DATE 02/20/2007  APPLICANT LARRY COLE ADDRESS 25370 NW		Columbia County Building Permit  This Permit Expires One Year From the Date of Issue PHONE 352-472-6850 STH PLACE  REWBERRY FL	<b>PERMIT</b> 000025554 32669
	DAVID & MELISSA WATSON	HONE 386.454.1999	
ADDRESS 103	SW BUCK COURT	FT. WHITE	32038
CONTRACTOR BO	BONNIE JORDAN	PHONE 352-472-6850	
LOCATION OF PROPERTY		47S, TL ON 27, TL ON BENJAIN ROAD, TL ON BUCK COURT,	
	STH LOT ON RIGHT		
TYPE DEVELOPMENT	POOL ENCLOSURE	ESTIMATED COST OF CONSTRUCTION 7059.00	00.00
HEATED FLOOR AREA		TOTAL AREA HEIGHT ST	STORIES
FOUNDATION	WALLS	ROOF PITCH FLOOR	
LAND USE & ZONING	A-3	MAX. HEIGHT 35	
Minimum Set Back Requirments:	irments: STREET-FRONT	30.00 REAR 25.00 SIDE	25.00
NO. EX.D.U. 1	FLOOD ZONE NA	DEVELOPMENT PERMIT NO.	
PARCEL ID 20-7S-17.	20-7S-17-10027-127	SUBDIVISION	
LOT BLOCK	PHASE	UNIT TOTAL ACRES 10.00	
	SCC056711	6711	
Culvert Permit No.	Culvert Waiver Contractor's	Contractor's License Number Applicant/Owner/Contractor	
EXISTING	X07-081	CS TH SO	z
Driveway Connection	Septic Tank Number	Zoning checked by Approved for Issuance	New Resident
COMMENTS: NOC ON	FILE, REPLACES PERMIT 254	COMMENTS: NOC ON FILE, REPLACES PERMIT 25400 PULLED AS OWNER BUILDER 1/12/07 for the enc	endorure
		Check # or Cash 1258	89
	FOR BUILDING	FOR BUILDING & ZONING DEPARTMENT ONLY	(footer/Slab)
Temporary Power	Found	Foundation	
	date/app. by	date/app. by date/	date/app. by
Under slab rough-in plumbing	guing	Slab Sheathing/Nailing	
	date/app. by	date/app. by	date/app. by
Framing		Rough-in plumbing above slab and below wood floor	
date/ann hv	) <del>,</del>	•	**

Columbia County Building Permit Application Ch#1258 Revised 9-23-0
Application Approved by - Zoning Official Date Received 2/15/07 By G Permit # 25554  Application Approved by - Zoning Official Date 2007 Plans Examiner OK 37H Date 2 - 16-07  Flood Zone NA Development Permit NA Zoning A-3 Land Use Plan Map Category A-3  Comments
Applicants Name Larry Cale Phone 353-473-6850
Address 25370 NW 8T Place NEW BERRY, F/ 32669
Owners Name David Watson Phone 318-0899 911 Address 103 Sw Bock Count Fi white 20 32038
911 Address 103 Sw Bock Court FT white IC 32038
Contractors Name Bonnie Jordan, Timberlake Alm. Const. Phone 352-472-6850
Address 25370 NW 8Th Place NEW BERRY F/ 32669
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address
Mortgage Lenders Name & Address
Circle the correct power company - Ft. Power & Light - Clay Elec Suwannee Valley Elec Progressive Energ
Property ID Number $80-75-/1-10037-/27$ Estimated Cost of Construction $2,059^{-30}$
Subdivision NameLotBlock Unit Phase
ving Directions US 27 South To BENSAmin LIEft SWAMAZON Gla House
At 18ud of Road.
Type of Construction Pocl EnClusure Number of Existing Dwellings on Property 2
Total Acreage Lot Size Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drivert Maiver</u>
Actual Distance of Structure from Property Lines - Front $168'$ Side $26'$ Side $261'$ Rear $1010'$
Total Building Height 1/4 Number of Stories 1/4 Heated Floor Area 1/4 Roof Pitch 1/4
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.
(M) Derus arolan
Owner Builder or Agent (Including Contractor)  Contractor Signature  Contractors License Number 5 00 56 711
STATE OF FLORIDA Competency Card Number
Sworn to (or affirmed) and subscribed before me  WY COMMISSION # DD 289931 EXPIRES: April 12, 2008
this 14 day of Feb 2007.
Personally known or Produced Identification Drivers Ligente Notary Signature

House at end H 318-0897 W 396-454-1999 Then whot picket vill in Road
fuce ends & Benjamin Road
be at Benjamin River To birth

Watson

FILE COPY

(O called

# RONNIE BRANNON

COLUMBIA COUNTY TAX COLLECTOR

NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS THIRD INSTALLMENT (DEC) 2006 129741.0000  $\mathbb{Z}$ 

ACCOUNT NUMBER	ESCROW CD	ESCROW CD   ASSESSED VALUE   EXEMPTIONS	EXEMPTIONS	TAXABLE VALUE	MILLAGE CODE
110027-127	666	37,253		37,253	003

WATSON DAVID F JR & MELISSA H 103 SW BUCK CT FT WHITE FL 32038

20-7S-17 5000/0200 10.06 Av COMM NW COR OF SW1/4 C RUN E 87.45 FT, S 370.28 FT POB, CONT S 354.70 FT, E 1235.48 FT, N 354.70 FT, W See Tax Roll For Extra Legal

-				Please Retain this Portion for your Records	$\sim$	
	TAXES LEVIED	325.07 28.31 185.33 74.51 18.31 83.82 5.14	720.49	AMOUNT 129.56 201.00	nt information	Jan 31 2007 177.77
000 Sept. 1	EXEMPTION AMOUNT TAXABLE VALUE	37,253 37,253 37,253 37,253 37,253 37,253 37,253	AD VALOREM TAXES	TE TE NON-AD VALOREM ASSESSMENTS	See reverse side for important information	If Paid By Please Pay
AD VALOREM TAXES	LAGE RATE	8.7260 0.7600 4.9750 2.0000 0.4914 2.2500 0.1380	19.3404	NON-AD VALOREM ASSESSMENTS RATE NON-AD VAL(	51.05	5.50 0.00 0.00
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Producer:	Lion Insurance Company 2739 U.S. Highway 19 N. Holiday, FL 34691	architecture ( de transit de la respectiva		upon the Cer	ite is issued as a matter tificate Holder. This Ce afforded by the policies	of information only and c rtificate does not amend, a s below.	nulecs en nyhrs extend or alter
	Phone 727-938-5562 Fax: 727-937-213	8			Insurers Affording Cove		NAIC »
insured:	South East Personnel Leasing, Inc.	ALCH AND TESTING SOCIAL PRINCIPLE TRANSPORTED TO BEST	ALUM STANOON	Insurer A:	Lion Insurance Company	THE SAME CERT STRACK BA COST CONTRACTOR AND THE SAME STRACK	1.1075
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la ' "	ers' Liability					E.L. Each Accident	\$ \$1000000
Any propi excluded	rietor/partner/executive officer/member ?	201170					\$1000000
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ACORD CONFORM ON 1986

# **Timberlake Aluminum Construction, Inc.**

25370 NW 8TH PLACE NEWBERRY, FL 32669 (352) 472-6850 FAX (352) 472-6855

THIS LETTER AUTHORIZES LASING COLE TO OBTAIN PERMITS AND SIGN ALL APPLICABLE FORMS AND AFFADAVITS NECESSARY FOR LICENSES FOR LICENSE NUMBER #SCCO56711.

BONNIE JORDAN
DATE

SWORN TO AND SUBSCRIBED BEFORE ME THIS /7 DAY OF JANUARY, 2007

NOTARY PUBLIC



## STATE OF FLORIDA



DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

CONSTRUCTION INDUSTRY LICENSING BOARD 1940 NORTH MONROE STREET TALLAHASSEE FL 32399-0783

(850) 487-1395

JORDAN, BONNIE I.
TIMBERLAKE ALUMINUM CONSTRUCTION INC
P O BOX 671
INVERNESS FI 34451



STATE OF FLORIDA

AC# 2762920

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

SCC056711

08/29/06 060062507

CERTIFIED SPECIALTY CONTRACTOR JORDAN, BONNIE L TIMBERLAKE AGUNINUM CONSTRUCTION

IS CERTIFIED under the provisions of th. 489 WK. Expiration date: AUG 31, 2008 106082904272

#### DETACH HERE

**3#2762920** 

#### STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ#106082904272

18/29/2006 060062507 SCC056711
The SPECIALTY STRUCTURE CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2008

JORDAN, BONNIE L TIMBERLAKE ALUMINUM CONSTRUCTION INC 1198 E MCKINLEY ST HERNANDO FL 34442

JEB, BUSH

SIMONE MARSTILLER

# 

	(For Recorder's use only)
NOTICE OF COMMENCEMEN	Key No.
Permit No	COUNTY OF LOLUMBIA
STATE OF FLORIDA  THE UNDERSIGNED hereby gives notice that improvement will be made to certain real prostatutes, the following information is provided in this Notice of Commencement:	perty, and in accordance with Chapter 713, Florida
Statutes, the following information is provided in this residence of the state of t	103 SW BUCK CT Ft White FL 3203
<ul> <li>THE UNDERSIGNED hereby gives notice that improvements that the following information is provided in this Notice of Commencement:</li> <li>Description of Property: Parcel No. 30 - 75 - 11 - 100+37 - 137 (Legal description of the property and street address if available)</li> </ul>	w. d. Oal
- Colon Colo	per en fre
I add coto Clare 174	01/12/3
Address	103 Sw Buck Ct.
3. Owner Information: Name Name State 2C Zip 338 Interest in Propert	y Owener
Name of Fee Simple Titleholder (If other than owner):	NA State NA Zip NA
Address City Address Address	25370 NW 8PR
State Of Zip 32669	NA City N/A
5. Surety NameAddress Amount of Bond:\$	
State7	NIA
6. Lender Name	70111
CityStateZip	other documents may be served as provided by Section
7. Persons within the State of Florida designated by Owner upon whom notices or 713.13(1) (a) (7), Florida Statutes:  NameAddressA	City N/A
State NA Zip NA	v/Ato receive a copy of
8. In addition to himself, Owner designates	
9. Expiration date of Notice of Commencement (the expiration date is (1) year from	m the date of recording unless specified.
Signature of Owner:	- : a,
Printed Name of Owner: 110140 Datson	-
STATE OF FLORIDA COUNTY OF LOLUMBIA The foregoing instrument was acknowledged before me this Feb 14-07, by personally known to me or () who produced Ocivers Liseure did or () did not take an oath.	Who is ( )  CRAIG C. TIMBERLAKE  MY COMMISSION # DD 289931  EXPIRES: April 12, 2008  Bonded Thru Notary Public Underwriters
Signature of Notary	Print, Type or Stamp Name of Notary
E:/HP9000/Forms/Notice of Commencement	

## **Timberlake Aluminum Construction, Inc.**

25370 NW 8TH PLACE NEWBERRY, FL 32669 (352) 472-6850 1-800-976-9890 FAX (352) 472-6855

David Watson 103 S.W. Buck Ct. Ft. White Fl. 32038

Sir:

Inclosed please find a notice of commencement that needs to be signed and noterized.

also, in order to obtain a permit, the county needs to have a copy of some proof of ownership (ex. deed, tax reciept).

Thank You for your cooperation;

**Larry Cole** 

J02120300Z

Timberlake Aluminum Construction, Inc.

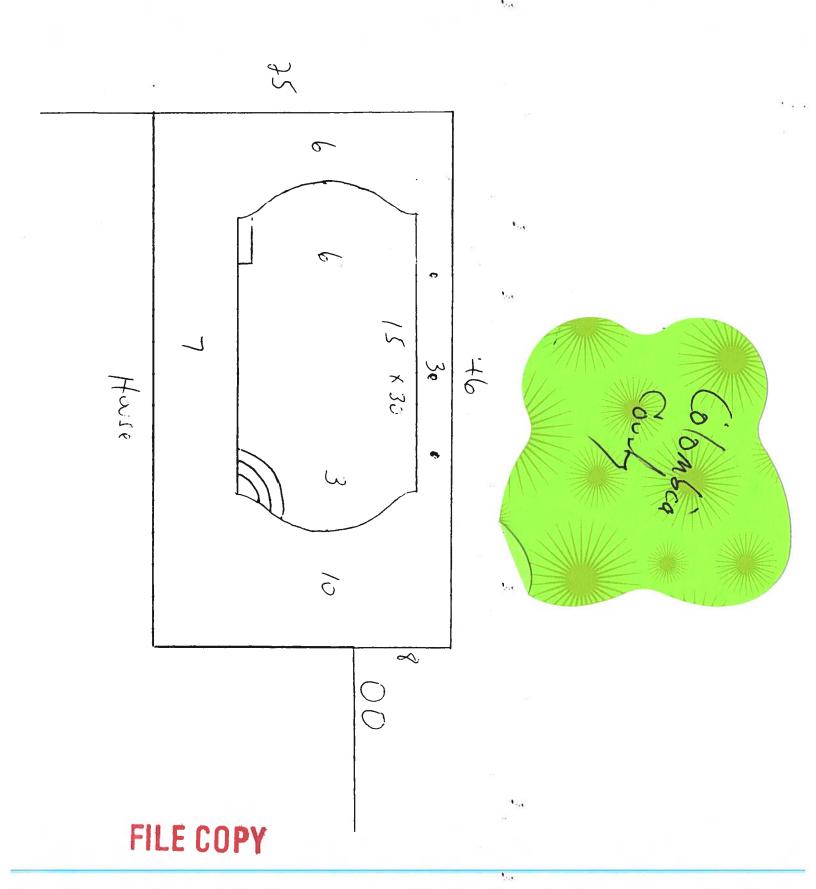
Don't clean it - Screen It!

License # SCC056711

Newberry Commercial Park 25370 NW 8th Place Newberry, Florida 32669

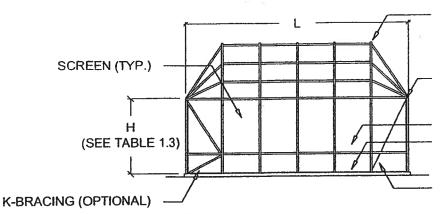
Phone (352) 472-6850 · Fax (352) 472-6855 · Toll Free 1 (800) 976-9890

PROPOSAL SUBMITTED TO	PHONE 318 0897 1 DATE/1-27-06
July of walton	JOB NAME
STREET 103 SW But Cold	
CITY, STATE and ZIP CODE	JOB LOCATION
SALESMAN	JOB NO. JOB PHONE
WE HEREBY SUBMIT SPECIFICATIONS AND ESTIMATES FOR:	
Pool Enclosure Walls 8 × H	6 x25 width: 8 Lange Wa
Super Gutter V-C/	
Downspours X 2	
Roof Style (Mansard or Hip Gabl	8
Wall Heigh	٤
36" Chair Rail around Perimeter	xe/
36" Doors X 2	·
Frame Color - Fronze or White	
18/14 Charcoal Screen	
THE Propose hereby to turnish material and labor - comp	lete in accordance with above specifications for the sum of:
(8 7059,00) Seven Thousand	+ titto nine
0 % at acceptance of Pro	sposal. / CO / fine completion of the job.
nt 4 All to be considered in a	Contractor Agent
All material is guaranteed to be as specified. All work to be completed in a workonanilke manner according to standard practices. Any alteration or deviation from the above specifications involving extra costs, will be executed.	
I only upon written arriess: And wat hacome an extra charge over and word or	Note: This proposal may be withdrawn by us it not accepted within(30) days.
estimate. All agreements contingent upon strikes, accidents or delays beyond our control. Owner to carry fire, tornade and other necessary insurance. Our	by us a flot accepted within
workers are fully covered by Workman's Compensation Insurance.	
Eleceptamice of Proposal - The above prices, specifications and condition	16 Son State
are substactory and are hereby accepted. You are authorized to do the work as specified. Payment will be made as outlined above.	Signature
Date of Acceptance:	Signature
If payment is not received within 30 (thirty days) from date of	invoice, legal action can be taken to collect any balance due.  added to the account to be collected.



SECTION 1

## **SCREENED ENCLOSURES**



PERIMETER MEMBER

CABLE CONNECTION (SEE DETAILS SECTION 1)

GIRT 1" x 2" (TYP.) GRADE CABLE CONNECTION (SEE DETAILS SECTION 1)

(TABLE 1.3 OR 1.4)

## **TYPICAL MODIFIED HIP ROOF - ELEVATION**

SCALE: N.T.S.

EXISTING STRUCTURE

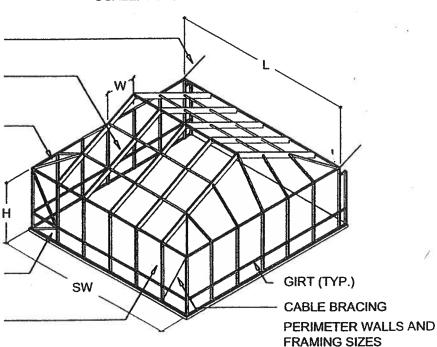
ALUMINUM BEAMS (TABLE 1.1 OR 1.8)

DIAGONAL ROOF BRACING (SEE SCHEMATIC SECTION 1)

K-BRACING (OPTIONAL)

ALUMINUM COLUMNS (TABLE 1.3 OR 1.6)

SIZE MEMBERS PER APPROPRIATE TABLES



## TYPICAL MODIFIED HIP ROOF - ISOMETRIC

SCALE: N.T.S.

# FILE COPY

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 32121 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556 Pool Gad.



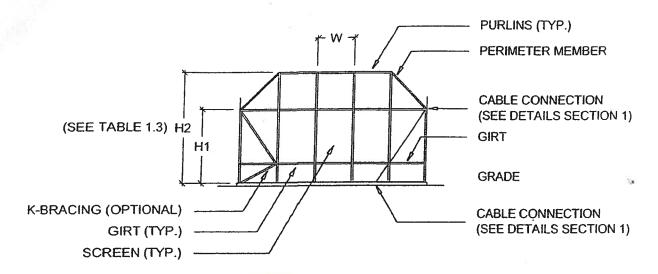
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## SECTION 1

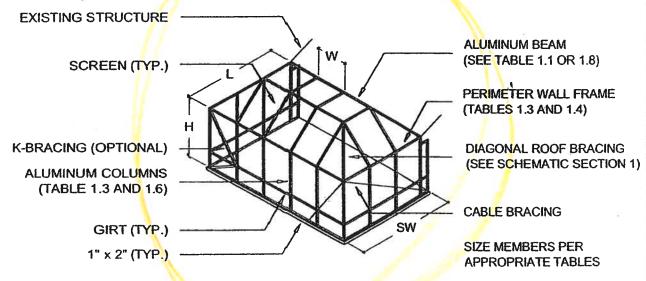
## SCREENED ENCLOSURES



NOTE: USE H2 FOR CABLE AREA CALCULATION

## TYPICAL MANSARD ROOF - ELEVATION

SCALE: N.T.S.



## TYPICAL MANSARD ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

## Lawrence E. Bennett, P.E. FL # 16644

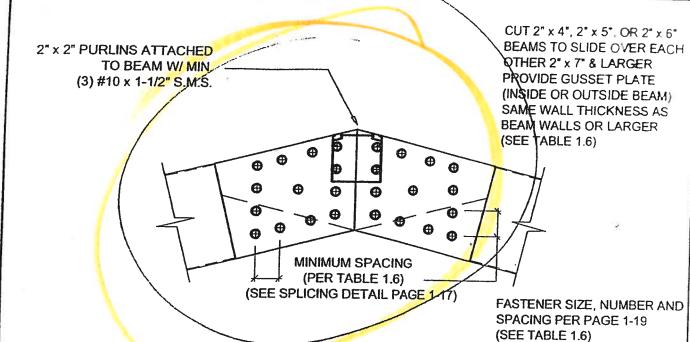
CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 32121 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556



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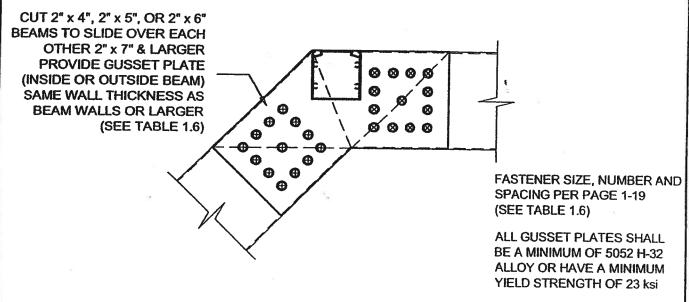
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SECTION 1



## TYPICAL SIDE PLATE CONNECTION DETAIL

SCALE: 3" = 1'-0"



## **TYPICAL SIDE PLATE CONNECTION DETAIL - MANSARD ROOF**

SCALE: 3" = 1'-0"



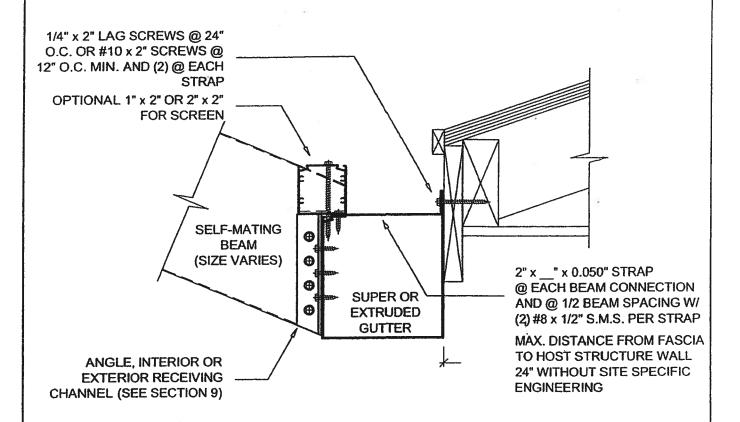
## Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 32121 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556

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# ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"



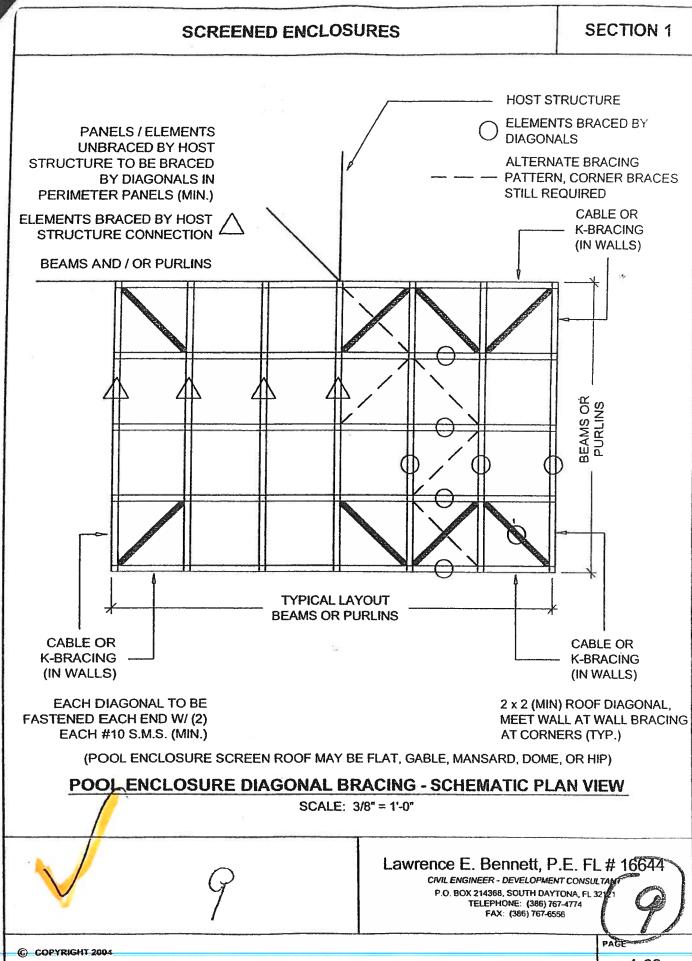
Lawrence E. Bennett, P.E. FL # 1664

CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 3212 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556

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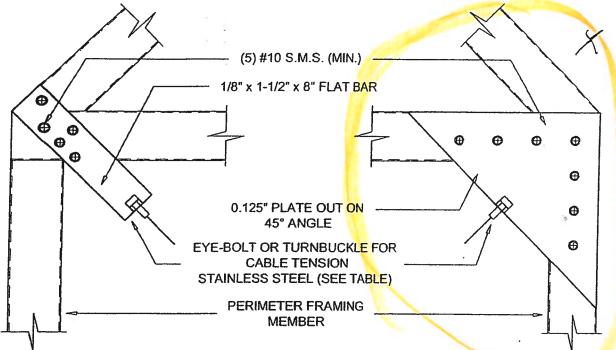
NOT TO BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE WRITTEN PERMISSION OF LAWRENCE E. BENNETT, P.E.



NOT TO BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE WRITTEN PERMISSION OF LAWRENCE E. BENNETT, P.E.

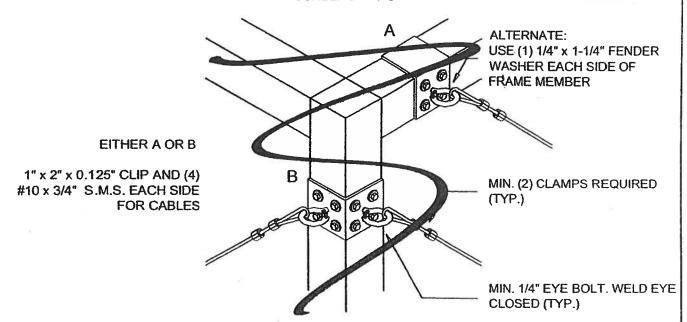


SECTION 1



## **TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1**

SCALE: 3" = 1'-0"



## **ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A**

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT

P.O. BOX 214368, SOUTH DAYTONA, FL 3212 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556

PAGE

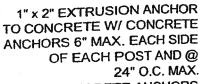
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## SECTION 1 SCREENED ENCLOSURES 2" x 2" x 0.125" ANGLE 2-1/4" x 1-1/2" CONCRETE STAINLESS STEEL CABLE ANCHORS (MIN.) CABLE CLAMP (SEE TABLE) MIN. 3-3/4" NOTE: (4" NOMINAL) SEE PAGE 1-41 FOR NUMBER SLAB OF CABLES REQUIRED DISTANCE FROM EDGE OF SLAB = 5(D) OF SCREW ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B SCALE: 3" = 1'-0" SELECT ANCHOR FROM TABLE 9-1, NIN. SHEAR 607# FOR 3/32" CABLE AND 594# FOR 1/8" CABLE FOR 3/32" CABLE (1) 1/4" x 1-1/2" CONCRETE ANCHOR (MIN.) @ 5d MIN. 5d (MIN.) 5-1/2" (6" NOMINAL) SLAB (MIN.) 5d (MIN.) 2500 P.S.I. CONCRETE 6 x 6 - 10 x 10 WELDED WIRE MESH OR FIBER MESH CONCRETE ALTERNATE GABLE CONNECTIONS AT FOUNDATION - DETAIL 2C SCALE: 3" = 1'-0" Lawrence E. Bennett, P.E. FL# 16644 CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 32121 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556 PAGE 1-43 NOT TO BE REPRODUCED IN WHOLE OR IN PART WITHOUT THE WRITTEN PERMISSION OF LAWRENCE & BENNETT, P.S.

## **SECTION 1**

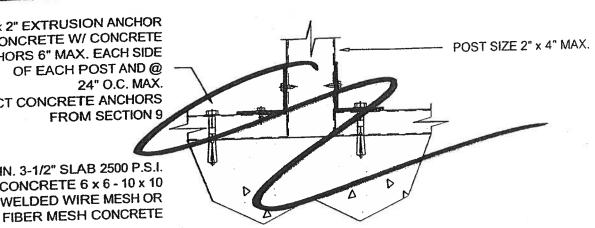
## SCREENED ENCLOSURES



SELECT CONCRETE ANCHORS

FROM SECTION 9

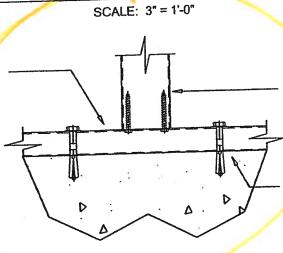
MIN. 3-1/2" SLAB 2500 P.S.I. **CONCRETE 6 x 6 - 10 x 10** WELDED WIRE MESH OR



# SIDE WALL POST TO PLATE TO CONCRETE DETAIL

1" x 2" EXTRUSION ANCHOR TO CONC. W/ CONC. ANCH. 6" MAX. EA. SIDE OF EA. POST AND @ 24" O.C. MAX. SELECT CONCRETE ANCHORS FROM SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I. CONC. 6 x 6 - 10 x 10 W.W.M. OR FIBER MESH CONC.



2" x 2", 2" x 3" OR 2" x 4" HOLLOW SECTION (SEE TABLES)

MIN. (3) #10 x 1-1/2" S.M.S. INTO SCREW BOSSES

MASONRY ANCHOR @ 6" EA. SIDE OF POST AND @ 24" O.C. MAX. SELECT CONCRETE **ANCHORS FROM SECTION 9** 

# SIDE WALL HOLLOW POST TO BASE DETAIL

SCALE: 3" = 1'-0"

# POOL ENCLOSURE UPRIGHT TO DECK ANCHOR REQUIREMENTS

## **General Notes and Specifications:**

1. The uplift load on a pool enclosure upright is calculated as 1/2 the beam span x the beam spacing x the screen load of 7# / Sq. Ft.

#### **EXAMPLE:**

FOR A 2" x 6" BEAM WITH A SPAN OF 23' AND A BEAM & UPRIGHT SPACING OF 7' USE:

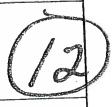
1/2 x 17'-11" x 7' x 10# / Sq. Ft. = 627.2# UPLIFT

- 2. Table 1.6 of this manual uses the worst case loads for all cases.
- 3. In all cases there must be a primary anchor within 6" of each side of the upright.

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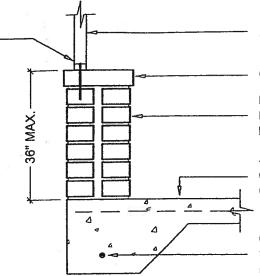
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**SECTION 1** 

1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND **ROWLOCK INTO FIRST** COURSE OF BRICKS

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL 1" WIDE x 0.063" THICK STRAP @ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4" S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR **FOOTING** 



**ALUMINUM FRAME SCREEN** WALL

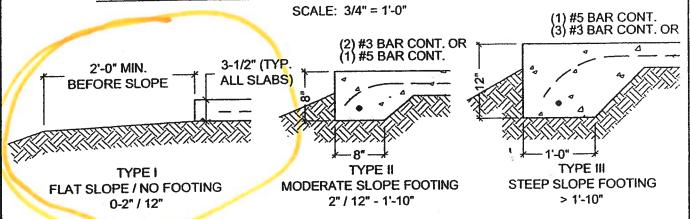
**CAP BRICK** 

**BRICK KNEEWALL TYPE 'S'** MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO **CONCRETE SLAB (SEE NOTES** CONCERNING FIBER MESH)

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

## BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS



## Notes for all foundation types:

- 1. No footing required except when addressing erosion until the slab width in the direction of the primary exceeds 32 ft., then a Type II footing is required under the load bearing wall only unless the side wall exceeds 16 ft. in height or the enclosure is in a "C" exposure catagory in which case a Type II footing is required for all walls.
- 2. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified, using a pocket penetrometer, field soil test, or by a soil testing lab, to be above 1,500 PSF prior to placing the slab.
- 3. The slab / foundation shall be cleared of debris and roots and compacted prior to placement of concrete.
- 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or crack control fiber mesh: Fibermesh ® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per maufacturer's specification may be used in lieu of wire mesh.
- 5. If local building codes require a minimum footing, use Type II footing or footing sections required by local code. Local code governs.

**SLAB-FOOTING DETAILS** 

SCALE: 3/4" = 1'-0"



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Table 1.1

## Allowable Spans for Primary Screen Roof Frame Members Aluminum Alloy 6063 T-6

For Areas with Wind Loads up to 150 M.P.H. and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

				Tı	ibutary	Loa	d Width	w.	= Beam	Spa	cing			
Hollow Sections	3'-0"	•	4'-0"		5'-0"	•	6'-0'	•	7'-0'	•	8'-0'	8'-0"		**
			Al	low	able Spa	n 'I	.' / bend	ing	b' or de	flec	tion 'd'			
2" x 2" x 0.044"	9'-10"	b	8'-7"	b	7'-8"	b	6'-11"	b	6'-6"	b	6'-1"	b	5'-8"	b
2" x 2" x 0.055"	10'-9"	b	9'-4"	b	8'-4"	b	7'-7"	b	7'-1"	b	6'-7"	b	6'-3"	b
2" x 3" x 0.045"	13'-4"	ь	11'-7"	b	10'-4"	b	9'-5"	b	8'-9"	b	8'-2"	b	7'-8"	b
2" x 4" x 0.050"	14'-8"	b	12'-8"	b	11'-4"	Ь	10'-4"	b	9'-7"	b	8'-11"	b	8'-5"	b

	[			Ti	ibutary l	.oa	d Width	W	= Beam \$	Spa	cing			
Self Mating Sections	3'-0"		4'-0"		5'-0"	-	6'-0"		03		8'-0"	'	<b>3</b> 0,	•
			Al	low	able Spa	n 'i	' / bendi	ng	'b' or def	lec	tion 'd'			
2" x 4" x 0.044 x 0.100"	19'-11"	ь	17'-4"	ь	15'-6"	b	14'-2"	b	13'-1"	Ь	12'-3"	b	11'-6"	b
2" x 5" x 0.050" x 0.100"	24'-9"	b	21'-5"	b	19'-2"	b	17'-6"	b	16'-2"	b	15'-2"	b	14'-3"	b
2" x 6" x 0.050" x 0.120"	28'-7"	ь	24'-9"	b	22'-2"	Ь	20'-3"	b	18'-9"	ь	17'-6"	b	16'-6"	b
2" x 7" x 0.055" x 0.120"	32'-3"	b	27'-11"	b	24'-11"	b	22'-9"	b	21'-1"	b	19'-9"	b	18'-7"	b
2" x 7" x 0.055" w/ insert	42'-10"	ь	37'-1"	Ь	33'-2"	b	30'-4"	b	28'-1"	b	26'-3"	b	24'-9"	b
200 10 20 00 70 10 10 20 70 70	41'-7"	ь	36'-1"	b	32'-3"	b	29'-5"	b		<b>→</b>	25'-6"	b	24'-0"	b
2" x 9" x 0.072" x 0.224"	45'-1"	đ	39'-1"	b	34'-11"	b	31'-11"	b	29'-6"	b	27'-8"	b	26'-1"	б
2" x 9" x 0.082" x 0.310"	49'-6"	b	42'-11"	ь	38'-4"	b	35'-0"	b	32'-5"	b	30'-4"	b	28'-7"	b
2" x 10" x 0.092" x 0.369"	59'-6"	b	51'-7"	b	46'-1"	b	42'-1"	b	38'-11"	b	36'-5"	b	34'-4"	b

				Tı	ibutary l	oa	d Width	W	= Beam	Spa	cing			
Snap Sections	3'-0"		4'-0"	Y	5'-0"		6'-0"	,	7'-0"		80.	•	9,-0,	•
			All	low	able Spa	ın 'L	'/ bend	ing	b' or de	flec	tion 'd'			
2" x 2" x 0.044"	11'-9"	b	10'-2"	ь	9'-1"	b	8'-4"	b	7'-8"	Ь	7'-2"	b	6'-9"	b
2" x 3" x 0.045"	15'-1"	b	13'-1"	ь	11'-8"	b	10'-8"	b	9'-10"	b	9'-3"	b	8'-8"	b
2" x 4" x 0.045"	18'-5"	ь	15'-11"	Ь	14'-3"	b	13'-0"	b	12'-1"	b	11'-3"	b	10'-8"	b
2" x 6" x 0.062"	31'-3"	ь	27'-1"	b	24'-2"	Ь	22'-1"	b	20'-5"	b	19'-2"	Ь	18'-0"	b
2" x 7" x 0.062"	34'-9"	ь	30'-1"	ь	26'-11"	b	24'-7"	b	22'-9"	b	21'-3"	b	20'-1"	b

#### Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 55' and a maximum upright height of 20'. Structures larger than these limits shall have site specific engineering.
- 3. Spans are based on a minimum of 10# / Sq. Ft. for up to a 150 M.P.H. wind load.
- 4. Span is measured from center of beam and upright connection to fascia or wall connection.
- 5. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 6. Purlin spacing shall not exceed 6'-8". For beam spans greater than 40'-0" the beam at the center purlin and one purlin for each 14'-0" on each side of the center purlin shall include lateral bracing as shown in detail (48'-0") span with purlins at 6'-8" o.c. center purlin and (2) purlins each side of center purlin need lateral bracing.

7. Spans may be interpolated.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 11'-4"



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Table 1.2 Allowable Spans for Secondary Screen Roof Frame Members
Aluminum Alloy 6063 T-6

For Areas with Wind Loads up to 150 M.P.H. and Latitudes Below 30°-30'-00" North (Jacksonville, FL) A. Sections Fastened To Beams With Clips

		Tributary Load Width 'W' = Purlin Space								pac	ing			
<b>Hollow Sections</b>	3'-6'	•	4'-0'	•	4'-6'	•	5'-0'	•	5'-6"		6'-0'	•	6'-8"	
			Al	low	able Spa	ın L	' / bendi	ng	b' or de	flect	ion 'd'			
2" x 2" x 0.044"	7'-8"	d	7'-4"	d	7'-0"	d	6'-9"	d	6'-6"	b	6'-3"	b	5'-11"	b
2" x 2" x 0.055"	8'-1"	đ	7'-9"	d	7'-5"	ď	7'-2"	ď	6'-11"	d	6'-9"	d	6'-6"	_ <u>b</u>
3" x 2" x 0.045"	8'-8"	d	8'-3"	d	7'-11"	d	7'-8"	d	7'-5"	d	7'-3"	d	6'-11"	
2" x 3" x 0.045"	10'-9"	d	10'-3"	đ	9'-9"	b	9'-3"	b	8'-10"	ь	8'-5"	b	8'-0"	<u> </u>
2" x 4" x 0.050"	12'-2"	ь	11'-4"	Ь	10'-8"	ь	10'-2"	ь	9'-8"	b	9'-3"	ĥ	8'-9"	_ <u>b</u>

				Tril	outary L	oad	Width 'V	N' =	Purlin S	Space	ing			
Snap Sections	3'-6'	•	4'-0'	•	4'-6'	•	5'-0'	•	5'-6'	•	6'-0"		6'-8'	•
			Al	low	able Spa	ın L	' / bendi	ing	b' or de	flec	tion 'd'			
2" x 2" x 0.044	8'-5"	d	8'-1"	d	7'-9"	d	7'-6"	d	7'-3"	d	7'-0"	d	6'-9"	d
2" x 3" x 0.045"	11'-7"	d	11'-1"	d	10'-8"	ď	10'-4"	d	9'-11"	b	9'-6"	ь	9'-0"	b
2" x 4" x 0.045"	14'-8"	d	14'-0"	d	13'-6"	d	12'-9"	Ь	12'-2"	ь	11'-8"	ь	11'-1"	b

B. Sections Fastened Through Beam Webs Into Screw Bosses

				Tril	butary Lo	oad	Width V	N' =	Purlin S	pac	ing			
<b>Hollow Sections</b>	3′-6′	•	4'-0'	•	4'-6"		5'-0"	. 1	1000		6'-0"		6'-8'	•
			Al	low	able Spa	n 'L	' / bendi	ng	'b' or def	lect	ion 'd'	100		
2" x 2" x 0.044"	9'-2"	b	8'-7"	ь	8'-1"	b	7'-8"	b	7'-4"	ь	6'-11"	Ь	6'-7"	b
2" x 2" x 0.055"	9'-11"	b	9'-4"	b	8'-10"	Ь	8'-4"	b	7'-11"	Ь	7'-7"	Ь	7'-3"	ь
2000年18月1日	12'-4"	b	11'-7"	b	10'-11"	b	10'-4"	b	75-100	3	9'-5"	ь	8'-11"	b
2" x 4" x 0.050"	13'-7"	b	12'-8"	b	11'-11"	ь	11'-4"	ь	10'-10"	b	10'-4"	h	9'-10"	h

			Tril	outary L	oad	Width 1	N' =	Purlin S	pac	ing			
Snap Sections	3'-6"	4'-	0"	4'-6	"	5'-0'	•	5'-6'	• 1	6'-0"		6'-8'	•
			Allow	able Sp	an 'L	'/ bend	ing 'l	b' or de	flect	ion 'd'			
2" x 2" x 0.044"	10'-11"	b 10'-2	* b	9'-7"	ь	9'-1"	ь	8'-8"	ь	8'-4"	ь	7'-11"	b

#### Notes

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Spans are based on a minimum of 10# / Sq. Ft. for up to a 150 M.P.H. wind load.3. Span is measured from center of beam and upright connection to fascia or wall connection.
- 3. Span is measured from center of beam and upright connection to fascia or wall connection.
- 4. Purlin spacing shall not exceed 6'-8". For beam spans greater than 40'-0" the beam at the center purlin and one purlin for each 14'-0" on each side of the center purlin shall include lateral bracing as shown in detail (48'-0") span with purlins at 6'- 8" o.c. center purlin and (2) purlins each side of center purlin need lateral bracing.
- 5. Spans may be interpolated.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS. Example:

Max. "L" for 2" x 4" x 0.050" hollow section fastened to beam with clips with "W" = 5'-0" = 10'-2"





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Table 1.3

Allowable Post / Upright Heights for Primary Screen Wall Frame Members Aluminum Allov 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.\*

	1			Tril	butary L	.oad	Width '	W' =	= Uprigh:	t Sp	acing			
<b>Hollow Sections</b>	3'-0'	•	4'-0'		5'-0'	•	6′-0′	•	7'-0"		8'-0'	•	9'-0'	ng .
			A	low	able He	ight	'H' / ber	ndin	g 'b' or o	lefle	ection 'd	•		
2" x 2" x 0.044"	8'-4"	b	7'-3"	b	6'-6"	b	5'-11"	b	<b>'5'-6"</b>	b	5'-1"	ь	4'-10"	ı
2" x 2" x 0.055"	9'-1"	b	7'-11"	b	7'-1"	b	6'-5"	Ь	5'-11"	b	5'-7"	b	5'-3"	1
2" x 3" x 0.045"	11'-3"	b.	9'-9"	Ь	8'-9"	b	7'-11"	b	7'-5"	b	6'-11"	ь	6'-6"	ŧ
2" x 4" x 0.050"	12'-5"	b	10'-9"	ь	9'-7"	b	8'-9"	b	8'-1"	b	7'-7"	b	7'-2"	1

	V=			Tri	butary L	oac	Width 7	N' :	= Upright	Sp	acing			
Self Mating Sections	3'-0"		4'-0"		5'-0"		6'-0"		100	37	8'-0"		9'-0"	
			Al	low	able Hei	ght	'H' / ben	din	g 'b' or d	efle	ection 'd'			- 1
2" x 4" x 0.044 x 0.100"	16'-11"	ь	14'-8"	b	13'-1"	b	11'-11"	b	11'-1"	b	10'-4"	b	9'-9"	b
A STATE OF THE PARTY OF THE PAR	20'-11"	ь	18'-1"	b	16'-2"	ь	14'-9"	b,		7	12'-10"	b	12'-1"	b
2" x 6" x 0.050" x 0.120"	24'-2"	b	20'-11"	b	18'-9"	b	17'-1"	b	15'-10"	b	14'-10"	b	13'-11"	b
2" x 7" x 0.055" x 0.120"	27'-3"	b	23'-7"	b	21'-1"	b	19'-3"	b	17'-10"	b	16'-8"	b	15'-9"	b
2" x 7" x 0.055" w/ insert	36'-3"	b	31'-4"	b	28'-1"	b	25'-7"	b	23'-9"	b	22'-2"	b	20'-11"	b
2" x 8" x 0.072" x 0.224"	35'-2"	b	30'-6"	b	27'-3"	ь	24'-10"	b	23'-0"	b	21'-6"	b	20'-4"	b
2" x 9" x 0.072" x 0.224"	38'-2"	b	33'-0"	b	29'-6"	Ь	26'-11"	b	24'-11"	b	23'-4"	b	22'-0"	b
2" x 9" x 0.082" x 0.310"	41'-10"	ь	36'-3"	ь	32'-5"	b	29'-7"	b	27'-5"	b	25'-8"	b	24'-2"	b
2" x 10" x 0.092" x 0.369"	50'-4"	ь	43'-7"	Ь	38'-11"	b	35'-7"	b	32'-11"	ь	30'-10"	b	29'-1"	ь

				Tri	butary L	.oac	Width '	W'=	Upright	Sp	acing			
Snap Sections	3'-0"	'	4'-0"		5'-0"	•	6'-0"		7'-0"	•	80.	•	9'-0"	,
- -			Al	low	able Hei	ght	'H' / ber	ıdin	g 'b' or c	defl	ection 'd	•		
2" x 2" x 0.044"	9'-11"	b	8'-7"	b	7'-8"	b	7'-0"	b	6'-6"	b	6'-1"	b	5'-9"	b
2" x 3" x 0.045"	12'-9"	b	11'-0"	b	9'-10"	b	9'-0"	ь	8'-4"	b	7'-10"	, b	7'-4"	ь
2" x 4" x 0.045"	15'-7"	b	13'-6"	Ь	12'-1"	b	11'-0"	Ь	10'-2"	b	9'-7"	ъ	8'-11"	b
2" x 6" x 0.062"	26'-5"	b	22'-10"	Ь	20'-5"	b	18'-8"	b	17'-3"	b	16'-2"	b	15'-3"	ь
2" x 7" x 0.062"	29'-5"	b	25'-5"	b	22'-9"	b	20'-9"	ь	19'-3"	b	17'-11"	ь	16'-11"	ь

<sup>\*</sup> For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specification page for tables at the beginning of this section and example below.

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- 3. Above heights do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 7. Heights may be interpolated.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

IF SPANS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

8.9 × 1.13 = 10.057

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Table 1.4 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.\*

A. Sections As Horizontals Fastened To Posts With Clips

	1				Tr	ribut	ary Loa	W b	idth "W"					
Hollow Sections	3'-6'		4'-0'	•	4'-6'	•	5'-0"	'	5'-6'	18	6'-0'	•	6'-8"	•
		1	Al	owa	ble Hei	ghts	'H' / be	ndir	ig 'b' or	defi	ection 'c	1'		
2" x 2" x 0.044"	6'-10"	d	6'-6"	ь	6'-1"	b	5'-9"	b	5'-6"	b	5'-3"	b	5'-0"	b
2" x 2" x 0.055"	7'-3"	d	6'-11"	d	6'-8"	b	6'-4"	b	6'-0"	b	5'-9"	b	5'-6"	b
3" x 2" x 0.045"	7'-9"	d	7'-5"	d	7'-1"	d	6'-10"	ď	6'-7"	b	6'-4"	ь	5'-11"	b
2" x 3" x 0.045"	9'-4"	ь	8'-9"	ь	8'-3"	b	7'-10"	b	7'-5"	b	7'-2"	ь	6'-9"	b
2" x 4" x 0.050"	10'-3"	ь	9'-7"	Ы	9'-0"	b	8'-7"	ь	8'-2"	ь	7'-10"	b	7'-5"	b

Snap Sections					ble Hei									
2" x 2" x 0.044"	7'-6"	d	7'-2"	d	6'-11"	d	6'-8"	b	6'-4"	ь	6'-1"	b	5'-9"	b

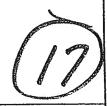
B. Sections As Horizontals Fastened To Posts Through Side into Screw Bosses

	7				Tr	ibut	ary Loa	d W	idth 'W'					
Hollow Sections	3'-6'		4'-0"		4'-6'	,	5'-0'	•	5'-6'	e	6'-0'		6'-8'	
			Al	owa	ble Hei	ghts	'H' / be	ndir	g 'b' or	def	ection 'c	1'		
2" x 2" x 0.044"	7'-9"	ь	7'-3"	b	6'-10"	b	6'-6"	b	6'-2"	ь	5'-11"	b	5'-7"	b
2" x 2" x 0.055"	8'-5"	ь	7'-11"	b	7'-5"	b	7'-1"	b	6'-9"	b	6'-5"	b	6'-1"	b
3" x 2" x 0.045"	9'-3"	ь	8'-8"	b	8'-2"	b	7'-9"	b	7'-5"	ь	7'-1"	b	6'-8"	b
2" x 3" x 0.045"	10'-5"	b	9'-9"	b	9'-2"	b	8'-9"	b	8'-4"	b	7'-11"	b	7'-7"	b
2" x 4" x 0.050"	11'-6"	ь	10'-9"	b	10'-1"	b	9'-7"	b	9'-2"	b	8'-9"	b	8'-4"	_ <u>b</u>
Snap Sections			All	owa	ble Hei	ghts	'H' / be	ndir	g 'b' or	defi	ection 'c	ľ		
2" x 2" x 0.044"	9'-2"	b	8'-7"	b	8'-1"	b	7'-8"	b	7'-4"	b	7'-0"	b	6'-8"	b

<sup>\*</sup> For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specifications for tables page at the beginning of this section and example below.

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 20' in mean roof height.
- Span is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of 2"  $\times$  2"  $\times$  0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10  $\times$  1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Spans may be interpolated.

IF HEIGHTS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.



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Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams to Wall Uprights or Beam Splicing

| Beam           | Upright | Minimum Purlin, Girt | Deck    | Notes       | Minim   | ım Number ol | Screws*    | Beam Stitching   |
|----------------|---------|----------------------|---------|-------------|---------|--------------|------------|------------------|
| Size           | Size    | & Knee Brace Size**  | Anchors |             | #8 x ½" | #10 x ½"     | #12 x 1/2" | Screw @ 24" O.C. |
| 2" x 3"        | 2" x 3" | 2" x 2" x 0.044"     | 2       | Full Lap    | 6       | 4            | 4          | -                |
| 2" x 4"        | 2" x 3" | 2" x 2" x 0.044"     | 2       | Full Lap    | 8       | 6            | 4          | #8               |
| 2" x 4"        | 2" x 4" | 2" x 2" x 0.044"     | 2       | Full Lap    | 8       | 6            | 4          | #10              |
| 2" x 5"        | 2" x 3" | 2" x 2" x 0.044"     | 2       | Full Lap    | 8       | 6            | 4          | #8               |
| 2" x 6"        | 2" x 3" | 2" x 2" x 0.044"     | 4       | Full Lap    | 10      | 8            | 6          | #10              |
| 2" x 6"        | 2" x 4" | 2" x 2" x 0.044"     | 4       | Partial Lap | 10      | 8            | 6          | #10              |
| 2" x 7"        | 2" x 4" | 2" x 2" x 0.044"     | 4       | Partial Lap | 14      | 12           | 10         | #12              |
| and the second | 370     | 2"×9" [13:044"       |         | Partial Lap | 16      | 14           |            |                  |
| 2" x 9"        | 2" x 6" | 2" x 3" x 0.045"     | 6       | Partial Lap | 18      | 16           | 14         | #14              |
| 2" x 9"**      | 2" x 7" | 2" x 4" x 0.050"     | 8       | Partial Lap | 20      | 18           | 16         | #14,             |
| 2" x 10"       | 2" x 8" | 2" x 4" x 0.050"     | 10      | Partial Lap | 20      | 18           | 16         | #14              |

| Screw Size  | Minimum Distance and | Spacing of Screws | Gusset Plate Th            | ickness        |
|-------------|----------------------|-------------------|----------------------------|----------------|
|             | Edge To Center       | Center To Center  | Beam Siże                  | Thickness      |
| #B          | 5/16"                | 5/8"              | 2" x 7" x 0.055" x 0.120"  | 1/16" = 0.063" |
| #10         | 3/8"                 | 3/4"              | 2" x 8" x 0.072" x 0.224"  | 1/8" = 0.125"  |
| #12         | 1/2"                 | 1"                | 2" x 9" x 0.072" x 0.224"  | 1/8" = 0 125"  |
| #14 or 1/4" | 3/4"                 | 1-1/2"            | 2" x 9" x 0.082" x 0306"   | 1/8" = 0.125"  |
| 5/16"       | 7/8"                 | 1-3/4"            | 2" x 10" x 0.092" x 0.369" | 1/4" = 0.25"   |
| 3/8"        | 1"                   | 2"                |                            |                |

<sup>\*</sup> Refers to each side of the connection of the beam and upright and each side of splice connection.

#### Note:

- 1. Connection of 2" x 6" to 2" x 3" shall use a full lap cut or 1/16" gusset plate.
- 2. All gusset plates shall be a minimum of 5052 H-32 Alloy or have a minimum yield strength of 23 ksi.
- 3. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 4. The number of screws is based on the maximum allowable moment of the beam.
- 5. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 6. Hollow splice connections can be made provided the connection is approved by the engineer.
- 7. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
- 8. All beam to upright connections for 2" x 7" beams or larger shall have an internal or external gusset plates. Gusset plates are required for mansard or gable splice connections.
- 9. For gusset plate connections 2" x 9" beams or larger use 3/4" long screws.
- 10. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
- 11. Connect beam to upright w/ H-bar, gusset plate, or angle clips for each splice with 1/2 the screws on each side of the cut.
- 12. For girt size use upright size (i.e. 2" x 6"). Read the 2" x 6" beam row for min. girt of 2" x 2" x 0.044".

Table 1.7 Minimum Size Screen Enclosure Knee Braces and Anchoring Required
Aluminum 6063 T-6

| Brace Length | Extrusion                | Anchoring System                                                                           |
|--------------|--------------------------|--------------------------------------------------------------------------------------------|
| 0' - 2'-0"   | 2" x 2" x 0.044"         | 2" H-Channel With (3) #10 x 1/2" EACH SIDE                                                 |
| To 3'-0"     | 2" x 3" x 0.045"         | 2" H-Channel With (3) #10 x 1/2" EACH SIDE                                                 |
| To 4'-6"     | 2" x 4" x 0.044" x 0.12" | 2" H-Channel With (4) 3/4" long screws (size to be determined by beam size, see table 9.6) |

(See Table 1.6 For Number And Size Of Screws)

#### Note:

1. For required knee braces greater than 4'-6" contact engineer for specifications and details.

2. Cantilever beam detail shown on page 1-32 shall be used for host structure attachment when knee brace length exceeds 4

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<sup>\*\* 0.082&</sup>quot; wall thickness, 0.310" flange thickness