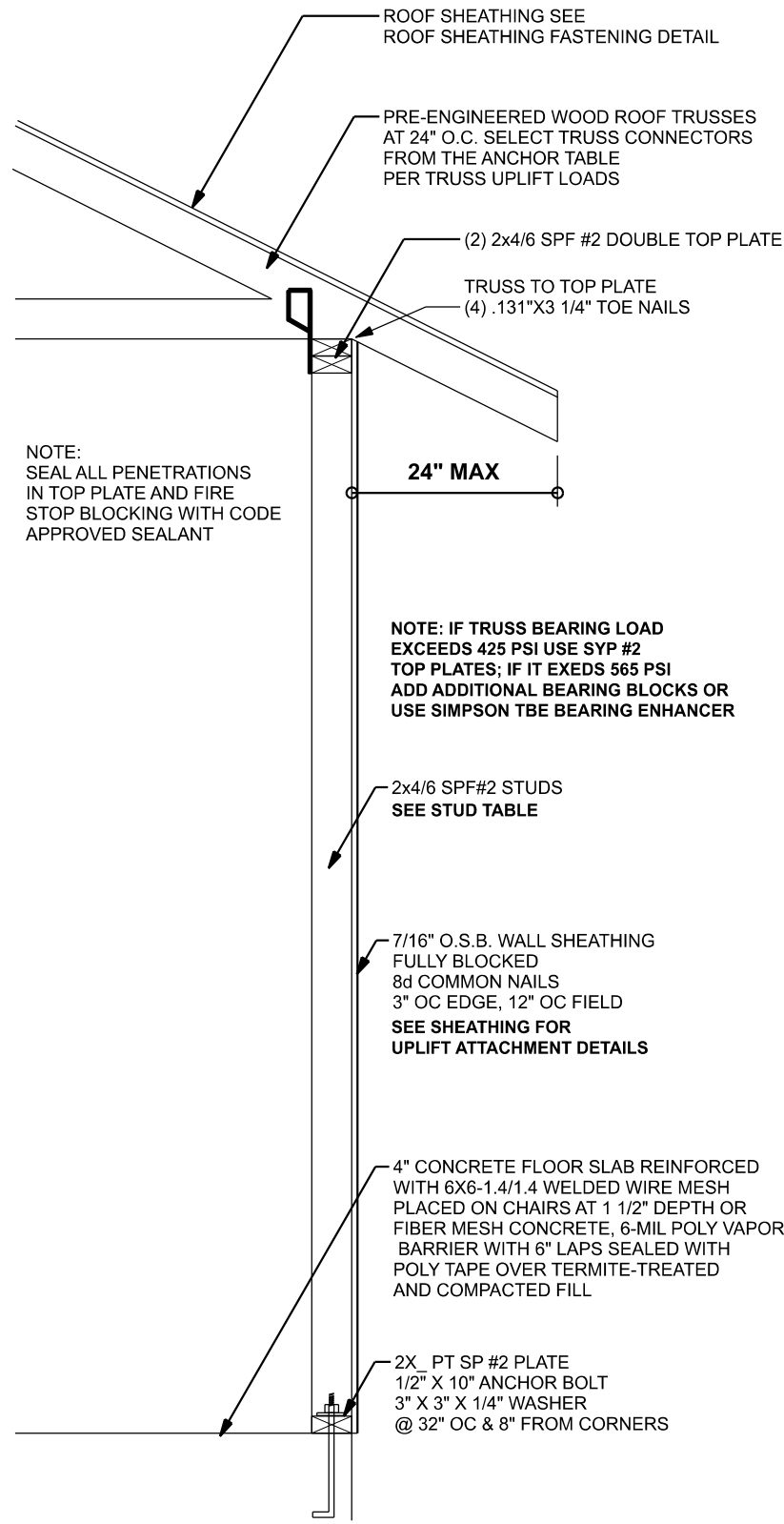


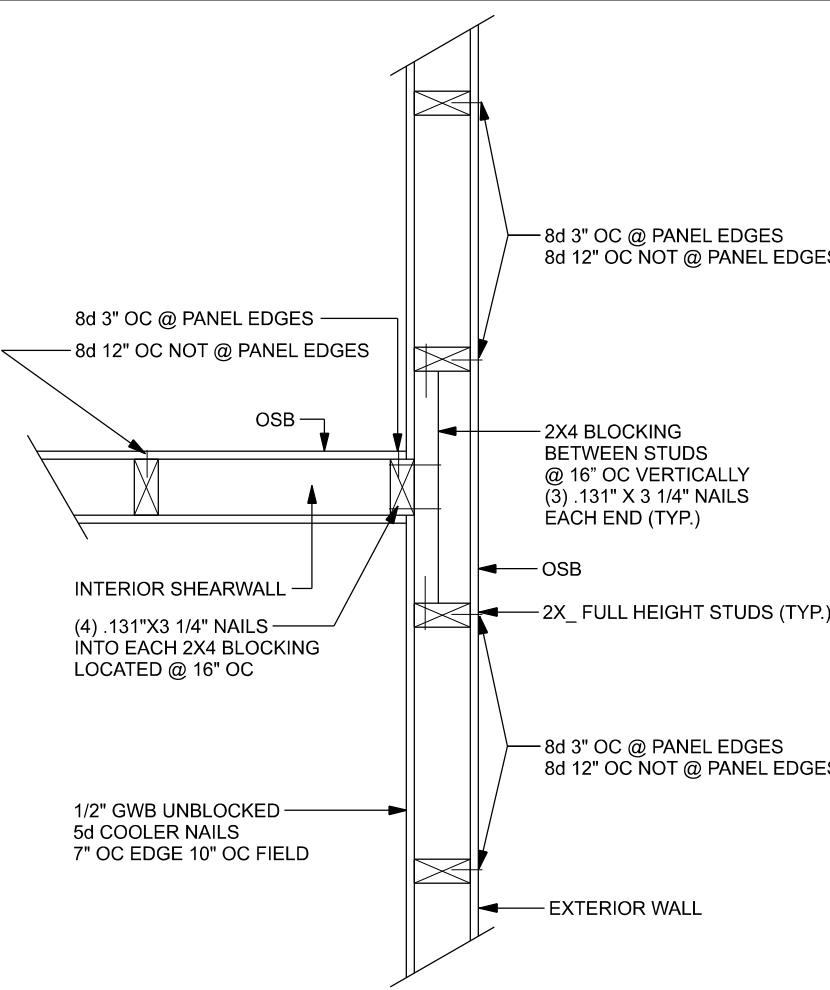
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RRS-01 (2 3/8" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

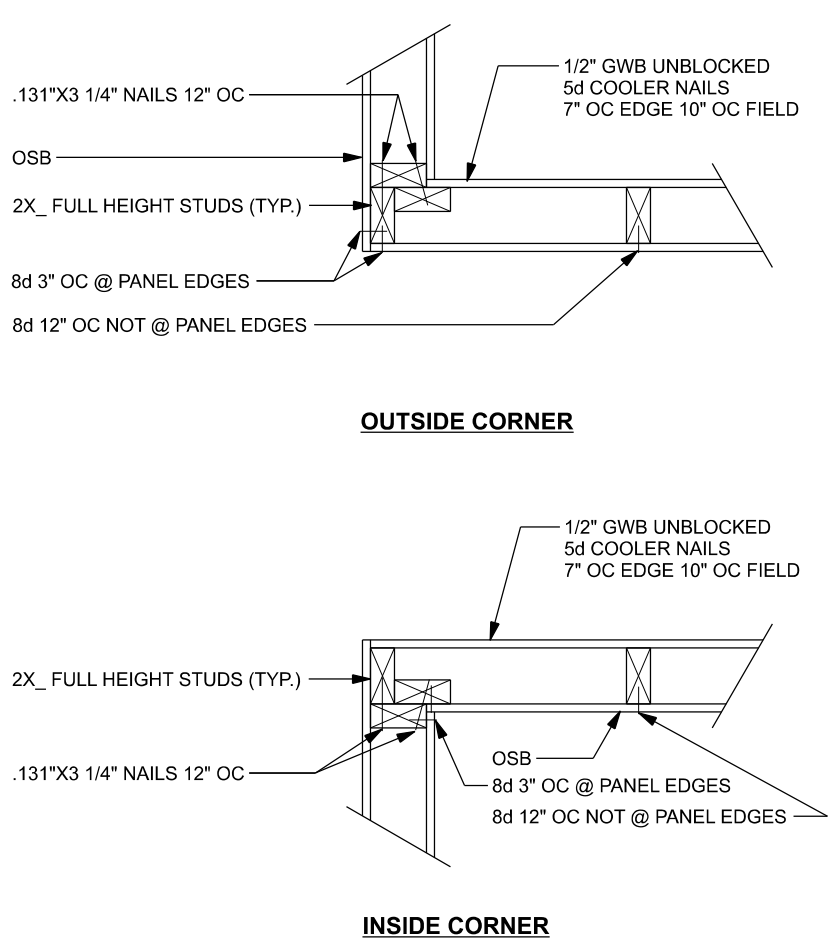
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including eaves and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based on the type of roofing material being used. See manufacturer Florida product approval.



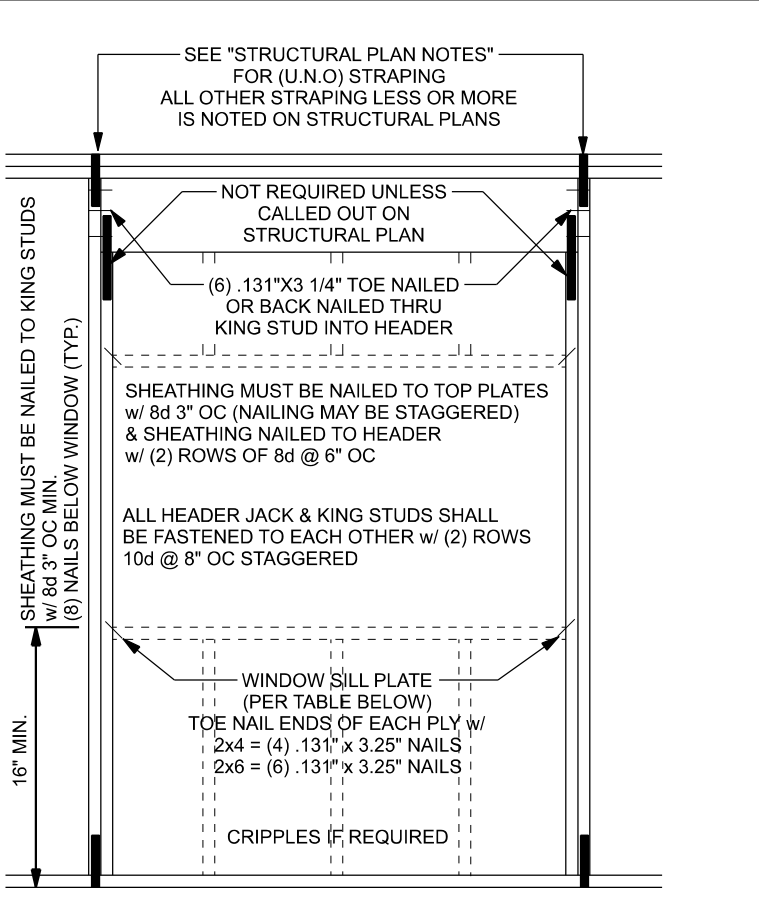
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



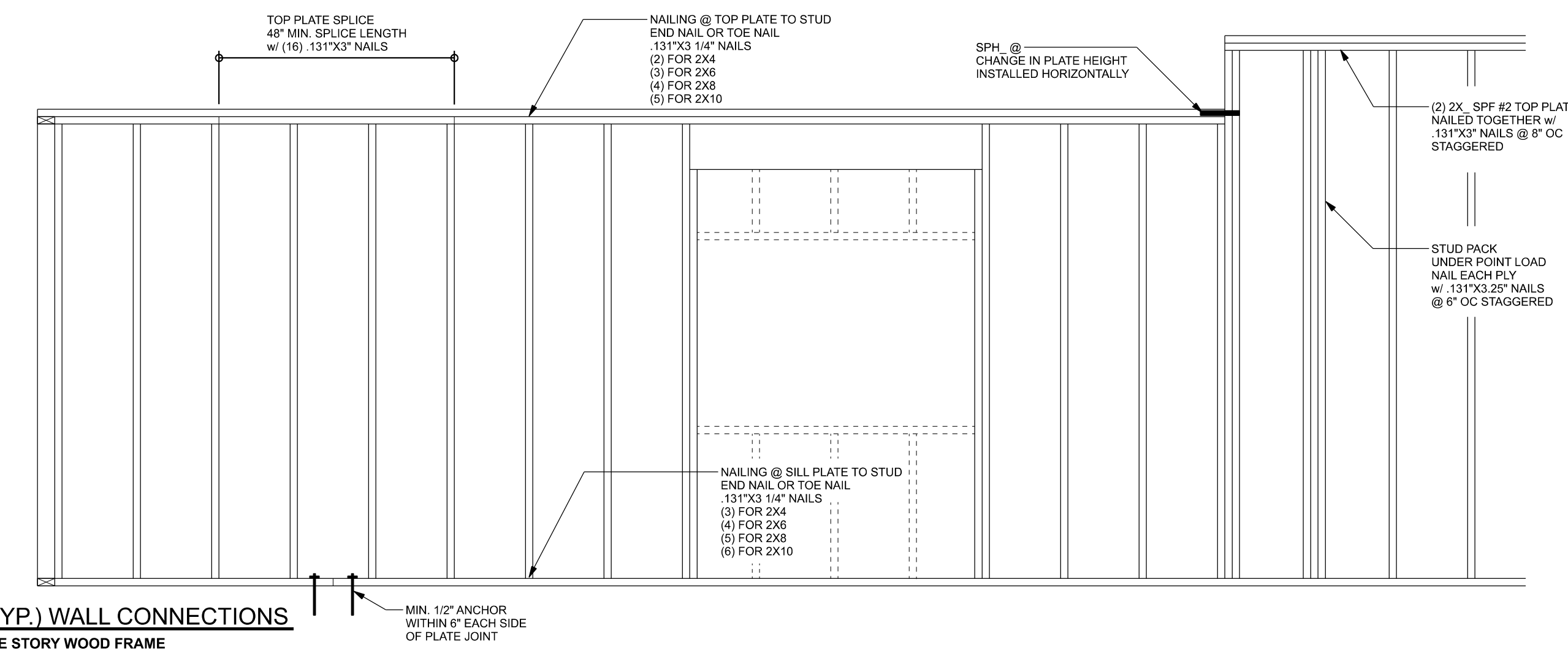
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



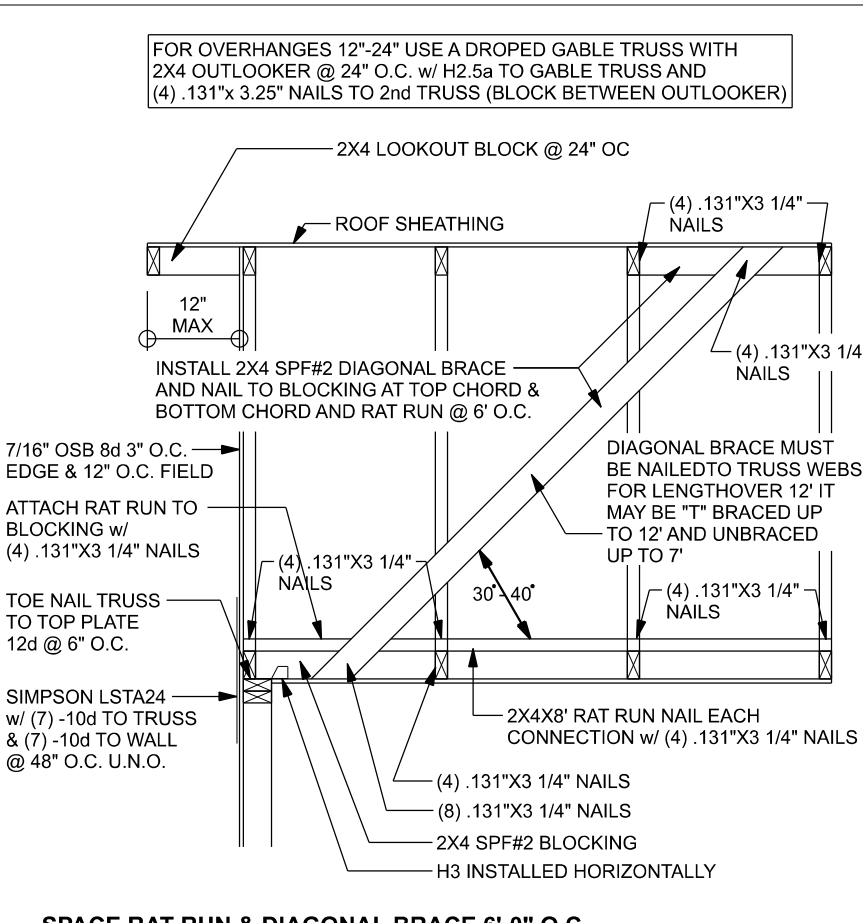
(TYP.) CORNER FRAMING WOOD FRAME



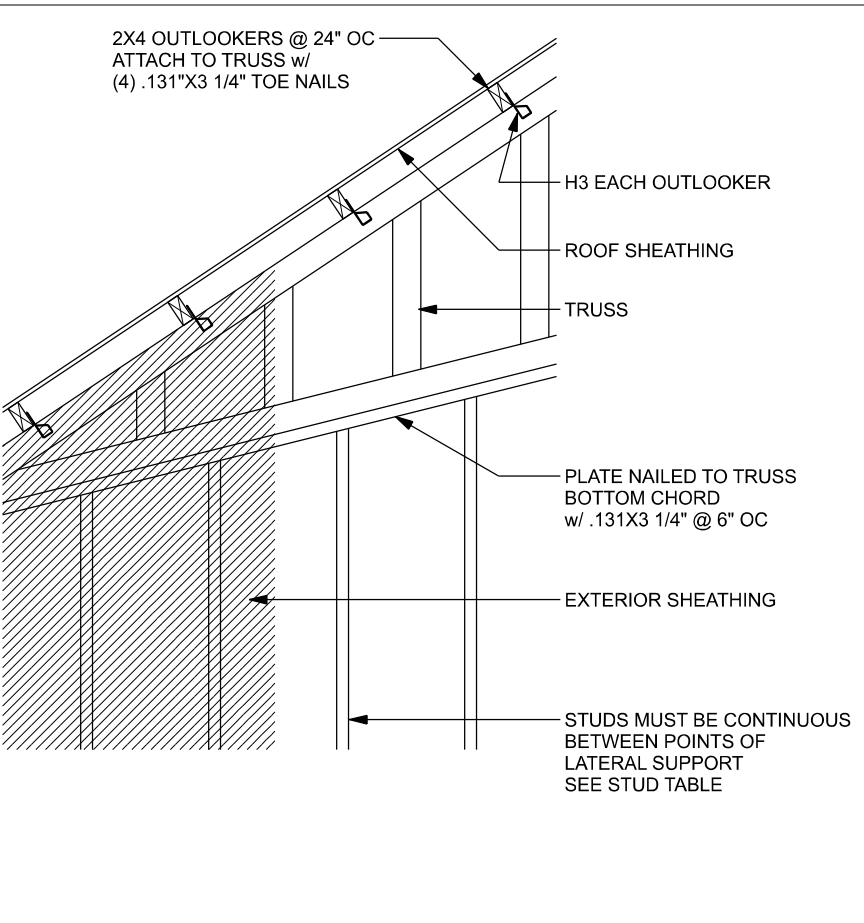
TYPICAL HEADER STRAPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



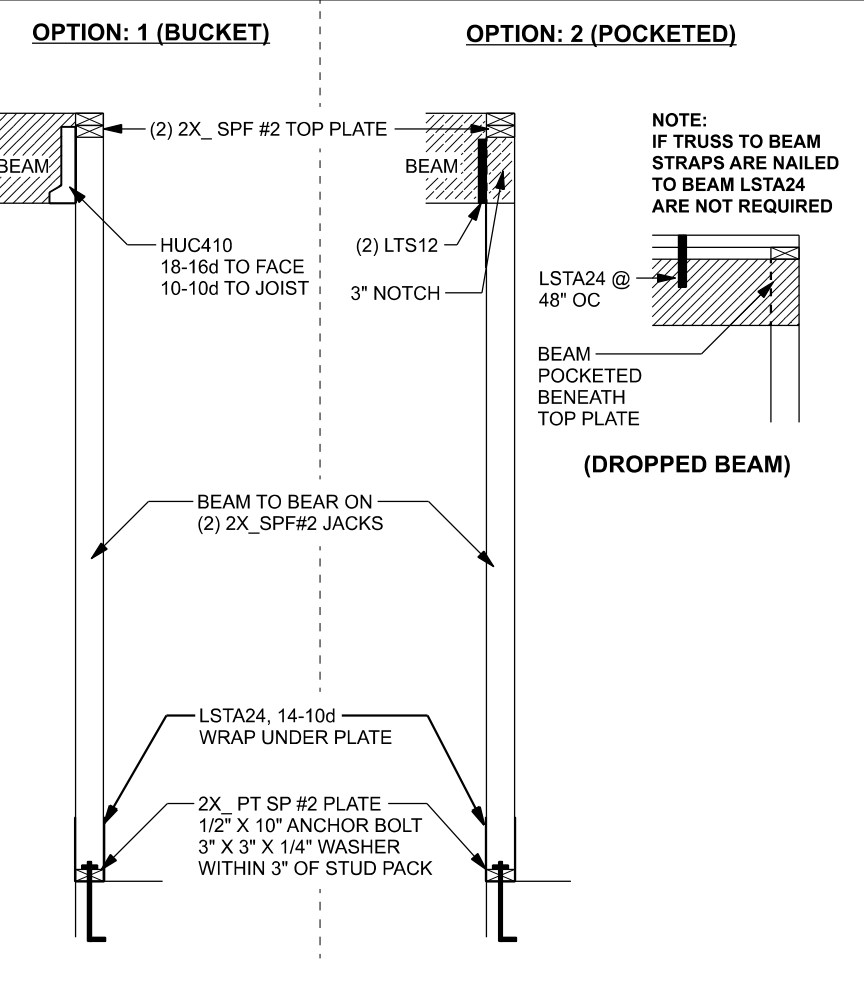
(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



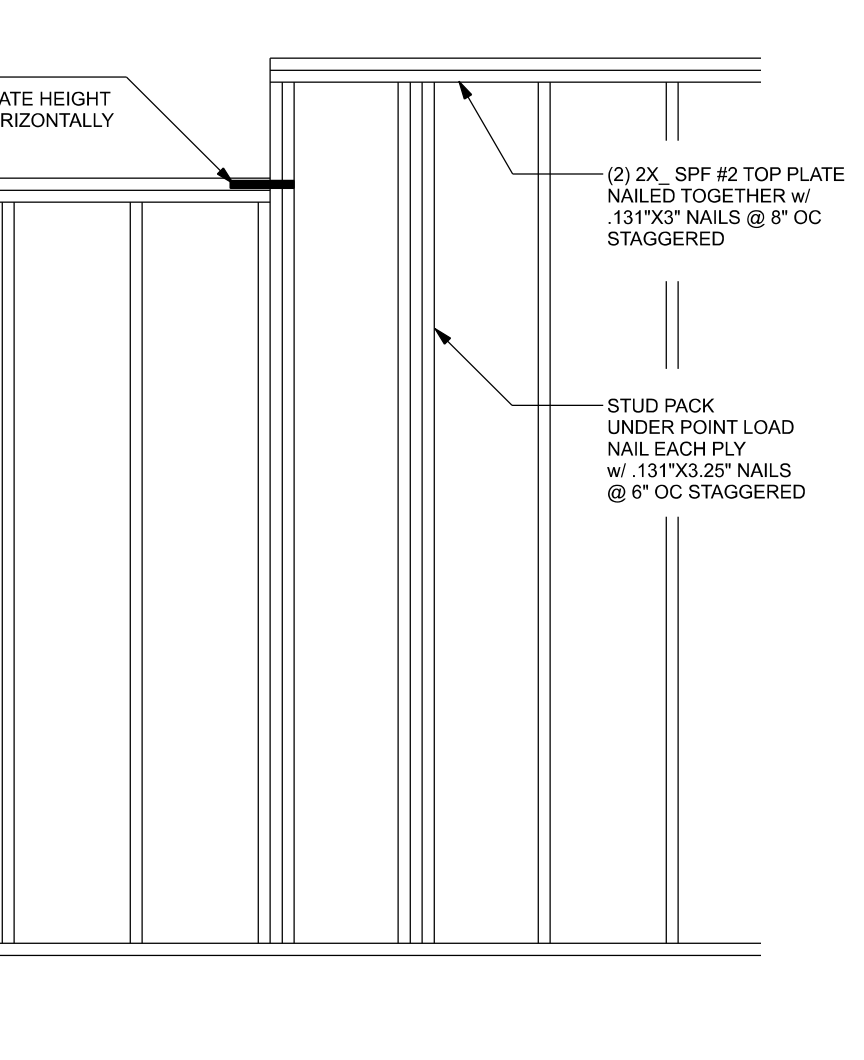
(TYP.) GABLE BRACING DETAIL WOOD FRAME



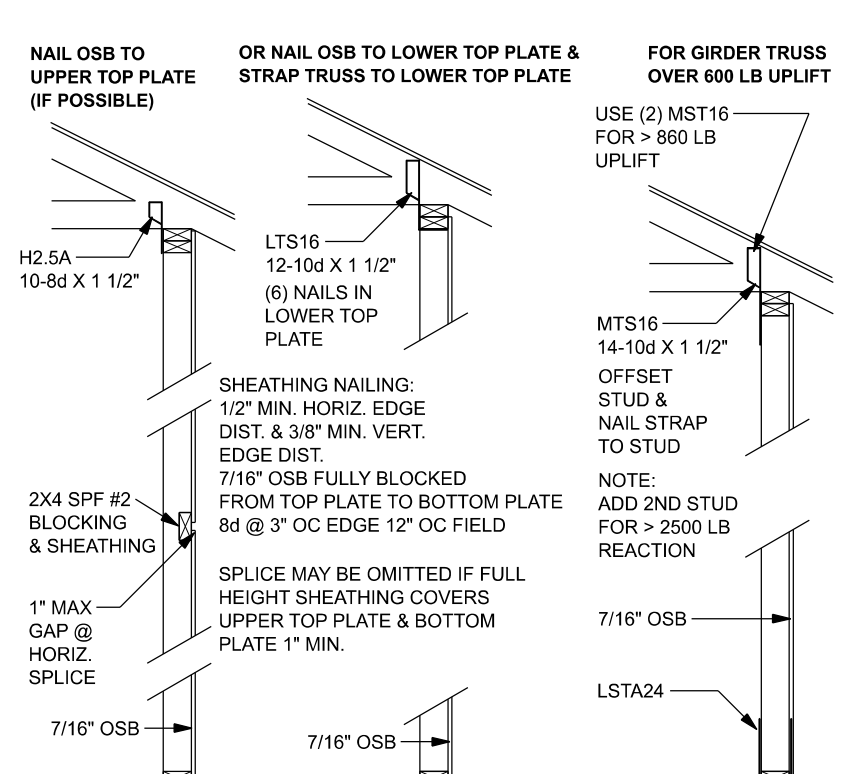
(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME



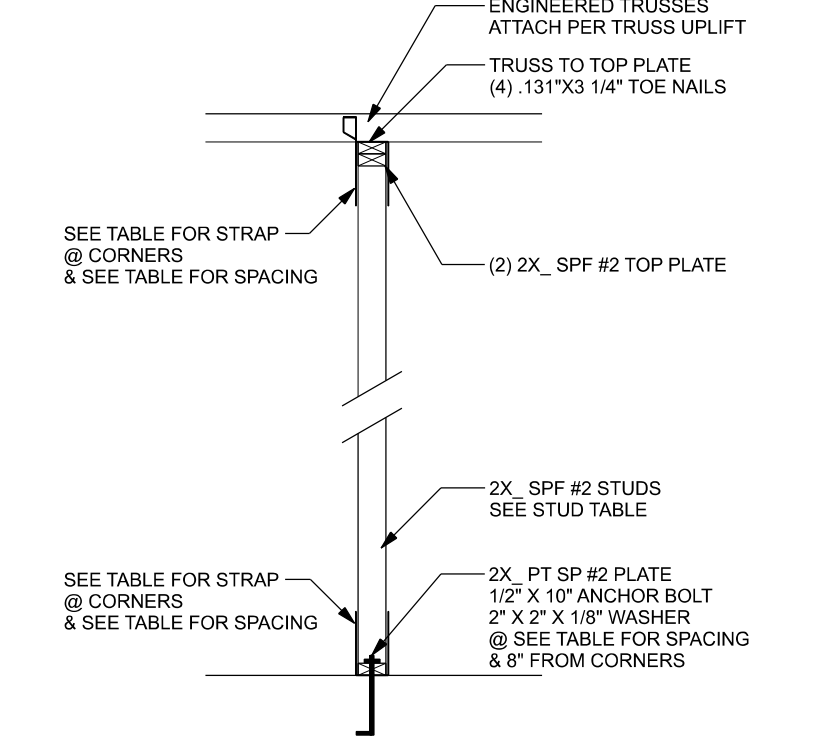
(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



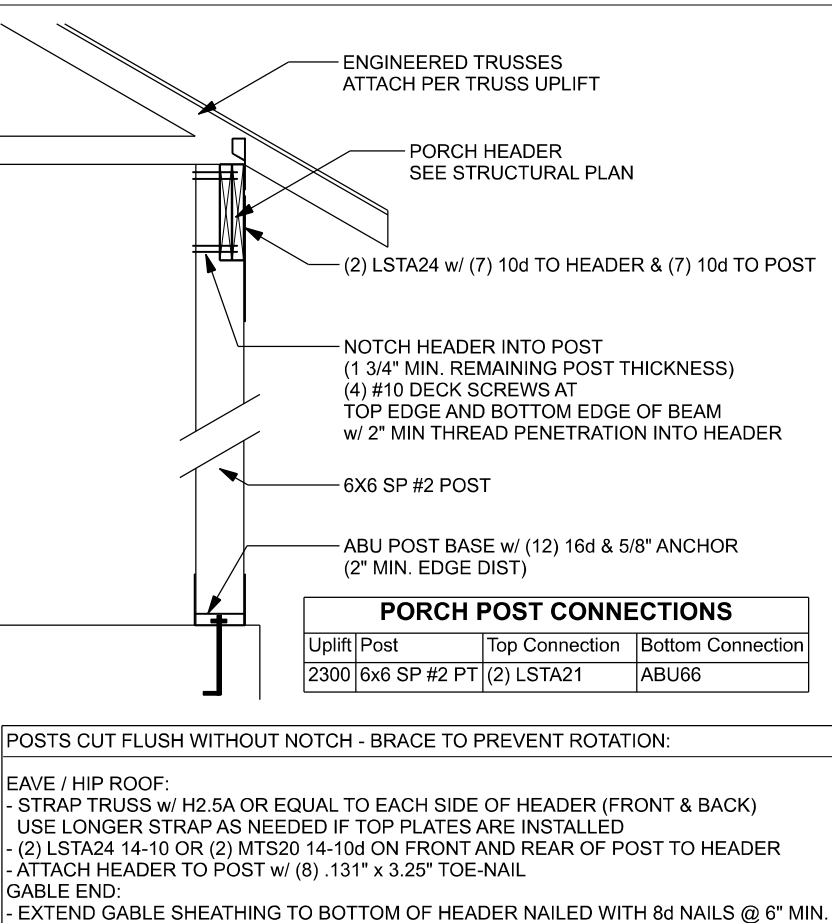
SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP TABLE

Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 pif	LSTA24, 14-10d @ 48" OC Wrap Under Plate	LSTA24, 14-10d @ 48" OC	48" OC
465 pif	LSTA24, 14-10d @ 32" OC Wrap Under Plate	LSTA24, 14-10d @ 32" OC	32" OC

(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST ONE STORY WOOD

CONNECTOR TABLE

Uplift SP	Uplift SPFF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"
575	495	H2-5A	5-8dX1 1/2"	5-8dX1 1/2"
1340	1015	H10A	9-10dX1 1/2"	9-10dX1 1/2"
720	620	LTS12-20	6-10dX1 1/2"	6-10dX1 1/2"
1000	860	MTS12-30	7-10dX1 1/2"	7-10dX1 1/2"
1450	1245	HTS20-30	12-10dX1 1/2"	12-10dX1 1/2"
Uplift SP	Uplift SPFF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPFF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1665	605	SP2	6-10d	6-10d
771	771	LSTA24	19-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPFF	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dX1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPFF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16dX1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPFF	Post Bases @ Stewall	To Post	Anchor
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPFF	Post Bases @ Mono	To Post	Anchor
2300		ABU44	12-16d	5/8"x7" Drill & Epoxy
2200		ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20BS, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACING SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

Stud Spacing	Minimum Stud Height
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

Grade	Species	F _b	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FCBR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FCBR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE DESIGNER ALSO OWES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SHEETS.



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FCBR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, CONNECTIONS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION #15 EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OF SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x 4, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC: W141, CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 10 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT C/W OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 44 KSI, ALL LAP SPICES 40" DB (25" FOR 65 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-06, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS, MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS' INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS WIND, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FCBR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)	
CODE FOR DESIGN LOADS	ASCE 7-16	
WINDLOADS		
BASIC WIND SPEED (BASED 7.10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
RISK CATEGORY (BUILDER MUST FIELD VERIFY)	I	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
MEAN ROOF HEIGHT	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&C DESIGN PRESSURES:	SEE TABLE	
FLOOR LOADING		
ROOFS OTHER THAN SLEEPING ROOMS	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	
COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (F _T)	ZONE 4 INTERIOR	ZONE 5 END OF FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vs/d) -27.8(Vs/d)	+25.6(Vs/d) -34.2(Vs/d)
0 - 20	+42.6(Vs/d) -46.2(Vs/d)	+42.6(Vs/d) -57(Vs/d)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(Vs/d) -25.5(Vs/d)	
16x7 GARAGE DOOR	+21.7(Vs/d) -24.1(Vs/d)	

Amira Builders
Van Duys Res.
PROJECT ADDRESS:
307 SW Station Ghin
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

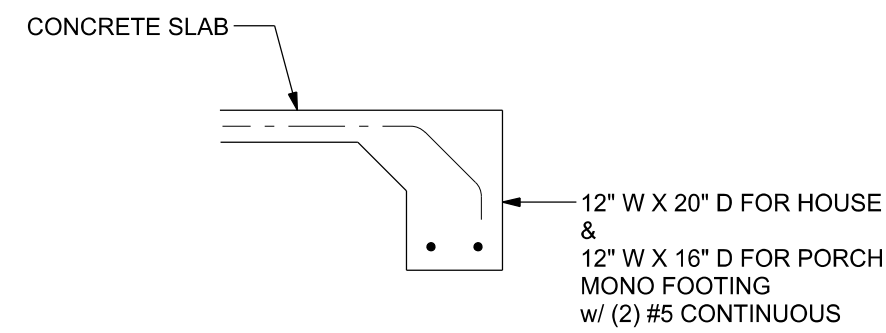
MARK DISOWAY P.E. 53915

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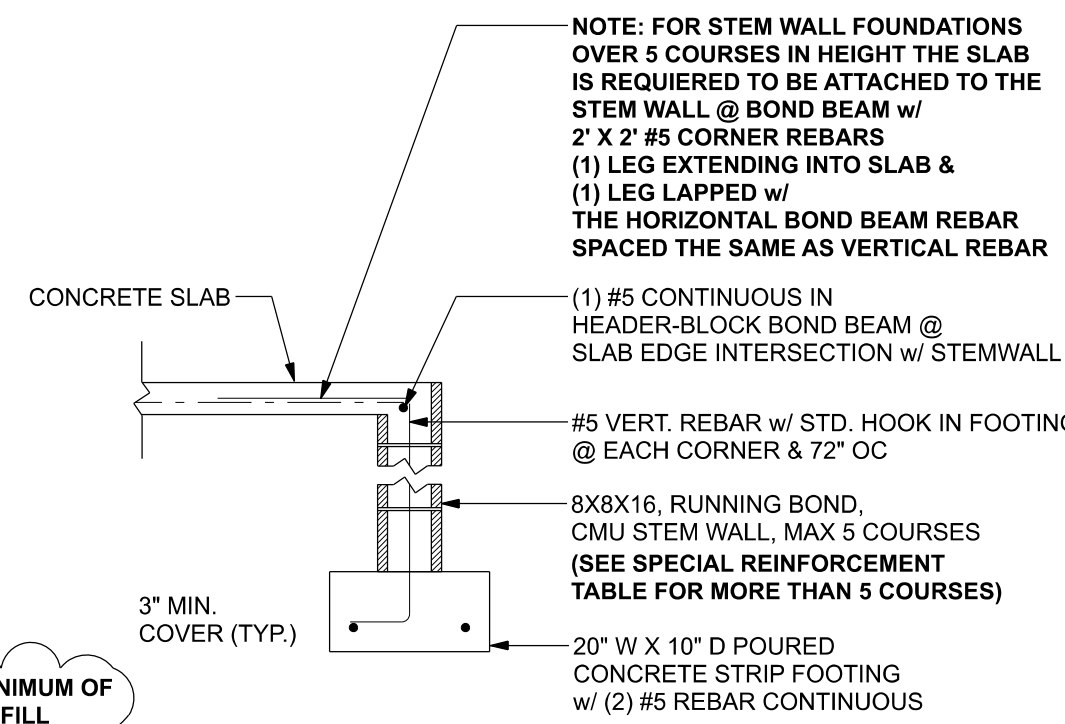
Wednesday, February 3, 2021

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JOB NUMBER:
210147
S-1
OF 3 SHEETS



F1 S-2 MONOLITHIC FOOTING (OPTIONAL)
SCALE: 1/2" = 1'-0"



F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

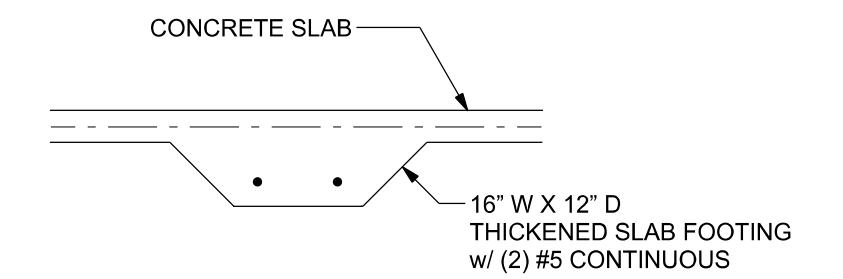
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

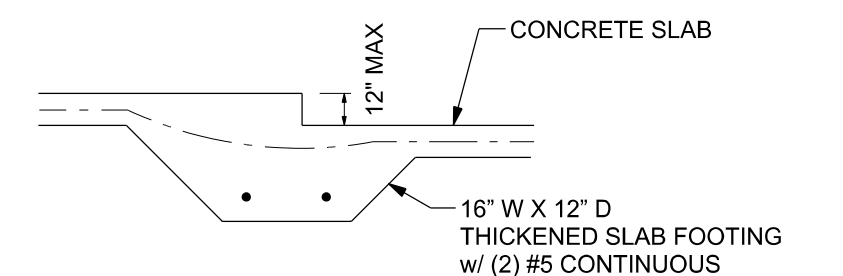
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)	
		#5	#7	#5	#8
3.3	3.0	96	96	96	96
4.0	3.7	96	96	96	96
4.7	4.3	88	96	96	96
5.3	5.0	56	96	96	96
6.0	5.7	40	96	80	96
6.7	6.3	32	56	80	96
7.3	7.0	24	40	56	80
8.0	7.7	16	32	48	64
8.7	8.3	8	24	32	48
9.3	9.0	8	16	24	40

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

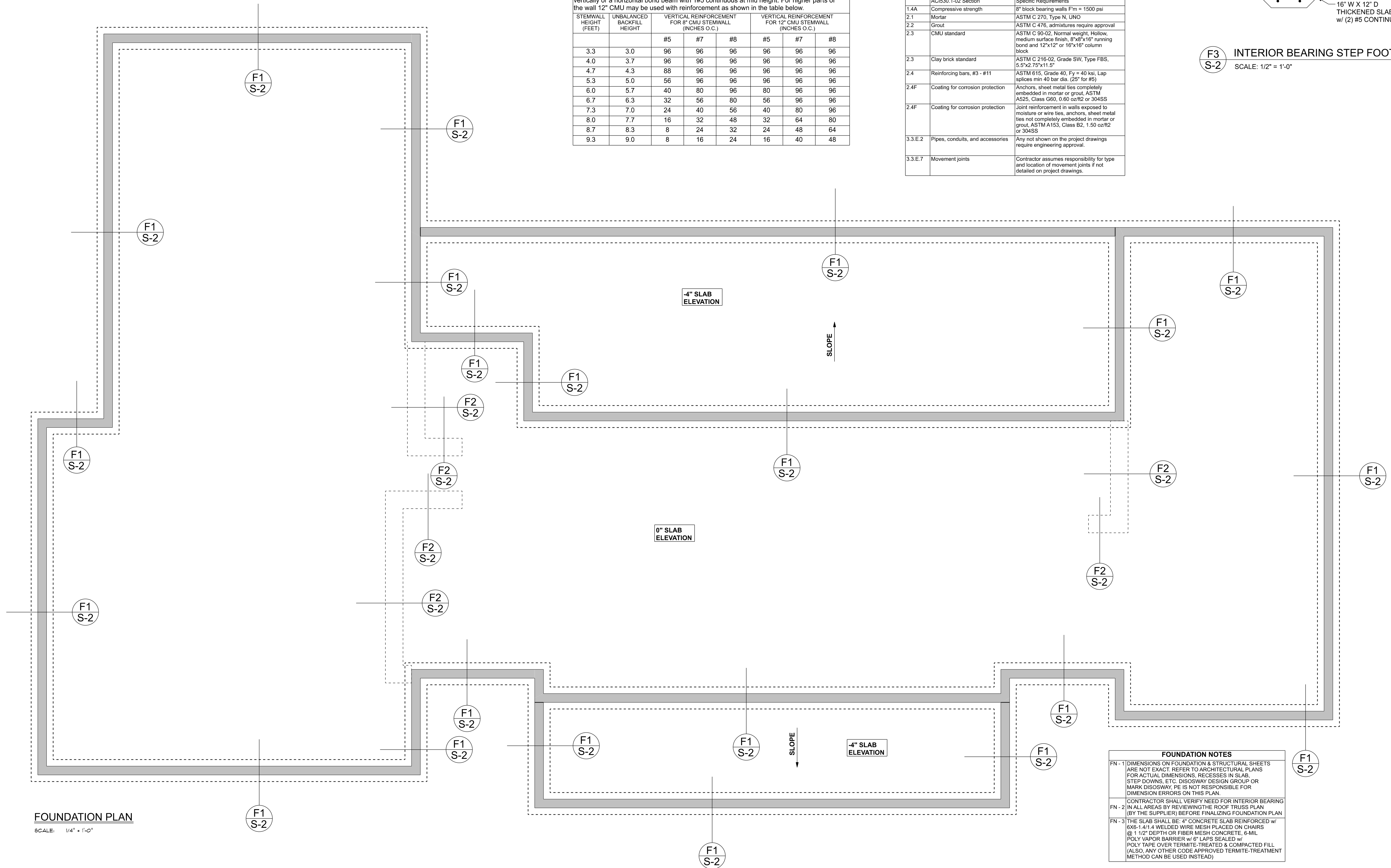
ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN-2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN-3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ #6-14" x WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES-TREATMENT METHOD CAN BE USED INSTEAD)

Amira Builders
Van Duys Res.
PROJECT ADDRESS:
307 SW Station Glen
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Wednesday, February 3, 2021

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JOB NUMBER:
210147
S-2
OF 3 SHEETS

