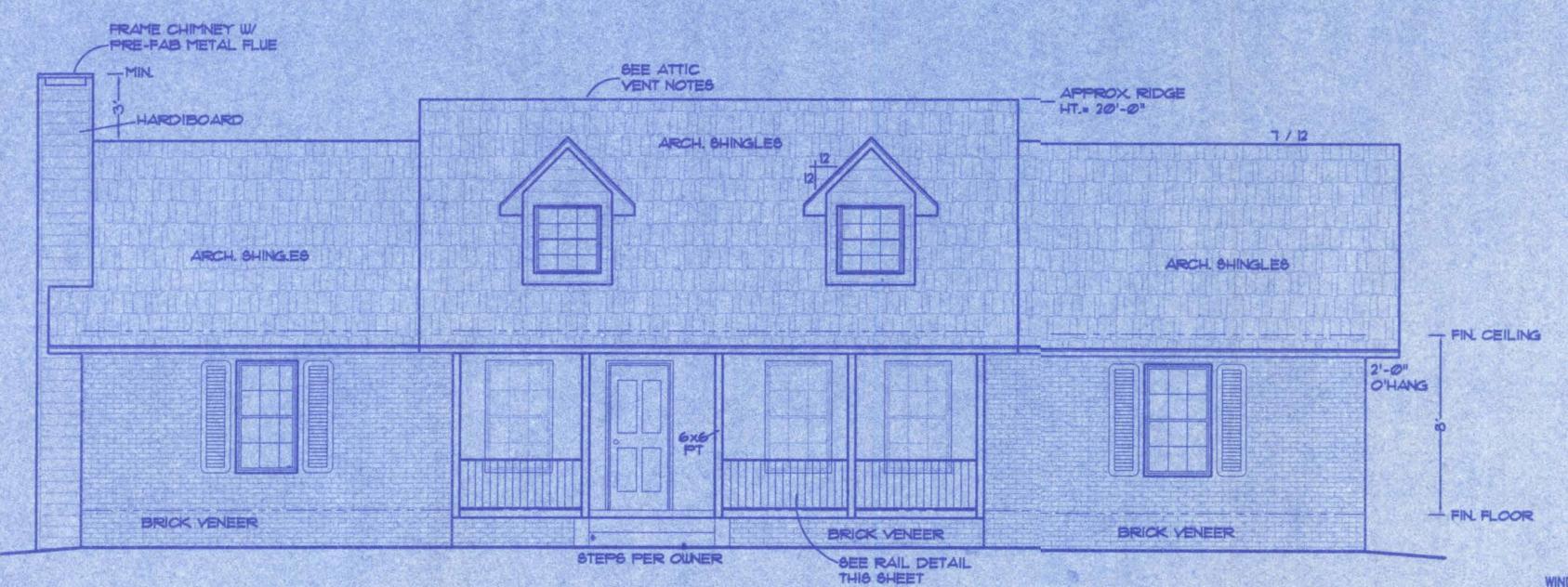
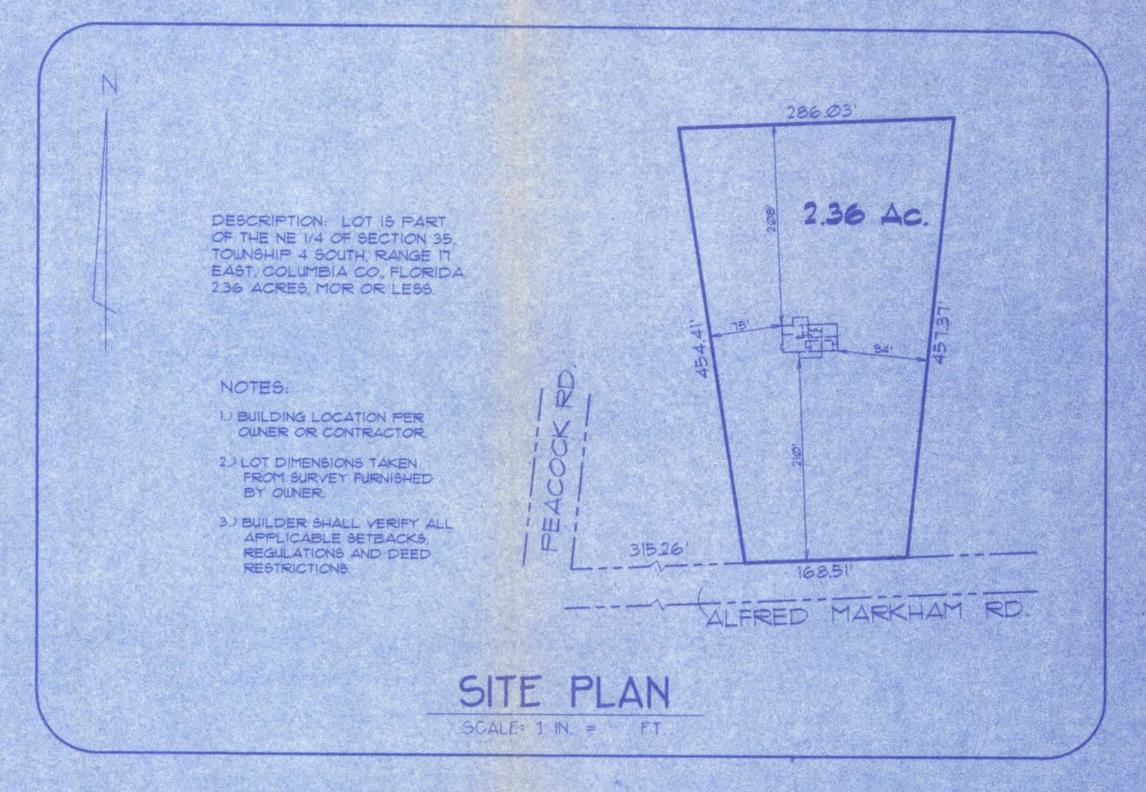
SWS = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 11 provides PATIO 3-1/2:11 or wider. SWS 2 - 3050 EGRE55 M. BATH VERIFY ALL FLOOR COVERINGS W/ OWNER KITCHEN BREAKFAST 10-9 x 9-1 MASTER 10-7 × 10-11 BEDROOM 15-11 x 12-11 CARPET 10'-11 1/4'-----13-4 × 6-3 DINING VINYL PLANK BATH 2 VINTE PLANK I CARPET BEDRM. 2 BEDRM. 3 CARPET 11-0 x 10-9 BRICK FACE CARPET + HEARTH 5-1 × 6-3 DETAIL SHT BEAMS PER ENGINEER PT POSTS | - | 5'-11 1/2' - | - | 5'-11 1/2' - | - | 5'-11 1/2' - | - | 5'-11 1/2' - |

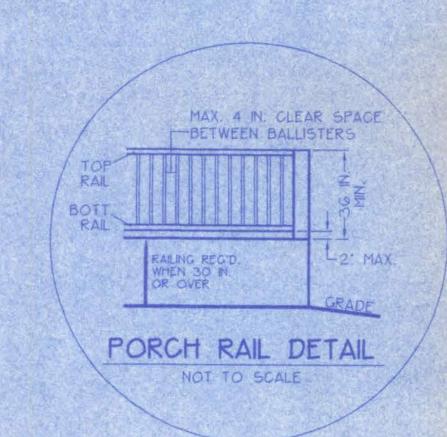
FLOOR PLAN SCALE: 1/4 IN. = 1 FT.



FRONT ELEVATION SCALE: 1/4 IN = 1 FT.

May Residence





ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion—resistant wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above sove or cornice vents with the balance of the required ventilation provided by save or cornice vents.

AREA SUMMARY

Index to Sheets

SHEET A-1 - - - - - - - - - SITE PLAN
+ FLOOR PLAN
+ ELEVATIONS

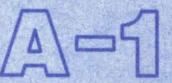
SHEET A-2 - - - - - - - - ELEVATIONS

SHEET A-3 - - - - - - - - FOUNDATION
+ SECTIONS

SHEET A-4 - - - - - - - ELECTRICAL

SHEET S-1 - - - - - - - WIND ENGINEERING





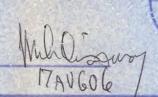
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 886-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: LAKE CITY, FLORIDA

Job No.:__



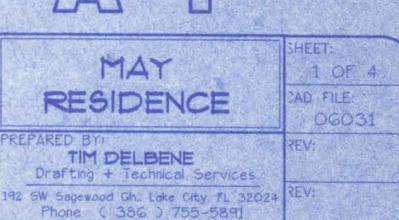
06-031

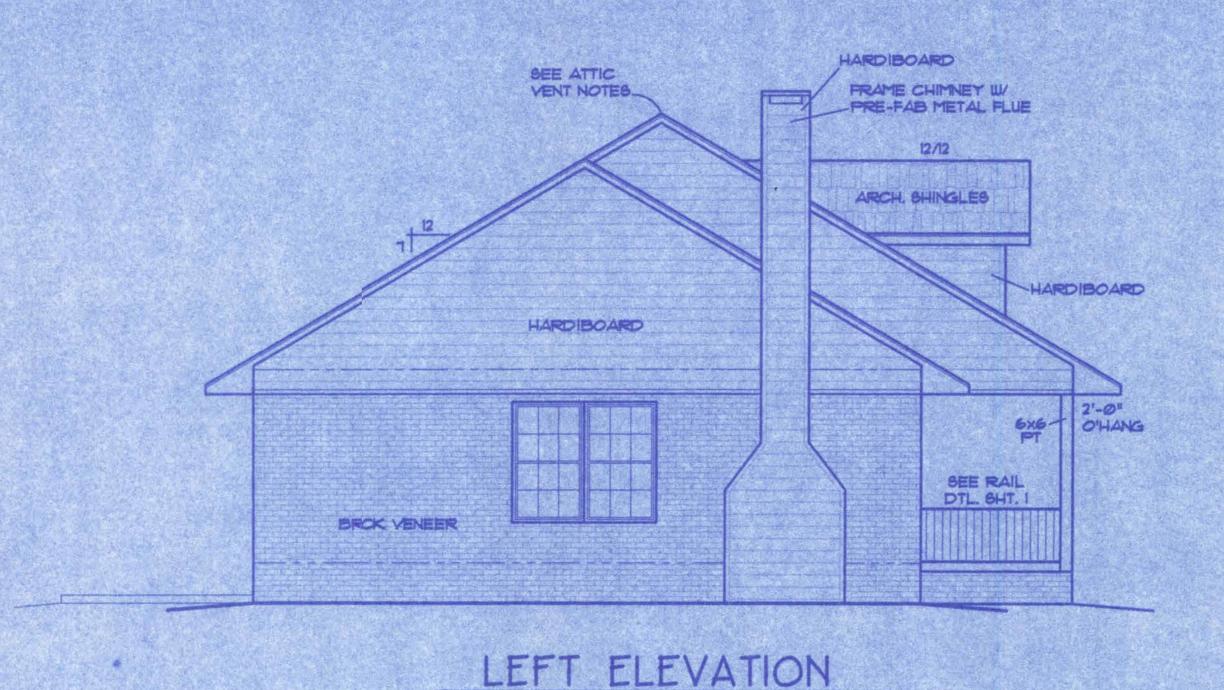
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RAWN:

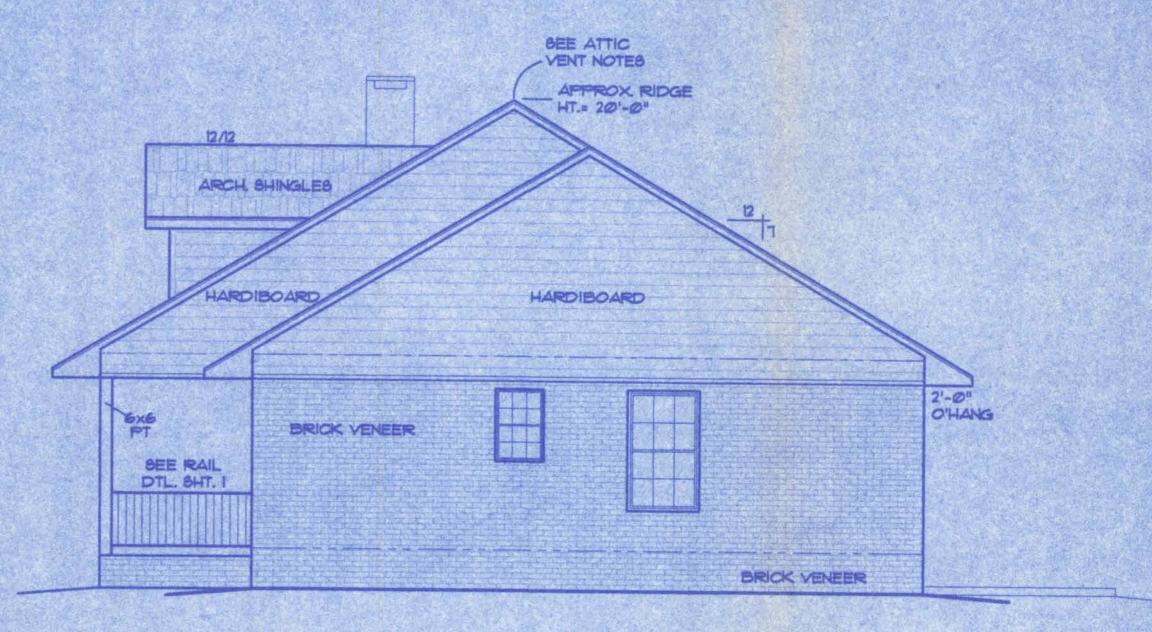
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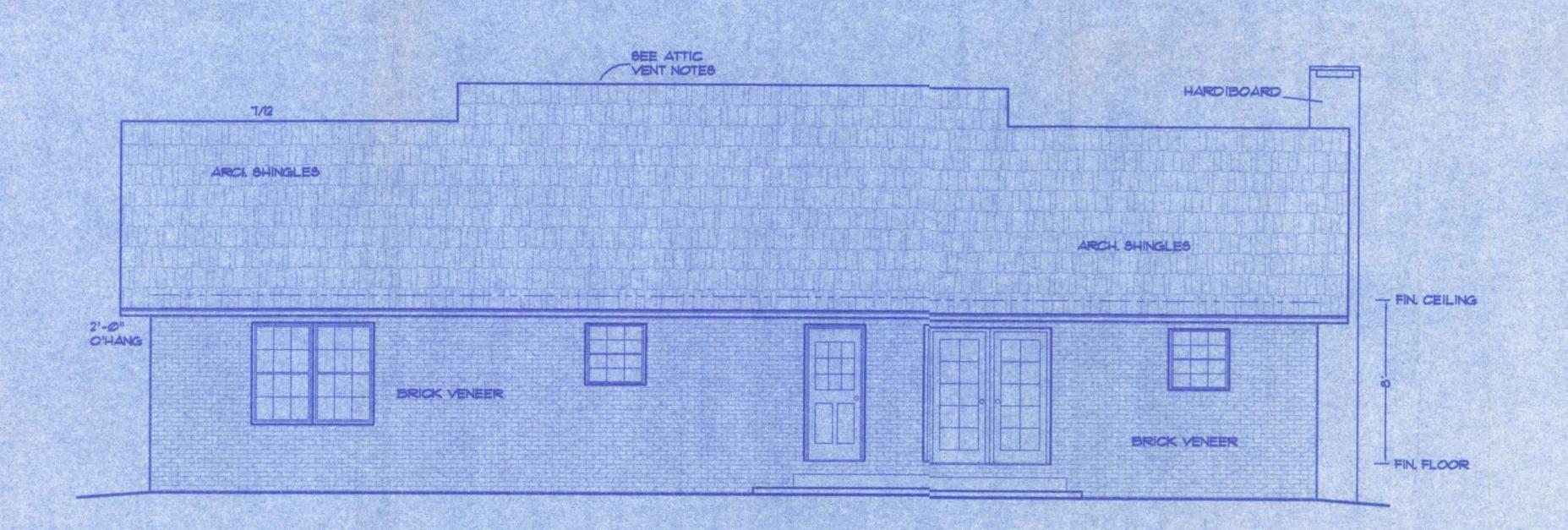




SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION 5CALE: 1/4 IN. = 1 FT.



REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.

GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufactuer shall prepare Shop Drawings Indicating
 Truss placement. Girder locations, Truss-to-Truss Connections
 and any point loads. The Contractor shall notify the Designer
 of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

ATTIC VENTILATION

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The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 00, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper partial of the space to be ventilated at least 3 feet (914 mm) above eave in cornice vents with the balance of the required ventilation provided by eave or cornice vents.

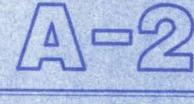
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

ALFRED MARKHAM ROAD Location: LAKE CITY, FLORIDA

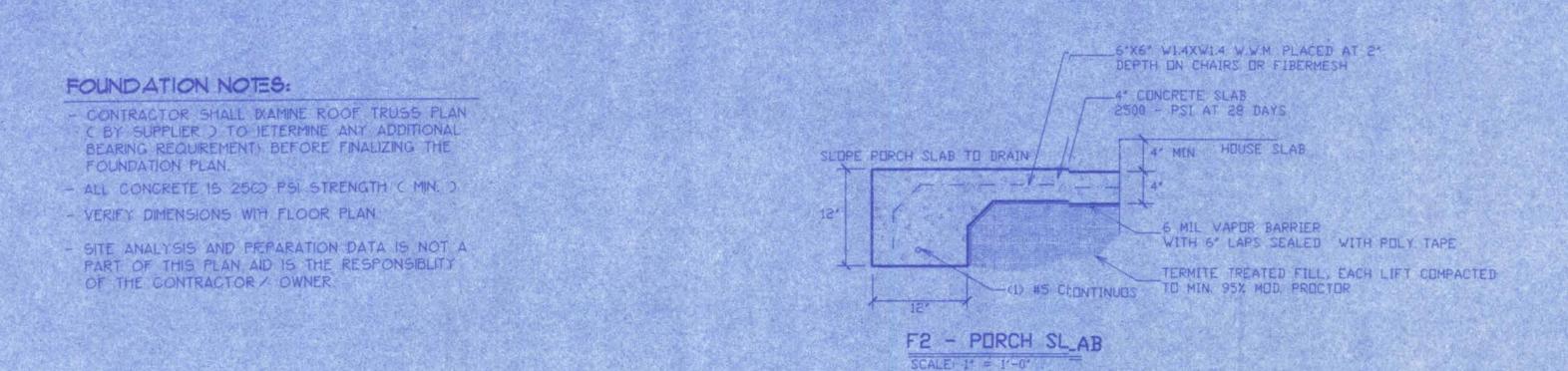
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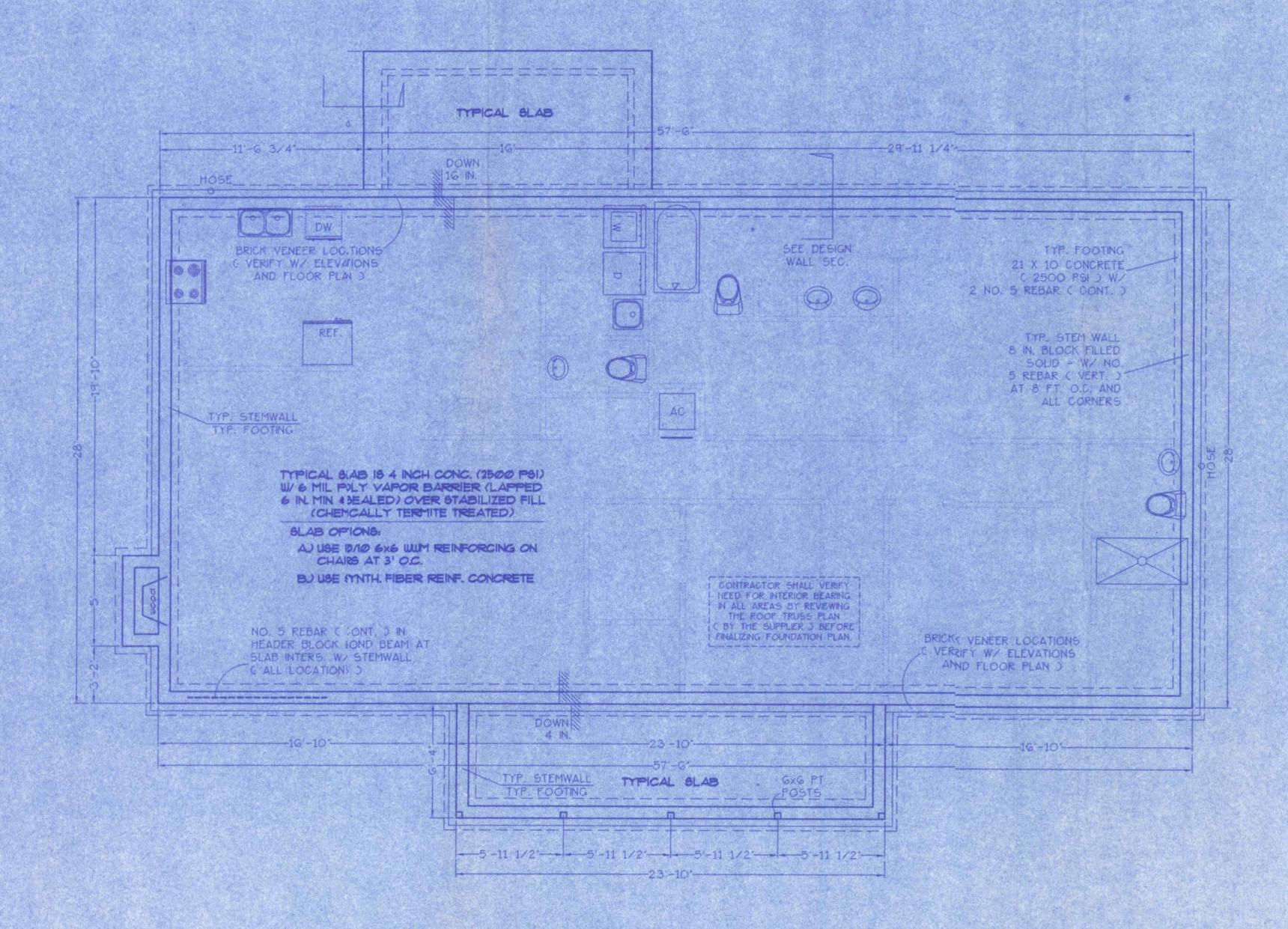


FILE:
OG-031 MAY 1 OF 4

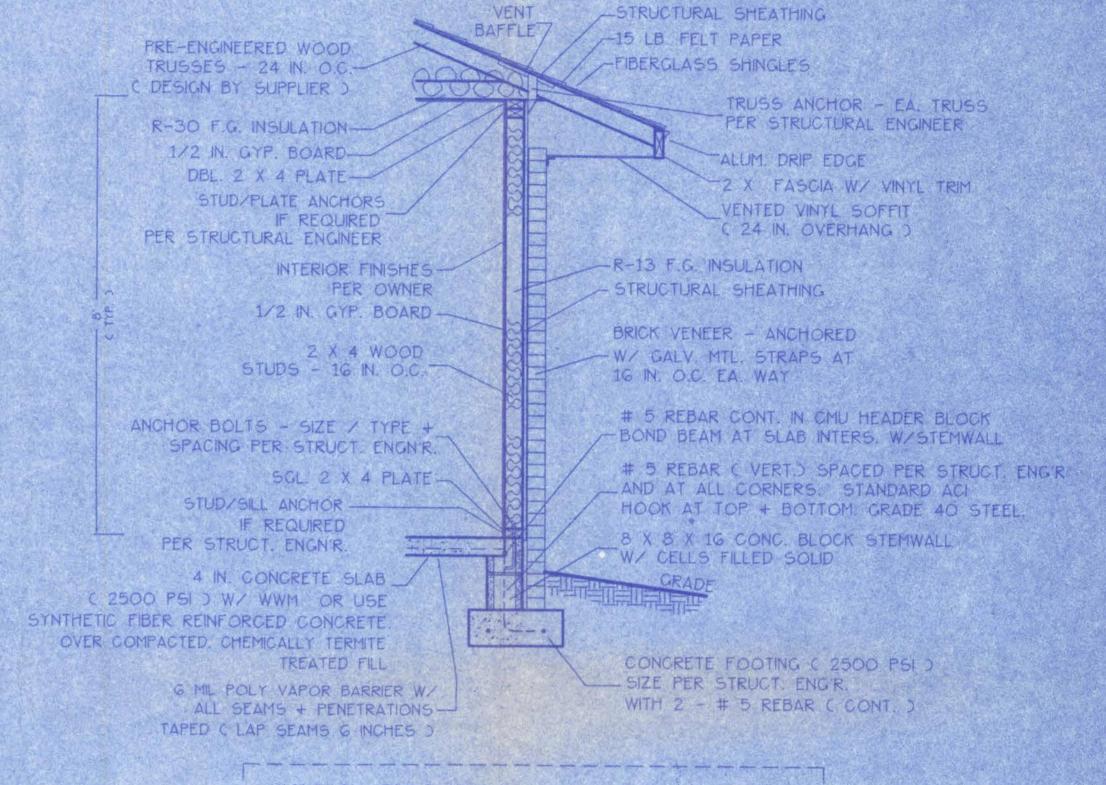
DATE:
7-29-06 RESIDENCE CAD FILE:
TAD PREPARED BY:
TIM DELBENE
Drafting + Technical Services
CHECK: 192 SW Sagewood Gin. Lake City, FL 32024 REV:

Phone (386) 755-5891





FOUNDATION PLAN
SCALE: 1/4 IN. = 1 FT.



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.

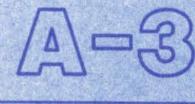
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

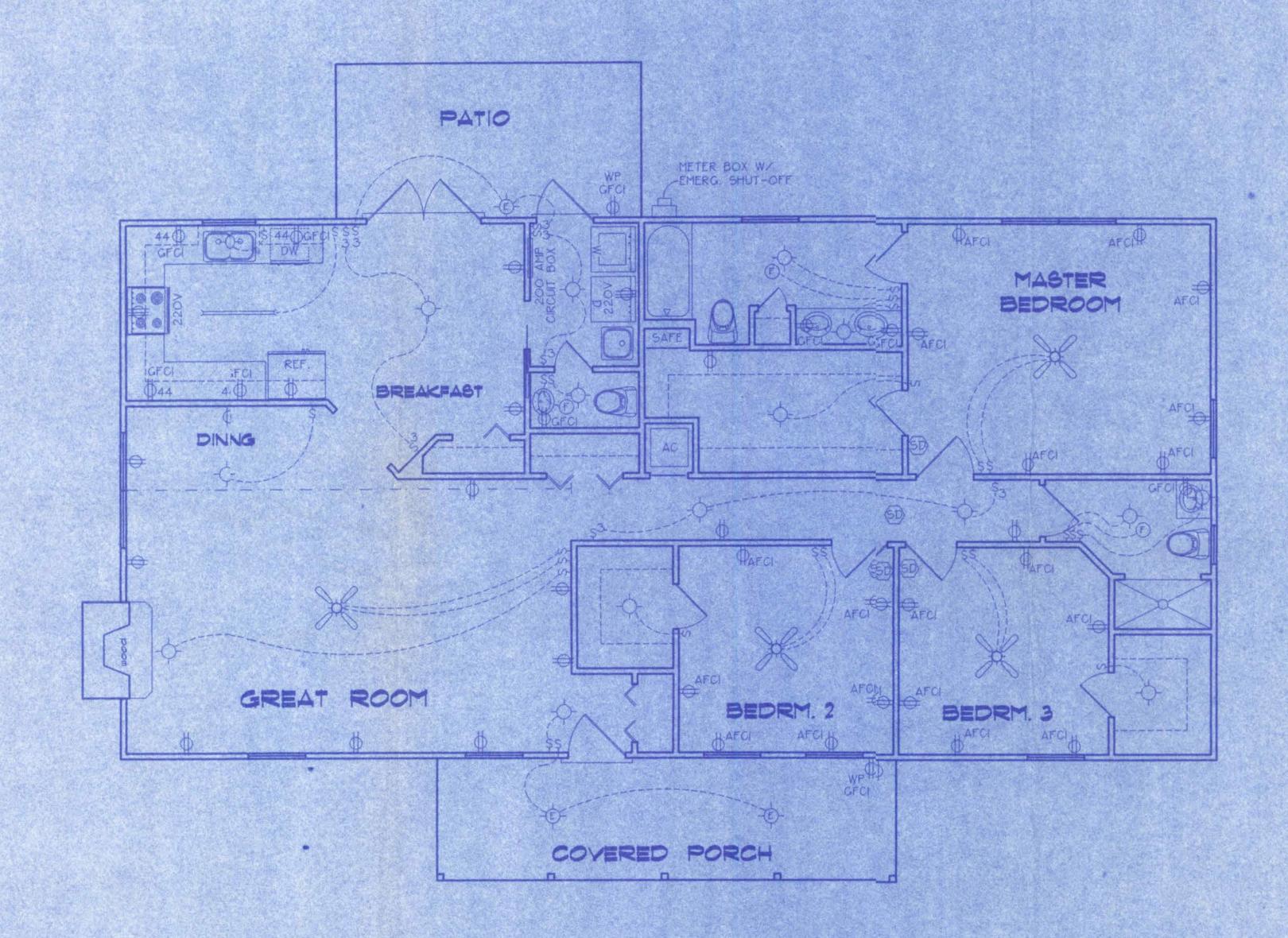
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: LAKE CITY, FLORIDA

_ Job No .: Who say







ELECTRICAL PLAN
NOT TO SCALE

ELECTRICAL	SYMBOL LEGEND
	= FLOURESCENT LIGHTING FIXTURE.
ф	= CEILING LIGHT
•	= EXTERIOR LIGHTING FIXTURE
\$	= LIGHT SWITCH.
\$ ₃	= THREE-WAY SWITCH
•	= 110 V. DUPLEX OUTLET,
₫42	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
⊕ GFCI	= GROUND FAULT CIRC. OUTLET
ф ^{AFCI}	= ARC FAULT CIRC. OUTLET
Ф	= 110 V. SINGLE REGEPTACLE OUTLET.
€ 220V	= 220 VOLT OUTLET (4 WIRE)
×	= FAN LOCATION C CEILING)
© .	= FAN LOCATION C EXHAUST)
60	= SMOKE DETECTOR

ELECTRICAL PLAN NOTES

-WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF, SPECIFICATIONS.

-CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

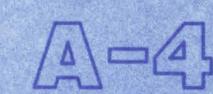
-ALL INSTALLATIONS SHALL BE PER NAT'L ELECTRIC CODE.

-ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
NEAR ALL BEDROOMS.

-TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
DIRECTIONS. + IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION.

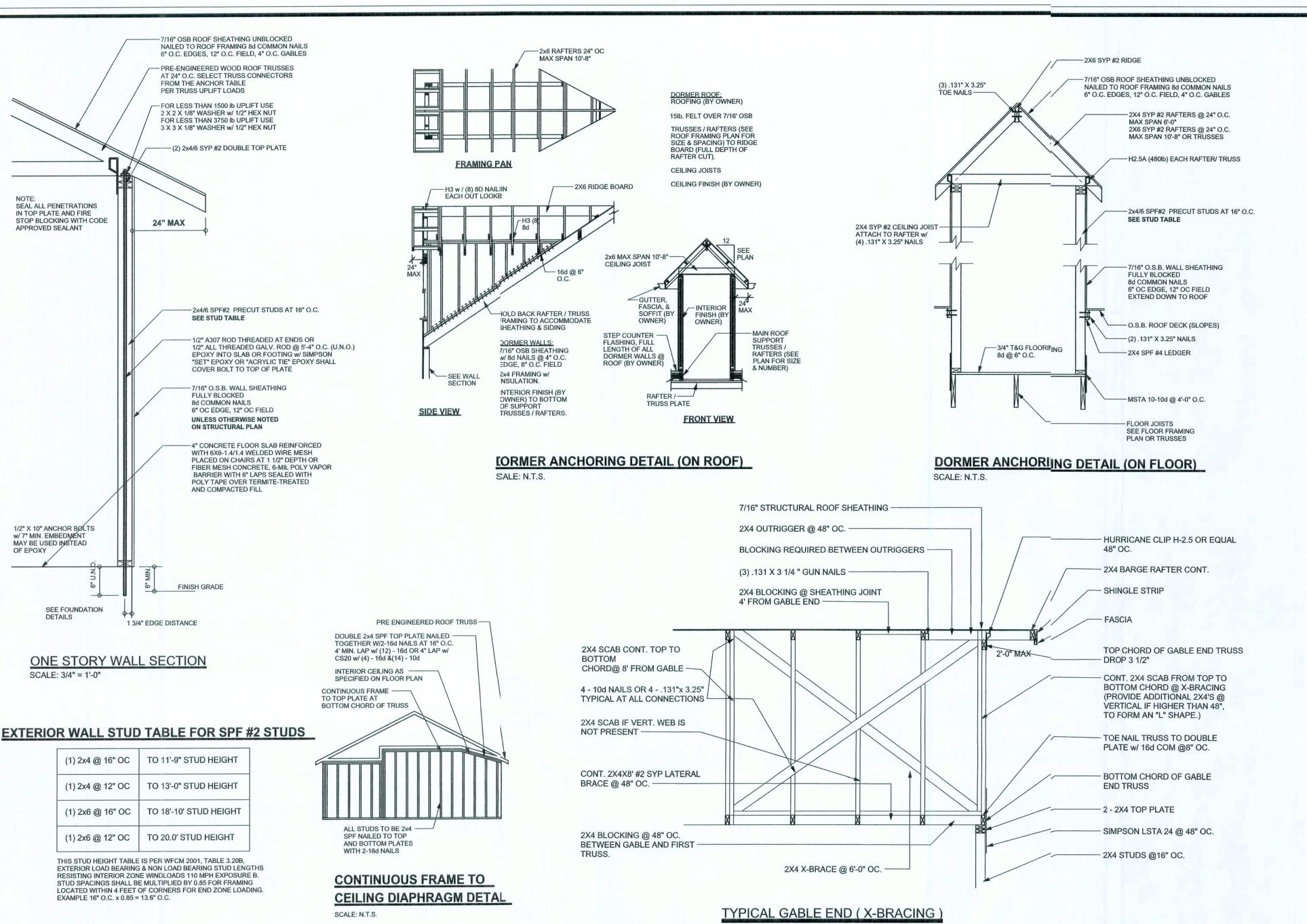
-ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

-ENTRY OF SERVICE C UNDERGROUND OR OVERHEAD)

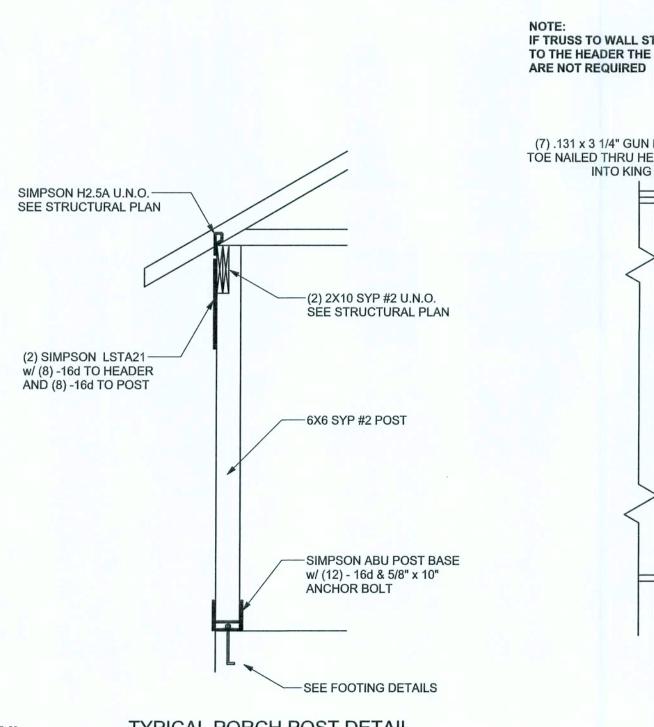


ALFRED MARKHAM ROAD LAKE CITY, FLORIDA

FILE: 06-031	MAY	SHEET:
7-29-06	RESIDENCE	CAD FILE: 06031
DRAWN: TAD	PREPARED BY: TIM DELBENE Drafting + Technical Services	RIV:
CHECK:	192 SW Sagewood Gin. Lake City. FL 32024 Phone (38G) 755-5891	R:Vi



ALL MEMBERS SHALL BE SYP



FOR LESS THAN 1500 Ib UPLIFT USE 2 X 2 X 1/8" WASHER IF TRUSS TO WALL STRAPS ARE NAILED FOR LESS THAN 3750 Ib UPLIFT USE TO THE HEADER THE SP4/6 @ 48" O.C. 3 X 3 X 1/8" WASHER -NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT -SP4/6 @ 48" O.C. (U.N.O.) / —(7) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD CRIPPLES IF REQUIRED (5) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL-INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) 1) 22X6 SPF #2 SILL UP TO 7'-6" U.N.O. (2) 22X4 SPF #2 SILL UP TO 7'-8" U.N.O. (1) 22X4 SPF #2 SILL UP TO 5'-1" U.N.O. (FOR: 1₁₂₀ MPH, 10'-0" WALL HEIGHT U.N.O.)

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

SPH4

SP6

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS

LTT19

LTTI31

PAHD42

HPAHD22

ABU44

ABU66

ABU88

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

MANUFACTURER'S ENGINEERING UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS < 420 < 245 3-8d < 455 < 265 H5 4-8d 4-8d < 360 < 235 4-8d 4-8d < 455 < 320 4-8d < 415 < 365 H2.5 5-8d 5-8d < 600 H2.5A 5-8d 5-8d < 950 < 820 8-8d 8-8d < 745 < 565 5-10d, 1 1/2" 5-10d, 1 1/2" < 1465 < 1050 H14-1 13-8d 12-8d, 1 1/2" < 1465 < 1050 H14-2 15-8d 12-8d, 1 1/2" < 990 < 850 H10-1 8-8d, 1 1/2" 8-8d, 1 1/2" < 760 < 655 H10-2 6-10d 6-10d < 1470 < 1265 H16-1 10-10d, 1 1/2" 2-10d, 1 1/2" < 1470 < 1265 10-10d, 1 1/2" 2-10d, 1 1/2 < 1000 < 860 MTS24C 7-10d 1 1/2" 7-10d 1 1/2" < 1450 < 1245 HTS24 12-10d 1 1/2" | 12-10d 1 1/2" < 2900 2 - HTS24 < 2490 < 2050 < 1785 LGT2 14 -16d 14 -16d **HEAVY GIRDER TIEDOWI** TO FOUNDATION 1-5/8" THREADED ROD < 3965 < 3330 22 -10d 12" EMBEDMENT 2-5/8" THREADED ROD < 10980 < 6485 HGT-2 12" EMBEDMENT 2-5/8" THREADED ROD < 10530 < 9035 HGT-3 16 -10d 12" EMBEDMENT 2-5/8" THREADED ROD < 9250 < 9250 HGT-4 12" EMBEDMENT STUD STRAP CONNECTOR TO STUDS < 435 SSP DOUBLE TOP PLATE < 435 4 -10d < 455 SSP SINGLE SILL PLATE 1 -10d 4 -10d < 825 DSP DOUBLE TOP PLATE < 825 8 -10d < 825 < 600 DSP SINGLE SILL PLATE 2 -10d

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: $6" \times 6"$ W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE WIND LOAD ENGINEER IMMEDIATELY.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

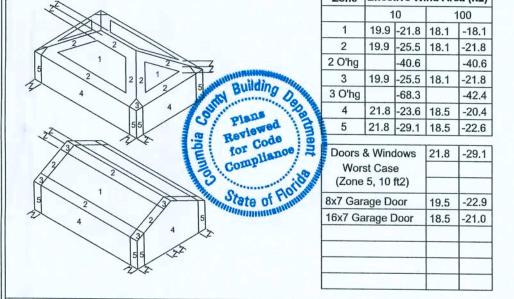
5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

WIN	D LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUII	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUII	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))
	Zene Effective Wind Avec (#2)



DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB868, Lake City, FL 32056, 386-754-549 Stated dimensions supercede scaled imensions. Refer all questions to

Mark Disosway, P.E. for resolution. Do not proceed without clarification. OPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E hereby expressly reserved common law copyrights and property right in nese instruments o service. This document is

not to be reproducel, altered or copied in any rm or manner without first the express written mission and consent of Mark Disosway. CERTIFICATION: I nereby certify that I have amined this plan, and that the applicable portions of the plan, relating to wind engineer comply with sectionR301.2.1, florida building ode residential 2004, to the best of my

IMITATION: This esign is valid for one uilding, at specifier location.

MARI DISOSWAY PE. 53915

> Mack Robinson Construction

May Residence

AIDRESS: Alfred Narkham Road

Mark Disosway P.E. P.O.Box 868

ColumbiaCounty, Florida

Lake City, Florida 32056 Phone: (336) 754 - 5419 Fax: (38) 269 - 4871

Augus 16, 2006 STRUCTURAL BY

PRINTED DATE:

FINALS DATE:

16 / Aug / 06

JOB NUMBER: 608151 DRAWING NUMBER

> S-1 OF ; SHEETS

REVISIONS

SUPPORTIVE POST TO BEAM **DETAIL FOR SINGLE BEAM**

- NON-SUPPORTIVE

SUPPORTIVE -

SIMPSON LSTA18'S (1-ONE SIDE, 2-ON -

OPPOSITE SIDE) EA.

SCALE: N.T.S.

NAILED WITH 14-10d

(2) 2X12 SYP #2 MIN. ---

SÉE STRUCTURAL PLAN

SIMPSON HUS412 MIN. -SEE STRUCTURAL PLAN

SCALE: N.T.S.

SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

BEAM MAY BE ATTACHED IN

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

-(4)-2x4 SPF #2 NAILED

NAILS AT 16" O.C.

BEAM MID-WALL CONNECTION DETAIL

TOGETHER W/2-16d

MIN. (SEE STRUCTURAL PLAN)

(2) 2X12 SYP #2 MIN. -

SÉE STRUCTURAL PLAN

2X4 LADDER BEAM

SUPPORTIVE BEAM -IF BEAM JOINT IS AT -POST CONNECTION, INSTALL ONE SIMPSON LSTA18 ON ONE SIDE 4-SIMPSON LSTA18 ---(2-ONE SIDE,2-ON

SUPPORTIVE CENTER POST TO BEAM JETAIL

(7) .131 x 3 1/4" GUN NAILS-TOE NAILED THRU HEADER

TYPICAL 1 STORY HEADER STRAPING DETAIL

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT RANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GRADE & SPECIES TABLE

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

LSL TIMBERSTRAND

GLB

Fb (psi) E (10⁶ psi)

1.6

1.6

1.6

1.8

1.7

2.0

1200

1050

975

2400

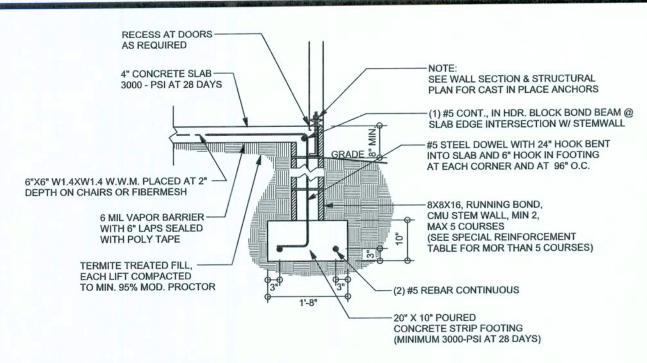
2900

2900

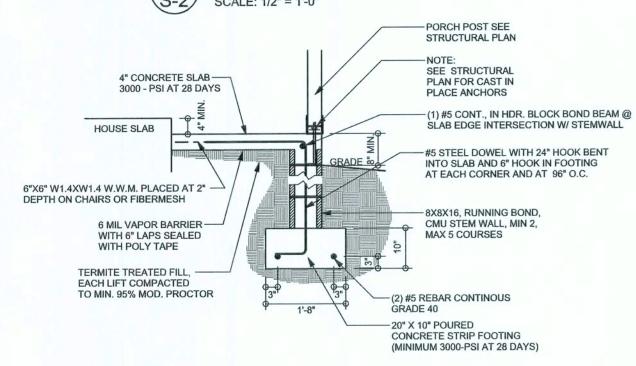
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN	
8' - 10'	24" O.C.	5" O.C.	5" O.C.	
11' - 15'	18" O.C.	4" O.C.	4" O.C.	
16' - 18'	16" O.C.	3" O.C.	3" O.C.	

2x6SYP #2 DOOR BUCK-BRACKET.

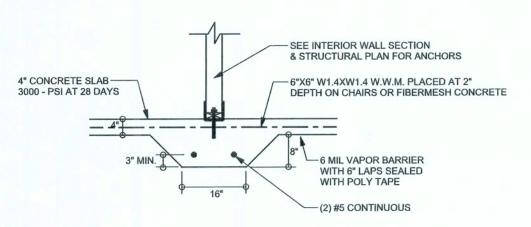
GARAGE DOOR BUCK INSTALLATION DETAIL



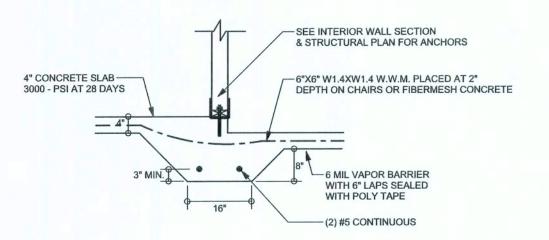
F9 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"



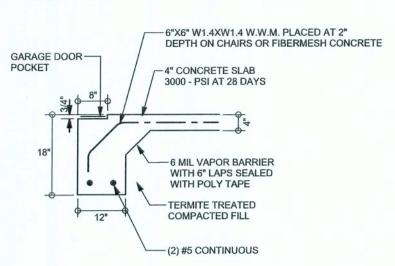
F12 ALT. STEM WALL PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



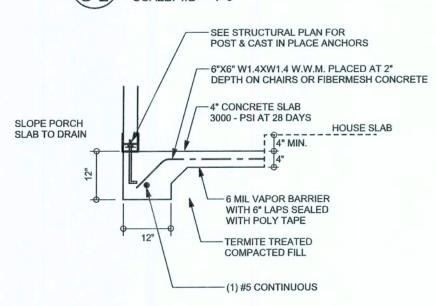
F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

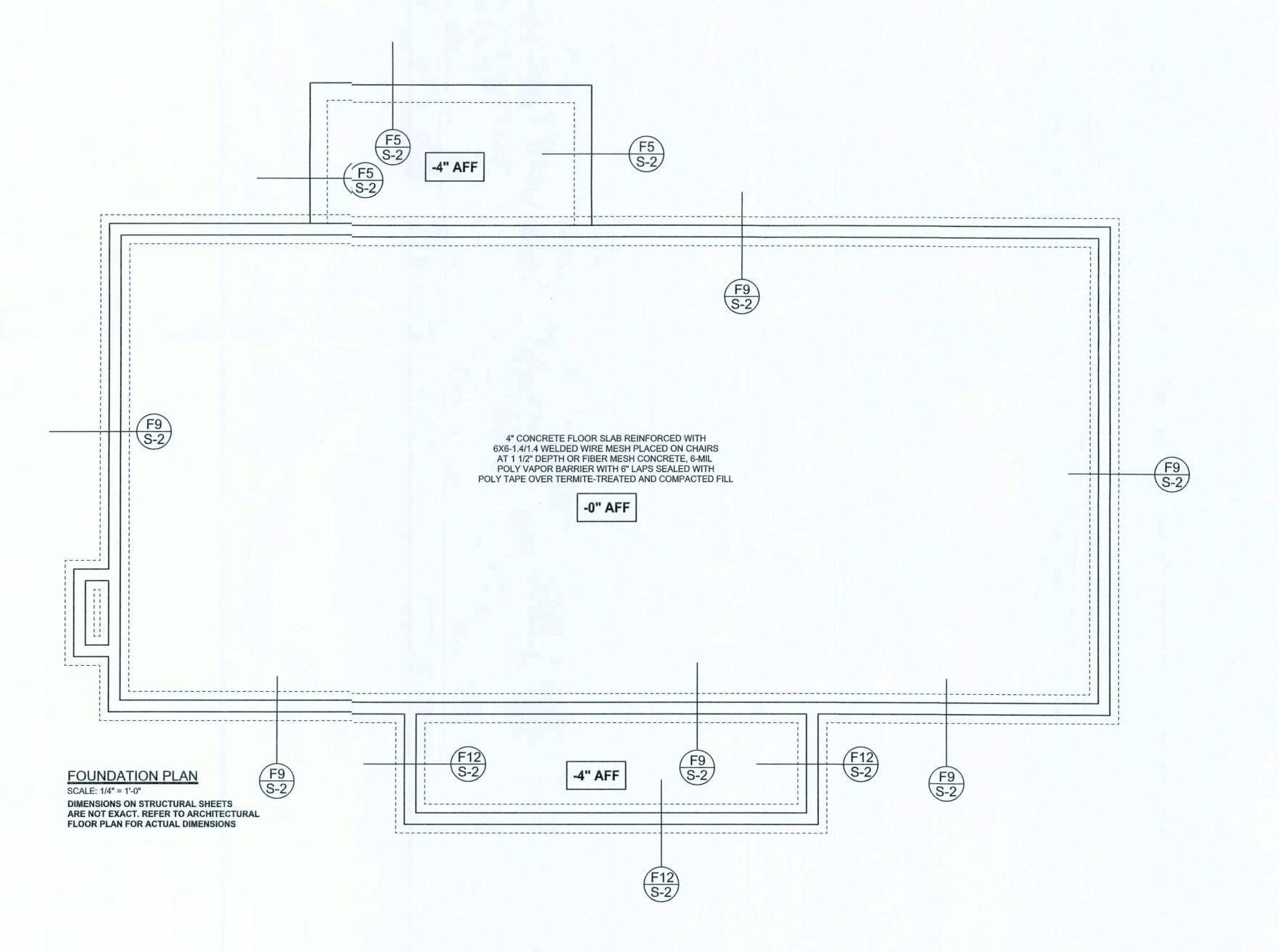


F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALI STEM WALL TABLE

The tablessumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforce-slab at the top. The vertical steel is to be placed toward the tension side of the CMU wa(away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8 ligh, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam wit 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinfreement as shown in the table below.

STEMWLL HEIGIT (FEE)	UNBALANCED BACKFILL HEIGHT	FOR 8	/ERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



REVISIONS

SOFTPIAN

WINDLOALENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 386754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions Refer all questions to Mark Disosvay, P.E. for resolution.
Do not proced without clarification.

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these instrunents of service. This document is
not to be reproduced, altered or copied in any
form or mamer without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply withsection R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one

building, at specified location.

MARK DISOSWAY
P.E. 53915

<u>Nack Robinson</u> <u>Construction</u>

May Residence

ADDRESS: Afred Markham Road Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 LakeCity, Florida 32056 Phore: (386) 754 - 5419 Fax (386) 269 - 4871

PRINTED DATE:
August 16, 2006

DRAWNBY: STRUCTURAL BY
David Disosway

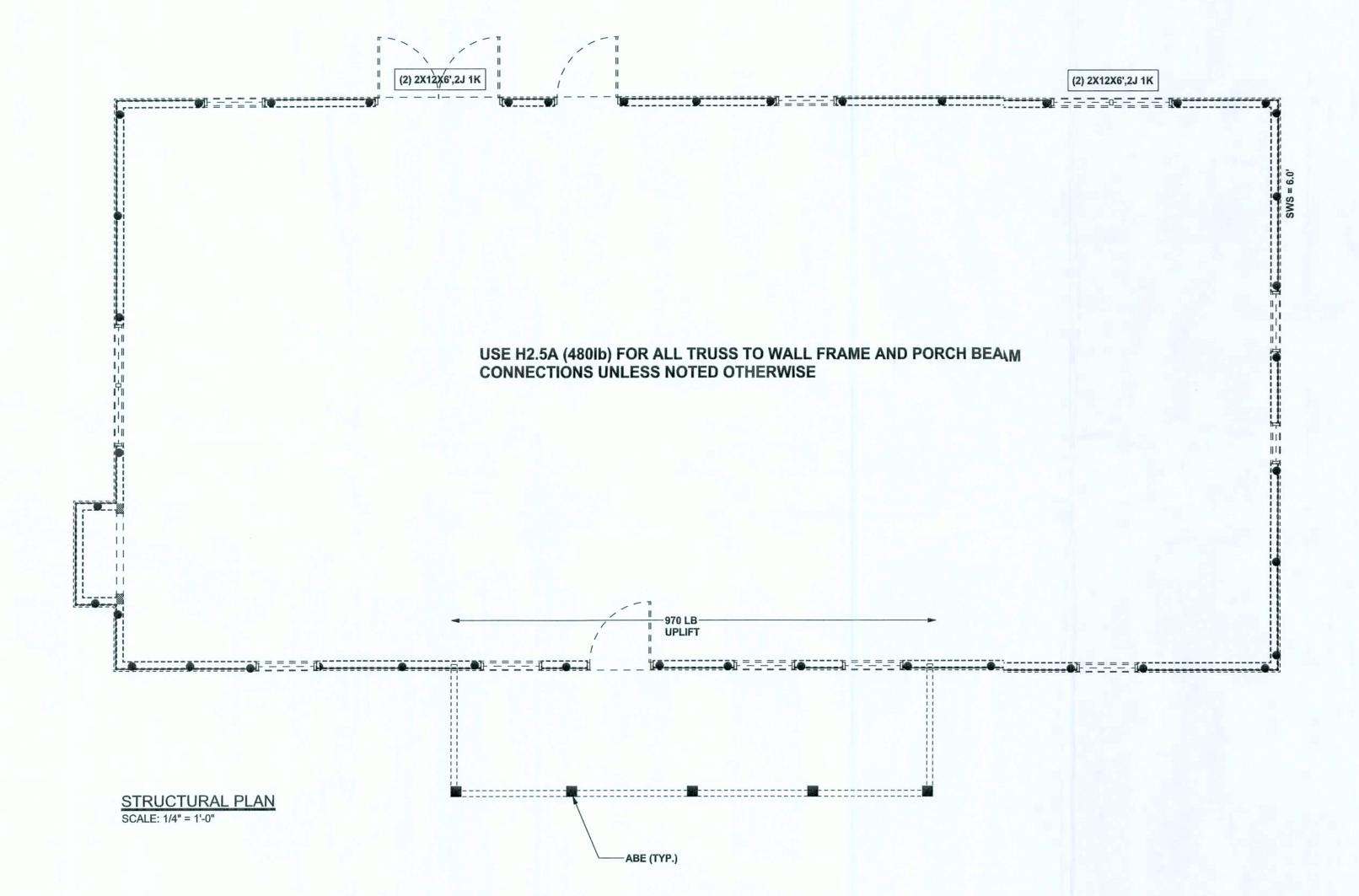
FINALSDATE: 16 / Aug / 06

> JOB NUMBER: 608151 CRAWING NUMBER

> > S-2 OF 3 SHEETS

REVISIONS

SOFTPI AN



STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

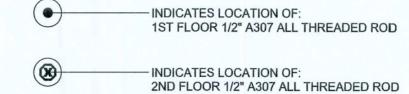
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

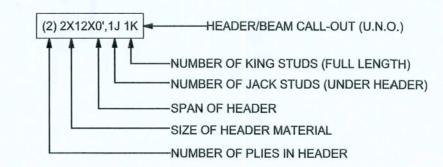
WALL LEGEND

sws = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW \$222222222222222222222222222222222222	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND



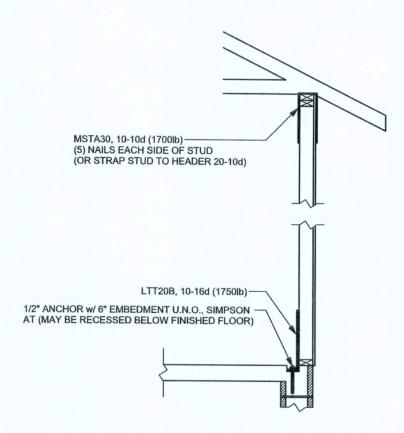
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

old interest to of the set of the				
	REQUIRED	ACTUAL		
RANSVERSE	32.5'	78.5'		
ONGITUDINAL	29.6'	42.0'		



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

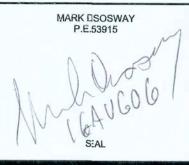
WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 86, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all cuestions to Mark Disosway, P.E. fr resolution. Do not proceed withou clarification.

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CERTIFICATION: I heeby certify that I have examined this plan, and that the applicable portions of the plan, reating to wind engineering comply with section R:01.2.1, florida building code residential 2004, o the best of my knowledge.

LIMITATION: This desgn is valid for one building, at specified loation.



Mack Robinson Construction

May Residence

ADDRESS: Alfred Makham Road Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: August '6, 2006

STRUCTURAL BY: DRAWN BY: David Disosway

OF 3 \$HEETS

FINALS DATE: 16 / Aug / 06

JOB NUMBER: 608151

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING DRAWING NUMBER FURNISHED BY BUILDER: ANDERSON TRUSS **S-3**

JOB #6-294