DATE 10/06/2010

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000028919

APPLICANT VINCE RICHARDSON PHONE 755-5779

ADDRESS 692 SW ARLINGTON BLVD LAKE CITY FL 32025

OWNER MARY ELLEN HODSON STRINGFELLOW PHONE 623-0763

ADDRESS 233 SW AIRPARK GLEN LAKE CITY FL 32025

CONTRACTOR VINCE RICHARDSON PHONE 755-5779

LOCATION OF PROPERTY 90W, TL ON SISTERS WELCOME RD, TL ON AIRPARK GLEN,
1ST HOUSE ON LEFT PAST RUNWAY & BEHIND OFFICE

TYPE DEVELOPMENT POOL ENCLOSURE ESTIMATED COST OF CONSTRUCTION 8800.00

HEATED FLOOR AREA _____ TOTAL AREA _____ HEIGHT 12.00 STORIES 1

FOUNDATION _____ WALLS SCREEN ROOF PITCH _____ FLOOR _____

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 1 FLOOD ZONE NA DEVELOPMENT PERMIT NO. _____

PARCEL ID 12-4S-16-02947-026 SUBDIVISION CANNON CREEK AIRPARK

LOT 25/26 BLOCK _____ PHASE _____ UNIT 0 TOTAL ACRES _____

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		110	X Vince Richardson

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	X10-356	BK	TC	N

COMMENTS: NOC ON FILE

Check # or Cash 2220

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 45.00 CERTIFICATION FEE $ 0.00 SURCHARGE FEE $ 0.00

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ _____ CULVERT FEE $ _____ **TOTAL FEE** 95.00

INSPECTORS OFFICE L. Hodges CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

NOTICE OF COMMENCEMENT

PERMIT NUMBER: _____

STATE OF: FLORIDA COUNTY OF: Columbia CITY OF: Lake City

THE UNDERSIGNED HEREBY gives notice that improvement(s) will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

DESCRIPTION OF PROPERTY

LOT: _____ BLOCK: _____ SECTION: _____ TOWNSHIP: _____ RANGE: _____

TAX PARCEL NUMBER: 12-4S-16-02947

SUBDIVISION: Cannon Creek Air Park PLATBOOK: _____ MAP PAGE: _____

STREET ADDRESS: 233 SW AirparK GlN Lake City, Fla. 32025

GENERAL DESCRIPTION OF IMPROVEMENTS

TO CONSTRUCT: Pool Enclosure

OWNER INFORMATION

OWNER NAME: Mary Ellen Hodson Stringfellow

ADDRESS: 233 SW Airpark GlN PHONE NUMBER: 386-623-0763

CITY: Lake City STATE: Fl. ZIP CODE: 32025

INTEREST IN PROPERTY: _____

FEE SIMPLE TITLEHOLDER NAME: _____

FEE SIMPLE TITLEHOLDER ADDRESS: _____

(if other than owner)

Inst 201012016153 Date 10/6/2010 Time 2:19 PM
Rec DC,P DeWitt Cason,Columbia County Page 1 of 1 B:1202 P:1660

CONTRACTOR NAME: Vince Richardson / Richardson Aluminum LLC.

ADDRESS: 692 SW Arlington Blvd PHONE NUMBER: 386-755-5779

CITY: Lake City STATE: Fla ZIP CODE: 32025

BONDING COMPANY: _____

ADDRESS: _____ PHONE NUMBER: _____

CITY: _____ STATE: _____ ZIP CODE: _____

LENDER NAME: _____

ADDRESS: _____ PHONE NUMBER: _____

CITY: _____ STATE: _____ ZIP CODE: _____

Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a) 7., Florida Statutes:

NAME: _____ ADDRESS: _____

In addition to himself, Owner designates _____

of _____ to receive a copy of Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Expiration date is one (1) year from date of recording unless a different date is specified.

SIGNATURE OF OWNER: _____

SWORN to and subscribed before me this 20 day of Sept, A.D. 2004.

Notary Public: _____

My commission Expires:

NOTARY PUBLIC STATE OF FLORIDA
SANDRA H. TILLOTSON
MY COMMISSION # DD 629530
EXPIRES: January 31, 2011
Bonded Thru Budget Notary Services

ck# 2220

Columbia County Building Permit Application APPL FEE ☑

For Office Use Only Application # 1009-43 Date Received 9/21 By JW Permit # 28919

Zoning Official BLK Date 28.09.10 Flood Zone N/A Land Use RES. Low Den. Zoning RSF-2

FEMA Map # N/A Elevation N/A MFE N/A River N/A Plans Examiner T.C. Date 9-23-10

Comments

☐ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ ~~State~~ Road Info ☐ ~~Parent~~ Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code

School = TOTAL Ø Accessory Structure

Septic Permit No. V-10-356 IN Z/E BOX. Fax

Name Authorized Person Signing Permit Vince Richardson Phone 386-755-5779

Address 692 S.W Arlington Blvd. Lake City, Fl. 32025

Owners Name Mary Ellen Hodson Stringfellow Phone 386-623-0763

911 Address 233 SW Airpark GLN. Lake City, Fl 32025

Contractors Name Vince Richardson / Richardson Aluminum L.L.C. Phone 386-755-5779

Address 692 SW Arlington Blvd. Lake City, Fl. 32025

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address LAWRENCE BENNETT, PE 315 HAMPTON ST. A. OXFORD, AL 32129

Mortgage Lenders Name & Address CASH.

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 12-4S-16-02947-026 Estimated Cost of Construction 8,800.00/xx

Subdivision Name Cannon Creek Air Park Lot 25/26 Block Unit Phase

Driving Directions Sisters Welcome Rd south From Hwy 90. Turn L @ Airpark GLN. 1st house on Left behind office.

Number of Existing Dwellings on Property 1

Construction of Pool Enclosure Total Acreage 1.239 Lot Size 209'x280'

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive) Total Building Height 12Ft

Actual Distance of Structure from Property Lines - Front 60' Side 45' Side 105' Rear 199

Number of Stories Heated Floor Area Total Floor Area Roof Pitch

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

"JW spoke w/ Owd: 9.28.10"

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Vinca Richardson

Contractor's Signature (Permitee)

Contractor's License Number _____
Columbia County
Competency Card Number 000110

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 20 **day of** Sept **20** 10.

Personally known ✓ **or Produced Identification** _____

Sandra H. Tillotson

State of Florida Notary Signature (For the Contractor)

SEAL:

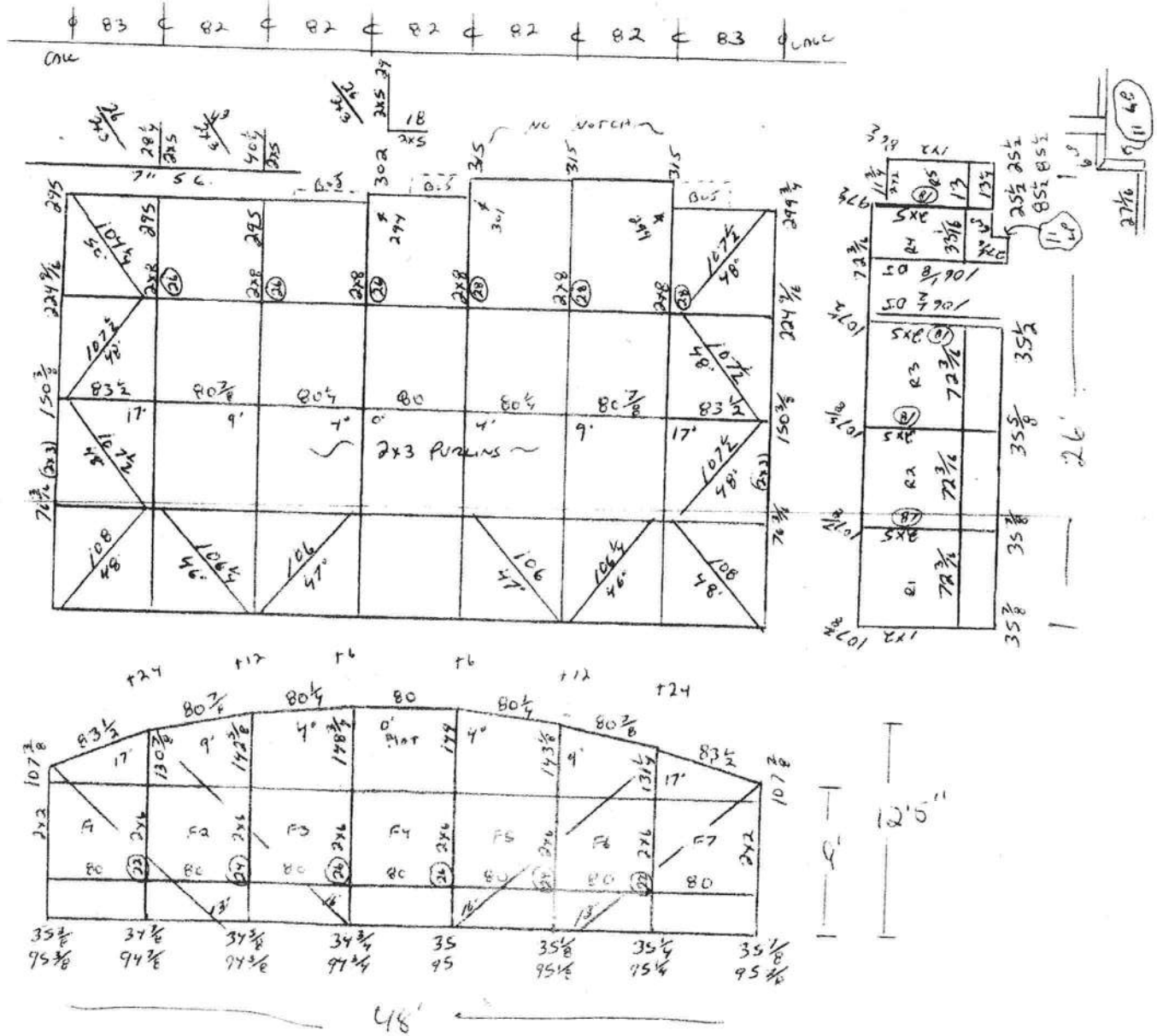
NOTARY PUBLIC STATE OF FLORIDA
SANDRA H. TILLOTSON
MY COMMISSION # DD 629530
EXPIRES: January 31, 2011
Bonded Thru Budget Notary Services

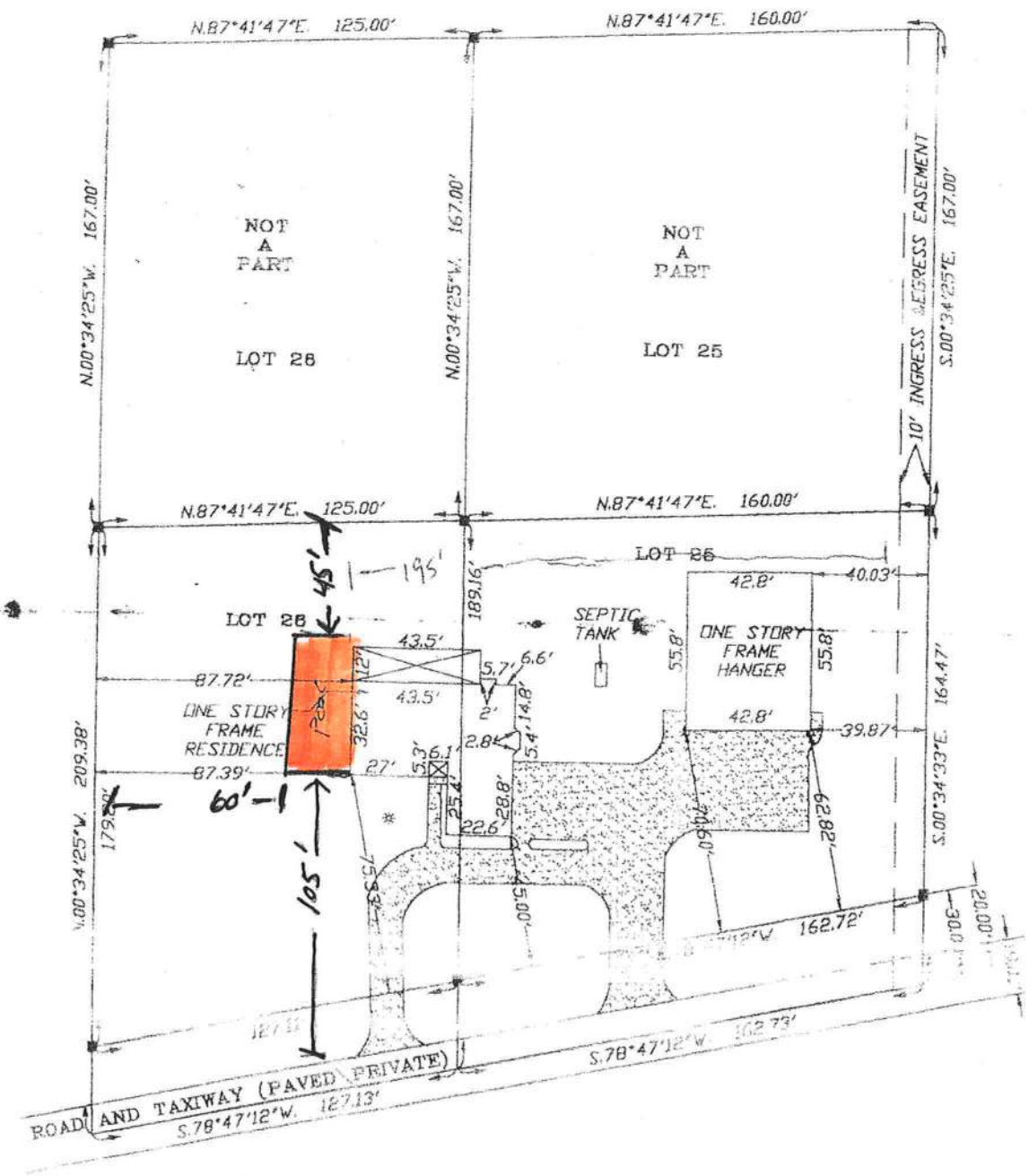
String fellow

MAIL LIST

6 pcs 2x8 38'
6 pcs 2x8 26'
2 pcs 2x6 26'
2 pcs 2x6 24'
2 pcs 2x6 22'
8 pcs 2x5 18'
1 pc 2x5 20'
(Gutter ups & Leaders)
15 - 2x3 30'
3 - 2x2 30'
4 - 2x2 24'
5 - 1x2 30'
5 - 1x2 24'
1 - D. Ramp

1 - Door
1 - D. Spout 10'
1 - A - Elbow
1 - 7" S.G. 17'
2 - Cables 13'
2 - Cables 16'





SCALE: 1" = 60'

DESCRIPTION:
LOTS 25 AND 26 OF CANNON CREEK AIRPARK, A SUBD.
THEREOF RECORDED IN PLAT BOOK 5, PAGE 38 OF THE
COUNTY, FLORIDA. LESS AND EXCEPT THE NORTH 167
EASEMENT OVER AND ACROSS THE SOUTH 30 FEET AN
TOGETHER WITH THE RIGHTS OF INGRESS AND EGRESS
OF LOT 25 AND THE WEST 50 FEET OF LOT 24, COLU

2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
3. THIS PARCEL IS IN ZONE X AND IS DETERMINED
PLAIN AS PER FLOOD RATE MAP, DATED 6 JANUAR
120070 0175 B. HOWEVER, THE FLOOD INSURANCE
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS S
DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS
THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENE
POLICY.

Existing Permit for
Addition 25537

Columbia County Property Appraiser

DB Last Updated: 8/5/2010

2009 Tax Roll Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator

Parcel: 12-4S-16-02947-026

<< Next Lower Parcel | Next Higher Parcel >> | Interactive GIS Map | Print

<< Prev Search Result: 2 of 2

Owner & Property Info

Owner's Name	STRINGFELLOW DONALD R &		
Mailing Address	MARY ELLEN HODSON STRINGFELLOW 233 SW AIRPARK GLN LAKE CITY, FL 32025		
Site Address	233 SW AIRPARK GLN		
Use Desc. (code)	SINGLE FAM (000100)		
Tax District	2 (County)	**Neighborhood**	12416
Land Area	1.239 ACRES	**Market Area**	06
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		
LOTS 25 & 26 EX N 167 FT CANNON CREEK AIR PARK S/D. ORB 644-335, 685-195, 702-619, 896-063,			

Property & Assessment Values

2009 Certified Values		
Mkt Land Value	cnt: (0)	$51,435.00
Ag Land Value	cnt: (1)	$0.00
Building Value	cnt: (1)	$174,801.00
XFOB Value	cnt: (4)	$31,931.00
Total Appraised Value		$258,167.00
Just Value		$258,167.00
Class Value		$0.00
Assessed Value		$212,906.00
Exempt Value	(code: HX)	$50,000.00
Total Taxable Value		Cnty: $162,906 Other: $162,906 | Schl: $187,906

2010 Working Values

NOTE:
2010 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
1/27/2000	896/63	WD	I	Q		$180,000.00
11/17/1989	702/619	WD	V	Q		$19,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	1991	AVERAGE (05)	2616	5610	$168,443.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

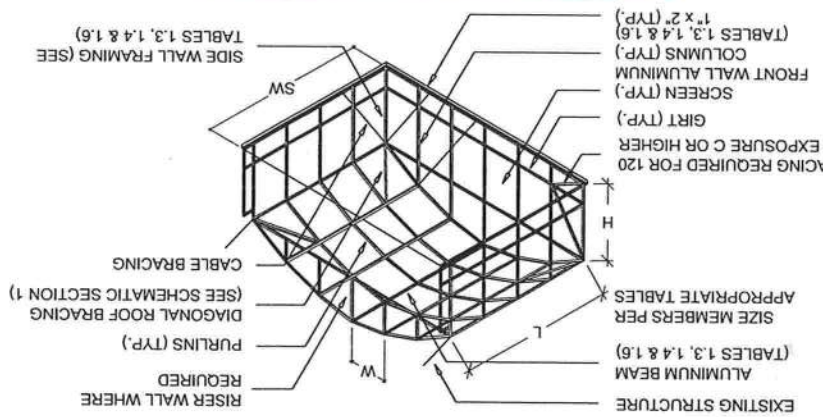
Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0190	FPLC PF	0	$1,200.00	0000001.000	0 x 0 x 0	(000.00)
0166	CONC,PAVMT	0	$4,130.00	0005900.000	0 x 0 x 0	PD (050.00)
0280	POOL R/CON	2007	$12,959.00	0000462.000	14 x 33 x 0	(000.00)
0282	POOL ENCL	2007	$9,547.00	0001248.000	26 x 48 x 0	(000.00)

RAISED SEAL COPIES REQUIRED FOR ENGINEERING TO BE VALID FOR PERMITTING

12-01-2009

TYPICAL DOME ROOF - ISOMETRIC

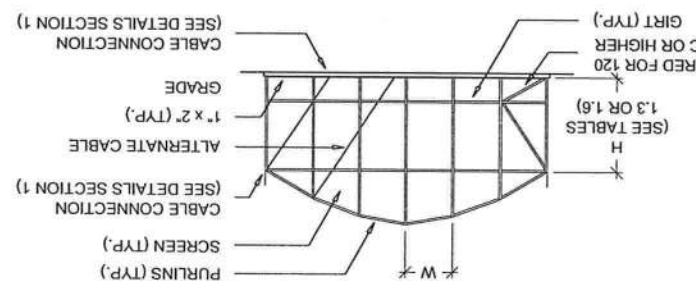
SCALE: N.T.S.



CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

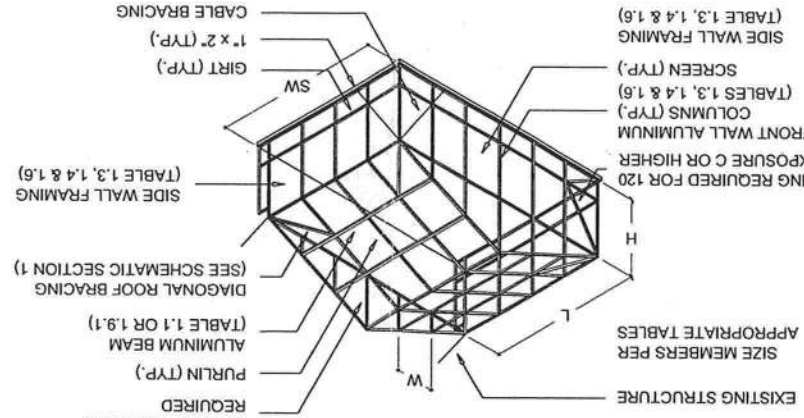
TYPICAL DOME ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL TRANSVERSE GABLE ROOF - ISOMETRIC

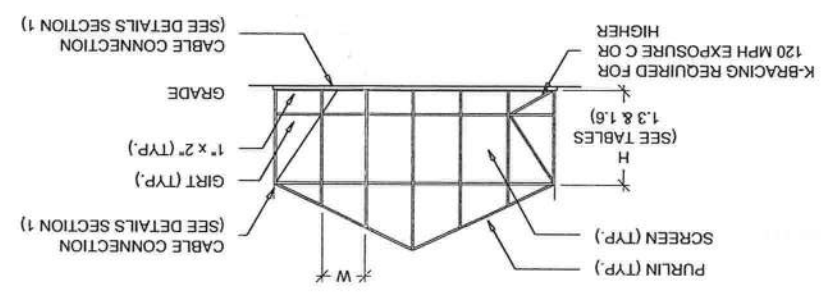
SCALE: N.T.S.



CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES

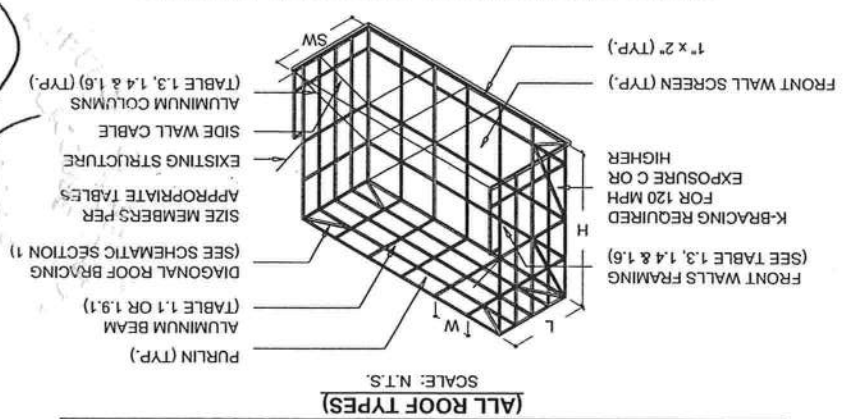
TYPICAL TRANSVERSE GABLE ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL TWO STORY POOL ENCLOSURE - ISOMETRIC (ALL ROOF TYPES)

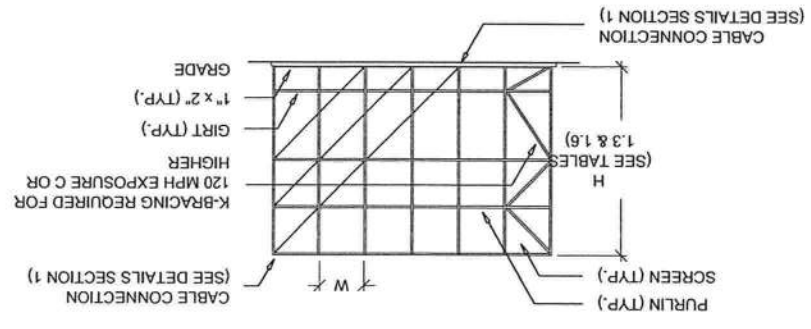
SCALE: N.T.S.



CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES

TYPICAL TWO STORY POOL ENCLOSURE - FRONT WALL ELEVATION (ALL ROOF TYPES)

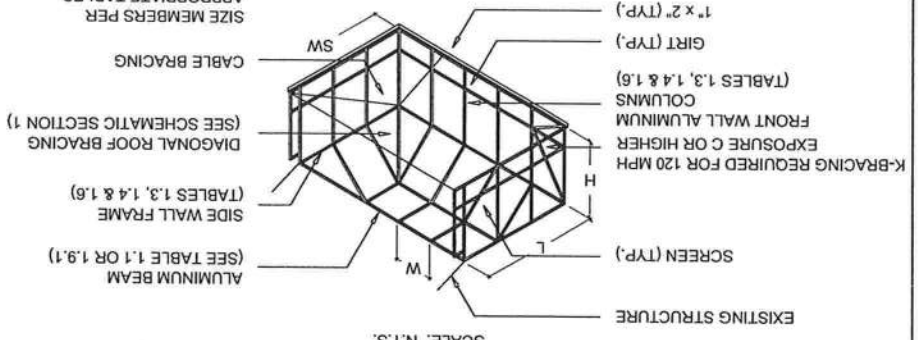
SCALE: N.T.S.



TYPICAL MANSARD ROOF - ISOMETRIC

SCALE: N.T.S.

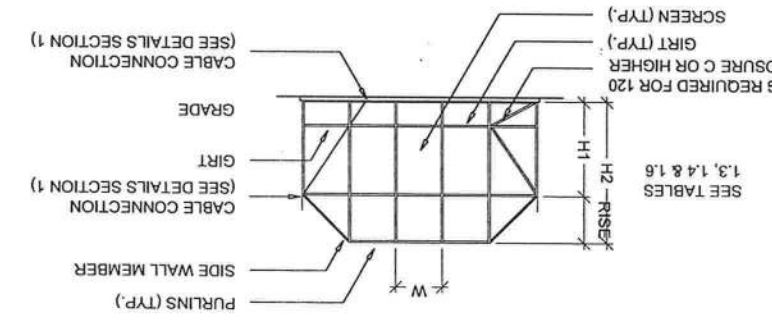
CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.



TYPICAL MANSARD ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.

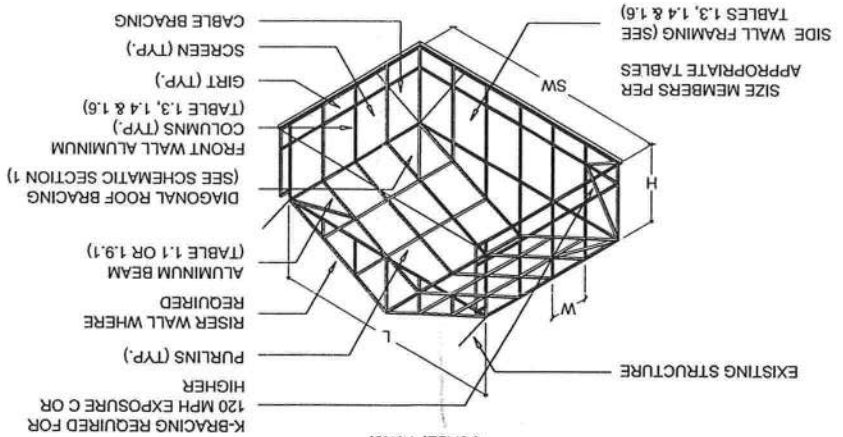
NOTE: USE H2 FOR CABLE AREA CALCULATION



TYPICAL GABLE ROOF - ISOMETRIC

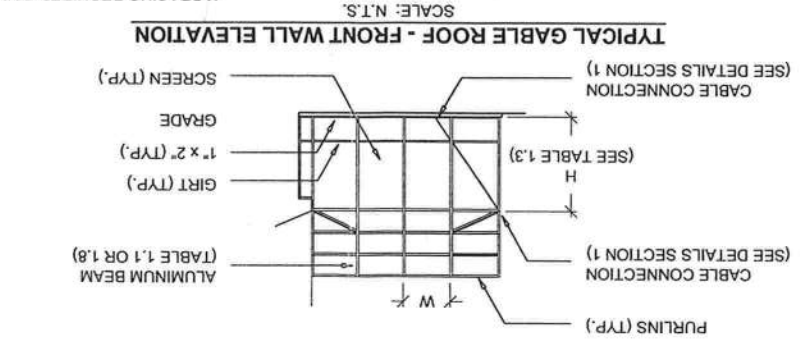
SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES



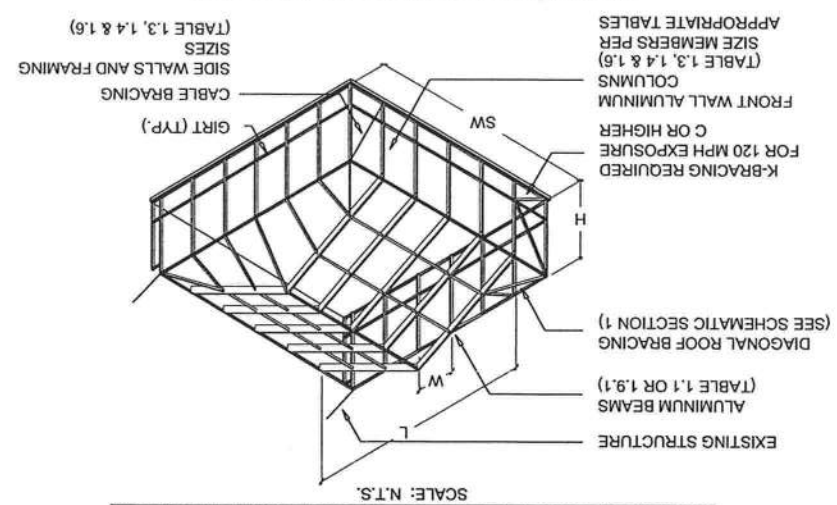
TYPICAL GABLE ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



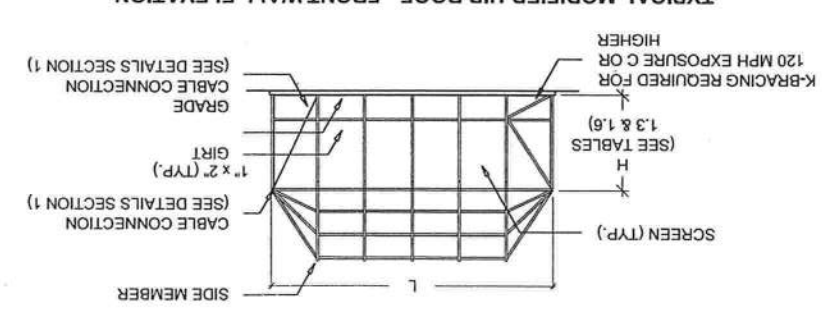
TYPICAL MODIFIED HIP ROOF - ISOMETRIC

SCALE: N.T.S.



TYPICAL MODIFIED HIP ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



ALUMINUM STRUCTURES DESIGN MANUAL
SCREEN ENCLOSURES
SECTION 1 DETAILS

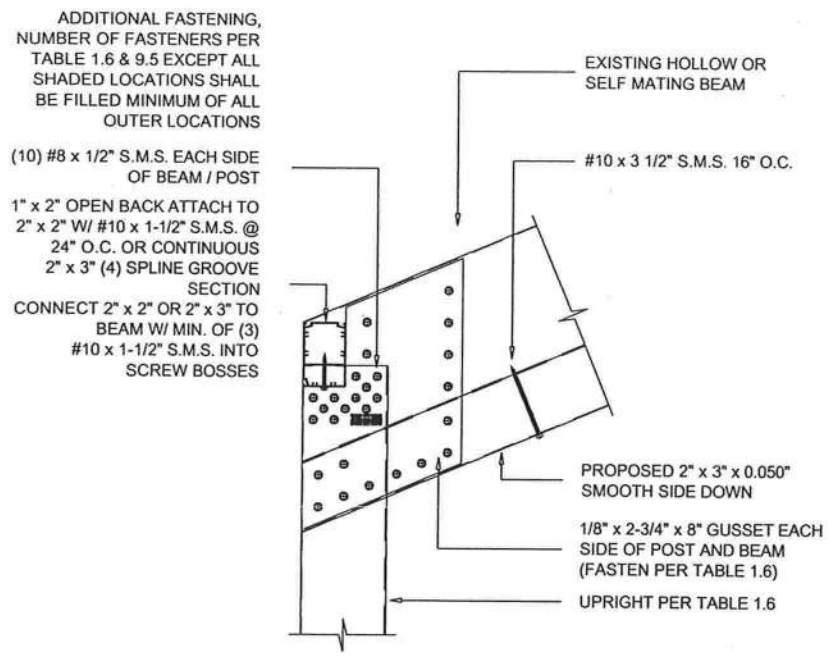
2007 FLORIDA BUILDING CODE
WITH 2009 SUPPLEMENTS - 2009 EDITION

Lawrence E. Bennett, P.E. FL # 16644
CIVIL & STRUCTURAL ENGINEERING
315 Herbert St., Port Orange, Fl 32129
Telephone #: (386) 767-4774 Fax #: (386) 767-6556
http://www.lebpe.com/

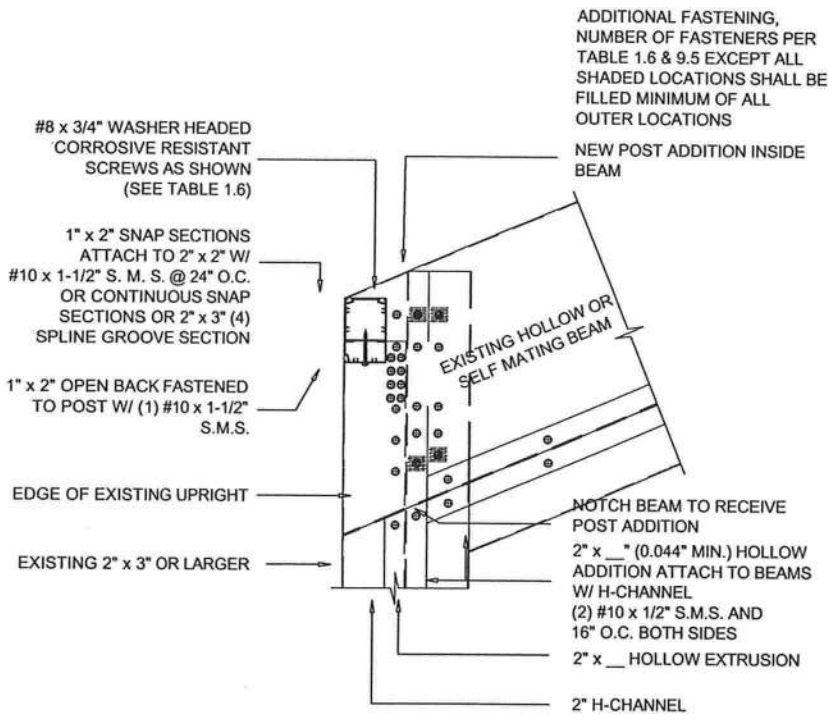
JAN 04 2010

SHEET 4 OF 21

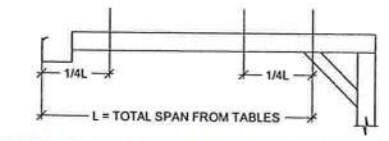
Town & Country
INDUSTRIES, INC.
Wholesale Aluminum Distributors
400 W. McNAB ROAD, FORT LAUDERDALE, FLORIDA 33309
1-800-432-6019
PHONE: (954) 970-0999
FAX: (954) 972-1338



ADDITION OF 2" x 3" TO EXISTING S.M.B.
SCALE: 2" = 1'-0"

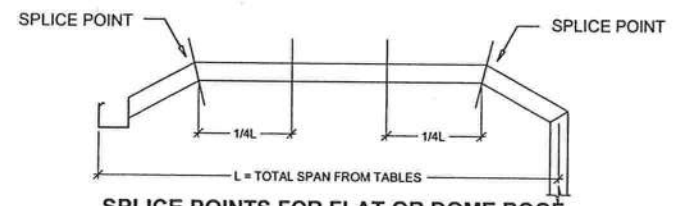


ALTERNATE POST / BEAM ADDITION OF 2" x __ " TO EXISTING 2" x __ "
SCALE: 2" = 1'-0"

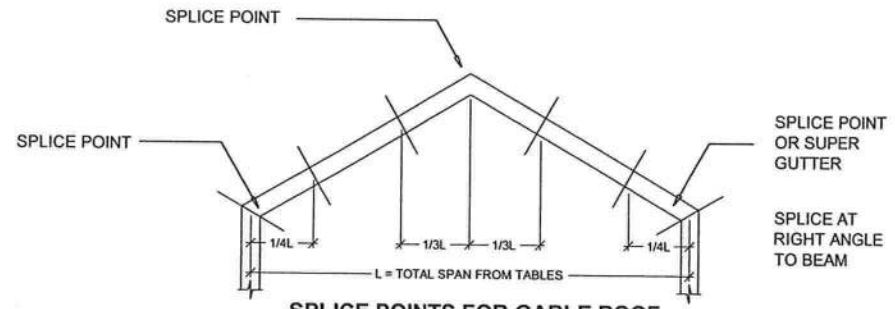


SPLICE POINTS FOR FLAT OR DOME ROOF
SCALE: N.T.S.

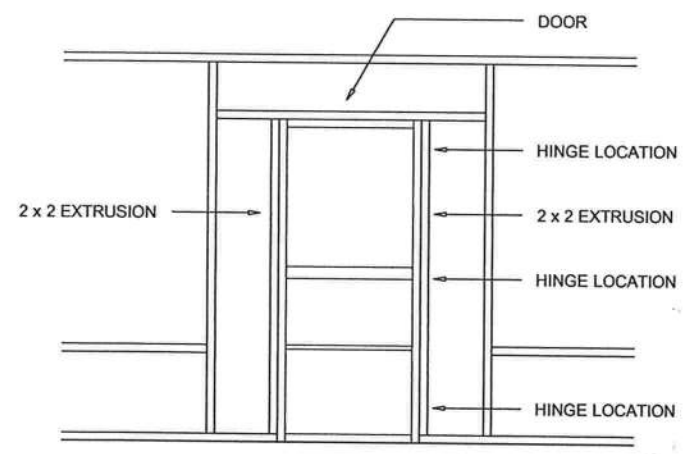
No Splices



SPLICE POINTS FOR FLAT OR DOME ROOF
SCALE: N.T.S.



SPLICE POINTS FOR GABLE ROOF
SCALE: N.T.S.



NOTES:

1. Door to be attached to structure with minimum two (2) hinges.
2. Each hinge to be attached to structure with minimum four (4) #12 x 3/4" S.M.S..
3. Each hinge to be attached to door with minimum three (3) #12 x 3/4" S.M.S..
4. Bottom hinge to be mounted between 10 inches and 20 inches from ground.
5. Top hinge to be mounted between 10 inches and 20 inches from top of door.
6. If door location is adjacent to upright a 1" x 2" x 0.044" may be fastened to upright with #12 x 1" S.M.S. at 12" on center and within 3" from end of upright.

TYPICAL SCREEN DOOR CONNECTION DETAIL
SCALE: N.T.S.

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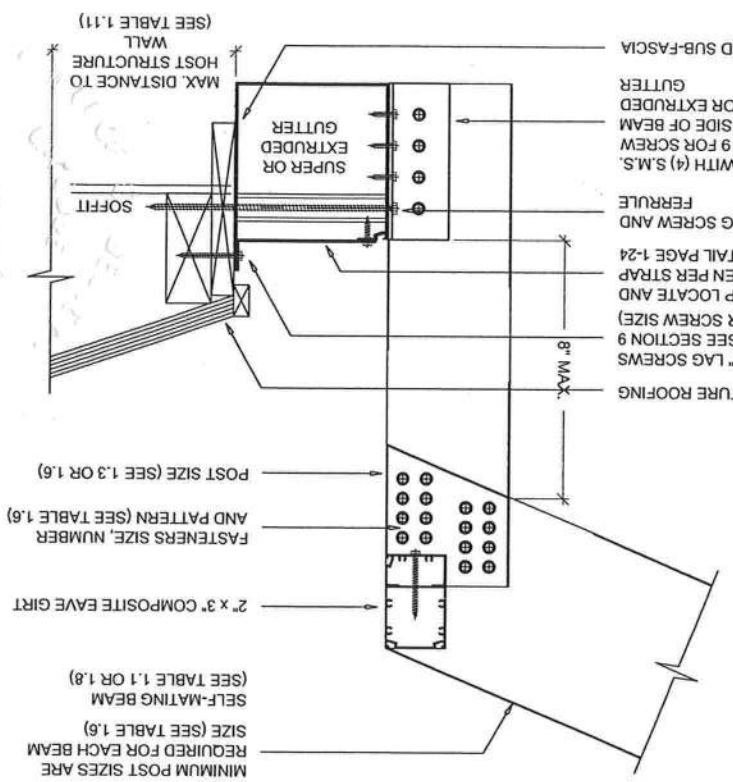
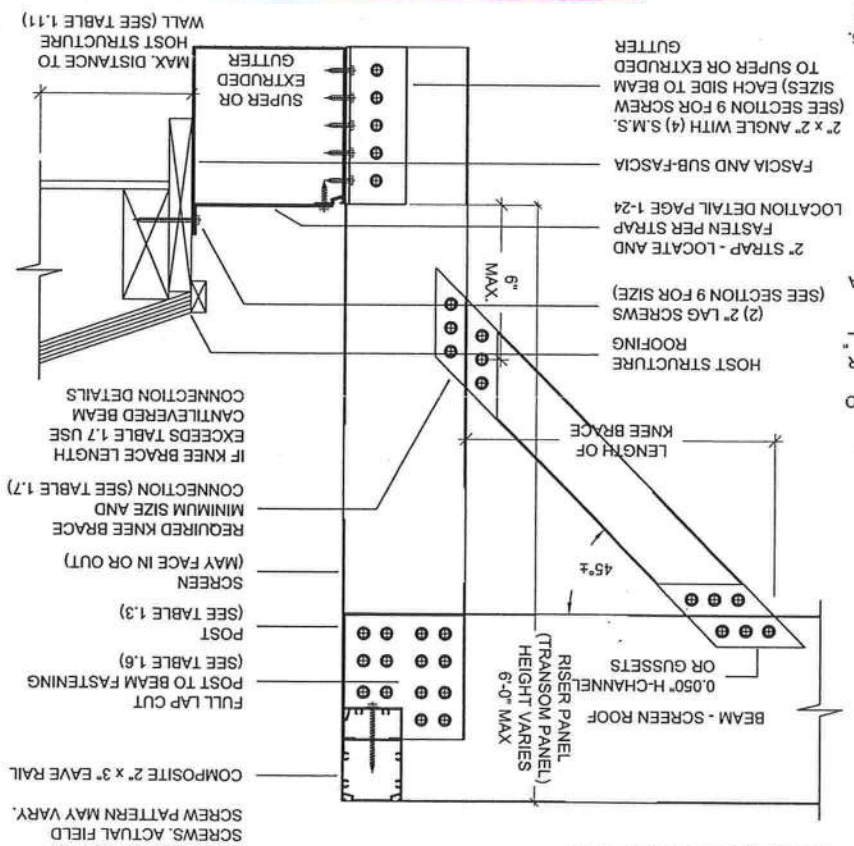
SUPER OR EXTRUDED GUTTER RISER (OR TRANSOM) WALL AT FASCIA - DETAIL 3

SCALE: 3" = 1'-0"

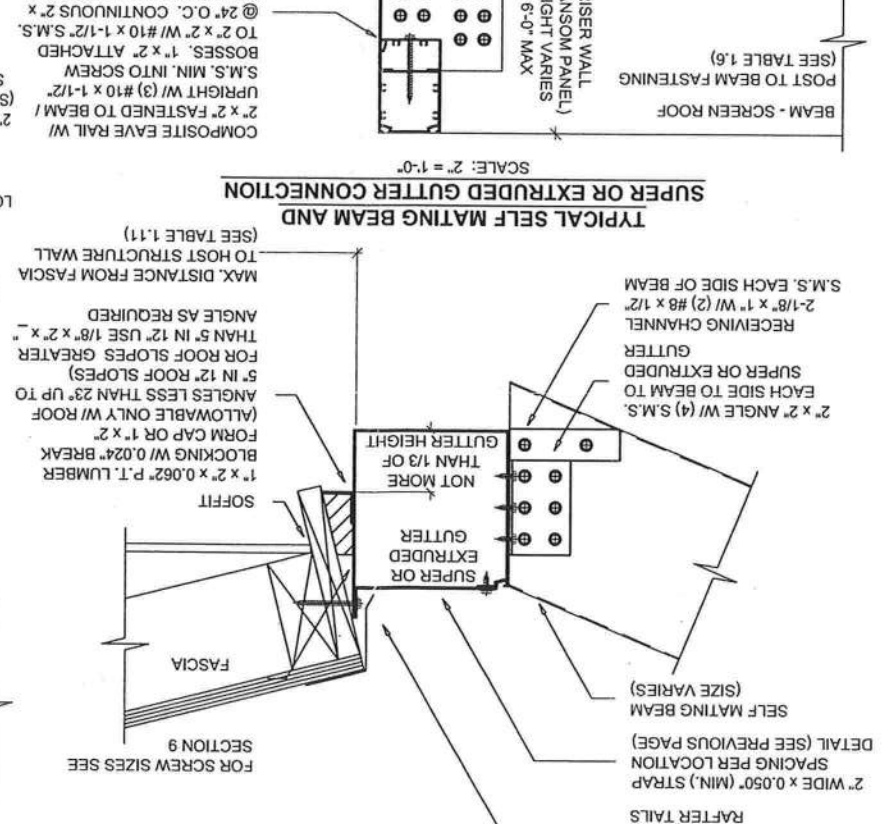


SUPER OR EXTRUDED GUTTER RISER (OR TRANSOM) WALL AT FASCIA - DETAIL 2

SCALE: 3" = 1'-0"



IF TRANSOM HEIGHT EXCEEDS 6'-0" USE CANTILEVER BRACE DETAIL



SUPER OR EXTRUDED GUTTER RISER (OR TRANSOM) WALL AT FASCIA - DETAIL 1

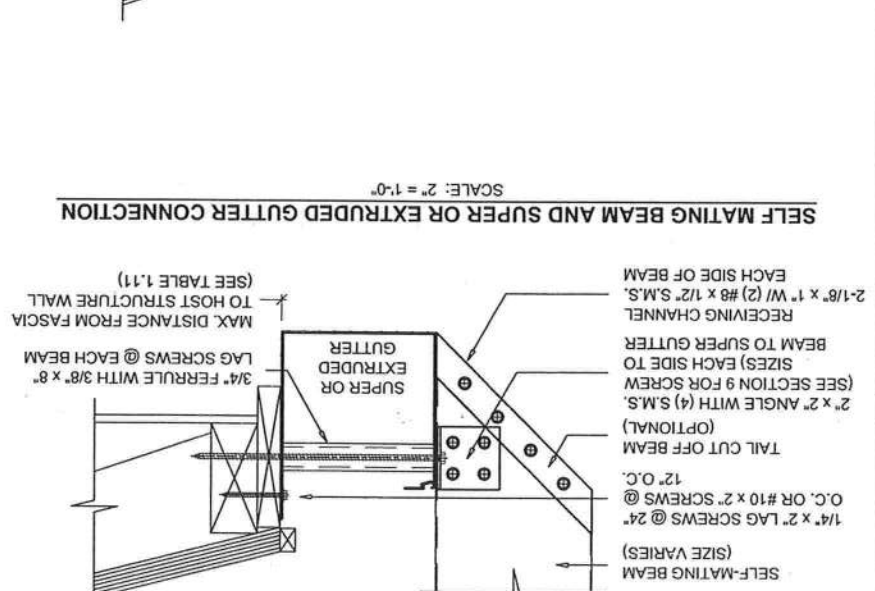
SCALE: 3" = 1'-0"

TYPICAL SELF MATING BEAM AND SUPER OR EXTRUDED GUTTER CONNECTION

SCALE: 2" = 1'-0"

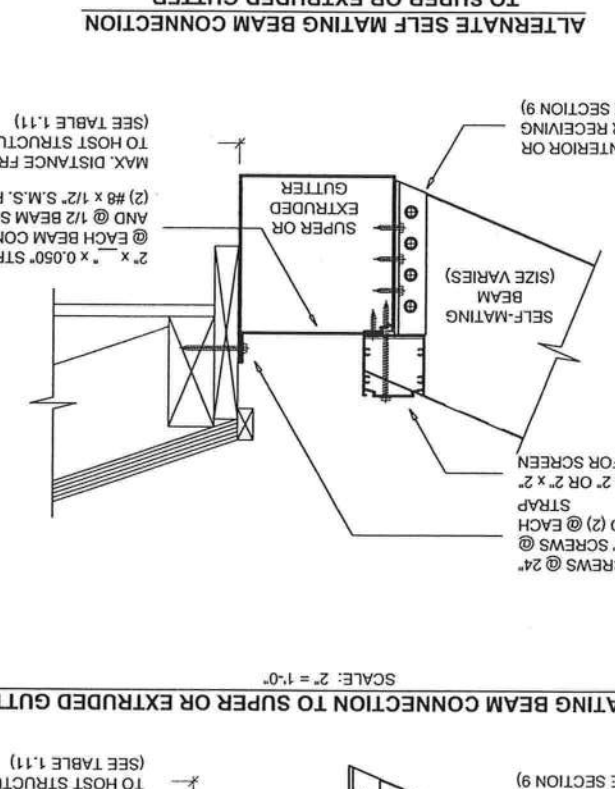
SELF MATING BEAM AND SUPER OR EXTRUDED GUTTER CONNECTION

SCALE: 2" = 1'-0"



ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 2" = 1'-0"



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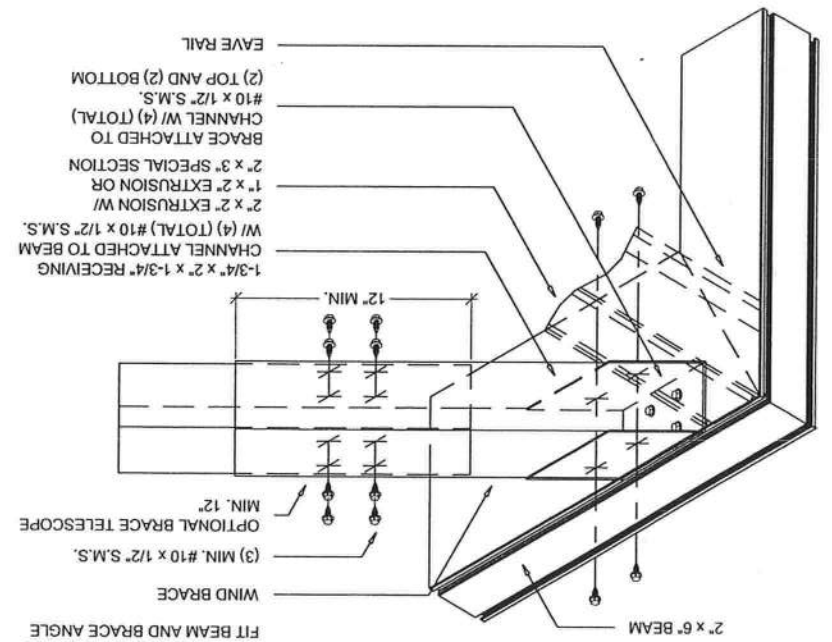
TELESCOPING WIND BRACE CONNECTION DETAIL

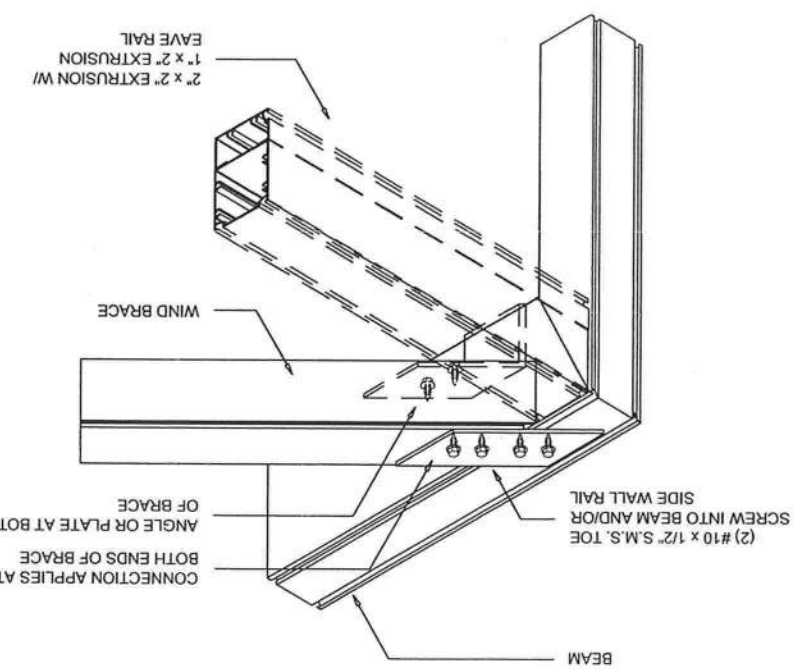
SCALE: 2" = 1'-0"



NOTES:
1. Wind bracing shall be provided at each side wall panel when enclosure projects more than three panels from host structure. Structures of four or more panels shall be spaced for even number of panels for opposing wind bracing.
2. Cut brace parts with min. 12" lap of larger and smaller brace.
3. Cut receiving channel with angle.

WIND BRACE CONNECTION DETAIL

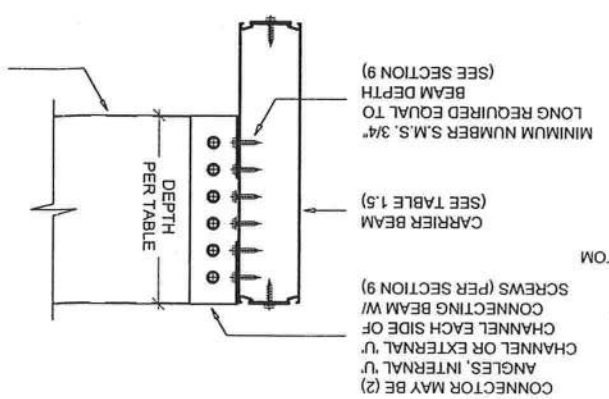
SCALE: 2" = 1'-0"



NOTES:
1. Wind bracing shall be provided at each side wall panel when enclosure projects more than (4) panels from host structure.

CARRIER BEAM TO BEAM CONNECTION DETAIL

SCALE: 2" = 1'-0"



BEAM TO WALL CONNECTION:
(2) 2" x 2" x 0.060" EXTERNALLY MOUNTED ANGLES ATTACHED TO WOOD FRAME WALL W/ MIN. (2) 3/8" x 2" LAG SCREWS PER SIDE OR TO CONCRETE W/ (2) 1/4" x 2-1/4" ANCHORS OR MASONRY WALL ADD (1) ANCHOR PER SIDE FOR EACH INCH OF BEAM DEPTH LARGER THAN 3"

EXTRUSIONS W/ INTERNAL SCREW BOSSES MAY BE CONNECTED W/ (3) #10 x 1-1/2" INTERNALLY

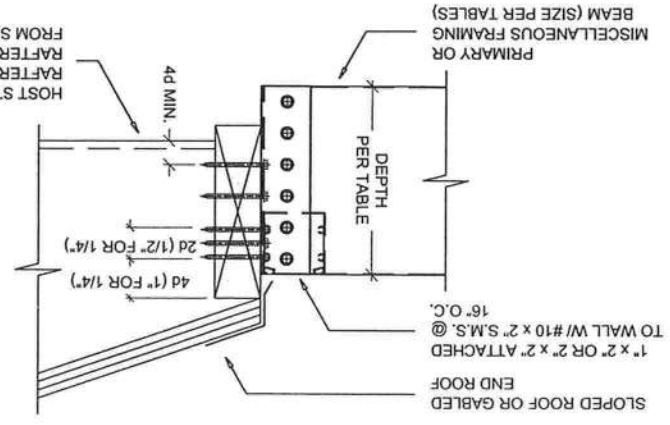
BEAM TO WALL CONNECTION DETAIL

SCALE: 2" = 1'-0"

ALTERNATE CONNECTION:
(1) 1-3/4" x 1-3/4" x 1-3/4" x 1/8" INTERNAL U-CHANNEL ATTACHED TO WOOD FRAME WALL W/ MIN. (3) 3/8" x 2" LAG SCREWS OR TO CONCRETE OR MASONRY WALL W/ (3) 1/4" x 2-1/4" ANCHORS OR ADD (1) ANCHOR PER SIDE FOR EACH INCH OF BEAM DEPTH LARGER THAN 3"

BEAM TO FASCIA CONNECTION DETAIL

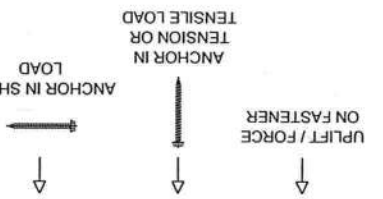
SCALE: 2" = 1'-0"

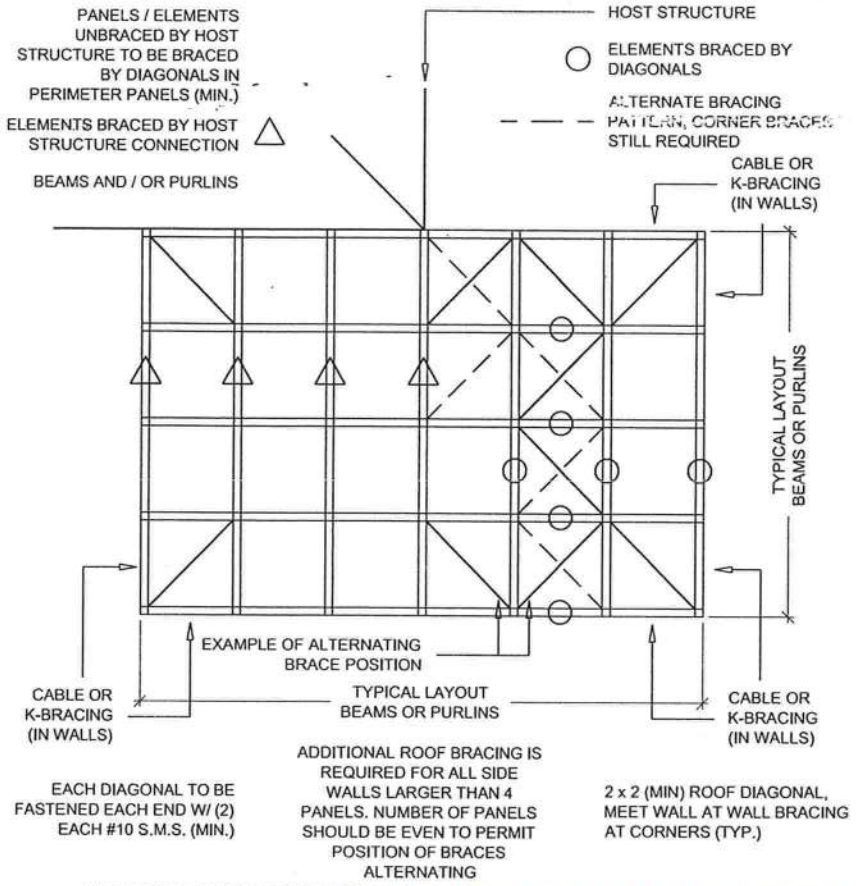


* FIND ROOF WIND LOAD IN DESIGN SPECIFICATIONS ON PAGE 3

$$\left[\frac{\text{ROOF WIND LOAD*} \times \text{BEAM SPACING} \times \left(\frac{\text{BEAM SPAN}}{2}\right)}{\text{ANCHOR ALLOWABLE LOAD}}\right] = \#\text{ OF ANCHORS}$$

CALCULATE THE NUMBER OF SCREWS REQUIRED BY SOLVING THE FOLLOWING EQUATION:

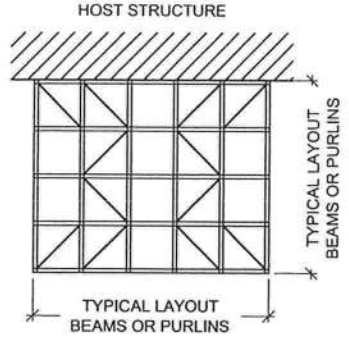




(POOL ENCLOSURE SCREEN ROOF MAY BE FLAT, GABLE, MANSARD, DOME, OR HIP)

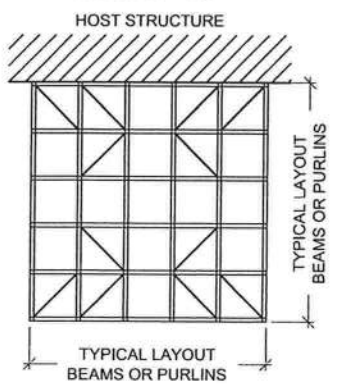
POOL ENCLOSURE DIAGONAL BRACING - SCHEMATIC PLAN VIEW

SCALE: 1/4" = 1'-0"



WIND BRACING PATTERN
TYPICAL FOR EVEN NUMBER OF SIDE PANELS OVER 4

SCALE: 1/8" = 1'-0"



WIND BRACING PATTERN
TYPICAL FOR ODD NUMBER OF SIDE PANELS OVER 4

SCALE: 1/8" = 1'-0"

CABLE BRACING

General Notes and Specifications:

1. The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:

a) FRONT WALL CABLES - 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
1/8"	445 Sq. Ft. / PAIR OF CABLES

* TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE: FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft.
SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft.
TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 19 STAINLESS STEEL

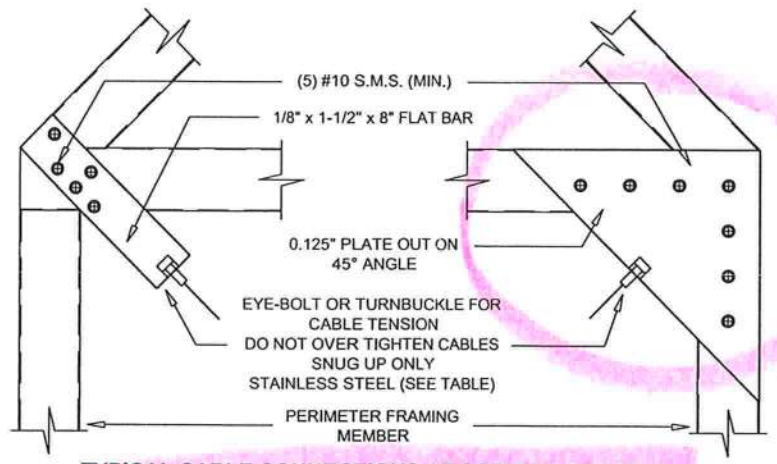
CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

** SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

c) To calculate the required pair of cables for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

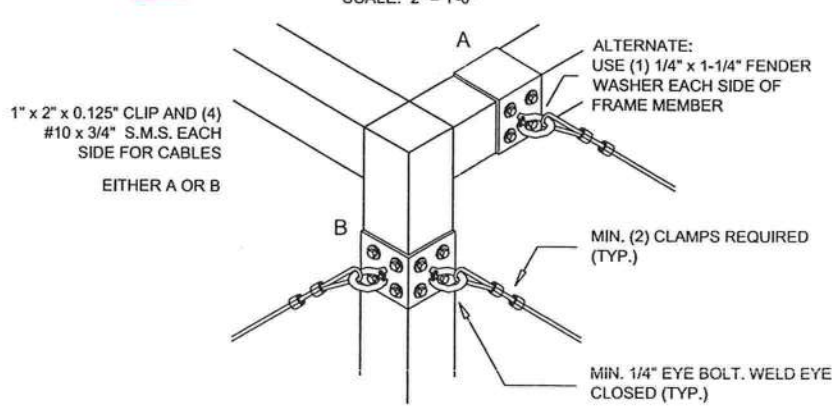
NOTES:

1. Where wall height is such that a girt is required between the top or eave rail and the chair rail, (i.e. a mid-rise girt), then the front wall shall have two cable pairs and they shall be attached to the top rail and the mid-rise rail. If more than one additional girt is required between the top or eave rail and the chair rail, then there shall be an additional front wall cable pair at that girt also.
2. Side walls do not require cables until the side wall area is greater than 233 Sq. Ft.. The side wall cable may be attached at the mid-rise girt or the top rail.
3. Standard rounding off rules apply. ie: if the number of cables calculated is less than 2.5 pairs use two cables; if the number of cables calculated is 2.5 pairs or greater use 3 pairs of cables.
4. Additional roof bracing is required for all side walls larger than 4 panels. Number of panels shall be even and position shall be alternating.
5. Cables shall be snugged up tight only to not put strain on cables.



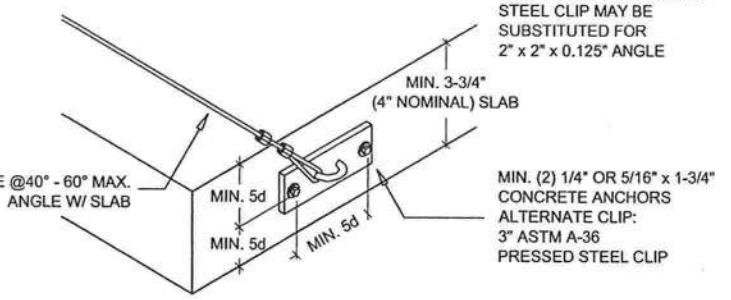
TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 2" = 1'-0"



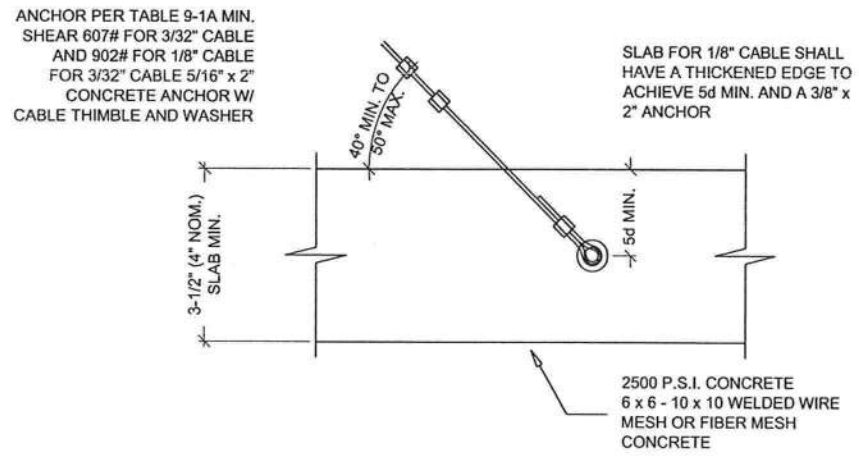
ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 2" = 1'-0"



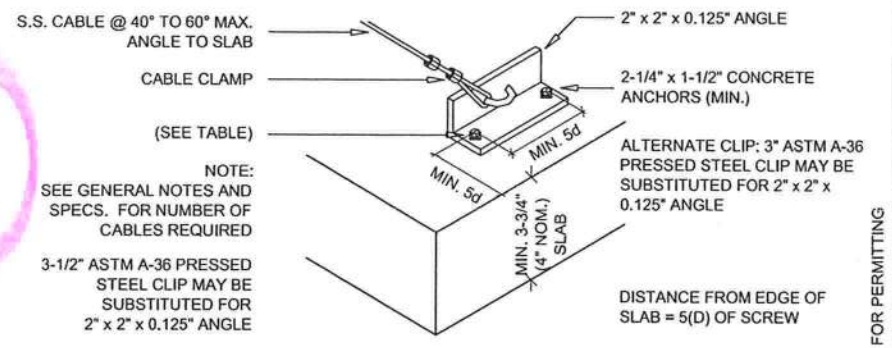
TYPICAL CABLE CONNECTION AT SLAB DETAIL - DETAIL 2

SCALE: 2" = 1'-0"



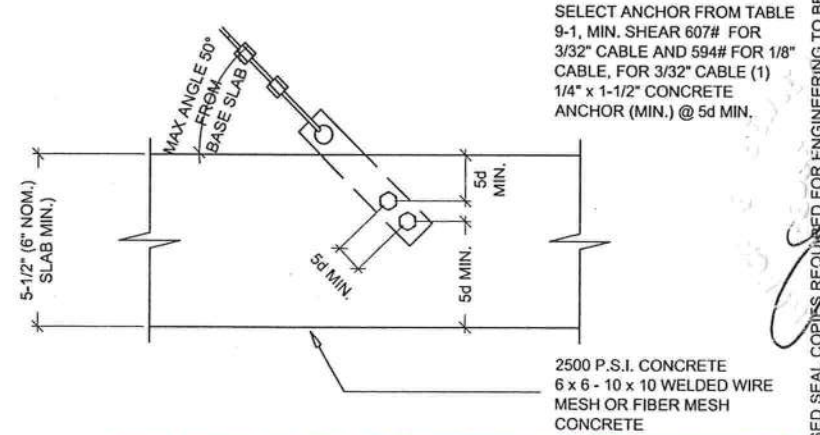
ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2A

SCALE: 2" = 1'-0"



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 2" = 1'-0"



ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 2" = 1'-0"

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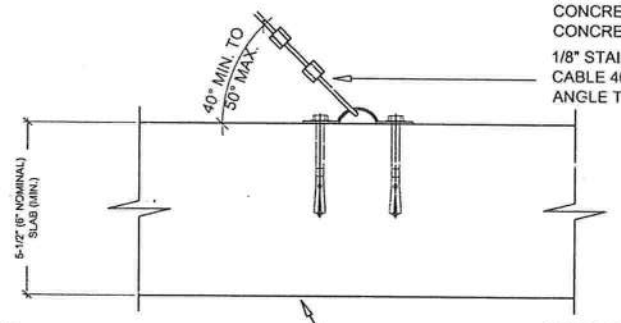


ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2D

SCALE: 2" = 1'-0"

K-BRACING

General Notes and Specifications:

1) The following shall apply to the installation of K-BRACINGas additional bracing to diagonal wind bracing for pool enclosures:

a) FRONT WALL K-BRACING - ONE SET FOR EACH 800 SF OF TOTAL WALL AREA
TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

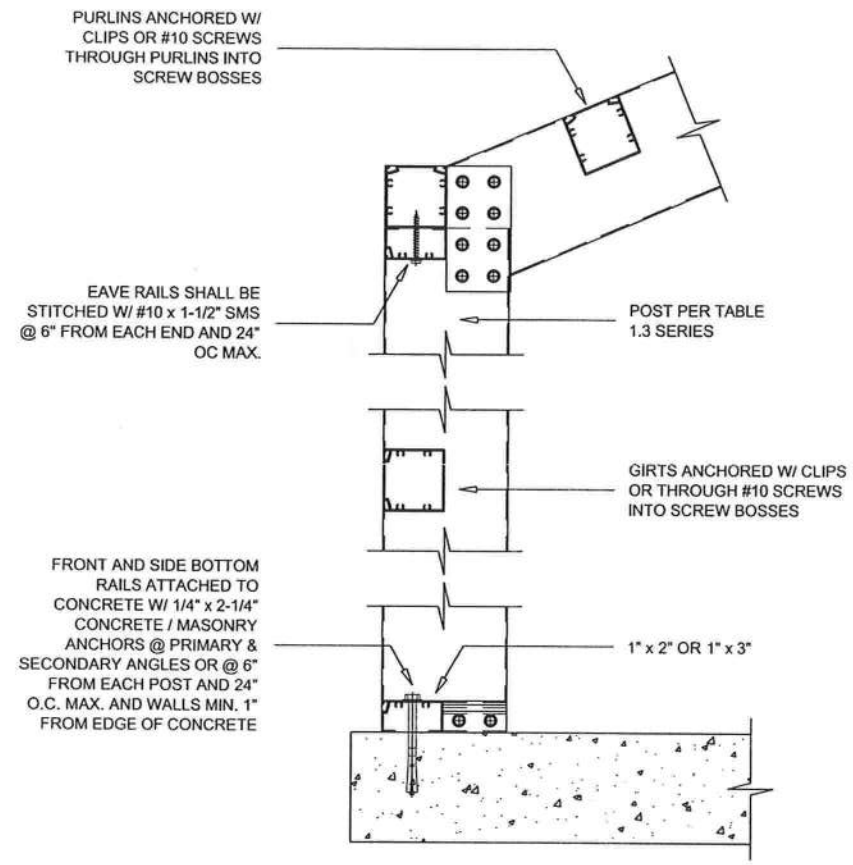
EXAMPLE: FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft.
SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft.
TOTAL WALL AREA = 336 Sq. Ft.
800 SF > 336 SF THUS ONE SET OF FRONT WALL K-BRACING IS REQUIRED.

b) SIDE WALL K-BRACING - ONE SET FOR 233 SF TO 800 SF OF WALL.

c) To calculate the required pair of k-bracing for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

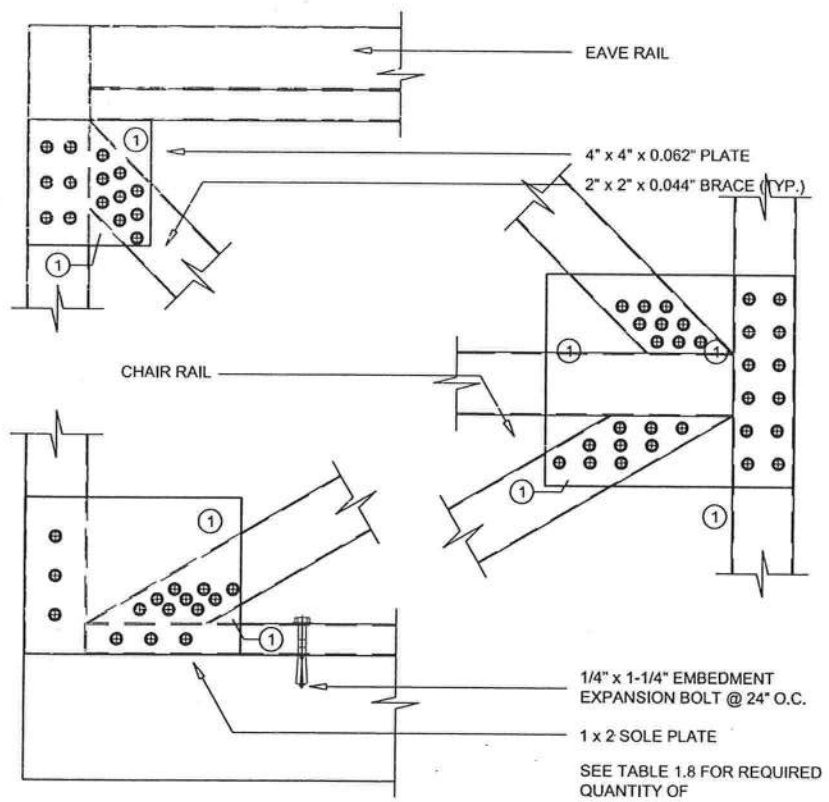
NOTES:

1. **K-bracing shall be used for all wind zones of 120 MPH EXPOSURE "C" and higher.**
2. Side walls do not require k-bracing until the side wall area is greater than 233 SF.
3. Standard rounding off rules apply. ie: if the number of k-bracing sets calculated is less than 1.5 sets use one set of k-braces; if the number of k-braces calculated is 1.5 sets or greater use 2 sets of k-bracing.



PURLIN & CHAIR RAIL DETAIL

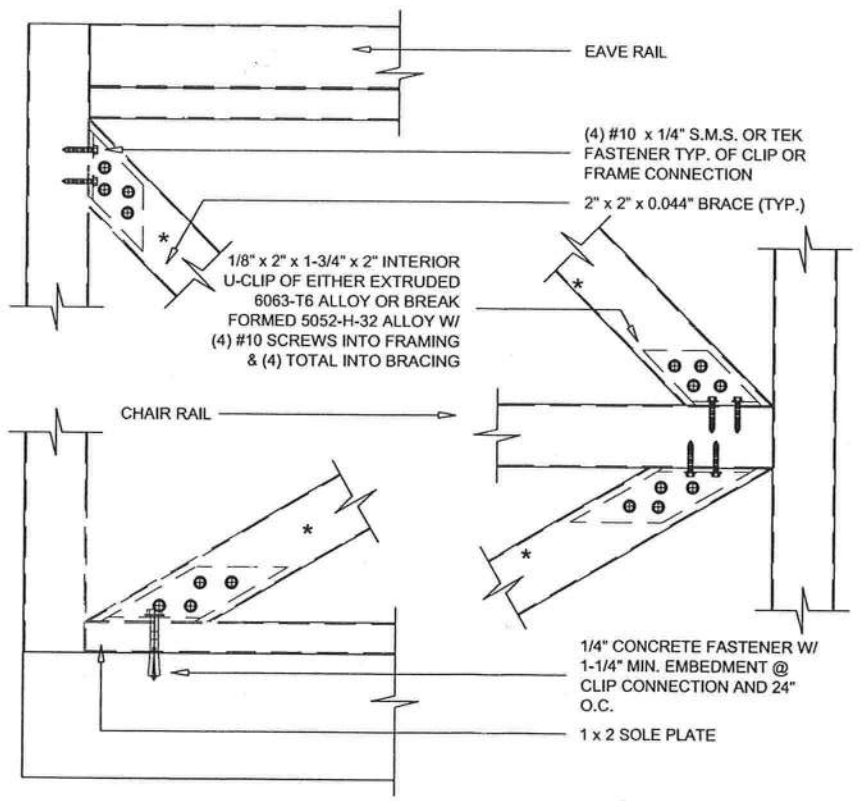
SCALE: 2" = 1'-0"



K-BRACING CONNECTION DETAILS

SCALE: 2" = 1'-0"

NOTES:
1. Can trim plate this area.
2. Alternate connections use 'H' bar cut to fit connections.

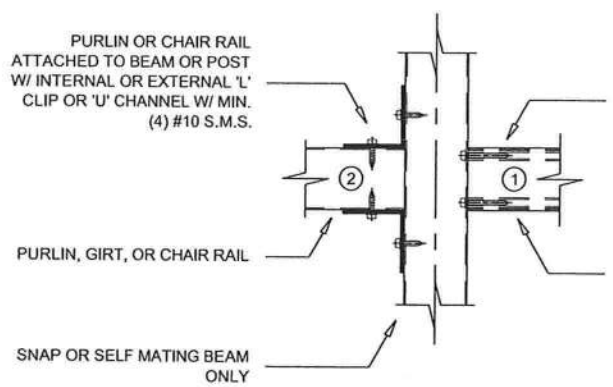


* TELSCOPING BRACE SYSTEM

ALTERNATE K-BRACING CONNECTION DETAILS

SCALE: 2" = 1'-0"

NOTE:
Alternate connections use 'H' bar cut to fit connections.



① FOR WALLS LESS THAN 6'-8" FROM TOP OF PLATE TO CENTER OF BEAM CONNECTION OR BOTTOM OF TOP RAIL THE GIRT IS DECORATIVE AND SCREW HEADS MAY BE REMOVED AND INSTALLED IN PILOT HOLES

② FOR ALL OTHER PURLINS AND GIRTS IF THE SCREW HEADS ARE REMOVED THEN THE OUTSIDE OF THE CONNECTION MUST BE STRAPPED FROM GIRT TO POST WITH 0.050" x 1-3/4" x 4" STRAP AND (4) #10 x 3/4" S.M.S. SCREWS TO POST AND GIRT

IF GIRT IS ON BOTH SIDES OF THE POST THEN STRAP SHALL BE 6" LONG AND CENTERED ON THE POST AND HAVE A TOTAL (12) #10 x 3/4" S.M.S.

PURLIN TO BEAM OR GIRT TO POST DETAIL

SCALE: 2" = 1'-0"

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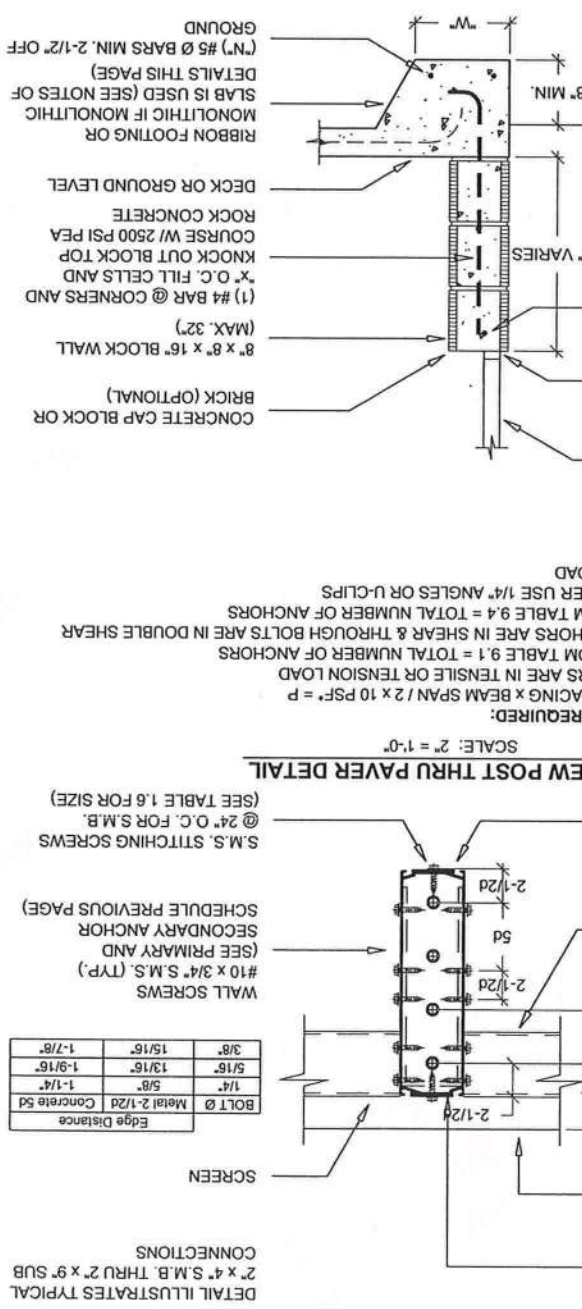
2" x 2" x 1/8" INTERIOR U-CLIP OF EITHER EXTRUDED 6005 T-5 ALLOY OR BREAK FORMED 6063 T-6 RO 5052 H-32 OR 34 ALLOY

DETAIL ILLUSTRATES TYPICAL 2" x 4" S.M.B. THRU 2" x 9" SUB CONNECTIONS

TOP VIEW POST THRU PAVER DETAIL

SCALE: 2" = 1'-0"



Edge Distance		
BOLT Ø	Metal 2-1/2d	Concrete 5d
1/4"	5/8"	1-1/4"
5/16"	13/16"	1-9/16"
3/8"	15/16"	1-7/8"

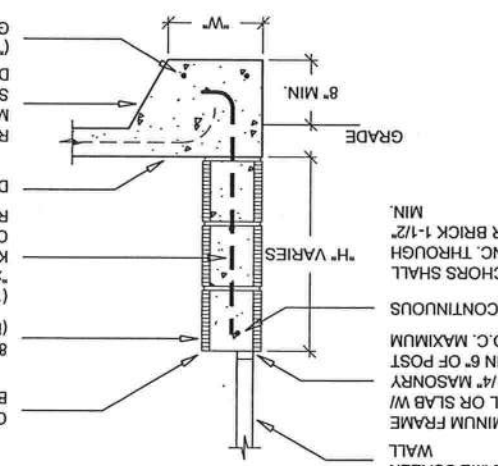
EXAMPLE OF NUMBER OF SRCREWS REQUIRED:
ANCHOR LOAD = BEAM / UPRIGHT SPACING x BEAM SPAN / 2 x 10 PSF = P

1. CONCRETE ANCHORS: ANCHORS ARE IN TENSILE OR TENSION LOAD
   P / ALLOWABLE LOAD FROM TABLE 9.1 = TOTAL NUMBER OF ANCHORS
2. UPRIGHT WALL ANCHORS: ANCHORS ARE IN SHEAR & THROUGH BOLTS ARE IN DOUBLE SHEAR
   P / ALLOWABLE LOAD FROM TABLE 9.4 = TOTAL NUMBER OF ANCHORS
3. FOR UPRIGHT 2" x 9" AND LARGER USE 1/4" ANGLES OR U-CLIPS

* SEE PAGE 3 FOR ROOF WIND LOAD

KNEE WALL FOOTING FOR SCREENED ENCLOSURES

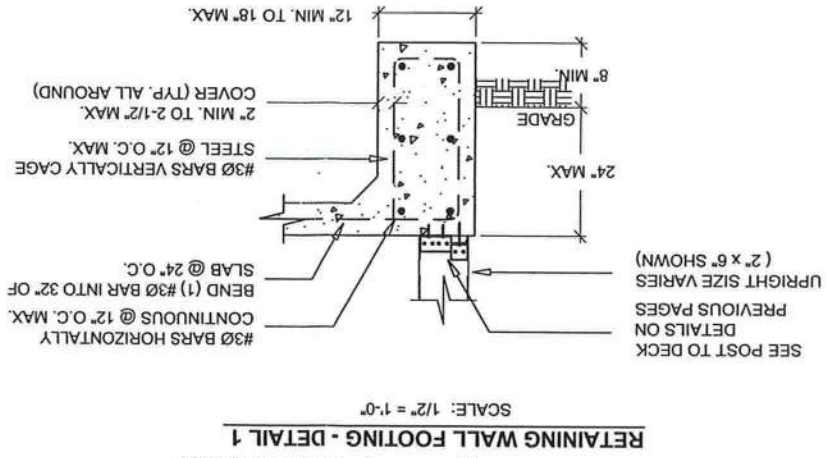
SCALE: 1/2" = 1'-0"



Knee Wall Table					
H	"W"	N			X
		#3	#4		
32"	4"	12"	3	2	10'-0"
40"	4"	12"	3	2	8'-0"
48"	8"	18"	N/A	3	6'-0"
56"	8"	18"	N/A	3	4'-0"
64"	12"	24"	N/A	3	2'-8"
72"	12"	30"	N/A	4	1'-8"

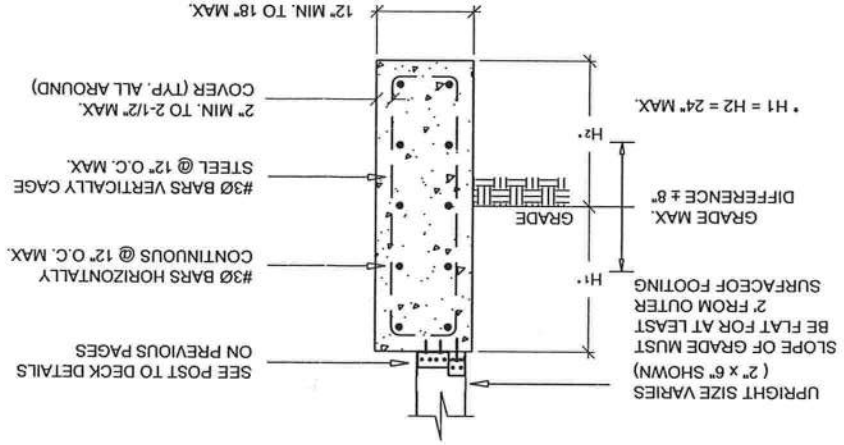
RETAINING WALL TO FOOTING - DETAIL 2

SCALE: 1/2" = 1'-0"



RETAINING WALL TO FOOTING - DETAIL 1

SCALE: 1/2" = 1'-0"



Allowable Beam Span for Wind Zone & Exposure Category

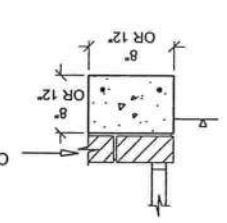
Ribbon Footing Data				100-125 MPH	126-134 MPH		135-144 MPH		145-150 MPH		Area sq. in.		Number of Bars	
Depth	x	n1*	n2**	B	C	B	C	B	C	B	Footing	Steel	#3Ø	#5Ø
8"	8"	2	1	15'4"	12'8"	15'4"	11'0"	12'8"	9'5"	11'0"	8'5"	64	0.12	2	1
12"	8"	2	1	23'0"	19'2"	23'0"	16'5"	19'2"	14'4"	16'5"	12'8"	72	0.13	2	1
8"	12"	2	1	23'0"	19'2"	23'0"	16'5"	19'2"	14'4"	16'5"	12'8"	72	0.13	2	1
12"	12"	2	2	34'6"	28'9"	34'6"	24'8"	28'9"	21'6"	24'8"	19'0"	108	0.19	3	2
8"	16"	3	2	36'0"	26'6"	31'9"	21'9"	25'6"	19'2"	21'9"	17'1"	192	0.35	4	2
12"	16"	3	2	36'0"	30'0"	36'0"	25'7"	30'0"	22'5"	25'7"	20'0"	216	0.39	4	2
8"	18"	3	2	40'0"	33'3"	40'0"	28'5"	33'3"	24'9"	28'5"	22'3"	252	0.46	5	3
12"	18"	3	2	40'0"	33'3"	40'0"	30'0"	34'3"	26'7"	30'0"	23'6"	288	0.52	5	3
8"	24"	4	3	48'0"	40'0"	48'0"	34'3"	40'0"	30'0"	34'3"	26'7"	384	0.69	7	4
12"	24"	4	3	48'0"	40'0"	48'0"	36'0"	40'0"	30'0"	36'0"	28'0"	432	0.78	8	4

Upright 4" Slab				100-125 MPH		126-134 MPH		135-144 MPH		145-150 MPH		Area sq. in.		Number of Bars	
Depth				B	C	B	C	B	C	B	C	Footing	Steel	#3Ø	#5Ø
Nominal 4" Slab															
12"	36"	5	4	69.1'	57.6'	69.1'	49.4'	57.6'	43.2'	49.4'	38.4'	432	0.78	-	3
12"	30"	4	3	57.6'	48.0'	57.6'	41.1'	48.0'	36.0'	41.1'	32.0'	360	0.65	-	3
12"	24"	4	3	48.0'	40.0'	48.0'	34.3'	40.0'	30.0'	34.3'	26.7'	288	0.52	-	2
12"	18"	3	2	37.9'	30.0'	36.0'	25.7'	30.0'	22.5'	25.7'	20.0'	216	0.39	4	2
12"	16"	3	2	36.0'	26.6'	31.9'	21.9'	25.6'	19.2'	21.9'	17.1'	192	0.35	4	2
12"	12"	2	2	34.6'	28.9'	34.6'	24.8'	28.9'	21.6'	24.8'	19.0'	108	0.19	3	2
8"	12"	2	1	23.0'	19.2'	23.0'	16.5'	19.2'	14.4'	16.5'	12.8'	72	0.13	2	1
12"	8"	2	1	23.0'	19.2'	23.0'	16.5'	19.2'	14.4'	16.5'	12.8'	72	0.13	2	1
8"	8"	2	1	15.4'	12.8'	15.4'	11.0'	12.8'	9.5'	11.0'	8.5'	64	0.12	2	1

"n1 = number of #3Ø bars @ 0.11 sq. in. grade 60 steel
"n2 = number of #5Ø bars @ 0.31 sq. in grade 60 steel

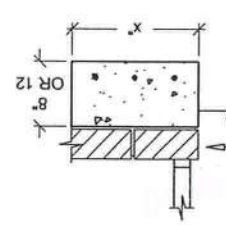
RIBBON FOOTING - TYPE 1

SCALE: 1/2" = 1'-0"



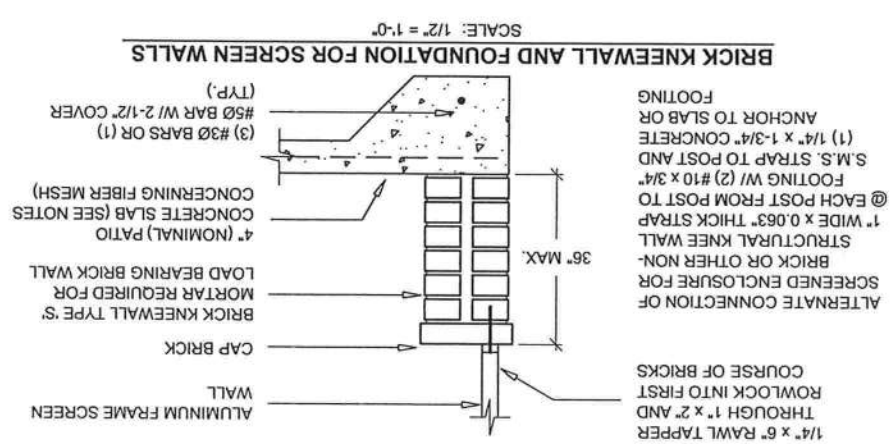
RIBBON FOOTING - TYPE 2

SCALE: 1/2" = 1'-0"



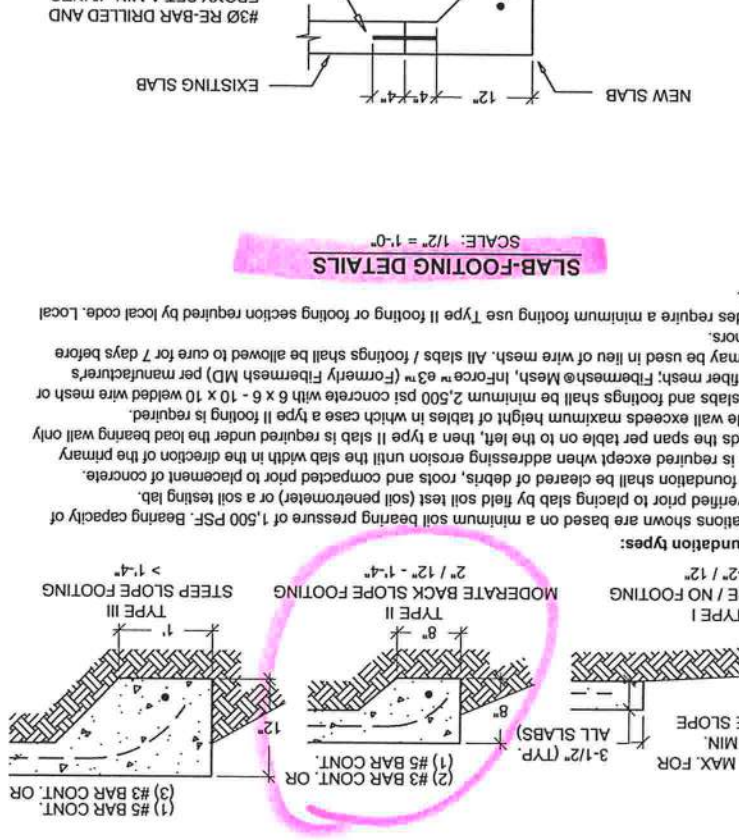
BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 1/2" = 1'-0"



SLAB-FOOTING DETAILS

SCALE: 1/2" = 1'-0"



Notes for all foundation types:

1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on to the left, then a type II slab is required under the load bearing wall only unless the side wall exceeds maximum height of tables in which case a type II footing is required.
4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, Inforce™ 63™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

DOWEL DETAIL FOR EXTENDING EXISTING 4" SLAB

SCALE: 3/4" = 1'-0"

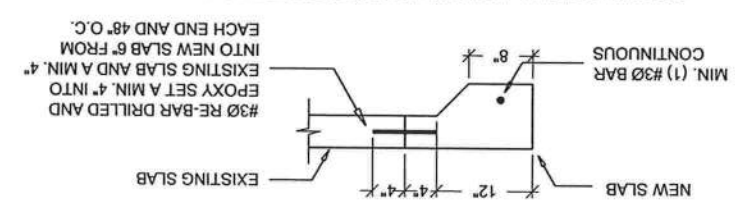


Table 1.1 110 — Allowable Beam Spans
6005 TCI — Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

For Wind Zones up to 110 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. Is also considered

Hollow Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd
3" x 2" x 0.045"	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd
2" x 3" x 0.045"	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd
2" x 3" x 0.060"	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd
2" x 4" x 0.050"	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	13'-11"	Ud
2" x 5" x 0.062"	20'-6"	Pd	20'-6"	Pd	20'-6"	Pd	20'-4"	Ud	19'-4"	Ud	18'-6"	Ud	17'-9"	Ud

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 4" x 0.046" x 0.100"	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-0"	Ud	15'-5"	Ud
2" x 5" x 0.050" x 0.096"	22'-9"	Pd	22'-9"	Pd	22'-9"	Pd	21'-10"	Ud	20'-8"	Ud	19'-10"	Ud	19'-1"	Ud
2" x 6" x 0.050" x 0.120"	28'-10"	Pd	28'-10"	Pd	27'-2"	Ud	25'-6"	Ud	24'-3"	Ud	23'-2"	Ud	22'-4"	Ud
2" x 7" x 0.060" x 0.120"	35'-3"	Pd	33'-5"	Ud	30'-11"	Ud	29'-2"	Ud	27'-9"	Ud	26'-6"	Ud	25'-4"	Ub
2" x 8" x 0.072" x 0.224"	45'-4"	Ud	41'-3"	Ud	38'-3"	Ud	36'-0"	Ud	34'-2"	Ud	32'-9"	Ud	31'-5"	Ud
2" x 9" x 0.072" x 0.224"	49'-11"	Ud	45'-4"	Ud	42'-1"	Ud	39'-7"	Ud	37'-7"	Ud	35'-11"	Ud	34'-7"	Ud
2" x 9" x 0.082" x 0.306"	53'-6"	Ud	48'-7"	Ud	45'-1"	Ud	42'-5"	Ud	40'-4"	Ud	38'-7"	Ud	37'-1"	Ud
2" x 10" x 0.092" x 0.374"	57'-4"	Ld	56'-4"	Ud	52'-4"	Ud	49'-3"	Ud	46'-9"	Ud	44'-9"	Ud	42'-11"	Ud

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Table 1.2 110 — Allowable Purlin Spans
6005 TCI — Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

For Wind Zones up to 110 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. Is also considered
A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd
3" x 2" x 0.045"	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd	6'-11"	Pd
2" x 3" x 0.045"	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd	9'-6"	Pd
2" x 3" x 0.060"	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd	11'-1"	Pd
2" x 4" x 0.050"	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd	14'-2"	Pd
2" x 5" x 0.062"	20'-6"	Pd	20'-6"	Pd	20'-6"	Pd	20'-6"	Pd	20'-6"	Pd	20'-4"	Ud	19'-8"	Ud

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	7'-3"	Pb	7'-3"	Pb	7'-3"	Pb	7'-3"	Pb	7'-3"	Pb	7'-3"	Pb	7'-3"	Pb
3" x 2" x 0.045"	9'-2"	Pb	9'-2"	Pb	9'-2"	Pb	9'-2"	Pb	9'-2"	Pb	9'-2"	Pb	9'-2"	Pb
2" x 3" x 0.045"	13'-6"	Pb	13'-6"	Pb	13'-6"	Pb	13'-6"	Pb	13'-6"	Pb	13'-6"	Pb	13'-6"	Pb
2" x 3" x 0.060"	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd	16'-7"	Pd
2" x 4" x 0.050"	21'-2"	Pd	21'-2"	Pd	21'-2"	Pd	21'-2"	Pd	21'-2"	Pd	20'-7"	Ub	19'-5"	Ub
2" x 5" x 0.062"	30'-9"	Pd	30'-8"	Ub	28'-10"	Ub	27'-3"	Ub	25'-11"	Ub	24'-8"	Ub	23'-4"	Ub

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Span is measured from center of beam and upright connection to fascia or wall connection.
3. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.
4. Spans may be interpolated.
5. 2" x 4" & 2" x 5" Hollow Girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

CHECK TABLE 1.6 FOR MINIMUM PURLIN SIZE FOR BEAMS.

Table 1.3 110 — Allowable Post / Upright Heights
6005 TCI — Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-9"	d	6'-3"	d	5'-11"	d	5'-7"	d	5'-4"	d	5'-2"	d
3" x 2" x 0.045"	8'-5"	d	7'-7"	d	7'-11"	d	7'-3"	d	6'-8"	d	6'-2"	d	5'-10"	b
2" x 3" x 0.045"	10'-4"	d	9'-5"	d	9'-8"	d	8'-10"	d	8'-2"	d	7'-8"	d	7'-3"	d
2" x 3" x 0.060"	11'-6"	d	10'-5"	d	11'-1"	d	10'-1"	d	9'-4"	d	8'-9"	d	8'-3"	d
2" x 4" x 0.050"	13'-6"	d	12'-3"	d	12'-7"	d	11'-5"	d	10'-7"	d	9'-11"	d	9'-4"	b
2" x 5" x 0.062"	17'-4"	d	15'-9"	d	15'-5"	d	14'-1"	d	12'-11"	b	12'-1"	b	11'-5"	b

Self Mating Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 4" x 0.046" x 0.100"	14'-11"	d	13'-7"	d	12'-8"	d	11'-10"	b	10'-11"	b	10'-2"	b	9'-7"	b
2" x 5" x 0.050" x 0.096"	18'-6"	d	16'-10"	d	15'-8"	d	14'-4"	b	13'-3"	b	12'-4"	b	11'-8"	b
2" x 6" x 0.050" x 0.120"	21'-9"	d	19'-7"	b	17'-6"	b	15'-11"	b	14'-8"	b	13'-9"	b	12'-11"	b
2" x 7" x 0.057" x 0.135"	24'-10"	d	21'-11"	b	19'-7"	b	17'-10"	b	16'-6"	b	15'-4"	b	14'-6"	b
2" x 8" x 0.072" x 0.224"	30'-7"	d	27'-10"	d	25'-10"	d	24'-4"	d	23'-1"	d	21'-11"	b	20'-8"	b
2" x 9" x 0.072" x 0.224"	33'-8"	d	30'-7"	d	28'-5"	d	26'-9"	d	25'-0"	b	23'-4"	b	22'-0"	b
2" x 9" x 0.082" x 0.206"	36'-1"	d	32'-10"	d	30'-5"	d	28'-8"	d	27'-3"	d	26'-0"	d	25'-0"	d
2" x 10" x 0.092" x 0.374"	41'-10"	d	38'-0"	d	35'-4"	d	33'-3"	d	31'-7"	d	30'-2"	d	29'-0"	d

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select upright length 'H'.
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. Spans may be interpolated.
9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Table 1.4 110 — Allowable Post / Girt / Chair Rail / Header Spans & Upright Heights
6005 TCI — Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.
A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width 'W' = Member Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-1"	d	6'-9"	d	6'-6"	d	6'-3"	d	6'-1"	d	5'-11"	d	5'-8"	d
3" x 2" x 0.045"	7'-11"	d	7'-7"	d	8'-1"	d	7'-8"	d	7'-3"	d	6'-11"	d	6'-6"	d
2" x 3" x 0.045"	9'-10"	d	9'-5"	d	10'-2"	d	9'-8"	d	9'-3"	d	8'-10"	d	8'-5"	d
2" x 3" x 0.060"	10'-11"	d	10'-5"	d	11'-8"	d	11'-1"	d	10'-6"	d	10'-1"	d	9'-7"	d
2" x 4" x 0.050"	12'-10"	d	12'-3"	d	13'-3"	d	12'-7"	d	11'-11"	d	11'-5"	d	10'-9"	d
2" x 5" x 0.062"	16'-5"	d	15'-9"	d	15'-11"	d	15'-1"	d	14'-4"	d	13'-8"	b	12'-11"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Member Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.040"	8'-6"	b	7'-11"	b	7'-6"	b	7'-1"	b	6'-9"	b	6'-6"	b	6'-2"	b
3" x 2" x 0.045"	9'-4"	b	8'-8"	b	8'-1"	b	7'-8"	b	7'-3"	b	6'-11"	b	6'-6"	b
2" x 3" x 0.045"	11'-7"	b	10'-10"	b	10'-2"	b	9'-8"	b	9'-3"	b	8'-10"	b	8'-5"	b
2" x 3" x 0.060"	13'-2"	b	12'-4"	b	11'-8"	b	11'-1"	b	10'-6"	b	10'-1"	b	9'-7"	b
2" x 4" x 0.050"	14'-11"	b	14'-0"	b	13'-3"	b	12'-7"	b	11'-11"	b	11'-5"	a	10'-9"	b
2" x 5" x 0.062"	18'-3"	b	17'-0"	b	15'-11"	b	15'-1"	b	14'-4"	b	13'-8"	b	12'-11"	b

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential gardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Table 1.5.1 110 — Town & Country Industries, Inc.
6005 TCI — Allowable Spans for Miscellaneous Framing Beams as Supporting Screen Roof Frame Members
Aluminum Alloy 6005 T-5

for Areas with Wind Loads up to 110 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. Is also considered

Single Self-Mating Beams	Tributary Load Width																			
	10'-0"		14'-0"		18'-0"		22'-0"		26'-0"		30'-0"		34'-0"		38'-0"		42'-0"		46'-0"		50'-0"		54'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																							
2" x 4" x 0.046" x 0.100"	16'-7"	Ub	13'-11"	Ub	12'-4"	Ub	11'-2"	Ub	10'-3"	Ub	9'-7"	Ub	8'-11"	Ub	8'-6"	Ub	8'-1"	Ub	7'-9"	Ub	7'-5"	Ub	7'-2"	Ub
2" x 5" x 0.050" x 0.096"	21'-4"	Ub	18'-0"	Ub	15'-11"	Ub	14'-5"	Ub	13'-3"	Ub	12'-4"	Ub	11'-7"	Ub	10'-11"	Ub	10'-5"	Ub	9'-11"	Ub	9'-7"	Ub	9'-2"	Ub
2" x 6" x 0.050" x 0.120"	24'-3"	Ub	20'-6"	Ub	18'-1"	Ub	16'-4"	Ub	15'-1"	Ub	14'-0"	Ub	13'-2"	Ub	12'-5"	Ub	11'-10"	Ub	11'-4"	Ub	10'-10"	Ub	10'-5"	Ub
2" x 7" x 0.060" x 0.120"	28'-4"	Ub	23'-11"	Ub	21'-1"	Ub	19'-1"	Ub	17'-7"	Ub	16'-4"	Ub	15'-4"	Ub	14'-6"	Ub	13'-10"	Ub	13'-2"	Ub	12'-8"	Ub	12'-2"	Ub
2" x 8" x 0.072" x 0.224"	30'-4"	Ud	27'-2"	Ud	24'-11"	Ud	23'-4"	Ud	22'-1"	Ud	20'-11"	Ub	19'-8"	Ub	18'-7"	Ub	17'-8"	Ub	16'-11"	Ub	16'-3"	Ub	15'-7"	Ub
2" x 9" x 0.072" x 0.224"	33'-5"	Ud	29'-10"	Ud	27'-5"	Ud	25'-8"	Ud	24'-3"	Ud	22'-9"	Ud	21'-5"	Ub	20'-3"	Ub	19'-3"	Ub	18'-5"	Ub	17'-8"	Ub	16'-11"	Ub
2" x 9" x 0.082" x 0.206"	35'-9"	Ud	31'-11"	Ud	29'-5"	Ud	27'-6"	Ud	26'-0"	Ud	24'-10"	Ud	23'-9"	Ub	22'-5"	Ub	21'-4"	Ub	20'-5"	Ub	19'-7"	Ub	18'-10"	Ub
2" x 10" x 0.092" x 0.374"	41'-6"	Ud	37'-1"	Ud	34'-1"	Ud	31'-11"	Ud	30'-2"	Ud	28'-9"	Ud	27'-7"	Ud	26'-7"	Ud	25'-4"	Ub	24'-2"	Ub	23'-2"	Ub	22'-4"	Ub

Double Self-Mating Beams	Tributary Load Width																			
	10'-0"		14'-0"		18'-0"		22'-0"		26'-0"		30'-0"		34'-0"		38'-0"		42'-0"		46'-0"		50'-0"		54'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																							
(2) 2" x 8" x 0.072" x 0.224"	51'-4"	Ub	43'-4"	Ub	38'-3"	Ub	34'-7"	Ub	31'-10"	Ub	29'-7"	Ub	27'-10"	Ub	26'-4"	Ub	25'-0"	Ub	23'-11"	Ub	22'-11"	Ub	22'-1"	Ub
(2) 2" x 9" x 0.072" x 0.224"	55'-9"	Ub	47'-2"	Ub	41'-7"	Ub	37'-7"	Ub	34'-7"	Ub	32'-3"	Ub	30'-3"	Ub	28'-7"	Ub	27'-3"	Ub	26'-0"	Ub	24'-11"	Ub	24'-0"	Ub
(2) 2" x 9" x 0.082" x 0.206"	61'-11"	Ub	52'-4"	Ub	46'-2"	Ub	41'-9"	Ub	38'-5"	Ub	35'-9"	Ub	33'-7"	Ub	31'-9"	Ub	30'-2"	Ub	28'-10"	Ub	27'-8"	Ub	26'-8"	Ub
(2) 2" x 10" x 0.092" x 0.374"	73'-4"	Ub	61'-11"	Ub	54'-8"	Ub	49'-5"	Ub	45'-6"	Ub	42'-4"	Ub	39'-9"	Ub	37'-7"	Ub	35'-9"	Ub	34'-2"	Ub	32'-10"	Ub	31'-7"	Ub

Note:
1. It is recommended that the engineer be consulted on any carrier beam that spans more than 50'
2. Span is measured from center of connection to fascia or wall connection.
3. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Spans may be interpolated.
5. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Example:
The Maximum 'L' for a 2" x 4" x 0.046" x 0.100" Single Self-Mating Beam with Tributary Load Width = 22'-0" is 11'-2"

Table 1.5.2 110 T — Town & Country Industries, Inc.
Allowable Spans for Miscellaneous Framing Beams as Supporting Screen Roof Frame Members
One End of Beam Attached to Host Structure
for Areas with Wind Loads up to 110 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered
Aluminum Alloy 6005 T-5

Single Single Self-Mating Beams	Tributary Load Width																			
	10'-0"		14'-0"		18'-0"		22'-0"		26'-0"		30'-0"		34'-0"		38'-0"		42'-0"		46'-0"		50'-0"		54'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																							
2" x 4" x 0.046" x 0.100"	12'-11"	Pd	12'-11"	Pd	12'-2"	Ub	11'-0"	Ub	10'-2"	Ub	9'-5"	Ub	8'-10"	Ub	8'-5"	Ub	7'-11"	Ub	7'-7"	Ub	7'-4"	Ub	7'-0"	Ub
2" x 5" x 0.050" x 0.116"	15'-9"	Pd	15'-9"	Pd	14'-8"	Ub	13'-4"	Ub	12'-3"	Ub	11'-5"	Ub	10'-8"	Ub	10'-1"	Ub	9'-7"	Ub	9'-2"	Ub	8'-10"	Ub	8'-6"	Ub
2" x 6" x 0.050" x 0.120"	18'-5"	Pd	18'-3"	Ub	16'-1"	Ub	14'-7"	Ub	13'-5"	Ub	12'-6"	Ub	11'-9"	Ub	11'-1"	Ub	10'-7"	Ub	10'-1"	Ub	9'-8"	Ub	9'-4"	Ub
2" x 7" x 0.055" x 0.120"	20'-11"	Pd	19'-10"	Ub	17'-6"	Ub	15'-10"	Ub	14'-7"	Ub	13'-7"	Ub	12'-9"	Ub	12'-1"	Ub	11'-6"	Ub	10'-11"	Ub	10'-6"	Ub	10'-1"	Ub
2" x 8" x 0.070" x 0.224"	25'-4"	Pd	25'-4"	Pd	24'-11"	Ub	22'-7"	Ub	20'-9"	Ub	19'-4"	Ub	18'-2"	Ub	17'-2"	Ub	16'-4"	Ub	15'-8"	Ub	14'-11"	Ub	14'-5"	Ub
2" x 9" x 0.070" x 0.204"	27'-3"	Pd	27'-3"	Pd	25'-9"	Ub	23'-3"	Ub	21'-5"	Ub	19'-11"	Ub	18'-9"	Ub	17'-9"	Ub	16'-10"	Ub	16'-1"	Ub	15'-5"	Ub	14'-10"	Ub
2" x 9" x 0.082" x 0.326"	29'-3"	Pd	29'-3"	Pd	29'-3"	Pd	27'-5"	Ub	25'-11"	Ub	24'-5"	Ub	22'-11"	Ub	21'-9"	Ub	20'-8"	Ub	19'-9"	Ub	18'-11"	Ub	18'-3"	Ub
2" x 10" x 0.090" x 0.374"	34'-2"	Pd	34'-2"	Pd	34'-2"	Pd	31'-11"	Ud	30'-3"	Ub	28'-10"	Ub	27'-8"	Ub	26'-2"	Ub	24'-10"	Ub	23'-9"	Ub	22'-10"	Ub	21'-11"	Ub

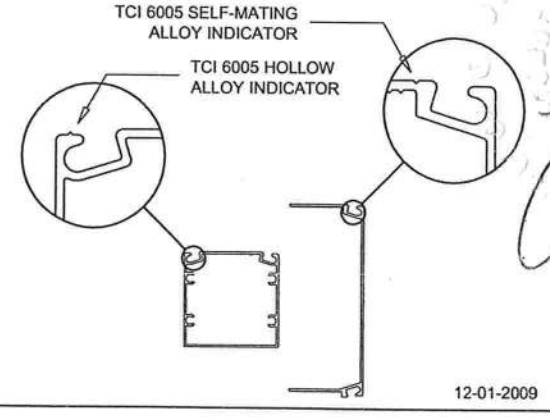
Note:
1. It is recommended that the engineer be consulted on any carrier beam that spans more than 50'
2. Span is measured from center of connection to fascia or wall connection.
3. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Spans may be interpolated.
5. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Town & Country 6005 Indicator Mark
(Instructions For Permit Purposes)

To: Plans Examiners and Building Inspectors,

These alloy indentification marks have been provided to contractors and yourself for permitting purposes. The details below illustrate the alloy indentification marks and the location of such marks. These alloy marks are used solely for our 6005 extrusions. It is ultimately the contractors responsibility that they receive and use only 6005 alloy shapes when using this engineering. We are providing this document to simplify the indentification of our 6005 alloy materials to be used in conjuction with our 6005 engineering.

A separate signed and sealed document from Town & Country will be provided to our pre-approved contractors once the materials are purchased.



12-01-2009

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2007 FLORIDA BUILDING CODE
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http://www.lebpe.com/

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Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

*Upright, purlins, and girts in this table are based on mimimum size requirements for the beams. Tables 1.3 and 1.4 must be checked to verify member spans.

Beam Size	*Minimum Upright / Column Size	*Minimum Purlin Size	*Minimum Girt & Knee Brace Size	Minimum Number of Screws Each Side			Beam Stitching Screws & Spacing
				#8 x ½"	#10 x ½"	#12 x ½"	
2" x 3" x 0.045" Hollow	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	8	6	4	-
2" x 4" x 0.050" Hollow	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	8	6	4	-
2" x 5" x 0.062" Hollow	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	8	6	4	-
2" x 4" x 0.046" x 0.100" SMB	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	8	6	4	#8 @ 24" O.C.
2" x 5" x 0.050" x 0.116" SMB	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	8	6	4	#8 @ 24" O.C.
2" x 6" x 0.050" x 0.120" SMB	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	2" x 2" x 0.044" Hollow	10	8	6	#10 @ 24" O.C.
2" x 7" x 0.055" x 0.120" SMB	2" x 4" Hollow or SMB	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	14	12	10	#12 @ 24" O.C.
2" x 8" x 0.072" x 0.224" SMB	2" x 5" Hollow or SMB	2" x 3" x 0.045" Hollow	2" x 2" x 0.044" Hollow	16	14	12	#14 @ 24" O.C.
2" x 9" x 0.072" x 0.224" SMB	2" x 6" x 0.050" x 0.120" SMB	2" x 3" x 0.045" Hollow	2" x 3" x 0.045" Hollow	18	16	14	#14 @ 16" O.C.
2" x 9" x 0.082" x 0.306" SMB	2" x 7" x 0.055" x 0.120" SMB	2" x 4" x 0.050" Hollow or SMB	2" x 3" x 0.045" Hollow	20	18	16	#14 @ 16" O.C.
2" x 10" x 0.092" x 0.374" SMB	2" x 8" x 0.072" x 0.224" SMB	2" x 5" x 0.060" Hollow or SMB	2" x 4" x 0.050" Hollow or SMB	20	18	16	#14 @ 16" O.C.

Screw Size	Minimum Distance and Spacing of Screws	
	Edge To Center	Center To Center
#8	5/16"	5/8"
#10	3/8"	3/4"
#12	1/2"	1"
#14 or 1/4"	3/4"	1-1/2"
5/16"	7/8"	1-3/4"
3/8"	1"	2"

Gusset Plate Thickness	
Beam Size	Thickness
2" x 7" x 0.055" x 0.120" SMB	0.063"
2" x 8" x 0.072" x 0.224" SMB	0.125"
2" x 9" x 0.072" x 0.224" SMB	0.125"
2" x 9" x 0.082" x 0.306" SMB	0.190"
2" x 10" x 0.092" x 0.374" SMB	0.250"

Connection Example:
2" x 7" Beam & 2" x 4" upright & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate (14) #8 x 1/2" sms ea. side of beam & upright.

Note:
1. All connections shall use a full lap cut or 1/16" interior gusset plate with # of fasteners per table.
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
4. Hollow splice connections can be made provided the connection is approved by the engineer.
5. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
6. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
7. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.219" s.m.b. w/ 2" x 6" x 0.050 x 0.135" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

Table 1.7 Minimum Size Screen Enclosure Knee Braces and Anchoring Required

Aluminum 6005 T-5

Brace Length*	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" each leg of channel
Up to 6'-0"	2" x 4" x 0.046" x 0.100"	2" H-Channel With (4) #10 x 1/2" each leg of channel

* Knee brace length shall be the horizontal and vertical length @ a 45° angle from the center of the connection to the face of the beam or upright.

Note:
1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
2. Cantilever beam detail shown on page 1-40 shall be used for transom wall to host structure attachment when knee brace length exceeds 6'-0".

Table 1.8 K-Bracing Fastening Schedule

Maximum Wall Width =	Number of #10 x 3/4" S.M.S. Required				
	Corner Post @ Top	Diagonals (K) per End	Intermediate Post @ Chair Rail	Corner Post @ Bottom	Plate to Sole Plate
20'-0"	2	2	4	2	2
30'-0"	2	2	4	2	2
40'-0"	3	4	6	2	2
50'-0"	4	5	8	3	3
60'-0"	6	7	12	3	3

*** Use screw sizes specified in the table below.**
Use front wall width when determining number of s.m.s. for the side wall K-bracing.
Use side wall width when determining number of s.m.s. for the front and / or back wall K-bracing.

Wind Zone	Screw Size
90 MPH	#10
100 MPH	#10
110 MPH	#10
120 MPH	#10
130 MPH	#12
140-1&2 MPH	#14
150 MPH	#14

Table 1.11 Maximum Overhang for Rafter / Truss Tails when Connected to Screen Roof

20' Max. Enclosure Span	Rafter / Truss Tail #2 Span / bending (b) or deflection (d)									
Wind Zone ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10		2x12	
100-110	4	2'-2"	b	5'-4"	b	9'-3"	b	15'-0"	b	22'-3"	b
120	4	2'-2"	b	5'-4"	b	9'-3"	b	15'-0"	b	22'-3"	b
123	4.3	2'-0"	b	4'-11"	b	8'-7"	b	13'-11"	b	20'-8"	b
130	5	1'-9"	b	4'-3"	b	7'-5"	b	12'-0"	b	17'-10"	b
140	6	1'-5"	b	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b
150	7	1'-3"	b	3'-0"	b	5'-3"	b	8'-7"	b	12'-9"	b
30' Max. Enclosure Span	**Rafter / Truss Tail #2 Span / bending (b) or deflection (d)**									
Wind Zone ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10		2x12	
100-110	4	1'-5"	b	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b
120	4	1'-5"	b	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b
123	4.3	1'-4"	b	3'-4"	b	5'-9"	b	9'-4"	b	13'-10"	b
130	5	1'-2"	b	2'-10"	b	4'-11"	b	8'-0"	b	11'-10"	b
140	6	0'-11"	b	2'-4"	b	4'-1"	b	6'-8"	b	9'-11"	b
150	7	0'-10"	b	2'-0"	b	3'-6"	b	5'-9"	b	8'-6"	b
40' Max. Enclosure Span	**Rafter / Truss Tail #2 Span / bending (b) or deflection (d)**									
Wind Zone ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10		2x12	
100-110	4	1'-1"	b	2'-8"	b	4'-7"	b	7'-6"	b	11'-1"	b
120	4	1'-1"	b	2'-8"	b	4'-7"	b	7'-6"	b	11'-1"	b
123	4.3	1'-0"	b	2'-6"	b	4'-4"	b	6'-11"	b	10'-4"	b
130	5	0'-10"	b	2'-2"	b	3'-8"	b	6'-0"	b	8'-11"	b
140	6	0'-9"	b	1'-9"	b	3'-1"	b	5'-0"	b	7'-5"	b
150	7	0'-7"	b	1'-6"	b	2'-8"	b	4'-4"	b	6'-4"	b

Note:

1. For overhangs with spans that exceed those listed above site specific engineering is required. If truss bottom cord extends more than 24" over the wall site specific engineering is required.
2. To convert from exposure "B" spans to "C" or "D" exposure spans see multipliers and example Table 1B on page3.

Example:
For a pool inclosure with 30' max. beam span, in a 123 MPH wind zone, "B" exposure. For 2 x 6 rafter / truss the max overhand from the wall of the host structure to the sub-fascia is 3'-4".

LATITUDES NORTH 30 - 30' - 00" NORTH (JACKSONVILLE, FL)

Table 1.9.1 Allowable Beam Spans
TCI 6005 Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

for Areas in Wind Zones up to 130 M.P.H., Exposure "B" and Latitudes North 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 15 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

Hollow Sections	Tributary Load Width 'W' = Beam Spacing																	
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"					
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																	
2" x 2" x 0.040"	5'-6"	Pb	5'-6"	Pb	5'-6"	Pb	5'-5"	Pb	5'-2"	Pb	4'-11"	Pb	4'-9"	Pb				
3" x 2" x 0.045"	6'-6"	Pb	6'-3"	Pb	6'-0"	Pb	5'-9"	Pb	5'-7"	Pb	5'-2"	Pb	4'-9"	Ub	4'-7"	Ub		
2" x 3" x 0.045"	9'-6"	Pd	8'-11"	Ud	8'-4"	Ud	7'-10"	Ud	7'-4"	Ub	6'-8"	Ub	6'-2"	Ub				
2" x 3" x 0.060"	10'-11"	Ud	9'-11"	Ud	9'-3"	Ud	8'-8"	Ud	8'-3"	Ud	7'-11"	Ud	7'-7"	Ud				
2" x 4" x 0.050"	12'-11"	Ud	11'-8"	Ud	10'-10"	Ud	10'-3"	Ud	9'-5"	Ub	8'-8"	Ub	7'-11"	Ub				
2" x 5" x 0.062"	16'-6"	Ud	14'-11"	Ud	13'-10"	Ub	12'-6"	Ub	11'-5"	Ub	10'-7"	Ub	9'-10"	Ub				

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing																	
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"					
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																	
2" x 4" x 0.046" x 0.100"	14'-4"	Ud	12'-11"	Ud	11'-8"	Ub	10'-6"	Ub	9'-8"	Ub	8'-11"	Ub	8'-4"	Ub				
2" x 5" x 0.050" x 0.096"	17'-8"	Ud	16'-0"	Ub	14'-2"	Ub	12'-10"	Ub	11'-10"	Ub	10'-11"	Ub	10'-3"	Ub				
2" x 6" x 0.050" x 0.120"	20'-8"	Ub	17'-9"	Ub	15'-9"	Ub	14'-3"	Ub	13'-1"	Ub	12'-2"	Ub	11'-4"	Ub				
2" x 7" x 0.060" x 0.120"	23'-2"	Ub	19'-11"	Ub	17'-8"	Ub	16'-0"	Ub	14'-9"	Ub	13'-8"	Ub	12'-9"	Ub				
2" x 8" x 0.072" x 0.224"	29'-2"	Ud	26'-6"	Ud	24'-8"	Ud	23'-2"	Ud	21'-6"	Ub	20'-0"	Ub	18'-10"	Ub				
2" x 9" x 0.072" x 0.224"	32'-1"	Ud	29'-2"	Ud	27'-1"	Ud	24'-8"	Ub	22'-9"	Ub	21'-2"	Ub	19'-11"	Ub				
2" x 9" x 0.082" x 0.206"	34'-5"	Ud	31'-3"	Ud	29'-0"	Ud	27'-4"	Ud	25'-11"	Ud	24'-4"	Ub	22'-11"	Ub				
2" x 10" x 0.092" x 0.374"	39'-11"	Ud	36'-3"	Ud	33'-8"	Ud	31'-8"	Ud	30'-1"	Ud	28'-9"	Ud	27'-8"	Ud				

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures Uniformed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 10'-10"

Table 1.9.2 Allowable Purlin Spans
Town & Country Industries, Inc.
Aluminum Alloy 6005 T-5

for Areas in Wind Zones up to 130 M.P.H., Exp. "B" and Latitudes North of 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 15 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing																	
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"					
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																	
2" x 2" x 0.044"	5'-6"	Pb	5'-6"	Pb	5'-6"	Pb	5'-5"	Pb	5'-2"	Pb	4'-11"	Pb	4'-9"	Pb				
3" x 2" x 0.045"	6'-6"	Pb	6'-3"	Pb	6'-0"	Pb	5'-9"	Pb	5'-7"	Pb	5'-4"	Pb	5'-1"	Pb				
2" x 3" x 0.045"	9'-4"	Ud	8'-11"	Ud	8'-7"	Ud	8'-4"	Ud	8'-1"	Ud	7'-9"	Ub	7'-2"	Ub				
2" x 3" x 0.060"	10'-5"	Ud	9'-11"	Ud	9'-7"	Ud	9'-3"	Ud	8'-11"	Ud	8'-8"	Ud	8'-5"	Ud				
2" x 4" x 0.050"	12'-3"	Ud	11'-8"	Ud	11'-3"	Ud	10'-10"	Ud	10'-6"	Ud	10'-3"	Ud	9'-8"	Ub				
2" x 5" x 0.062"	15'-8"	Ud	14'-11"	Ud	14'-5"	Ud	13'-10"	Ub	13'-2"	Ub	12'-6"	Ub	11'-9"	Ub				

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing																	
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"					
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)																	
2" x 2" x 0.044"	7'-3"	Pb	7'-3"	Pb	6'-11"	Ub	6'-6"	Ub	6'-1"	Ub	5'-9"	Ub	5'-4"	Ub				
3" x 2" x 0.045"	8'-7"	Pb	7'-10"	Ub	7'-3"	Ub	6'-9"	Ub	6'-4"	Ub	5'-11"	Ub	5'-6"	Ub				
2" x 3" x 0.045"	10'-9"	Ub	10'-1"	Ub	9'-6"	Ub	8'-10"	Ub	8'-3"	Ub	7'-9"	Ub	7'-2"	Ub				
2" x 3" x 0.060"	12'-4"	Ub	11'-6"	Ub	10'-10"	Ub	10'-3"	Ub	9'-10"	Ub	9'-5"	Ub	8'-11"	Ub				
2" x 4" x 0.050"	13'-11"	Ub	13'-1"	Ub	12'-2"	Ub	11'-6"	Ub	10'-10"	Ub	10'-4"	Ub	9'-8"	Ub				
2" x 5" x 0.062"	16'-10"	Ub	15'-8"	Ub	14'-8"	Ub	13'-10"	Ub	13'-2"	Ub	12'-6"	Ub	11'-9"	Ub				

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures Uniformed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-10"

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SCREEN ENCLOSURES
SECTION 1 TABLES
2007 FLORIDA BUILDING CODE
WITH 2009 SUPPLEMENTS - 2010 EDITION

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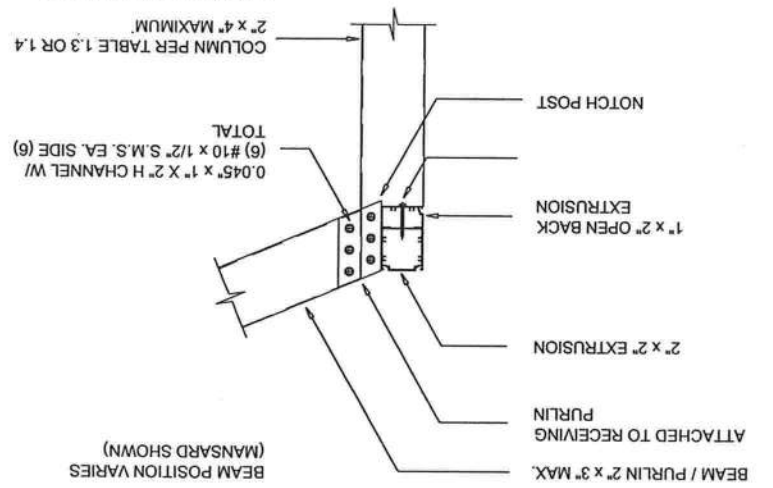
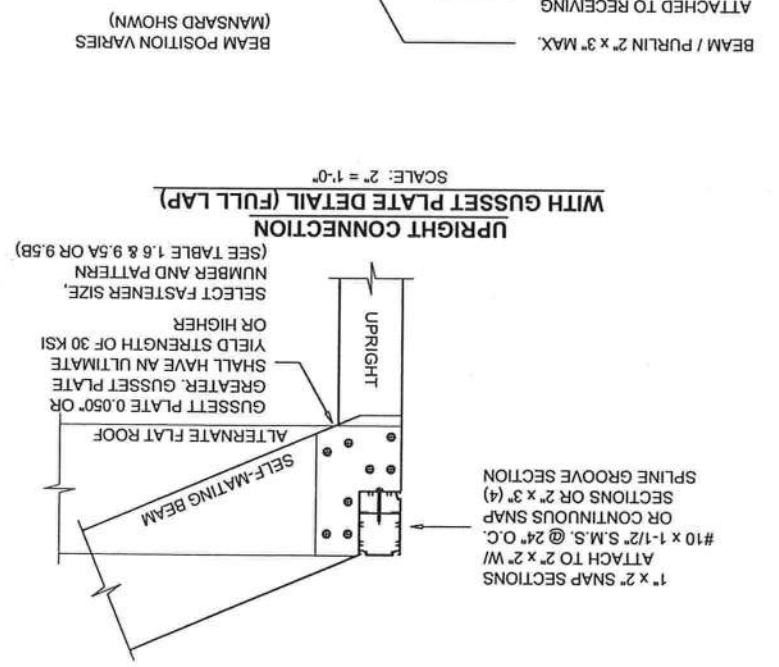
BEAM TO POST CONNECTION

SCALE: 2" = 1'-0"



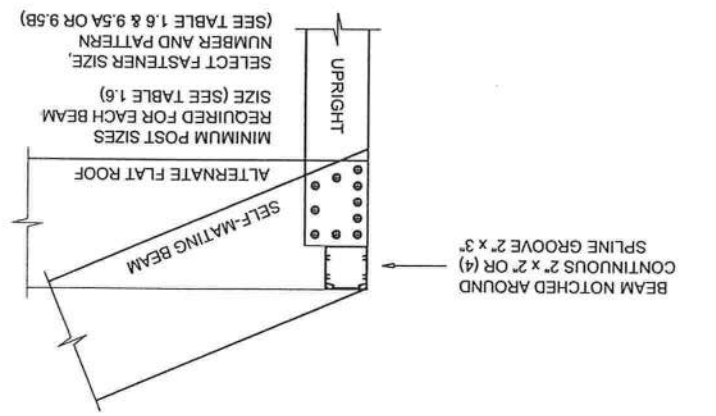
UPRIGHT CONNECTION WITH GUSSET PLATE DETAIL (FULL LAP)

SCALE: 2" = 1'-0"



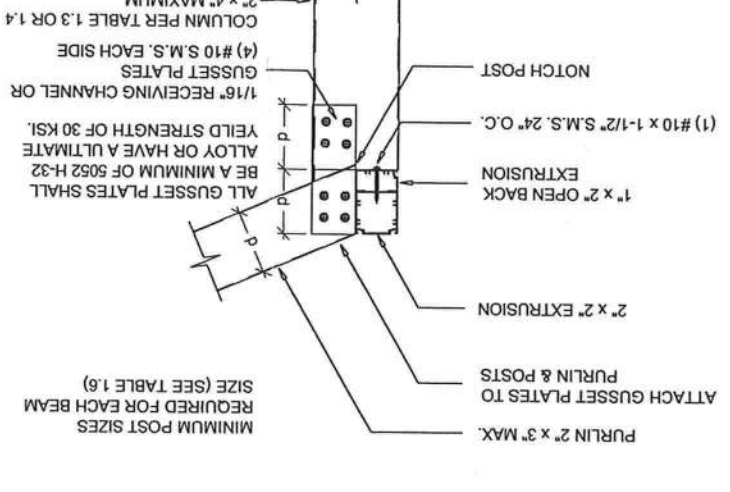
BEAM TO UPRIGHT CONNECTION DETAIL (FULL LAP)

SCALE: 2" = 1'-0"



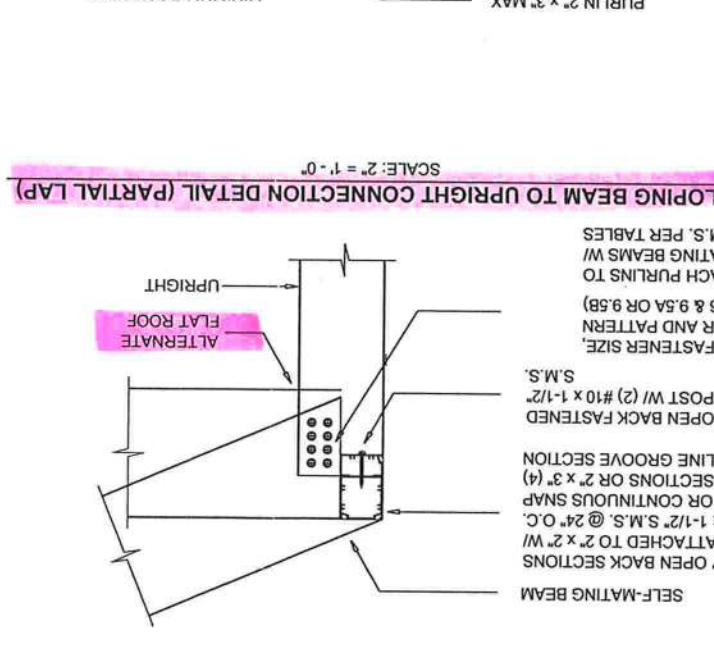
SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 2" = 1'-0"



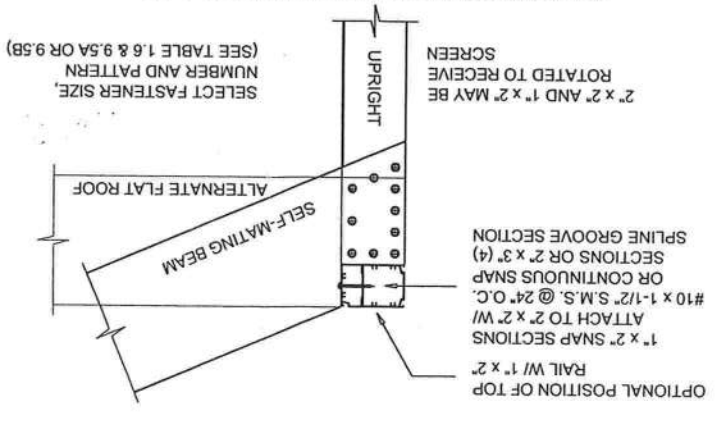
SIDE WALL TO PURLIN DETAIL

SCALE: 2" = 1'-0"



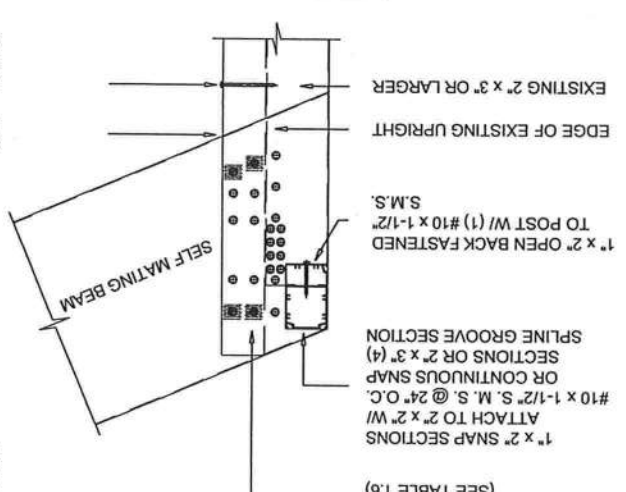
UPRIGHT CONNECTION DETAIL (FULL LAP)

SCALE: 2" = 1'-0"



COMPOSITE BEAM W/ ADDITION OF AUXILLARY MEMBER TO EXISTING HOLLOW MEMBER FOR EQUIVALENT HOLLOW MEMBER

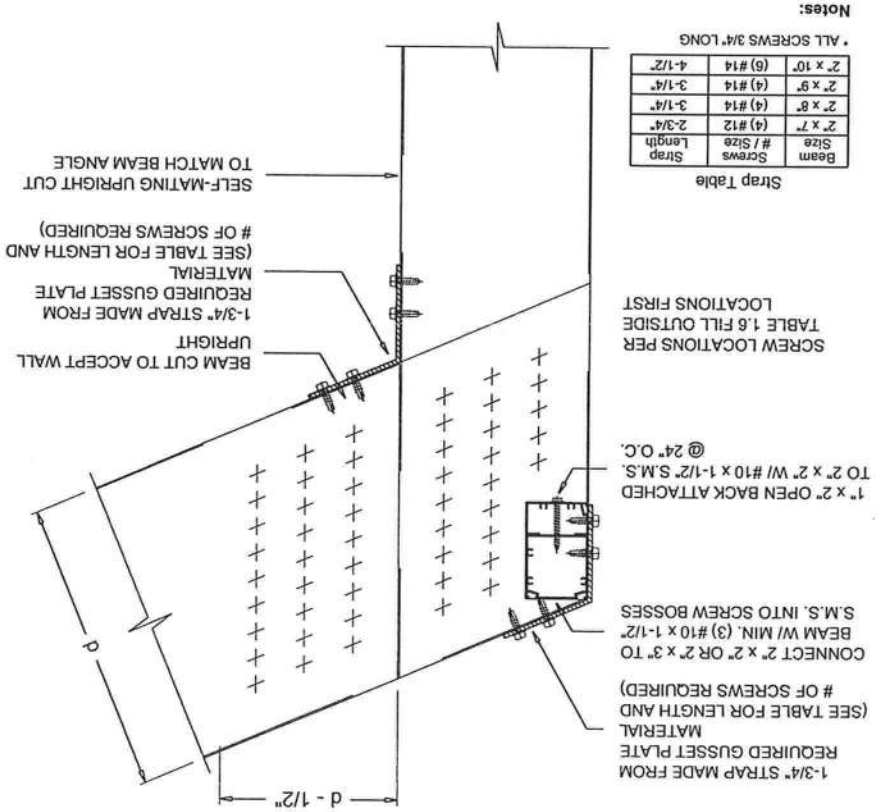
SCALE: 3" = 1'-0"



Existing Hollow Member	Member Added	Equivalent Hollow Member
2" x 2" x 0.040"	1" x 2" x 0.044"	2 x 3" x 0.045
	2" x 2" x 0.044"	
2" x 3" x 0.040"	1" x 2" x 0.044"	2 x 3" x 0.045
	2" x 2" x 0.044"	
2" x 4" x 0.040"	1" x 2" x 0.044"	2 x 4" x 0.045
	2" x 4" x 0.044" *	

* Joined using 2" "H" channel.

ALTERNATE BEAM TO POST CONNECTION (FULL LAP)



Strap Table

Beam Size	Screws # / Size	Strap Length
2" x 7"	(4) #12	2-3/4"
2" x 8"	(4) #14	3-1/4"
2" x 9"	(4) #14	3-1/4"
2" x 10"	(6) #14	4-1/2"

* ALL SCREWS 3/4" LONG

Notes:

1) Fill outer screw positions first until required number of screws is achieved.
2) See Table 1.6 for screw sizes and number.
3) Screw pattern layout with spacing between screws greater than minimum is allowed so that equal spacing is achieved.
4) 2" x 8" beam with 2" x 5" upright shown. Other beam to upright combinations per table 1.6 may be used.