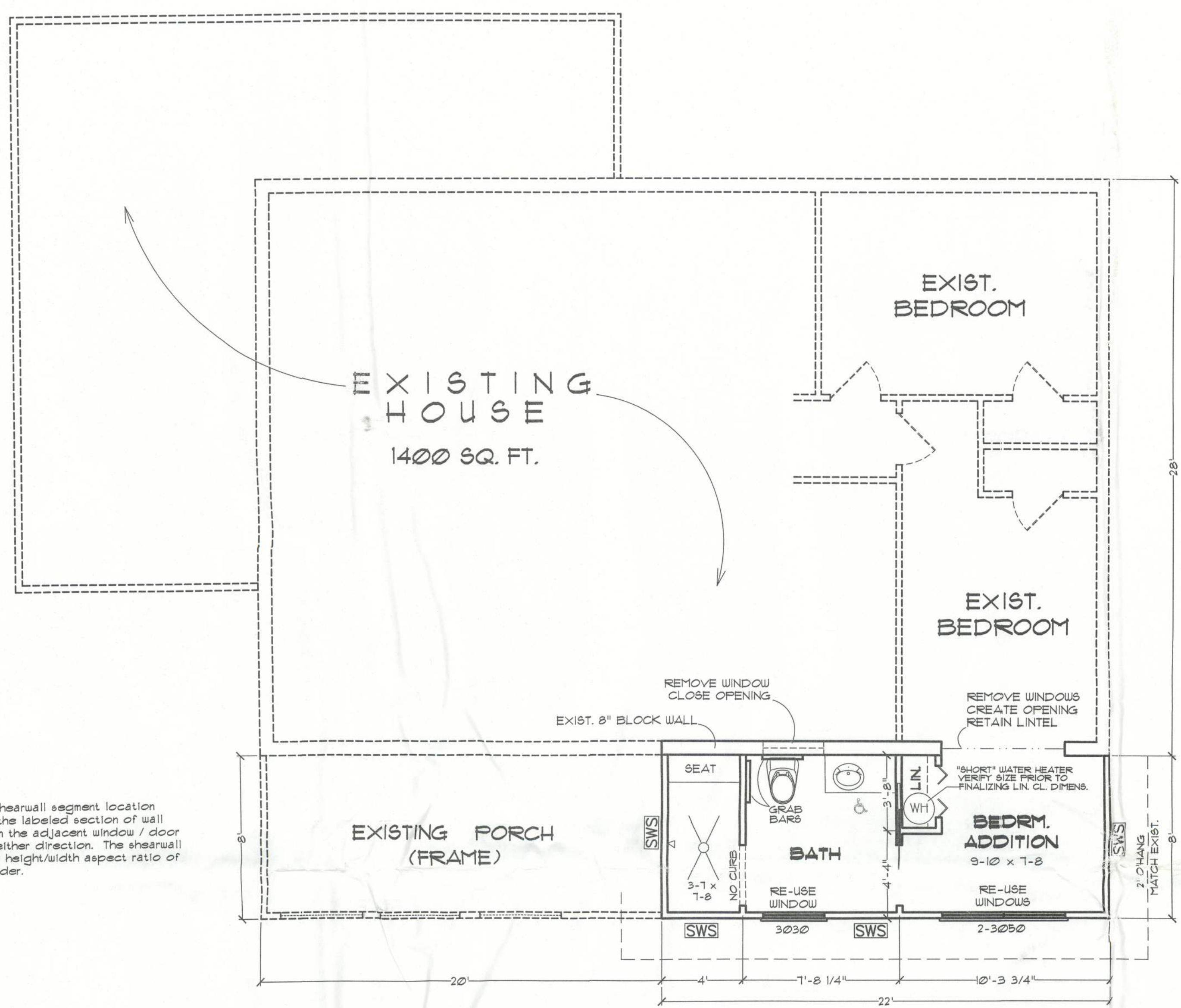
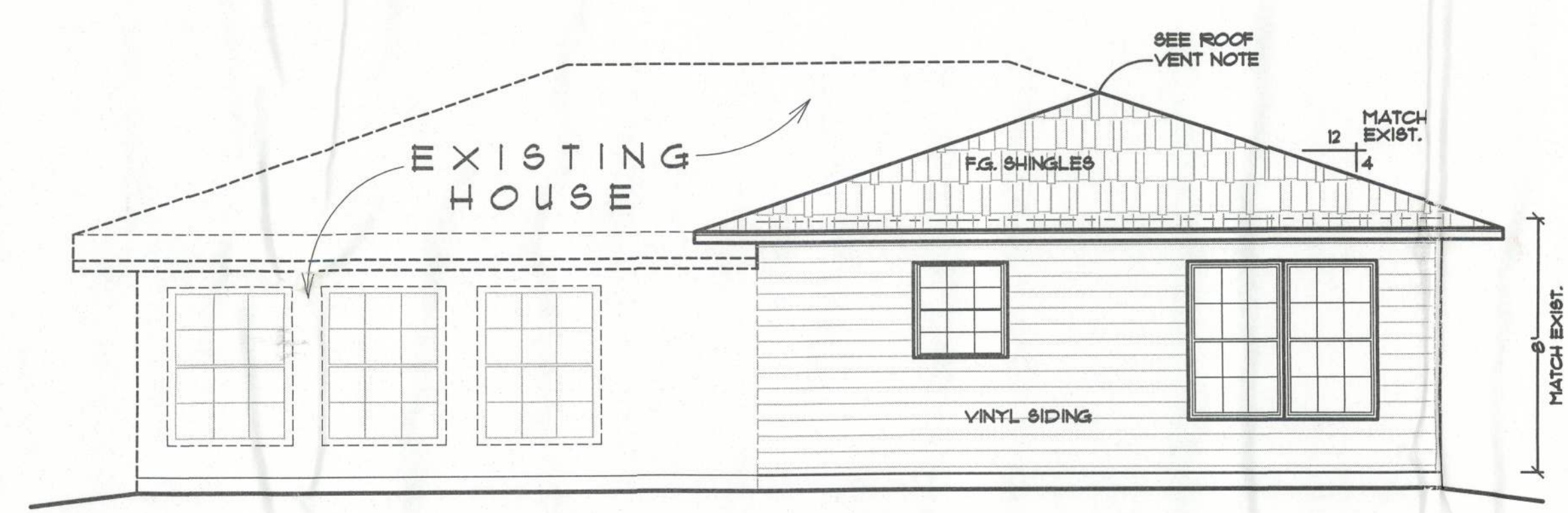


# Delgado Addition

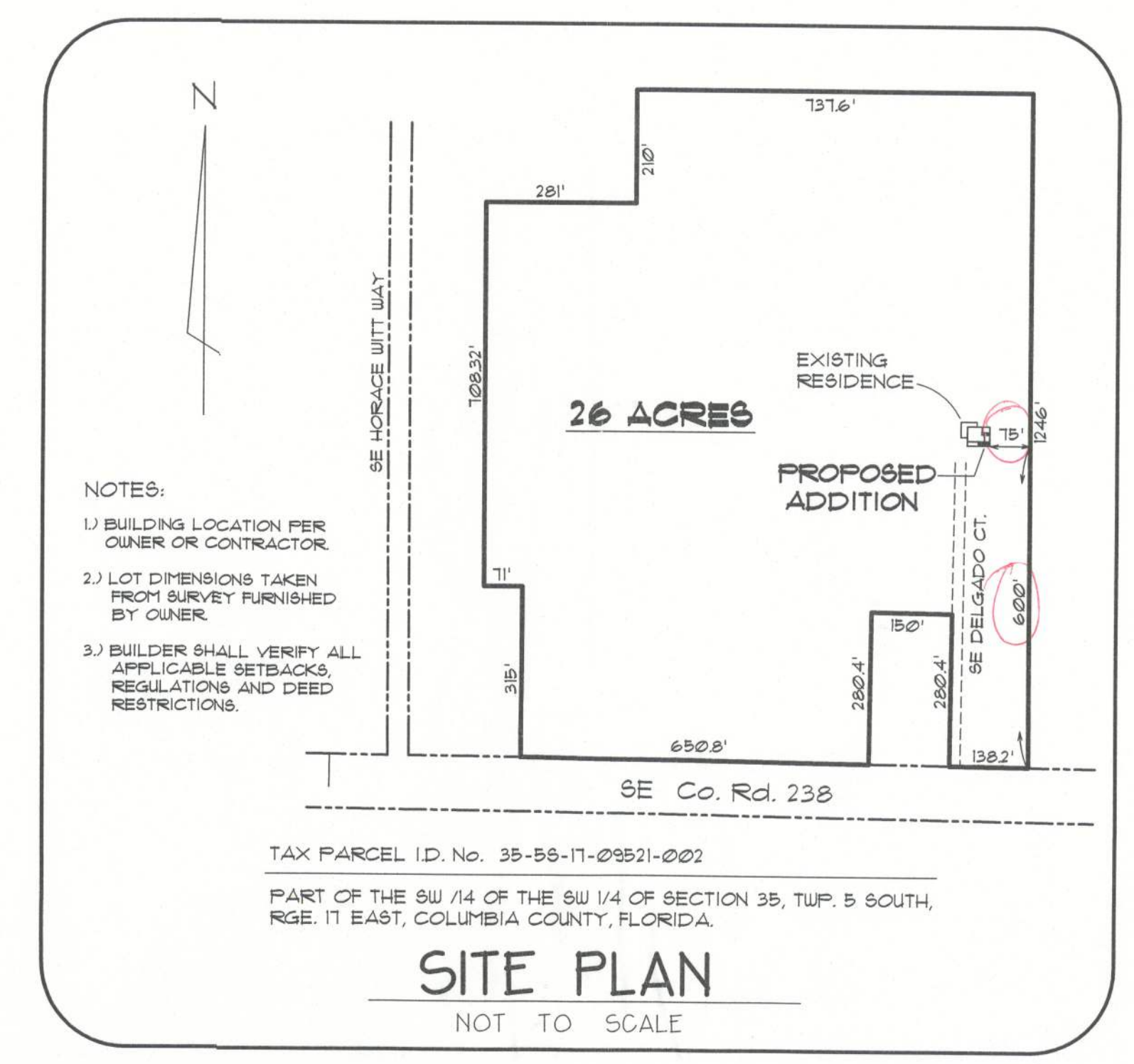
SWS • Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.



**FLOOR PLAN**  
SCALE: 1/4 IN. = 1 FT.



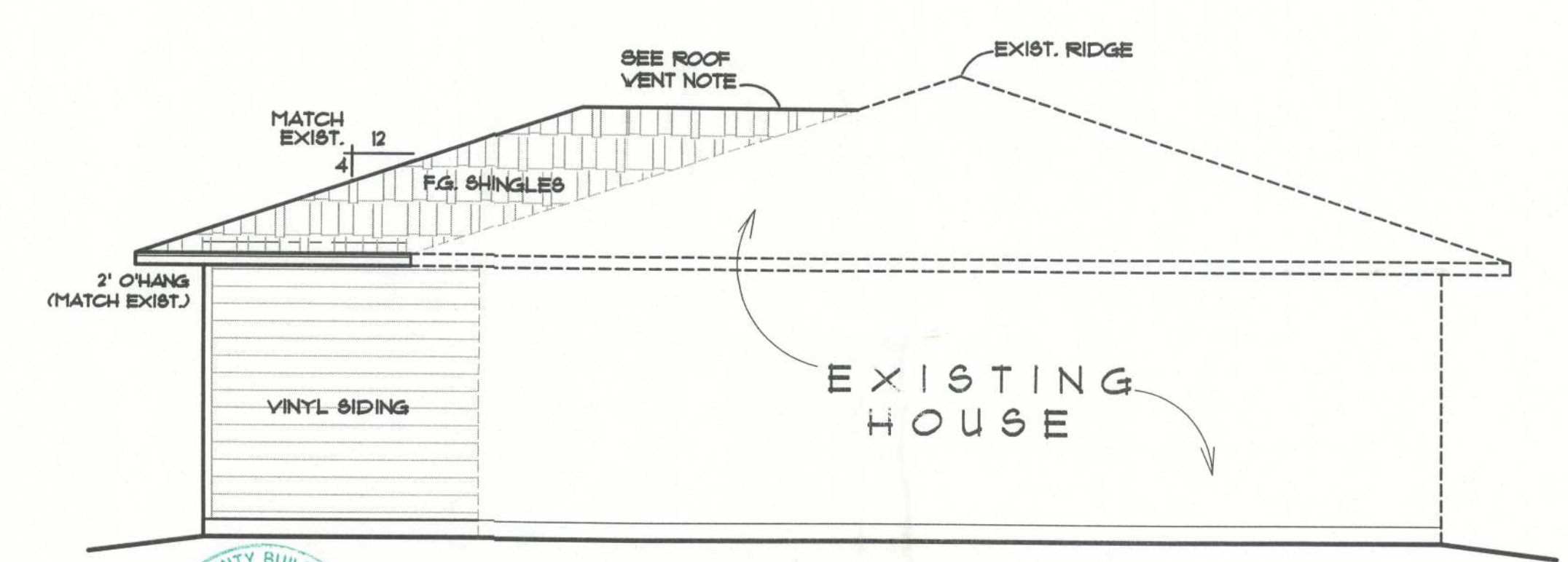
**FRONT ELEVATION**  
SCALE: 1/4 IN. = 1 FT.



## ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with a 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.



**SIDE ELEVATION**  
SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2010, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

238 SE DELGADO CT  
Location: LAKE CITY, FL 32055 Job No.:

1

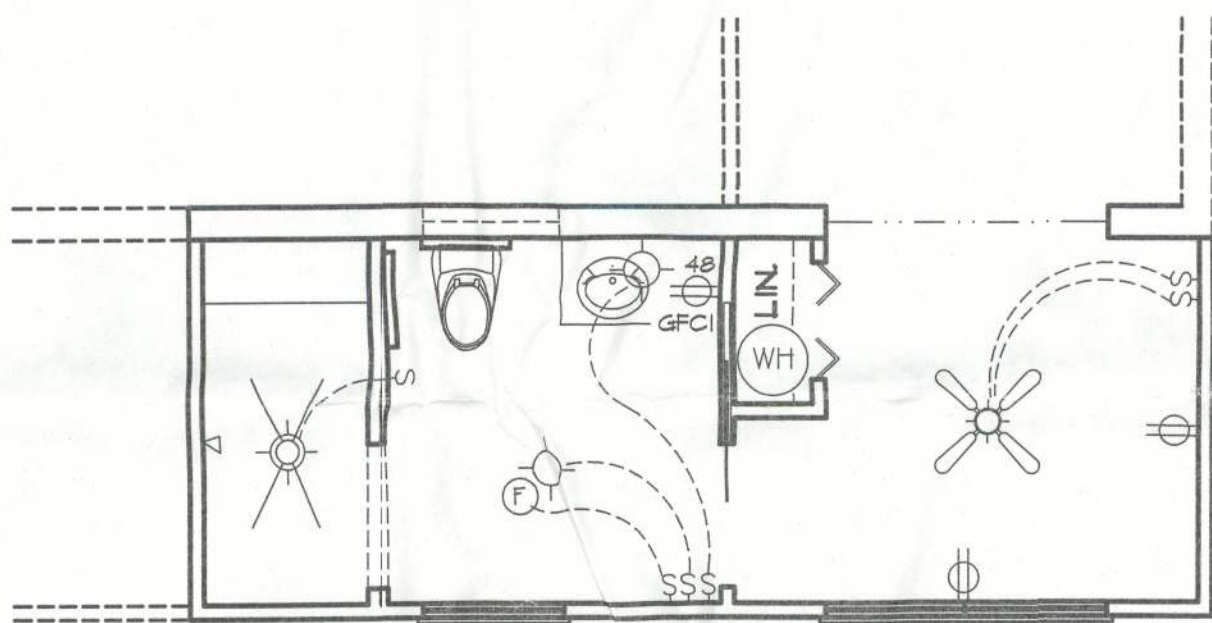


FILE: 13-003	<b>DELGADO ADDITION</b>	SHEET: 1 OF 2
DATE: 2-9-13		CAD FILE: 13-003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting Technical Services	REV:
CHECK: T A D	192 SW Sagewood Cn. Lake City, FL 32024 Phone (386) 755-5891	REV:



GENERAL NOTES

- 1.) See "Wind Load Detail Sheet S-1" and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) Roof Truss design is the responsibility of the supplier.
- 4.) The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 5.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 6.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.
- 7.) HVAC duct system to be expanded to serve addition.



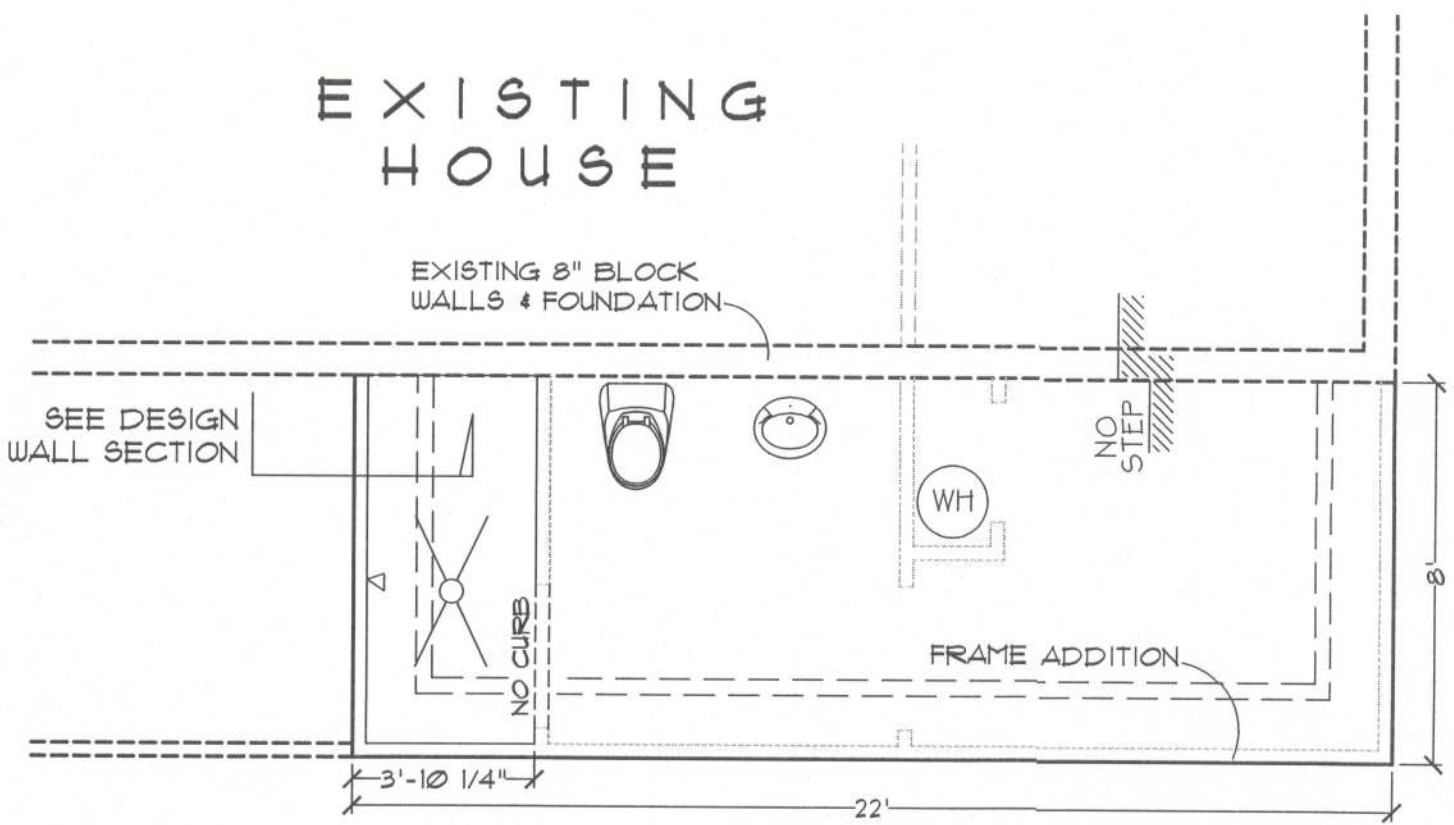
ELECTRICAL SYMBOLS	
	• STANDARD CEILING LIGHTING FIXTURE OR CHANDELIER
	• FAN LOCATION (CEILING)
	• FAN LOCATION (EXHAUST)
	• 110V DUPLEX OUTLET AFCI, UNLESS NOTED
	• 110V DUPLEX OUTLET AFCI, UNLESS NOTED (SPECIAL HEIGHT NOTED)
	• 110V DUPLEX OUTLET GROUND FAULT CIRCUIT INTERRUPTER TYPE

ELECTRICAL PLAN NOTES

- ALL INSTALLATIONS ARE PER NAT'L. ELECTRIC CODE (NEC) 2008.
- ALL RECEPTACLES, UNLESS NOTED OTHERWISE, SHALL BE ARC FAULT CIRCUIT INTERRUPTER (AFCI) TYPE. ALSO, RECEPTACLES, UNLESS NOTED, SHALL BE TAMPER RESISTANT.
- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 2008.
- THE ADDITION SHALL UTILIZE EXPANSION OF CURRENT ELECTRICAL SERVICE AND CIRCUIT BOX. ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING SUITABILITY OF CURRENT SERVICE.

ELECTRICAL PLAN

NOT TO SCALE



FOUNDATION PLAN

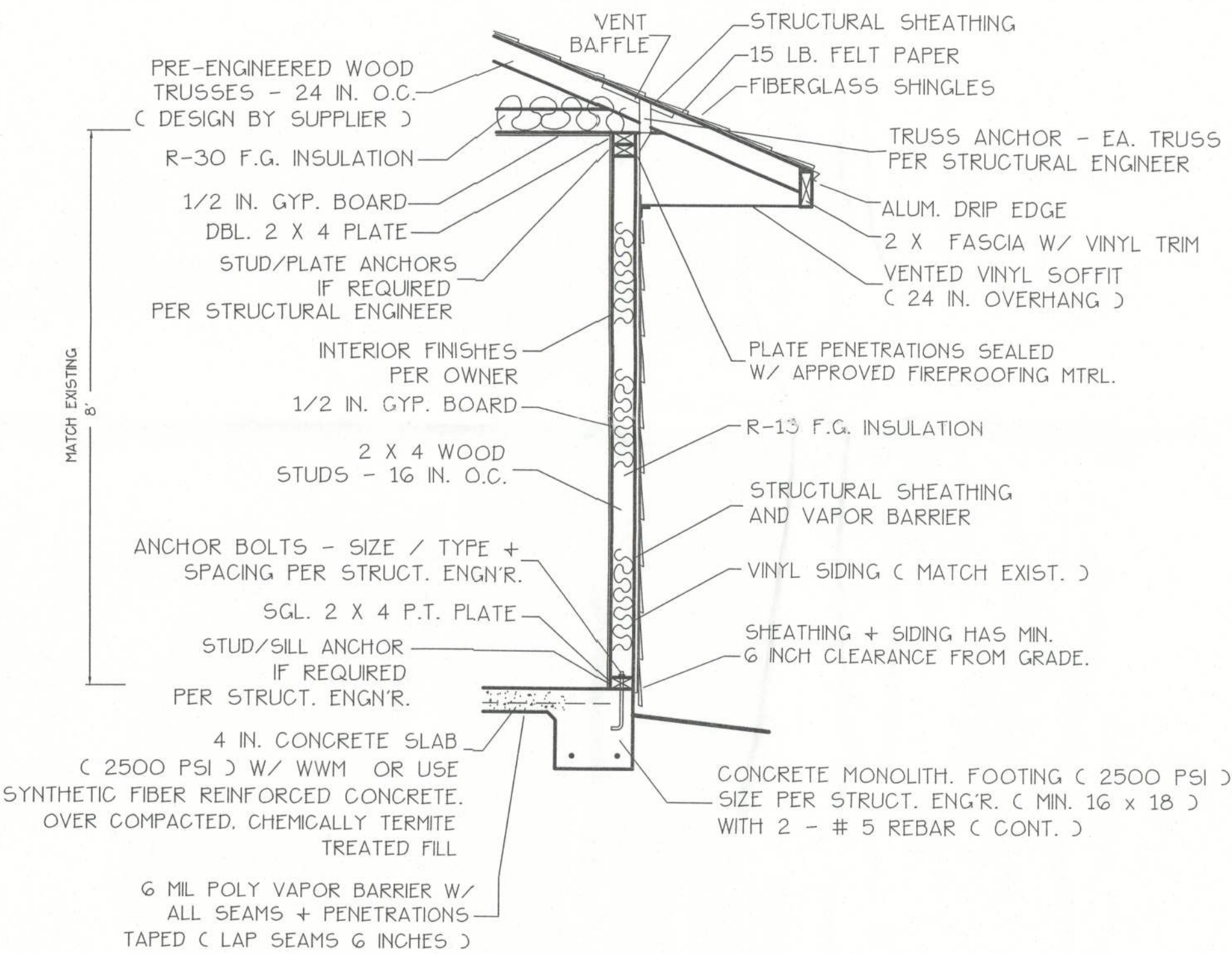
SCALE: 1/4 IN. = 1 FT.

FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

SLAB IS 4 INCH CONC. (2500 PSI) W/ 6 MIL POLY VAPOR BARRIER (LAPPED 6 IN. MIN & SEALED) OVER STABILIZED FILL (CHEMICALLY TERMITE TREATED)

- SLAB OPTIONS:
  - A) USE 10/10 6x6 WWM REINFORCING ON CHAIRS AT 3' O.C.
  - B) USE SYNTH. FIBER REINF. CONCRETE



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 1/2 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2010, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

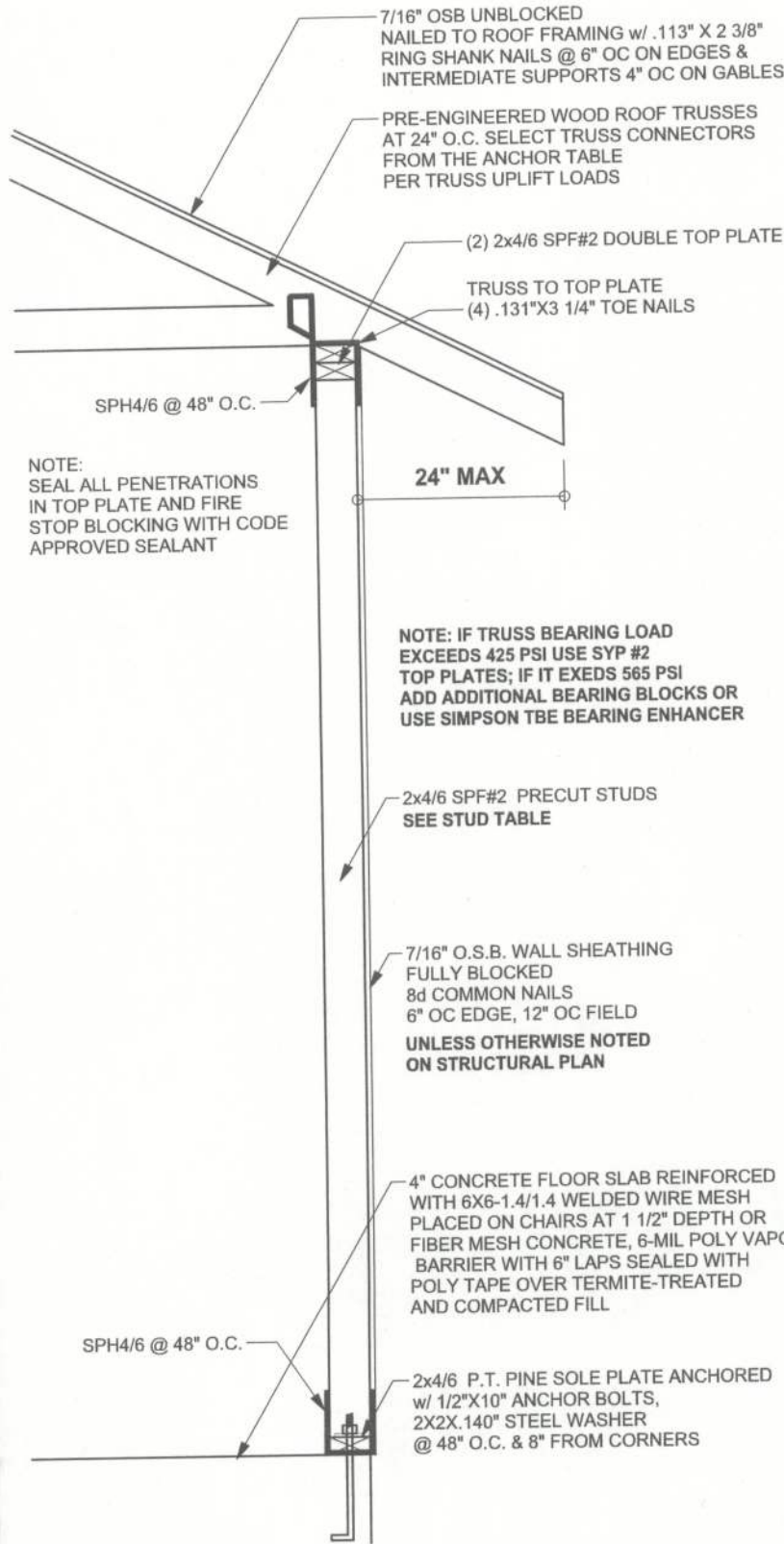
239 SE DELGADO CT  
LAKE CITY, FL 32055

Job No.:

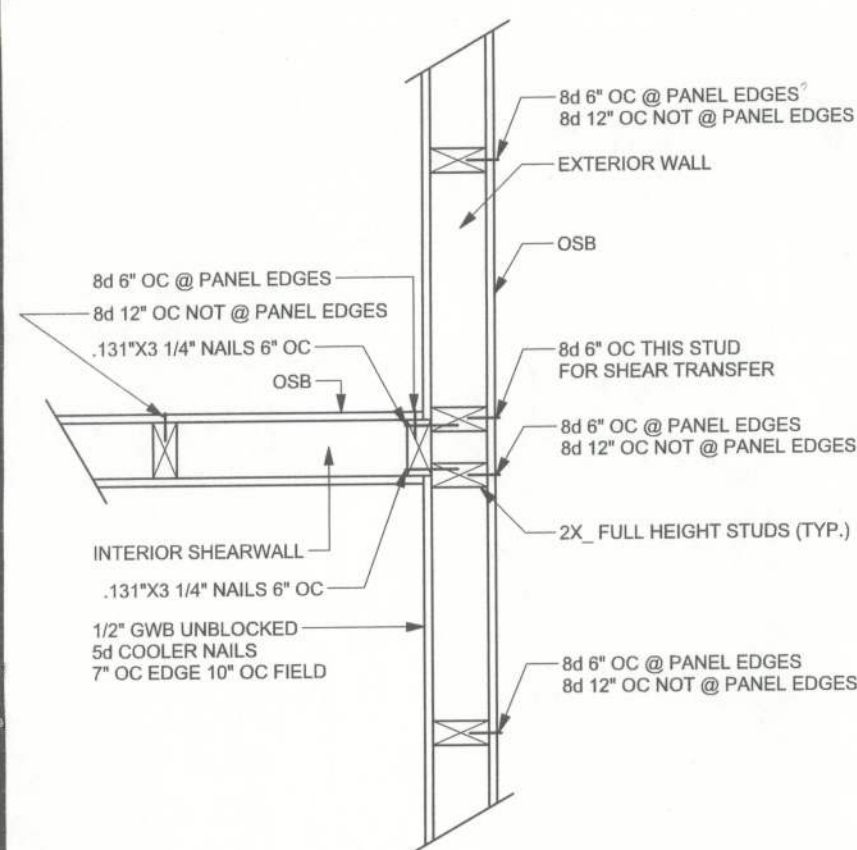
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FILE: 13-003	DELGADO ADDITION	SHEET: 2 OF 2
DATE: 2-9-13		CAD FILE: 13-003
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Segewood Gln. Lake City, FL 32024 Phone (386) 755-5891	REV:
CHECK: T A D		REV:

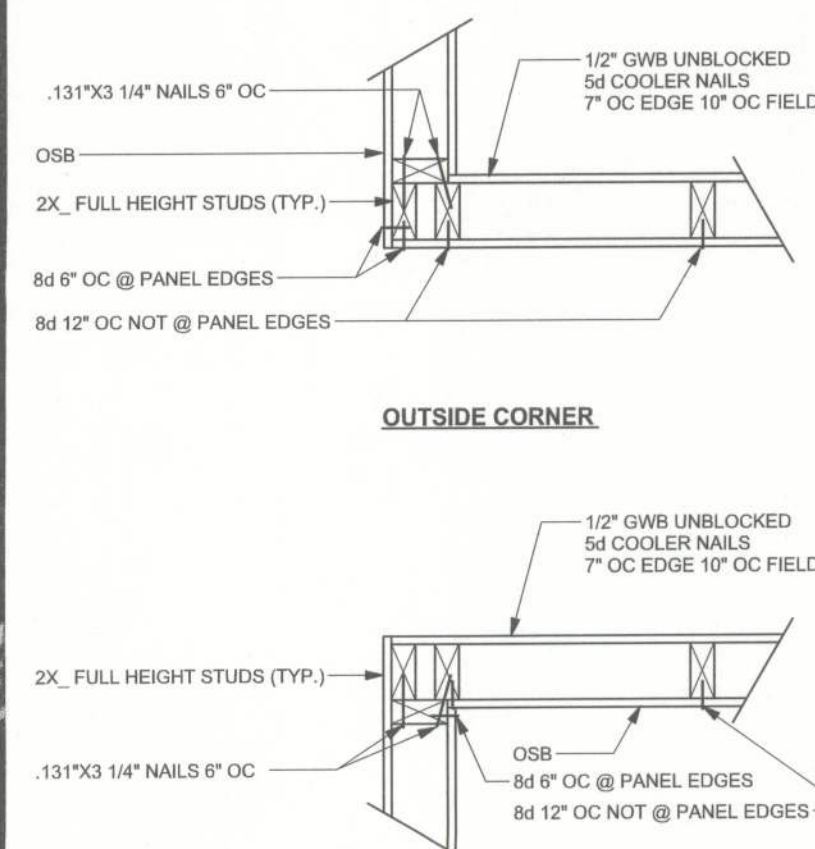




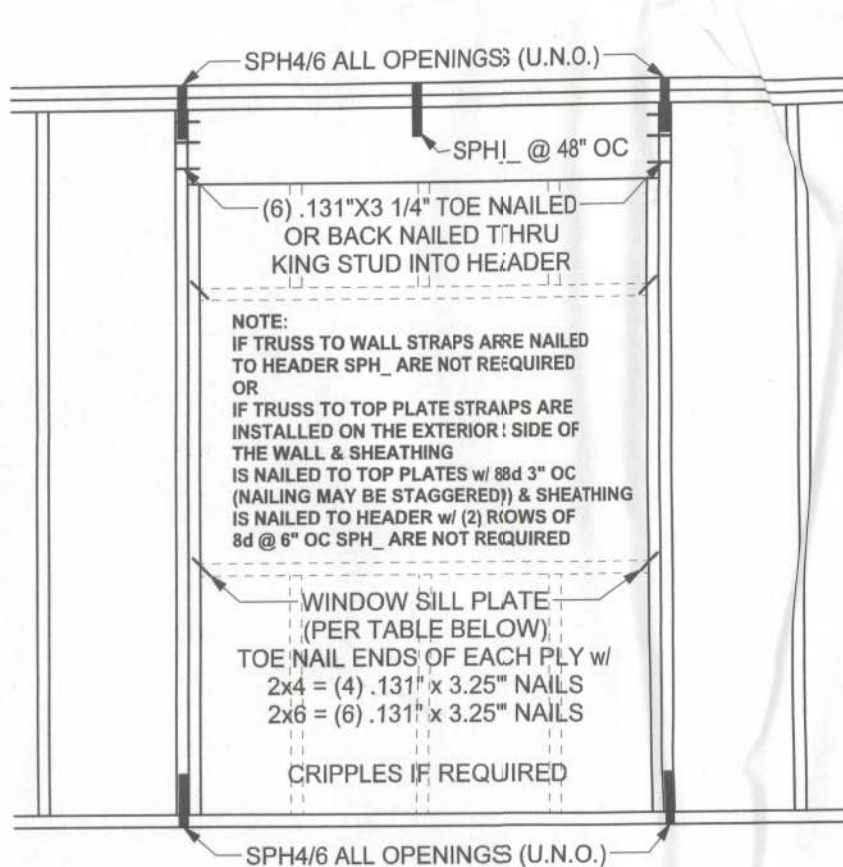
**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



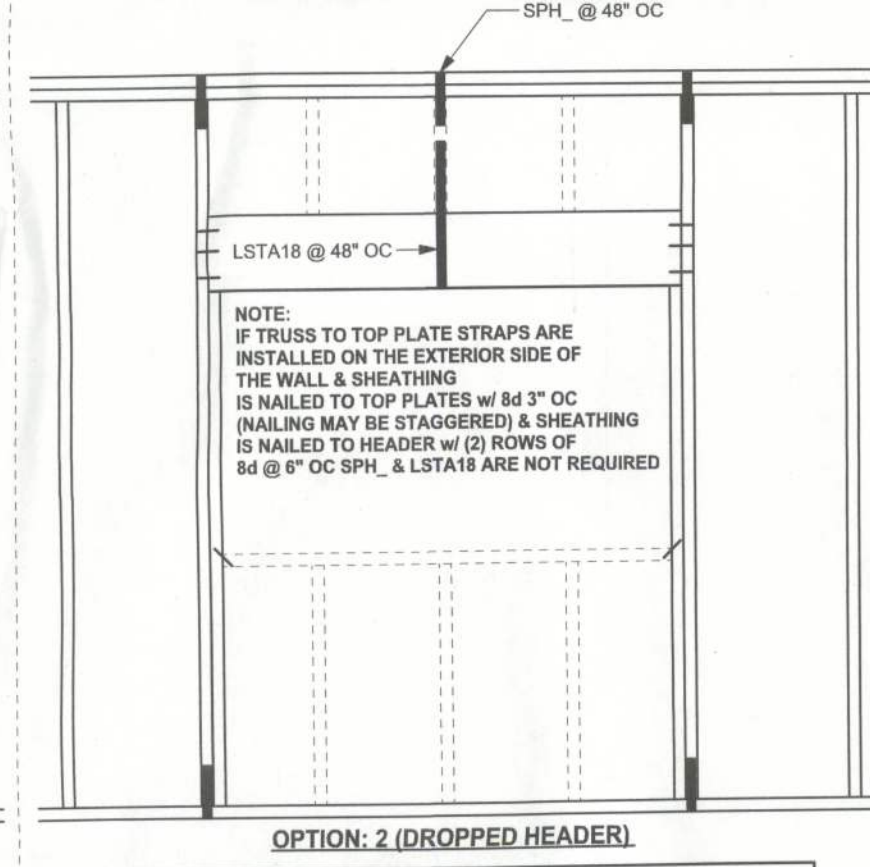
**(TYP.) INTERSECTING WALL FRAMING WOOD FRAME**



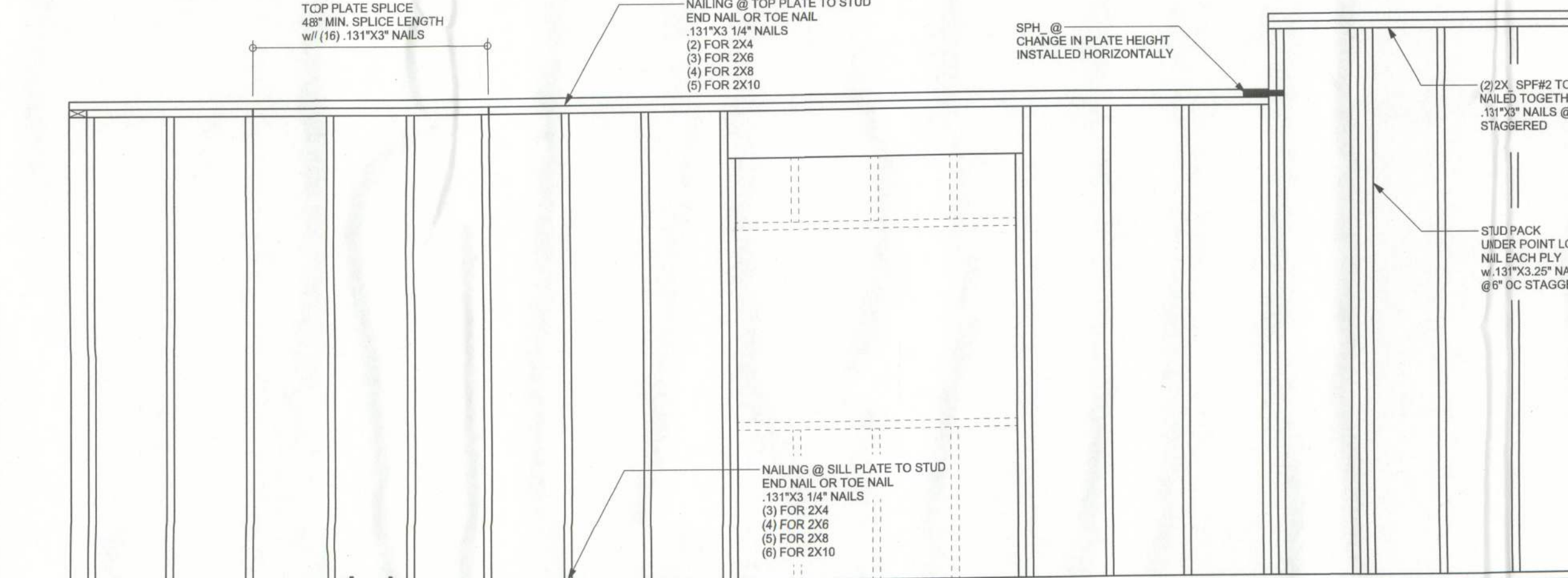
**(TYP.) CORNER FRAMING WOOD FRAME**



**TYPICAL HEADER STRAPPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN	MAX. SPANS FOR SPF #2			
WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
115-130 MPH	5'-3"	7'-0"	7'-0"	11'-4"
140-150 MPH	4'-4"	6'-0"	6'-5"	9'-0"
160 MPH	4'-0"	6'-0"	5'-11"	8'-0"



**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLES 20B4, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" CYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C. STUD DEFLECTION LIMIT H240 (NOT OK FOR SOME BUTTE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

## GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

## GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION @ 15LB EACH END, 2X6 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W1.4 x W1.4, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, UNO.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2400$  psi,  $E = 1800000$  psi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/64"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN COMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 850	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2000	< 2400	2 - HTS24			
< 2050	< 1785	LG2	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	DSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	DSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA16	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*			TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H20A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1																			
ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS.																			
MEAN ROOF HEIGHT																			
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE																			
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION																			
1) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)																			
2) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY																			
3) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY																			
4) RISK CATEGORY = II, (MRI = 700 YR)																			
5) ROOF ANGLE = 7.45 DEGREES																			
6) MEAN ROOF HEIGHT = <30 FT																			
7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)																			
8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))																			
<div> <table border="1"> <thead> <tr> <th>Zone</th><th>Effective Wind Area (ft<sup>2</sup>)</th></tr> </thead> <tbody> <tr> <td>1</td><td>39 -43</td></tr> <tr> <td>2</td><td>39 -68</td></tr> <tr> <td>3</td><td>39 -100</td></tr> <tr> <td>4</td><td>43 -46</td></tr> <tr> <td>5</td><td>43 -57</td></tr> </tbody> </table> </div> <div> <table border="1"> <thead> <tr> <th>Garage Door</th><th>2010 FBCR Table R301.2(4)</th></tr> </thead> <tbody> <tr> <td>8x7 Garage Door</td><td>37 -42</td></tr> <tr> <td>16x7 Garage Door</td><td>36 -40</td></tr> </tbody> </table> </div>		Zone	Effective Wind Area (ft <sup>2</sup> )	1	39 -43	2	39 -68	3	39 -100	4	43 -46	5	43 -57	Garage Door	2010 FBCR Table R301.2(4)	8x7 Garage Door	37 -42	16x7 Garage Door	36 -40
Zone	Effective Wind Area (ft <sup>2</sup> )																		
1	39 -43																		
2	39 -68																		
3	39 -100																		
4	43 -46																		
5	43 -57																		
Garage Door	2010 FBCR Table R301.2(4)																		
8x7 Garage Door	37 -42																		
16x7 Garage Door	36 -40																		
DESIGN LOADS																			
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)																			
30 PSF (SLEEPING ROOMS)																			
30 PSF (ATTICS WITH STORAGE)																			
10 PSF (ATTICS WITHOUT STORAGE, <3.12)																			
ROOF 20 PSF (FLAT OR <4.12)																			
16 PSF (4.12 TO <12.12)																			
12 PSF (12.12 AND GREATER)																			
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)																			
SOIL BEARING CAPACITY 1500 PSF																			
NOT IN FLOOD ZONE (BUILDER TO VERIFY)																			

## REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

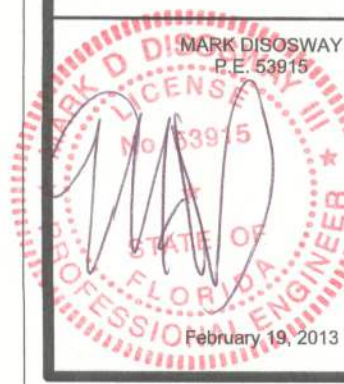
WINDLOAD ENGINEER: Mark Disoway,  
PE No.53915, POB 866, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**LIMITATION:** This design is valid for one building, at specified location.



## Blake Construction

### Delgado Addition

ADDRESS:  
239 SE Delgado Ct  
Lake City, FL 32055

Mark Disoway P.E.  
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Lake City, Florida 32056  
Phone: (386) 754 - 5419  
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PRINTED DATE:  
February 19, 2013

DRAWN BY: STRUCTURAL BY:

FINALS DATE:  
19Feb12

JOB NUMBER:  
1302033

DRAWING NUMBER

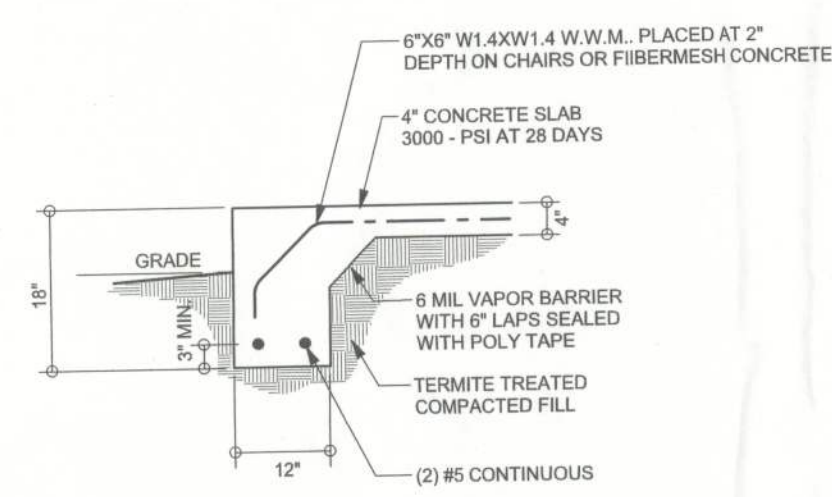
S-1

OF 2 SHEETS

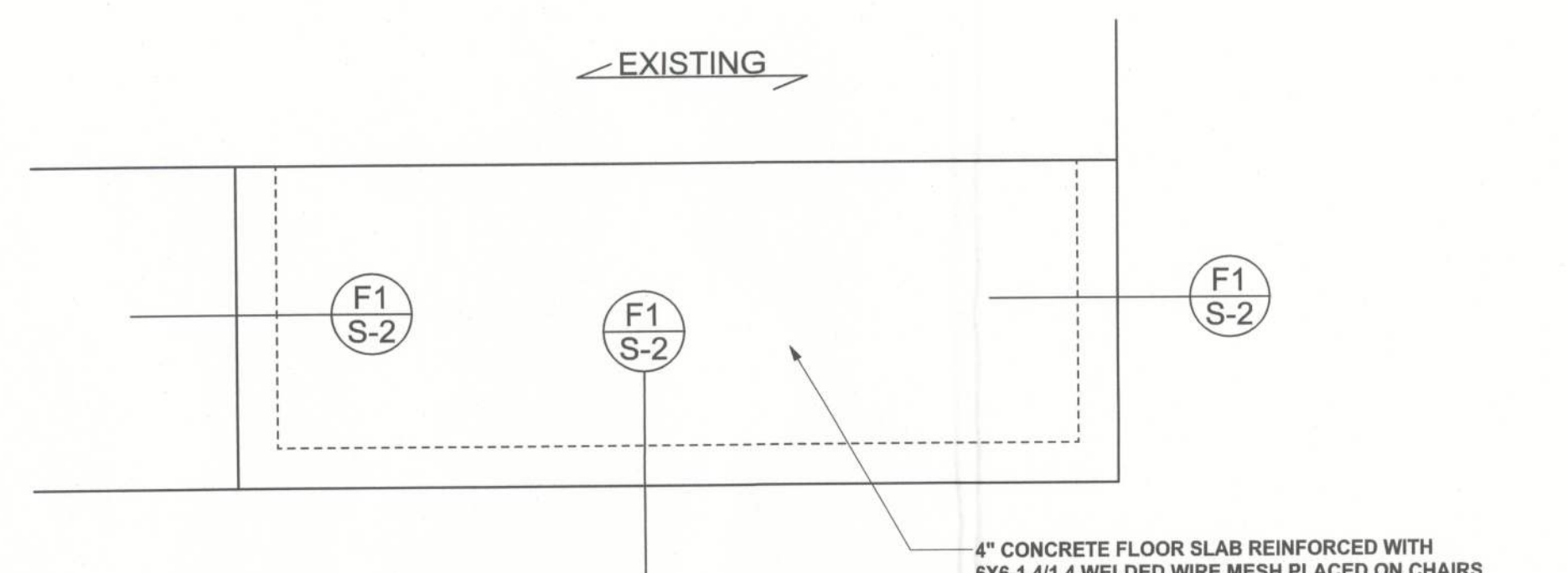


REVISIONS	

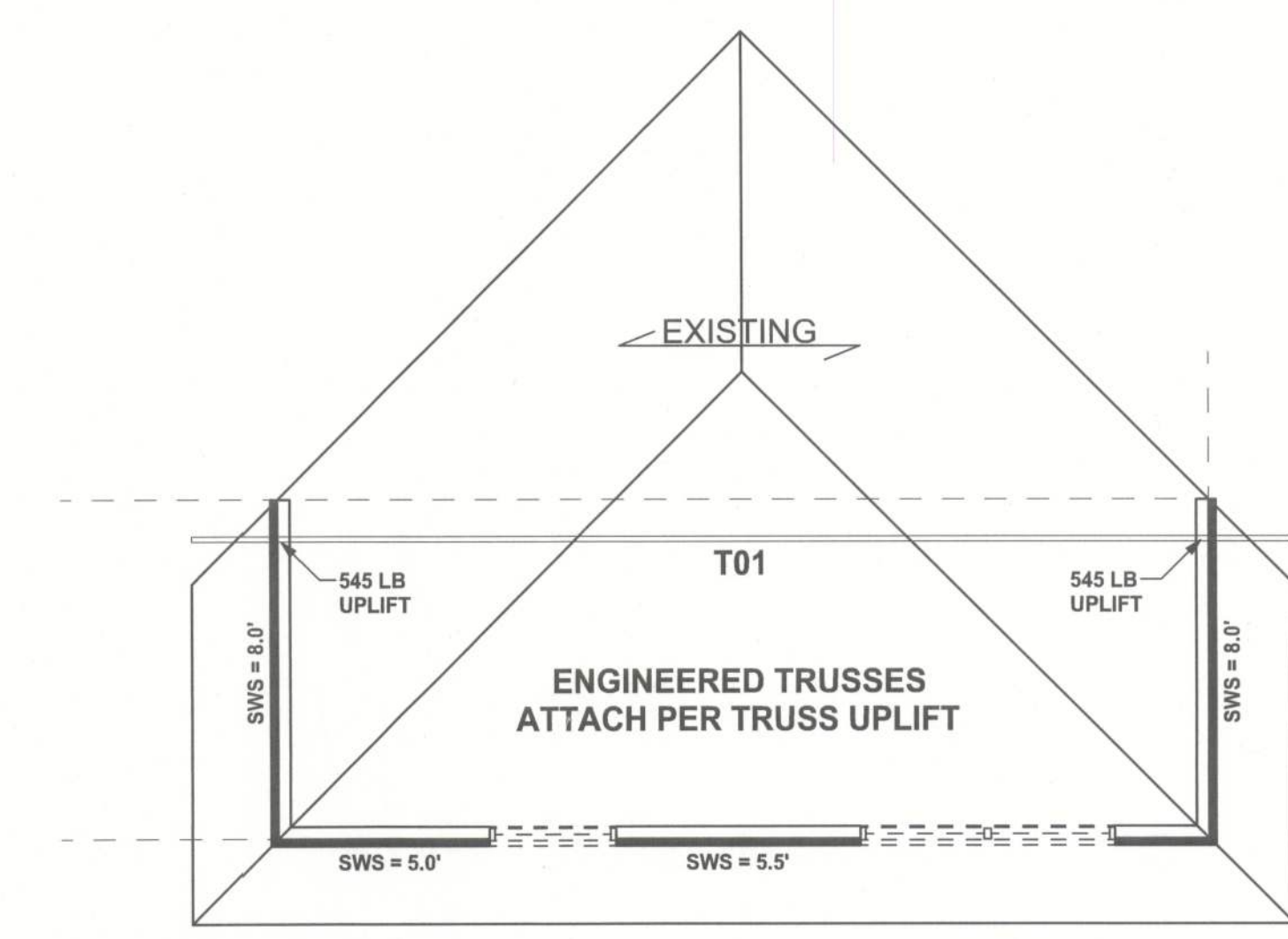
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



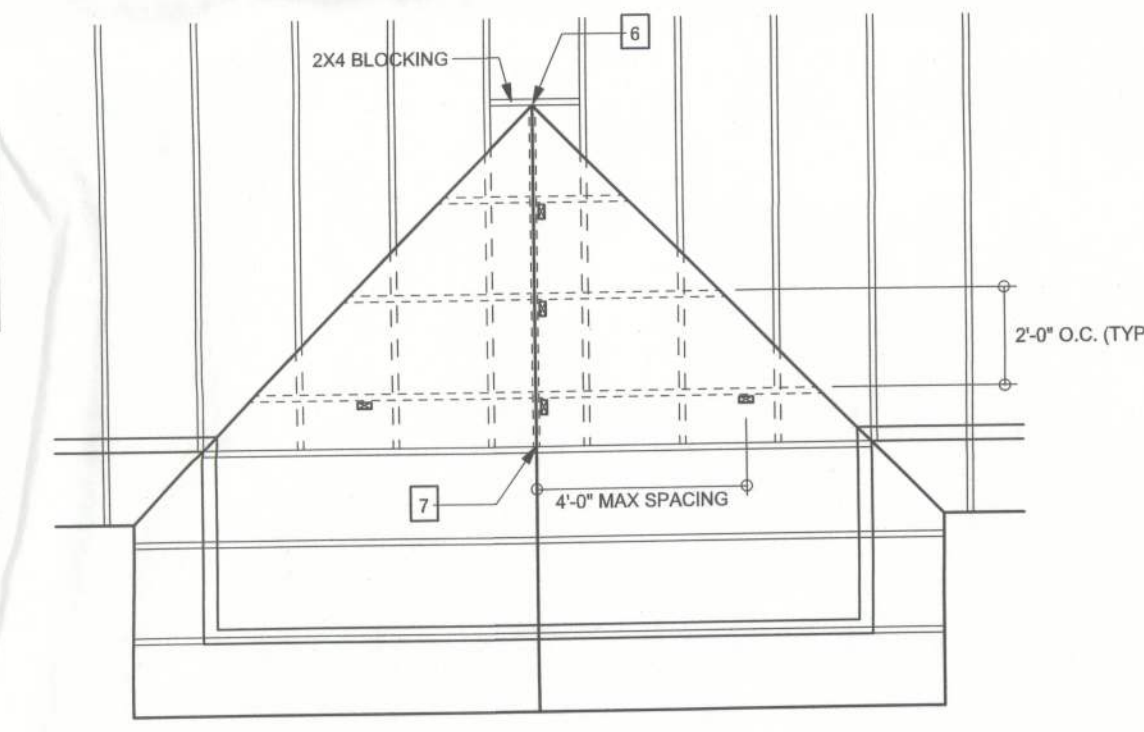
**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 U.N.O.
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER - SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



### VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- - - TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

### CONNECTION REQUIREMENT NOTES

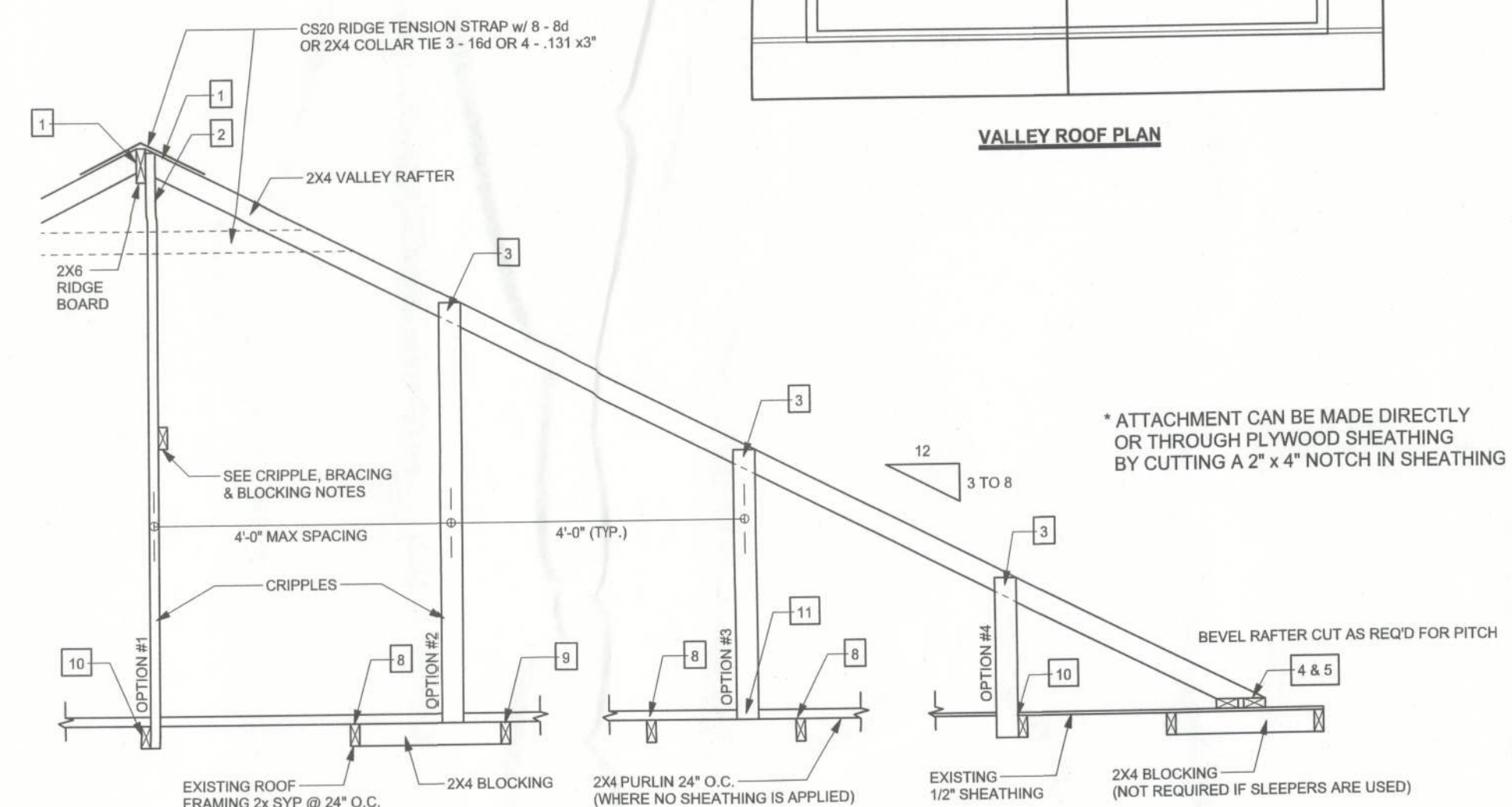
1 2X4 RAFTERS TO RIDGE	3-16d OR 6 - .131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	3-16d OR 6 - .131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	3-16d OR 6 - .131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12 - .131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4-16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK	3-16d OR 6 - .131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	3-16d OR 6 - .131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP.)	3-16d OR 6 - .131 x 3" NAILS
9 PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8 - .131 x 3" NAILS
10 TRUSS TO BLOCKING	3-16d OR 6 - .131 x 3" END NAILS
11 CRIPPLE TO TRUSS	3-16d OR 6 - .131 x 3" FACE NAILS
12 CRIPPLE TO PURLIN	3-16d OR 6 - .131 x 3" FACE NAILS

### GENERAL NOTES

- MAXIMUM RAFTER SPANS  
8'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2
- MAXIMUM ROOF AREA PER SUPPORT  
1602 IN ZONES 2 & 3, 2402 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 6'-0" SPAN = 1602)
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS SHOULD OVERLAP SHEATHING TRUSS SPACING MINIMUM IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 6-16d COMMON WIRE NAIL.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:  
- SPANS (DISTANCE BETWEEN HEELS) 4'-0" OR LESS  
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS  
- MAXIMUM WIND SPEED: 130 MPH  
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET  
- MAXIMUM TOTAL LOADING: 40 psf  
- MEETS FBC 2010/2015 1-10 WIND REQUIREMENTS  
- EXPOSURE CATEGORY "C", I = 1.0, Kz = 1.0  
- ENCLOSED BUILDING

### CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILED TO FLAT EDGE OF CRIPPLE WITH 6 NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB'S OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



**SECTION CUT PARALLEL TO VALLEY RAFTER**

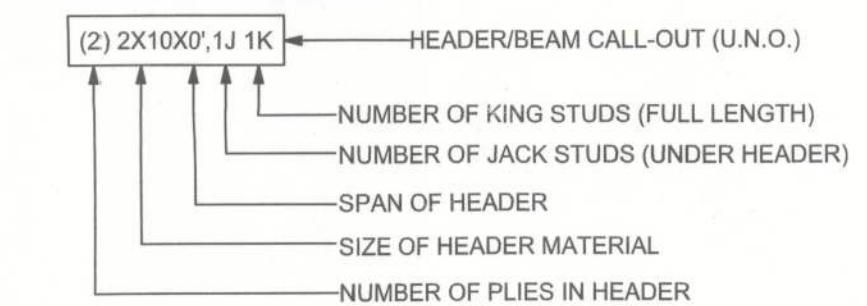
### RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

### WALL LEGEND

—	EXTERIOR WALL
- - -	INTERIOR NON-LOAD BEARING WALL
-----	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
=====	INTERIOR LOAD BEARING WALL w/ UPLIFT

### HEADER LEGEND



### TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	10.0'	11.5'
LONGITUDINAL	8.0'	16.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #468104

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, PCB 686, Lake City, FL 32056, 386-754-5419

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