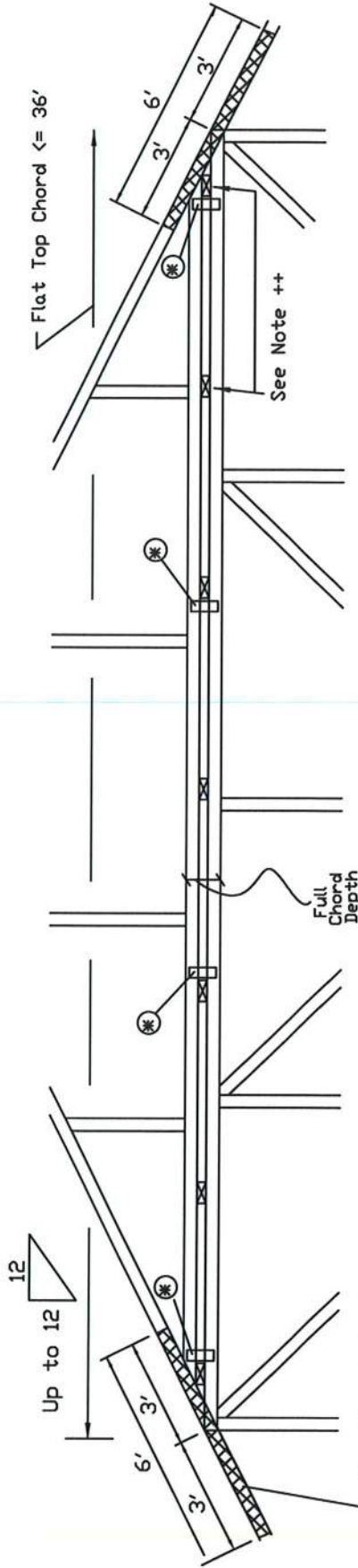
DRWG PB160160118

SPACING	24.0'
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Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Part, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL= 50 psf (min), Kzt=1.0.
 Dr 160 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Part, Enclosed Bldg, located anywhere in roof, Exp D, wind DL= 50 psf (min), Kzt=1.0.
 Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The Building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.
 Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.
 Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

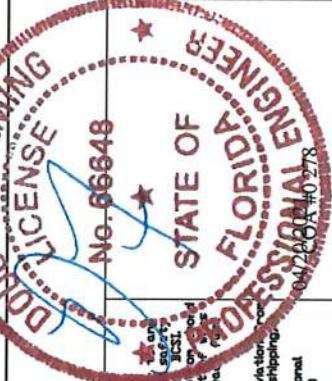
Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.



Top Chord Scab (Typical Each End)

■ In addition, provide connection with one of the following methods:

Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate Use 28PB wave piggyback plate to each face 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (0.115"x2") with perimeter gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach 8' o.c. with (6) 10d box nails (0.128"x3") per scab. Attach cap along chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Guiding Component Safety Information, by TPI and SCSA for details. Unless noted otherwise, installers shall provide temporary bracing per BCSI. Trusses shall have a properly attached 16d ceiling. Locations shown for applicable. Apply plates to each side of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the design shown. Any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or use of the truss, shall be the responsibility of the installer.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineinc.com TPI: www.tpi.net SCSA: www.scsa.org ID: www.idcsa.org

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118

SPACING	24.0'
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ALPINE
 AN ITW COMPANY
 11514 Earth City Expressway
 Suite 242
 Earth City, MO 63045

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12' max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = $(2 \times L) + (B)$

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4' o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4' o.c., rows staggered.

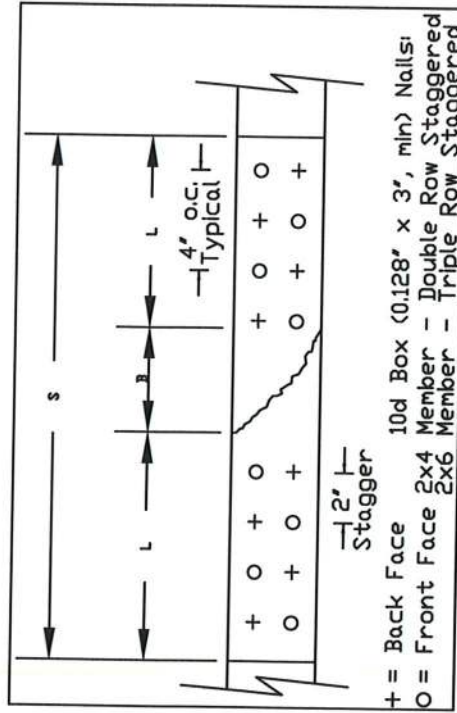
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650F.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.

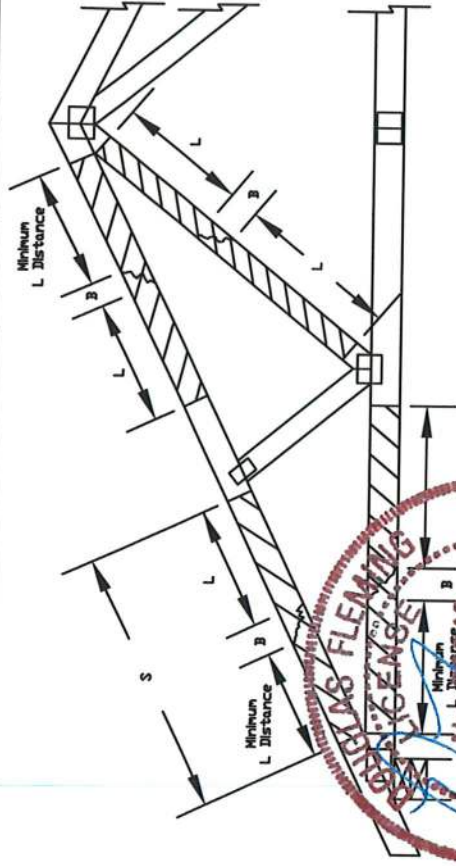


Nail Spacing Detail

IMPORTANT! READ AND FOLLOW ALL NOTES ON THIS DRAWING
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of ICCI Guiding Component Safety Information, by TPI and SCSA for proper practices for these functions. Installers shall provide temporary bracing per ICCI. Unless noted otherwise, temporary bracing shall be provided by the installer. The installer shall have a properly attached rigid collar. Location of bracing shall be determined by the installer or truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 10d-2 for standard plate positions.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviations from the design shown. The installer shall be responsible for building the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing or any other function. A seal on this drawing or cover page listing this drawing, indicating acceptance of professional engineering responsibility is the responsibility of the design shown. The suitability and use of this drawing for any structure is the responsibility of the building designer per ANSI/TPI 1, Sec.2.
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineinc.com TPI: www.tpi.org SCSA: www.scsa.org ICCI: www.icci.org

ALPINE
AN ITW COMPANY
11514 Earth City Expressway
Suite 242
Earth City, MO 63045

Maximum Member Axial Force						
Member	Size	L	SPF-C	HF	DF-L	SYP
Web Only	2x4	12'	620#	635#	730#	800#
Web Only	2x4	18'	975#	1055#	1295#	1415#
Web or Chord	2x4	24'	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30'	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36'	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42'	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48'	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014

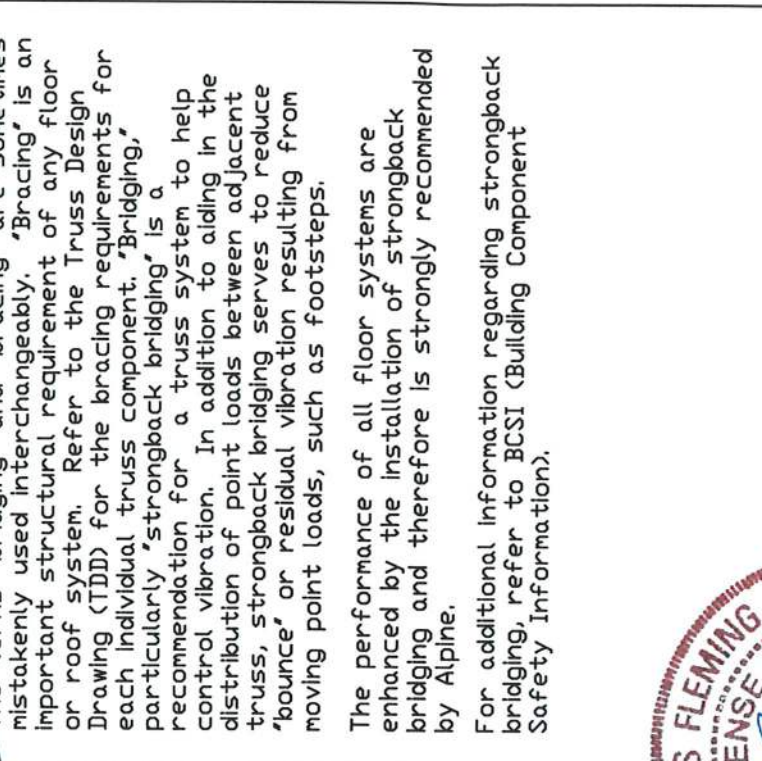
SPACING 24.0' MAX

FLORIDA PROFESSIONAL ENGINEER
No. 06648
(4/28/2014)

STRONGBACK BRIDGING RECOMMENDATIONS



► The terms "bridaño" and "bracino" are sometimes



TC LL	PSF	REF	STRONGBACK
TC DL	PSF	DATE	10/01/14
BC DL	PSF	DRWG	STRBIBR1014
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

Commentary:

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to 'sag.'
- Avoids 'dips' in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

Deflection and Camber

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

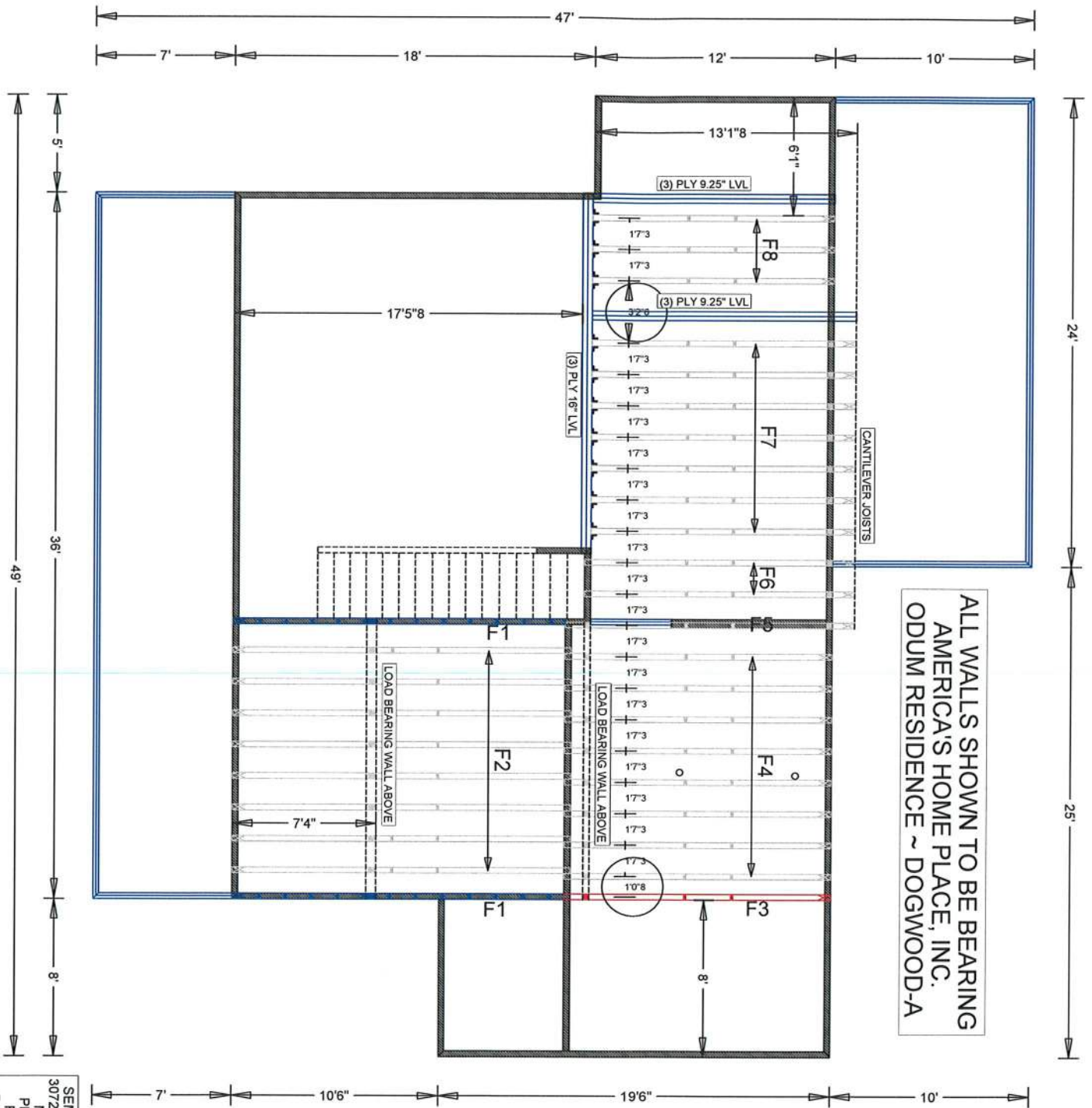
Truss Type	L/D	Deflection Limits Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75' (horizontal)	1.25' (horizontal)

Recommended Camber

Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual deflection may be considerably less than the design-deflection.

 115141 Earth City Expressway 11 Suite 242 11 Earth City, MO 63045	IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Guiding Components Safety Information, by TPI and BCSI for safety. Unless noted otherwise, top chord shall have temporary bracing per BCSI. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral bracing. Refer to drawings 1004-2 for standard plate positions. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the instructions. Any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of the truss, shall be the responsibility of the contractor. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The authority and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com TPI: www.tpiinc.org BCSI: www.bcsiindustry.org BOD: www.bodusa.org	REF	DEFLEC/CAMB
		DATE	10/01/14
		DRWG	DEFLCAMB1014
		04/20/2014 10:27:38	



ALL WALLS SHOWN TO BE BEARING
AMERICA'S HOME PLACE, INC.
ODUM RESIDENCE ~ DOGWOOD-A

SEMINOLE TRUSSES INC.
30726 Bluestar Memorial Hwy.
MIDWAY FL 32343
Phone (850) 575-0102
Fax (850) 575-4413
Design By Robert J. Little

Job Name: ODUM RESIDENCE
Customer: America's Home Place
Designer: ROBERT J. LITTLE
PlanName: DOGWOOD-A
Created : 04-20-2021
SemRef# : B53428a

JOB NO:
B53428a

PAGE NO:
1 OF 1