

**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"



**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS	
October 14, 2019	

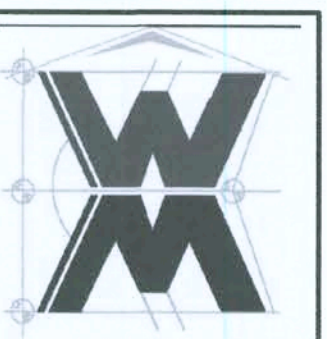
**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

**FRONT & REAR ELEVATIONS**  
SCALE: 1/4" = 1'-0"

**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

MODEL 2145 (LEFT-HAND) FOR:  
**THE HAMRICKS**  
Address: Lot 27, SW Mandiba Drive, Lake City, Florida 32024 (Parcel # 18-55-17-09280-127)  
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.  
426 SW COMMERCE DR, STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wm@wmymyers.net



JOB NUMBER  
20190930

SHEET NUMBER  
**A.1**  
OF 4 SHEETS

*Wm C Myers*





**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

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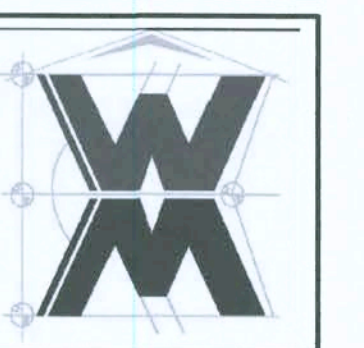


**EXTERIOR ELEVATIONS**  
SCALE: 1/4" = 1'-0"

**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

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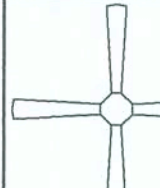

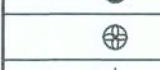

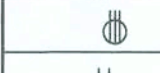
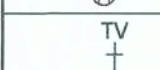

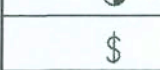
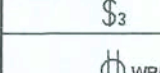
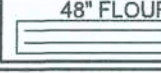

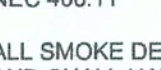

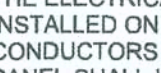
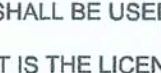
SHEET NUMBER  
**A.2**  
OF 4 SHEETS

*Wm C. Myers*







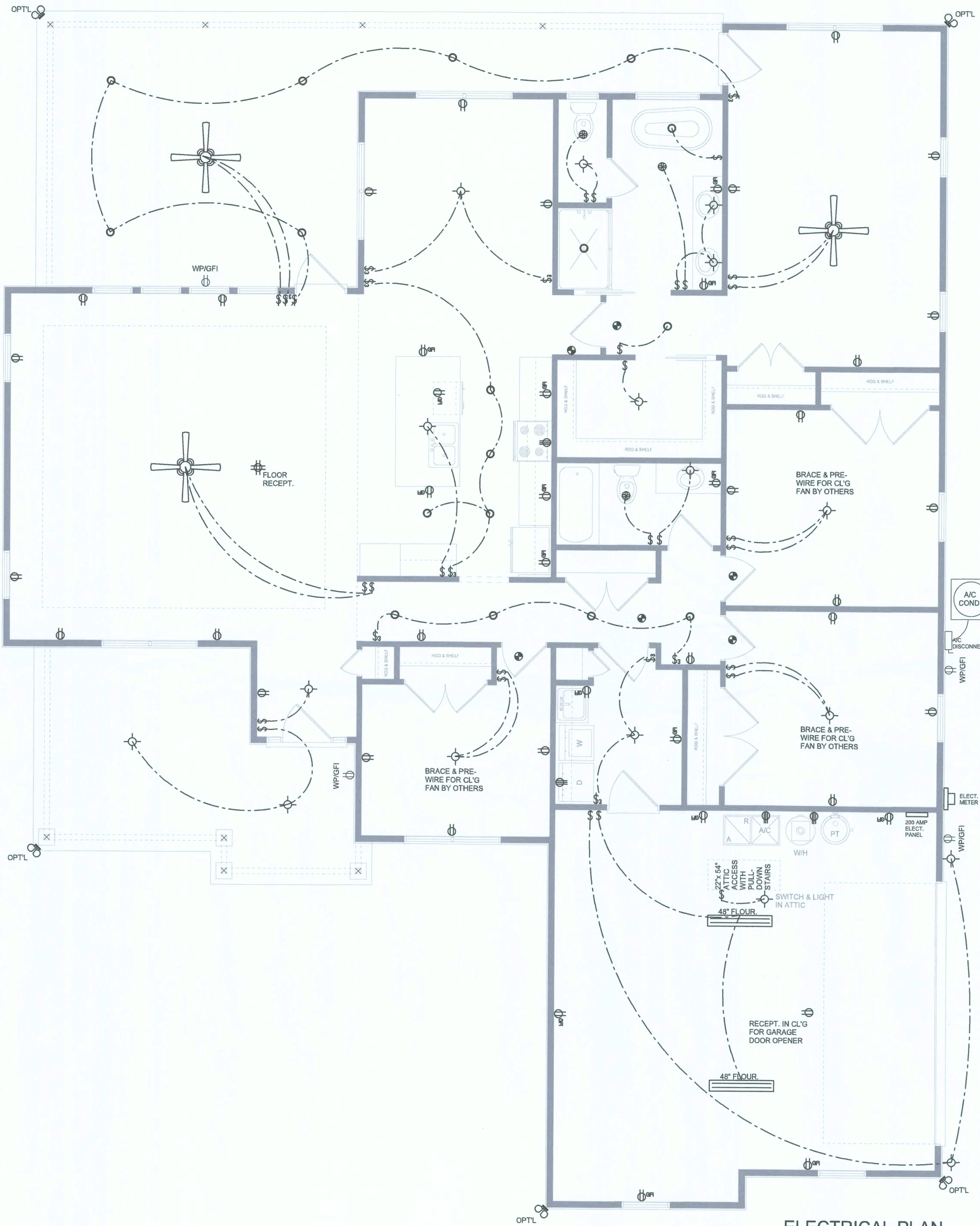
ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:  
ALL INTERIOR RECEPTACLES SHALL BE AFCI  
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER  
NEC 406.11

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR  
AND SHALL HAVE BATTERY BACKUP POWER  
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY  
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE  
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.  
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB  
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR  
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL  
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL  
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

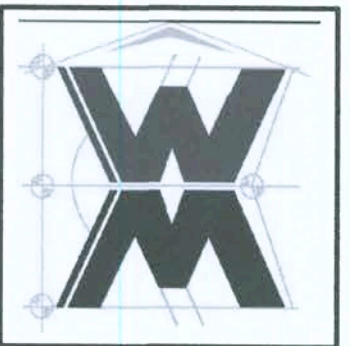
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October 14, 2019



ELECTRICAL PLAN  
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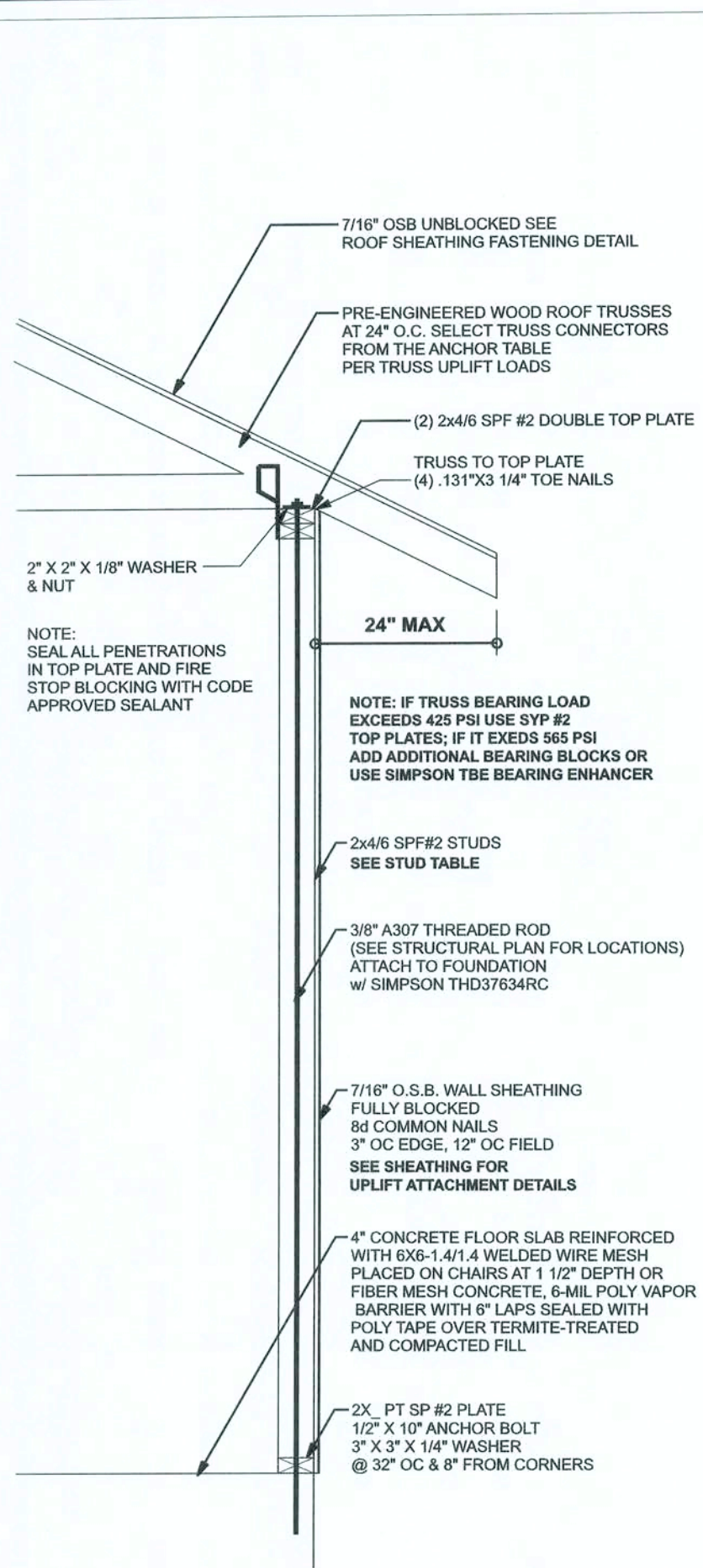


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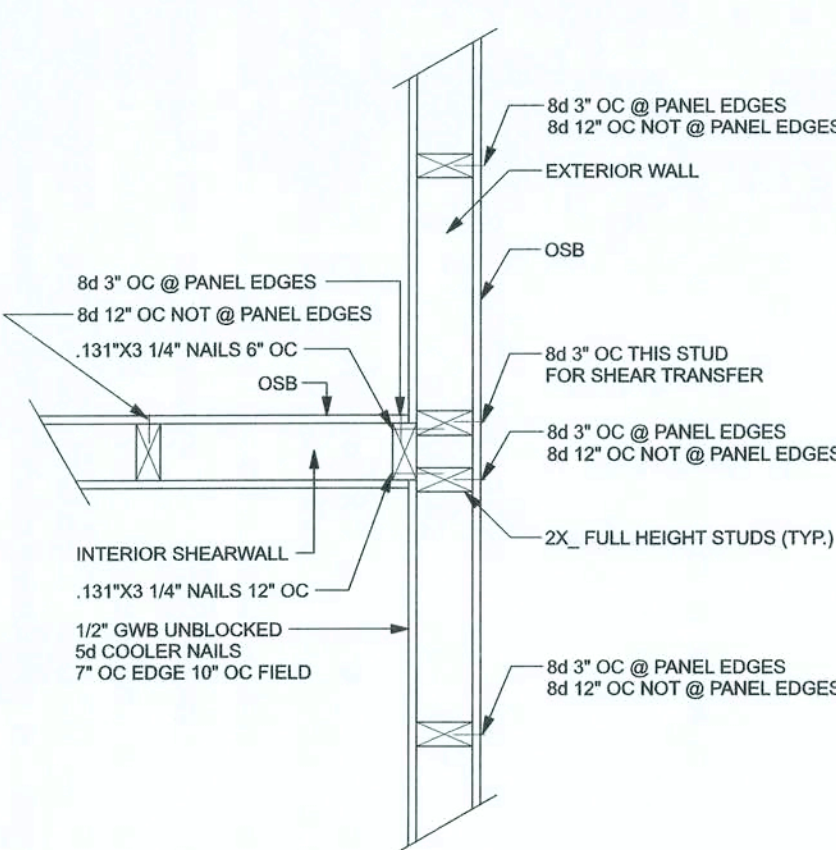
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**A.4**  
OF 4 SHEETS

*Wm C. Myers*

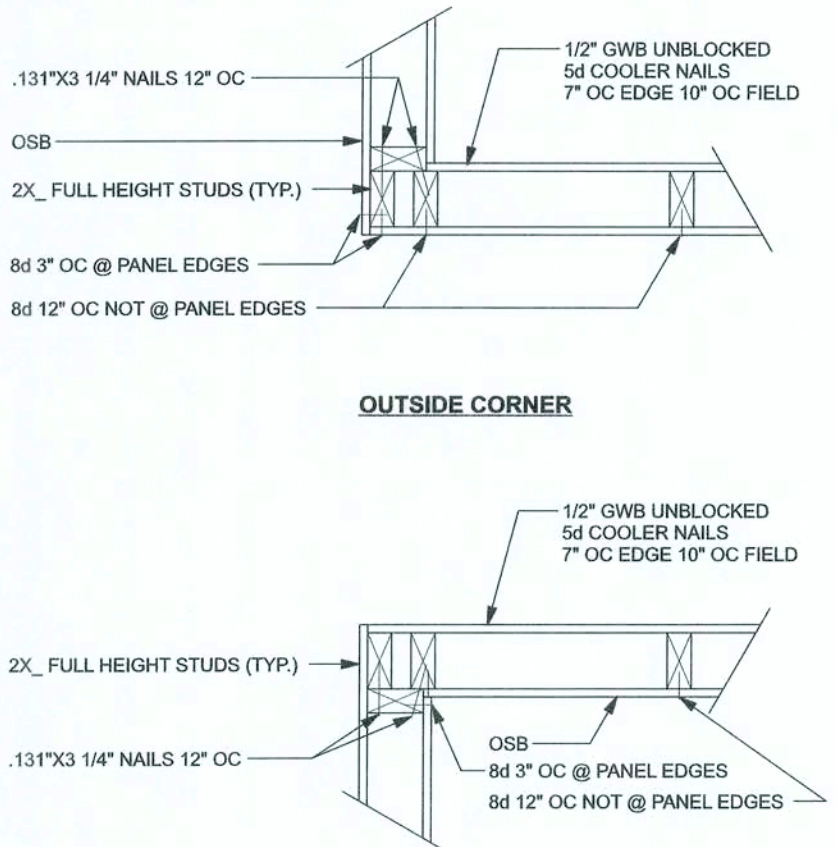




**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME

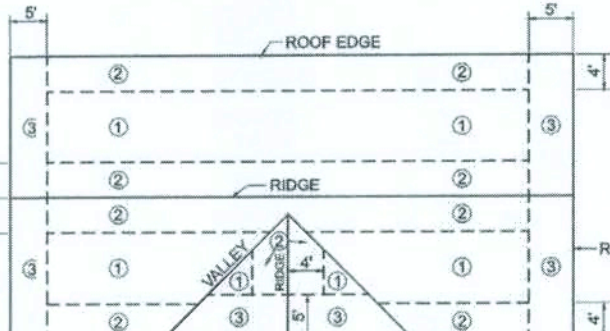


**(TYP.) CORNER FRAMING**  
WOOD FRAME

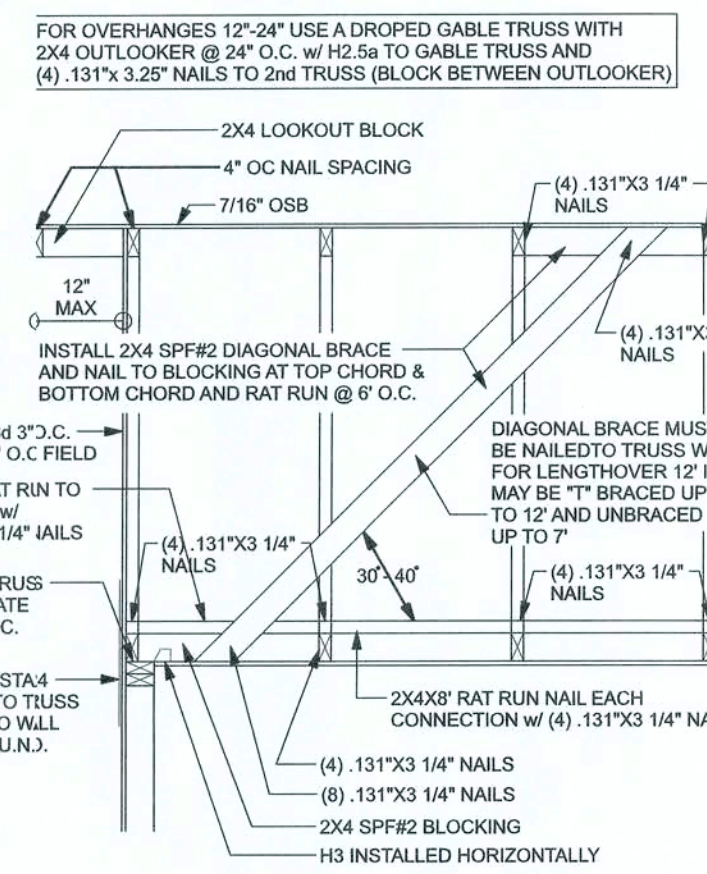
- RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
- 0.113 INCH NOMINAL ROOT SHANK DIAMETER
  - RING DIAMETER OF 0.010 OVER SHANK DIAMETER
  - 16 TO 20 RINGS PER INCH
  - 0.280 INCH FULL ROUND HEAD DIAMETER
  - 2-3/8 INCH NAIL LENGTH

NAILING PATTERN SHALL BE:

4\"/>

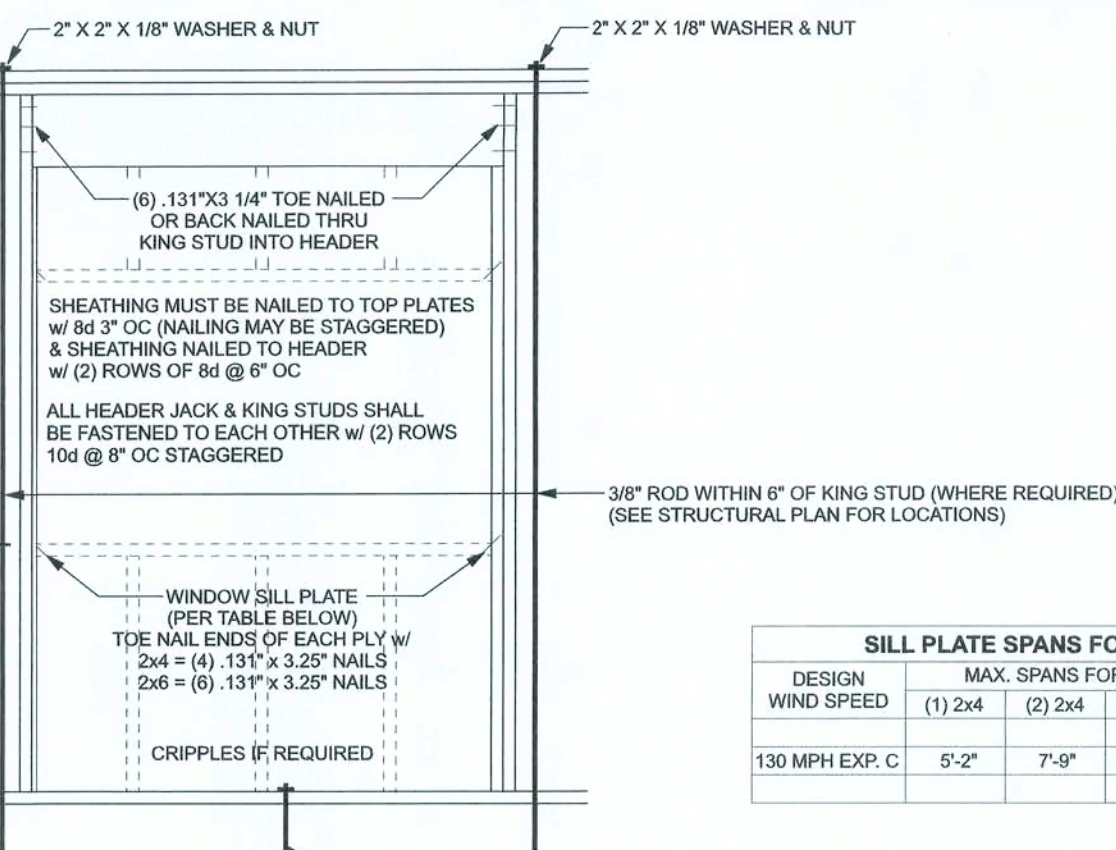


**ROOF SHEATHING FASTENING**

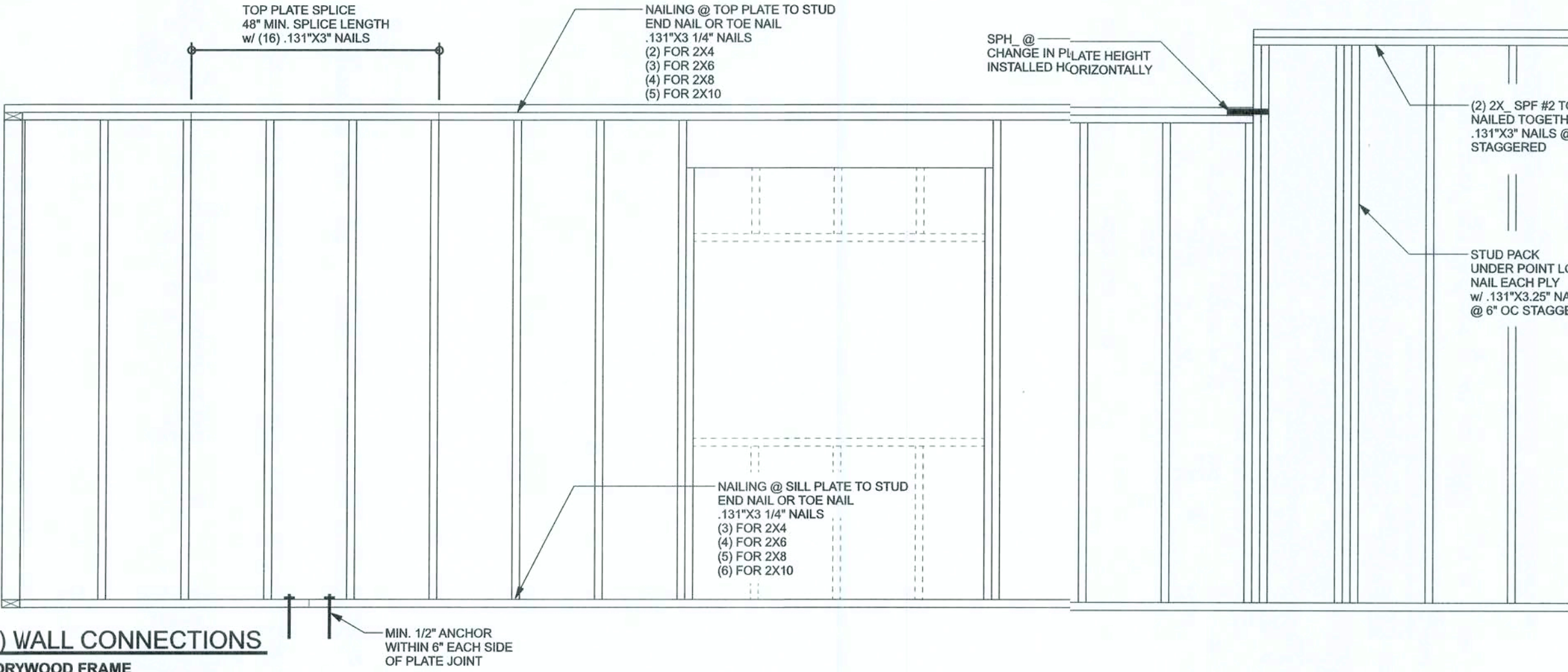


**SPACE R&T RUN & DIAGONAL BRACE 6'-0\"/>**

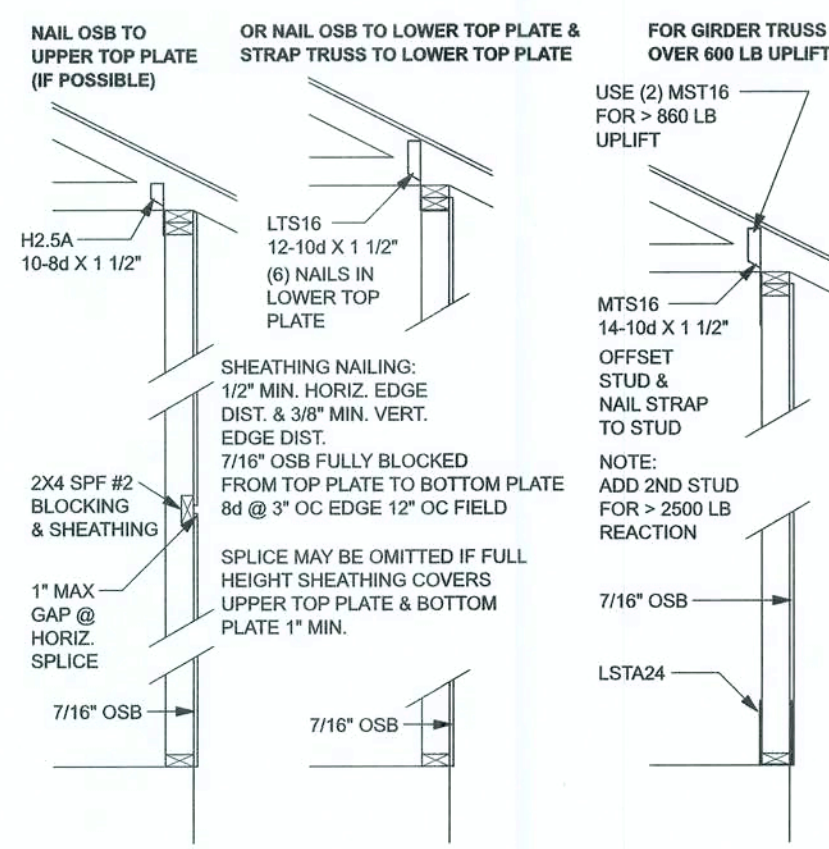
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



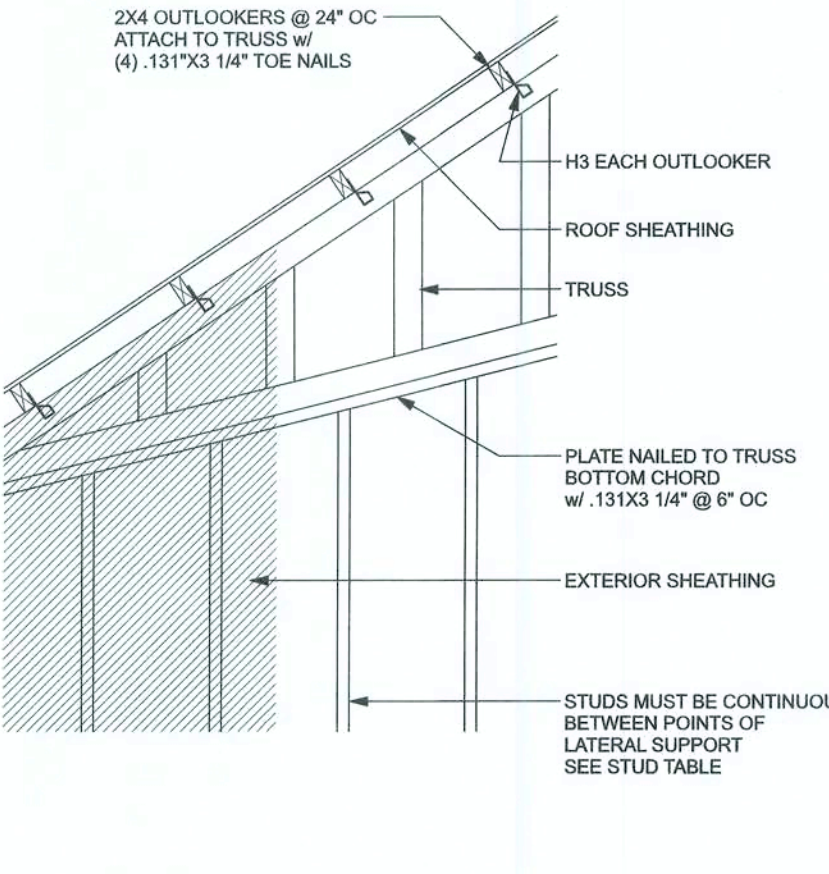
**TYPICAL HEADER STRAPING DETAIL**  
ONE STORY WOOD FRAME



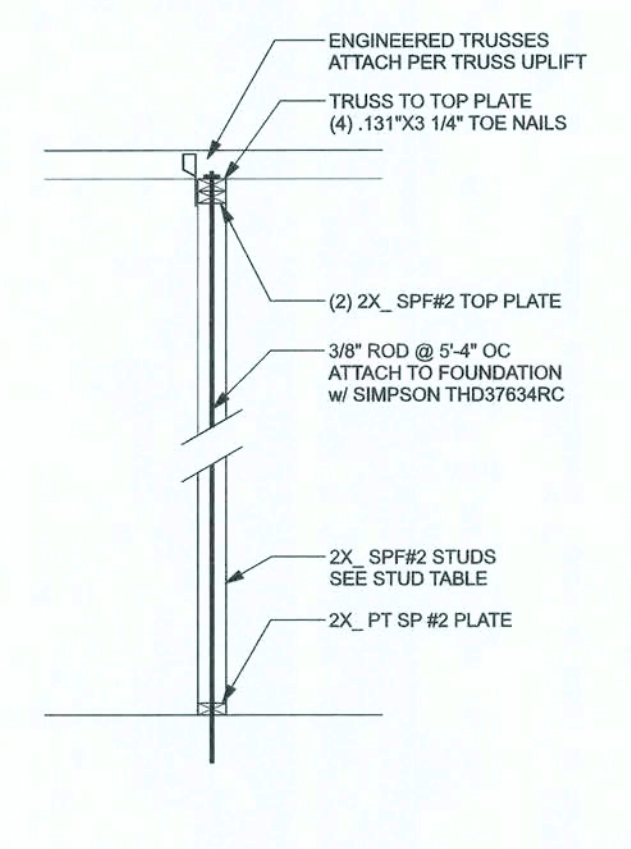
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



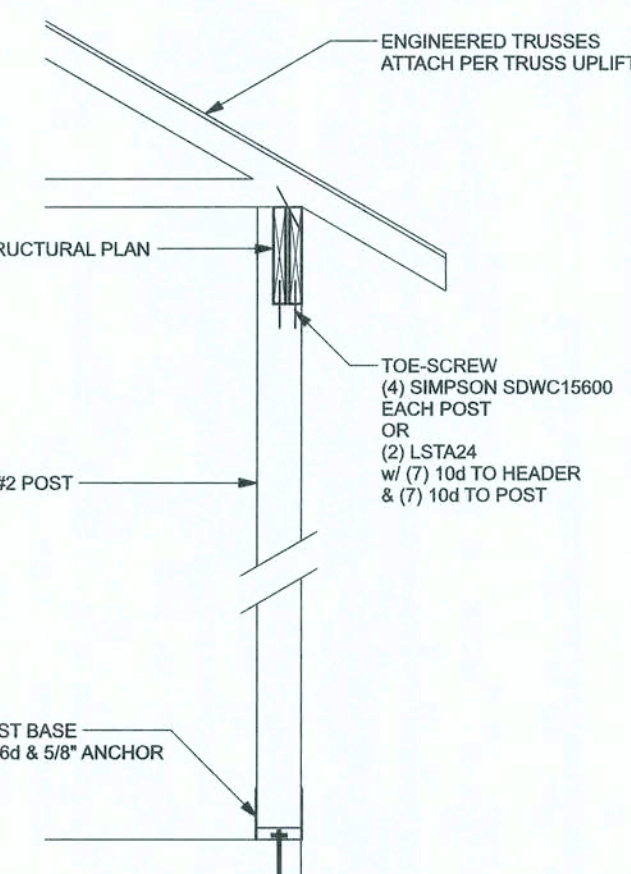
**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME



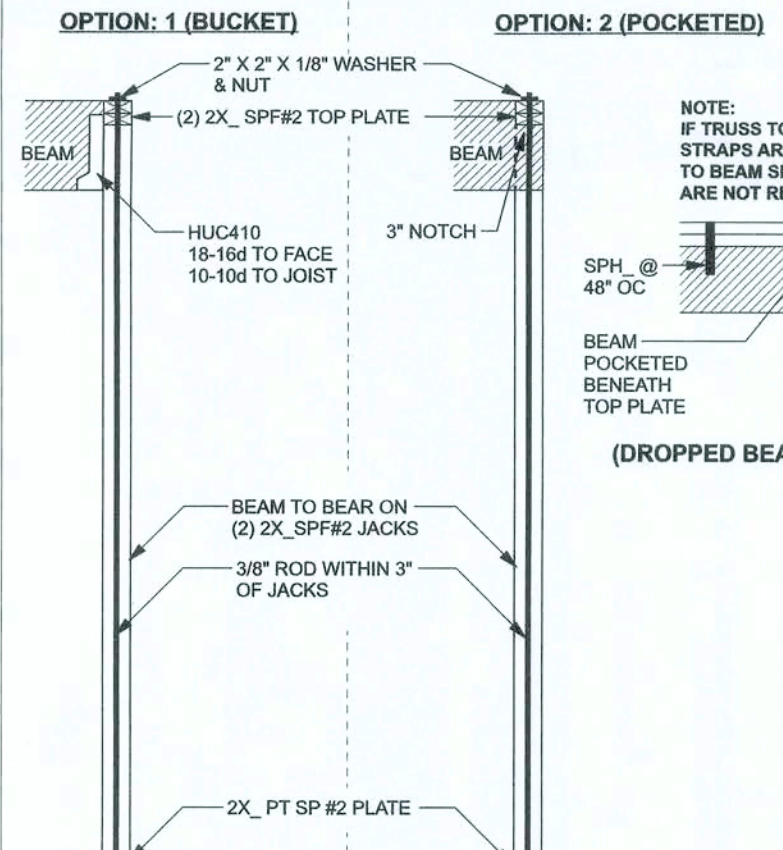
**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME



**(TYP.) INTERIOR BEARING WALL**  
ONE STORY WOOD FRAME w/ RODS



**(TYP.) PORCH POST**  
ONE STORY WOOD



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ RODS

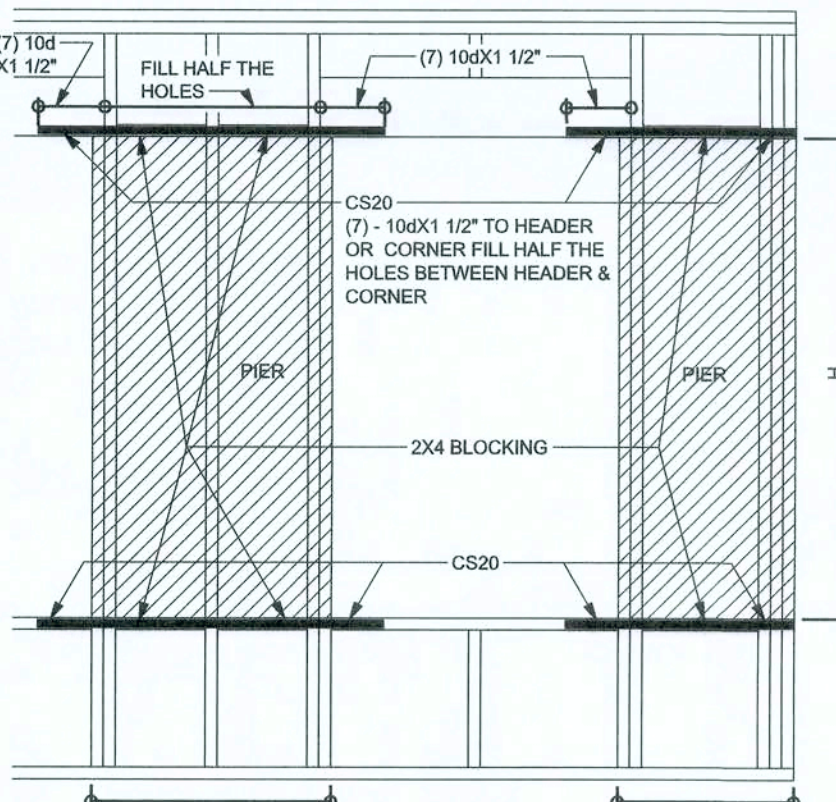
CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
415	495	SDWC15600	-	-	
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"	
575	495	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"	
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-30	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MFTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1065	955	SP2	8-10d	8-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
985	535	SP1	6-10d	4-10d	
1065	955	SP2	8-10d	8-10d	
771	171	LSTA24	15-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stairwell	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 14"x1 1/2"	12"x12" Titen HD	
4235	3640	HTT4	18-16dX2 1/2"	12"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 14"x1 1/2"	12"x12" Titen HD	
4235	3640	HTT4	18-16dX2 1/2"	12"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stairwell	To Post	Anchor	
2200		ABJ44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABJ66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABJ44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABJ66	12-16d	5/8"x12" Drill & Epoxy	

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2\"/>

(1) 2x4 @ 16\"/>
------------------

GRADE & SPECIES TABLE		
	Fb	E
2x8	SP #2	925 / 1.4
2x10	SP #2	800 / 1.4
2x12	SP #2	750 / 1.4
GLB	24F-V3 SP	2600 / 1.9
LSL	TIMBERSTRAND	1700 / 1.7
LVL	MICROLAM	2950 / 2.0
PSL	PARALAM	2900 / 2.0



NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BEHIND THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT  $h/w \leq 1.5$  WHERE  $h$  IS THE PIER HEIGHT FOR WINDOWS NOT GREATER THAN 7' HEIGHT OR 3' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

**OPENING FORCE TRANSFER**  
WOOD FRAME

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERS SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x8 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F'_c = 2500$  PSI. WELDED WIRE REINFORCED SLAB: 8\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/OUT REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPLICES 40\"/>

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE, AND BACKFILL, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

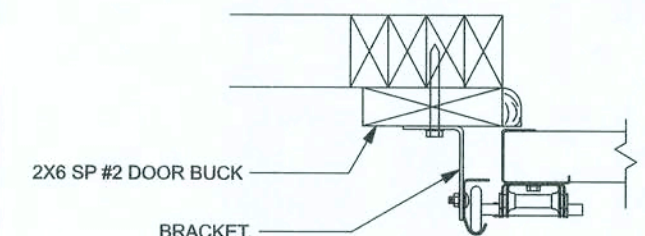
## DESIGN CRITERIA & LOADS:

BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 33.8.1)	130 MPH
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	C
ENCLOSURE CLASSIFICATION (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	36 FT
<b>C&amp;C DESIGN PRESSURES</b> SEE TABLE	
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR $< 4:12$	20 PSF LIVE LOAD
4:12 TO $< 12:12$	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b> 1500 PSF	
<b>FLOOD ZONE</b> THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (A1)			
EFFECTIVE WIND AREA (F2)	INTERIOR	ZONE 4 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+22.6 -46.2	+42.6 -57	

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (A1D)			
9x7 GARAGE DOOR	+22.6 -25.5		
16x7 GARAGE DOOR	+21.7 -24.1		

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME



## 2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\"/>

DOOR WIDTH	38\"/>
------------	--------

Ciblatrar Contracting, LLC

2145 Model - The Hamrick's

PROJECT ADDRESS:  
Loc 27 SW Mandella Drive  
Lauderhill, FL 33304

DIMENSIONS: Stalled dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915



Friday, October 25, 2019

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Lake City, Florida 32025  
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disowaydesign@gmail.com

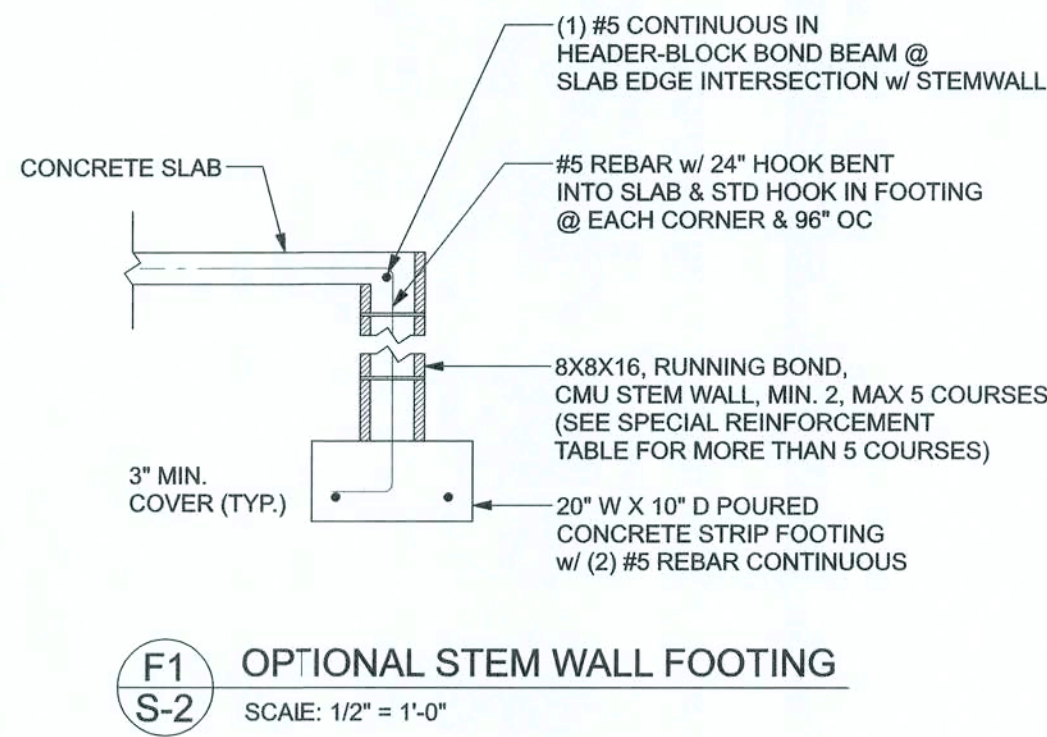
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191184

S-1

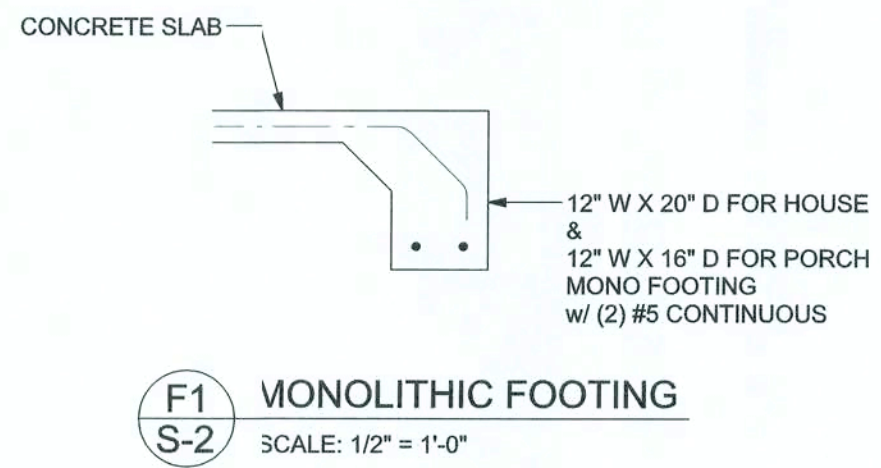
OF 3 SHEETS





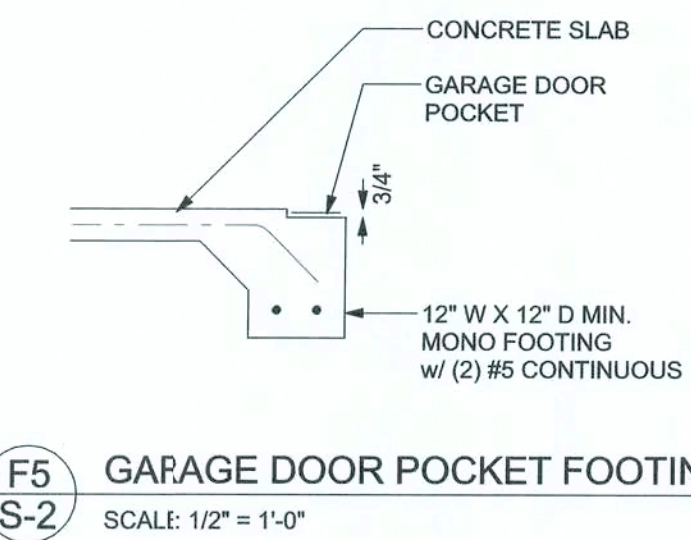
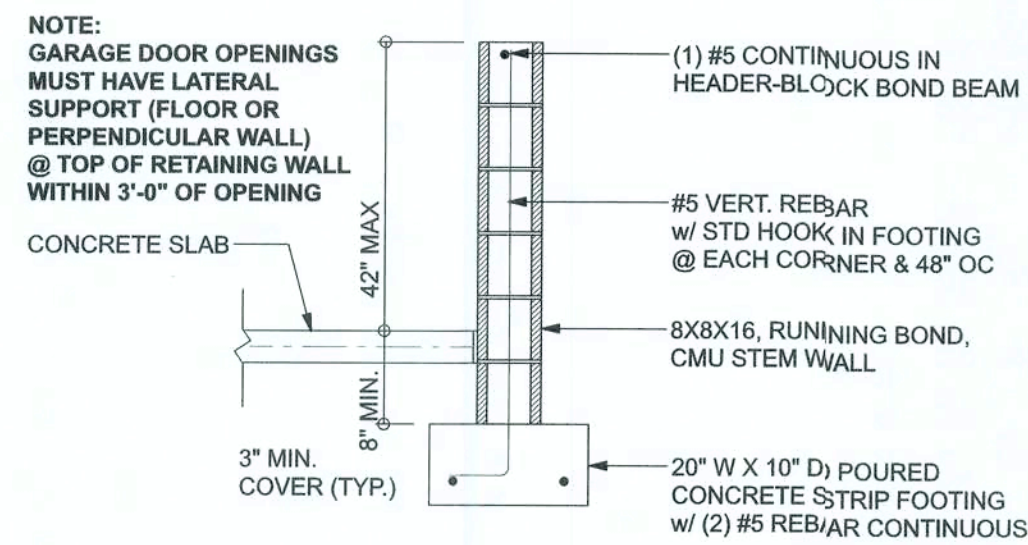
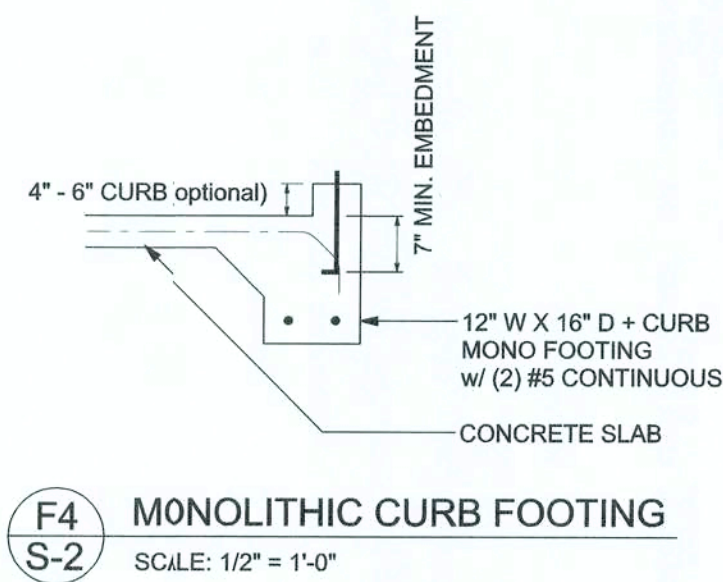
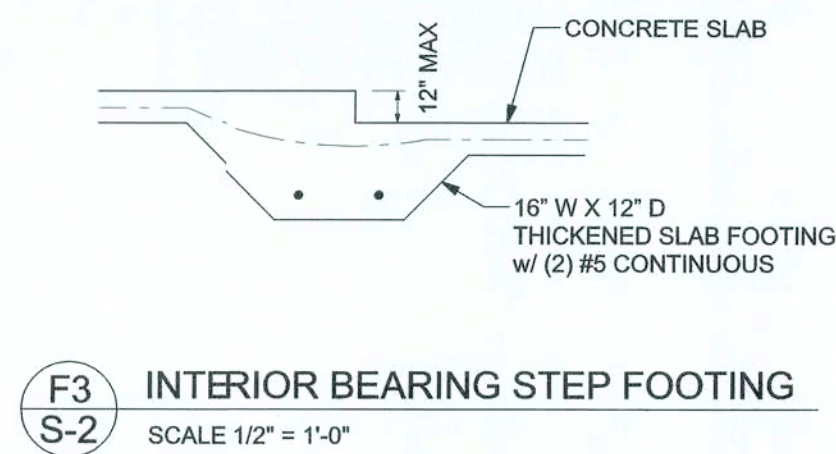
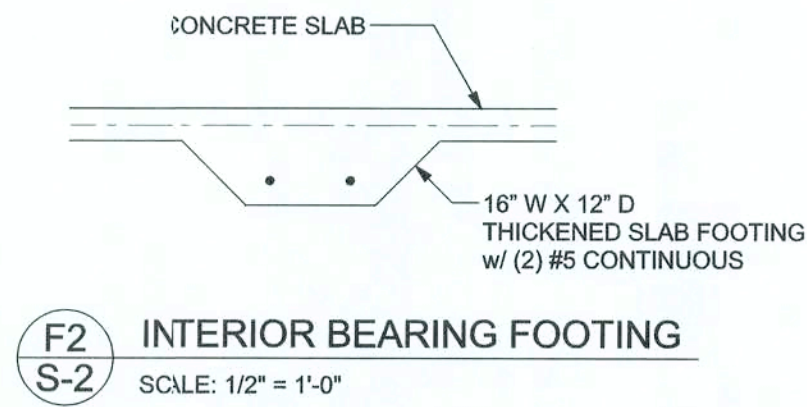
**TALL STEM WALL TABLE:**  
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diaphragm ladder reinforcement at 16" OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI/ASCE/TMS Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints



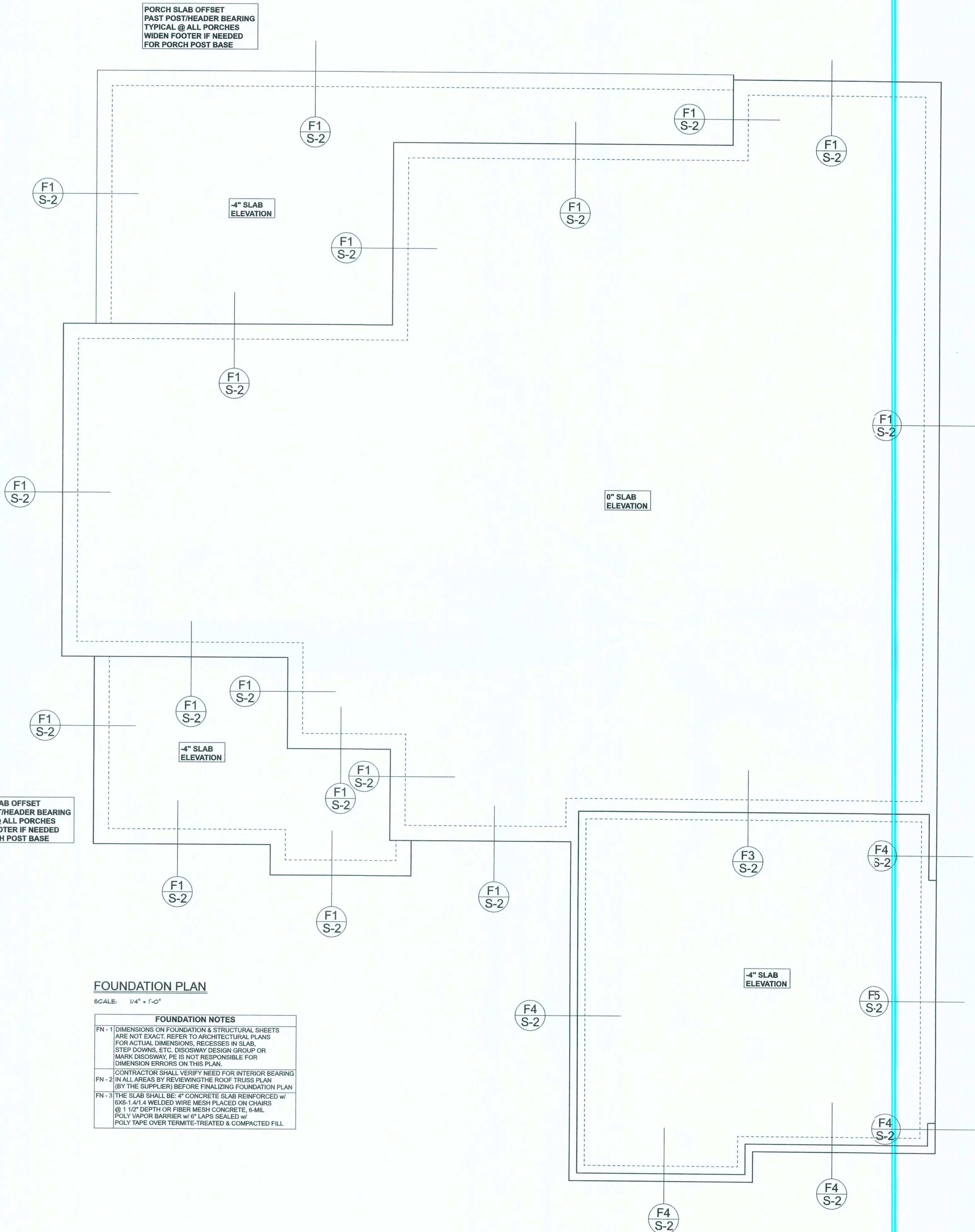
PORCH SLAB OFFSET PAST POST/HEADER BEARING TYPICAL @ ALL PORCHES WIDEN FOOTER IF NEEDED FOR PORCH POST BASE

#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

- 1) DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- 2) CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- 3) THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-14#4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL.



Gibraltar Contracting, LLC

2145 Model - The Hamrick's

PROJECT ADDRESS:  
Lot 27, SW Mandala Drive  
Lake City, FL 32024

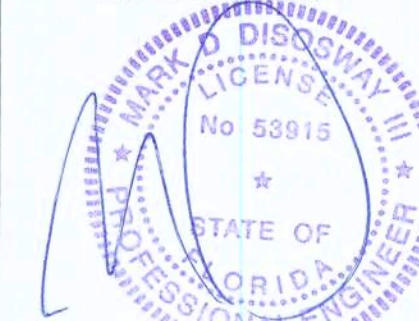
**DIMENSIONS:**  
Slated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Friday, October 25, 2019

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disoswaydesign@gmail.com

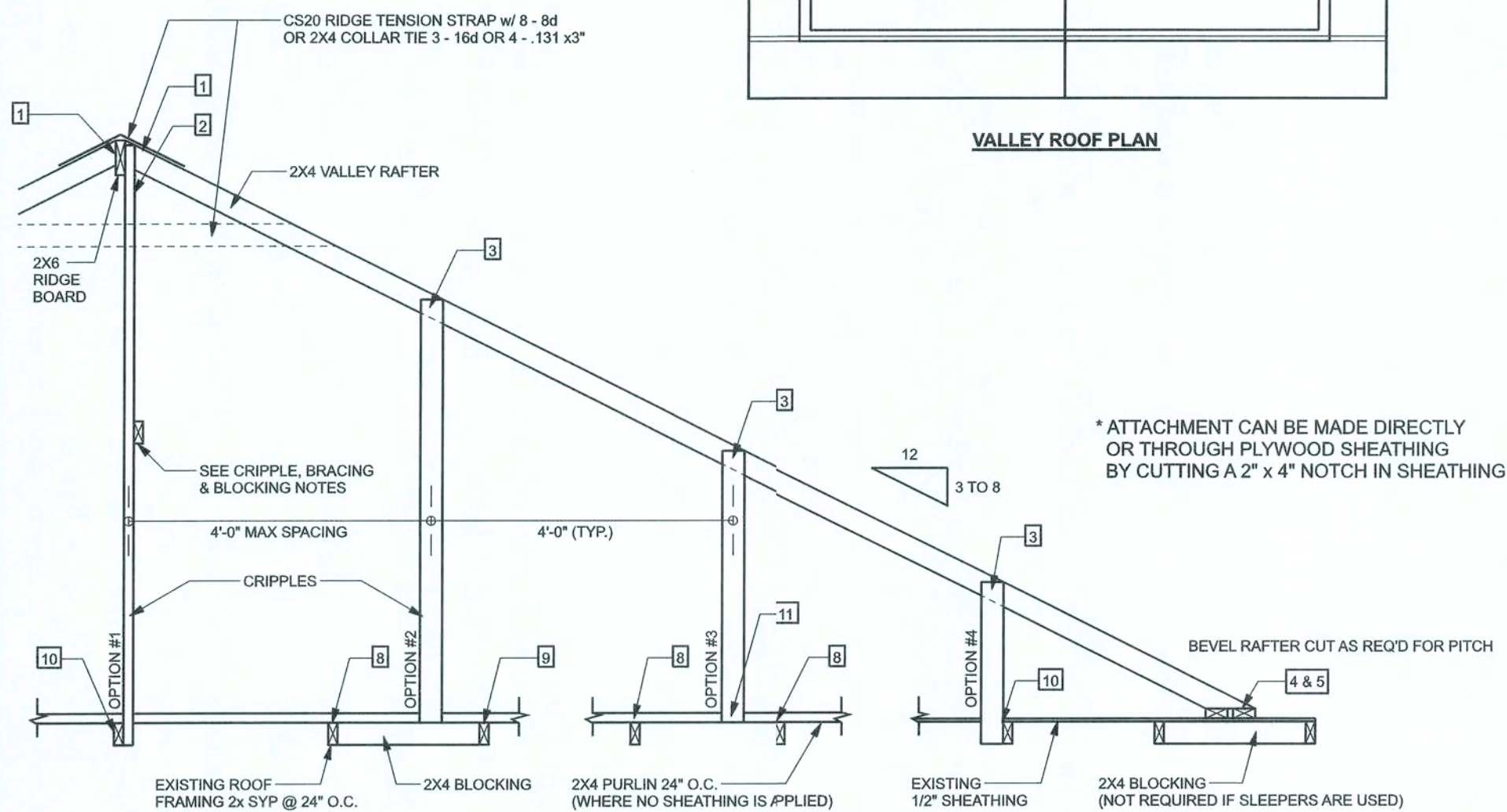
JOB NUMBER:  
191184

**S-2**  
OF 3 SHEETS



# LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACINGS	2X4 SPF #2
SLEEPERS	2X4 WIDTH OF RAFTER (SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

## VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

## CONNECTION REQUIREMENT NOTES

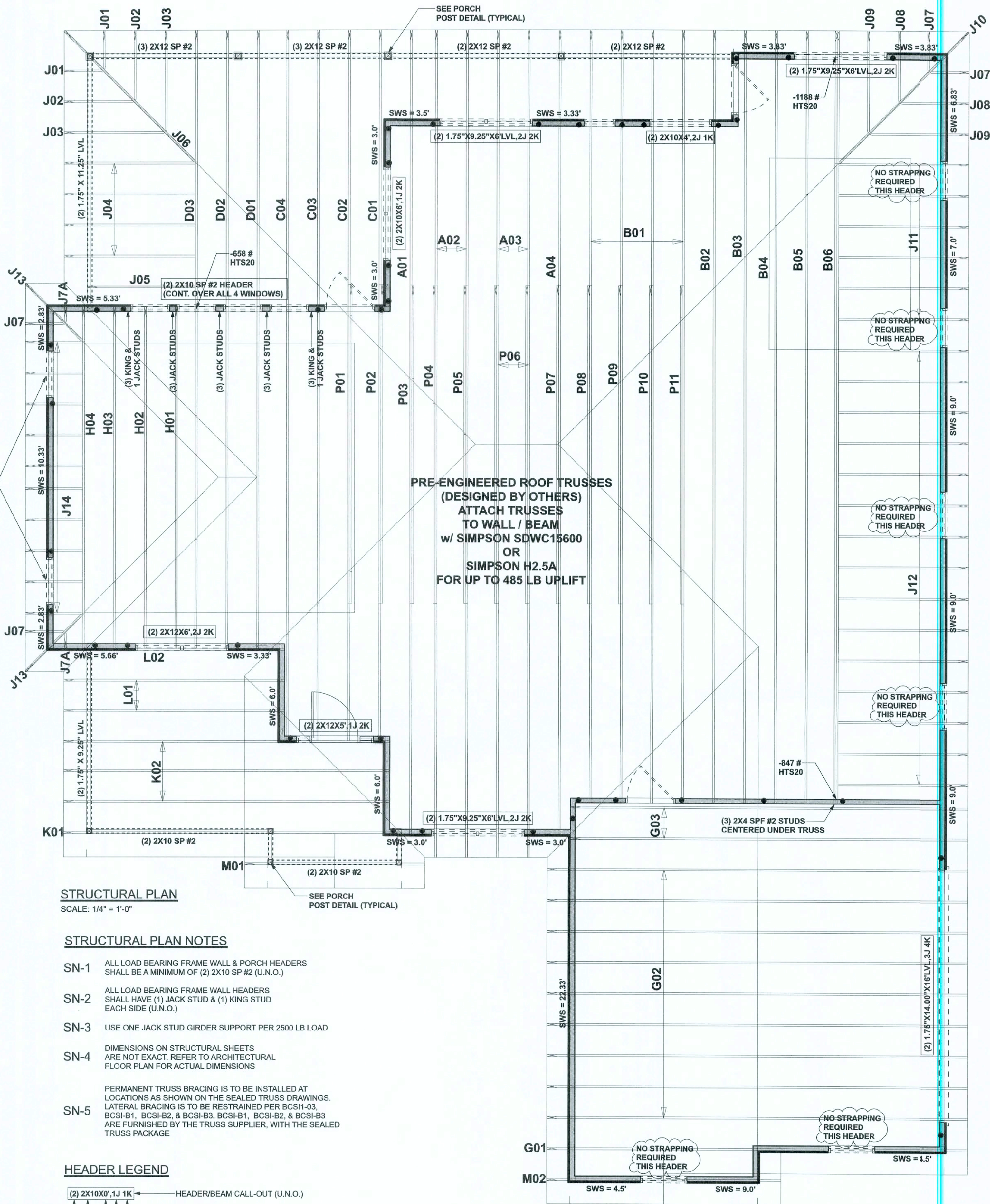
1. 2X4 RAFTERS TO RIDGE	3-16d OR 6 - 13d x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6 - 13d x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	3-16d OR 6 - 13d x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12 - 13d x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 8 - 13d x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO ROOF BLOCK	3-16d OR 6 - 13d x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	3-16d OR 6 - 13d x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP.)	3-16d OR 6 - 13d x 3" NAILS
9. PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8 - 13d x 3" NAILS
10. TRUSS TO BLOCKING	3-16d OR 6 - 13d x 3" END NAILS
11. CRIPPLE TO TRUSS	3-16d OR 6 - 13d x 3" FACE NAILS
12. CRIPPLE TO PURLIN	3-16d OR 6 - 13d x 3" FACE NAILS

## GENERAL NOTES

- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 7'-0" FOR 2X6 SPF #2 OR SYP #2.
- MAXIMUM ROOF AREA PER SUPPORT: 1802 IN ZONES 2 & 3, 2402 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1802 OR 2'-0" X 6'-0" SPAN = 1802).
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
- IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
  - SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
  - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
  - MAXIMUM WIND SPEED: 130 MPH
  - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
  - MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS
  - EXPOSURE CATEGORY "C", 1 = 1.0, K<sub>z</sub> = 1.0
  - ENCLOSED BUILDING

## CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W/ 2 - 16d NAILS OR 2X4 "T" OR SCAB BRACE NAILED TO PLAT EDGE OF CRIPPLE WITH 8d NAILS @ 9" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER.
- AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



## STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## HEADER LEGEND

- (2) 2X10X0', 1.1 1K - HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

## THREADED ROD LEGEND

- INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

## ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	23316 LBF	18658 LBF
REQUIRED	15084 LBF	16367 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND TRUSS CO. JOB #19-3641

Gibraltar Contracting, LLC

2145 Model - The Hamrick's

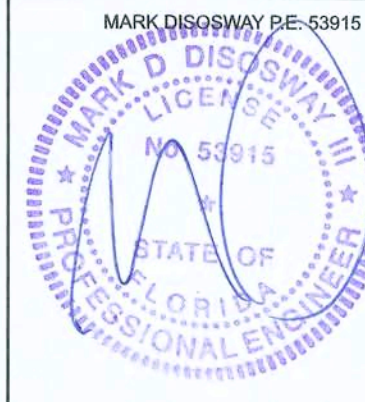
PROJECT ADDRESS:  
Lakewood Shores  
Lake City, FL 32024

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 2017 Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Friday, October 25, 2019

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Lake City, Florida 32025  
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JOB NUMBER:

191184

S-3

OF 3 SHEETS