DATE 11/15/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025228

APPLICANT HUGO ESCALANTE PHONE 386.288.8666

ADDRESS POB 280 FT. WHITE FL 32038

OWNER MICHAEL KARCHER PHONE 352 213-8582

ADDRESS 1096 SW CUMBERLAND ST FT. WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 386.288.8666

LOCATION OF PROPERTY 47S, TL ON 27, TR ON FRY, TR ON CUMBERLAND, 1 MILE ON LEFT, TURN NEXT TO MAILBOX .

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 90000.00

HEATED FLOOR AREA 1800.00 TOTAL AREA 2739.00 HEIGHT 20.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 16-7S-16-04226-166 SUBDIVISION SHILOH RIDGE

LOT 66 BLOCK PHASE UNIT TOTAL ACRES 10.00

		CRC1326967		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
EXISTING	06-0970-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: 1 FOOT ABOVE.

Check # or Cash 4732

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 450.00 CERTIFICATION FEE $ 13.70 SURCHARGE FEE $ 13.70

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 552.40

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0611-18 Date Received 11/8 By JW Permit # 25228

Application Approved by - Zoning Official BLK Date 08-11-06 Plans Examiner OKTH Date 11-8-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments - NOC - ~~SHAW~~

Applicants Name Hugo Escalante Phone 386-288-8666

Address P.O. Box 280, Ford White, FL 32038

Owners Name Michael Bruce & Edith Karcher Phone 352-213-8582

911 Address 1096 S.W. Cumberland ST, Ford White, FL 32038

Contractors Name Hugo Escalante (EWPL Inc) Phone 386-288-8666

Address P.O. Box 280 Ford White, FL 32038

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec) - Suwannee Valley Elec. - ~~Progressive Energy~~

Property ID Number 16-7S-16-04226-166 Estimated Cost of Construction $210,000

Subdivision Name Shiloh Ridge Lot 66 Block ____ Unit ____ Phase ____

Driving Directions 47 South, T/L @ US 27, Go 2miles to FRY Road T/R to Cumberland T/L Follow driveway to house.

Type of Construction New Single Family Residence Number of Existing Dwellings on Property 0

Total Acreage 10 Acre Lot Size 10 Acre Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 200' Side 100' Side 100' Rear 200'

Total Building Height 20'-0" Number of Stories 1 Heated Floor Area 1800 SF Roof Pitch 6-12

TOTAL 2739

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
Owner Builder or Agent (Including Contractor)

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

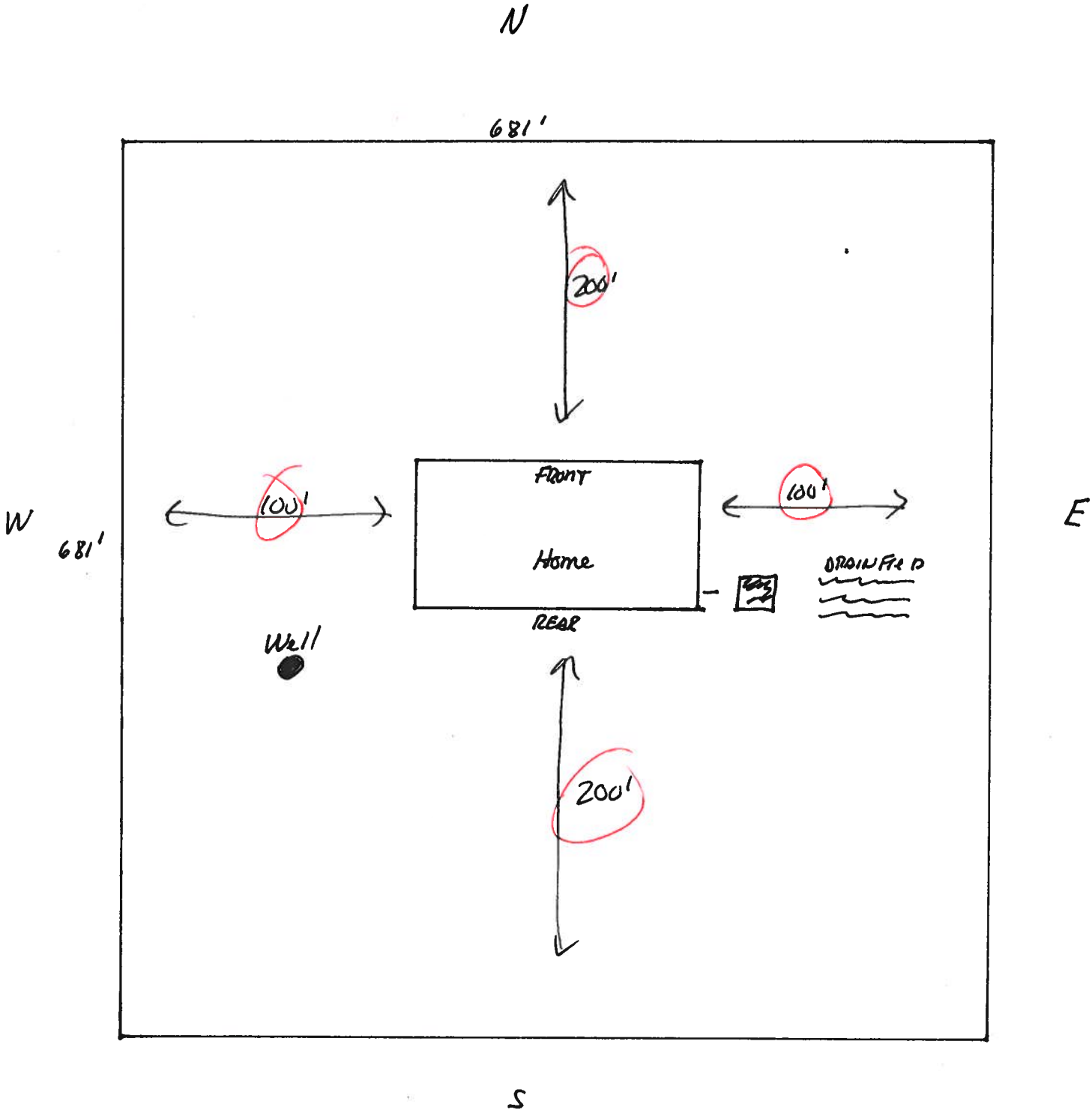
Lawanda Y. Collins
MY COMMISSION # DD246441 EXPIRES
October 29, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

Sworn to (or affirmed) and subscribed before me this 8 day of November 20 06.

Personally known ____ or Produced Identification ✓

Lawanda Y. Collins
Notary Signature

Bruce Karcher
Parcel # 16-75-16-04226-166
Lot 66 Shiloh Ridge



Columbia County Property Appraiser

DB Last Updated: 10/4/2006

2006 Proposed Values

Parcel: 16-7S-16-04226-166

Tax Record | Property Card | Interactive GIS Map | Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	KARCHER MICHAEL BRUCE &
Site Address	SHILOH RIDGE UNREC
Mailing Address	EDITH S KARCHER 401 SW DAHLED AVE PORT ST LUCIE, FL 34953
Description	NW1/4 OF NE1/4 OF SE1/4. (AKA LOT 66 SHILOH RIDGE S/D UNREC) ORB 849-2113, 879-2204, CT 1042-1321, WD 1045-2729.

Use Desc. (code)	NO AG ACRE (009900)
Neighborhood	15716.01
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	10.000 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$64,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$64,000.00

Just Value	$64,000.00
Class Value	$0.00
Assessed Value	$64,000.00
Exempt Value	$0.00
Total Taxable Value	$64,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
5/1/2005	1045/2729	WD	V	U	08	$45,000.00
3/16/2005	1042/1321	CT	V	U	01	$2,500.00
10/1/1999	889/1380	WD	V	Q		$27,500.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	10.000 AC	1.00/1.00/1.00/1.00	$6,400.00	$64,000.00

Columbia County Property Appraiser

DB Last Updated: 10/4/2006

1 of 1

RETURN TO

U. S. Title
642 N.E. Santa Fe Blvd.
High Springs, FL 32643
USH-30320

Inst:2005011211 Date:05/12/2005 Time:11:19
Doc Stamp-Deed : 315.00
DC,P.DeWitt Cason,Columbia County B:1045 P:2729

PARCEL ID# **R04226-166**
BUYER'S TIN#

WARRANTY DEED

THIS INDENTURE, Made this **1st** day of **May, 2005,** BETWEEN **THE SHILOH RIDGE COMPANY, a Florida Corporation** grantor whose address is **5345 ORTEGA BOULEVARD, SUITE 7, JACKSONVILLE, FL 32210,** and **MICHAEL BRUCE KARCHER and EDITH S. KARCHER, HUSBAND AND WIFE** grantee, whose post-office address is: **401 SW DAHLED AVENUE, PORT ST. LUCIE, FL 34953.**

[The terms "grantor" and "grantee" herein shall be construed to include all genders and singular or plural as the context indicates.]

W I T N E S S E T H: That said grantor, for and in consideration of the sum of Ten ($10.00) Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs, successors and assigns forever, the following described land, situate, lying and being in **COLUMBIA** County, Florida, to wit:

SEE ATTACHED EXHIBIT "A"

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

IN WITNESS WHEREOF, Grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in the presence of:

Heather S. Loveland
WITNESS Heather S. Loveland

James T. Lane, Jr.
WITNESS James T. Lane, Jr.

THE SHILOH RIDGE COMPANY

BY Lee D. Wedekind
Lee D. Wedekind, Jr.
President

STATE OF **FLORIDA**
COUNTY OF **DUVAL**

[CORPORATE SEAL]

The foregoing instrument was acknowledged before me this 1st day of May, 2005, by Lee D. Wedekind, Jr., President of THE SHILOH RIDGE COMPANY on behalf of the corporation. She/He is personally known to me or who has produced a driver's license as identification and who did take an oath.

Heather S. Loveland
Notary Public, State of Florida
My Commission Expires:
My Commission Number:

Notary Public State of Florida
Heather S Loveland
My Commission DD388572
Expires 03/11/2009

MICHAEL BRUCE KARCHER and EDITH S. KARCHER, HUSBAND AND WIFE grantee, whose post-office address is: **401 SW DAHLED AVENUE, PORT ST. LUCIE, FL 34953.**

[The terms "grantor" and "grantee" herein shall be construed to include all genders and singular or plural as the context indicates.]

W I T N E S S E T H: That said grantor, for and in consideration of the sum of Ten ($10.00) Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs, successors and assigns forever, the following described land, situate, lying and being in **COLUMBIA** County, Florida, to wit:

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THE SHILOH RIDGE COMPANY

Signed, sealed and delivered in the presence of:

Heather S. Loveland
WITNESS Heather S. Loveland

BY _____
Lee D. Wedekind, Jr.
President

James T. Lane, Jr.
WITNESS James T. Lane, Jr.

STATE OF **FLORIDA**
COUNTY OF **DUVAL**

[CORPORATE SEAL]

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Heather S. Loveland
Notary Public, State of Florida
My Commission Expires:
My Commission Number:

Notary Public State of Florida
Heather S Loveland
My Commission DD388572
Expires 03/11/2009

RECORD & RETURN TO:
THIS INSTRUMENT WAS PREPARED BY: JANNETTE S. BOYD, an employee of U.S. TITLE, 642 N.E. SANTA FE BLVD., HIGH SPRINGS, FLORIDA 32643, as a necessary incident to fulfill the requirements of a Title Insurance Binder issued by it. USH-3030.

Inst:2005011211 Date:05/12/2005 Time:11:19
Doc Stamp-Deed : 315.00
DC,P.DeWitt Cason,Columbia County B:1045 P:2730

EXHIBIT "A"

Lot 66, Shiloh Ridge

The NW ¼ of the NE ¼ of the SE ¼, Section 16, Township 7 South, Range 16 East, Columbia County, Florida. The East 30 feet and the North 30 feet of said lands being subject to an easement for ingress and egress.

TOGETHER WITH AN EASEMENT FOR INGRESS AND EGRESS OVER AND ACROSS THE FOLLOWING DESCRIBED PROPERTY:

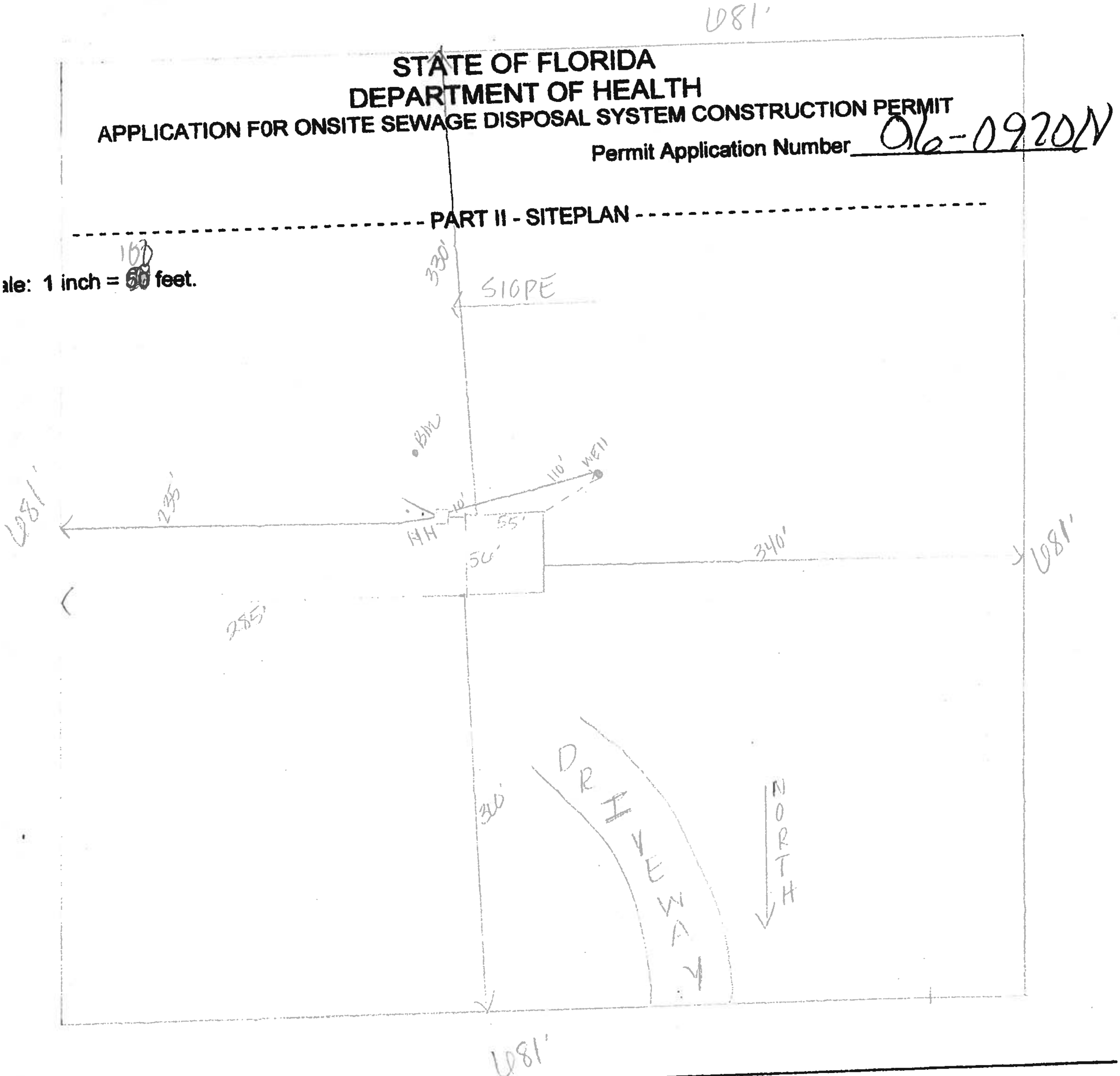
A strip of land 60 feet in width being 30 feet each side of a centerline described as follows:
Commence at the Southeast corner of the SW ¼ of the NE ¼, Section 15, Township 7 South, Range 16 East, Columbia County, Florida and run thence South 89 deg 03'48" West, 20.45 feet to the West line of Fry Road and to the point of beginning; thence continue South 89 deg 03'48" West, 3952.99 feet to the East line of Section 16, Township 7 South, Range 16 East; thence South 89 deg 06'19" West, 661.99 feet to reference point "C"; thence continue South 89 deg 06'19" West, 1323.98 feet to reference point "D" and to the point of termination; Also begin at reference point "C" and run thence North 00 deg 45'21" West 701.45 feet to the radius point of a cul-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "C" and run thence South 00 deg 45'01" East, 1323.20 feet; thence South 00 deg 44'52" East, 701.59 feet to the radius point of a cul-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "D" and run thence North 00 deg 46' 46" West, 701.37 feet to the radius point of a cul-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "D" and run thence South 00 deg 46'12" East, 1323.42 feet; thence South 00 deg 46'00" East, 701.68 feet to the radius point of a cul-de-sac having a 50 foot radius and to the point of termination.

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0920N

PART II - SITEPLAN

Scale: 1 inch = 100 feet.



Notes: ______

Site Plan submitted by: Rocky Ford MASTER CONTRACTOR

Plan Approved X Not Approved ____ Date OCT 30 2006

By Sallie Gladdy ES II Columbia CHD County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-6)

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	KARCHER RESIDENCE	Builder:	Bruce & Sue Karcher
Address:	Lot: , Sub: , Plat:	Permitting Office:	COLUMBIA
City, State:	Lake City, FL 32024-	Permit Number:	25228
Owner:	EWPL INC	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New		
2. Single family or multi-family	Single family		
3. Number of units, if multi-family	1		
4. Number of Bedrooms	3		
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1800 ft²		
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default)	Area 194.5 ft²	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	194.5 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft		
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior	R=13.0, 1556.0 ft²		
b. Frame, Wood, Adjacent	R=13.0, 216.0 ft²		
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1800.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 130.0 ft		
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 36.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 36.0 kBtu/hr	
	HSPF: 7.20	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.92	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.17

Total as-built points: 25024

Total base points: 28091

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 10-16-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-**	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	50.0	0.92	3	1.00	2635.00	1.00	7905.0	
				As-Built Total:						**7905.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
10226	**9961**	**7905**	**28091**	**6454**	**10665**	**7905**	**25024**

PASS

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**15876.1**	**Winter As-Built Points:**					**20395.9**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
15876.1	0.6274	9960.6	20395.9	1.00	1.162	0.474	0.950	10665.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1800.0	12.74	4127.8	Double, Clear	N	1.5	7.5	74.7	24.58	1.00	1837.3
				Double, Clear	N	8.0	4.0	12.5	24.58	1.03	314.9
				Double, Clear	W	1.5	5.5	30.0	20.73	1.03	639.3
				Double, Clear	S	1.5	8.0	42.0	13.30	1.04	581.4
				Double, Clear	S	11.0	8.0	63.0	13.30	3.18	2664.7
				Double, Clear	S	1.5	5.0	16.0	13.30	1.20	254.7
				Double, Clear	E	1.5	7.5	23.3	18.79	1.02	448.6
				Double, Clear	E	1.5	2.0	15.0	18.79	1.21	341.5
				Double, Clear	S	1.5	6.0	30.0	13.30	1.12	445.8
				As-Built Total:				**306.5**			**7528.3**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	216.0	3.60	777.6	Frame, Wood, Exterior			13.0	1556.0	3.40		5290.4
Exterior	1556.0	3.70	5757.2	Frame, Wood, Adjacent			13.0	216.0	3.30		712.8
Base Total:	**1772.0**		**6534.8**	**As-Built Total:**				**1772.0**			**6003.2**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	18.0	11.50	207.0	Exterior Insulated				33.0	8.40		277.2
Exterior	53.0	12.30	651.9	Exterior Insulated				20.0	8.40		168.0
				Adjacent Insulated				18.0	8.00		144.0
Base Total:	**71.0**		**858.9**	**As-Built Total:**				**71.0**			**589.2**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1800.0	2.05	3690.0	Under Attic			30.0	1800.0	2.05 X	1.00	3690.0
Base Total:	**1800.0**		**3690.0**	**As-Built Total:**				**1800.0**			**3690.0**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	194.0(p)	8.9	1726.6	Slab-On-Grade Edge Insulation			0.0	194.0(p	18.80		3647.2
Raised	0.0	0.00	0.0								
Base Total:			**1726.6**	**As-Built Total:**				**194.0**			**3647.2**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1800.0	-0.59	-1062.0					1800.0	-0.59		-1062.0

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-**	PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 23969.9				**Summer As-Built Points:**						**23941.4**
Total Summer Points	X	System Multiplier	=	Total Component (System - Points)	X	Cap Ratio	X	Duct Multiplier (DM x DSM x AHU)	X	System Multiplier	X	Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 36000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)										
23969.9		0.4266		23941 **23941.4**		1.00 **1.00**		(1.09 x 1.147 x 0.91) **1.138**		0.263 **0.263**		0.902 **0.902**		6453.9 **6453.9**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL, 32024-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1800.0	20.04	6493.0	Double, Clear	N	1.5	7.5	74.7	19.20	0.96	1378.5
				Double, Clear	N	8.0	4.0	12.5	19.20	0.62	149.6
				Double, Clear	W	1.5	5.5	30.0	38.52	0.90	1036.6
				Double, Clear	S	1.5	8.0	42.0	35.87	0.92	1390.8
				Double, Clear	S	11.0	8.0	63.0	35.87	0.48	1087.6
				Double, Clear	S	1.5	5.0	16.0	35.87	0.81	463.1
				Double, Clear	E	1.5	7.5	23.3	42.06	0.95	931.0
				Double, Clear	E	1.5	2.0	15.0	42.06	0.59	374.1
				Double, Clear	S	1.5	6.0	30.0	35.87	0.86	921.2
				As-Built Total:				**306.5**			**7732.5**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	216.0	0.70	151.2	Frame, Wood, Exterior			13.0	1556.0	1.50		2334.0
Exterior	1556.0	1.70	2645.2	Frame, Wood, Adjacent			13.0	216.0	0.60		129.6
Base Total:	**1772.0**		**2796.4**	**As-Built Total:**				**1772.0**			**2463.6**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	18.0	2.40	43.2	Exterior Insulated				33.0	4.10		135.3
Exterior	53.0	6.10	323.3	Exterior Insulated				20.0	4.10		82.0
				Adjacent Insulated				18.0	1.60		28.8
Base Total:	**71.0**		**366.5**	**As-Built Total:**				**71.0**			**246.1**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1800.0	1.73	3114.0	Under Attic			30.0	1800.0	1.73 X 1.00		3114.0
Base Total:	**1800.0**		**3114.0**	**As-Built Total:**				**1800.0**			**3114.0**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	194.0(p)	-37.0	-7178.0	Slab-On-Grade Edge Insulation			0.0	194.0(p	-41.20		-7992.8
Raised	0.0	0.00	0.0								
Base Total:			**-7178.0**	**As-Built Total:**				**194.0**			**-7992.8**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1800.0	10.21	18378.0					1800.0	10.21		18378.0

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

EWPL INC, Lot: , Sub: , Plat: , Lake City, FL, 32024-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 36.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1800 ft²		c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 194.5 ft²		a. Electric Heat Pump	Cap: 36.0 kBtu/hr
	b. SHGC:				HSPF: 7.20
	(or Clear or Tint DEFAULT) 7b.	(Clear) 194.5 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 50.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1556.0 ft²			EF: 0.92
	b. Frame, Wood, Adjacent	R=13.0, 216.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1800.0 ft²	15.	HVAC credits	PT, CF,
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 130.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

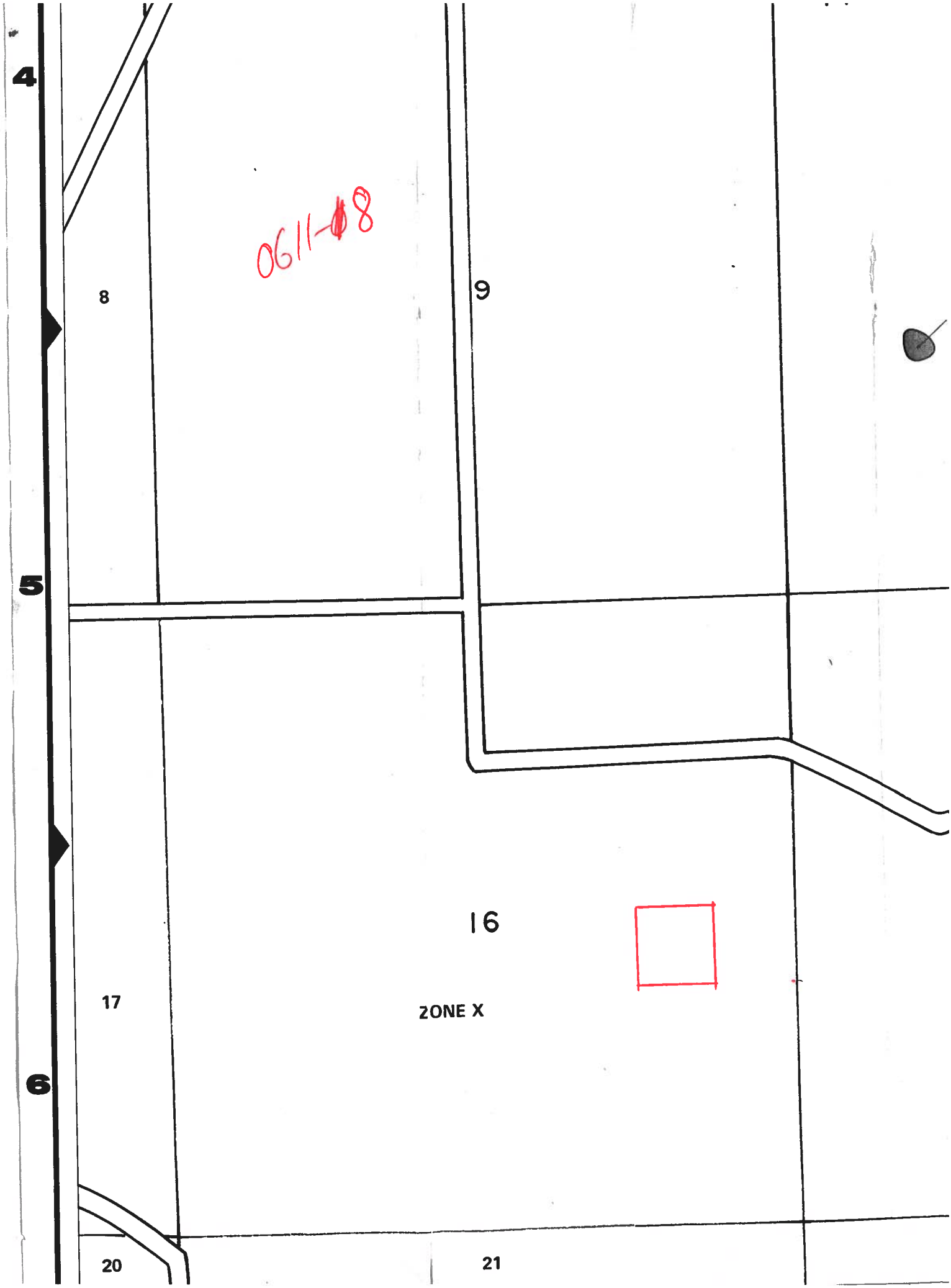
I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar℠ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.


4
5
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16
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21
ZONE X
0611-08

25228

This instrument prepared by and
After recordation should be returned to:

Wells Fargo Bank, N.A.-Construction Lending
2701 Wells Fargo Way, MAC# X9999-01M
Minneapolis, Minnesota 55467

Inst:2006030552 Date:12/28/2006 Time:17:08
DC,P.DeWitt Cason,Columbia County B:1106 P:876

0071201063

Permit No. ______________________ Tax Folio No.

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF ALACHUA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property (legal description of the property and street address if available):
SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF FOR ALL PURPOSES

LOT 46 SHILOH RIDGE, FORT WHITE, FLORIDA 32037

2. General description of improvement:
3. Owner Information:
(a) Name and address: MICHAEL BRUCE KARCHER and EDITH S. KARCHER

P.O. BOX 937, FORT WHITE, FLORIDA 32038
(b) Interest in property: ______________________
(c) Name and address of fee simple titleholder (if other than borrower):
____
4. Contractor(Name and address): EWPL, INC.
P.O. BOX 200, FORT WHITE, FLORIDA 32038
5. Surety (if applicable):
(a) Name and address: ______________________
(b) Amount of Bond: $ ______________________
6. Lender (Name and address): WELLS FARGO BANK, N.A.
2700 WELLS FARGO WAY, MAC# X9901-05T
MINNEAPOLIS, MINNESOTA 55407
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13 (1) (a)7, Florida Statutes
Name and address: ______________________
____

[illegible]

8. In addition to himself, Owner designates WELLS FARGO BANK, N.A.
2799 WELLS FARGO WAY, MAC# X9941-03T, MINNEAPOLIS, MINNESOTA 55467
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified): ______________________

MICHAEL BRUCE KARCHER EDITH S. KARCHER

Sworn to and subscribed before me this 20 day of December 2006.
He/She is personally known to me or produced Florida Drivers License as identification.

My Commission expires: Sept 22, 2009 Notary Public
Deanna Burkhart

Inst:2006030552 Date:12/28/2006 Time:17:06
DC,P.DeWitt Cason,Columbia County B:1106 P:877

Escrow File No.: FT061246

EXHIBIT "A"

Lot 66, Shiloh Ridge:
The NW 1/4 of the NE 1/4 of the SE 1/4, Section 16, Township 7 South, Range 16 East, Columbia County, Florida. The East 30 feet and the North 30 feet of said lands being subject to an easement for ingress and egress.

Together with an easement for ingress and egress over and across the following described property:

A strip of land 60 feet in width being 30 feet each side of a centerline described as follows: Commence at the Southeast corner of the SW 1/4 of the NE 1/4, Section 15, Township 7 South, Range 16 East, Columbia County, Florida and run thence South 89 deg 03' 48" West, 20.45 feet to the West line of Fry Road and to the point of beginning; thence continue South 89 deg 03' 48" West, 3952.99 feet to the East line of Section 26, Township 7 South, Range 16 East; thence South 89 deg 06' 19" West, 661.99 feet to reference point "C"; thence continue South 89 deg 06' 19" West, 1323.98 feet to reference point "D" and to the point of termination; Also begin at reference point "C" and run thence North 00 deg 45' 21" West 701.45 feet to the radius point of a cul-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "C" and run thence South 00 deg 45' 01" East, 1323.20 feet; thence South 00 deg 44' 52" East, 701.59 feet to the radius point of a cul-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "D" and run thence North 00 deg 46' 46" West, 701.37 feet to the radius point of a cu-de-sac having a radius of 50 feet and to the point of termination. Also begin at reference point "D" and run thence South 00 deg 46' 12" East, 1323.42 feet; thence South 00 deg 46' 00" East, 701.68 feet to the radius point of a cul-de-sac having a 50 foot radius and to the point of termination.

Parcel ID#16-7S-16-04226-166

Inst:2006030552 Date:12/28/2006 Time:17:08
DC,P.DeWitt Cason,Columbia County B:1106 P:878

Residential System Sizing Calculation

Summary

EWPL INC

Lake City, FL 32024-

Project Title:
KARCHER RESIDENCE

Code Only
Professional Version
Climate: North

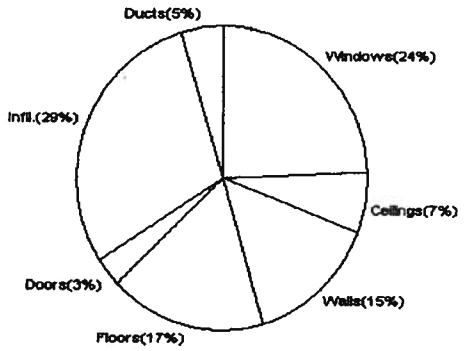
10/16/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**35459**	**Btuh**	**Total cooling load calculation**	**34714**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	36000	Btuh
Submitted as % of calculated	101.5	%	Submitted as % of calculated	103.7	%

WINTER CALCULATIONS

Winter Heating Load (for 1800 sqft)

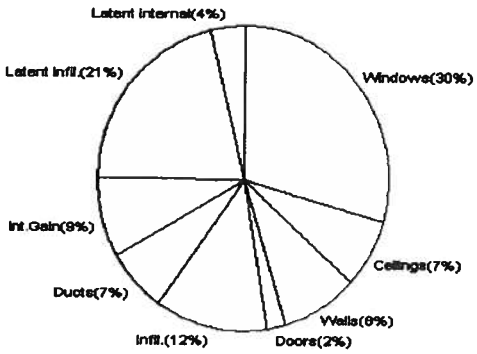
Load component			Load	
Window total	307	sqft	8674	Btuh
Wall total	1772	sqft	5169	Btuh
Door total	71	sqft	1141	Btuh
Ceiling total	1800	sqft	2340	Btuh
Floor total	194	ft	6130	Btuh
Infiltration	240	cfm	10317	Btuh
Subtotal			**33771**	**Btuh**
Duct loss			1689	Btuh
TOTAL HEAT LOSS			**35459**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1800 sqft)

Load component			Load	
Window total	307	sqft	10296	Btuh
Wall total	1772	sqft	2932	Btuh
Door total	71	sqft	720	Btuh
Ceiling total	1800	sqft	2556	Btuh
Floor total			0	Btuh
Infiltration	210	cfm	4166	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**23670**	**Btuh**
Duct gain			2367	Btuh
Total sensible gain			**26037**	**Btuh**
Latent gain(infiltration)			7297	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**8677**	**Btuh**
TOTAL HEAT GAIN			**34714**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 10-16-06

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC

Project Title:
KARCHER RESIDENCE

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

10/16/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	74.7	28.3	2113 Btuh
2	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
3	2, Clear, Metal, DEF	W	30.0	28.3	849 Btuh
4	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
5	2, Clear, Metal, DEF	S	63.0	28.3	1783 Btuh
6	2, Clear, Metal, DEF	S	16.0	28.3	453 Btuh
7	2, Clear, Metal, DEF	E	23.3	28.3	660 Btuh
8	2, Clear, Metal, DEF	E	15.0	28.3	424 Btuh
9	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
	Window Total		307		8674 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1556	3.1	4824 Btuh
2	Frame - Adjacent	13.0	216	1.6	346 Btuh
	Wall Total		1772		5169 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		33	18.3	605 Btuh
2	Insulated - Exter		20	18.3	367 Btuh
3	Insulated - Adjac		18	9.4	169 Btuh
	Door Total		71		1141Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1800	1.3	2340 Btuh
	Ceiling Total		1800		2340Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	194.0 ft(p)	31.6	6130 Btuh
	Floor Total		194		6130 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	18000(sqft)	240	10317 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			240	10317 Btuh

Totals for Heating	**Subtotal**	**33771 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1689 Btuh**
	Total Btuh Loss	**35459 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC

Lake City, FL 32024-

Project Title:
KARCHER RESIDENCE

Code Only
Professional Version
Climate: North

10/16/2006

Totals for Cooling	**Subtotal**	**23670**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**2367**	**Btuh**
	Total sensible gain	**26037**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**7297**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**34714**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC

Project Title:
KARCHER RESIDENCE

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F

10/16/2006

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	7.5	74.7	0.0	74.7	22	22	1643	Btuh
2	2, Clear, DEF, N, N	N	8	4	12.5	0.0	12.5	22	22	275	Btuh
3	2, Clear, DEF, N, N	W	1.5	5.5	30.0	4.5	25.5	22	72	1936	Btuh
4	2, Clear, DEF, N, N	S	1.5	8	42.0	21.0	21.0	22	37	1239	Btuh
5	2, Clear, DEF, N, N	S	11	8	63.0	21.0	42.0	22	37	2016	Btuh
6	2, Clear, DEF, N, N	S	1.5	5	16.0	16.0	0.0	22	37	352	Btuh
7	2, Clear, DEF, N, N	E	1.5	7.5	23.3	1.2	22.1	22	72	1618	Btuh
8	2, Clear, DEF, N, N	E	1.5	2	15.0	10.5	4.5	22	72	556	Btuh
9	2, Clear, DEF, N, N	S	1.5	6	30.0	30.0	0.0	22	37	660	Btuh
	Window Total				307					10296	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	13.0	1556.0	1.7	2707	Btuh
2	Frame - Adjacent	13.0	216.0	1.0	225	Btuh
	Wall Total		1772.0		2932	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	33.0	10.1	335	Btuh
2	Insulated - Exter	20.0	10.1	203	Btuh
3	Insulated - Adjac	18.0	10.1	183	Btuh
	Door Total	71.0		720	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1800.0	1.4	2556	Btuh
	Ceiling Total		1800.0		2556	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	194.0 ft(p)	0.0	0	Btuh
	Floor Total		194.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.70	18000	210.4	4166	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			210	4166	Btuh

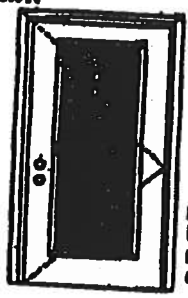
Internal gain	Occupants	Btuh/occupant		Appliance	Load	
	6	X 300	+	1200	3000	Btuh

X
Glazed Inswing Unit

COP-WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Design Pressure
+50.5/-60.5

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:

1/2 GLASS:

Entergy
Entry Systems

PREMDOOR
Masonite
Masonite International Corporation

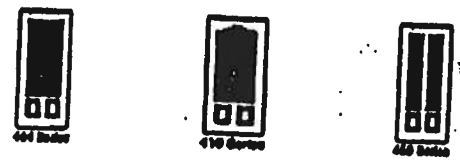
X
Glazed Inswing Unit

COP WL F04141-02

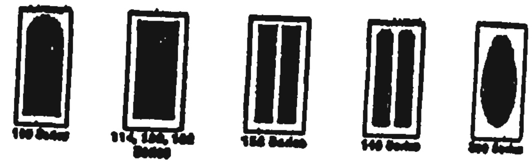
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portnoy, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 25-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

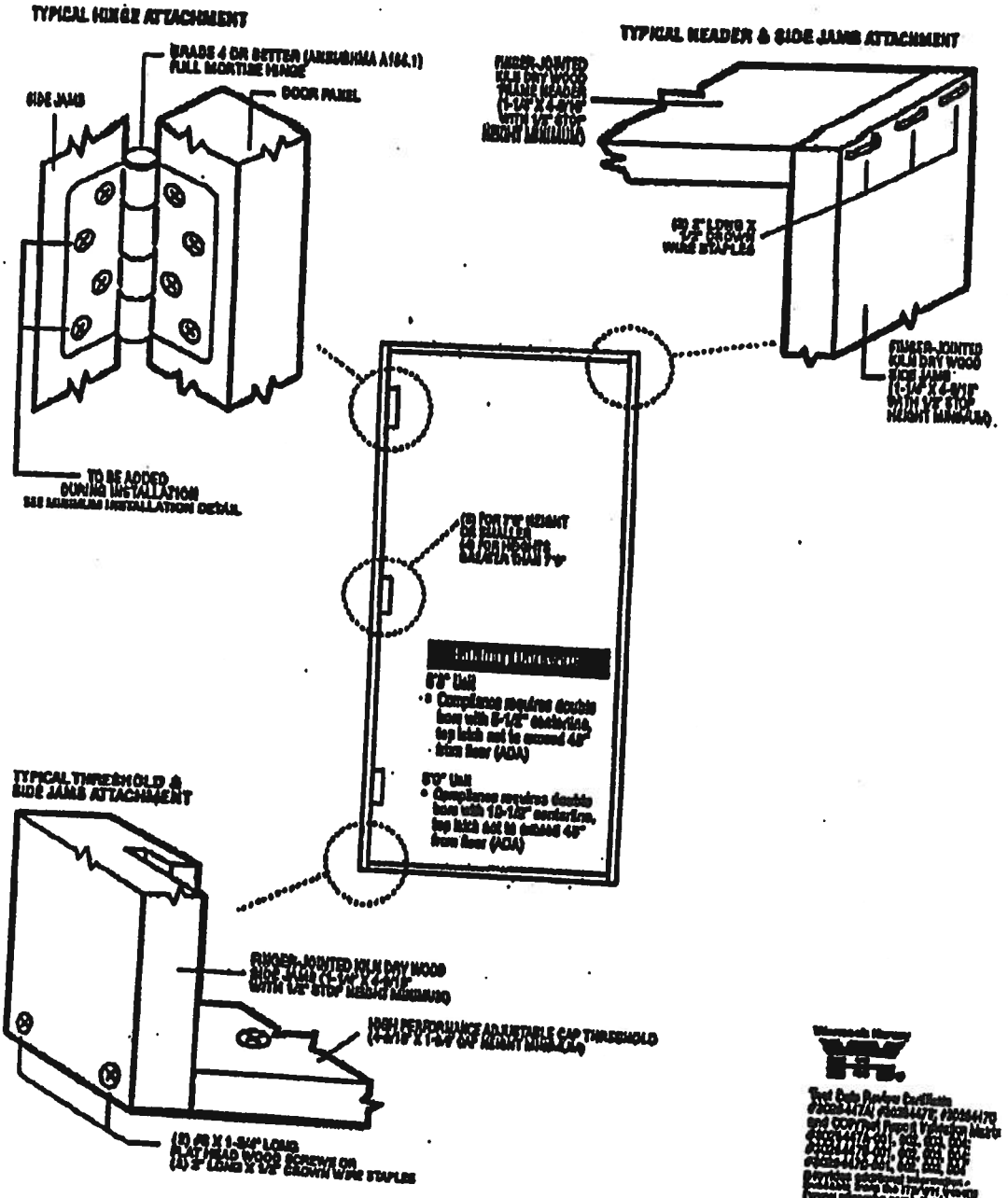
Entergy
Entry Systems
June 17, 2002

FREMDOOR
Sold solely from
Masonite
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR

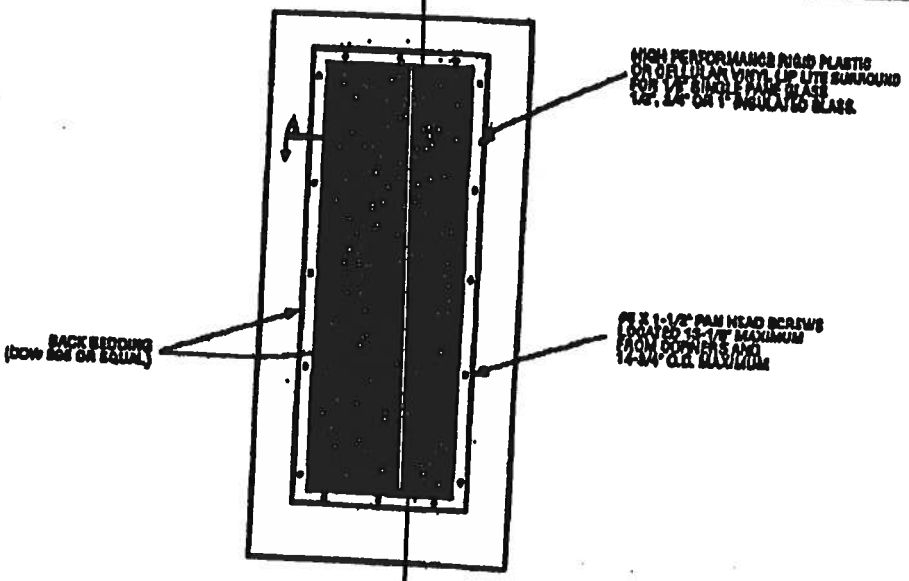


October 14, 2003

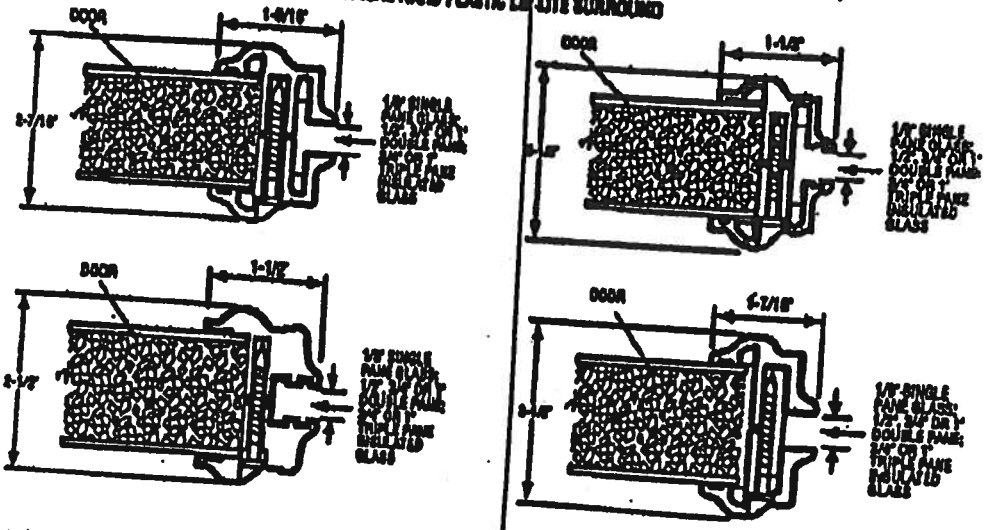
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LITE SURROUND

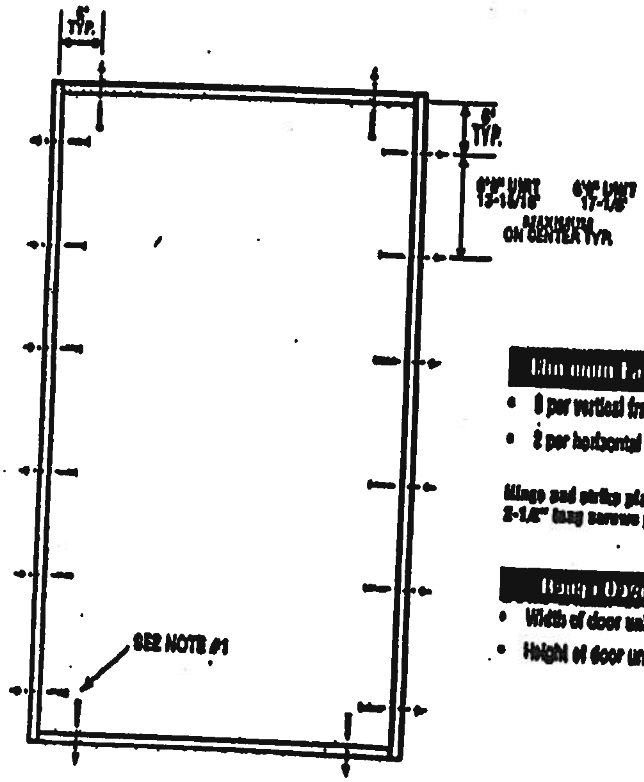


*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- [illegible] per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

[illegible]

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8245*, 8255*, 8241*, 8248, 8251* or 8258 Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

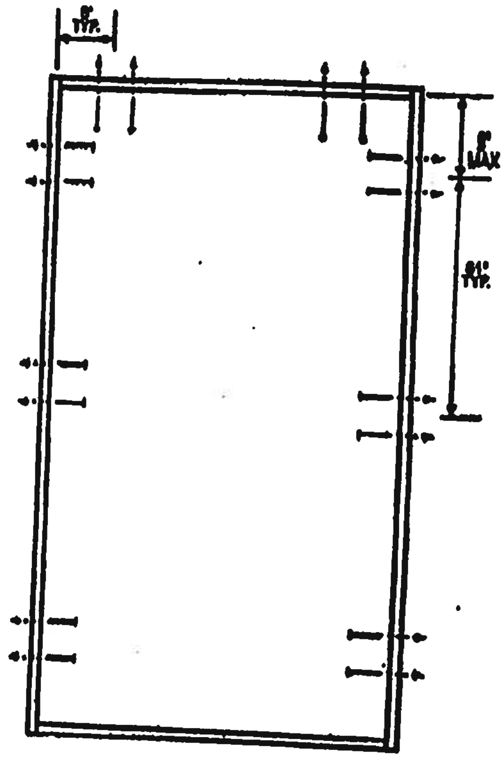
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate [illegible] and COP/Test Report Validation Matrix [illegible] additional information - available from the [illegible] website [illegible] or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 3240*, 3238*, 3241*, 3248, 3251* or 3256
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

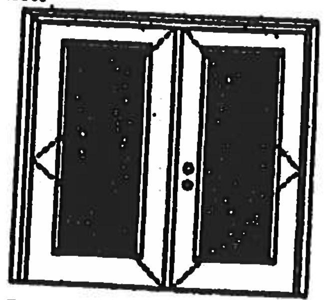
1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

XX
Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Double Door

Design Pressure

+50.6/-50.6

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

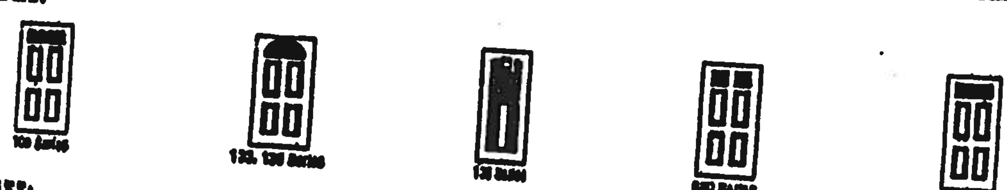
Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

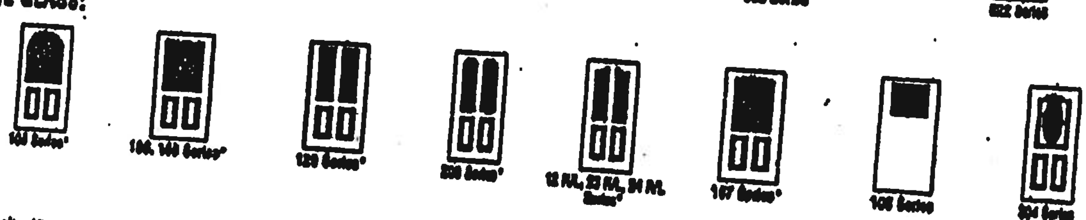
Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



1/2 GLASS:



Entergy
Entry Systems

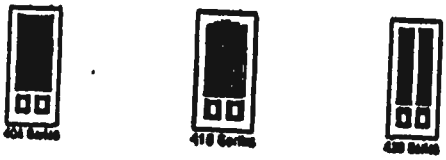
Premdor
Exclusively from Masonite
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
1/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NOTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Baltzer

State of Florida, Professional Engineer
Kurt Baltzer, P.E. – License Number 56533

Entergy
Entry Systems

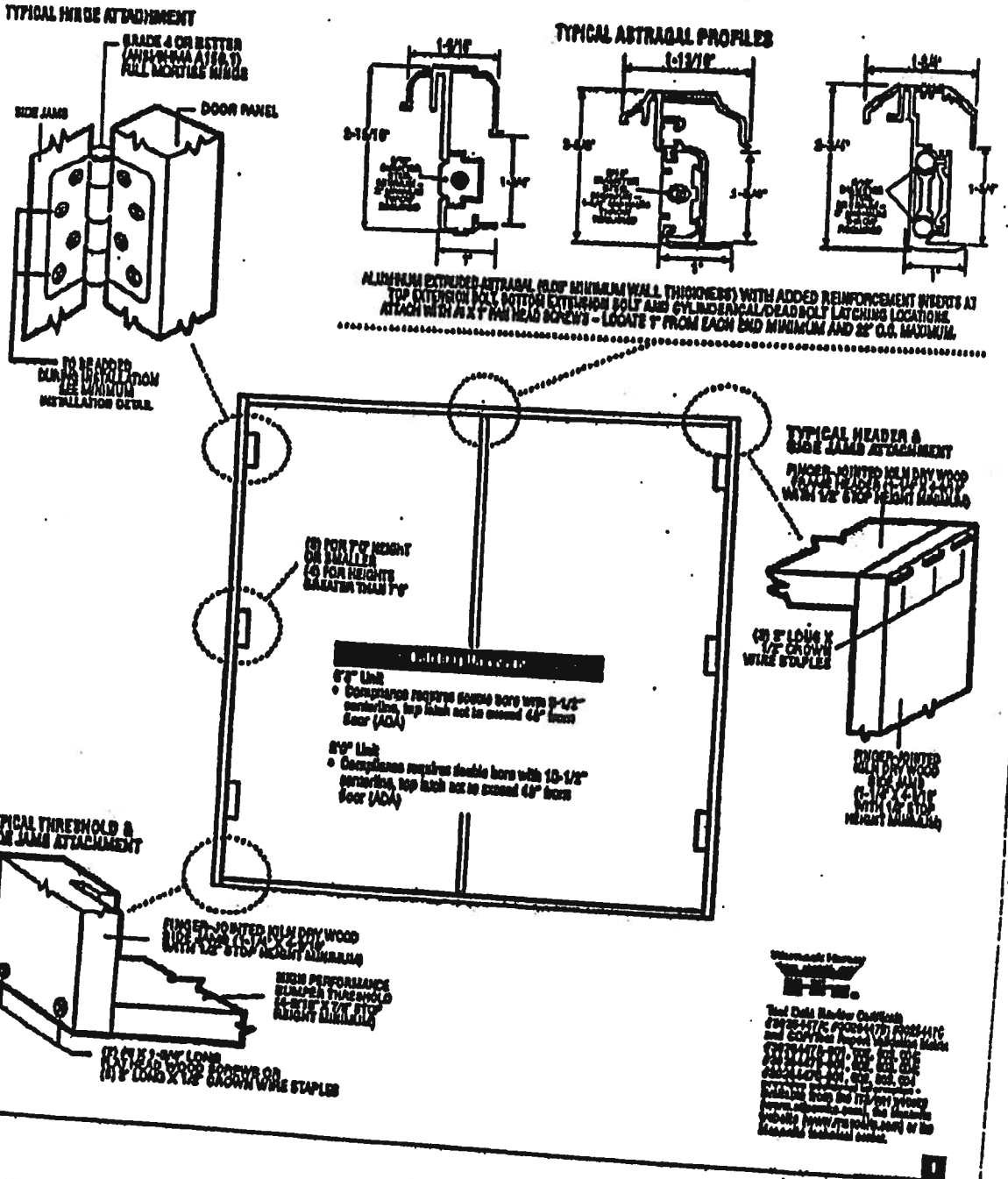
June 17, 2004

Masonite
Masonite International Corporation

XX
Unit

MAD WL-MA0012-02

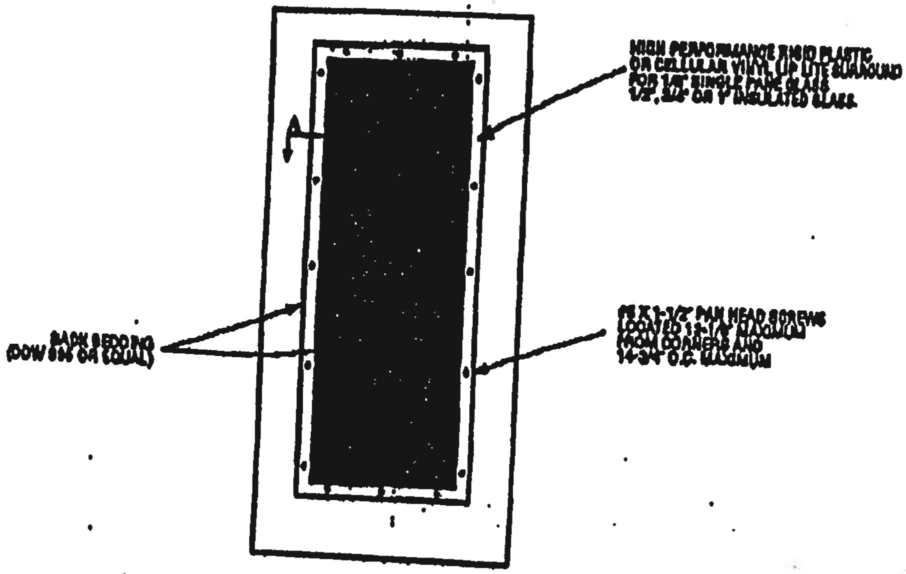
OUTSWING UNITS WITH DOUBLE DOOR



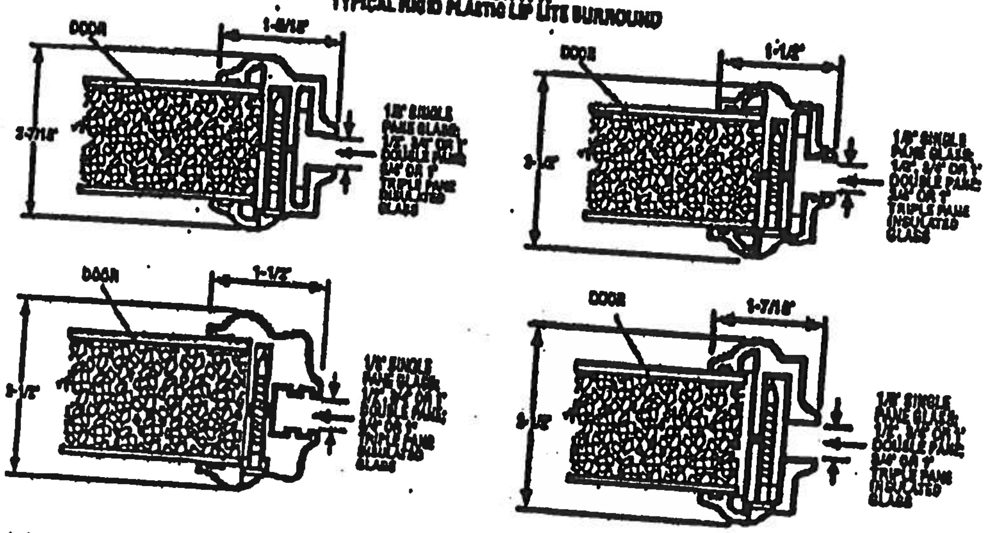
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



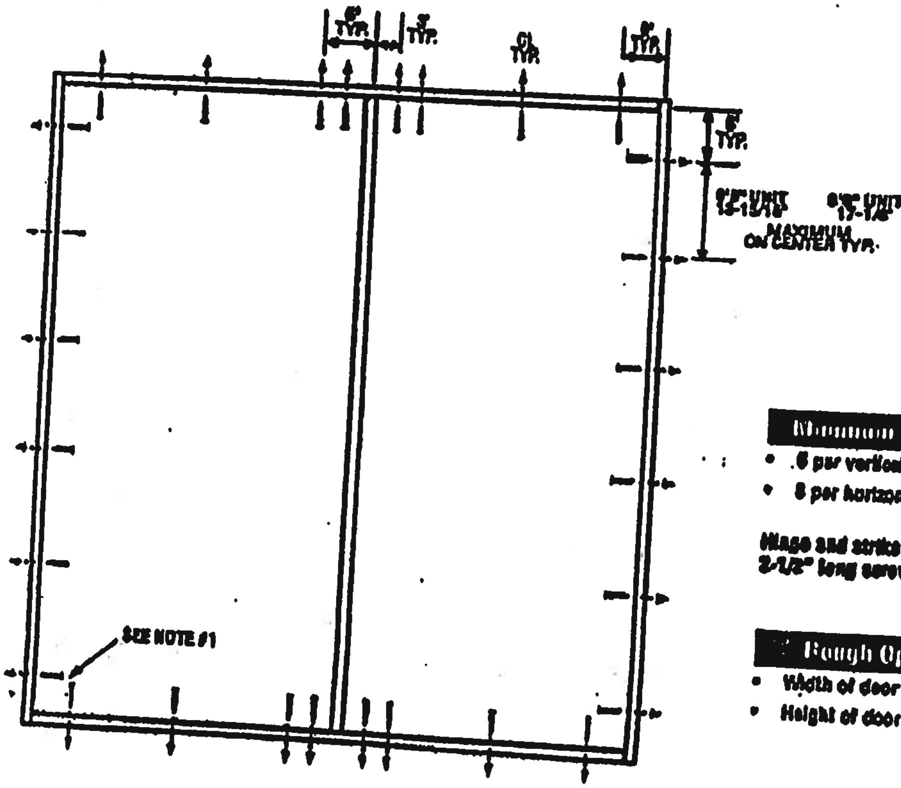
*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4".

[illegible] Test Data Review Certificate [illegible] additional information [illegible] (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8257*, 3242*, 8247, 8232* or 8257
Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

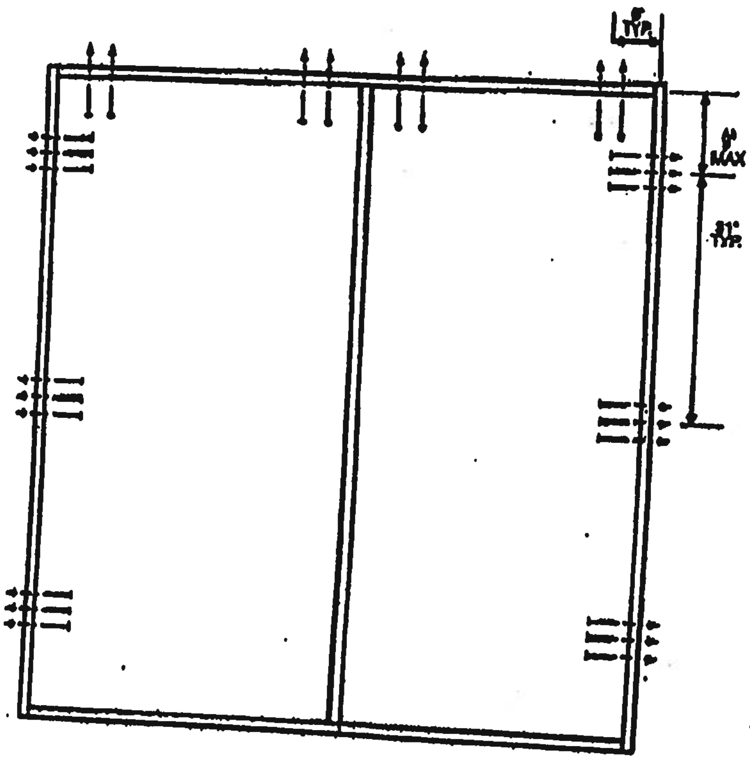
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Maximum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3026447[illegible] and COP/Test Report Validation Matrix #3026447[illegible]-001, 002, 003, 004; [illegible] additional information - available from the ITS/WH website [illegible], the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8257*, 8242*, 8247, 8252* or 8257
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
113 193
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

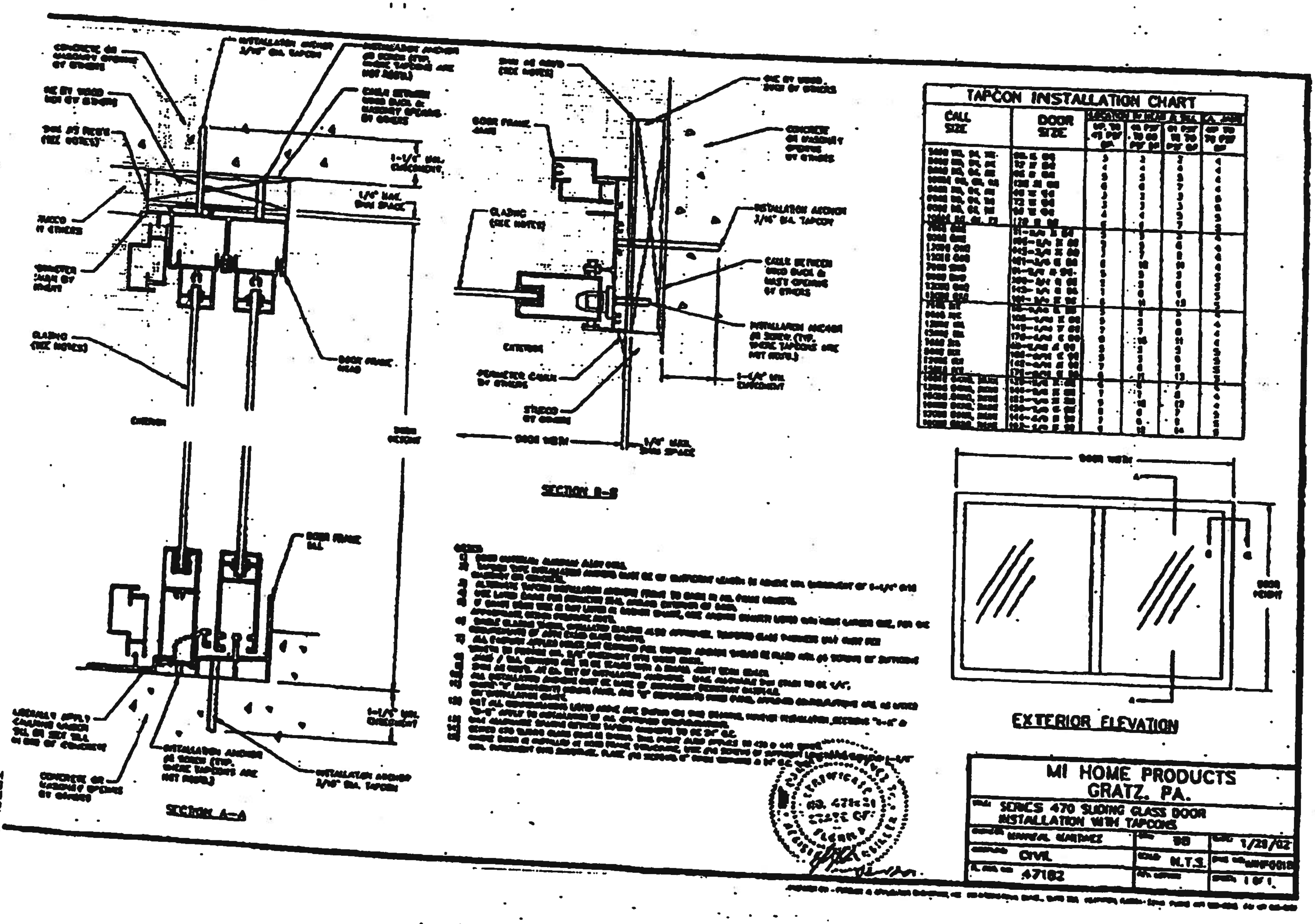
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP + 40.0 / - 55.4
- 3'- 0" WIDE DP + 40.0 / - 48.5
- 4'- 0" WIDE DP + 40.0 / - 40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686


TAPCON INSTALLATION CHART
CALL SIZE
DOOR SIZE
SECTION A-A
SECTION B-B
EXTERIOR ELEVATION
MI HOME PRODUCTS
GRATZ, PA.
SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS
CIVIL
47182
N.T.S.
1/29/02

TOTAL P.04

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen M. Reeves
15 FEBRUARY 2002

ALLEN M. REEVES CERTIFICATE 0. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE NO. 18356
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

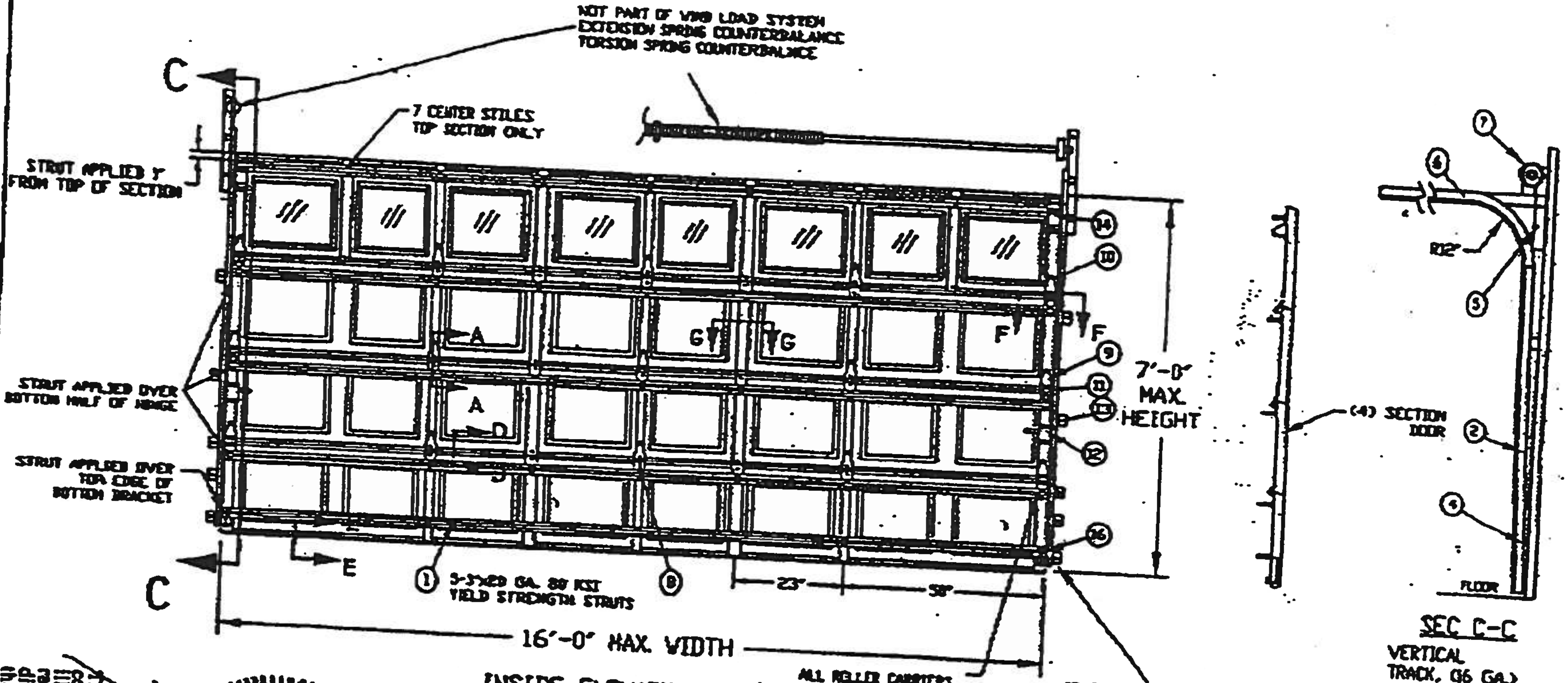
Allen M. Reeves

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

NOTES

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. △ DROP IN TYPE OF INSULATION IS OPTIONAL.

REV.	DATE	BY	DESCRIPTION
A-A	01-10-00	DW	SEE E.C.N. 031



INSIDE ELEVATION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s) dimensions and installation(s) of the door as tested.

SEAL
PE No. 024280
NORTH CAROLINA PROFESSIONAL ENGINEER

LISTED SBCCI PST & ESI
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 400293

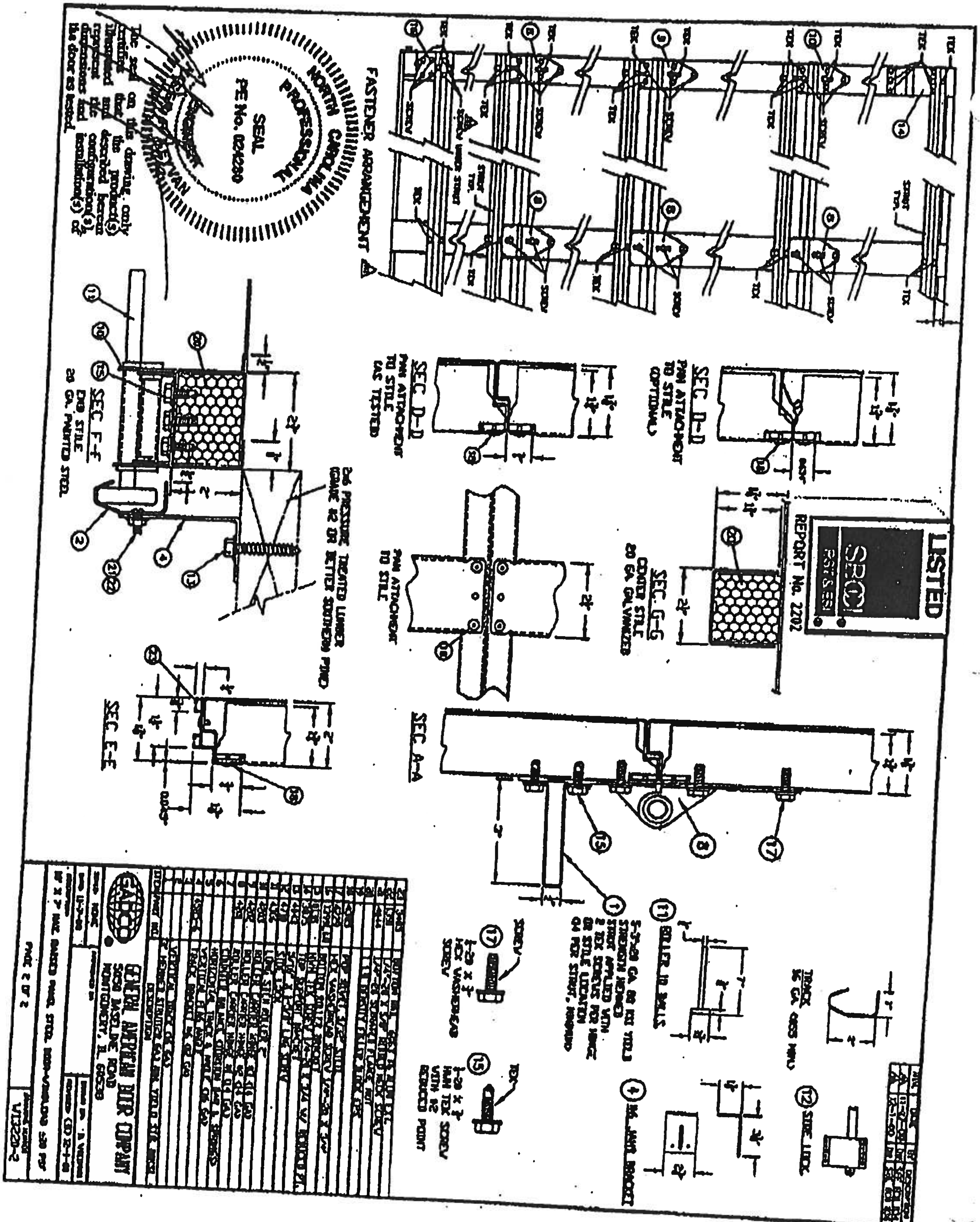
GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7524, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

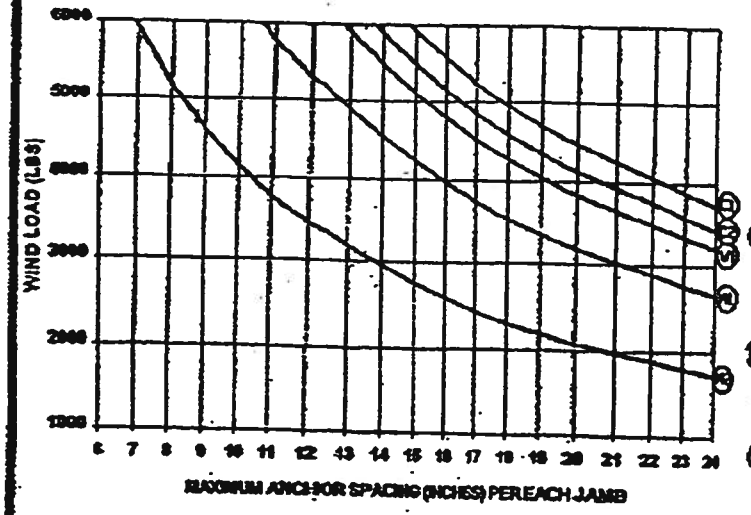
GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

DRAWN BY: D. WICKMAN
DATE: 10-20-00
REVISED: (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -W/DLOAD ±20 PSF
PAGE 1 OF 2
DRAWING NUMBER: V13220-1



FASTENER ARRANGEMENT
LISTED
SBCCI PST & ESI
REPORT No. 2202
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)
SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)
SEC G-G
CENTER STILE
20 GA. GALVANIZED
PAN ATTACHMENT TO STILE
SEC A-A
TRACK
16 GA. (.055 MIN.)
SIDE LOCK
ROLLER 10 BALLS
16 GA. JAMB BRACKET
HEX WASHERHEAD SCREW
HWH TEK SCREW WITH #2 REDUCED POINT
2x6 PRESSURE TREATED LUMBER (GRADE #2 OR BETTER SOUTHERN PINE)
SEC F-F
END STILE
20 GA. PAINTED STEEL
SEC E-E
NORTH CAROLINA
PROFESSIONAL
SEAL
PE No. 024280
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538
GADCO
PAGE 2 OF 2
VI3220-2

WIND LOAD vs ANCHOR SPACING



(I) CONCRETE BACKUP
HILTI KWIK BOLT II EXPANSION ANCHOR
3/8" DIA.
1-5/8" EMBEDMENT

(II) CONCRETE BACKUP
RAWL LOK/BOLT SLEEVE ANCHOR
3/8" DIA.
1-3/8" EMBEDMENT

(III) MASONRY/CMU BACKUP
RAWL LOK/BOLT SLEEVE ANCHOR
3/8" DIA.
1-3/8" EMBEDMENT

(IV) MASONRY/CMU BACKUP
ITW BUILDEX TAPCON MASONRY ANCHOR
1/4" DIA.
1-3/4" EMBEDMENT

(V) WOOD STUD BACKUP
LAG SCREWS
5/16" DIA.
1-1/2" EMBEDMENT

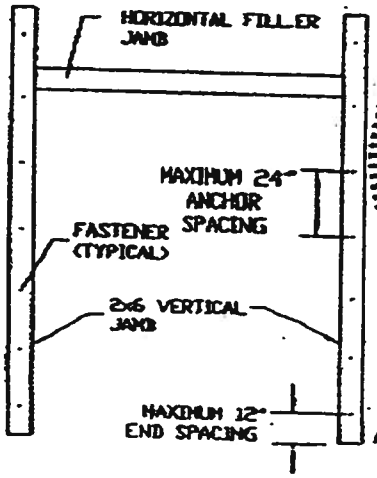
DESIGN LOAD (LBS/FT²) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(I) USE 22" SPACING
(II) USE 21" SPACING
(V) USE 19" SPACING
(III) USE 16" SPACING
(IV) USE 10" SPACING

SEE NOTE 10 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER NASER R KEYVAN

NASER R KEYVAN NC 24774 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/2/2002

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2850 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

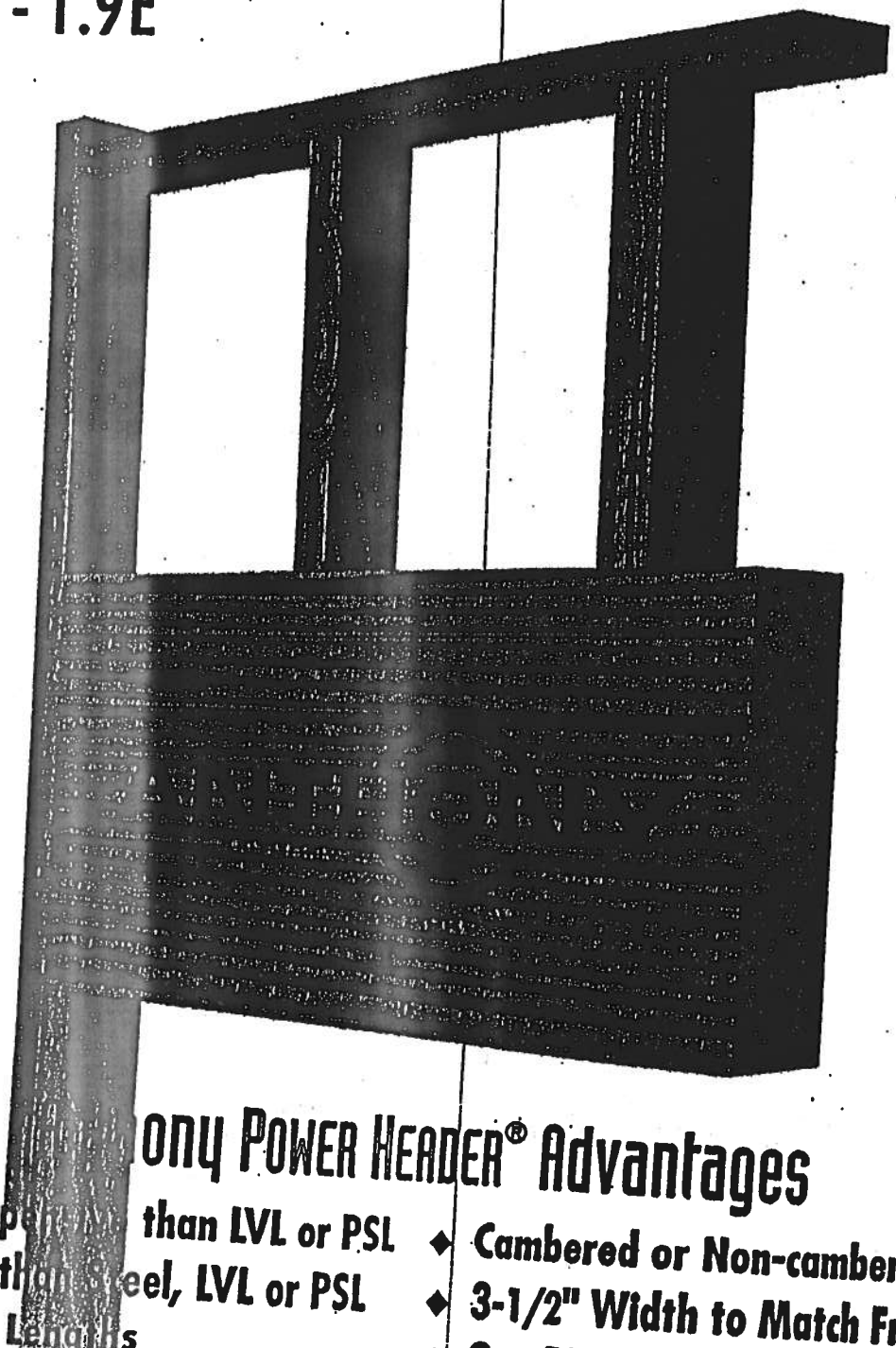
11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

ISSUED NONE	APPROVED	DRAWN BY BV
DATE 8-30-99		ISSUED
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	DRAWING NUMBER A10560	

Anthony POWER HEADER®

2600F_b - 1.9E



ony POWER HEADER® Advantages

- Less Expe[illegible] than LVL or PSL
- Lighter th[illegible]eel, LVL or PSL
- Pre-Cut Len[illegible]s
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony POWER HEADER®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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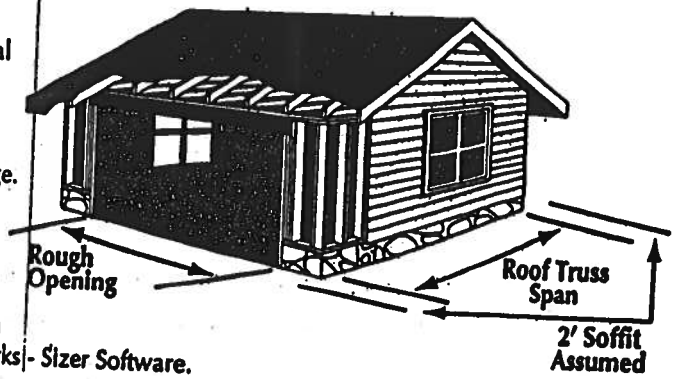
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F_b POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582					
107	153	169	245	260	380	368	540	501	715	664	864	840
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅓"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations.
**Check for product availability.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. Eaves...

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 16-7S-16-04226-166 **Building permit No.** 000025228

Use Classification SFD/UTILITY **Fire:** 43.16

Permit Holder HUGO ESCALANTE **Waste:** 67.00

Owner of Building MICHAEL KARCHER **Total:** 110.16

Location: 1096 SW CUMBERLAND ST, FT. WHITE, FL

Date: 06/08/2007

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L215867		
Builder:	HUGO ESCALANTE	Date:	10/31/2006
Lot:	N/A	Start Number:	**1299**
Subdivision:	1096 CUMBERLAND RD.	SEI Ref:	L215867
County or City:	COLUMBIA COUNTY		
Truss Page Count:	33		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 40

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1031061299	10/31/2006				
2	CJ3	1031061300	10/31/2006				
3	CJ5	1031061301	10/31/2006				
4	EJ7	1031061302	10/31/2006				
5	EJ7A	1031061303	10/31/2006				
6	EJ7B	1031061304	10/31/2006				
7	EJ7C	1031061305	10/31/2006				
8	HJ9	1031061306	10/31/2006				
9	T01	1031061307	10/31/2006				
10	T02	1031061308	10/31/2006				
11	T03	1031061309	10/31/2006				
12	T04	1031061310	10/31/2006				
13	T05	1031061311	10/31/2006				
14	T06	1031061312	10/31/2006				
15	T07	1031061313	10/31/2006				
16	T08	1031061314	10/31/2006				
17	T09	1031061315	10/31/2006				
18	T10	1031061316	10/31/2006				
19	T11	1031061317	10/31/2006				
20	T12	1031061318	10/31/2006				
21	T13	1031061319	10/31/2006				
22	T14	1031061320	10/31/2006				
23	T15	1031061321	10/31/2006				
24	T16	1031061322	10/31/2006				
25	T17	1031061323	10/31/2006				
26	T18	1031061324	10/31/2006				
27	T19	1031061325	10/31/2006				
28	T20	1031061326	10/31/2006				
29	T21	1031061327	10/31/2006				
30	T22	1031061328	10/31/2006				
31	T23	1031061329	10/31/2006				
32	T24	1031061330	10/31/2006				
33	T25	1031061331	10/31/2006				

OCT 3 1 2006

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Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)** **EWPL INC (DBA Name)**
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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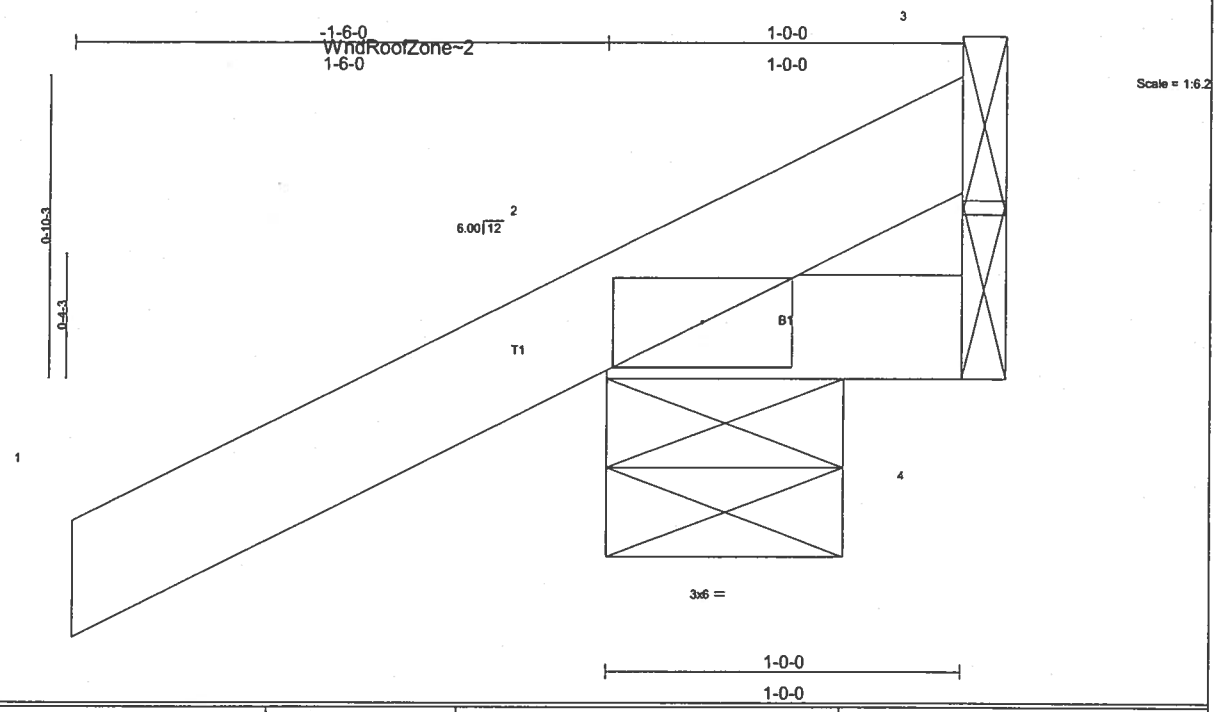
October 31, 2006 TRUSS Design Engineer:
Thomas E. Miller PE 56877, Byron K. Anderson PE 60987
Structural Engineering and Inspections, Inc EB 9196
16105 N. Florida Ave. Ste B, Lutz, FL 33549

Dwg.#1031061299

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	CJ1	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:00:14 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=195/0-8-0, 4=14/Mechanical, 3=-47/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-230(load case 5), 3=-47(load case 1)
Max Grav 2=195(load case 1), 4=14(load case 1), 3=82(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/48
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 47 lb uplift at joint 3.

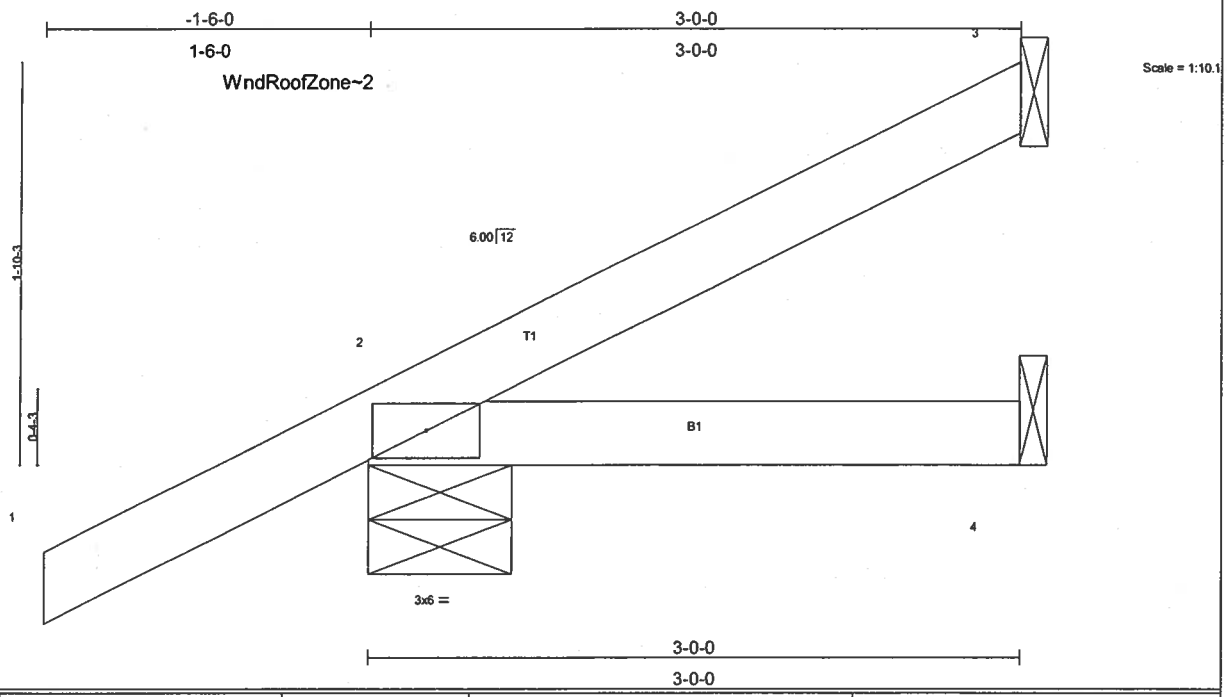
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061300

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	CJ3	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:00:27 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=35/Mechanical, 2=243/0-8-0, 4=39/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-38(load case 6), 2=-245(load case 5), 4=-31(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-53/11
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 245 lb uplift at joint 2 and 31 lb uplift at joint 4.

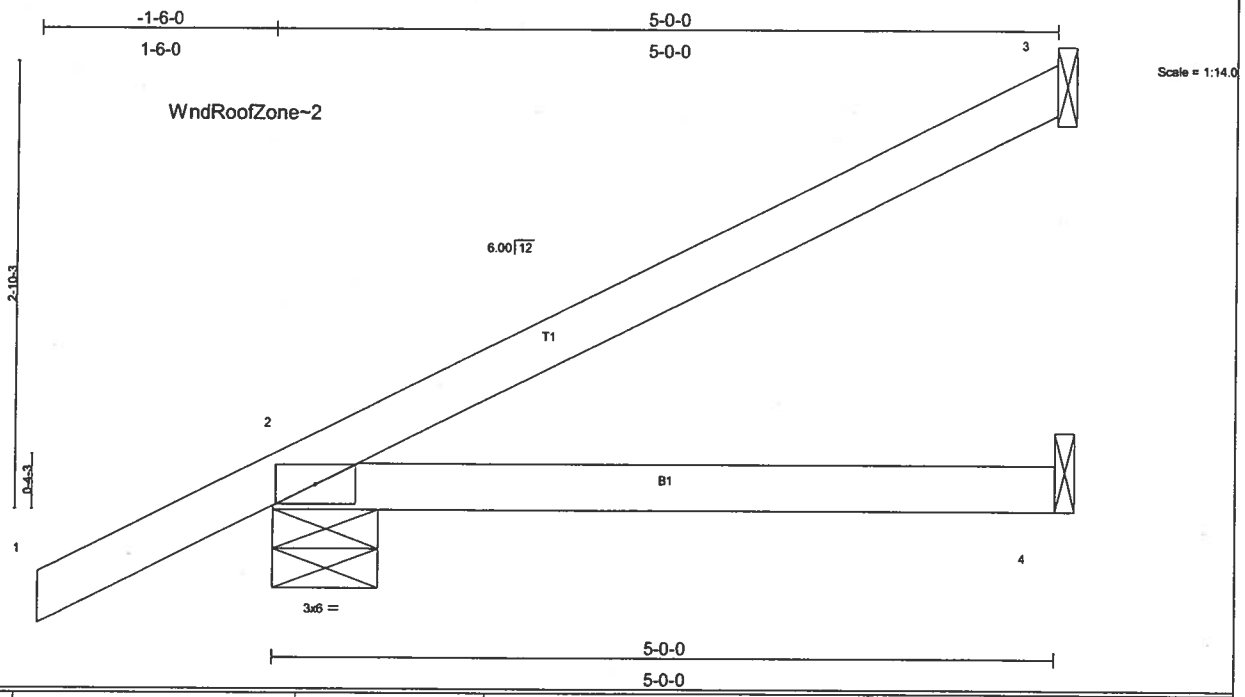
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061301

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	CJ5	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:00:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.09	2-4	>628	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.08	2-4	>714	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=105/Mechanical, 2=312/0-8-0, 4=69/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-115(load case 5), 2=-279(load case 5), 4=-55(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-109/38
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.16

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 3, 279 lb uplift at joint 2 and 55 lb uplift at joint 4.

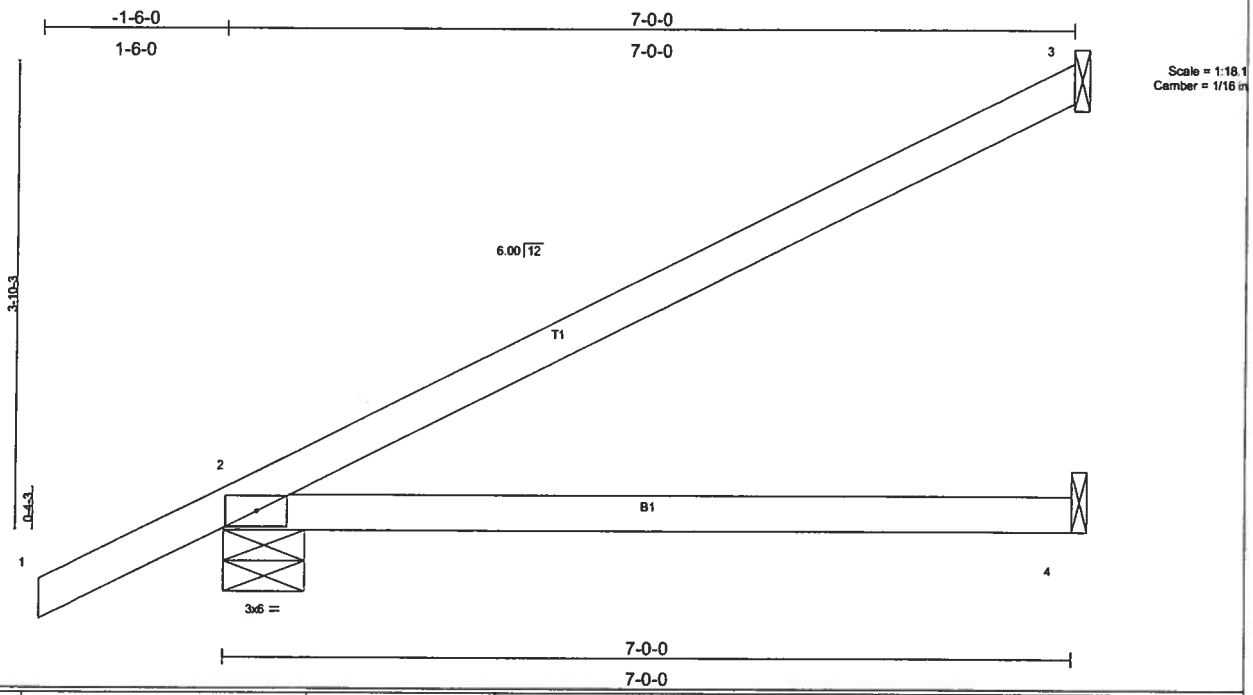
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061302

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	EJ7	MONO TRUSS	41	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:01:09 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.28	2-4	>284	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.24	2-4	>335	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=158/Mechanical, 2=390/0-8-0, 4=105/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-176(load case 5), 2=-327(load case 5), 4=-87(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-108/57
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.61

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 3, 327 lb uplift at joint 2 and 87 lb uplift at joint 4.

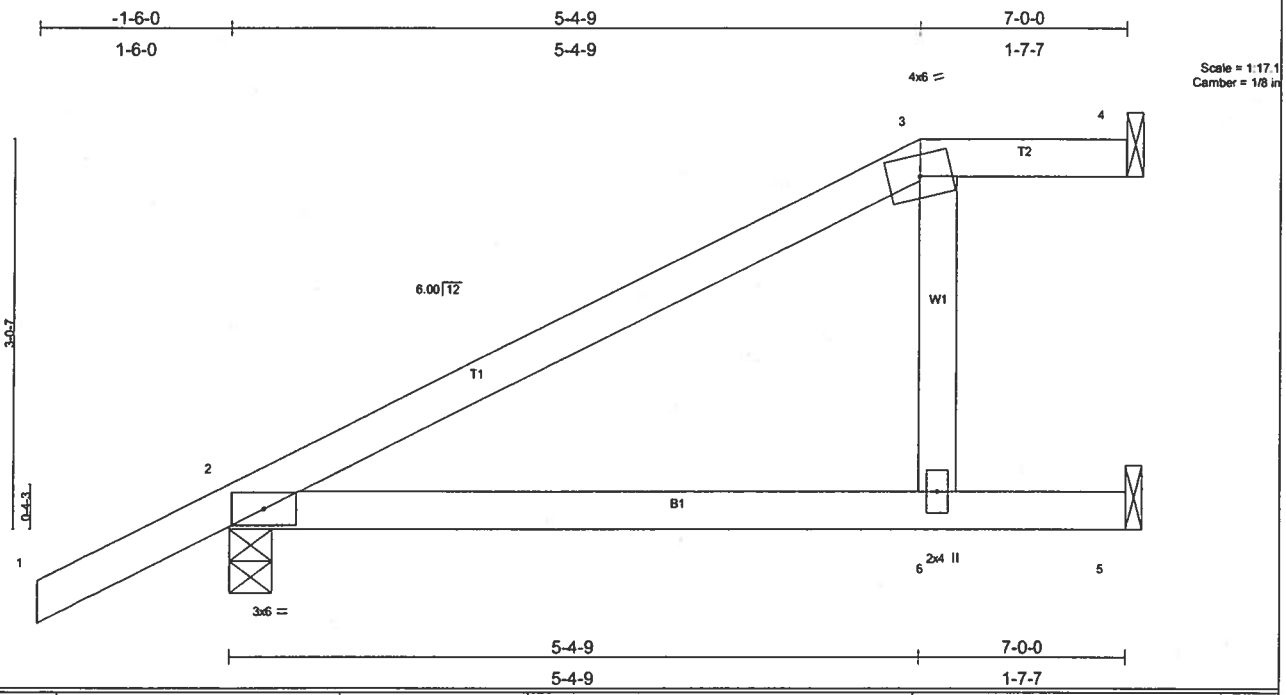
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061303

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	EJ7A	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:01:59 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.15	2-6	>525	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>324	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.08	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=111/Mechanical, 2=385/0-4-0, 5=162/Mechanical
Max Horz 2=172(load case 5)
Max Uplift4=-33(load case 4), 2=-187(load case 5), 5=-80(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-85/33, 3-4=-0/1
BOT CHORD 2-6=-7/4, 5-6=0/0
WEBS 3-6=-79/123

JOINT STRESS INDEX
2 = 0.26, 3 = 0.64 and 6 = 0.07

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 4, 187 lb uplift at joint 2 and 80 lb uplift at joint 5.

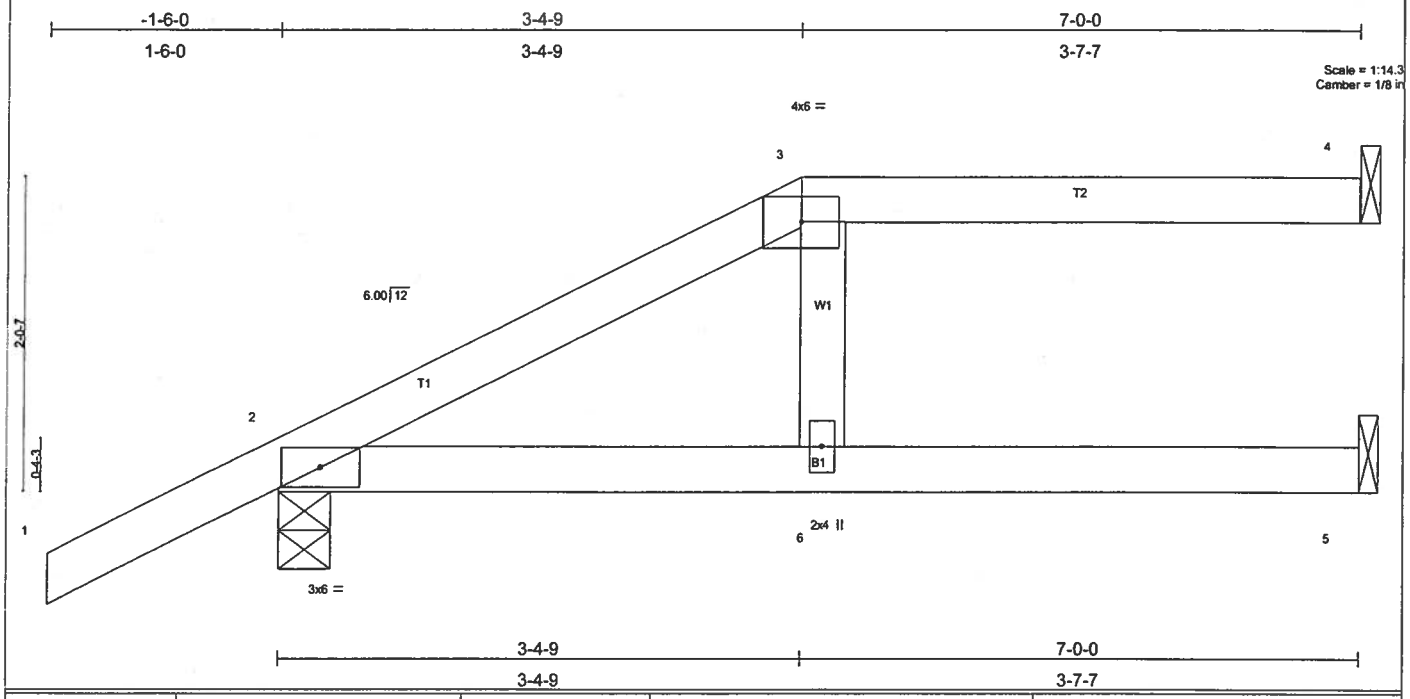
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061304

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:02:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.16	6	>523	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.25	6	>325	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.13	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=149/Mechanical, 2=385/0-4-0, 5=124/Mechanical
Max Horz 2=126(load case 5)
Max Uplift4=-78(load case 4), 2=-191(load case 5), 5=-19(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-81/2, 3-4=-0/0
BOT CHORD 2-6=-7/5, 5-6=0/0
WEBS 3-6=-56/86

JOINT STRESS INDEX
2 = 0.30, 3 = 0.73 and 6 = 0.05

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 4, 191 lb uplift at joint 2 and 19 lb uplift at joint 5.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061305

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	EJ7C	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:04:57 2006 Page 1

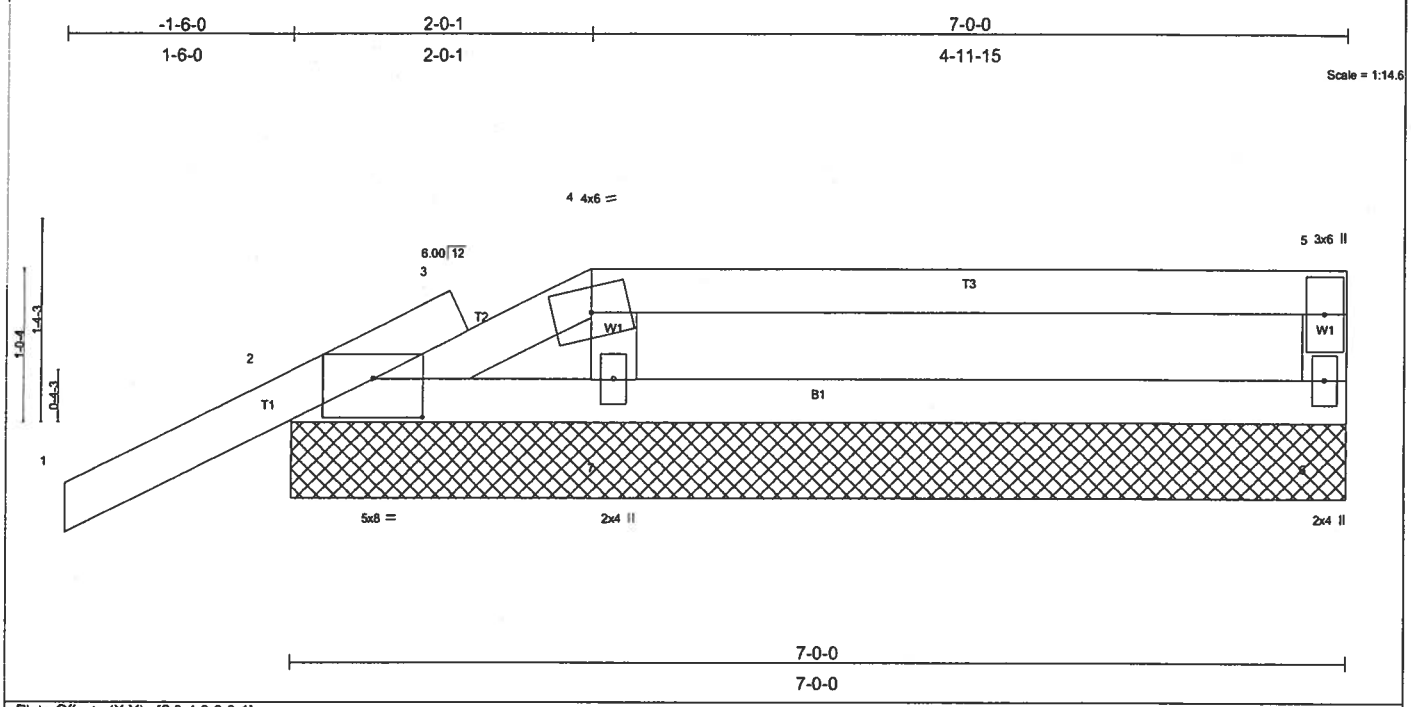


Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.01	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.02	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=319/7-0-0, 6=283/7-0-0, 7=330/7-0-0
Max Horz 2=80(load case 5)
Max Uplift2=-199(load case 5), 6=-110(load case 3), 7=-106(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3/56, 2-3=-342/127, 3-4=-279/118, 5-6=-211/123, 4-5=-265/108
BOT CHORD 2-7=-134/309, 6-7=-108/265
WEBS 4-7=-224/143

JOINT STRESS INDEX
2 = 0.56, 3 = 0.00, 4 = 0.51, 5 = 0.46, 6 = 0.58 and 7 = 0.08

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 2, 110 lb uplift at joint 6 and 106 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 2-6=-30, 4-5=-87(F=-33)

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061306

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	HJ9	MONO TRUSS	6	1	Job Reference (optional)

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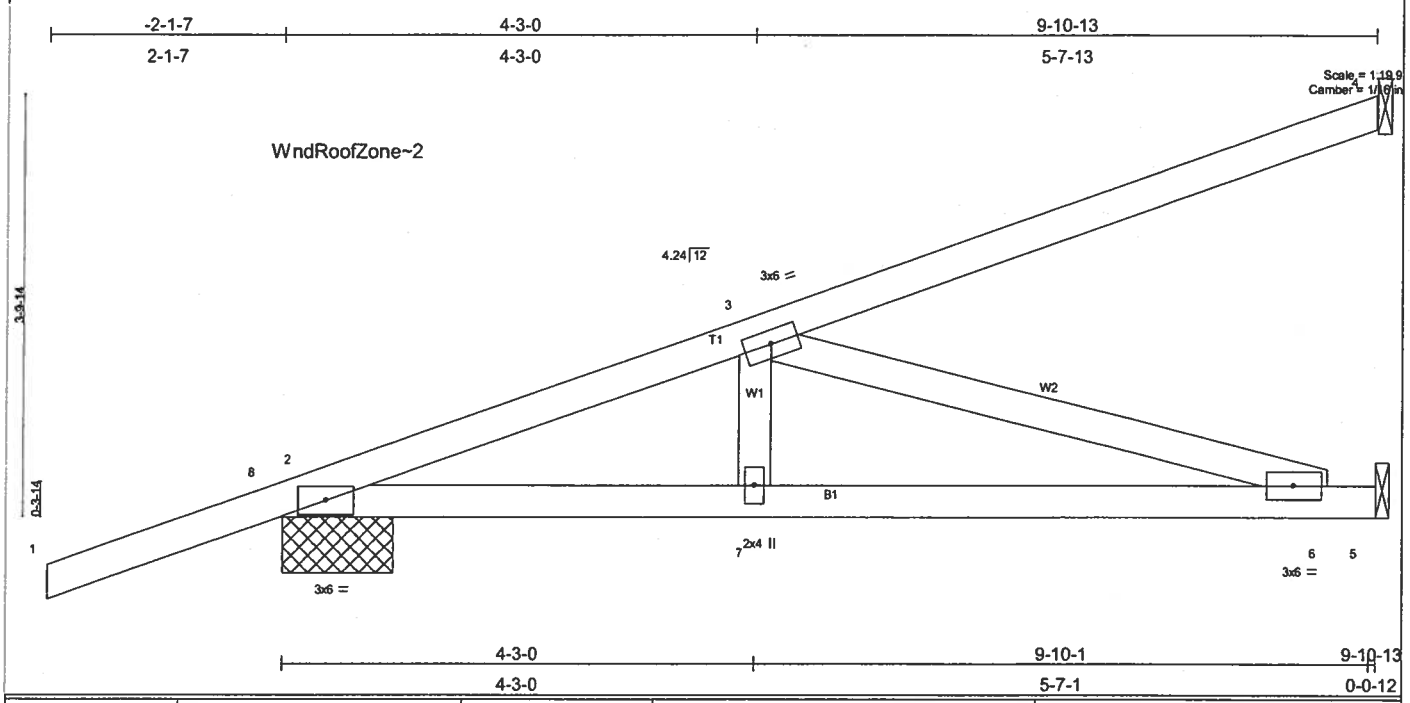

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.12	6-7	>974	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-3 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=484/1-0-1, 5=385/Mechanical
Max Horz 2=296(load case 2)
Max Uplift4=-283(load case 2), 2=-420(load case 2), 5=-242(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=0/33, 2-8=0/33, 2-3=-929/516, 3-4=-122/65
BOT CHORD 2-7=-722/862, 6-7=-722/862, 5-6=0/0
WEBS 3-7=-121/211, 3-6=-898/752

JOINT STRESS INDEX
2 = 0.77, 3 = 0.31, 6 = 0.25 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 283 lb uplift at joint 4, 420 lb uplift at joint 2 and 242 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-54
Trapezoidal Loads (plf)
Vert: 8=0(F=27, B=27)-to-4=-134(F=-40, B=-40), 2=-3(F=14, B=14)-to-5=-74(F=-22, B=-22)

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Dwg.#1031061307

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	T01	HIP	1	1	Job Reference (optional)

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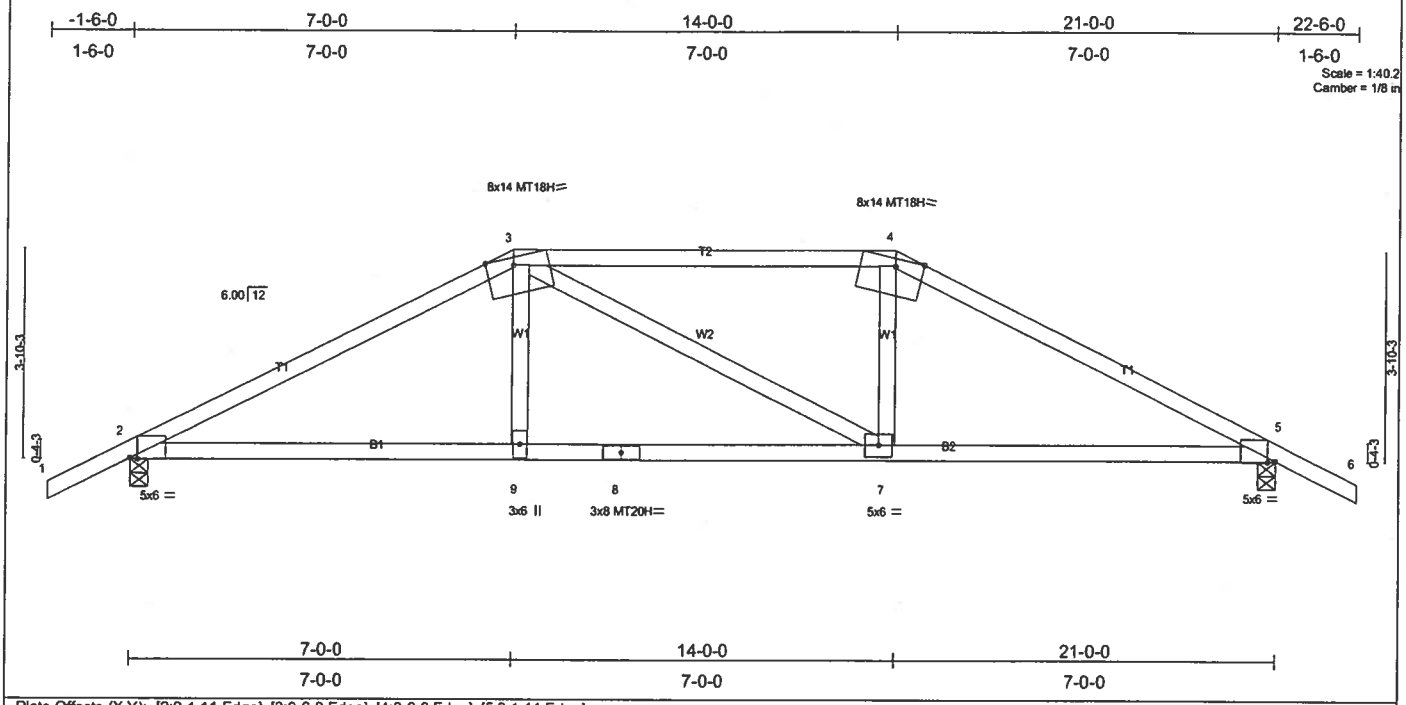

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-6-3,Edge], [4:0-6-3,Edge], [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.23	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.38	7-9	>659	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.30	Horz(TL)	0.10	5	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.

REACTIONS (lb/size) 2=1835/0-4-0, 5=1835/0-4-0
Max Horz 2=-78(load case 5)
Max Uplift2=-810(load case 4), 5=-810(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3378/1371, 3-4=-2979/1304, 4-5=-3379/1371, 5-6=0/35
BOT CHORD 2-9=-1180/2942, 8-9=-1189/2978, 7-8=-1189/2978, 5-7=-1136/2943
WEBS 3-9=-212/889, 3-7=-133/136, 4-7=-233/933

JOINT STRESS INDEX
2 = 0.84, 3 = 0.95, 4 = 0.96, 5 = 0.84, 7 = 0.33, 8 = 0.90 and 9 = 0.29

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 810 lb uplift at joint 2 and 810 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 14-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-117(F=-63), 4-6=-54, 2-9=-30, 7-9=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 9=-539(F) 7=-539(F)

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061308

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	T02	HIP	1	1	Job Reference (optional)

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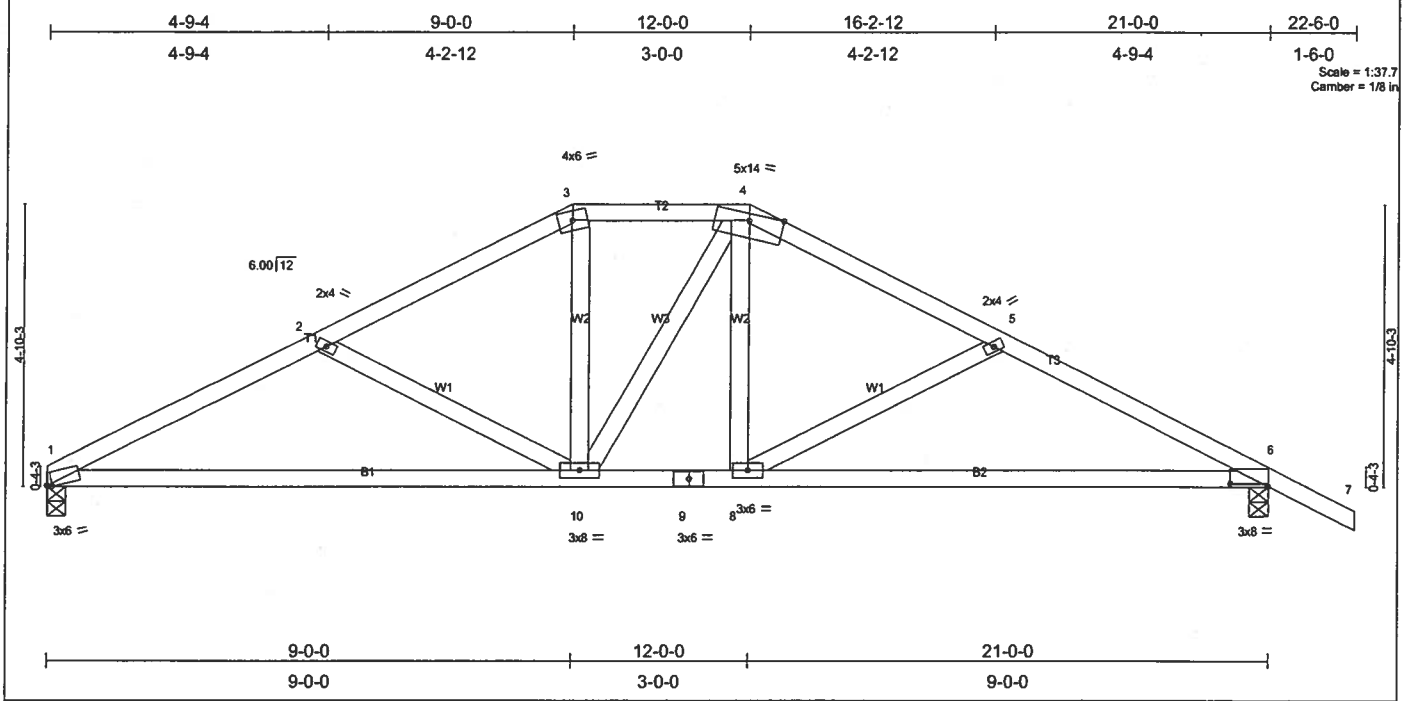

Plate Offsets (X,Y): [1:0-1-1,0-0-7], [6:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.18	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.31	1-10	>794	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 103 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-12 oc bracing.

REACTIONS (lb/size) 1=864/0-4-0, 6=962/0-4-0
Max Horz 1=-110(load case 6)
Max Uplift1=-272(load case 5), 6=-371(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1455/488, 2-3=-1178/362, 3-4=-1009/365, 4-5=-1171/348, 5-6=-1439/461, 6-7=0/35
BOT CHORD 1-10=-417/1269, 9-10=-173/1003, 8-9=-173/1003, 6-8=-301/1249
WEBS 2-10=-307/245, 3-10=-75/318, 4-10=-99/114, 4-8=-62/308, 5-8=-291/223

JOINT STRESS INDEX
1 = 0.89, 2 = 0.34, 3 = 0.29, 4 = 0.30, 5 = 0.34, 6 = 0.73, 8 = 0.35, 9 = 0.62 and 10 = 0.60

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 1 and 371 lb uplift at joint 6.

LOAD CASE(S) Standard

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Dwg.#1031061309

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	T03	COMMON	6	1	Job Reference (optional)

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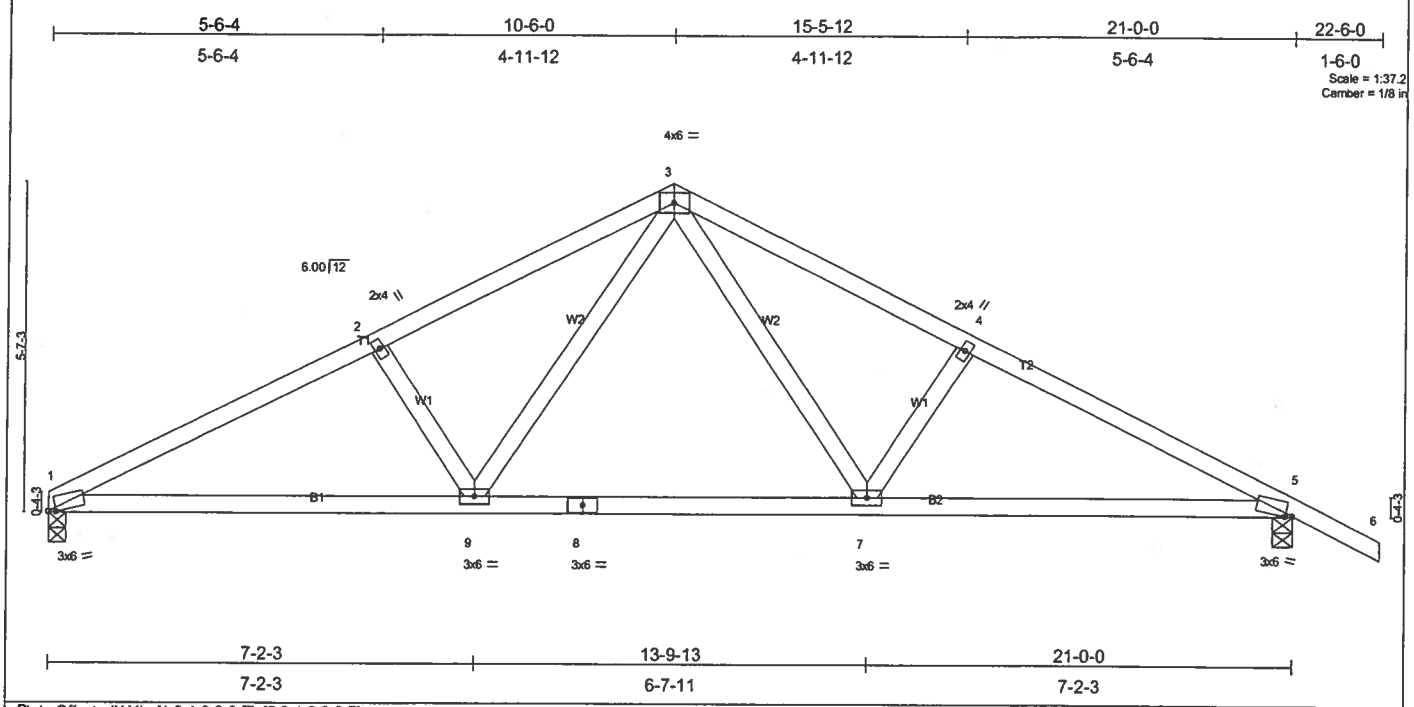

Plate Offsets (X,Y): [1:0-1-9,0-0-7], [5:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.29	7-9	>859	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-10 oc bracing.

REACTIONS (lb/size) 1=1031/0-3-8, 5=1129/0-4-0
Max Horz 1=-120(load case 6)
Max Uplift1=-344(load case 5), 5=-443(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1861/628, 2-3=-1721/626, 3-4=-1704/602, 4-5=-1856/602, 5-6=0/35
BOT CHORD 1-9=-543/1611, 8-9=-273/1098, 7-8=-273/1098, 5-7=-429/1590
WEBS 2-9=-256/231, 3-9=-276/740, 3-7=-249/716, 4-7=-242/217

JOINT STRESS INDEX
1 = 0.81, 2 = 0.34, 3 = 0.59, 4 = 0.34, 5 = 0.81, 7 = 0.56, 8 = 0.61 and 9 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 344 lb uplift at joint 1 and 443 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-30, 7-9=-80(F=-50), 5-7=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061310

Job	Truss	Truss Type	Qty	Ply	KARCHER RESIDENCE
L215867	T04	MONO HIP	1	1	Job Reference (optional)

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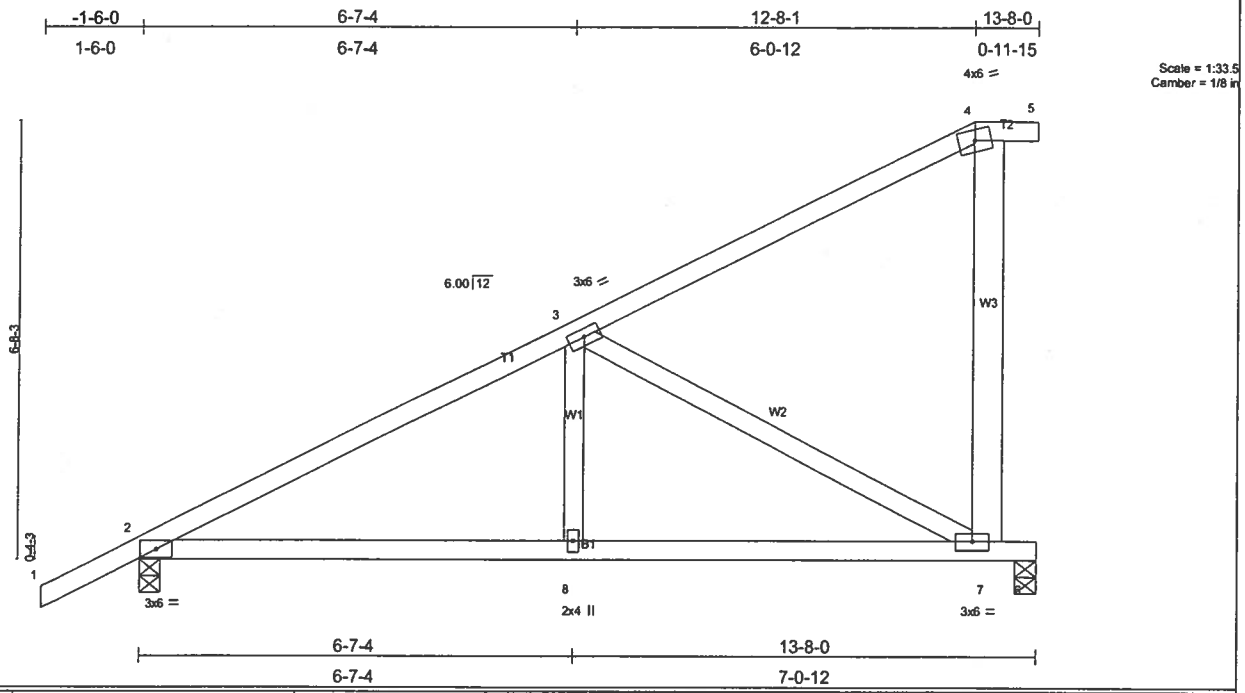

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.19	7-8	>822	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.32	7-8	>502	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 74 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-12 oc bracing.

REACTIONS (lb/size) 2=656/0-4-0, 6=563/0-4-0
Max Horz 2=341(load case 5)
Max Uplift2=-242(load case 5), 6=-286(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-840/150, 3-4=-95/37, 4-5=-0/0
BOT CHORD 2-8=-343/682, 7-8=-343/682, 6-7=0/0
WEBS 3-8=-5/321, 3-7=-764/382, 4-7=-182/160

JOINT STRESS INDEX
2 = 0.42, 3 = 0.24, 4 = 0.14, 7 = 0.22 and 8 = 0.23

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 2 and 286 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061311

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T05	HIP	1	1	Job Reference (optional)

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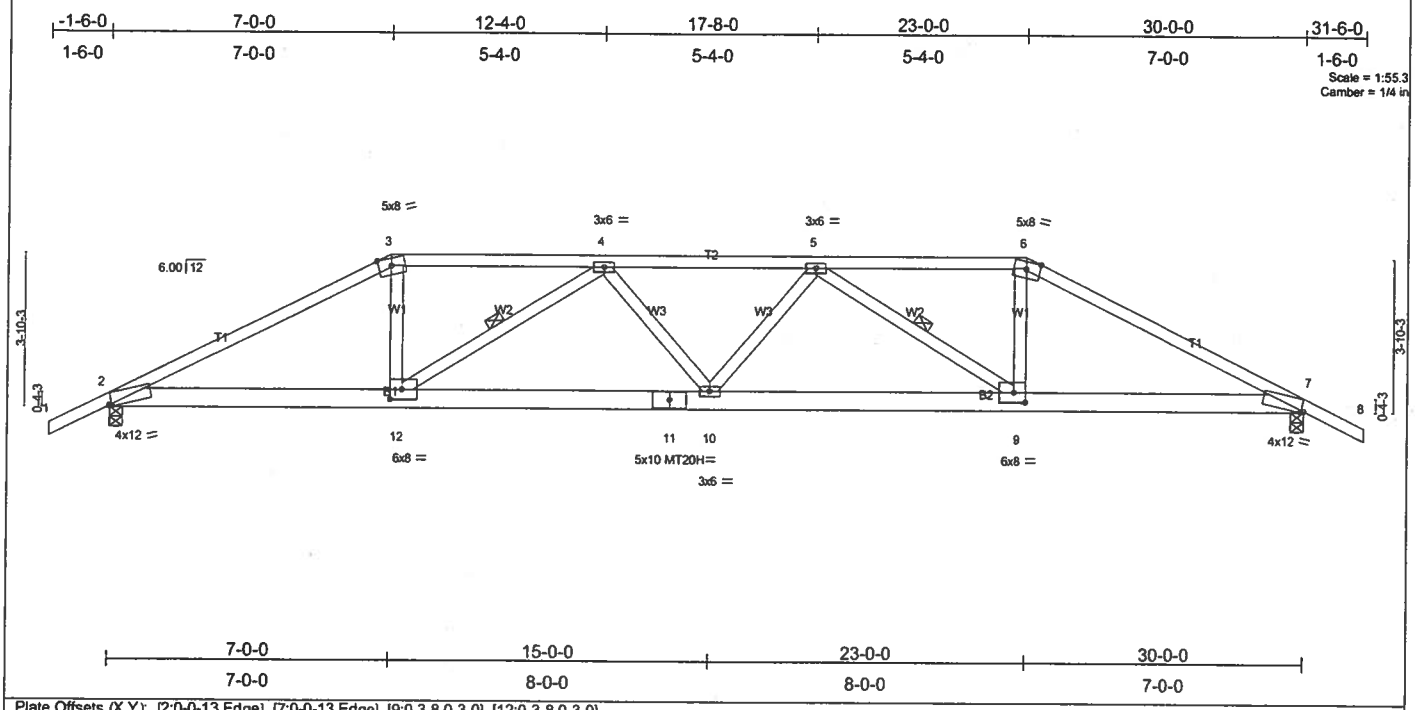

Plate Offsets (X,Y): [2:0-0-13,Edge], [7:0-0-13,Edge], [9:0-3-8,0-3-0], [12:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.38	10-12	>934	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.61	10-12	>581	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.59	Horz(TL)	0.15	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-7-2 oc bracing.
WEBS 1 Row at midpt 4-12, 5-9

REACTIONS (lb/size) 2=2654/0-4-0, 7=2654/0-4-0
Max Horz 2=80(load case 4)
Max Uplift2=-1495(load case 4), 7=-1495(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5203/2827, 3-4=-4648/2606, 4-5=-6196/3446, 5-6=-4648/2606, 6-7=-5203/2827, 7-8=0/39
BOT CHORD 2-12=-2493/4571, 11-12=-3347/5985, 10-11=-3347/5985, 9-10=-3326/5985, 7-9=-2451/4571
WEBS 3-12=-973/1841, 4-12=-1709/1058, 4-10=-75/380, 5-10=-75/380, 5-9=-1709/1058, 6-9=-973/1841

JOINT STRESS INDEX
2 = 0.78, 3 = 1.00, 4 = 0.50, 5 = 0.50, 6 = 1.00, 7 = 0.78, 9 = 0.51, 10 = 0.39, 11 = 0.75 and 12 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1495 lb uplift at joint 2 and 1495 lb uplift at joint 7.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 23-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-117(F=-63), 6-8=-54, 2-12=-30, 9-12=-65(F=-35), 7-9=-30
Concentrated Loads (lb)
Vert: 12=-539(F) 9=-539(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061312

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T06	SPECIAL	1	1	Job Reference (optional)

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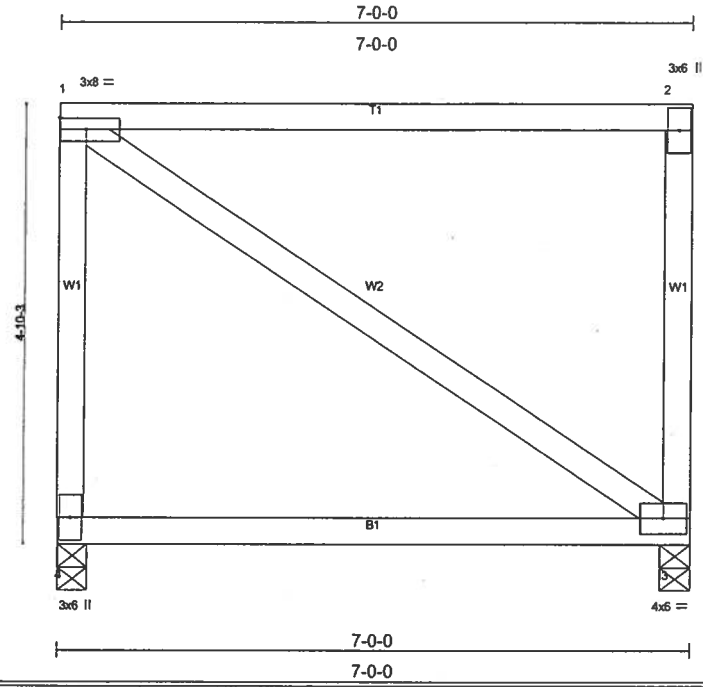


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.13	3-4	>621	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	0.11	3-4	>707	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=282/0-4-0, 3=282/0-4-0
Max Uplift4=-191(load case 3), 3=-191(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-181/126, 1-2=-55/59, 2-3=-181/126
BOT CHORD 3-4=-59/55
WEBS 1-3=-1/0

JOINT STRESS INDEX
1 = 0.59, 2 = 0.43, 3 = 0.79 and 4 = 0.43

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 4 and 191 lb uplift at joint 3.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061313

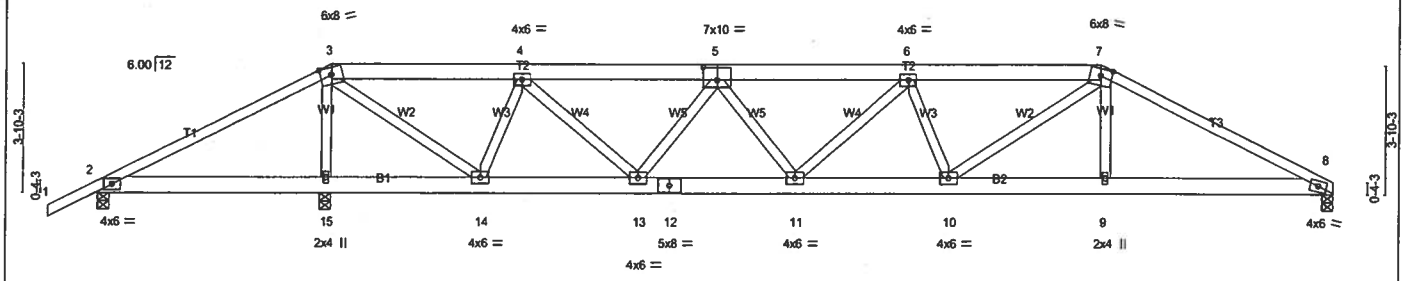
Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T07	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:26:25 2006 Page 1

1-6-0	7-0-0	12-9-7	18-7-0	24-4-8	30-2-0	37-2-0
1-6-0	7-0-0	5-9-7	5-9-8	5-9-8	5-9-8	7-0-0

Scale = 1:66.0
Camber = 1/16 in

* Uplift in gravity load case.



6-10-0	7-0-0	11-6-6	16-2-13	20-11-3	25-7-10	30-2-0	30-4-0	37-2-0
6-10-0	0-2-0	4-6-6	4-8-6	4-8-6	4-8-6	4-6-6	0-2-0	6-10-0

Plate Offsets (X,Y): [5:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.12	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.18	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 226 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T1 2 X 4 SYP No.1D, T3 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 8=1128/0-4-0, 2=-243/0-4-0, 15=2299/0-4-0
Max Horz 15=99(load case 4)
Max Uplift8=-321(load case 2), 2=-443(load case 9), 15=-835(load case 3)
Max Grav 8=1129(load case 9), 2=81(load case 2), 15=2299(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-432/1324, 3-4=-437/201, 4-5=-1562/600, 5-6=-2131/790, 6-7=-2174/793, 7-8=-2073/652
BOT CHORD 2-15=-1132/424, 14-15=-1051/415, 13-14=-306/825, 12-13=-685/1955, 11-12=-685/1955, 10-11=-756/2243, 9-10=-525/1773, 8-9=-525/1783
WEBS 3-14=-651/1864, 4-14=-1119/488, 4-13=-324/1050, 5-13=-702/299, 5-11=-36/308, 6-11=-157/85, 6-10=-270/200, 7-10=-270/624, 7-9=-1/233, 3-15=-2017/726

JOINT STRESS INDEX
2 = 0.48, 3 = 0.95, 4 = 0.45, 5 = 0.20, 6 = 0.33, 7 = 0.35, 8 = 0.82, 9 = 0.34, 10 = 0.33, 11 = 0.27, 12 = 0.39, 13 = 0.45, 14 = 0.77 and 15 = 0.74

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 8, 443 lb uplift at joint 2 and 835 lb uplift at joint 15.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061314

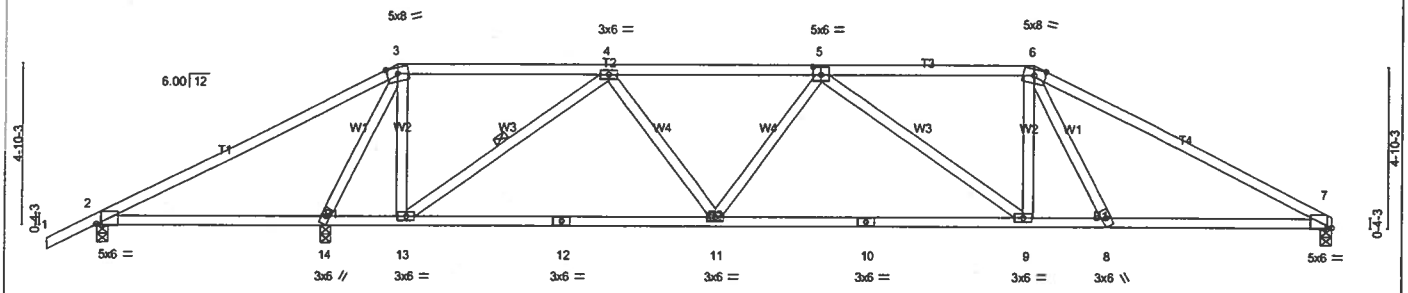
Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T08	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:27:20 2006 Page 1

1-6-0 | 8-11-15 | 15-4-10 | 21-9-5 | 28-2-0 | 37-2-0

1-6-0 | 8-11-15 | 6-4-11 | 6-4-11 | 6-4-11 | 9-0-0

Scale = 1:66.0
Camber = 3/16 in



6-10-0 | 8-11-15 | 18-7-0 | 28-2-0 | 30-3-15 | 37-2-0

6-10-0 | 2-1-15 | 9-7-0 | 9-7-0 | 2-1-15 | 6-10-1

Plate Offsets (X,Y): [2:0-1-10,Edge], [5:0-3-0,0-3-0], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.24	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.39	9-11	>922	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-14 oc bracing.
WEBS 1 Row at midpt 4-13

REACTIONS (lb/size) 7=1170/0-4-0, 2=-81/0-4-0, 14=2094/0-4-0
Max Horz 2=110(load case 5)
Max Uplift7=-350(load case 6), 2=-291(load case 10), 14=-617(load case 4)
Max Grav 7=1170(load case 1), 2=89(load case 3), 14=2094(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-325/1081, 3-4=-128/148, 4-5=-1630/589, 5-6=-1666/576, 6-7=-2066/599
BOT CHORD 2-14=-859/343, 13-14=-21/98, 12-13=-429/1257, 11-12=-429/1257, 10-11=-564/1787, 9-10=-564/1787, 8-9=-447/1658, 7-8=-456/1748
WEBS 3-14=-2100/659, 3-13=-290/1056, 4-13=-1499/572, 4-11=-102/648, 5-11=-275/175, 5-9=-315/215, 6-9=-84/385, 6-8=-69/204

JOINT STRESS INDEX
2 = 0.68, 3 = 0.82, 4 = 0.47, 5 = 0.47, 6 = 0.82, 7 = 0.68, 8 = 0.56, 9 = 0.69, 10 = 0.87, 11 = 0.47, 12 = 0.87, 13 = 0.69 and 14 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 7, 291 lb uplift at joint 2 and 617 lb uplift at joint 14.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061315

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:28:07 2006 Page 1

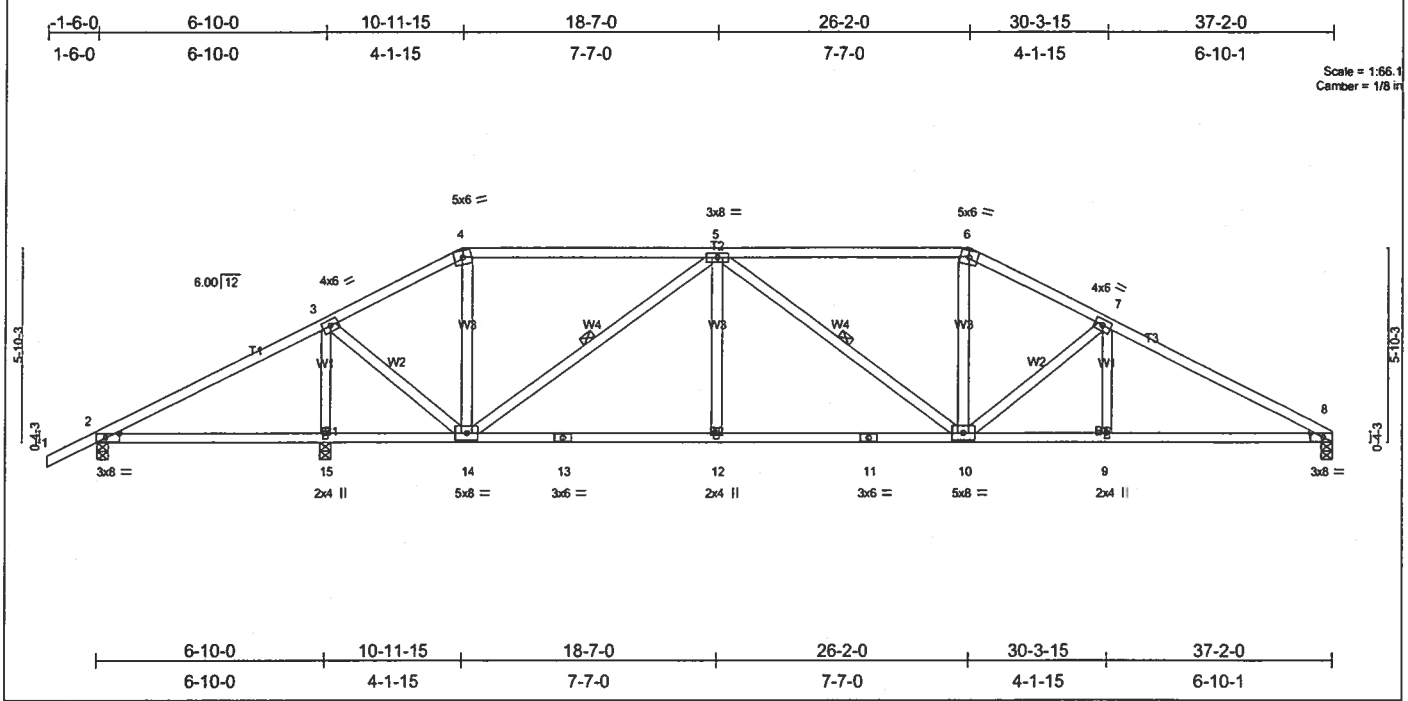


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.15	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.25	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-14, 5-10

REACTIONS (lb/size) 8=1187/0-4-0, 2=20/0-4-0, 15=1977/0-4-0
Max Horz 2=124(load case 5)
Max Uplift8=-369(load case 6), 2=-172(load case 10), 15=-524(load case 5)
Max Grav 8=1187(load case 1), 2=73(load case 9), 15=1977(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-206/769, 3-4=-623/256, 4-5=-509/258, 5-6=-1476/525, 6-7=-1689/552, 7-8=-2134/636
BOT CHORD 2-15=-618/243, 14-15=-618/243, 13-14=-413/1453, 12-13=-413/1453, 11-12=-413/1453, 10-11=-413/1453, 9-10=-475/1827, 8-9=-475/1827
WEBS 3-15=-1773/558, 3-14=-409/1429, 4-14=-40/80, 5-14=-1193/394, 5-12=0/244, 5-10=-123/116, 6-10=-93/435, 7-10=-466/273, 7-9=0/196

JOINT STRESS INDEX
2 = 0.78, 3 = 0.68, 4 = 0.53, 5 = 0.58, 6 = 0.53, 7 = 0.68, 8 = 0.76, 9 = 0.65, 10 = 0.67, 11 = 0.67, 12 = 0.34, 13 = 0.67, 14 = 0.67 and 15 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 8, 172 lb uplift at joint 2 and 524 lb uplift at joint 15.

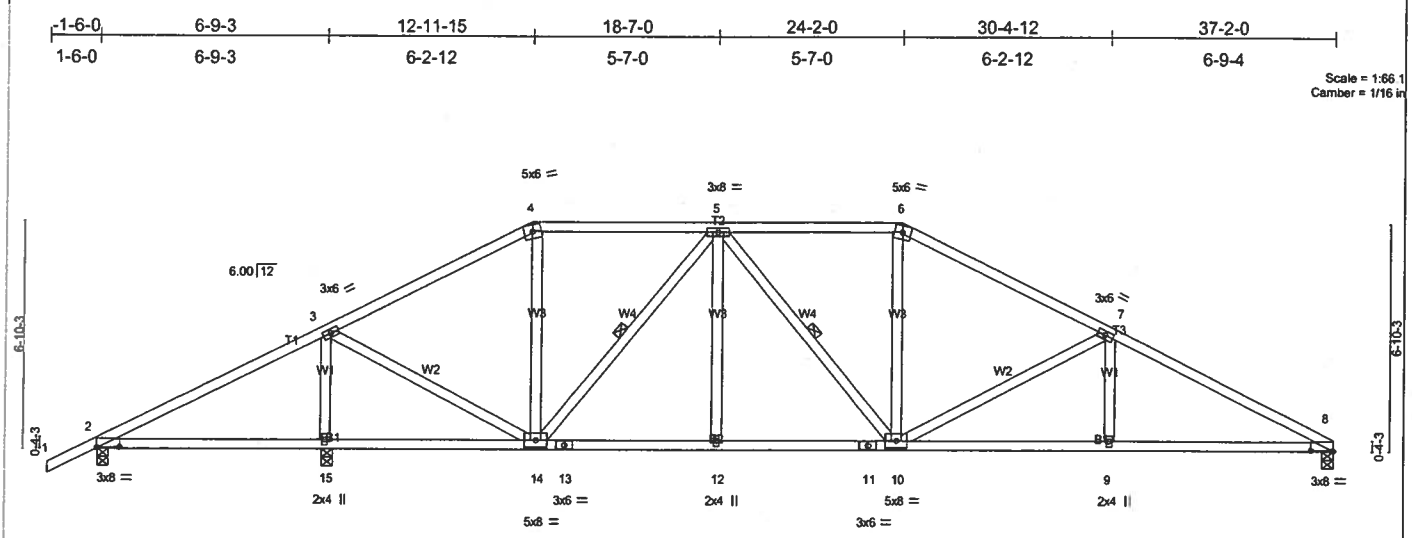
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061316

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:28:36 2006 Page 1



6-10-0	12-11-15	18-7-0	24-2-0	30-4-12	37-2-0
6-10-0	6-1-15	5-7-0	5-7-0	6-2-12	6-9-4

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.13	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.21	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-14, 5-10

REACTIONS (lb/size) 8=1208/0-4-0, 2=94/0-4-0, 15=1882/0-4-0
Max Horz 2=138(load case 5)
Max Uplift8=-388(load case 6), 2=-124(load case 5), 15=-534(load case 5)
Max Grav 8=1208(load case 1), 2=153(load case 9), 15=1882(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-147/582, 3-4=-951/341, 4-5=-779/348, 5-6=-1346/523, 6-7=-1576/521, 7-8=-2203/684
BOT CHORD 2-15=-450/215, 14-15=-450/215, 13-14=-290/1249, 12-13=-290/1249, 11-12=-290/1249, 10-11=-290/1249, 9-10=-522/1893, 8-9=-522/1893
WEBS 3-15=-1647/563, 3-14=-317/1376, 4-14=-10/159, 5-14=-783/250, 5-12=0/160, 5-10=-102/173, 6-10=-49/355, 7-10=-632/343, 7-9=0/230

JOINT STRESS INDEX
2 = 0.68, 3 = 0.85, 4 = 0.48, 5 = 0.57, 6 = 0.48, 7 = 0.85, 8 = 0.67, 9 = 0.60, 10 = 0.63, 11 = 0.45, 12 = 0.34, 13 = 0.45, 14 = 0.63 and 15 = 0.60

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 8, 124 lb uplift at joint 2 and 534 lb uplift at joint 15.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

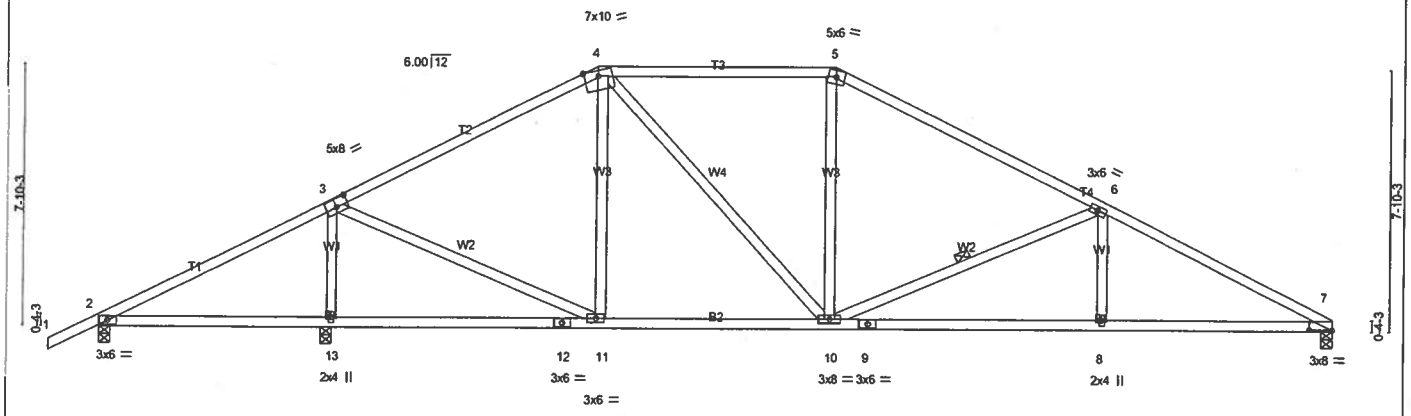
Dwg.#1031061317

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:29:44 2006 Page 1

1-6-0	7-0-0	14-11-15	22-2-0	30-1-15	37-2-0
1-6-0	7-0-0	7-11-15	7-2-1	7-11-15	7-0-1

Scale = 1:66.1
Camber = 1/8 in



7-0-0	14-11-15	22-2-0	30-1-15	37-2-0
7-0-0	7-11-15	7-2-1	7-11-15	7-0-1

Plate Offsets (X,Y): [3:0-4-0,0-3-0], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.16	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.27	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-10

REACTIONS (lb/size) 7=1210/0-4-0, 2=170/0-4-0, 13=1803/0-4-0
Max Horz 2=152(load case 5)
Max Uplift7=-401(load case 6), 2=-134(load case 5), 13=-545(load case 5)
Max Grav 7=1210(load case 1), 2=226(load case 9), 13=1803(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-126/422, 3-4=-1096/381, 4-5=-1203/517, 5-6=-1439/496, 6-7=-2219/717
BOT CHORD 2-13=-300/185, 12-13=-274/175, 11-12=-274/175, 10-11=-162/886, 9-10=-554/1908, 8-9=-554/1908, 7-8=-554/1908
WEBS 3-13=-1539/584, 3-11=-253/1251, 4-11=-292/147, 4-10=-194/541, 5-10=-10/218, 6-10=-773/419, 6-8=0/261

JOINT STRESS INDEX
2 = 0.32, 3 = 0.72, 4 = 0.86, 5 = 0.80, 6 = 0.41, 7 = 0.68, 8 = 0.34, 9 = 0.67, 10 = 0.57, 11 = 0.71, 12 = 0.15 and 13 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 7, 134 lb uplift at joint 2 and 545 lb uplift at joint 13.

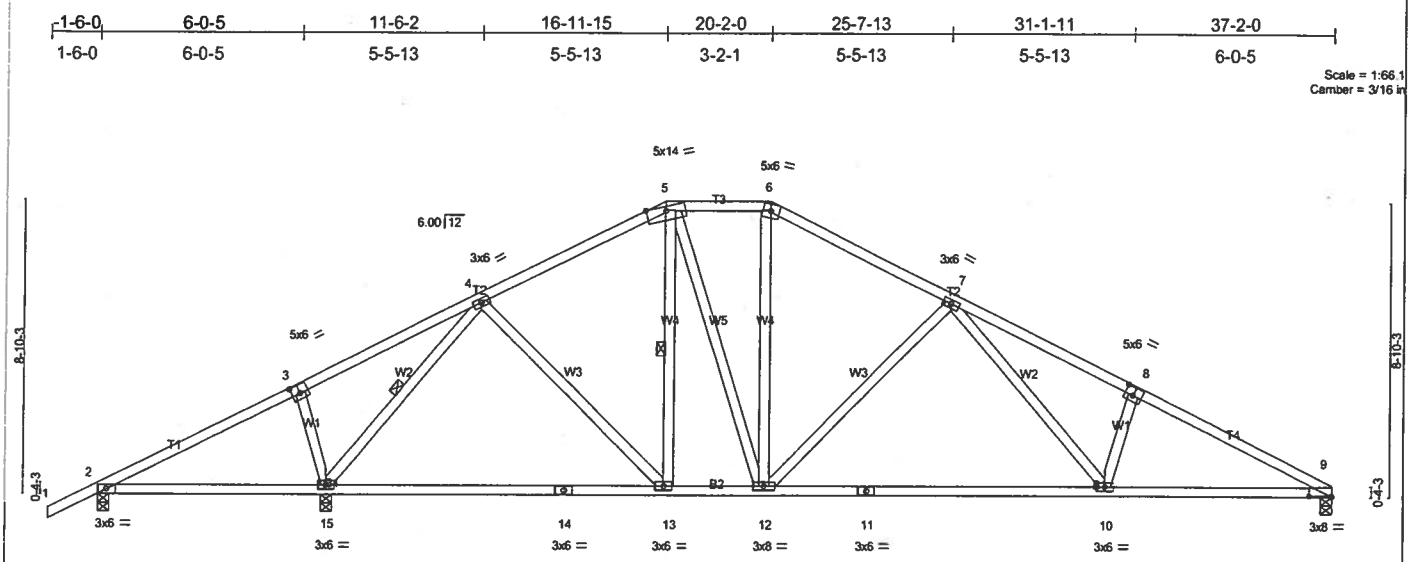
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061318

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T12	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:30:41 2006 Page 1



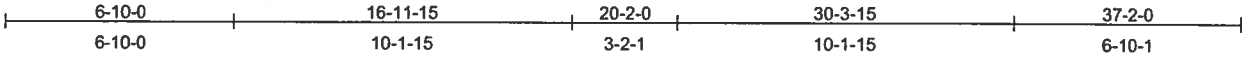


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [8:0-3-0,0-3-0], [9:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.30	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.50	10-12	>717	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-13

REACTIONS (lb/size) 2=116/0-4-0, 15=1859/0-4-0, 9=1208/0-4-0
Max Horz 2=166(load case 5)
Max Uplift2=-110(load case 5), 15=-587(load case 5), 9=-408(load case 6)
Max Grav 2=184(load case 9), 15=1859(load case 1), 9=1208(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-168/456, 3-4=-96/532, 4-5=-1074/432, 5-6=-1017/476, 6-7=-1203/474, 7-8=-2157/800, 8-9=-2253/731
BOT CHORD 2-15=-338/238, 14-15=-152/601, 13-14=-152/601, 12-13=-120/896, 11-12=-341/1427, 10-11=-341/1427, 9-10=-573/1941
WEBS 3-15=-305/262, 4-15=-1624/497, 4-13=-38/475, 5-13=-148/64, 5-12=-174/449, 6-12=-95/282, 7-12=-595/374, 7-10=-257/709, 8-10=-260/255

JOINT STRESS INDEX
2 = 0.23, 3 = 0.66, 4 = 0.54, 5 = 0.51, 6 = 0.33, 7 = 0.47, 8 = 0.48, 9 = 0.68, 10 = 0.51, 11 = 0.75, 12 = 0.67, 13 = 0.37, 14 = 0.47 and 15 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 2, 587 lb uplift at joint 15 and 408 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061319

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T13	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:32:22 2006 Page 1

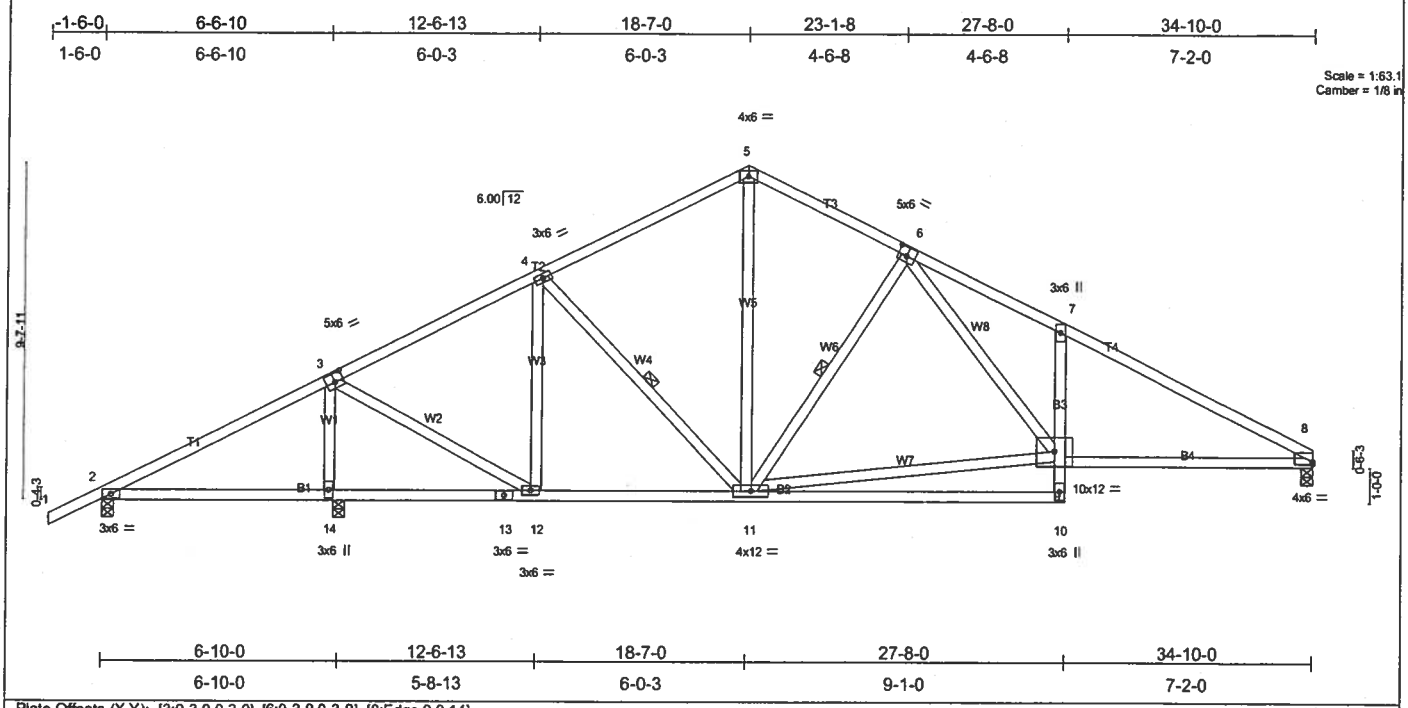


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:Edge,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.17	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.28	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-11, 6-11

REACTIONS (lb/size) 2=125/0-4-0, 8=1125/0-4-0, 14=1738/0-4-0
Max Horz 2=209(load case 5)
Max Uplift2=-96(load case 5), 8=-376(load case 6), 14=-585(load case 5)
Max Grav 2=198(load case 9), 8=1125(load case 1), 14=1738(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-207/504, 3-4=-881/335, 4-5=-975/436, 5-6=-956/428, 6-7=-1868/760, 7-8=-2013/654
BOT CHORD 2-14=-369/215, 13-14=-333/203, 12-13=-333/203, 11-12=-198/720, 10-11=-5/196, 9-10=0/138, 7-9=-243/268, 8-9=-480/1706
WEBS 3-14=-1511/611, 3-12=-310/1209, 4-12=-445/189, 4-11=-15/220, 5-11=-211/504, 6-11=-582/339, 9-11=-237/934, 6-9=-352/837

JOINT STRESS INDEX
2 = 0.35, 3 = 0.78, 4 = 0.41, 5 = 0.48, 6 = 0.40, 7 = 0.47, 8 = 0.71, 9 = 0.90, 10 = 0.73, 11 = 0.54, 12 = 0.70, 13 = 0.15 and 14 = 0.25

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 2, 376 lb uplift at joint 8 and 585 lb uplift at joint 14.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061320

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:38:43 2006 Page 1

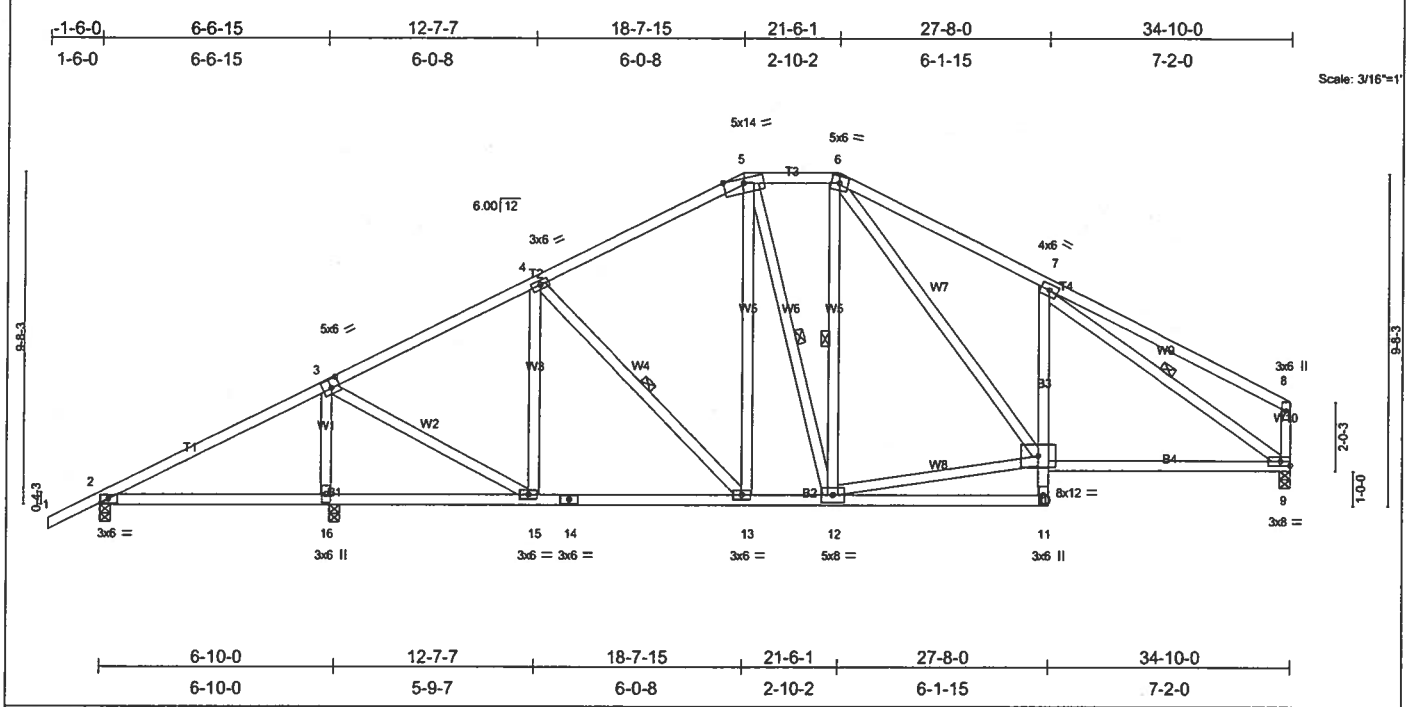


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	0.10	2-16	>765	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	0.08	2-16	>952	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-16,15-16.
WEBS 1 Row at midpt 4-13, 5-12, 6-12, 7-9

REACTIONS (lb/size) 2=227/0-4-0, 16=1614/0-4-0, 9=1148/0-4-0
Max Horz 2=267(load case 5)
Max Uplift2=-183(load case 5), 16=-655(load case 5), 9=-366(load case 6)
Max Grav 2=247(load case 9), 16=1614(load case 1), 9=1148(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-176/288, 3-4=-976/320, 4-5=-1004/407, 5-6=-853/424, 6-7=-1423/636, 7-8=-332/214, 8-9=-304/220
BOT CHORD 2-16=-177/80, 15-16=-143/70, 14-15=-252/805, 13-14=-252/805, 12-13=-184/832, 11-12=-36/91, 10-11=0/99, 7-10=-140/289, 9-10=-324/1224
WEBS 3-16=-1394/587, 3-15=-271/1090, 4-15=-360/191, 4-13=-36/129, 5-13=-54/153, 5-12=-85/212, 6-12=-202/129, 10-12=-144/777, 6-10=-330/593, 7-9=-1251/281

JOINT STRESS INDEX
2 = 0.40, 3 = 0.75, 4 = 0.41, 5 = 0.62, 6 = 0.31, 7 = 0.53, 8 = 0.68, 9 = 0.58, 10 = 0.97, 11 = 0.38, 12 = 0.36, 13 = 0.37, 14 = 0.26, 15 = 0.63 and 16 = 0.23

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 2, 655 lb uplift at joint 16 and 366 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

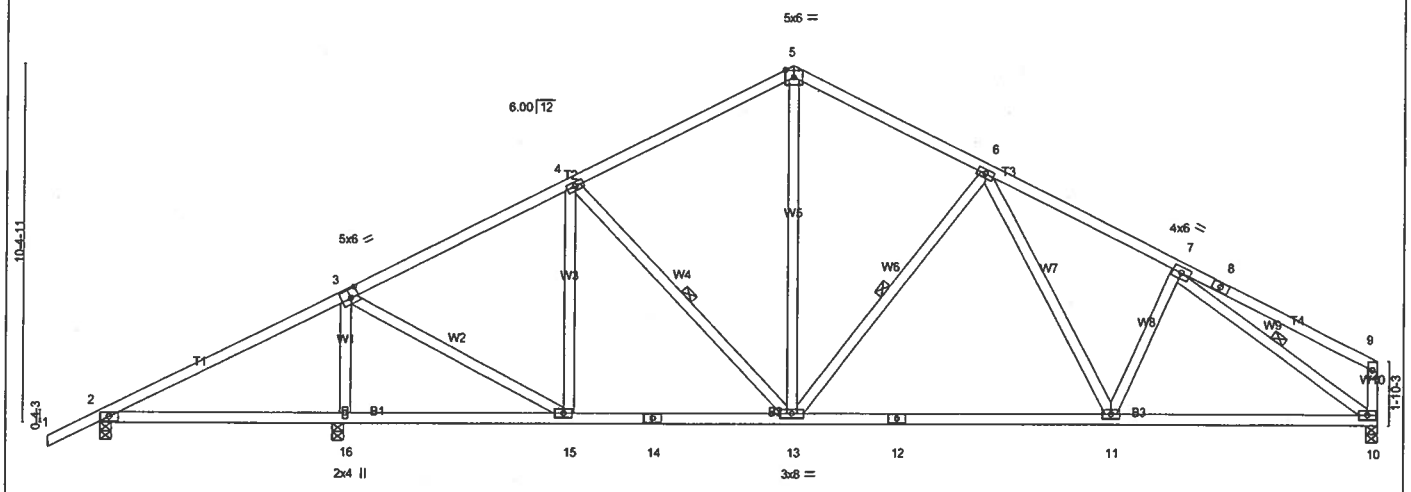
Dwg.#1031061321

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T15	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:39:11 2006 Page 1

-1-6-0 | 7-0-10 | 13-6-13 | 20-1-0 | 25-8-3 | 31-3-5 | 37-2-0
1-6-0 | 7-0-10 | 6-6-3 | 6-6-3 | 5-7-3 | 5-7-3 | 5-10-11

Scale: 3/16"=1'
Camber = 1/8 in



6-10-0 | 13-6-13 | 20-1-0 | 29-4-6 | 37-2-0
6-10-0 | 6-8-13 | 6-6-3 | 9-3-6 | 7-9-10

Plate Offsets (X,Y): [3:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.18	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.31	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-13, 7-10

REACTIONS (lb/size) 2=218/0-4-0, 16=1749/0-4-0, 10=1219/0-4-0
Max Horz 2=239(load case 5)
Max Uplift2=-115(load case 5), 16=-603(load case 5), 10=-406(load case 6)
Max Grav 2=260(load case 9), 16=1749(load case 1), 10=1219(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-178/345, 3-4=-1021/379, 4-5=-1072/474, 5-6=-1057/460, 6-7=-1506/589, 7-8=-144/154, 8-9=-268/143, 9-10=-245/167
BOT CHORD 2-16=-221/155, 15-16=-190/144, 14-15=-245/839, 13-14=-245/839, 12-13=-271/1157, 11-12=-271/1157, 10-11=-391/1281
WEBS 3-16=-1503/630, 3-15=-304/1177, 4-15=-415/189, 4-13=-57/181, 5-13=-222/552, 6-13=-480/319, 6-11=-111/315, 7-11=-20/173, 7-10=-1404/414

JOINT STRESS INDEX
2 = 0.44, 3 = 0.78, 4 = 0.41, 5 = 0.39, 6 = 0.43, 7 = 0.40, 8 = 0.15, 9 = 0.47, 10 = 0.65, 11 = 0.47, 12 = 0.60, 13 = 0.57, 14 = 0.36, 15 = 0.68 and 16 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 3x6 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 2, 603 lb uplift at joint 16 and 406 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061322

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T16	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 10:39:34 2006 Page 1

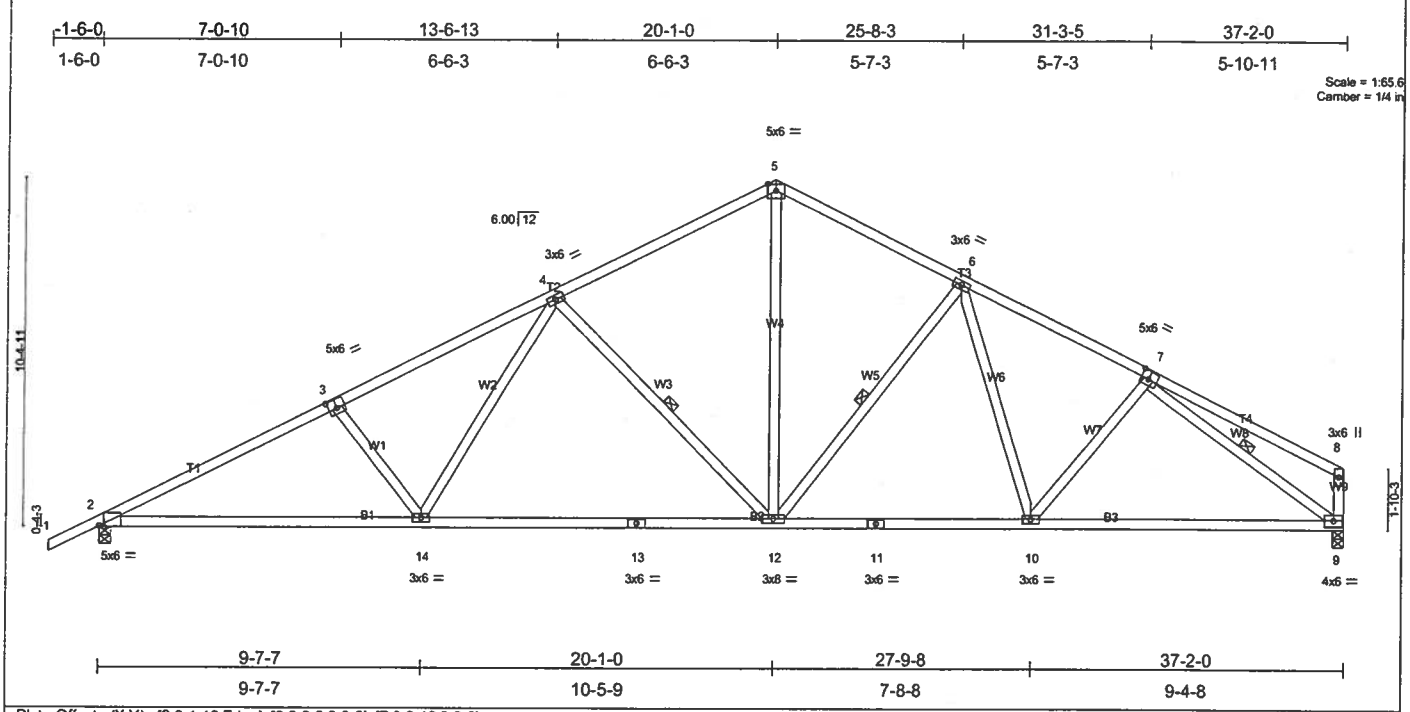


Plate Offsets (X,Y): [2:0-1-10,Edge], [3:0-3-0,0-3-0], [7:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.33	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.56	12-14	>790	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS 1 Row at midpt 4-12, 6-12, 7-9

REACTIONS (lb/size) 2=1640/0-4-0, 9=1546/0-4-0
Max Horz 2=239(load case 5)
Max Uplift2=-606(load case 5), 9=-490(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2834/917, 3-4=-2627/892, 4-5=-1689/631, 5-6=-1674/648, 6-7=-2001/695, 7-8=-351/140, 8-9=-286/161
BOT CHORD 2-14=-915/2468, 13-14=-640/1935, 12-13=-640/1935, 11-12=-434/1682, 10-11=-434/1682, 9-10=-505/1669
WEBS 3-14=-324/291, 4-14=-201/685, 4-12=-729/423, 5-12=-365/1104, 6-12=-456/298, 6-10=-61/224, 7-10=0/179, 7-9=-1803/575

JOINT STRESS INDEX
2 = 0.71, 3 = 0.62, 4 = 0.46, 5 = 0.40, 6 = 0.44, 7 = 0.66, 8 = 0.54, 9 = 0.75, 10 = 0.51, 11 = 0.61, 12 = 0.57, 13 = 0.96 and 14 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 606 lb uplift at joint 2 and 490 lb uplift at joint 9.

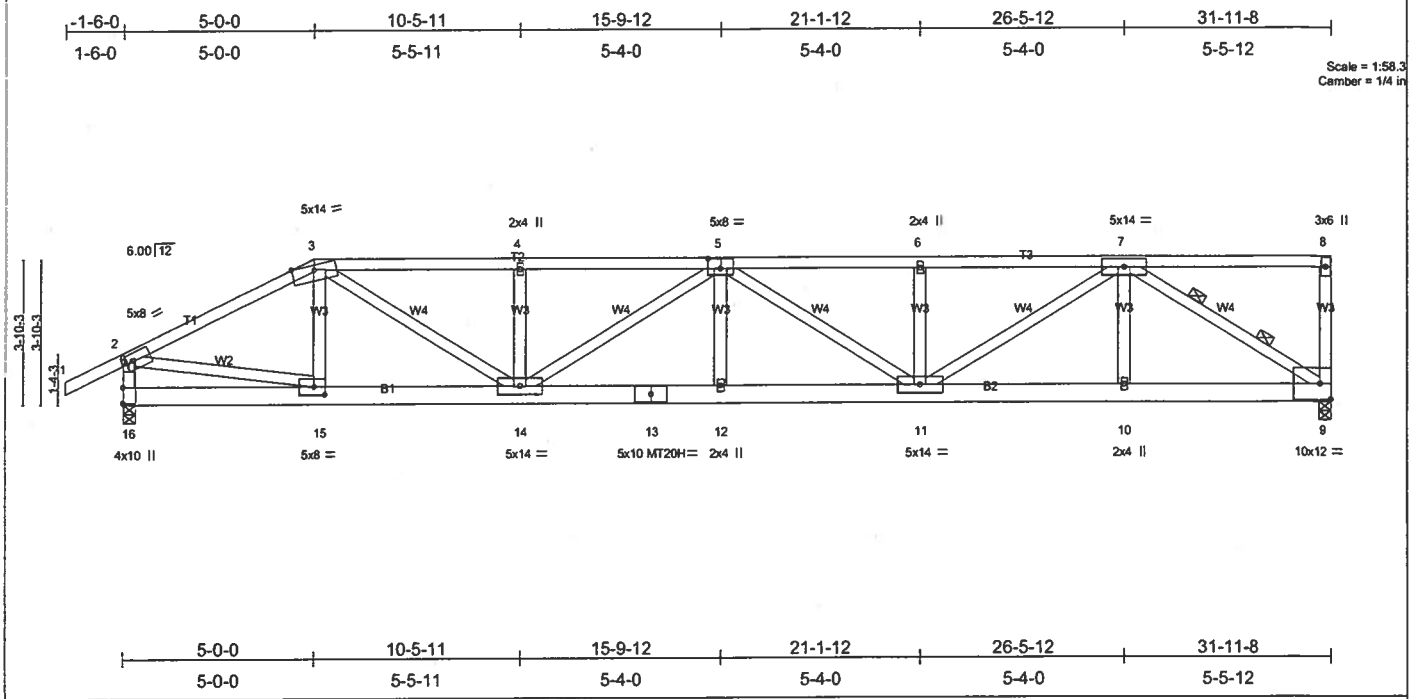
LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061323

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T17	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:33:55 2006 Page 1



5-0-0 | 10-5-11 | 15-9-12 | 21-1-12 | 26-5-12 | 31-11-8
5-0-0 | 5-5-11 | 5-4-0 | 5-4-0 | 5-4-0 | 5-5-12

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [5:0-4-0,0-3-0], [15:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	-0.43	11-12	>875	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.70	11-12	>547	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.89	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-9 oc bracing.
WEBS 2 Rows at 1/3 pts 7-9

REACTIONS (lb/size) 9=2903/0-4-0, 16=2854/0-4-0
Max Horz 16=190(load case 4)
Max Uplift9=-1312(load case 3), 16=-1179(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-4146/1819, 3-4=-5985/2712, 4-5=-5985/2712, 5-6=-6003/2714, 6-7=-6003/2714, 7-8=-113/66, 8-9=-300/237, 2-16=-2740/1167
BOT CHORD 15-16=-240/277, 14-15=-1655/3654, 13-14=-3064/6767, 12-13=-3064/6767, 11-12=-3064/6767, 10-11=-1725/3820, 9-10=-1725/3820
WEBS 3-15=-95/175, 3-14=-1275/2758, 4-14=-641/537, 5-14=-933/433, 5-12=0/379, 5-11=-912/417, 6-11=-610/498, 7-11=-1180/2604, 7-10=0/348, 7-9=-4423/1980, 2-15=-1538/3451

JOINT STRESS INDEX
2 = 0.96, 3 = 0.96, 4 = 0.34, 5 = 0.79, 6 = 0.34, 7 = 0.75, 8 = 0.64, 9 = 0.76, 10 = 0.34, 11 = 0.73, 12 = 0.34, 13 = 0.92, 14 = 0.77, 15 = 0.81 and 16 = 0.42

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1312 lb uplift at joint 9 and 1179 lb uplift at joint 16.
6) Girder carries hip end with 0-0-0 right side setback, 5-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 385 lb down and 198 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-8=-117(F=-63), 15-16=-30, 9-15=-65(F=-35)
Concentrated Loads (lb)
Vert: 15=-385(F)

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061324

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:30:12 2006 Page 1

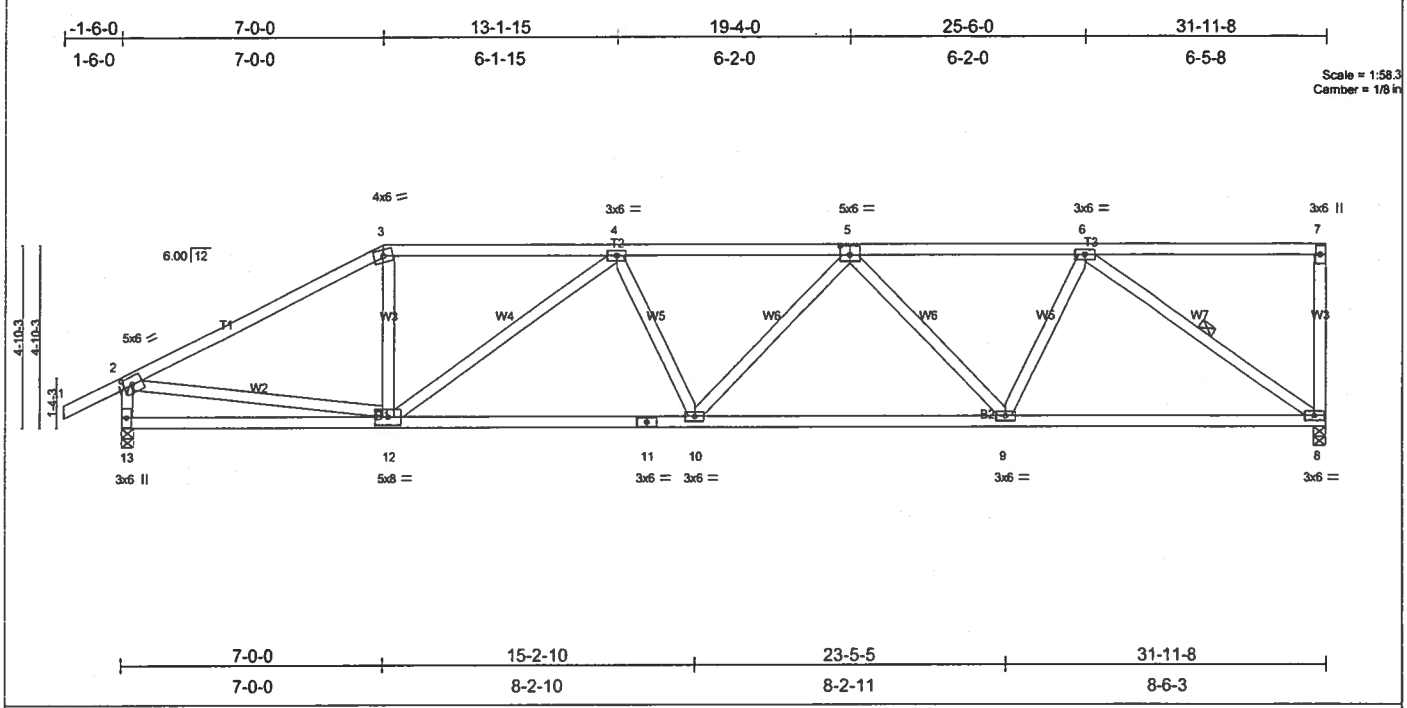


Plate Offsets (X,Y): [2:0-2-11,0-2-8], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.19	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.32	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-1 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1328/0-4-0, 13=1421/0-4-0
Max Horz 13=238(load case 5)
Max Uplift8=-498(load case 4), 13=-445(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1910/616, 3-4=-1646/592, 4-5=-2317/838, 5-6=-1819/653, 6-7=-61/14, 7-8=-164/109, 2-13=-1316/508
BOT CHORD 12-13=-307/263, 11-12=-854/2256, 10-11=-854/2256, 9-10=-849/2217, 8-9=-581/1505
WEBS 3-12=-102/466, 4-12=-765/370, 4-10=0/149, 5-10=-11/149, 5-9=-595/291, 6-9=-175/763, 6-8=-1780/699, 2-12=-457/1385

JOINT STRESS INDEX
2 = 0.75, 3 = 0.81, 4 = 0.47, 5 = 0.45, 6 = 0.65, 7 = 0.40, 8 = 0.71, 9 = 0.65, 10 = 0.47, 11 = 0.79, 12 = 0.63 and 13 = 0.48

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 8 and 445 lb uplift at joint 13.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061325

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:29:59 2006 Page 1

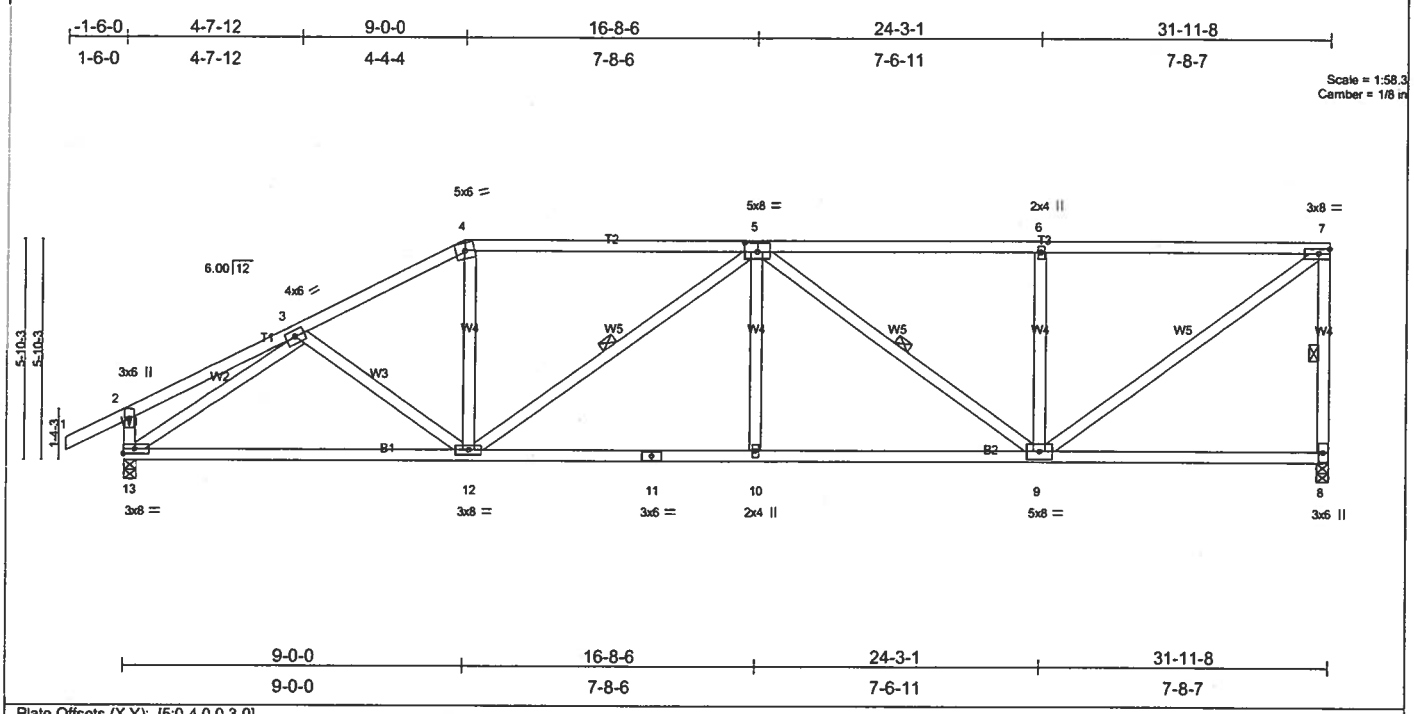


Plate Offsets (X,Y): [5:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.15	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.25	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 188 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-2 oc bracing.
WEBS 1 Row at midpt 7-8, 5-12, 5-9

REACTIONS (lb/size) 8=1328/0-4-0, 13=1421/0-4-0
Max Horz 13=284(load case 5)
Max Uplift8=-494(load case 4), 13=-459(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-363/102, 3-4=-1811/586, 4-5=-1598/561, 5-6=-1448/535, 6-7=-1448/535, 7-8=-1216/508, 2-13=-371/225
BOT CHORD 12-13=-584/1471, 11-12=-720/1955, 10-11=-720/1955, 9-10=-720/1955, 8-9=-21/54
WEBS 3-12=-94/141, 4-12=-53/430, 5-12=-443/244, 5-10=0/219, 5-9=-630/230, 6-9=-421/298, 7-9=-639/1730, 3-13=-1512/483

JOINT STRESS INDEX
2 = 0.32, 3 = 0.45, 4 = 0.66, 5 = 0.40, 6 = 0.34, 7 = 0.74, 8 = 0.41, 9 = 0.81, 10 = 0.34, 11 = 0.76, 12 = 0.57 and 13 = 0.72

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 494 lb uplift at joint 8 and 459 lb uplift at joint 13.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061326

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T20	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:29:40 2006 Page 1

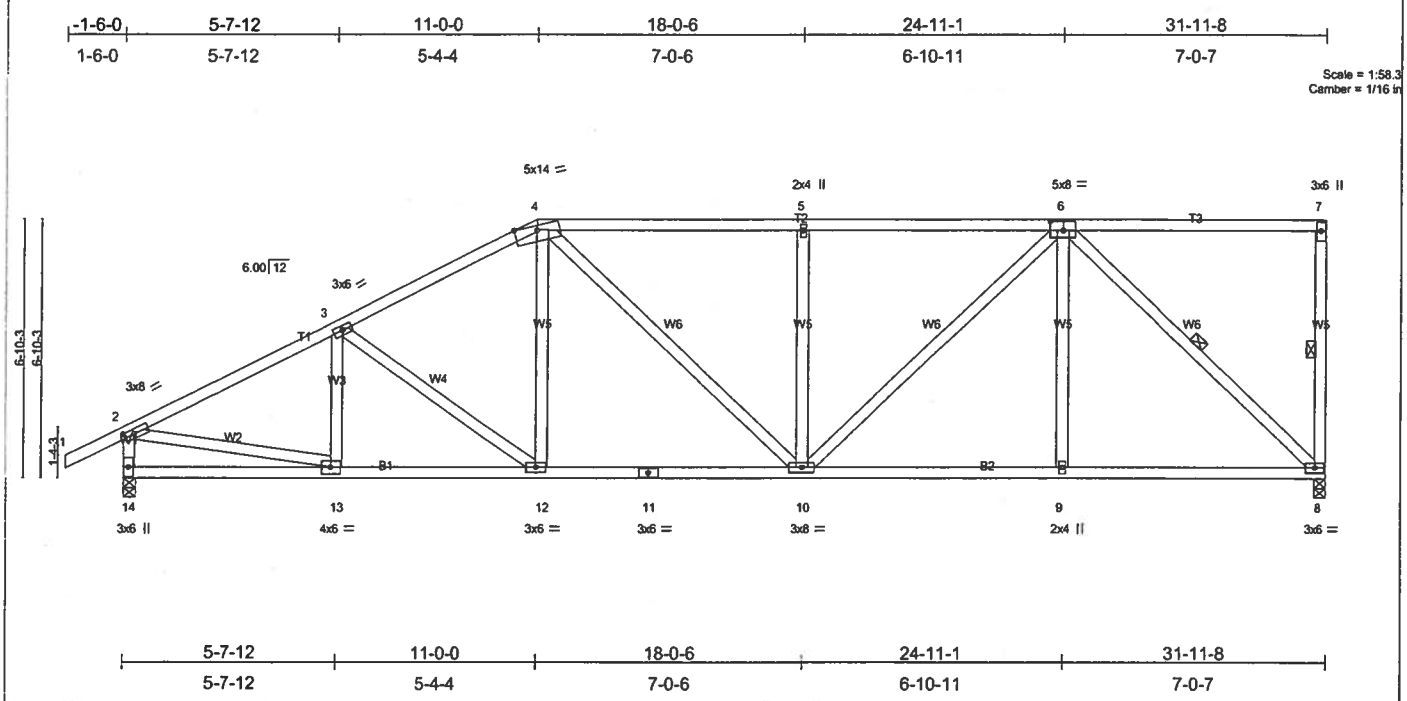


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.20	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-5 oc bracing.
WEBS 1 Row at midpt 7-8, 6-8

REACTIONS (lb/size) 8=1328/0-4-0, 14=1421/0-4-0
Max Horz 14=330(load case 5)
Max Uplift8=-488(load case 4), 14=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1845/513, 3-4=-1715/539, 4-5=-1625/586, 5-6=-1625/586, 6-7=-34/13, 7-8=-171/117, 2-14=-1327/483
BOT CHORD 13-14=-332/184, 12-13=-637/1585, 11-12=-516/1488, 10-11=-516/1488, 9-10=-418/1144, 8-9=-418/1144
WEBS 3-13=-140/101, 3-12=-126/175, 4-12=-64/266, 4-10=-156/186, 5-10=-376/270, 6-10=-232/663, 6-9=0/210, 6-8=-1532/559, 2-13=-327/1429

JOINT STRESS INDEX
2 = 0.97, 3 = 0.41, 4 = 0.91, 5 = 0.34, 6 = 0.45, 7 = 0.37, 8 = 0.57, 9 = 0.34, 10 = 0.65, 11 = 0.63, 12 = 0.35, 13 = 0.63 and 14 = 0.39

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 8 and 469 lb uplift at joint 14.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061327

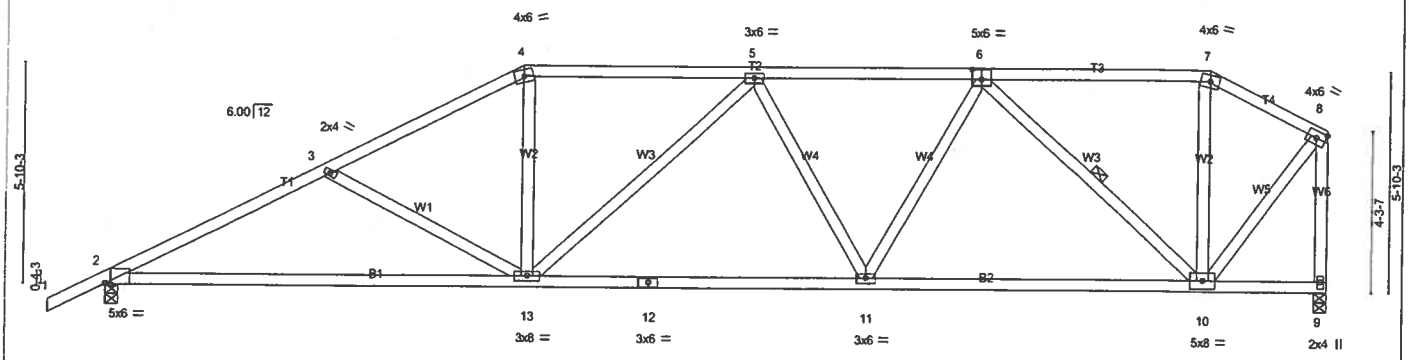
Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:29:02 2006 Page 1

-1-6-0 | 5-9-3 | 11-0-0 | 17-0-10 | 23-1-6 | 29-2-1 | 32-3-8

1-6-0 | 5-9-3 | 5-2-13 | 6-0-10 | 6-0-11 | 6-0-11 | 3-1-7

Scale = 1:58.0
Camber = 1/4 in



5-9-3 | 11-0-0 | 20-1-0 | 29-2-1 | 32-3-8

5-9-3 | 5-2-13 | 9-1-0 | 9-1-1 | 3-1-7

Plate Offsets (X,Y): [2:0-1-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.37	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.63	2-13	>605	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-10

REACTIONS (lb/size) 2=1435/0-4-0, 9=1341/0-4-0
Max Horz 2=254(load case 5)
Max Uplift2=-491(load case 5), 9=-396(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2383/700, 3-4=-2070/615, 4-5=-1807/585, 5-6=-1828/620, 6-7=-728/255, 7-8=-823/255, 8-9=-1346/392
BOT CHORD 2-13=-749/2082, 12-13=-670/1939, 11-12=-670/1939, 10-11=-557/1575, 9-10=-6/1
WEBS 3-13=-331/275, 4-13=-102/602, 5-13=-300/211, 5-11=-238/149, 6-11=-88/533, 6-10=-1175/458, 7-10=0/130, 8-10=-369/1188

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.65, 5 = 0.45, 6 = 0.43, 7 = 0.50, 8 = 0.66, 9 = 0.49, 10 = 0.58, 11 = 0.45, 12 = 0.69 and 13 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 491 lb uplift at joint 2 and 396 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061328

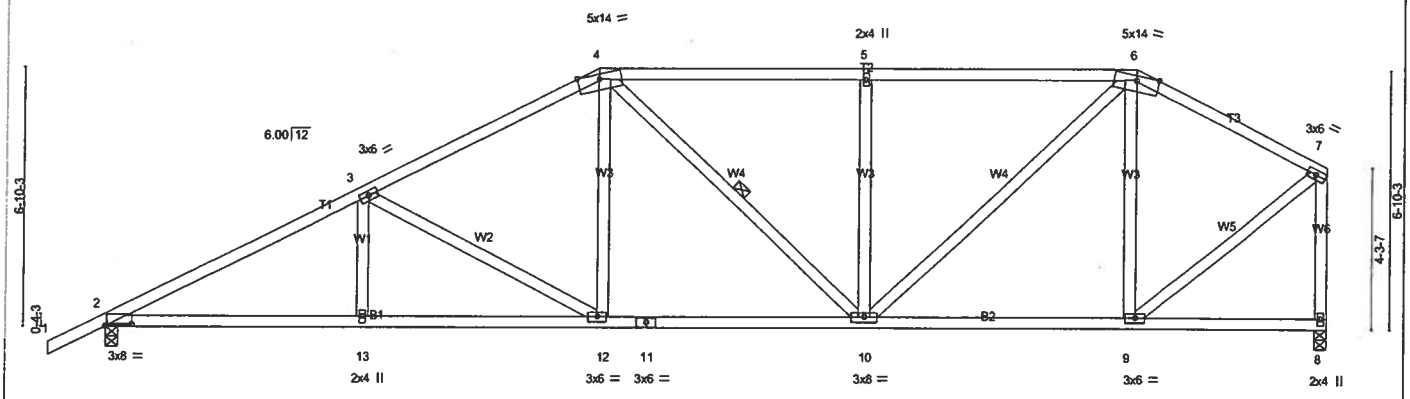
Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:29:17 2006 Page 1

-1-6-0 | 6-9-3 | 13-0-0 | 20-1-0 | 27-2-1 | 32-3-8

1-6-0 | 6-9-3 | 6-2-13 | 7-1-0 | 7-1-1 | 5-1-7

Scale = 1:58.0
Camber = 1/16 in



6-9-3 | 13-0-0 | 20-1-0 | 27-2-1 | 32-3-8

6-9-3 | 6-2-13 | 7-1-0 | 7-1-1 | 5-1-7

Plate Offsets (X,Y): [2:0-8-8,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.15	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.24	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-2-7 oc bracing.
WEBS 1 Row at midpt 4-10

REACTIONS (lb/size) 2=1435/0-4-0, 8=1341/0-4-0
Max Horz 2=268(load case 5)
Max Uplift2=-506(load case 5), 8=-349(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2468/700, 3-4=-1871/563, 4-5=-1589/544, 5-6=-1589/544, 6-7=-1075/336, 7-8=-1274/396
BOT CHORD 2-13=-754/2124, 12-13=-754/2124, 11-12=-480/1616, 10-11=-480/1616, 9-10=-253/905, 8-9=-17/26
WEBS 3-13=0/215, 3-12=-590/314, 4-12=-109/475, 4-10=-117/128, 5-10=-406/288, 6-10=-333/964, 6-9=-517/245, 7-9=-334/1135

JOINT STRESS INDEX
2 = 0.78, 3 = 0.41, 4 = 0.93, 5 = 0.34, 6 = 0.86, 7 = 0.80, 8 = 0.63, 9 = 0.68, 10 = 0.95, 11 = 0.54, 12 = 0.35 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 506 lb uplift at joint 2 and 349 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061329

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:31:06 2006 Page 1

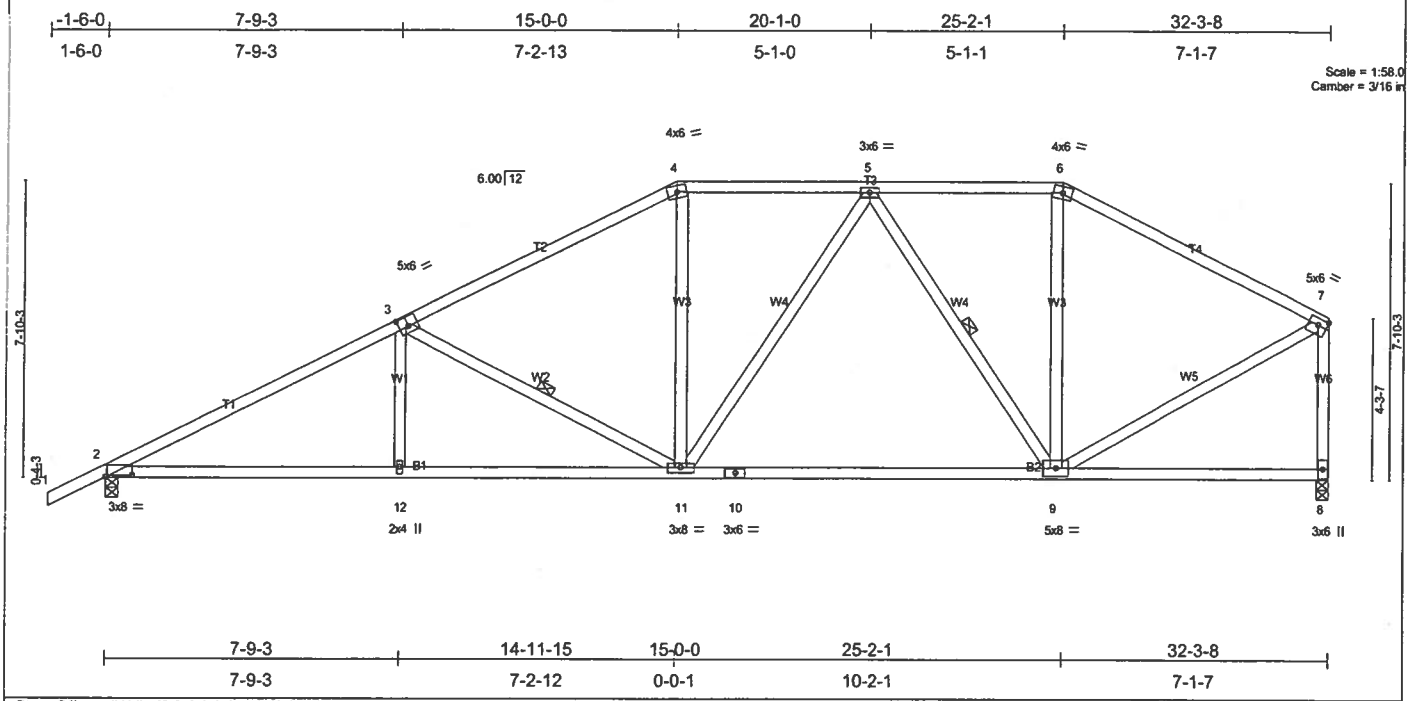


Plate Offsets (X,Y): [2:0-8-8,0-0-10], [3:0-3-0,0-3-0], [7:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.25	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.43	9-11	>898	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing.
WEBS 1 Row at midpt 3-11, 5-9

REACTIONS (lb/size) 2=1435/0-4-0, 8=1341/0-4-0
Max Horz 2=282(load case 5)
Max Uplift2=-519(load case 5), 8=-368(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2421/715, 3-4=-1718/536, 4-5=-1460/546, 5-6=-1034/389, 6-7=-1235/376, 7-8=-1251/389
BOT CHORD 2-12=-771/2078, 11-12=-771/2078, 10-11=-369/1320, 9-10=-369/1320, 8-9=-40/63
WEBS 3-12=0/239, 3-11=-712/383, 4-11=-36/385, 5-11=-125/282, 5-9=-591/243, 6-9=-13/267, 7-9=-270/1114

JOINT STRESS INDEX
2 = 0.76, 3 = 0.79, 4 = 0.78, 5 = 0.43, 6 = 0.81, 7 = 0.80, 8 = 0.32, 9 = 0.51, 10 = 0.48, 11 = 0.58 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 519 lb uplift at joint 2 and 368 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061330

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T24	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:31:38 2006 Page 1

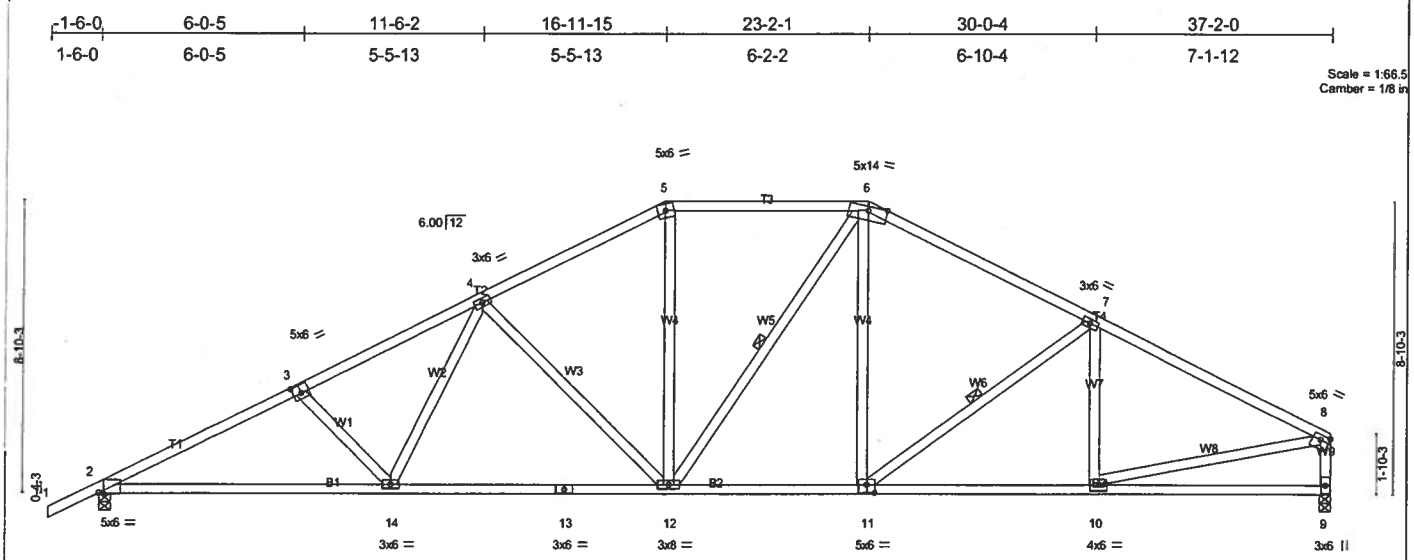


Plate Offsets (X,Y): [2:0-1-10,Edge], [3:0-3-0,0-3-0], [8:Edge,0-1-12], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.22	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.37	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-11 oc bracing.
WEBS 1 Row at midpt 6-12, 7-11

REACTIONS (lb/size) 2=1640/0-4-0, 9=1546/0-4-0
Max Horz 2=217(load case 5)
Max Uplift2=-589(load case 5), 9=-471(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2871/899, 3-4=-2668/853, 4-5=-1937/659, 5-6=-1684/642, 6-7=-1869/613, 7-8=-2070/628, 8-9=-1436/485
BOT CHORD 2-14=-888/2505, 13-14=-664/2099, 12-13=-664/2099, 11-12=-356/1596, 10-11=-472/1776, 9-10=-103/220
WEBS 3-14=-272/243, 4-14=-138/530, 4-12=-609/353, 5-12=-147/526, 6-12=-110/300, 6-11=-102/346, 7-11=-270/226, 7-10=-154/136, 8-10=-379/1596

JOINT STRESS INDEX
2 = 0.72, 3 = 0.47, 4 = 0.41, 5 = 0.55, 6 = 1.00, 7 = 0.41, 8 = 0.72, 9 = 0.50, 10 = 0.74, 11 = 0.58, 12 = 0.58, 13 = 0.89 and 14 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 589 lb uplift at joint 2 and 471 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1031061331

Job	Truss	Truss Type	Qty	Ply	0 0
L215867	T25	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 30 11:32:13 2006 Page 1

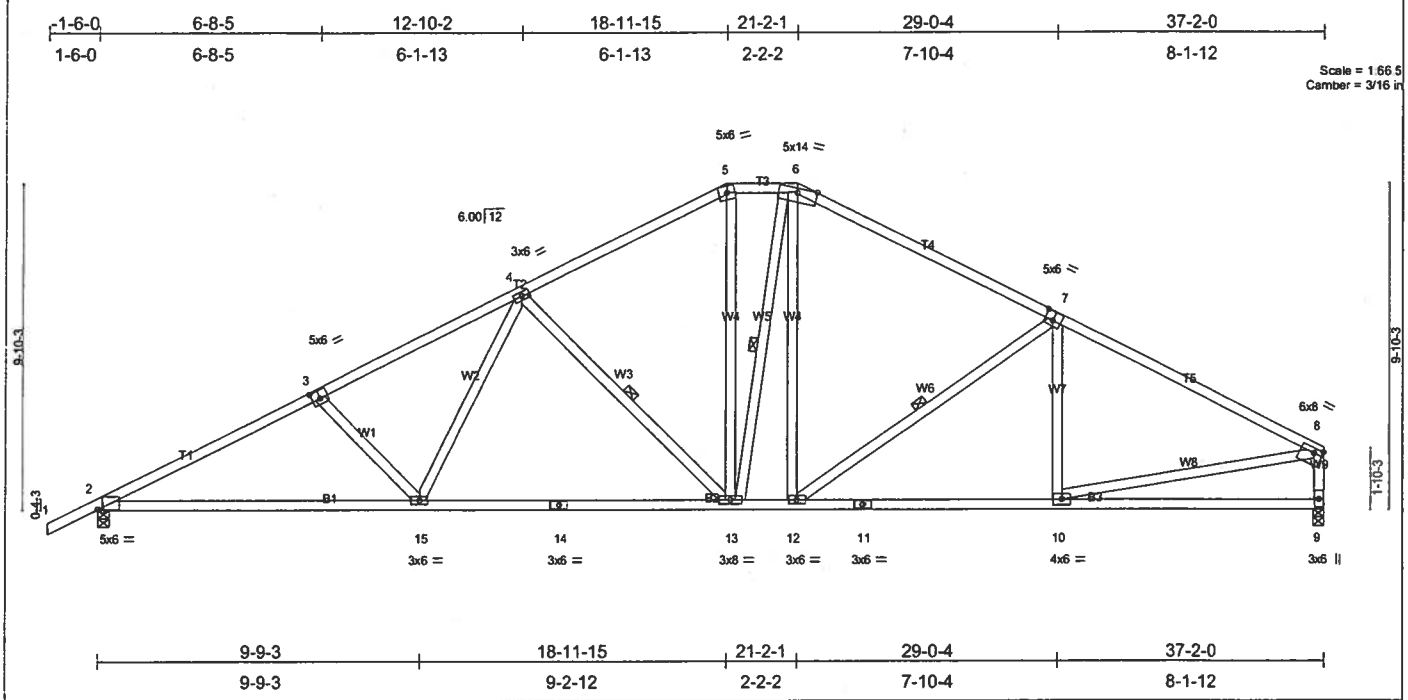


Plate Offsets (X,Y): [2:0-1-10,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-4], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.27	2-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.46	2-15	>967	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 226 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-14 oc bracing.
WEBS 1 Row at midpt 4-13, 6-13, 7-12

REACTIONS (lb/size) 2=1640/0-4-0, 9=1546/0-4-0
Max Horz 2=231(load case 5)
Max Uplift2=-600(load case 5), 9=-484(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2833/914, 3-4=-2603/860, 4-5=-1766/635, 5-6=-1518/627, 6-7=-1764/625, 7-8=-2110/655, 8-9=-1423/501
BOT CHORD 2-15=-909/2470, 14-15=-649/2000, 13-14=-649/2000, 12-13=-350/1492, 11-12=-484/1802, 10-11=-484/1802, 9-10=-138/277
WEBS 3-15=-316/278, 4-15=-161/606, 4-13=-704/401, 5-13=-233/568, 6-13=-117/332, 6-12=-148/369, 7-12=-442/290, 7-10=-65/132, 8-10=-381/1555

JOINT STRESS INDEX
2 = 0.71, 3 = 0.55, 4 = 0.42, 5 = 0.43, 6 = 0.99, 7 = 0.75, 8 = 0.78, 9 = 0.60, 10 = 0.70, 11 = 0.73, 12 = 0.35, 13 = 0.60, 14 = 0.90 and 15 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 600 lb uplift at joint 2 and 484 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
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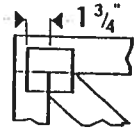
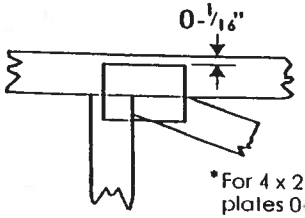
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

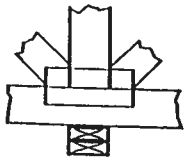
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

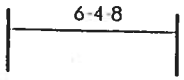


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

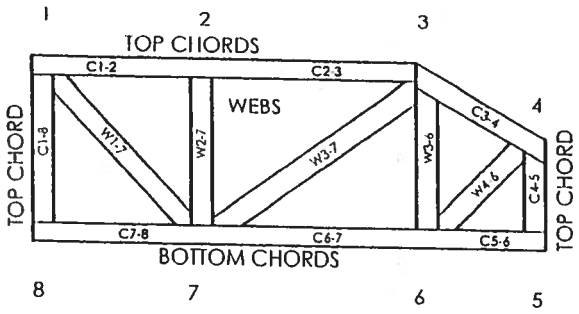
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

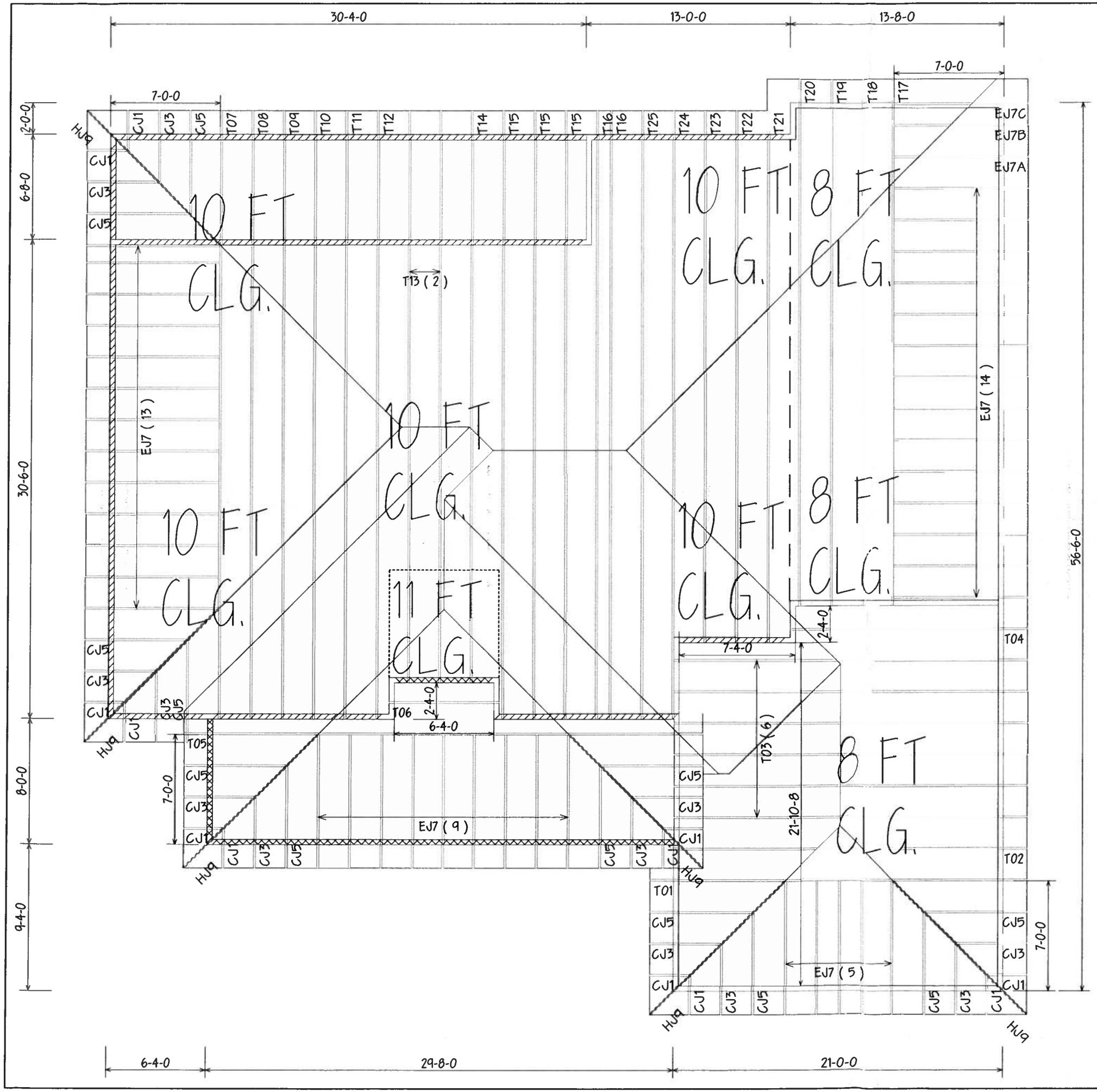
TEE-LOK®

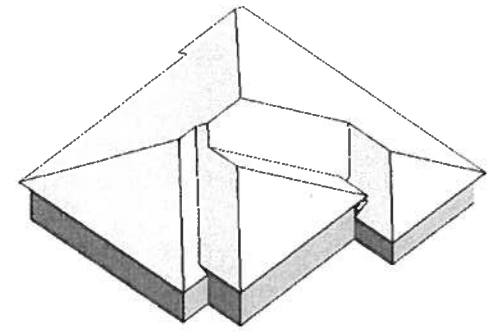
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.





BEARING HEIGHT SCHEDULE

- 8 FT
- 10 FT
- 11 FT

6/12 PITCH
18"
O.H.

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904 772 1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407 322 5553

BUILDER: HUGO ESCALANTE

LEGAL ADDRESS: KARCHER RESIDENCE

MODEL: CUSTOM

REVISION:

SCALE: NTS

DATE: 10/30/06

DRAWN BY: JOE

JOB #: L215867