STEEL GRADE SHALL BE ASTM A570 GRADE C (FY= 40 ksi). NOTE: DEFORMATIONS DO NOT EFFECT STRUCTURAL CAPACITY. FOR SPANS LESS THAN 16'-0" BOX LINTELS TO BE 20 GA. FOR SPANS GREATER THAN OR EQUAL TO 16'-0" BOX LINTELS TO BE 20 GA. F TO BE 16 GA.

2) SHORE LINTELS AS REQUIRED TO COMPENSATE FOR DEAD OAD DEFLECTION ON NON-CURED MASONRY GROUT, ALL LINTELS GREATER THAN 18'-0" ARE BUILT WITH 1/2" CAMBER.

3) LINTEL TO BE USED WITH BRICK OR CONCRETE MASONRY ÚNITS HAVING MINIMUM f'm AS SHOWN. 4) STEEL SURFACES IN CONTACT WITH GROUT AND/OR MORTAR SHALL BE UNPAINTED AND FREE OF MATERIAL THAT MIGHT

5) DESIGN BEARING OF POWERS STEEL LINTELS IS 8" FOR ALL LINTELS GREATER THAN 18'-0" IN SPAN OR GREATER THAN 32" IN DEPTH. ALL OTHER LINTELS REQUIRE A MINIMUM OF 4" BEARING PER THE STANDARD AND FLORIDA BUILDING CODES. 6) f'm = 1500 psi. MASONRY UNITS SHALL CONFORM TO ASTM C90,

7) GROUT = 3,000 psi. SLUMP RANGE: 8" TO 11". ROD OR VIBRATE GROUT ADEQUATELY TO ENSURE CONSOLIDATION OF GROUT (NO AIR POCKETS). GROUT SHALL COMPLY WITH ASTM C476-83 ÀND BE EITHER CÓARSE OR FINE GROUT. 8) MORTAR: TYPE "S" OR TYPE "M" 1800 psi.

9) TOP REINFORCING OR TOP OF WALL REINFORCING, IS A STRUCTURE AND TO PROVIDE FOR UPLIFT RESISTANCE

10) ATTACHMENTS TO TOP OF WALL PER ARCHITECTURAL AND/OR ENGINEERING DRAWINGS. 11) LIMITATIONS:

THE LINTELS SHALL NOT EXCEED THE ALLOWABLE DESIGN LOADS AND SPANS SHOWN IN THIS REPORT. THE LINTELS SHALL NOT BE USED IN A FIRE RESISTANCE RATED ASSEMBLY UNLESS A TEST REPORT DOCUMENTING FIRE RESISTANCE IS SUBMITTED TO THE BUILDING OFFICIAL

A PROPER BARRIER IS REQUIRED WHEN USING CORROSIVE LUMBER PRODUCTS IN CONTACT WITH THE STEEL LINTELS. A PROPER BARRIER WOULD BE A POLYETHYLENE BARRIER WITH A 10 MIL THICKNESS OR TO MAINTAIN A MIN. 1/4" SPACING BETWEEN THE CORROSIVE LUMBER AND STEEL LINTER

12) DEFLECTION LIMITS ARE SET TO L/600 FOR ALL LOADS SHOWN ABOVE THE DARKENED SOLID LINE. DEFLECTION LIMITS ARE SET TO L/360 [LIVE LOAD] AND L/240 [DEAD + LIVE LOAD] FOR ALL LOADS SHOWN BELOW DARKENED SOLID LINE.

13) ALL LOADS SHOWN IN TABLES ARE SUPERIMPOSED LOADS. TABLES ARE DATED 10/2017 AND CLEARLY INDICATE SUPERIMPOSED LOADS.

14) #5 REINFORCING BAR(S) GRADE 40 ARE TO SET APPROX. 1-1/2" FROM TOP OF ALL LINTEL DESIGNS AND IN SOME CASES THE BOTTOM OF LINTEL AS SHOWN ON LOAD TABLES. TOP HORIZONTAL REINFORCEMENT IS TO BE A CONTINUOUS TIE AS NOTED IN NOTE #9. IN THE CASE THAT THE LINTEL IS NOT WITHIN A COMPOSITE BOND BEAM SYSTEM, TOP HORIZONTAL REINFORCEMENT IS TO EXTEND 2'-0" PAST INSIDE OF JAMBS. 15) MANUFACTURER:

POWERS STEEL 4118 E. ELWOOD PHOENIX, AZ 85040 PH# (602) 437-1160 FAX# (602) 437-5409 16) TECHNICAL DATA AND ENGINEERING POWERS LINTELS HÁVE BEEN DESIGNED IN ACCORDANCE WITH

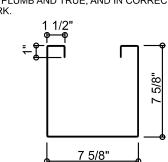
+ NASPEC / AISI LIGHT GAGE COLD FORMED STEEL DESIGN - 2012 + ACI 530-13/ASCE 5-13/TMS 402-13 TECHNICAL ASSISTANCE IS AVAILABLE FROM THE MANUFACTURER ON SPECIAL DESIGN CONCERNS OR

+ FLORIDA BUILDING CODE

LINTEL DEPTHS DIFFERENT THAN THOSE SHOWN IN THE LOAD TABLES. STRUCTURAL ENGINEER FOR THESE LINTELS IS: S.E. CONSULTANTS, INC. 5800 E. THOMAS RD. SUITE 104 SCOTTSDALE, AZ 85251

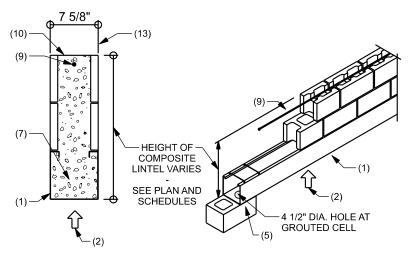
IF AN INSPECTOR, CONTRACTOR, SUBCONTRACTOR, OR PLANS EXAMINER HAS ANY TECHNICAL QUESTIONS PLEASE CALL.

POWERS LINTELS ARE TO BE INSTALLED IN ACCORDANCE WITH STANDARD CONSTRUCTIONS PRACTICES, SET TO PROPER LINE AND LEVEL, PLUMB AND TRUE, AND IN CORRECT RELATION TO OTHER WORK.



PowerBox8 STEEL LINTELS NOTE: NO SCALE 16 GAGE IS .060 INCHES THICK. 20 GAGE IS .039 INCHES THICK.

Powers Steel Product Lintel DETAIL PER POWER BOX TECH SUPPORT



POWERBOX8 LENTEL DETAILS FLORIDA PRODUCT APPROVAL NUMBER FL11383 - POWER BOX

GRADE & SPECIES TABLE							
		Fb	E				
2x8	SP #2	925	1.4				
2x10	SP #2	800	1.4				
2x12	SP #2	750	1.4				
GLB	24F-V3 SP	2600	1.9				
LSL	TIMBERSTRAND	1700	1.7				
ΙVΙ	MICROLAM	2950	20				

PARALAM |2900|2.0

	PowerBOX8 // Lintels // 8" block width LINTEL LOAD TABLE (IN POUNDS PER LINEAL FOOT)								
	GRAVITY LOAD TABLE (3000 psi grout)								
				S ARE SU					
		20ga	. < 16'-0"	span // 16g	ga. >= 16'-	0" span			
SPAN (ft)	PSbox8-8"			PSbox8-12"				PSbox8-24"	
	#5 top	#5 top⊥	#5 top	#5 top⊥	#5 top	#5 top⊥	· ·	#5 top⊥	
1'-6"	5568	5568	7369	7369	9170	9170	12772	12772	
2'-2"	3836	3836	5074	5074	6313	6313	8789	8789	
2'-8"	3106	3106	4107	4107	5107	5107	7109	7109	
3'-2"	2606	2606	3444	3444	4283	4283	5960	5960	
4'-0"	2051	2051	2709	2709	3366	3366	4682	4682	
4'-6"	1816	1816	2398	2398	2980	2980	4143	4143	
5'-2"	1574	1574	2077	2077	2580	2580	3586	3586	
6'-2"	1291	1309	1726	1726	2143	2143	2977	2977	
7'-0"	989	1146	1510	1510	1874	1874	2602	2602	
8'-0"	743	996	1311	1311	1625	1625	2255	2255	
9'-2"	552	760	1133	1133	1404	1404	1946	1946	
10'-0"	454	629	977	1031	1277	1277	1770	1770	
11'-2"	352	492	766	887	1132	1132	1567	1567	
12'-0"	297	418	652	756	1045	1045	1446	1446	
12'-8"	261	369	576	670	984	984	1361	1361	
13'-4"	229	328	511	596	893	929	1285	1285	
14'-0"	203	292	456	532	799	879	1215	1215	
16'-0"	215	284	386	530	688	775	1524	1524	
18'-0"	157	211	286	400	519	588	1201	1336	
18'-8"	142	192	260	365	474	539	1104	1282	
20'-8"	104	145	195	281	365	518	869	1037	
22'-8"	76	110	147	219	284	327	693	832	
24'-0"							625	918	
26'-0"	l	1		l			507	757	

- ROOF SHEATHING SEE

3/4" T&G FLOORING

w/ 8d @ 4" OC EDGE

TRUSS FLOOR

SYSTEM

DESIGNED

BY OTHERS

SEE STRUCTURAL

PLAN ON SHEET S-3

LEDGER -

- 4" CONCRETE

TWO STORY CMU

SLAB

6" OC FIELD

ROOF SHEATHING FASTENING TABLE

TRUSSES AT 24" O.C.

PRE-ENGINEERED WOOD ROOF

SELECT TRUSS CONNECTORS

- 8" BOND BEAM POURED SOLID

#5 VERT. IN FULLY GROUTED

CELLS IN CORNERS & EACH

8" BOND BEAM POURED SOLID

#5 VERT. IN FULLY GROUTED

ALL REBAR IS TO BE GRADE 40

- ATTACH CMU WALL TO SLAB @ BOND BEAM w/ 2'x2' #5 CORNER REBAR (1) LEG EXTENDING INTO SLAB &

EG LAPPAD w/ THE HORIZONTA

BOND BEAM REBAR, SPACED 80" OC MAX

UNLESS NOTED OTHERWISE

CELLS IN CORNERS & EACH

SIDE OF OPENINGS &

@ 64" OC MAX IN WALL

ALL LAPS TO BE 25" MIN

-#5 CONTNUOUS IN

SLAB EDGE INTERSECTION

WALL / COLUMN WITH 8'-0" OC MAX

SPACING UNDER OPENINGS AND

URBED SOIL OR ENGINEERED FILL

SEE SPECIAL REINFORCEMENT

TABLE FOR MOR THAN 5 COURSES)

8X8X16. RUNNING BOND

CMU STEM WALL, MIN 2,

3) #5 REBAR CONTINOUS

CONCRETE STRIP FOOTING

CMU WALI

SCALE: 1/2" = 1'-0"

(MINIMUM 3000-PSI AT 28 DAYS)

MAX 5 COURSES

TYPICAL WALL SECTION (2 STORY)

w/ STD HOOK

BOND BEAM @

(3000 PSI) CONCRETE w/

#5 CONTINUOUS REBAR

SIDE OF OPENINGS &

@ 64" OC MAX IN WALL

(3000 PSI) CONCRETE w/

#5 CONTÍNUOUS REBAR

2" FROM TOP

FROM THE ANCHOR TABLE

PER TRUSS UPLIFT LOADS

NOTE: All lintels grater than 22'-8" in length will require (2) #5 bars top or (2) #5 bars top & bottom ABLE IS PARTIĂL TABLE PER POWEŘ STEEL & WIRE - FLORDA LOAD TABLES date: 10/2017

	PowerBOX8 // Lintels // 8" block width LINTEL LOAD TABLE (IN POUNDS PER LINEAL FOOT) LATERAL LOAD TABLE (3000 psi grout) ALL LOADS ARE SUPERIMPOSED 20ga. < 16'-0" span // 16ga. >= 16'-0" span							
SPAN (ft)	PSbox8-8" #5 top	PSbox8-8" #5 top⊥		PSbox8-12" #5 top⊥	PSbox8-16" #5 top	PSbox8-16" #5 top⊥	PSbox8-24" #5 top	PSbox8-24" #5 top⊥
1'-6"	3256	3256	4097	4097	4960	4960	6732	6732
2'-2"	2254	2254	2836	2836	3434	3434	4661	4661
2'-8"	1832	1832	2304	2304	2790	2790	3787	3787
3'-2"	1543	1543	1941	1941	2349	2349	3189	3189
4'-0"	1221	1221	1536	1536	1860	1860	2524	2524
4'-6"	1085	1085	1366	1366	1653	1653	2244	2244
5'-2"	945	945	1189	1189	1440	1440	1954	1954
6'-2"	792	792	996	996	1206	1206	1637	1637
7'-0"	698	698	878	878	1063	1063	1272	1306
8'-0"	611	611	738	749	822	838	974	1000
9'-2"	492	496	562	570	626	638	742	762
10'-0"	413	417	472	479	526	536	623	640
11'-2"	331	334	379	384	422	430	500	513
12'-0"	287	289	328	333	366	373	433	444
12'-8"	257	260	294	299	328	334	388	399
13'-4"	232	234	266	270	296	302	351	360
14'-0"	211	213	241	244	269	274	318	326
16'-0"	271	272	296	298	319	322	361	366
18'-0"	214	215	234	235	252	254	285	289
18'-8"	199	200	218	219	235	236	265	269
20'-8"	162	163	177	178	191	193	216	219
22'-8" 24'-0" 26'-0"	135	135	148	148	159	160	180 163 138	182 166 141

Il lintels grater than 22'-8" in length will require (2) #5 bars top or (2) #5 bars top & bottom TABLE IS PARTIAL TABLE PER POWER STEEL & WIRE - FLORDA LOAD TABLES date: 10/2017

	PowerBOX8 // Lintels // 8" block width LINTEL LOAD TABLE (IN POUNDS PER LINEAL FOOT) UPLIFT LOAD TABLE (3000 psi grout) ALL LOADS ARE SUPERIMPOSED 20ga. < 16'-0" span // 16ga. >= 16'-0" span							
SPAN (ft)	PSbox8-8"	PSbox8-8"	PSbox8-12"	PSbox8-12"	PSbox8-16"	PSbox8-16"	PSbox8-24"	PSbox8-24"
	#5 top	#5 top⊥	#5 top	#5 top⊥	#5 top	#5 top⊥	#5 top	#5 top⊥
1'-6" 2'-2" 2'-8"	5637 3914 3187	5637 3914 3187	7586 5267 4289	7585 5267 4289	9561 6640 5408	9561 6640 5408	9420 7673	9420 7673
3'-2"	2689	2689	3620	3620	4565	4565	6478	6478
4'-0"	2136	2136	2877	2877	3629	3629	5150	5150
4'-6"	1903	1903	2563	2563	3233	3233	4589	4589
5'-2"	1662	1662	2239	2239	2825	2825	4010	4010
6'-2"	1398	1398	1885	1885	2378	2378	3376	3376
7'-0"	1236	1236	1667	1667	2103	2103	2987	2987
8'-0"	956	956	1365	1365	1703	1703	2387	2387
9'-2"	739	739	1052	1052	1313	1313	1843	1843
10'-0"	626	626	892	892	1115	1115	1565	1565
11'-2"	509	509	726	726	908	908	1275	1275
12'-0"	446	446	636	636	795	795	1118	1118
12'-8"	404	404	576	576	721	721	1014	1014
13'-4"	368	368	525	525	657	657	925	925
14'-0"	337	337	481	481	603	603	849	849
16'-0"	347	347	461	461	558	558	754	754
18'-0"	282	282	376	376	456	456	618	618
18'-8"	265	265	353	353	429	429	582	582
20'-8"	223	223	298	298	363	363	494	494
22'-8" 24'-0" 26'-0"	191	191	257	257	313	313	428 577 507	428 577 507

NOTE: Above loads include 1/3 increase for wind All lintels grater than 22'-8' in length will require (2) #5 bars top or (2) #5 bars top & bottom
TABLE IS PARTIAL TABLE PER POWER STEEL & WIRE - FLORDA LOAD TABLES date: 10/2017

LINTEL DESCRIPTION (PowerBOX8) - INDICATES MIN. HEIGHT OF GROUT ----- INDICATES MIN. REINFORCING & LOCATIONS top = TOP REINFORCING OR TOP OF WALL REINFORCING PSBox8-8" #5top bot = BOTTOM REINFORCING

Powers Steel Product Lintel - Callout

POWERBOX LINTEL INSTALLATION INSTRUCTIONS

2. Set Lintel on block with 8" of bearing each end, minimum 4" - see specification note #5. 3. Apply bed joint on front and back of block. Apply head joint on end of block. Place Lintel on block level and square Proceed as needed with the next course of block. Shore Lintel as required to compensate for dead oad deflection on non cured masonry and grout.

6. Shore lintel prior to laying block and grouting.7. It is recommended to shore lintel in the middle of spans greater than 8'-0" and less than 15'-0". On spans greater than 16'-0" use 2 shores spaced equal distance apart at approximately 5'-4" spacing. On spans greater than 16'-0" place shoring at 8'-0" centers Make sure shoring is square to Lintel and secure. Make sure shoring supports the entire

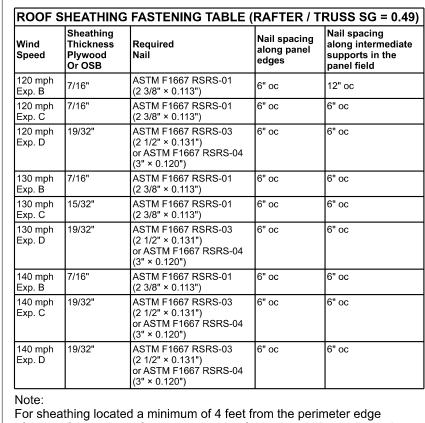
width of the bottom of the Lintel. (see manufacturer's shoring details) 8. Rod grout adequately to ensure consolidation of grout (no air pockets) 9. Place top #5 rebar grade 40 Set 1.5" from top of all Lintel designs and in some cases in the bottom of the Lintel as shown on Load Tables. (see manufacturer's rebar details)

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

(1) 2x6 @ 12" OC

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 140 MPH, EXPOSURE C STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.) TO 9'-7" STUD HEIGHT TO 10'-7" STUD HEIGHT (1) 2x4 @ 12" OC 1) 2x6 @ 16" OC TO 14'-10" STUD HEIGHT

TO 16'-4" STUD HEIGHT



of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.

— CMU WALL

OPENING SIZE / TYPE

WINDOWS & DOORS UP TO 4' W 14" OC

WINDOWS & DOORS UP TO 6' W 10" OC

WINDOWS & DOORS UP TO 10' W 9" OC

SLIDING DOORS UP TO 8' TALL 9" OC

GARAGE DOOR UP TO 10' WIDE 9" OC

GARAGE DOOR UP TO 18' WIDE 4" OC

1 1/4" MIN. EMBEDMENT, 3" MIN. SPACING

STD HOOK

CMU WALL

- IN THE CASE THAT THE LINTEL IS NOT WITHIN

— KNOCK - OUT BLOCK

-#5 REINFORCING BAR(S) GRADE 40

LOAD TABLES, TOP HORIZONTAL

1. FILL LINTEL AND ALL CELLS ABOVE LINTEL.

2. VERIFY THAT ALL REINFORCEMENT HAS BEEN PLACED PROPERLY

3. SEE LINTEL TYPE DESIGNATION TABLE

- FILLED CELL WITH #5 VERTICAL -SEE STRUCTRUAL PLANS FOR LOCATIONS

- CLEAN OUT RQD FOR GROUT LIFT > 5'-0" —

TYPICAL FILLED LINTEL ASSEMBLY

FOR ADDITIONAL INFORMATION.

NOTES: OPENING

ARE TO SET APPROX. 1-1/2" FROM TOP

OF ALL LINTEL DESIGNS AND IN SOME CASES

THE BOTTOM OF LINTEL AS SHOWN ON

REINFORCEMENT IS TO BE A CONTINUOUS

TIE AS NOTED IN NOTE #9 (LINTEL DETAIL)

PRECAST LINTELS STD. 90° HOOK —

WITH (1) #5

A COMPOSITE BOND BEAM SYSTEM, TOP HORIZONTAL

REINFORCEMENT IS TO EXTEND 2'-0" PAST INSIDE OF JAMBS

SCALE: 1/2" = 1'-0"

- TAPCON IN FACE OF CMU: 2 1/2" MIN. EDGE DISTANCE

- WINDOWS AND DOORS MAY BE ATTACHED DIRECTLY TO CMU

- A 1x PT "SPACER" BUCK MAY BE USED IF WINDOW / DOOR IS

DOOR & WINDOW BUCK ATTACHMENT

PER MANUFACTURER AND FLORIDA PRODUCT APPROVAL.

ATTACHED TO CMU PER FLORIDA PRODUCT APPROVAL.

This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

ON ALL SIDES OF OPENING

--- WINDOW OR DOOR ATTACHED TO BUCK PER

- 2x4 OR 2x6 PT SP#2 WINDOW OR DOOR BUCK

— ATTACH BUCK TO CMU WITH TAPCON

— GARAGE DOOR ATTACHED TO BUCK PER

- ATTACH BUCK TO CMU WITH TAPCON OR

16" OC

14" OC

FASTENERS PER TABLE BELOW

CMU WALL WITH REINFORCED CELL

MANUFACTURER AND FLORIDA PRODUCT APPROVAL

MANUFACTURER AND FLORIDA PRODUCT APPROVAL

- 2x6 PT SP#2 DOOR BUCK ON SIDES OF OPENING

ANCHOR BOLT FASTENERS PER TABLE BELOW

3/16" TAPCON 1/4" TAPCON 1/2" ANCHOR BOLTS

N/A

4) 1/2" x 8" ANCHOR

4) 1/2" x 8" ANCHOR

- REINFORCEMENT IN GROUTED CELLS -

CONTINUITY OF FIRST & SECOND FLOOR VERTICAL WALL REINFORCEMENT

UPLIFT STRAP AT CORNERS -

UPLIFT STRAP AT CORNERS

AND ALONG WALL BUILDER TO SELECT STRAPPING

BASED ON TRUSS UPLIFT LOADS

BUILDER TO SELECT STRAPPING

BASED ON TRUSS UPLIFT LOADS

& SPACING FROM TABLE BELOW

AND ALONG WALL

STD HOOK

- BOND BEAM REBAR CONTINUOUS

OVER WALL & OPENING

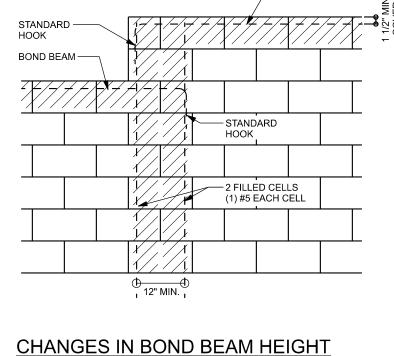
BOLTS PER BUCK

VENLY SPACED

BÓLTS PER BUCK

EVENLY SPACED

MAX SPACING | MAX SPACING | INTO FILLED CELL



(BASED ON FBC FIG. R609.2.5) SCALE: 1/2" = 1'-0"

1	
 	1
	BOND BEAM REINFORCMENT
	STANDARD HOOK LAPPED WITH
ii	WALL REINFORCMENT
	GROUTED CELL
	#5 CORNER BAR LAP BOND BEAM REBAR 25" MIN.
+++	
' '	╢ <i>-</i> ┏∕ ╢ ╢ ┼ :

CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT

(BASED ON FBC FIG. R609.2.4) SCALE: 3/4" = 1'-0"

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over
1235	1235	LSTA24	14-10d	wrap under or over
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
1900		ABU44	12-16d	5/8"x12" Drill & Epo
2450		ABU66	12-16d	5/8"x12" Drill & Epo
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900		ABU44	12-16d	5/8"x7" Drill & Epox
2450		ABU66	12-16d	5/8"x7" Drill & Epox

MASONKY TRUSS ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT	
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

GENERAL NOTES:

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

RUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116. CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED. SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB 30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 1/2" CDX PLYWOOD SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 113" X 2 3/8" RING SHANK NAILS @ 6" OC ON EDGES & INTERMEDIATE SUPPORTS 4" OC ON GABLES

STRUCTURAL CONNECTORS: MANUFACTURERS & PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTE! FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKEILL HEIGHT WIND SPEED AND DEBRIS ZONE. AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. **ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, S BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE T REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF

DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

	XCEPTIONS TO ACI 530.1- NGINEER IN WRITING.	02 MUST BE APPROVED BY
	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings

INT. FRAME WALL TO CMU CONNECTION

16" OC

16" OC

3/16" TAPCON | 1/4" TAPCON

MAX SPACING | MAX SPACING

1 8" OC

16" OC

3/16" OR 1/4" TAPCON —

FOR WALL CONNECTIONS

FOR SHEARWALLS AND

FASTENERS 8" TO 12"

(SEE TABLE FOR SPACING BETWEEN)

WALL END STUD (SEE WALL SECTIONS

BEARING WALLS)

(SEE WALL SECTION)

8" CMU WALL

INTERIOR SHEAR WALL

INTERIOR BEARING WALL

INTERIOR NON-BEARING WALL 48" OC

@ 2ND FLOOR

ENGINEERED TRUSSES

- TRUSS TO TOP PLATE

-2X SPF#2 STUDS

SEE STUD TABLE

ATTACH PER TRUSS UPLIFT

(4) .131"X3 1/4" TOE NAILS

UPLIFT STRAPS

NOT REQUIRED

WITH NO UPLIFT

@ SEE TABLE FOR SPACING

FOR WALLS

1/2" x 10" ANCHOR BOLTS OR 1/2" X 6" SIMPSON TITEN

w/ 2" X 2" X 1/8" WASHER

& 8" FROM CORNERS

INTERIOR BEARING WALL UPLIFT STRAP TABLE

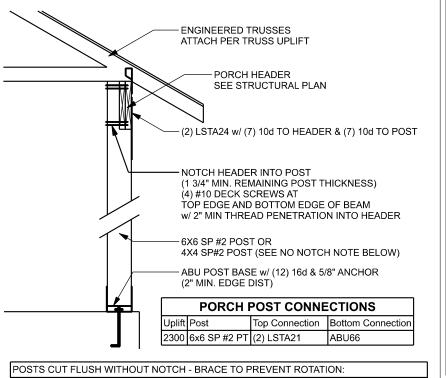
(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

.STA24, 14-10d @ 48" OC │LSTA24, 14-10d @ 48" OC │48" OC

LSTA24, 14-10d @ 32" OC LSTA24, 14-10d @ 32" OC 32" OC

- 2X PT SP #2 PLATE



(TYP.) F

ONE STORY WOOD

			_	
				CODE FOR DESIG
				WINDLOADS
	— ENGINEERED T ATTACH PER TE			BASIC WIND SPE (ASCE 7-22, 3S G
		HEADER RUCTURAL PLAN		WIND EXPOSURE (BUILDER MUST I EXPOSURE PER STARTING CONS
				TOPOGRAPHIC F (BUILDER MUST I
<u> [</u>	— (2) LSTA24 w/ (7	7) 10d TO HEADE	R & (7) 10d TO POST	RISK CATEGORY
				ENCLOSURE CLA
\	— NOTCH HEADE			INTERNAL PRESS
	(1 3/4" MIN. REM (4) #10 DECK S	MAINING POST TH	HICKNESS)	ROOF ANGLE
		BOTTOM EDGE	OF BEAM	MEAN ROOF HEI
	w/ 2" MIN THRE	AD PENETRATIO	N INTO HEADER	C&C DESIGN
1		T OR (SEE NO NOTCH	LNOTE BELOWN	FLOOR LOAD
		E w/ (12) 16d & 5/	,	ROOMS OTHER T SLEEPING ROOM
1/	(2" MIN. EDGE [SLEEPING ROOM
	PORCH	POST CONNE	CTIONS	ROOF LOADI
- '₽')	Jplift Post	Top Connection	Bottom Connection	FLAT OR < 4:12
	2300 6x6 SP #2 PT		ABU66	4:12 TO < 12:12
	.500 0x0 3F #2 F I	(2) L3 1A2 1	AB000	SOIL BEARIN
CUT FLUSH WITHOUT NO	TCH - BRACE TO F	PREVENT ROTATI	ON:	FLOOD ZONE
IP ROOF:				
IP ROOF.				

POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:
EAVE / HIP ROOF: - STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK) USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED - (2) LSTA24 14-10 OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER - ATTACH HEADER TO POST w/ (8) .131" x 3.25" TOE-NAIL
GABLE END: - EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN.
INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE

SER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED . 14-10 OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER EADER TO POST w/ (8) .131" x 3.25" TOE-NAIL	EF WII
D: SABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. SS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE EADER TO POST W/ (8) .131" x 3.25" TOE-NAIL	0 -
PORCH POST	9x7

DESIGN CRITERIA & LOAD	S:
BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY EXPOSURE PER SITE BEFORE STARTING CONSTRUCTION)	C (Surface Roughness Categories: Open terrain with scatte obstructions having heights generally less than 30'. This category includes flat open country and grasslands.)
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	FBC R301.2(2) & TABLE R301.2(4) SEE TABLE BELOW
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

-LOOD ZONE	IIII	THIS BUILDING IS NOT IN THE PLOOD ZOINE		
COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)				
EFFECTIVE VIND AREA (FT2)	ZONE 4 (w INTERIOR		ZONE 5 (walls) END 4' FROM ALL OUTSIDE CORNER	
) - 20	Vult = +43 Vasd = +26		Vult = +43 / -57 Vasd = +26 / -35	
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)				

LIMITATION: This design is valid for one building, at specified location. Mark Disosway P.E. 163 SW Midtown Place

Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

Mark Disosway FL PE 53915

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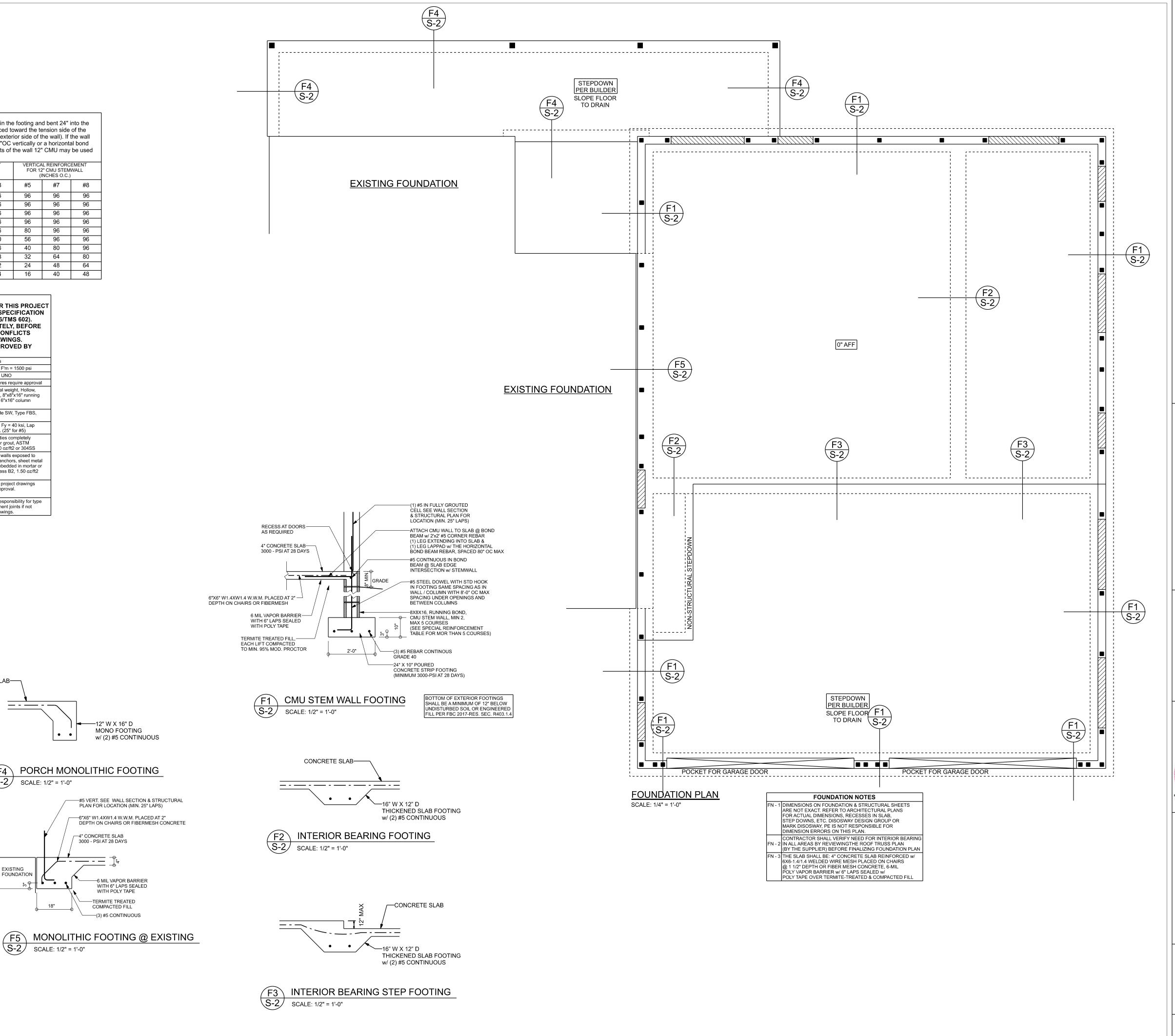
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S-1 OF 3 SHEETS



TALL STEM WALL TABLE: The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below. STEMWALL UNBALANCED VERTICAL REINFORCEMENT VERTICAL REINFORCEMENT HEIGHT (FEET) BACKFILL FOR 8" CMU STEMWALL FOR 12" CMU STEMWALL HEIGHT (INCHES O.C.) (INCHES O.C.)

4.3 88 96 5.3 5.0 56 96 6.7 6.3 32 56 8.0 7.7 16 32 48 32

Specific Requirements

8" block bearing walls F'm = 1500 psi ASTM C 270, Type N, UNO

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running oond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"

ASTM 615, Grade 40, Fy = 40 ksi, Lap

bedded in mortar or grout, ASTM 525, Class G60, 0.60 oz/ft2 or 304SS Joint reinforcement in walls exposed to

Any not shown on the project drawings quire engineering approval.

Contractor assumes responsibility for type

and location of movement joints if not

detailed on project drawings.

CONCRETE SLAB-

S-2 SCALE: 1/2" = 1'-0"

EXISTING

FOUNDATION

—12" W X 16" D

F4 PORCH MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"

MONO FOOTING

PLAN FOR LOCATION (MIN. 25" LAPS)

---6"X6" W1.4XW1.4 W.W.M. PLACED AT 2"

—6 MIL VAPOR BARRIER

TERMITE TREATED

—(3) #5 CONTINUOUS

COMPACTED FILL

WITH 6" LAPS SEALED WITH POLY TAPE

—4" CONCRETE SLAB

3000 - PSI AT 28 DAYS

w/ (2) #5 CONTINUOUS

noisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2

plices min 40 bar dia. (25" for #5) Anchors, sheet metal ties completely

8.7 8.3 8 24

MASONRY NOTE:

THE ENGINEER IN WRITING. ACI530.1-02 Section

Clay brick standard

nforcing bars, #3 - #11

Coating for corrosion protection

Coating for corrosion protection

3.3.E.2 Pipes, conduits, and accessories

9.3 9.0 8 16 24 16

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

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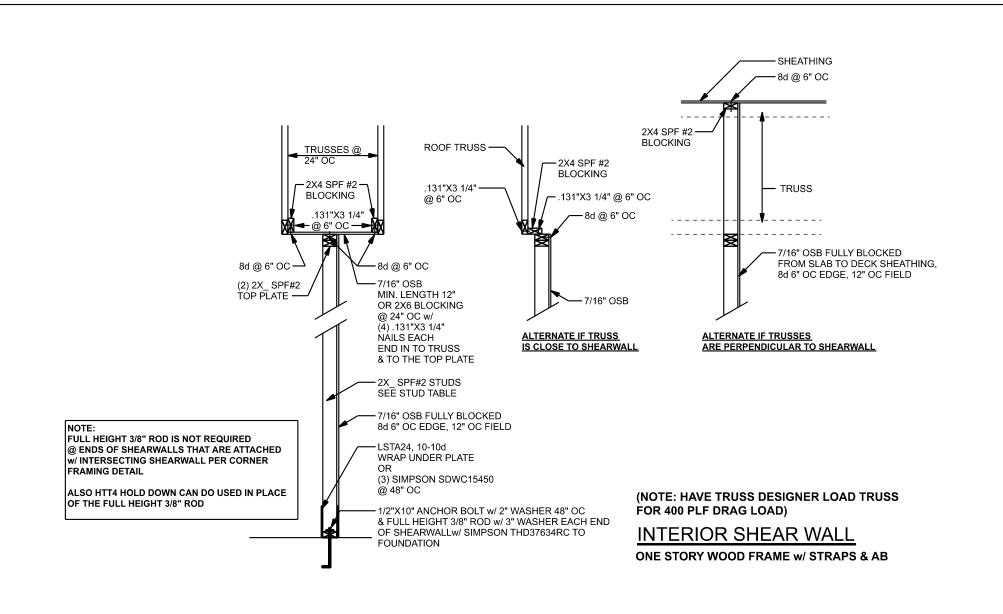
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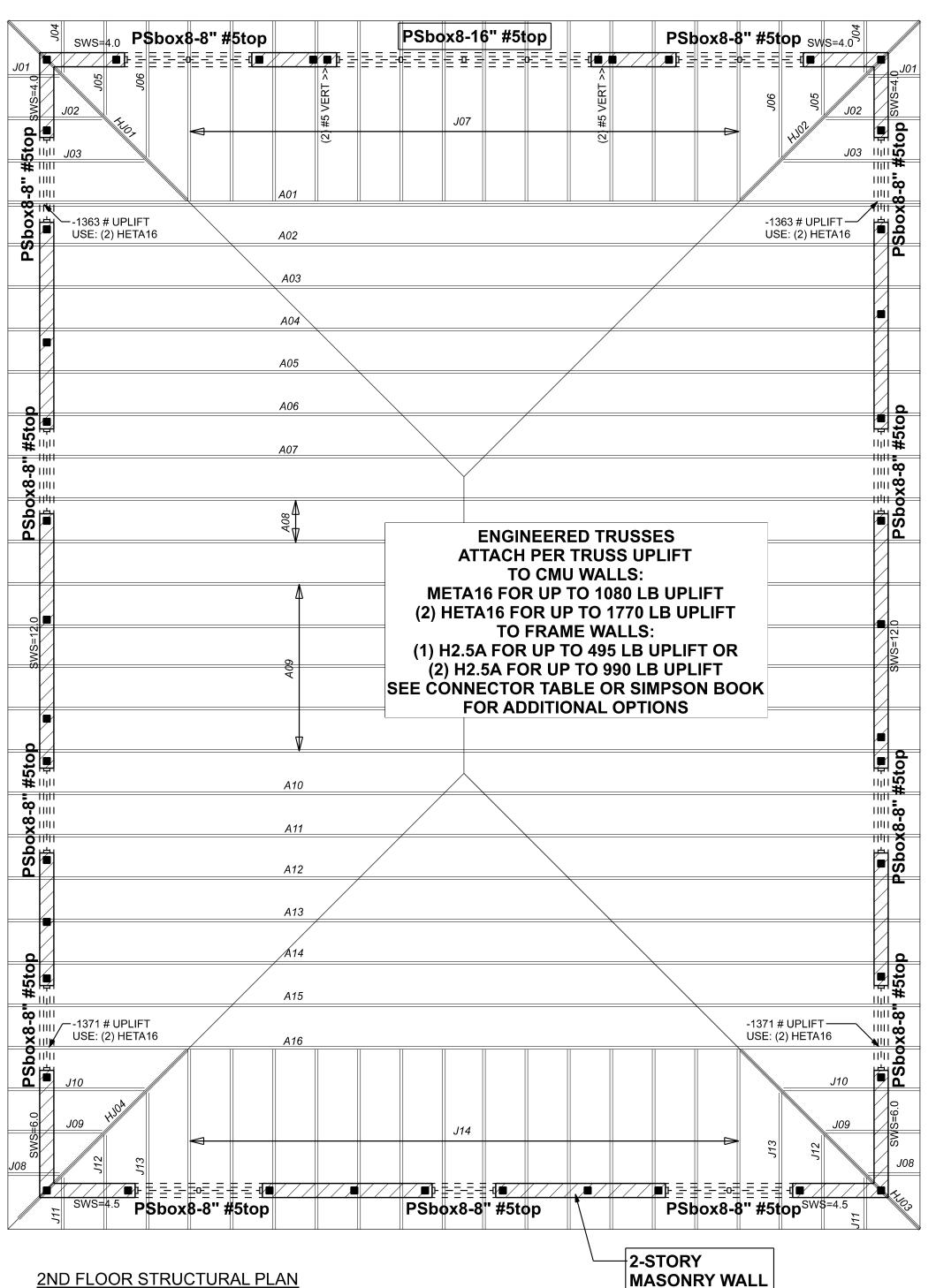
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241248 **S-2** OF 3 SHEETS



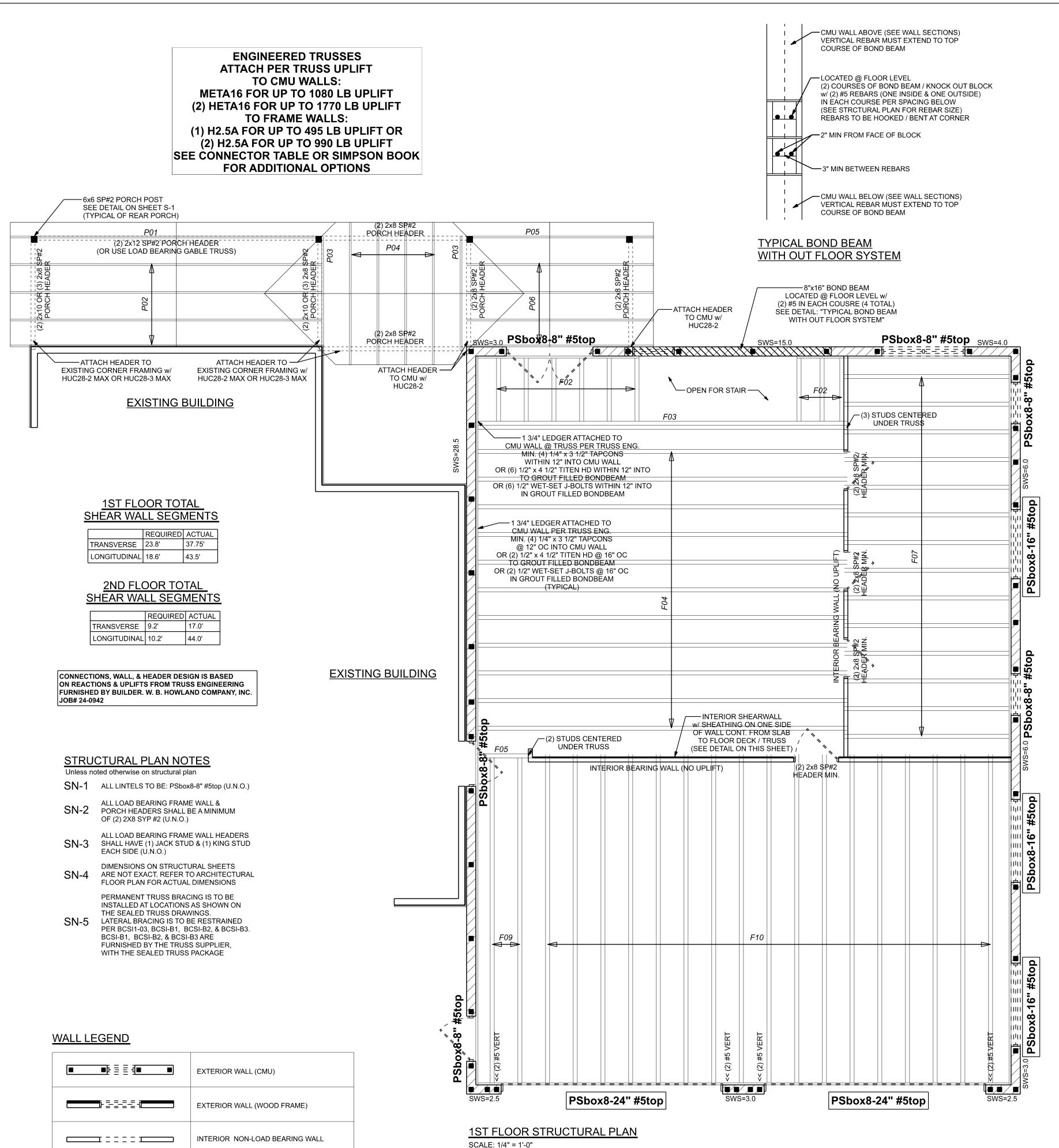


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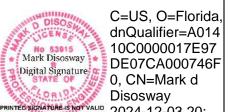
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INTERIOR LOAD BEARING WALL w/ NO UPLIFT

INTERIOR LOAD BEARING WALL w/ UPLIFT



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