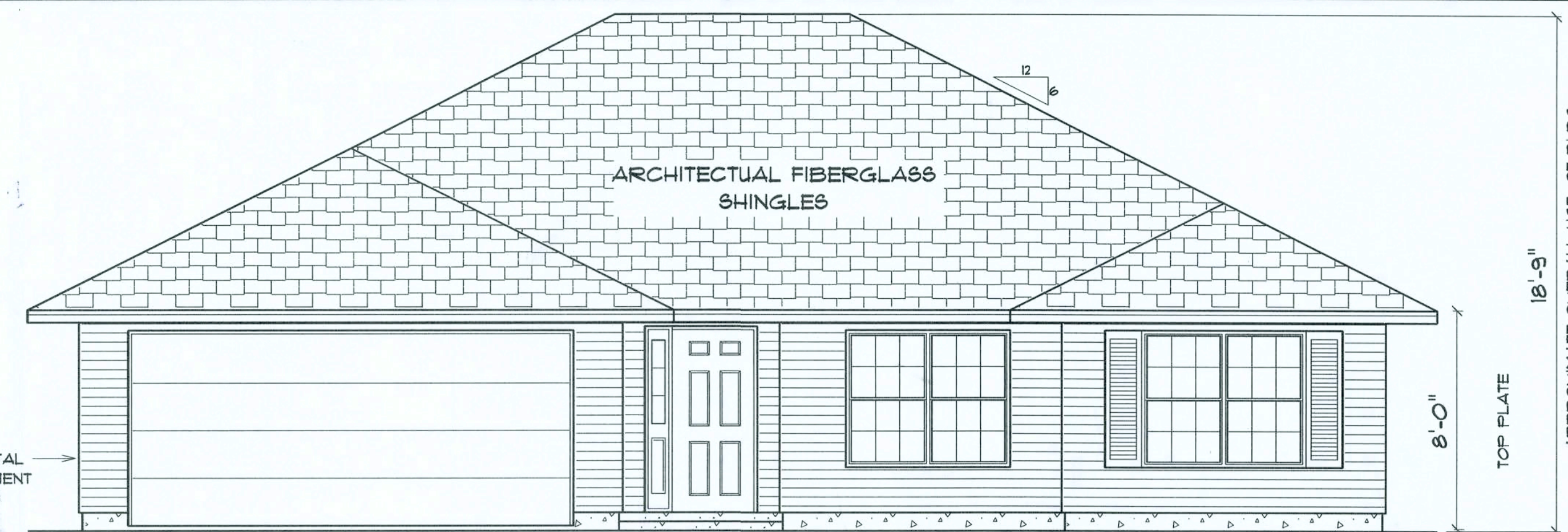


REVISIONS		

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



* FRONT ELEVATION *

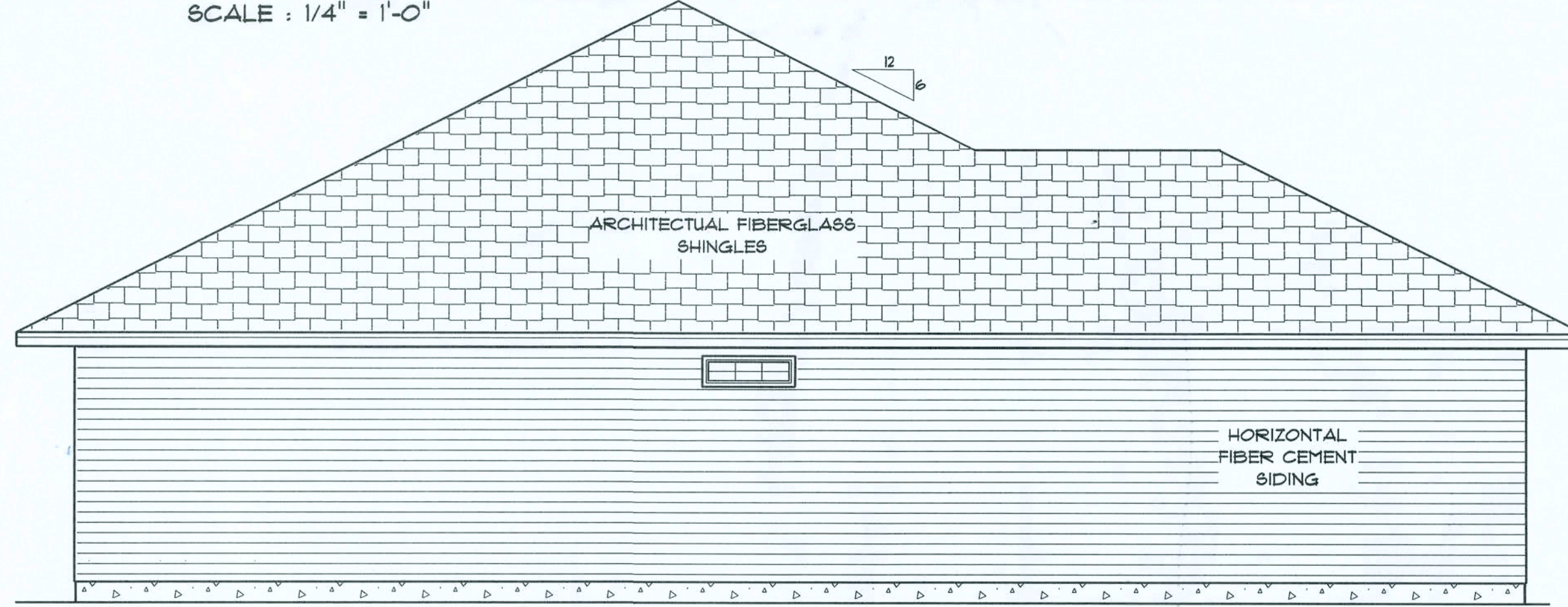
SCALE : 1/4" = 1'-0"

NOTE: 24" OVERHANG ON EAVES
12" OVERHANG ON GABLE ENDS



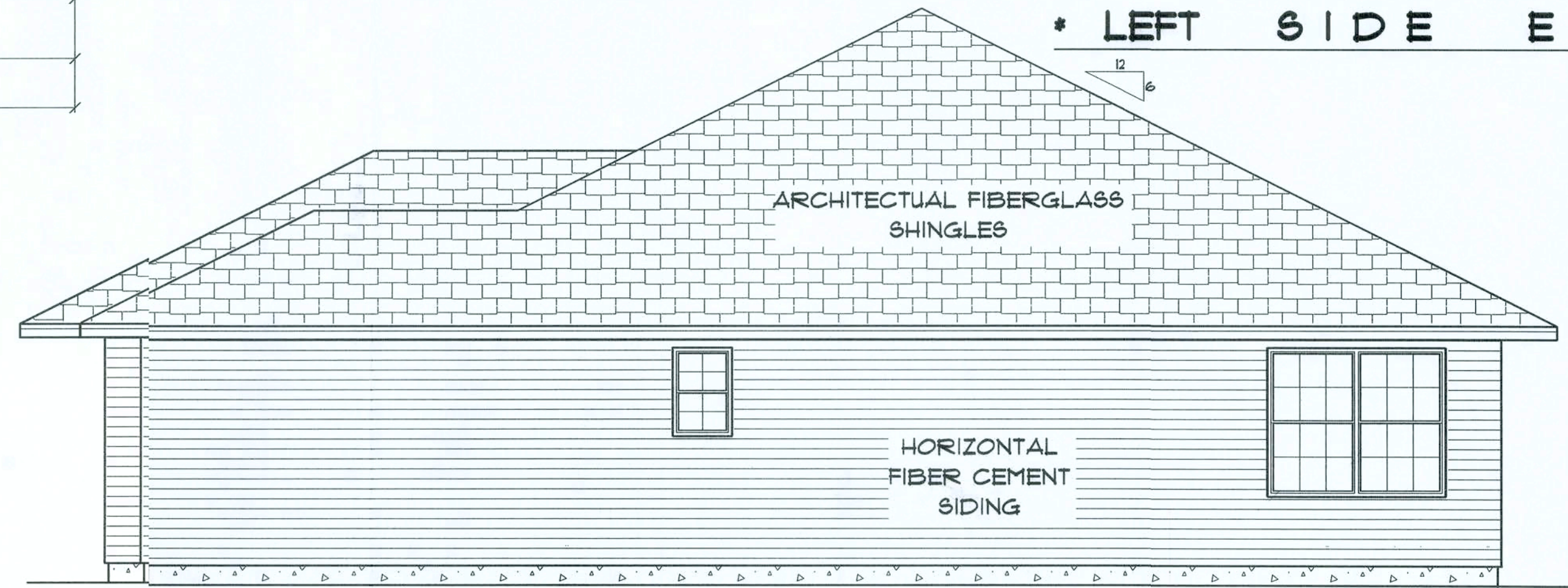
* REAR ELEVATION *

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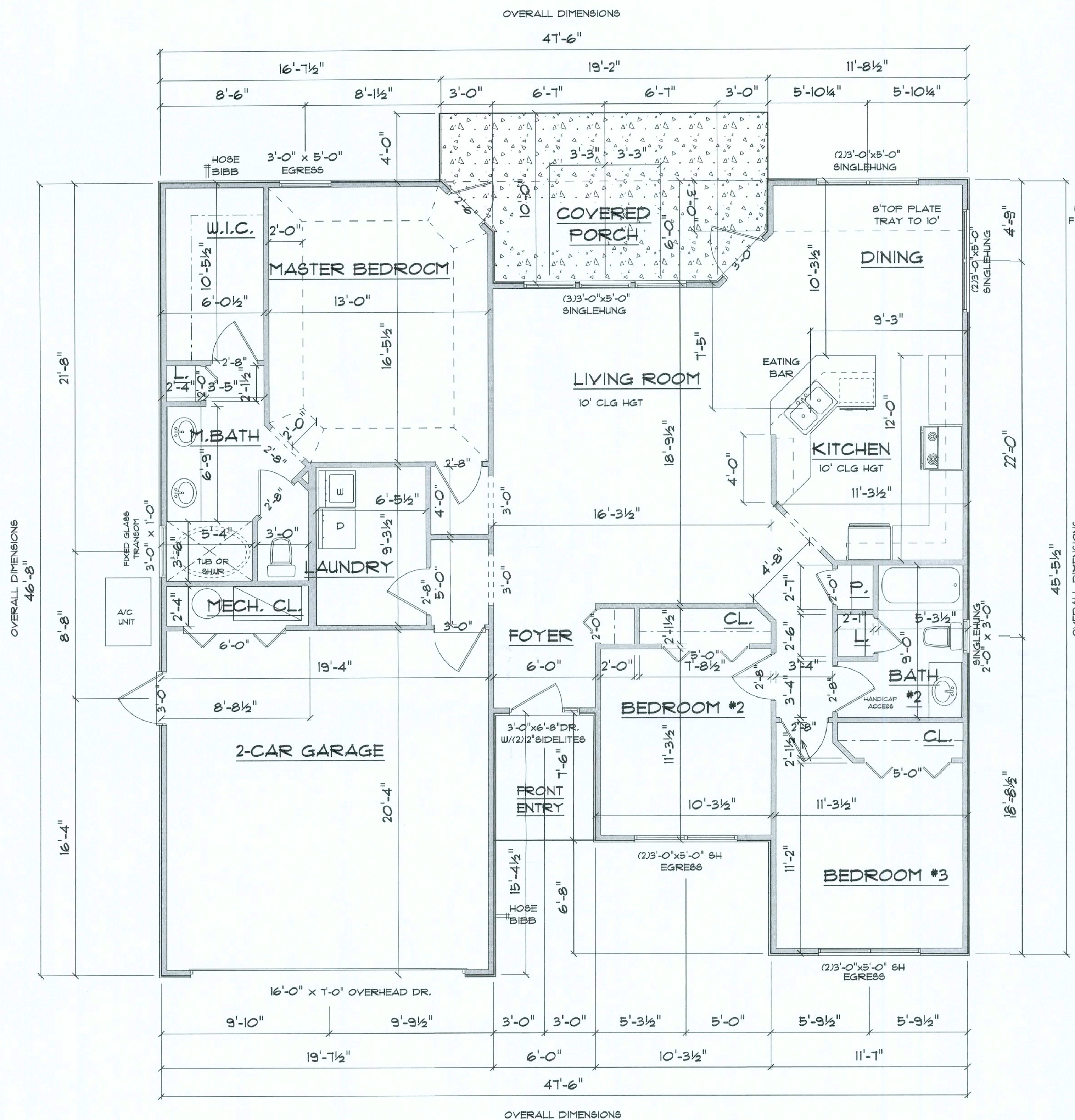
* LEFT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"



* RIGHT SIDE ELEVATION *

SCALE : 1/4" = 1'-0"



* FLOOR PLAN *

R302.5.1 Opening protection:
Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches in thickness, solid or honeycomb-core steel doors not less than 1 3/8 inches thick, or 20-minute fire-rated doors, equipped with a self-closing device.

TABLE R302.6 DWELLING/GARAGE SEPARATION:	
SEPARATION	MATERIAL
From the residence and attics	Not less than 1/2-inch gypsum board or equivalent applied to the garage side
From all habitable rooms above the garage	Not less than 5/8-inch Type X gypsum board or equivalent
Structure(s) supporting floor/ceiling assemblies used for separation required by this section	Not less than 1/2-inch gypsum board or equivalent
Garages located less than 3 feet from a dwelling unit on the same lot	Not less than 1/2-inch gypsum board or equivalent applied to the interior side of exterior walls that are within this area

A/C LIVING AREA =	1500 S.F.
FRONT ENTRY =	44 S.F.
2-CAR GARAGE & MECH.CL. =	430 S.F.
REAR PORCH (UNDER ROOF) =	100 S.F.
TOTAL AREA UNDER ROOF =	2074 S.F.



1500 Model
- Left -

Lot 19 Haight Ashbury
Columbia County, FL

PRINTED DATE:
Tuesday, August 13, 2019

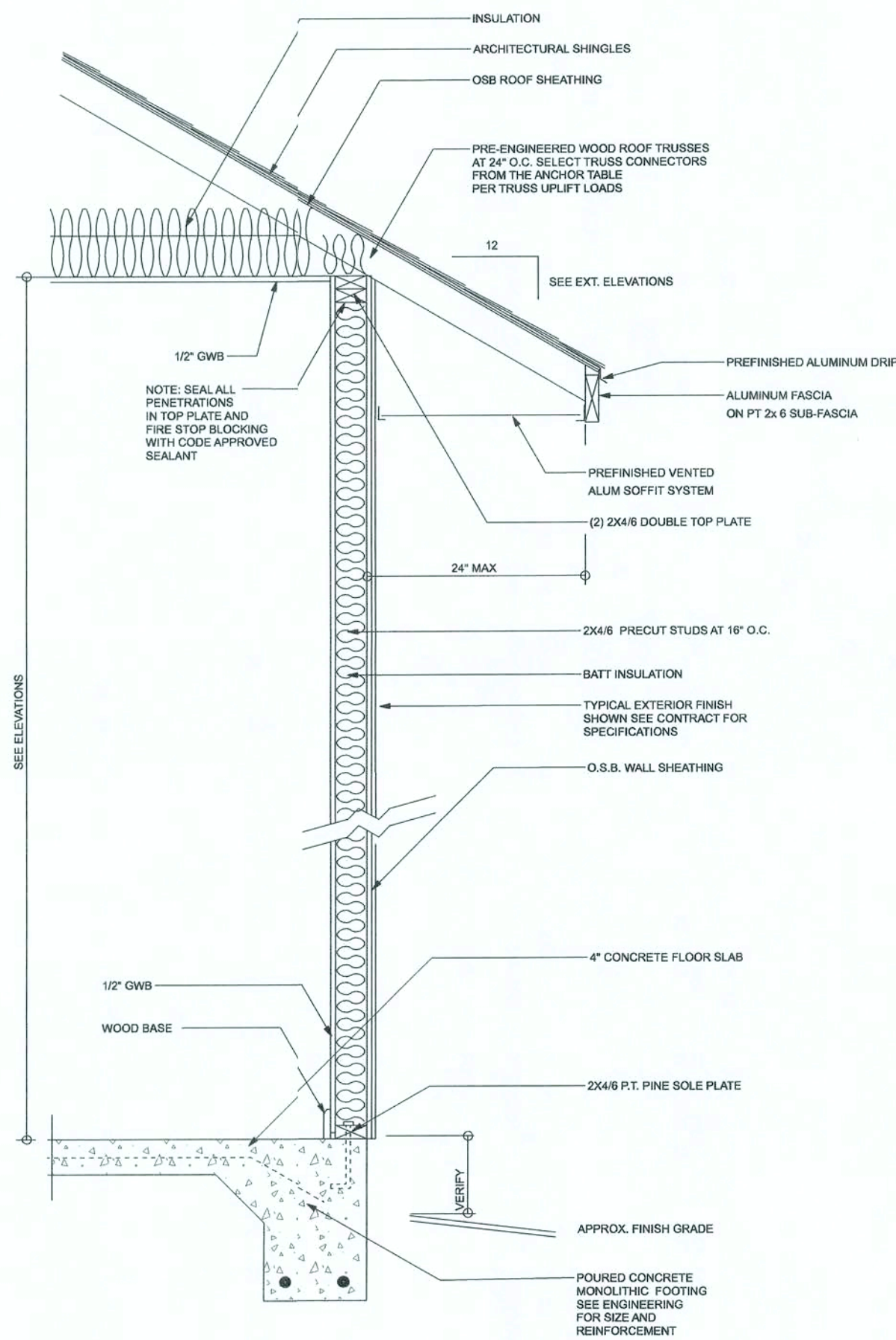
DRAWING NUMBER

A-1

OF 2 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

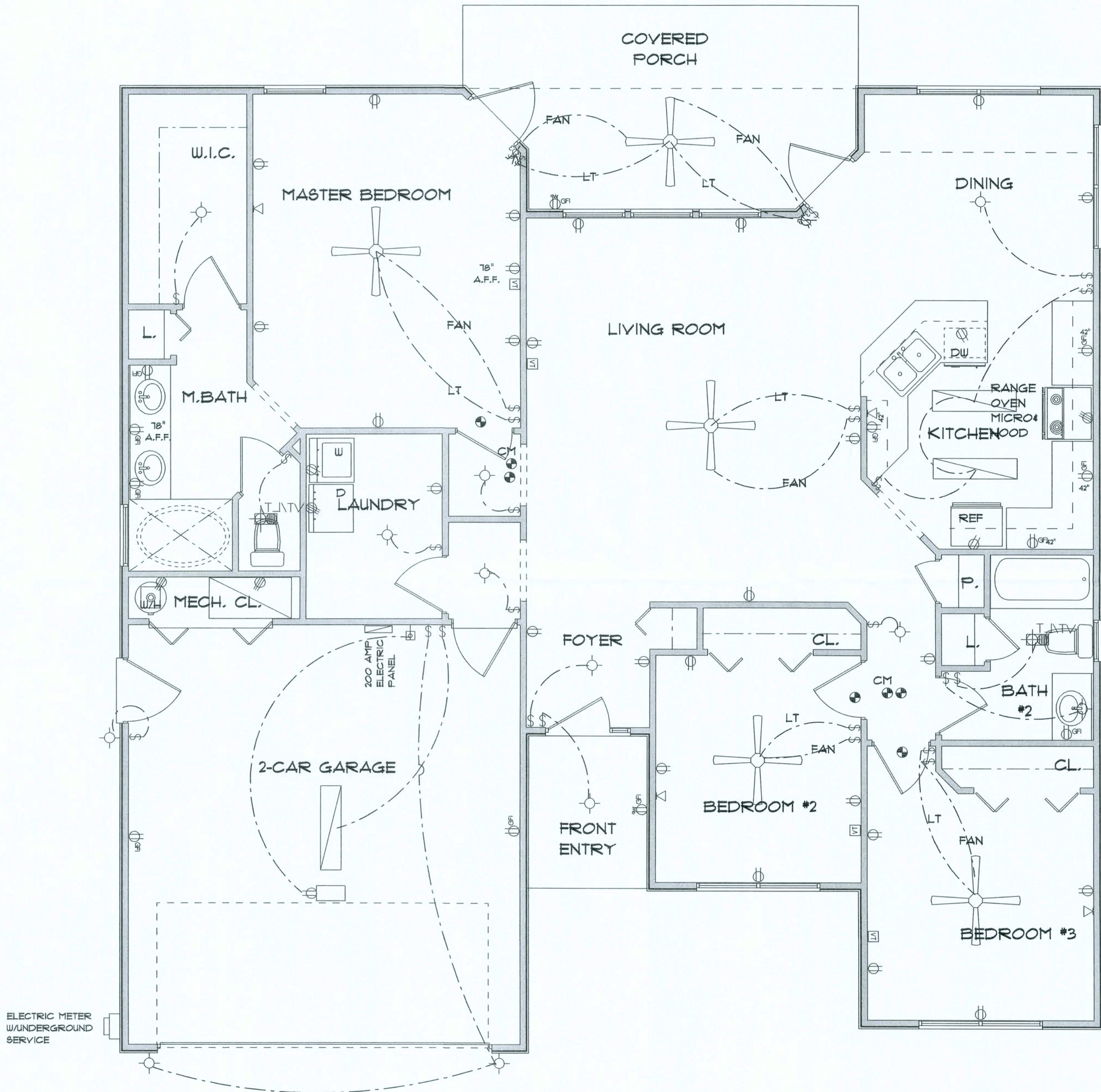


TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 3/4\" = 1'-0"

ELECTRICAL	COUNT	SYMBOL
ceiling fan	3	
200 amp service panel	1	
Light	2	
automatic garage dr opener	1	
cable tv outlet	4	
ceiling fan	2	
electric meter	1	
fluorescent lt fixture	3	
garage dr opener	1	
gfi waterproof outlet	2	
light	10	
outlet	29	
outlet 220v	4	
outlet gfi	10	
smoke detector	5	
switch	22	
switch 3 way	6	
telephone	4	
vent light combo	2	

ELECTRICAL PLAN NOTES:	
E - 1	WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
E - 2	CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
E - 3	ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
E - 4	ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND) NEAR ALL BEDROOMS.
E - 5	TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
E - 6	ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
E - 7	ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
E - 8	ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT: FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO) PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
E - 9	ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.
E - 10	A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
E - 11	CARBON MONOXIDE ALARMS SHALL BE REQUIRED) WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
E - 12	ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.



* ELECTRICAL PLAN *

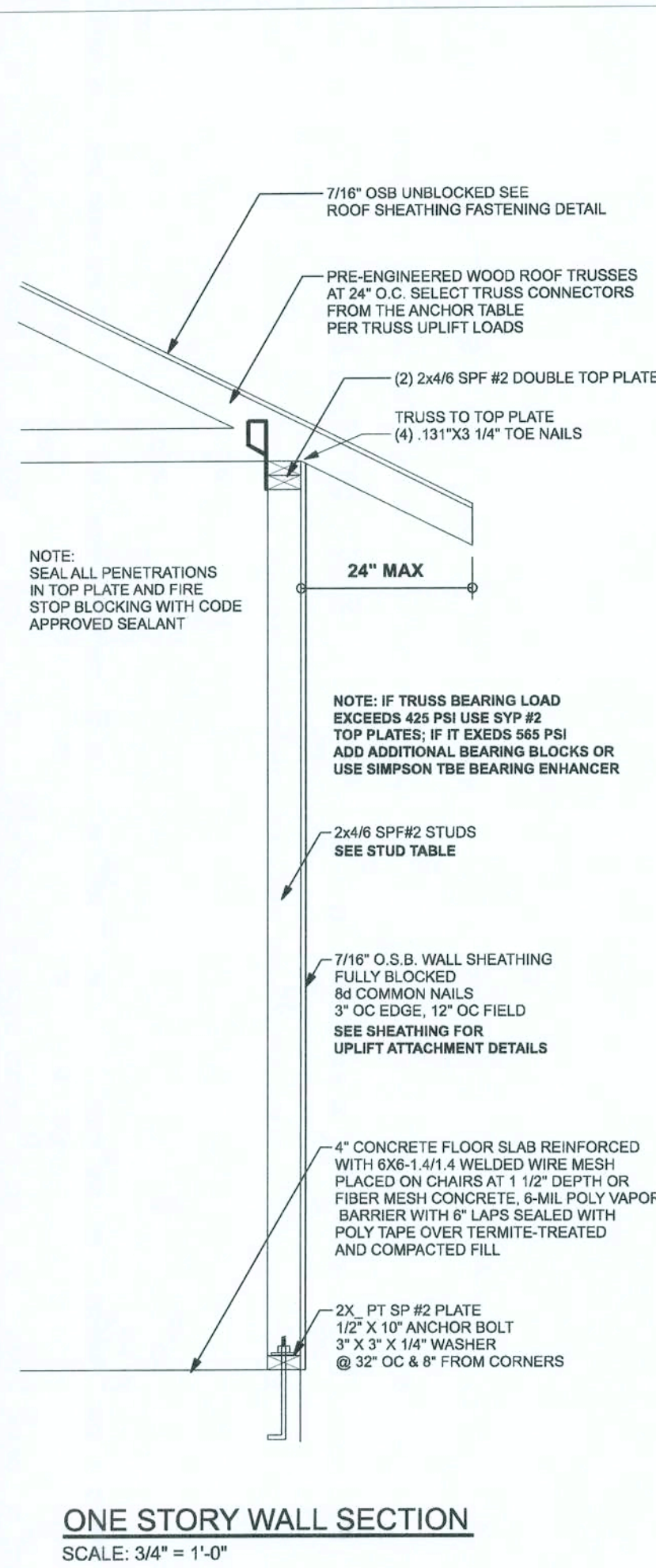
SCALE : 1/4\" = 1'-0"

1500 Model
- Left -

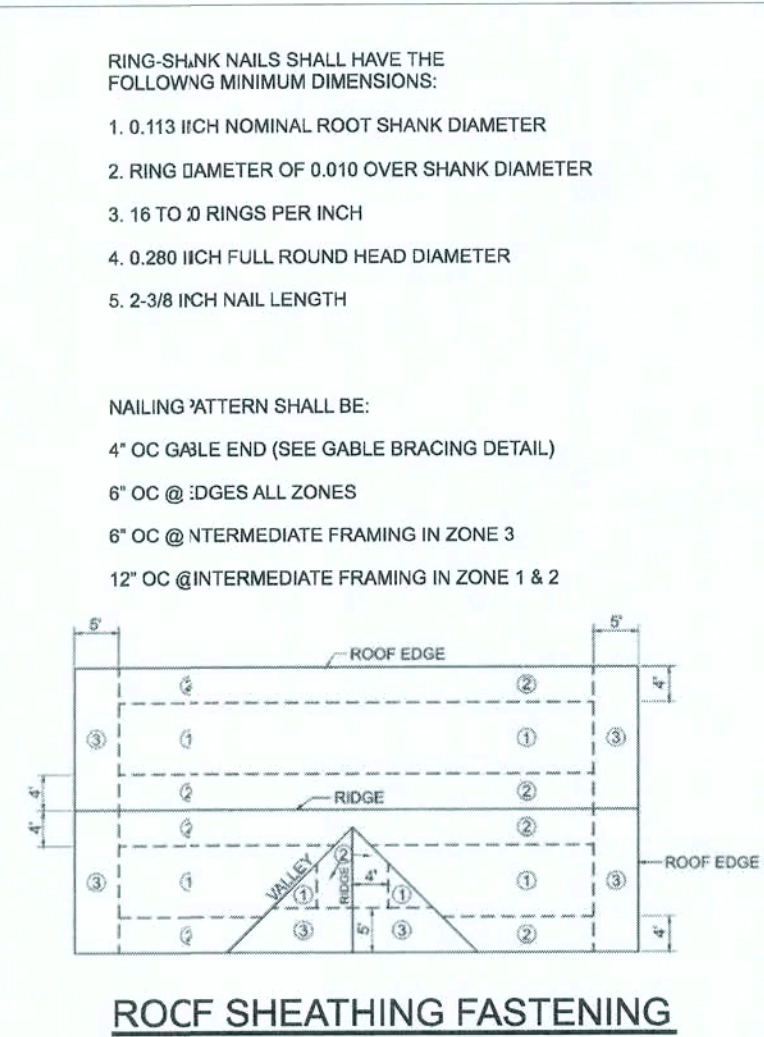
Lot 19 Haight Ashbury
Columbia County, FL

PRINTED DATE:
Tuesday, August 13, 2019

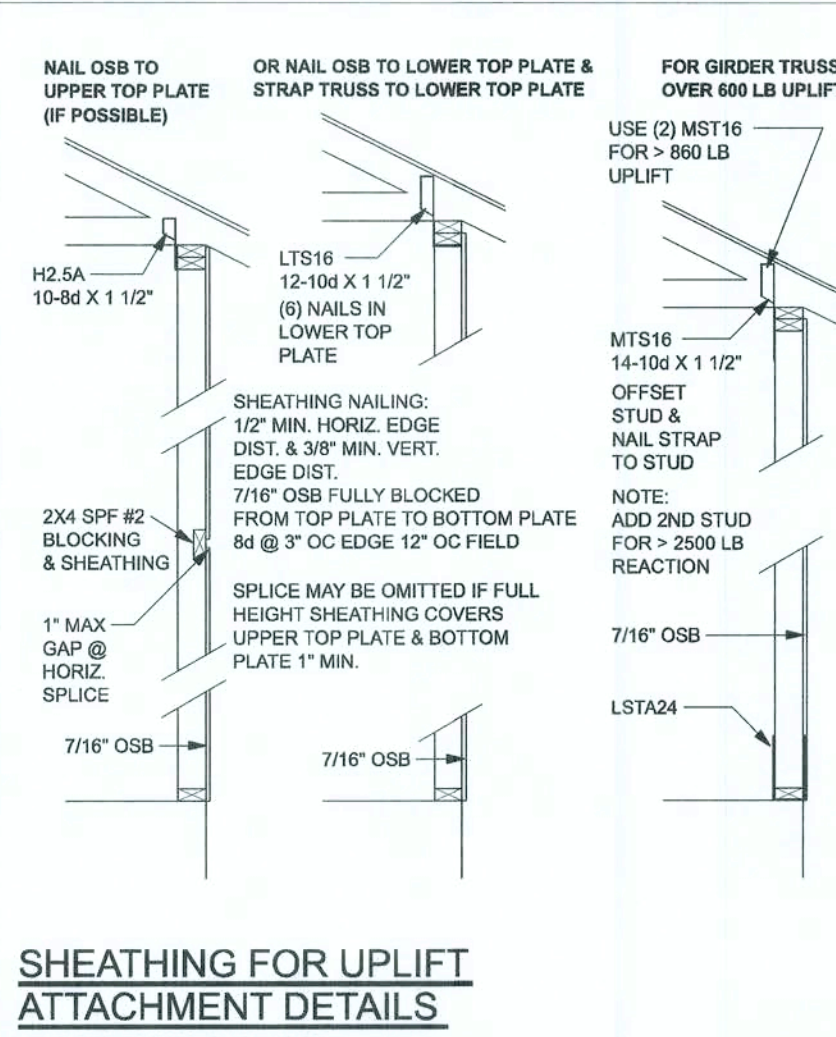
DRAWING NUMBER
A-2
OF 2 SHEETS



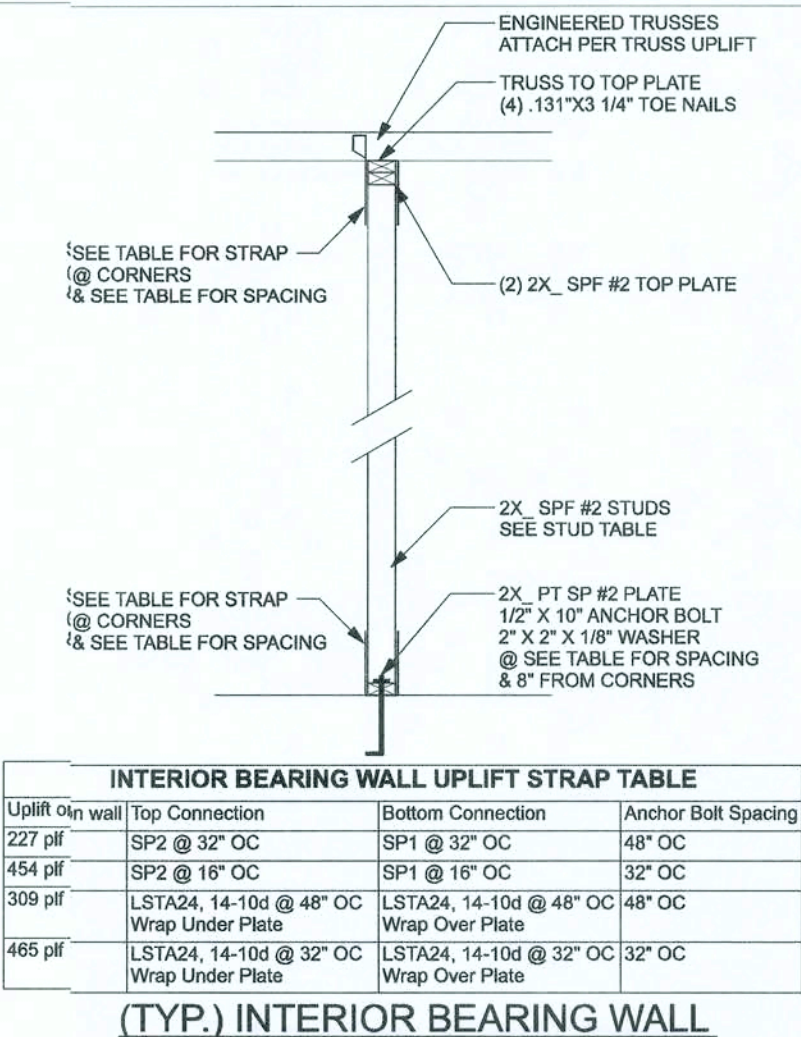
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



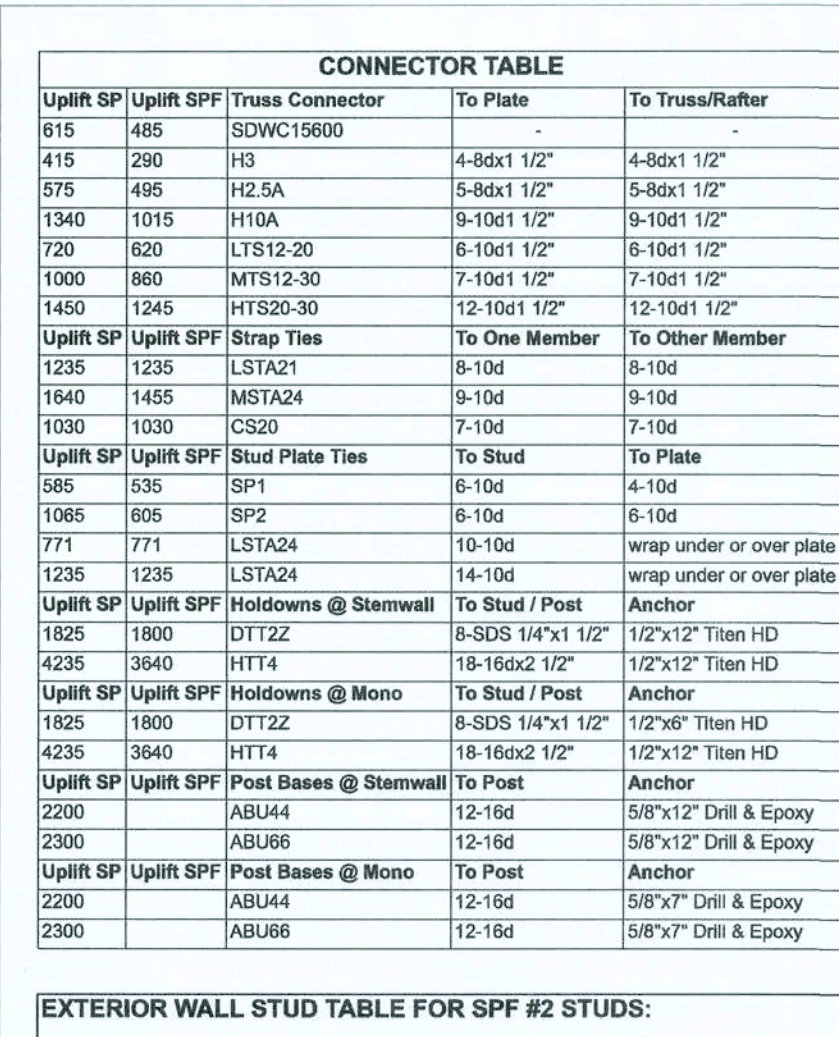
ROOF SHEATHING FASTENING



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



CONNECTOR TABLE

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 8" W14 x W14, FB = 89SL, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185 LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WEIGHT RATIOS OF EXPOSED AREA SHALL NOT EXCEED 1.5 TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. RECOMMEND LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPLICES 40" DB FOR #6 BARS; UNCL. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION, SOIL BEARING CAPACITY, GRADE, AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

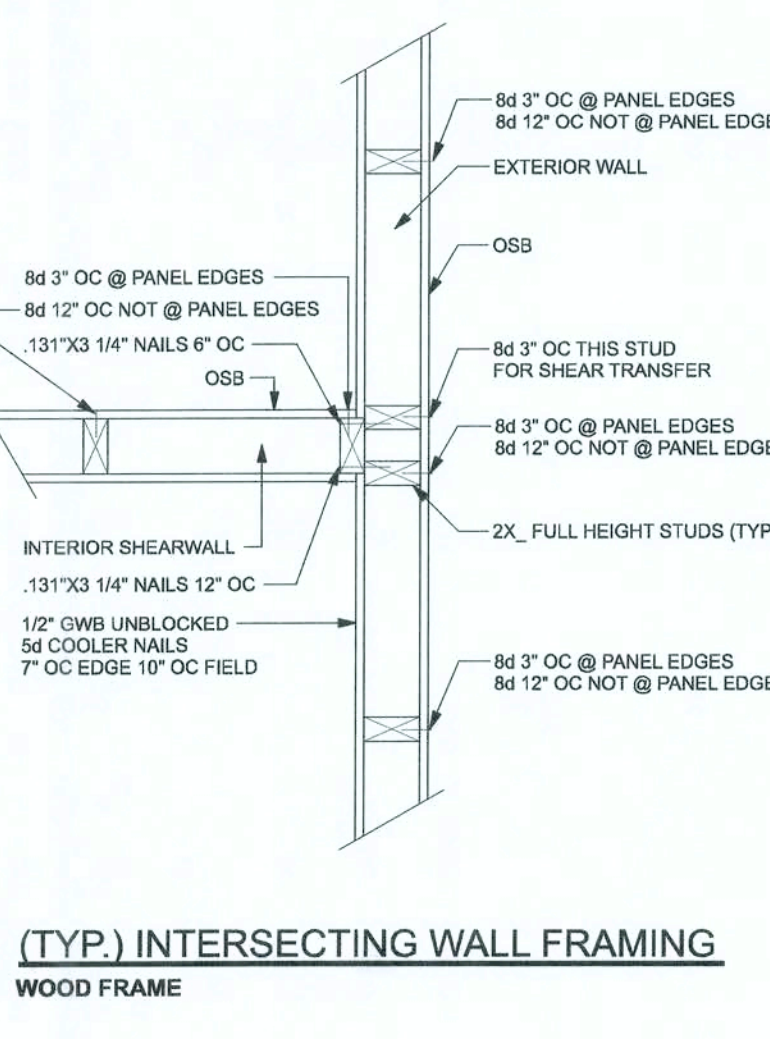
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN LIMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

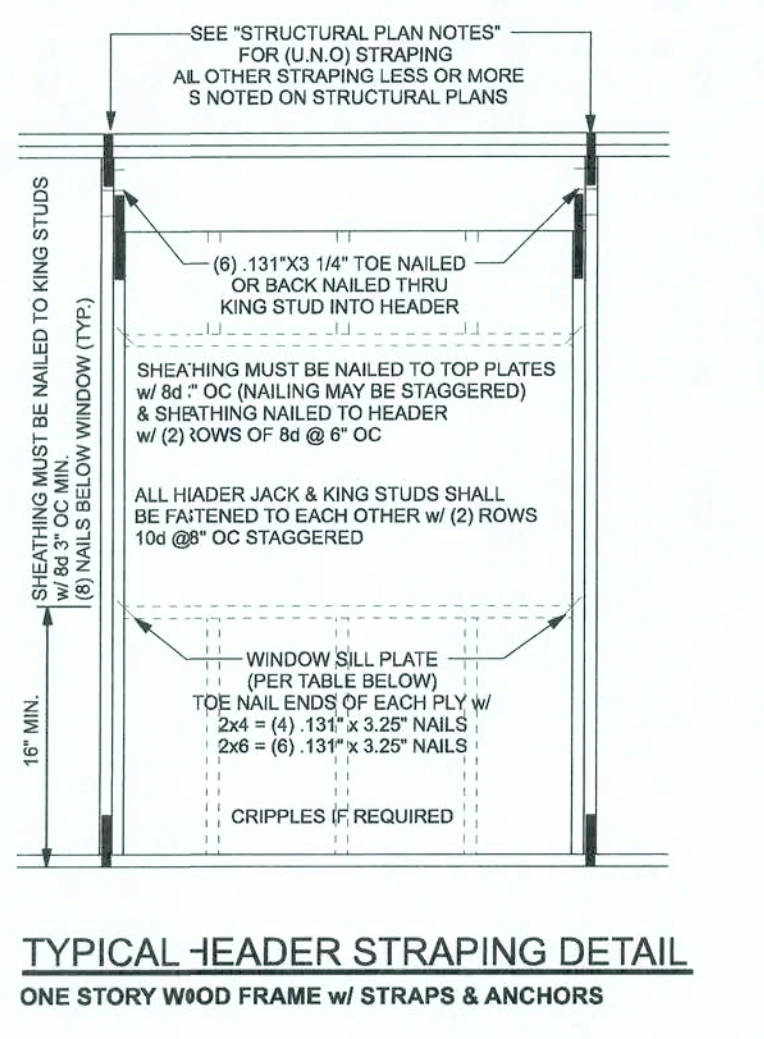
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

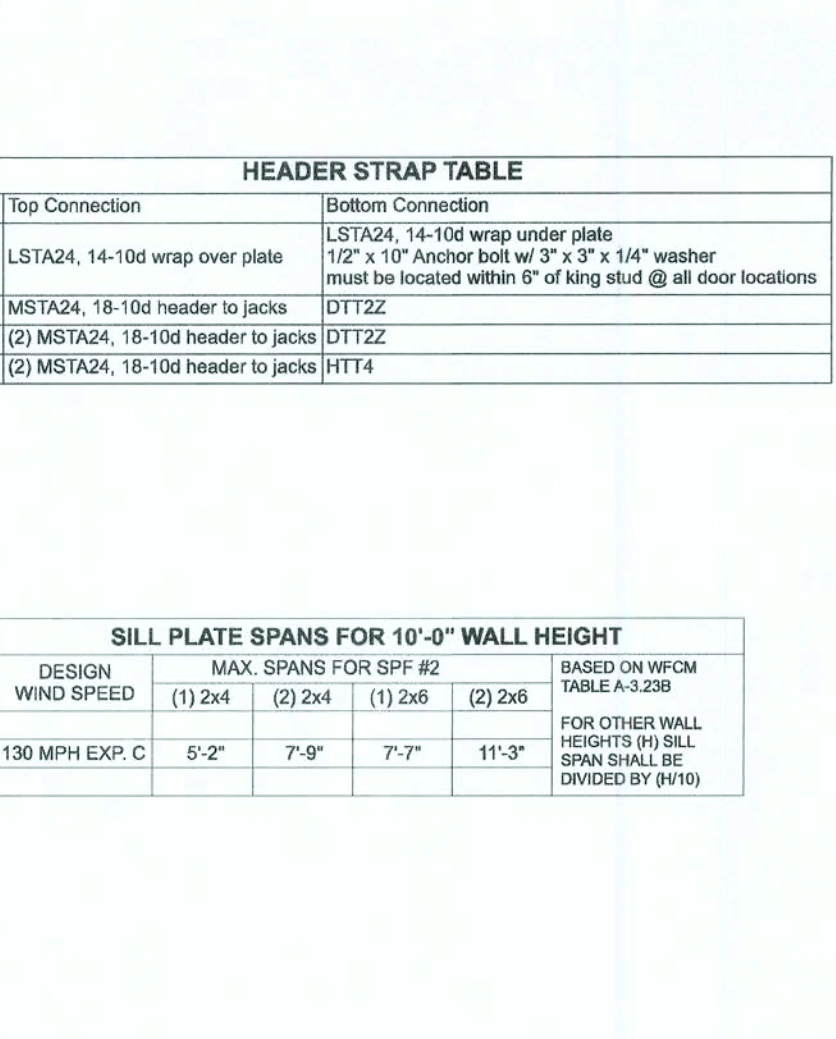
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



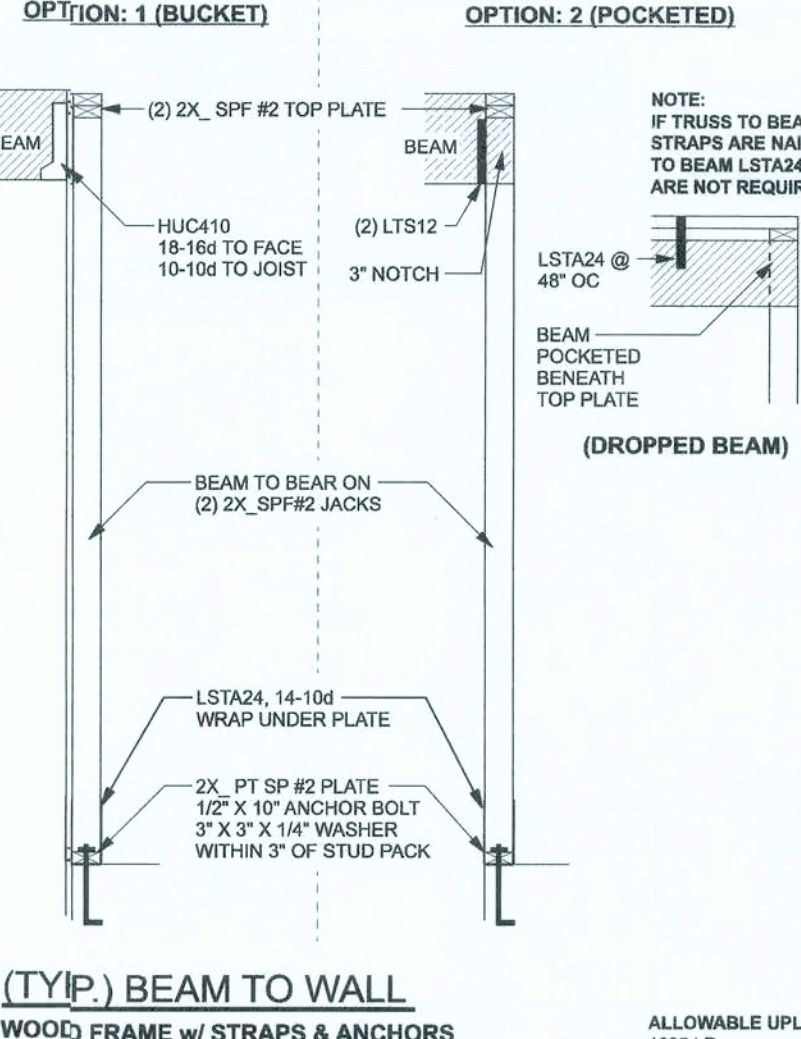
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



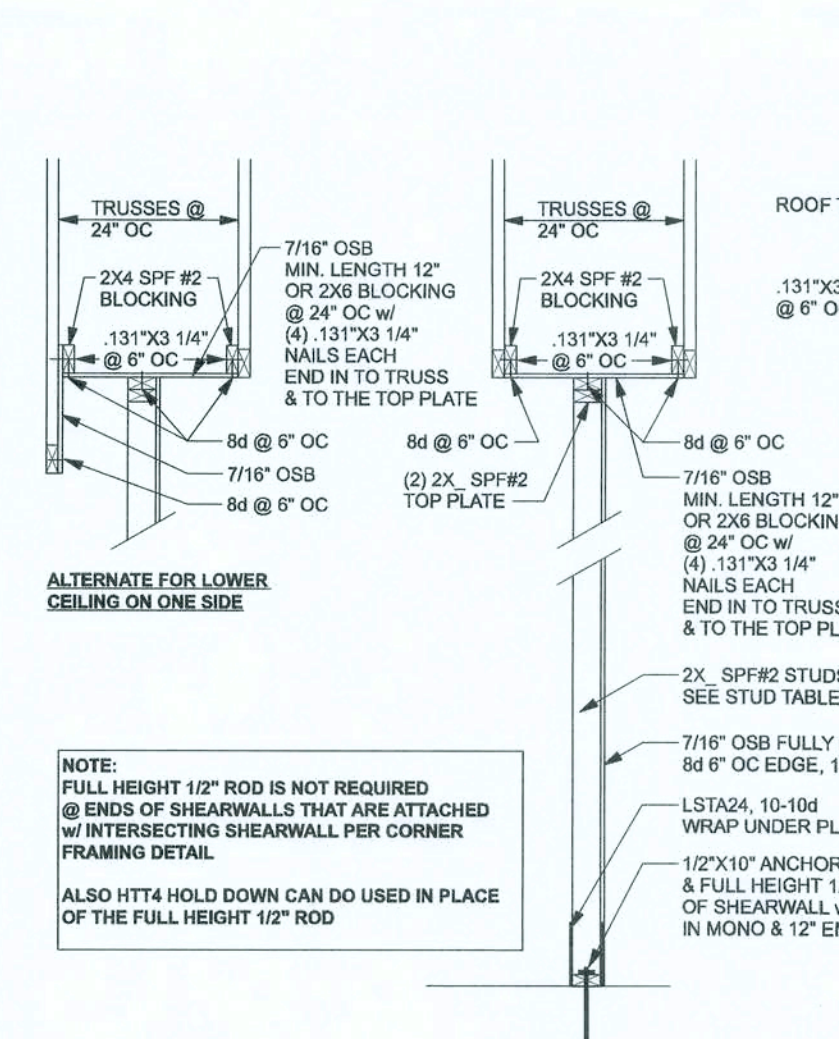
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



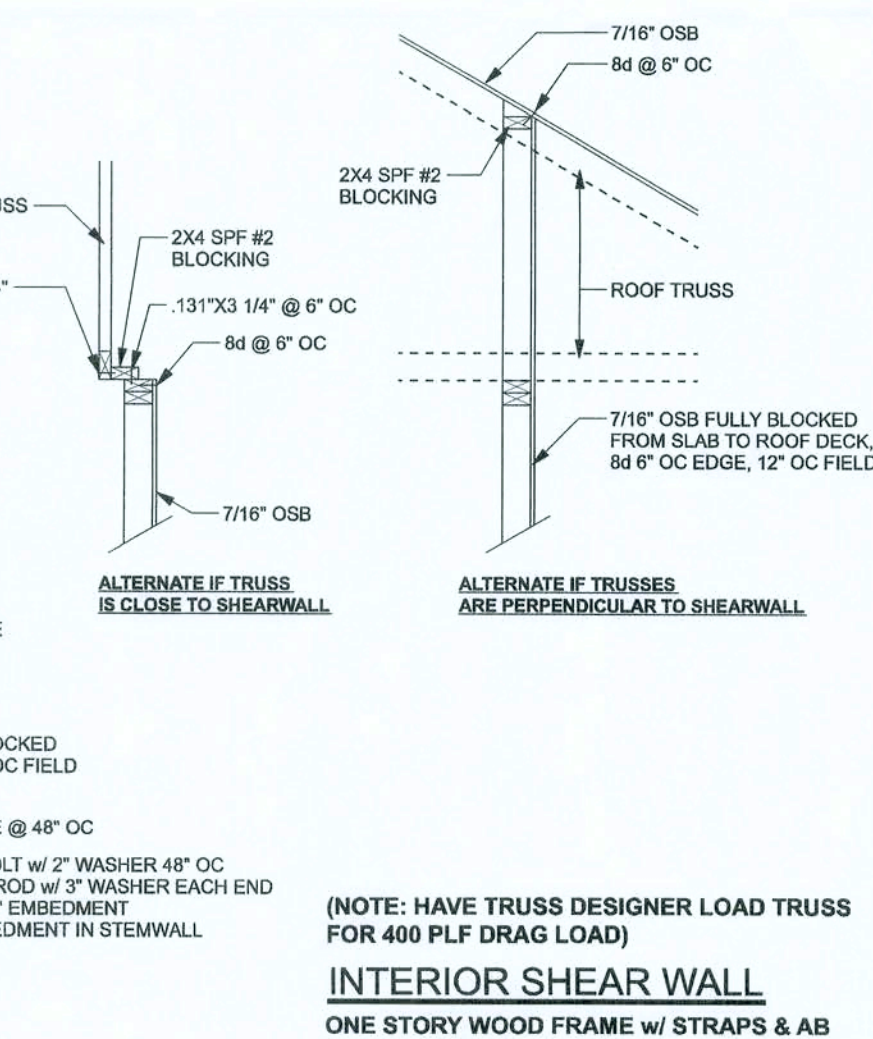
HEADER STRAP TABLE



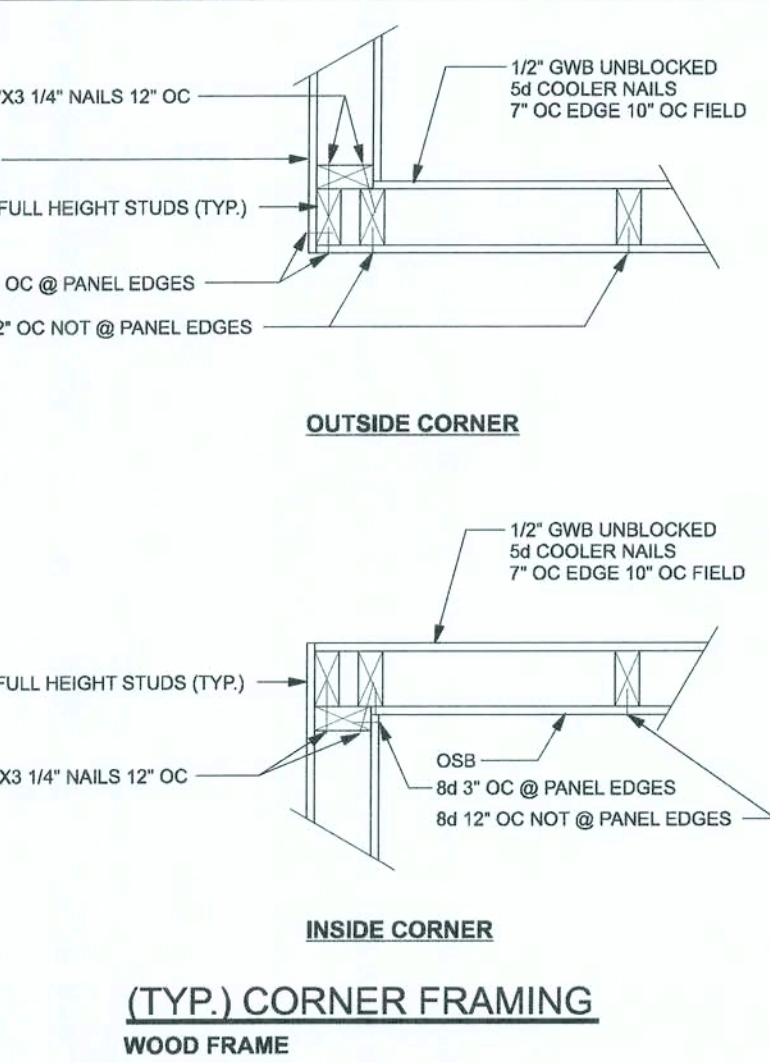
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



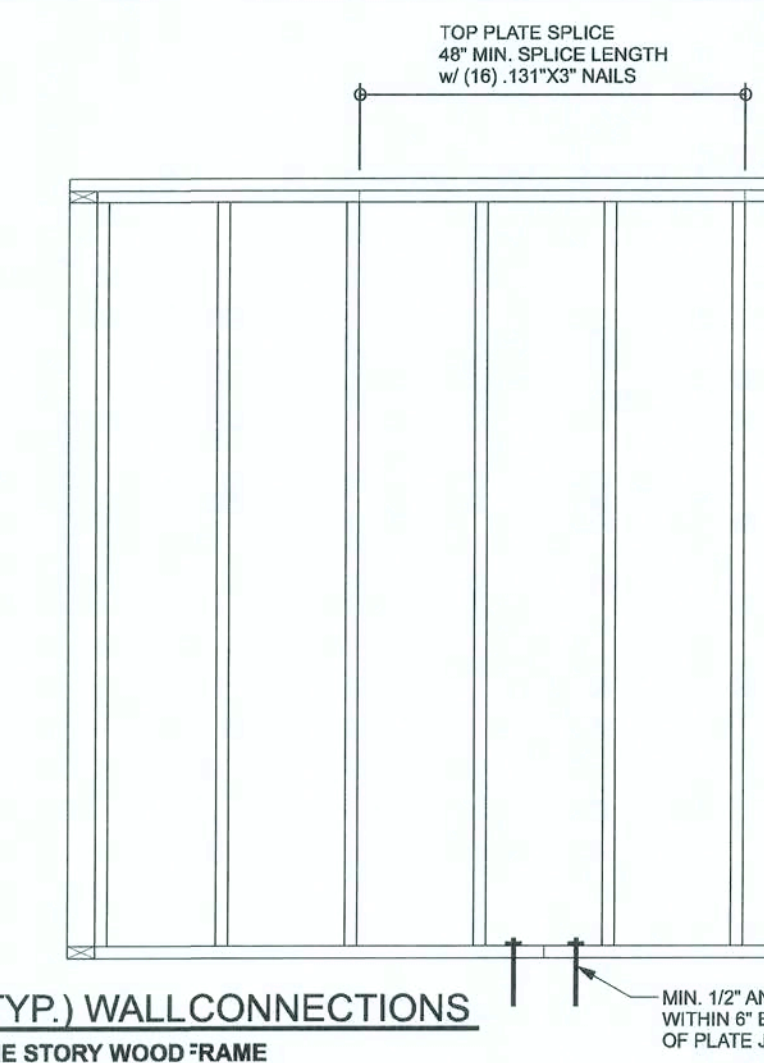
OPTION: 1 (BUCKET)



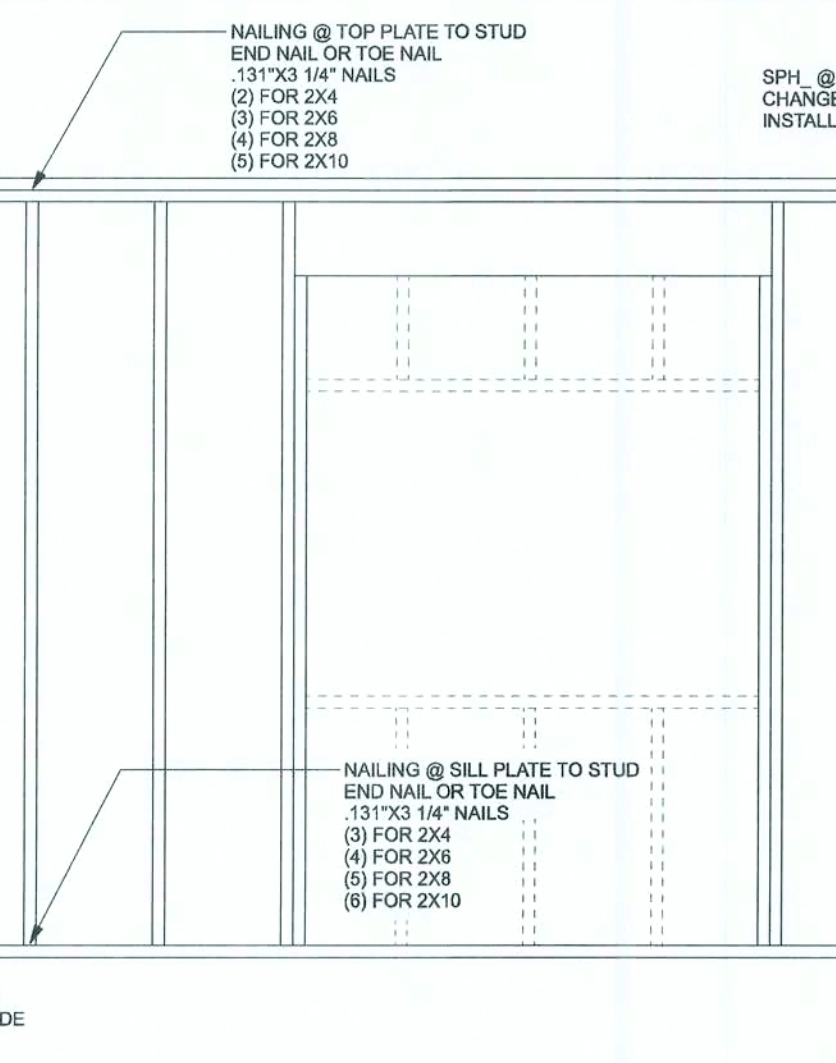
OPTION: 2 (POCKETED)



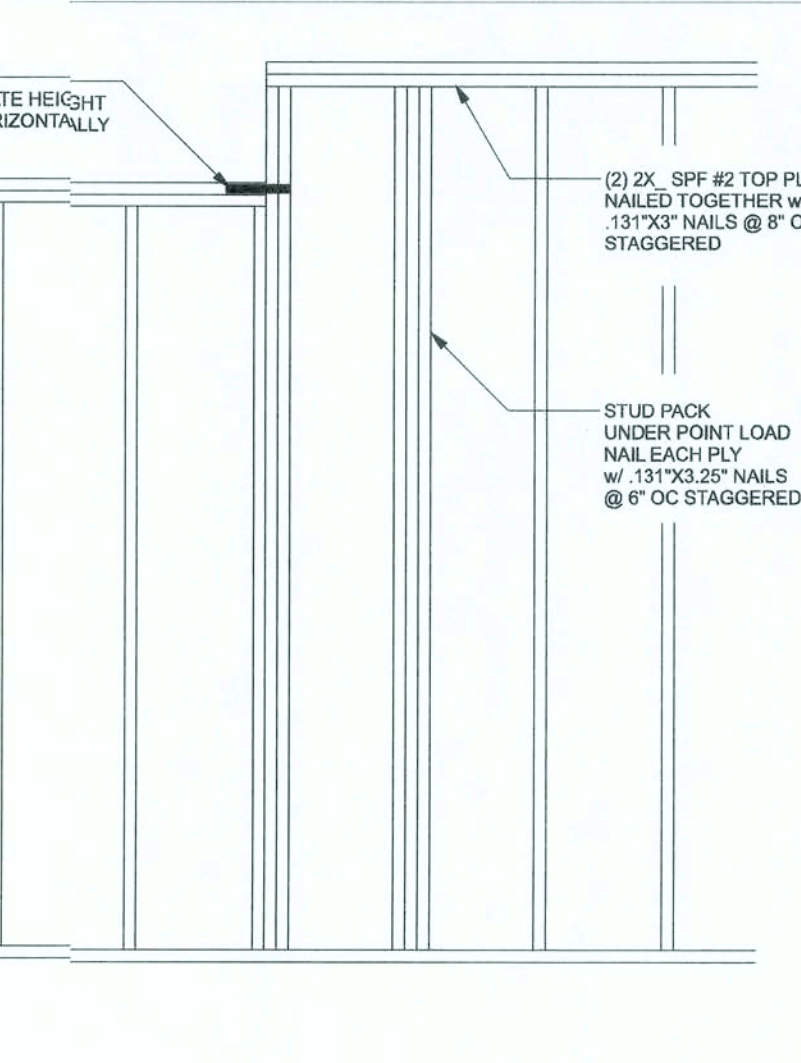
(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



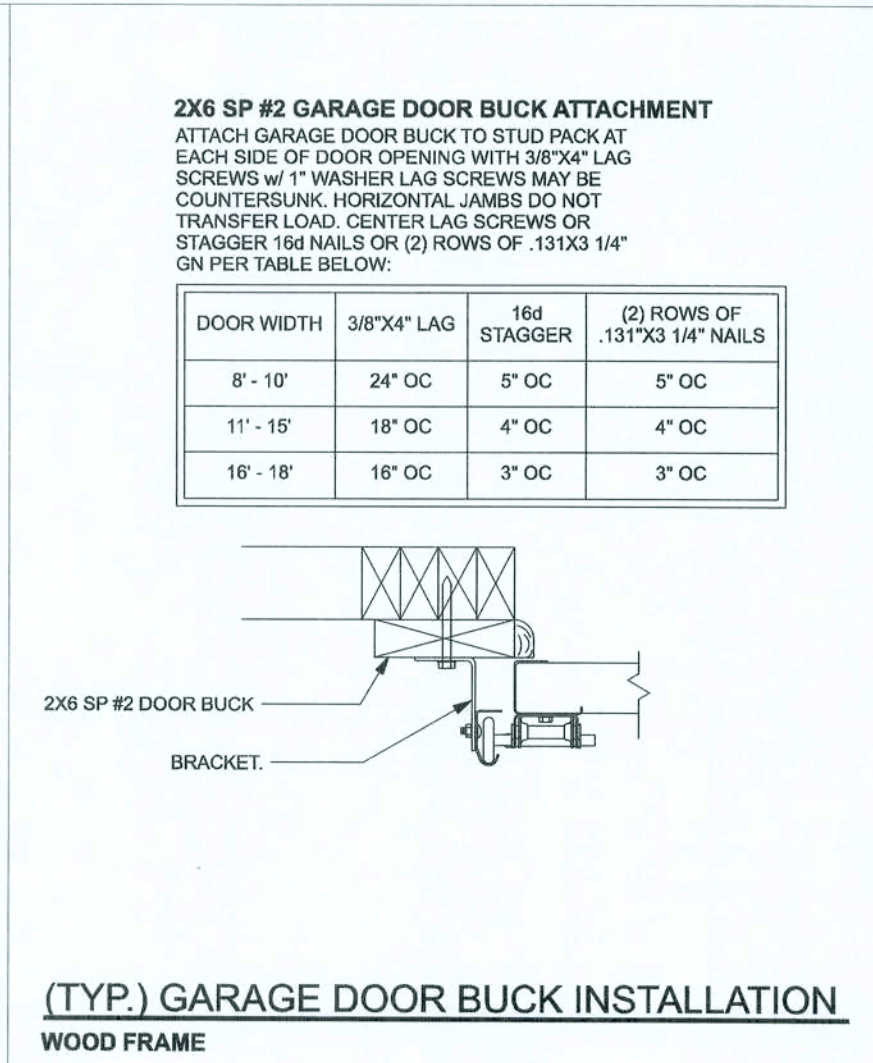
SILL PLATE SPANS FOR 10'-0\"/>



(TYP.) GARAGE DOOR BUCK ATTACHMENT
WOOD FRAME



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME



INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB

DESIGN CRITERIA & LOADS:		
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)	
CODE FOR DESIGN LOADS	ASCE 7-10	
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&C DESIGN PRESSURES		
FLOOR LOADING	SEE TABLE	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (Vult)		
EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+42.6 -46.2	+42.6 -57
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)		
8x7 GARAGE DOOR	+22.8 -26.5	
16x7 GARAGE DOOR	+21.7 -24.1	

Cornerstone Development

1500 Model - Left - Lot 19 Height Ashbury

PROJECT ADDRESS: Lot 19 Height Ashbury, Columbia County, FL

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

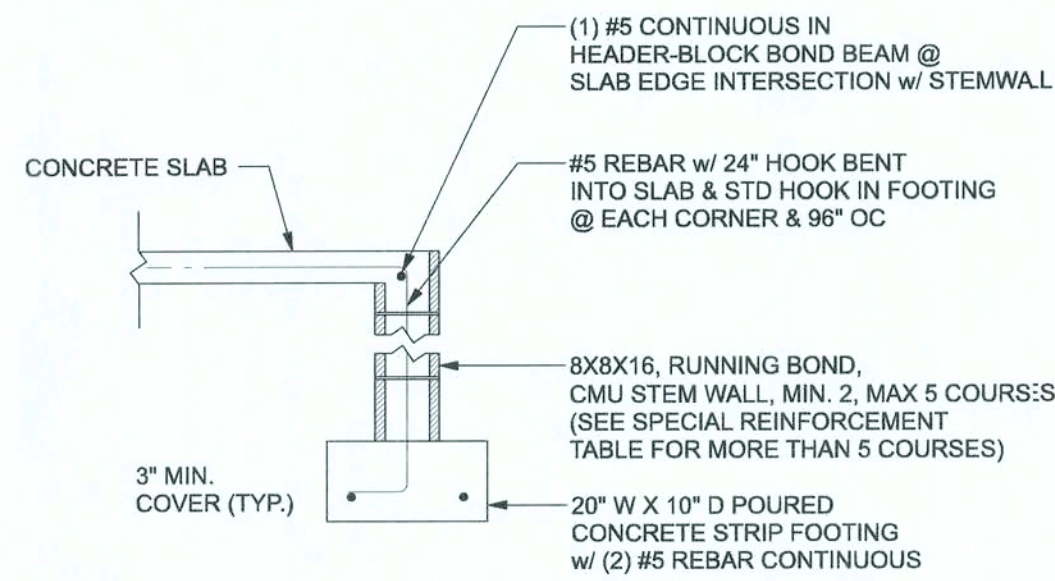
MARK DISCOWAY P.E. 53915

STATE OF FLORIDA
PROFESSIONAL ENGINEER
No. 53915
Expiry Date: August 13, 2019

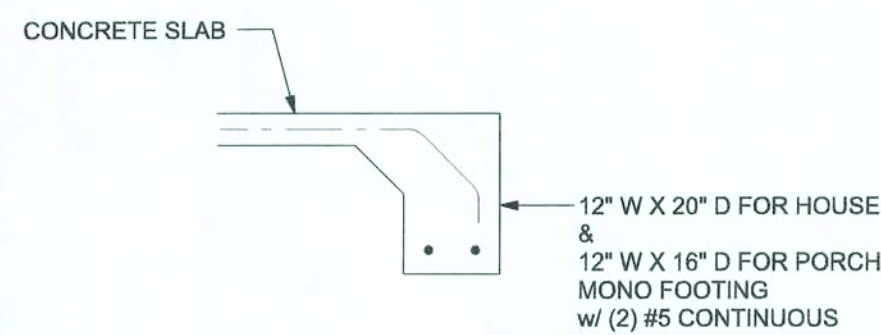
Mark Discoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32825
386.754.5419
discowaydesign@gmail.com

JOB NUMBER: 190927

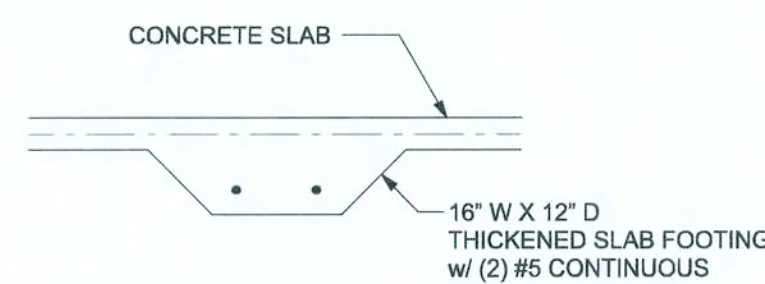
S-1
OF 3 SHEETS



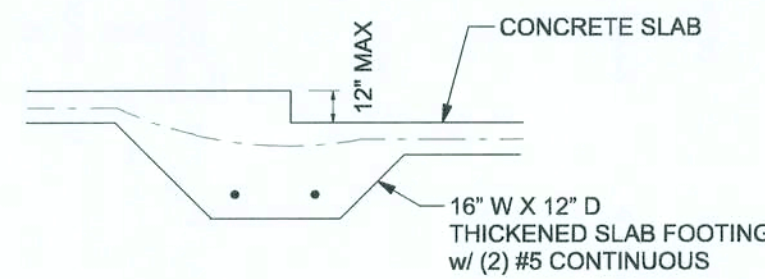
F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



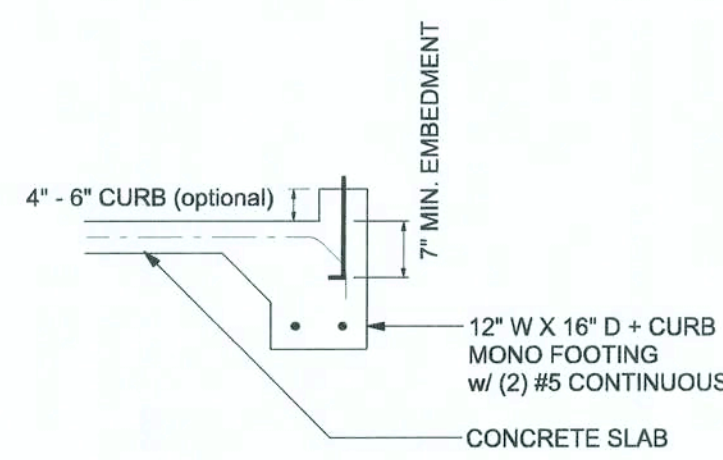
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



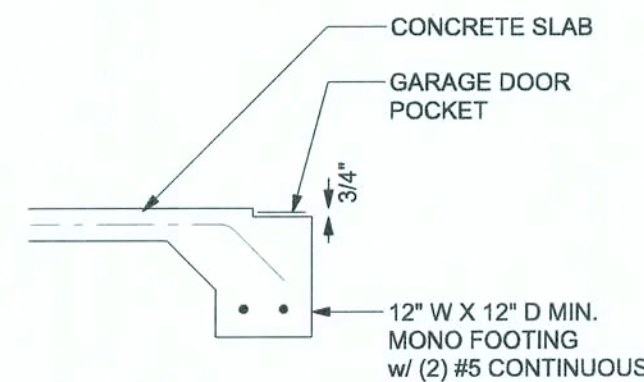
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"

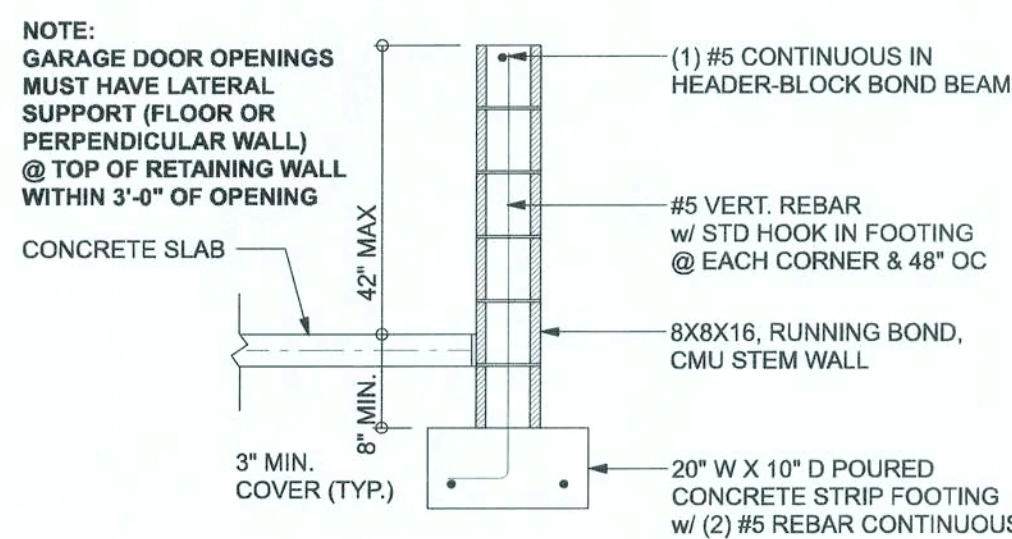
TALL STEM WALL TABLE:
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

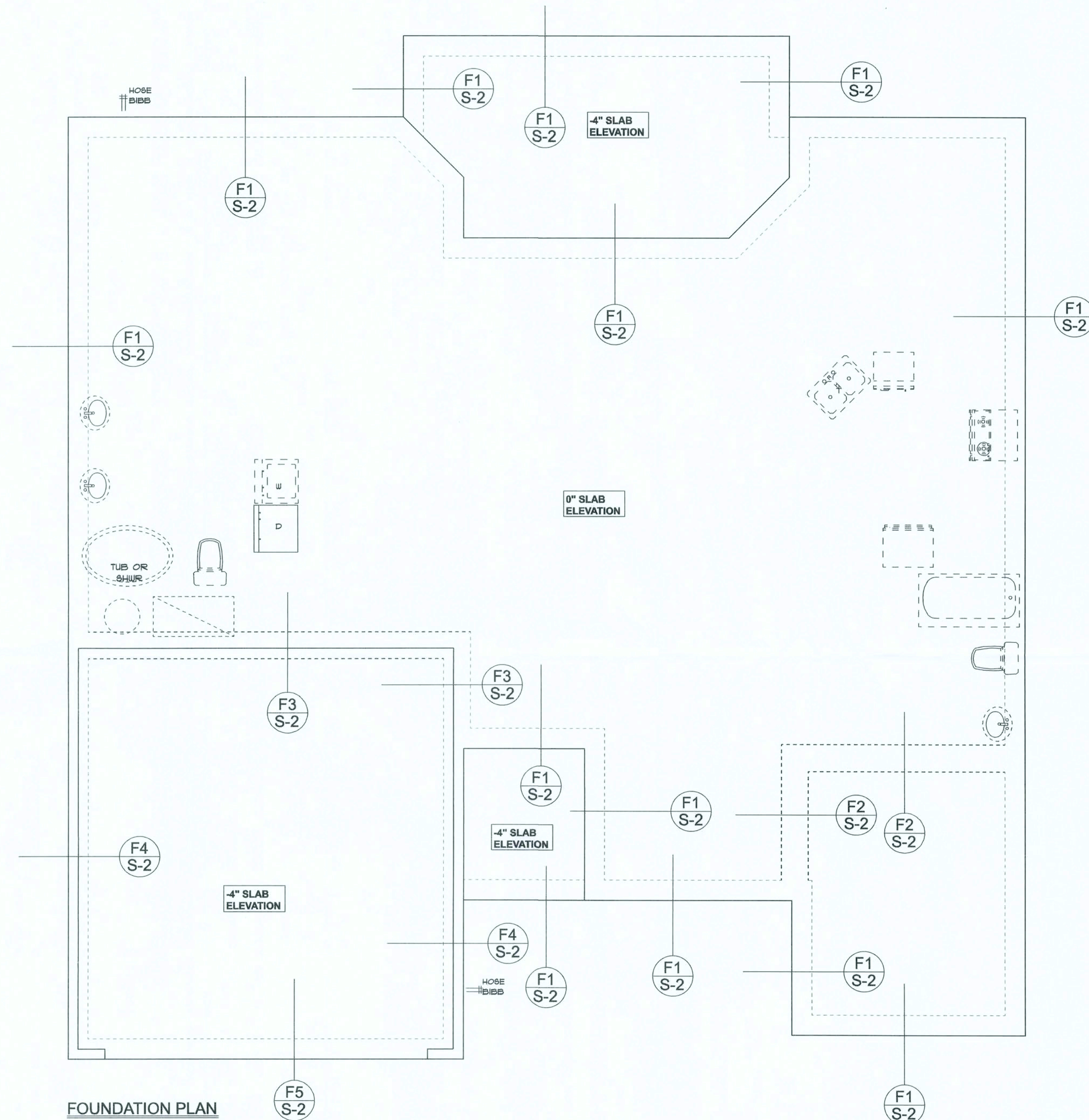
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM A615, Grade 60, Fy = 60 ksi. Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G90, 0.60 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017 RES. SECTION R403.1.4



F4 S-2 OPTIONAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 8X8-14#4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL.

Cornerstone Development

1500 Model - Left - Lot 19 Height Ashbury

PROJECT ADDRESS:
Lot 19 Height Ashbury
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915



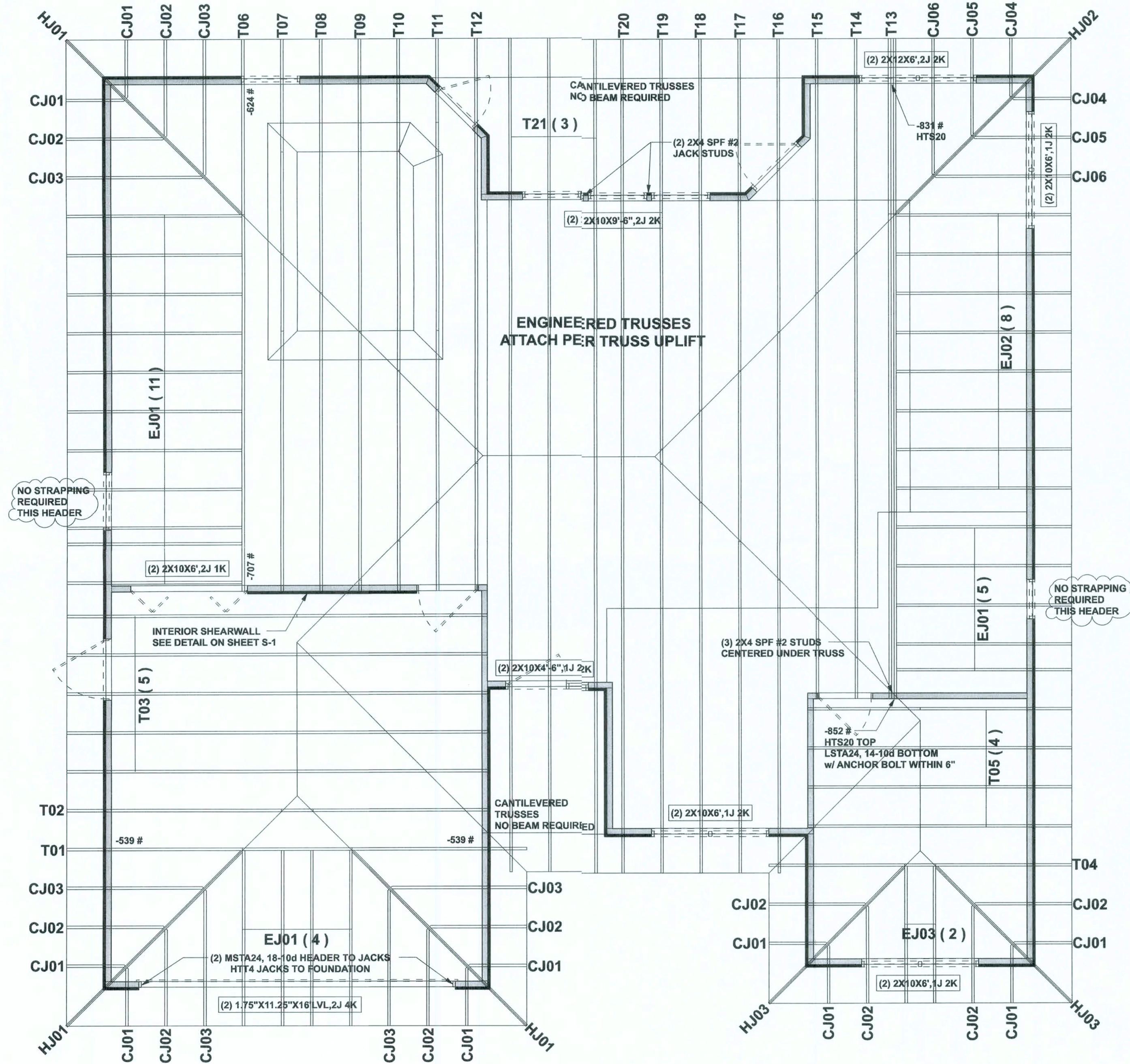
Tuesday, August 13, 2019

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
190927

S-2

OF 3 SHEETS

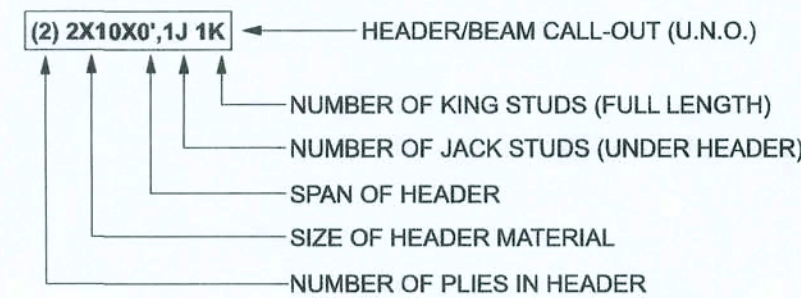


STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03, BCSH-01, BCSH-02, & BCSH-03. BCSH-01, BCSH-02, & BCSH-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	25536 LBF	10728 LBF
REQUIRED	9102 LBF	8439 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #1752715

Cornerstone Development

1500 Model - Left - Lot 19 Haight Ashbury

PROJECT ADDRESS:
Lot 19 Haight Ashbury
Columbia County, FL

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY, P.E. 53915



Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
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S-3
OF 3 SHEETS