BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 20 / 7TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED, ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

Education of the second	
OCCUPANCY/RISK CATEGORY	II - Normal le1.00
WIND LOAD ULTIMATE	117 MPH NOMINAL 90.63 MPH WIND EXPOSURE C
CLOSURE TYPE	Enclosed
INTERNAL WIND COEFFICIENT	<u>-0.18 / 0.18</u>
COLLATERAL DEAD LOAD	PSF
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes)
DEAD LOAD (BLDG "A")	2.400 PSF (FOR ROOF PANELS AND PURLINS)
DEAD LOAD (BLDG "B")	2.300 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	
SPECTRAL RESPONSE Ss _0.0	913 S1_0.0529 Sds_0.0971 Sd1_0.0832
SITE CLASS d	DESIGN RISK CATEGORY B Cs 0.0324
RESPONSE MODIFICATION FACTOR, I	R <u>3.000*</u> FRAMES <u>3.000*</u> BRACING
BASIC SEISMIC FORCE RESISTING ST	YSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SERVICEABILITY CRITERIA

BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) (BLDG "A") = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES

			SLISHIC KESISTANCE
	MINIMUM DESIGN	N DEFLECTIONS	
Endwall Column	= 120	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 60
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) (BLDG "B") = ORDINARY STEEL MOMENT FRAMES

GENERAL NOTES

A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS-ALLOWABLE STRESS DESIGN"

2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL

3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.

4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"

C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER TRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO A529 OR A572. FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD

POINT OF 50,000 psi. WEB MATERIAL CONFORMS TO ASTM-A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.

2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD

POINT OF 42,000 psi,
3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM—A500, CRADE B WITH A MINIMUM YIELD

POINT OF 46,000 psi.
4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITHMINIMUM YIELD POINT OF 50,000 PSI, HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.

5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.

6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM—A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM—ZINC ALLOY

IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT

8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUC-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:

a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC—BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".

b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.

c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS - REVERSALS ON THE CONNECTIONS. d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 - SC".

10) SECONDARY MEMBERS AND FLANCE BRACE CONNECTIONS SHALL ALWAYS BE SNUC TIGHT, UND.

11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.

ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:

1) BE MADE IN CONTRASTING INK.

2) HAVE_ALL INSTANCES OF CHANGE CLEARLY INDICATED.

3) BE LEGIBLE AND UNAMBIGUOUS.

B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.

C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION, THIS MAY IMPACT THE DELIVERY SCHEDULE.

D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED

F) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED, HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL DISAPPROVAL ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.

IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE,

ERECTOR / CONTRACTOR RESPONSIBILITIES

A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

REQUIRED.

OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, 13TH ED.)

D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 13TH ED.)
E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

OF PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR
IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE
MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE
THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS
AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS
CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.

IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.

THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

1) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 13TH ED.)

J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)

K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.



BUILDINGS AND MORE

FLORIDA PRODUCT APPROVAL NUMBER

16" CENTRAL SPAN 14016.2 PBR WALL PANEL 36876.1

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NO PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT

	FRAM	ING	COLO	RS		
Rigid Frame: RO Flange brace: RO Angle: RO	= G		d Oxide ey Prim Ivanized	er		
Grt	Pur	EvSt	Jmb	88	End Col	wall Raf
U SECTION: RO	RO	RO	RO	RO:	RO	RO
C SECTION: RO	RO	RO	RO	RO	RO	RO
D SECTION: RO	RO	RO	RO	RO	RO	RO
Z SECTION: RO	RO	RO	RO	RO	RO	RO
E SECTION: RO	RO	RO	RO	RO	RO	RO
R SECTION: RO	RO	RO	RO	RO	RO	RO
W SECTION: RO	RO	FRO I	RO	RO T	RO I	RO.

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED



BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY

WALLS:

CABLE:

EAVE:

CORNER

CUTTER

DOWNSPOUTS

BURNISHED SLATE

	U	RAWING INDEX					
REV.	PAGE	DESCRIPTION					
	0	COVER PAGE					
	1	ANCHOR BOLT LAYOUT					
	1.1	ANCHOR BOLT DETAILS					
	1.2	ANCHOR BOLT REACTIONS					
	2	ROOF FRAMING LAYOUT					
	2.1-2.3	RIGID FRAME CROSS SECTION					
	3-3.2	ENDWALL FRAMING LAYOUT					
	4-4.3	SIDEWALL FRAMING LAYOUT					
	5-5.5	FRAMING DETAILS					
	6-6.1	ROOF PANELS & TRIM					
	6.2	ROOF PANEL DETAILS					
	7	SIDEWALL PANEL DETAILS					
	8	ENDWALL PANEL DETAILS					
	9	SPECIAL DETAILS					

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF

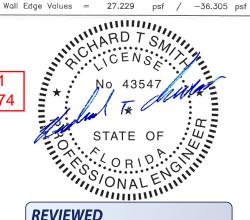
FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS.

1.0 PSF COLL ONLY ALLOW LIGHTING AND HVAC DUCT TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICILLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE.

	Ton
	COMPONENTS & CLADDING (unfactored)
$\langle A \rangle$	Wall Field Values = 27.229 psf / −29.498 psf
	Wall Edge Values = 27.229 psf $/ -36.305$ psf
	COMPONENTS & CLADDING (unfactored)
$\langle H \rangle$	Wall Field Values = 27.229 osf $/ -29.498$ osf

PE # 43547 Ph-706-888-4874

l510 Lee Rd 281 Salem AL, 36874



REVIEWED By Richard T Smith at 4:00 pm, Apr 26, 2023	FROM BUILD 792 \$	LAKE
DRAWING STATUS	JOB NO : 803	9
FOR APPROVAL; THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION, NOT, FINAL, AND ARE FOR CONCEPTUAL	DATE:	24/23
REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.	BY: DAR	scale : NONE
FOR PERMIT: THESE DRAWINGS. BEING FOR PERMIT, ARE BY DEFINITION	TITLE : COVER	PAGE
NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED, ONLY DRAWINGS ISSUED FOR CONSTRUCTION "CAN BE CONSIDERED AS COMPLETE. FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION	PAGE	Ξ Ο

COLORS: CALVALUME LIGHT STONE BURNISHED SLATE BURNISHED SLATE BURNISHED SLATE FRAMED OPENINGS: BURNISHED SLATE BURNISHED SLATE LIGHT STONE

DR. NORRIS 32025 MORE BASCOM AND NS.

32055

CITY,

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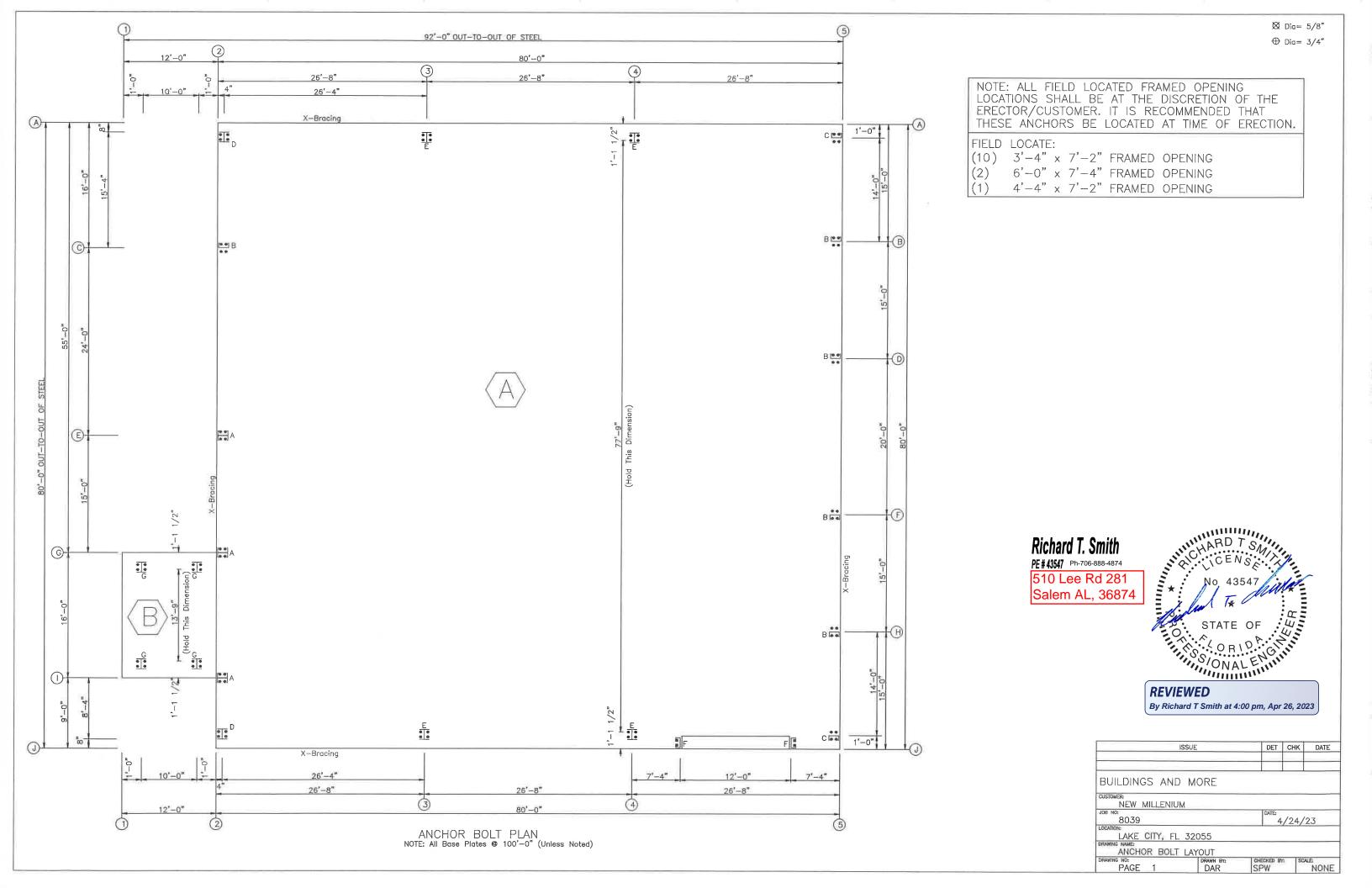
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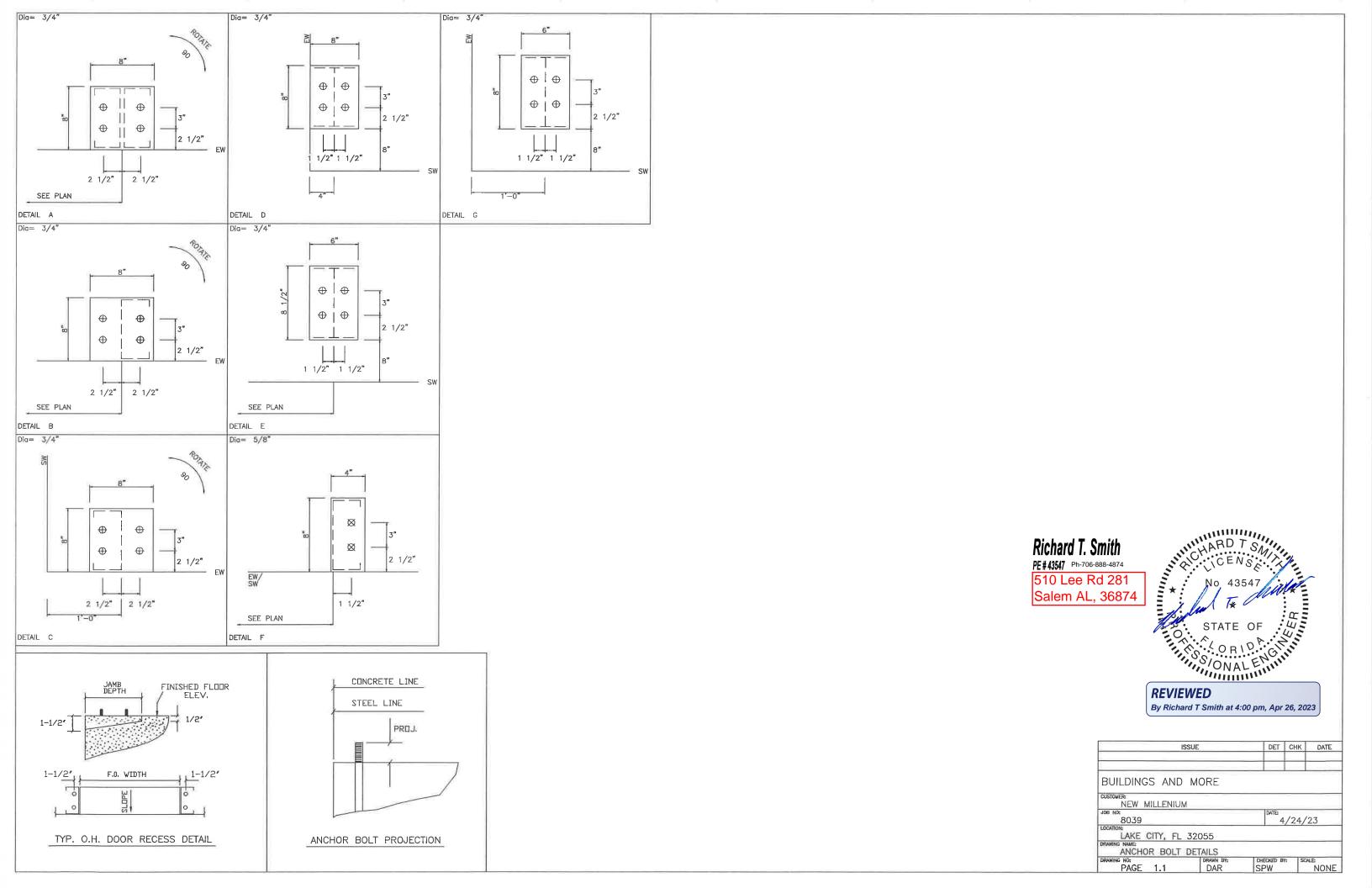
NEW

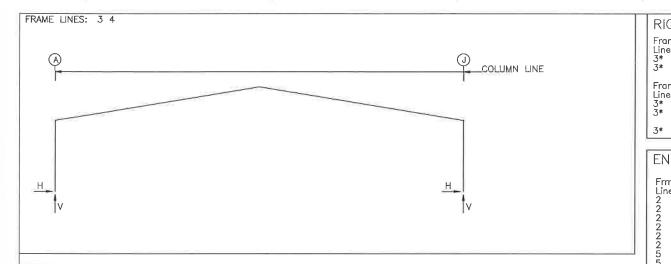
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CITY,







RIGID FRAME: ANCHOR BOLTS & BASE PLATES

	Col Line	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (іп)
3* 3*	A		6.000 6.000		0.500 0.500	0.0

3* Frame lines: 3 4

END	WALL	COI	_UMN:	ANCI	HOR BOL	TS & B	ASE PLATES
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base Width	_Plate (i Length	in) Thick	Grout (in)
2	Α	4	0.750	8.000	8.000	0.375	0.0
2	С	4	0.750	8.000	8.000	0.250	0.0
2	Ę	4	0.750	8.000	8.000	0.375	0.0
2	G	4	0.750	8.000	8.000	0.375	0.0
2	1	4	0.750	8.000	8.000	0.250	0.0
2	J	4	0.750	8.000	8.000	0.375	0.0
5	J	4	0.750	8.000	8.000	0.250	0.0
5	Н	4	0.750	8.000	8.000	0.250	0.0
5	F	4	0.750	8.000	8.000	0.250	0.0
5	D	4	0.750	8.000	8.000	0.250	0.0
5	В	4	0.750	8.000	8.000	0.250	0.0
5	Α	4	0.750	8.000	8.000	0.250	0.0

ANCH	OK BO	_ 50	MMAR	Υ
Qty	Locate Jamb Endwall Frame	Dia (in) 5/8" 3/4"	Type A307 GR36 GR36	Projection (in) 1.50 1.50 2.50
Ψ 10	1 (dille	3/4	GNOU	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUIL	DINC	BRA	CINC	RE	ACTIO	SNC		
	ıll — Line	Col Line	——W Horz	React nd — Vert	ions(k —Sei Horz) smic – Vert	Panel (lb) Wind	Shear ft) Seis
L_EW F_SW R_EW B_SW	2 J 5 A	E,G 2,3 H,F 3,2	2.4 6.4 2.4 6.4	2.9 2.9 2.7 2.9	0.2 0.6 0.2 0.6			

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

IGIE	FRAN	ЛЕ: E	BASIC COL	UMN REA	CTIONS (k)								
ame ne	Column Line A J	Horiz 3.1 -3.1	-Dead Vert 3.7 3.7	Colla Horiz 1.0 -1.0	teral— Vert 1.1 1.1	 Horiz 11.7 -11.7	-Live Vert 12.8 12.8	Wind Horiz -19.7 12.2	_Left1- Vert -21.5 -16.5	-Wind_ Horiz -12.2 19.7	Right1- Vert -16.5 -21.5	Wind. Horiz -13.3 5.8	_Left2- Vert -11.8 -6.8	
ame ne	Column Line A J	-Wind_ Horiz -5.8 13.3	Right2- Vert -6.8 -11.8	Wind Horiz -13.0 14.4	Long1- Vert -21.0 -17.2	Wind Hariz -14.4 13.0	Long2- Vert -17.2 -21.0	-Seism Horiz -0.2 -0.2	ic_Left Vert -0.1 0.1	Seismic Horiz 0.2 0.2	_Right Vert 0.1 -0.1			
:	Frame lin	es:	3 4											
NDV	VALL (COLUM	1N: BA	ASIC COLL	MN REAC	CTIONS (k)							

		_											
END	WAL	L COL	UMN:	BASIC (EACTIONS	. ,						
Frm Line 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	Cole CEGLJJHFDBA	Dead Vert 0.3 1.0 1.1 0.7 0.7 0.2 0.3 0.7 0.8 0.8 0.7	Collat Vert 0.1 0.3 0.2 0.2 0.0 0.1 0.2 0.3 0.3 0.2	Live Vert 1.4 6.1 5.7 3.7 3.9 0.7 1.7 4.2 4.9 4.2	Wind Left1 Vert -1.9 -8.9 -6.1 -2.6 -3.7 -0.9 -6.5 -5.8 -3.7 -4.0 -1.8	Wind Right1 Vert -1.6 -5.7 -5.7 -5.6 -0.8 -3.4 -4.2 -5.9 -6.4 -2.2	Wind Left2 Vert -1.0 -6.0 -0.9 -1.9 -0.4 -1.2 -4.4 -3.9 -1.8 -1.9	Wind Right2 Vert -0.8 -2.7 -4.0 -3.8 -0.9 -1.4 -2.3 -4.0 -4.3 -1.2	Wind Press Horz -1.8 -3.2 -4.0 -2.8 -1.8 -1.1 -2.4 -3.3 -2.4 -1.1	Wind Suct Horz 2.0 3.5 4.4 3.0 2.0 2.0 1.2 2.6 3.7 2.6 1.2	Wind Long1 Vert -2.3 -9.0 -6.0 -2.6 -3.5 -0.8 -6.2 -6.1 -4.0 -3.6 -1.7	Wind Long 2 Vert -1.5 -5.2 -5.8 -5.5 -1.3 -3.7 -3.9 -6.4 -2.7	Seis Left Vert 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Frm Line 2222255555555555555555555555555555555	Ole Cin CEGLJJHFDBA	Seis Right Vert 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Seis Long Vert 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0										

Building reactions are based on the following building data: Width (ft) = 80.0 Length (ft) = 80.0 Eave Height (ft) = 14.0/ 14.0 Roof Slope (Rise/12) = 2.0/ 2.0 Dead Load (psf) = 2.4 Collateral Load (psf) = 1.0 Roof Live Load(psf) = 12.0 Wind Speed (mph) = 117.0 Wind Code = FBC 20/ 7TH EDITION Exposure = C Enclosed/Open/Partial | ENCLOSED | Importance Wind = 1.00 Seismic Zone = B Seismic Coeff (Fa*Ss) = 0.15

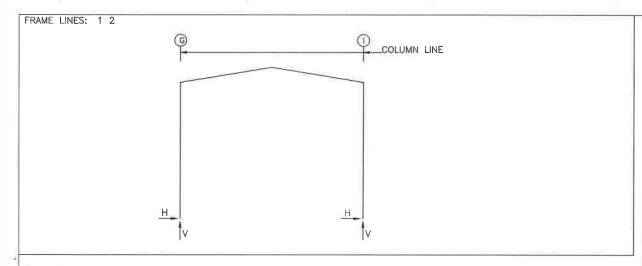
Richard T. SmithPE # 43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874



REVIEWED

BUILDING "A"				
IS	SUE	DET	CHK	DATE
				_
BUILDINGS AND	MORE			
USTOMER: NEW MILLENIUM				
08 NO: 8039		DATE:	/24/	23
LAKE CITY, FL	32055			
RAWING NAME: ANCHOR BOLT	REACTIONS			
RAWING NO: PAGE 1.2	DRAWN BY: DAR	SPW	: SC/	NONE



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Frm Col Line Line		_Bolt Dia	Base. Width	Grout (in)		
1	G I	4		6.000 6.000		0.375 0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col		_Bolt	Base	Grout		
Line	Line		Dia	Width	(in)		
2 2	G I	4 4	0.750 0.750	6.000 6.000		0.375 0.375	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Туре	Projection (in)
0	16	Frame	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

RIGID	FRAN	ЛЕ: E	BASIC COL	UMN REA	CTIONS (k)								
Frame Line 1 1 2 2	Column Line G I G	Horiz 0.0 0.0 0.0 0.0	-Dead Vert 0.3 0.3 0.3 0.3	-—-Colla Horiz 0.0 0.0 0.0 0.0	teral— Vert 0.0 0.0 0.0	Horiz 0.1 -0.1 0.1 -0.1	Live Vert 1.0 1.0 1.0	Wind Horiz -1.0 -0.9 -1.0 -0.9	Left1- Vert -2.0 -0.2 -2.0 -0.2	-Wind_ Horiz 0.9 1.0 0.9 1.0	Right1- Vert -0.2 -2.0 -0.2 -2.0	Wind Horiz -1.2 -0.6 -1.2 -0.6	_Left2- Vert -1.6 0.2 -1.6 0.2	
Frame Line 1 1 2 2	Column Line G I G	-Wind_ Horiz 0.6 1.2 0.6 1.2	Right2- Vert 0.2 -1.6 0.2 -1.6	Wind Horiz 0.4 -0.4 0.4 -0.4	_Long1- Vert -0.9 -0.8 -0.9 -0.8	Wind Horiz 0.4 -0.4 0.4 -0.4	_Long2- Vert -0.8 -0.9 -0.8 -0.9							

NOTES FOR REACTION	NS
Building reactions are based the following building data:	on
Width (ft)	= 16.0 = 12.0
Length (ft) Eave Height (ft)	= 12.0
Roof Slope (Rise/12)	= 2.0/2.0
Dead Load (psf) Collateral Load (psf)	= 12.0/ 12.0 = 2.0/ 2.0 = 2.3 = 1.0 = 20.0
Live Load (psf) Wind Speed (mph)	= 20.0 = 117.0
Wind Code	= FBC 20/ 7TH EDITION
Exposure Enclosed/Open/Partial	= C = ENCLOSED
Importance Wind	= 1.00
Importance Seismic Seismic Zone	= 1.00 = B
Seismic Coeff (Fa*Ss)	= 0.15

BUILDING BRACING REACTIONS	
Wall _ Col ± Reactions(k) Panel_Shear _ Line	Note
L_EW 1 F_SW I BLDG-A TIE-IN	(h)
R_EW 2 B_SW G BLDG-A TIE-IN	(h)
(h)Rigid frame at endwall	

Richard T. Smith
PE#43547 Ph-706-888-4874
510 Lee Rd 281
Salem AL, 36874

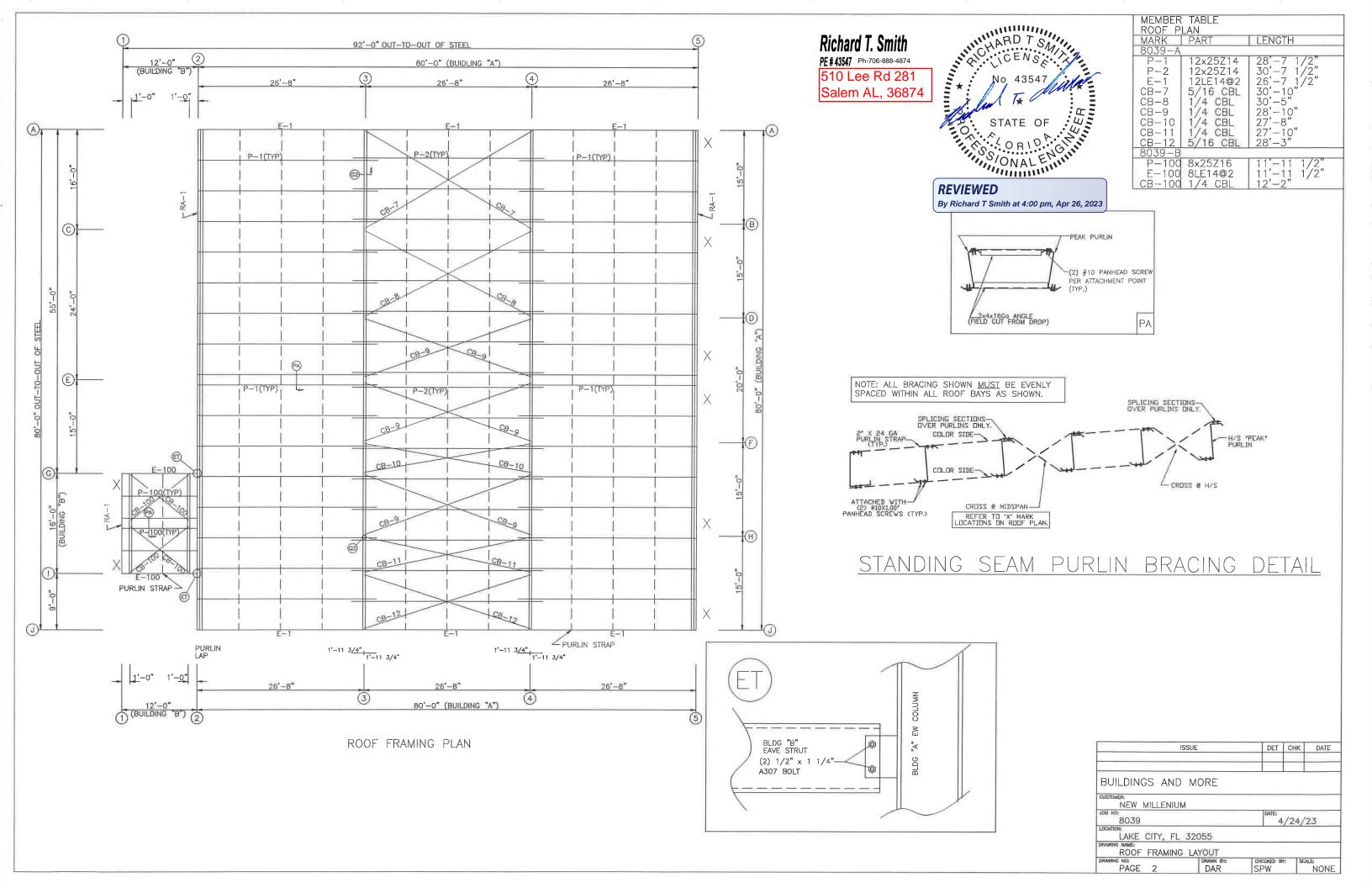


REVIEWED

By Richard T Smith at 4:00 pm, Apr 26, 2023

SUE	DET	СНК	DATE
		-	
MORE			
	DATE:	/24/:	23
32055			
REACTIONS			
DRAWN BY: DAR	SPW	SCA	iE: NONE
	MORE 32055 REACTIONS DRAWN BY:	MORE DATE: 4/ 32055 REACTIONS DRAWN BY: CHECKED BY:	MORE DATE 4/24/3 32055 REACTIONS DREWED BY: SOF

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

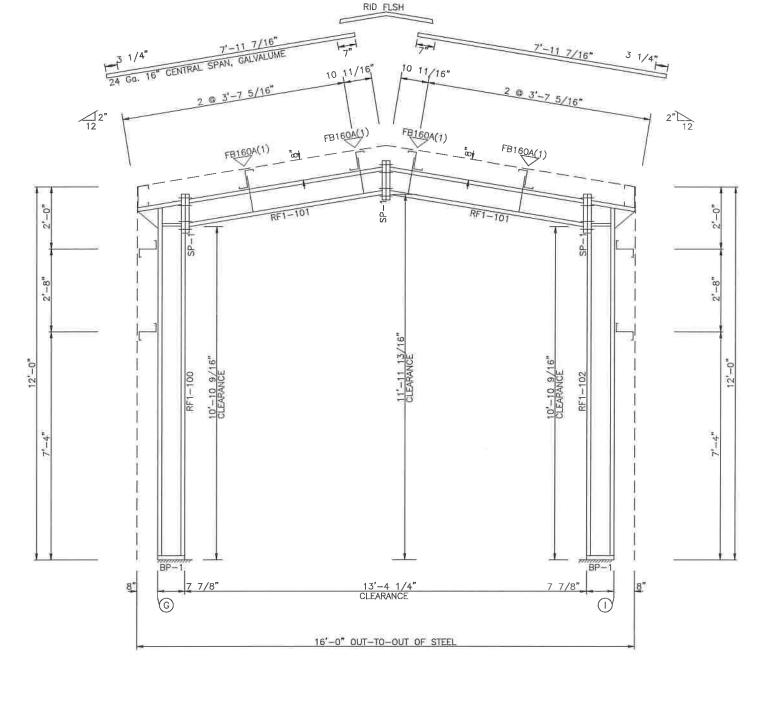


SPLICE BOLT TABLE | Qty | Top | Bot | Int | TYPE | DIA | Length | 4 | 4 | 0 | A325 | 5/8" | 2"

COL PLATE SIZE MARK Width THICK Length BP-1 6" 3/8" 8"

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MEMBER SIZE TABLE MEMBER LENGTH WEIGHT MARK RF1-100 RF1-101 RF1-102 W8X10 W8X10 W8X10 11'-5 1/4" 6'-9 1/16" 11'-5 1/4" 148 92 148



RIGID FRAME ELEVATION: FRAME LINE 1

Richard T. Smith

PE # 43547 Ph-706-888-4874 510 Lee Rd 281 Salem AL, 36874 ORIDA GINE

REVIEWED

By Richard T Smith at 4:00 pm, Apr 26, 2023

BUILDING "B"				
IS	SUE	DET	СНК	DATE
BUILDINGS AND	MORE			
NEW MILLENIUM		77		
08 NO: 8039		DATE:	/24/.	23
LAKE CITY, FL	32055			
RIGID FRAME CF	ROSS SECTIO	N		
PAGE 2.1	DRAWN BT: DAR	SPW SPW	SCA	NONE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

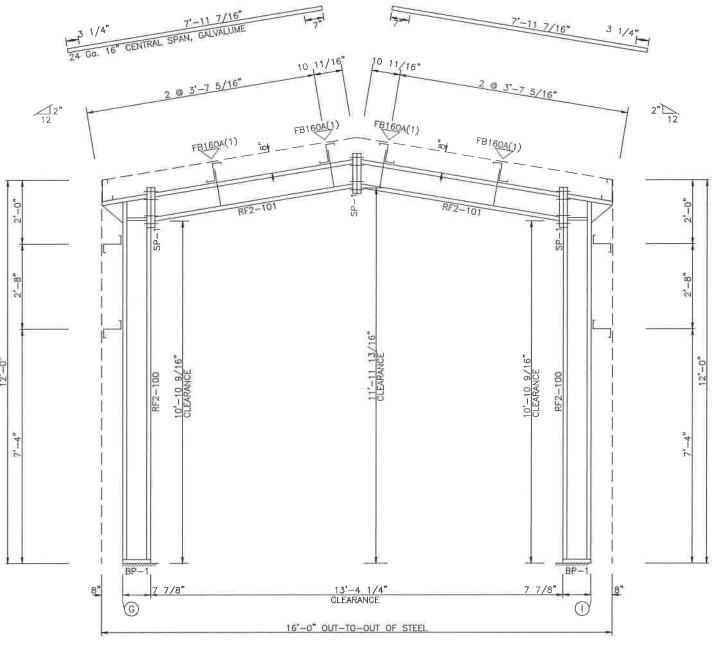
FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

 MEMBER
 SIZE
 TABLE

 MARK
 MEMBER
 LENGTH
 WEIGHT

 RF2-100
 W8X10
 11'-5 1/4"
 138

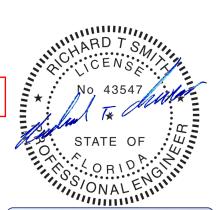
 RF2-101
 WBX10
 6'-9 1/16"
 92



RIGID FRAME ELEVATION: FRAME LINE 2

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510 Lee Rd 281
Salem AL, 36874

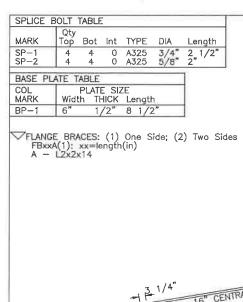


REVIEWED

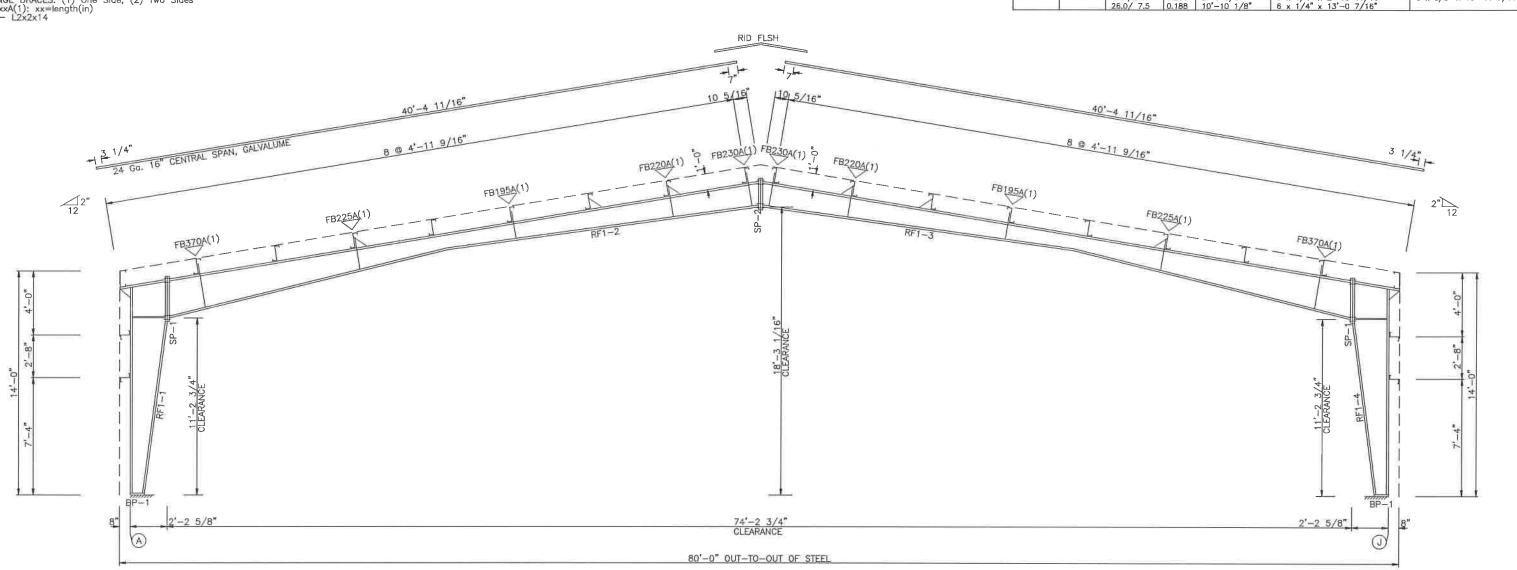
By Richard T Smith at 4:00 pm, Apr 26, 2023

JILDING "B"				
ISS	UE	DET	СНК	DATE
JILDINGS AND	MORE			
DMER: NEW MILLENIUM				
ю: 8039		DATE:	/24/2	.3
LAKE CITY, FL 3	2055			
RIGID FRAME CR	OSS SECTIO	N		
PAGE 2.2	DRAWN BY: DAR	SPW	: SCAL	E: NONE

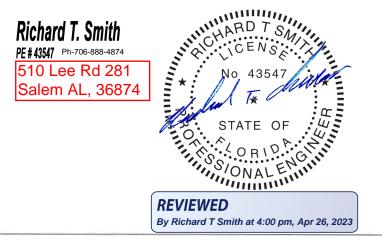
NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



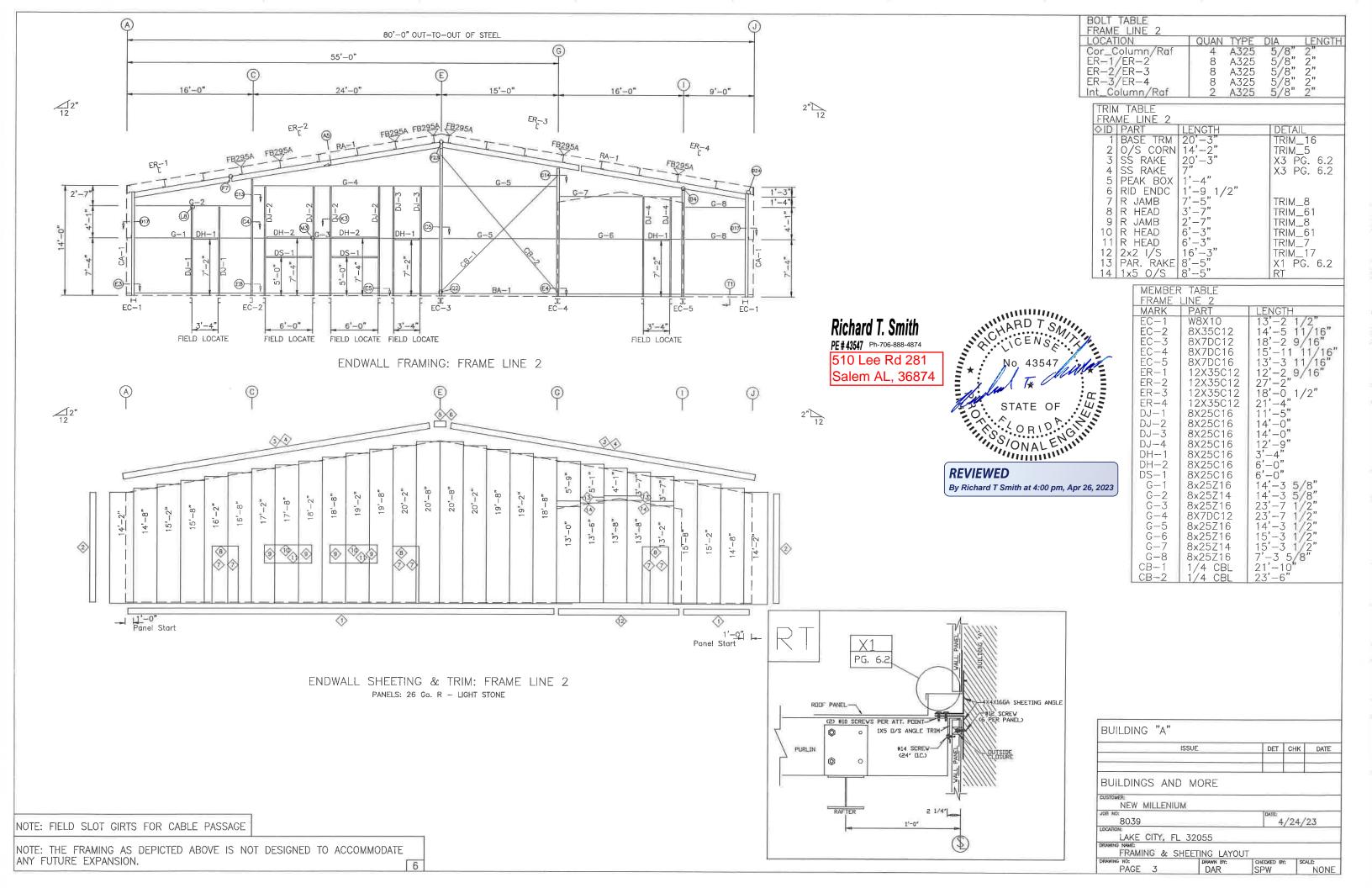
MADIZ	Mr. L.	Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF1-1	392	7.5/26.0	0.188	10'-10 1/8"	6 x 1/4" x 13'-0 7/16"	6 x 3/8" x 10'-11 7/16"
		26.0/26.0	0.250	2'-6 5/8"	6 x 1/4" x 2'-10 11/16"	
RF1-2	821	26.0/12.2	0.188	14'-11"	6 x 1/4" x 20'-0"	6 x 3/8" x 17'-11 3/16"
		12.2/ 9.5	0.188	2'-11 9/16"	6 x 1/4" x 17'-6 3/16"	6 x 1/4" x 19'-9 3/8"
	1	9.5/14.4	0.135	14'-11"		
		14.4/16.0	0.135	5'-1"		III.
RF1-3	821	16.0/14.4	0.135	5'-1"	6 x 1/4" x 17'-6 3/16"	6 x 1/4" x 19'-9 3/8"
		14.4/ 9.5	0.135	14'-11"	6 x 1/4" x 20'-0"	6 x 3/8" x 17'-11 3/16"
		9.5/12.2	0.188	2'-11 9/16"		,
		12.2/26.0	0.188	14'-11"		
RF1-4	393	26.0/26.0	0.250	2'-6 5/8"	6 x 1/4" x 2'-10 11/16"	6 x 3/8" x 10'-11 7/16"

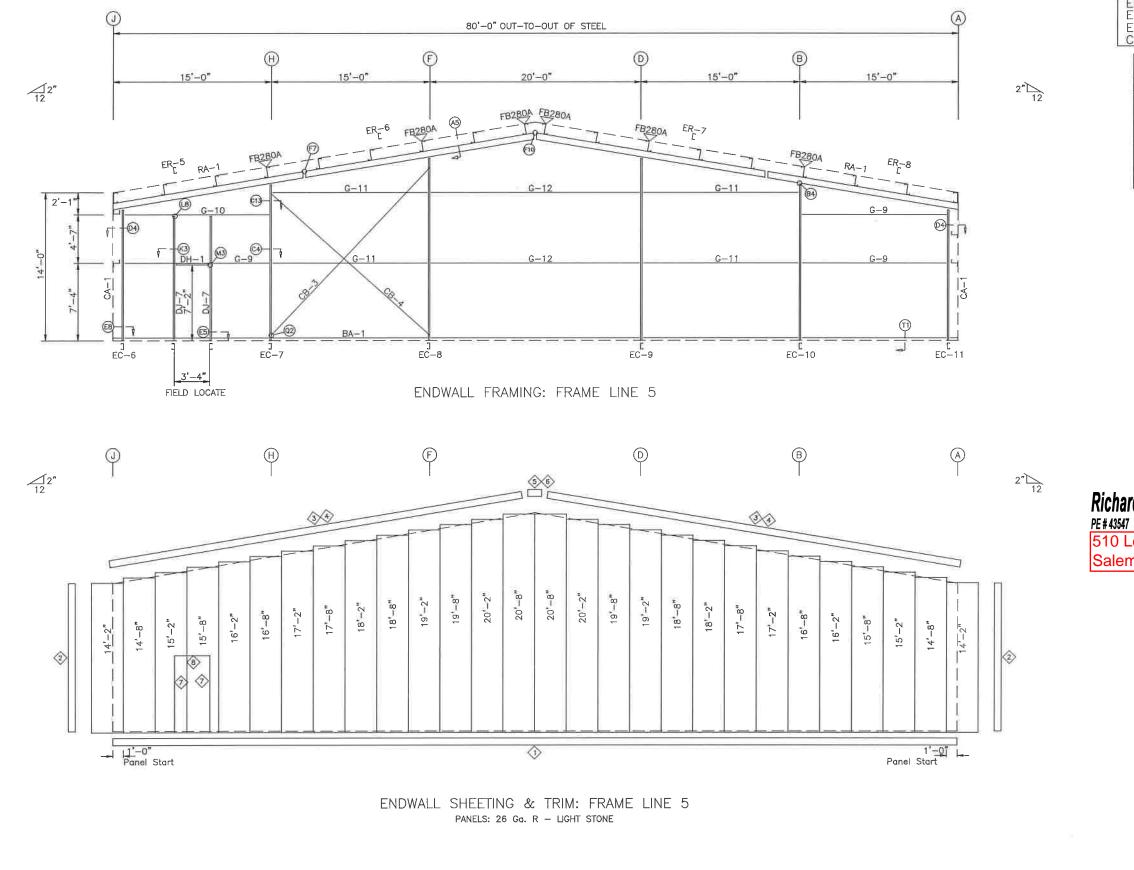


RIGID FRAME ELEVATION: FRAME LINE 3 4



BUILDING "A"				
i I	SSUE	DET	СНК	DATE
BUILDINGS AND	MORE			
CUSTOMER: NEW MILLENIUM	А			
108 NO: 8039		DATE:	/24/	23
LAKE CITY, FL	32055			
RIGID FRAME C		ıN .		
PAGE 2.3	DRAWN BY:	SPW SPW	: SC	ME: NONE





BOLT TABLE FRAME LINE 5 LOCATION ER-5/ER-6 ER-6/ER-7 ER-7/ER-8 A325 A325 A325 A325 5/8" 5/8" 5/8" 5/8" 888 Columns/Raf TRIM TABLE FRAME LINE 5 ♦ID | PART TRIM_16 TRIM_5 X3 PG. 6.2 X3 PG. 6.2 4 SS RAKE 5 PEAK BOX 6 RID ENDC 7 R JAMB 8 R HEAD 1'-9 1/2" | 7'-5" | 3'-7" TRIM_8 TRIM_61 MEMBER TABLE FRAME LINE 5 MARK PART LENGTH

12'-3 3/4"

14'-7 3/4"

17'-1 3/4"

14'-7 3/4"

12'-3 3/4"

18'-4 1/4"

22'-2 1/8"

22'-2 1/8"

18'-4 1/4"

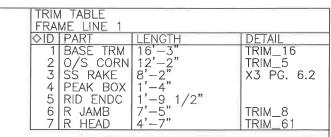
11'-11"

3'-4" 8X35C16 EC-6 EC-7 EC-8 EC-9 EC-10 EC-11 ER-5 ER-6 ER-7 8X35C12 8X35C12 8X35C12 8X35C12 8X35C16 8X35C12 8X35C12 8X35C12 ER-8 8X35C12 DJ-7 DH-1 G-9 8X25C16 8X25C16 13'-7 1 13'-7 1 14'-7 1 19'-11 22'-8" 20'-11" 8x25Z16 8x25Z16 8x25Z14 8x25Z16 8x25Z12 1/4 CBL 1/4 CBL G-10 G-11 G-12 CB-3 CB-4 HARD T SMIN Richard T. Smith PE # 43547 Ph-706-888-4874 510 Lee Rd 281 Salem AL, 36874 ORID. GIA **REVIEWED** By Richard T Smith at 4:00 pm, Apr 26, 2023 BUILDING "A" ISSUE DET CHK DATE BUILDINGS AND MORE NEW MILLENIUM 4/24/23 8039 LAKE CITY, FL 32055 FRAMING & SHEETING LAYOUT SPW PAGE 3.1 DAR

LENGTH

NOTE: FIELD SLOT GIRTS FOR CABLE PASSAGE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

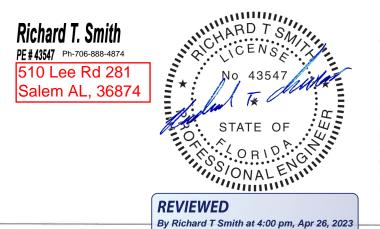


MEMBER		
FRAME L	INE 1	
MARK	PART	LENGTH
DJ-100	8X25C16	10'-0"
DH-100	8X25C16	4'-4"
G-100	8x25Z16	14'-7 1/2"
G-101	8x25Z14	14'-7 1/2"

12"	G 16'-0"OUT-TO-OUT OF STEEL	2" 12	2"	© *		2" 12
7'-4" 2'-8" 2'-0"	RA-1 C-101 C-100 DH-100 DH-100 C-100	CA-1-A-7	€	12'-4"	13'-4"	12'-10"
, ,	BA-1 T) FIELD LOCATE			2'-0" Panel Start	<u> </u>	L 2'-0" Panel Start

ENDWALL FRAMING: FRAME LINE 1

ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. R - LIGHT STONE



BUILDING "B"

ISSUE DET CHK DATE

BUILDINGS AND MORE

CUSTOMER:
NEW MILLENIUM

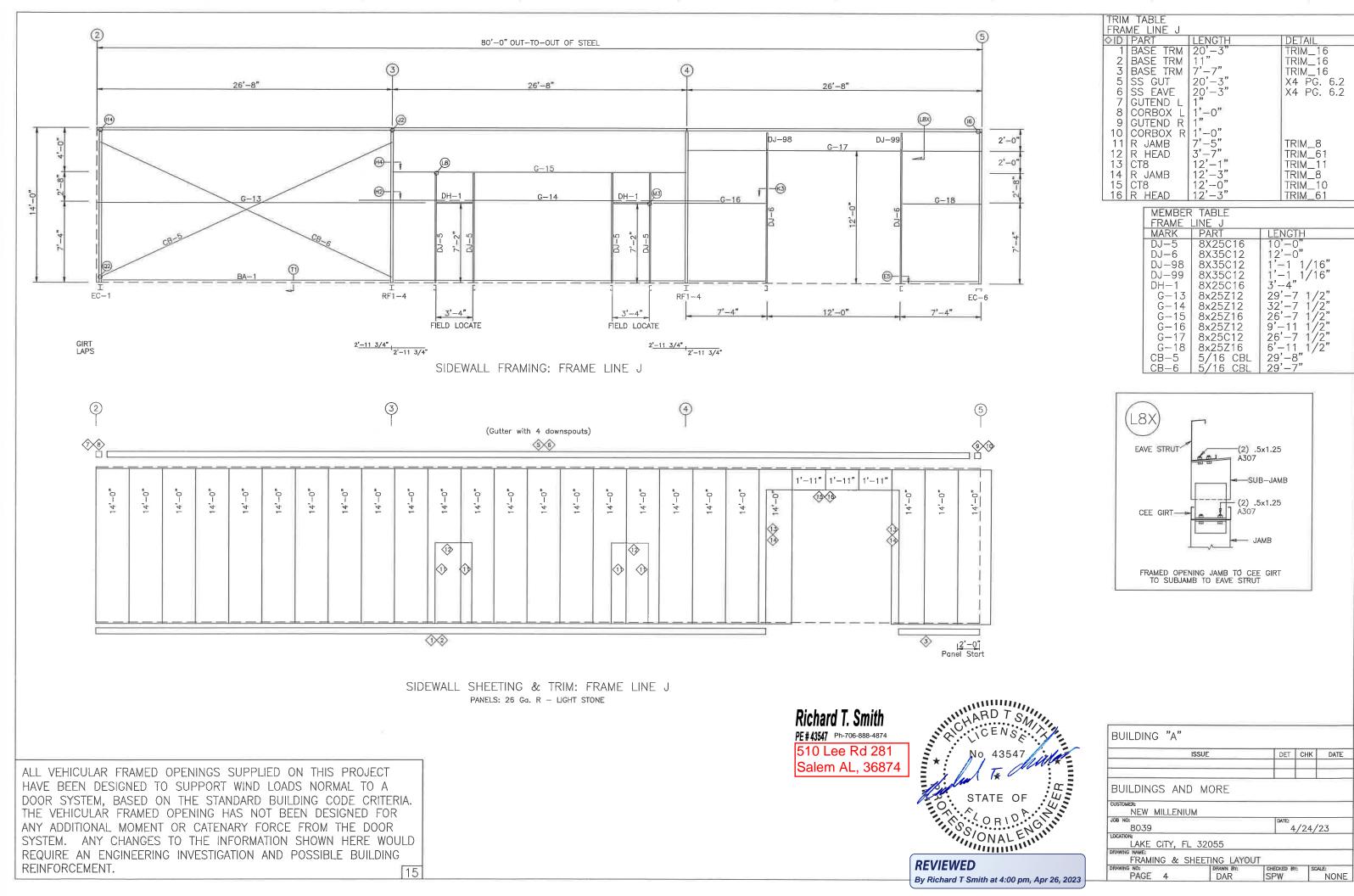
JOB NO:
80.39
LOCATION:
LAKE CITY, FL 32055

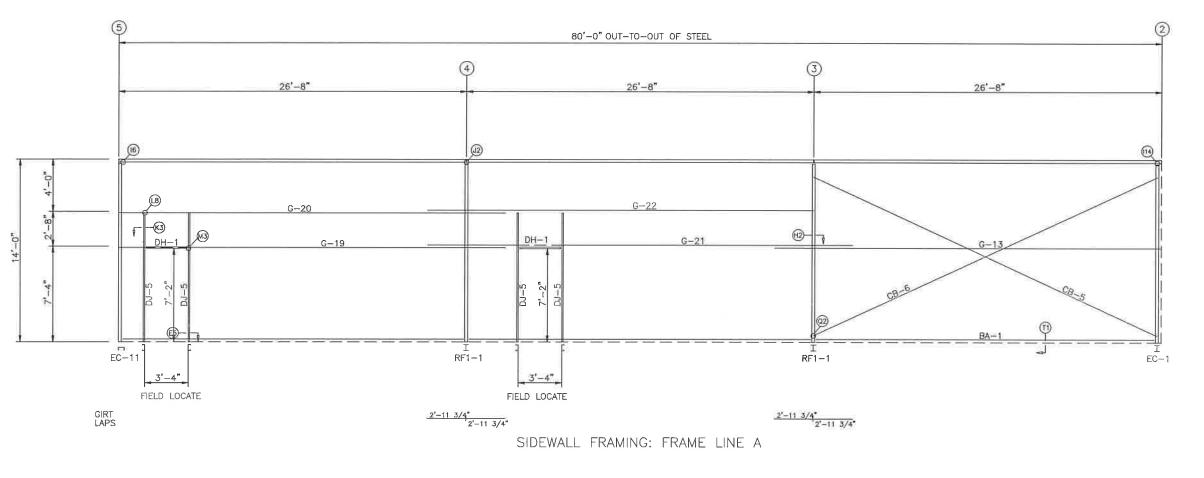
DRAWING NAME:
FRAMING & SHEETING LAYOUT

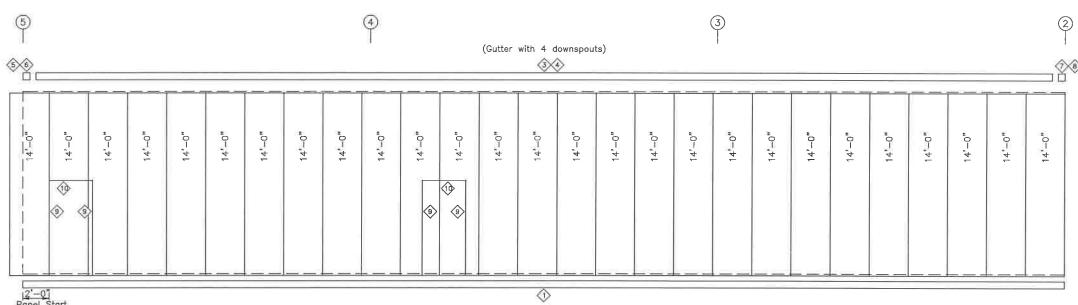
DRAWING NO:
PAGE 3.2
DRAWING FOR SPW

SCALE:
NONE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



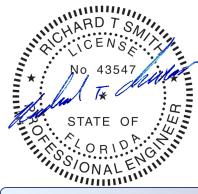




SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: 26 Ga. R - LIGHT STONE

Richard T. Smith
PE#43547 Ph-706-888-4874

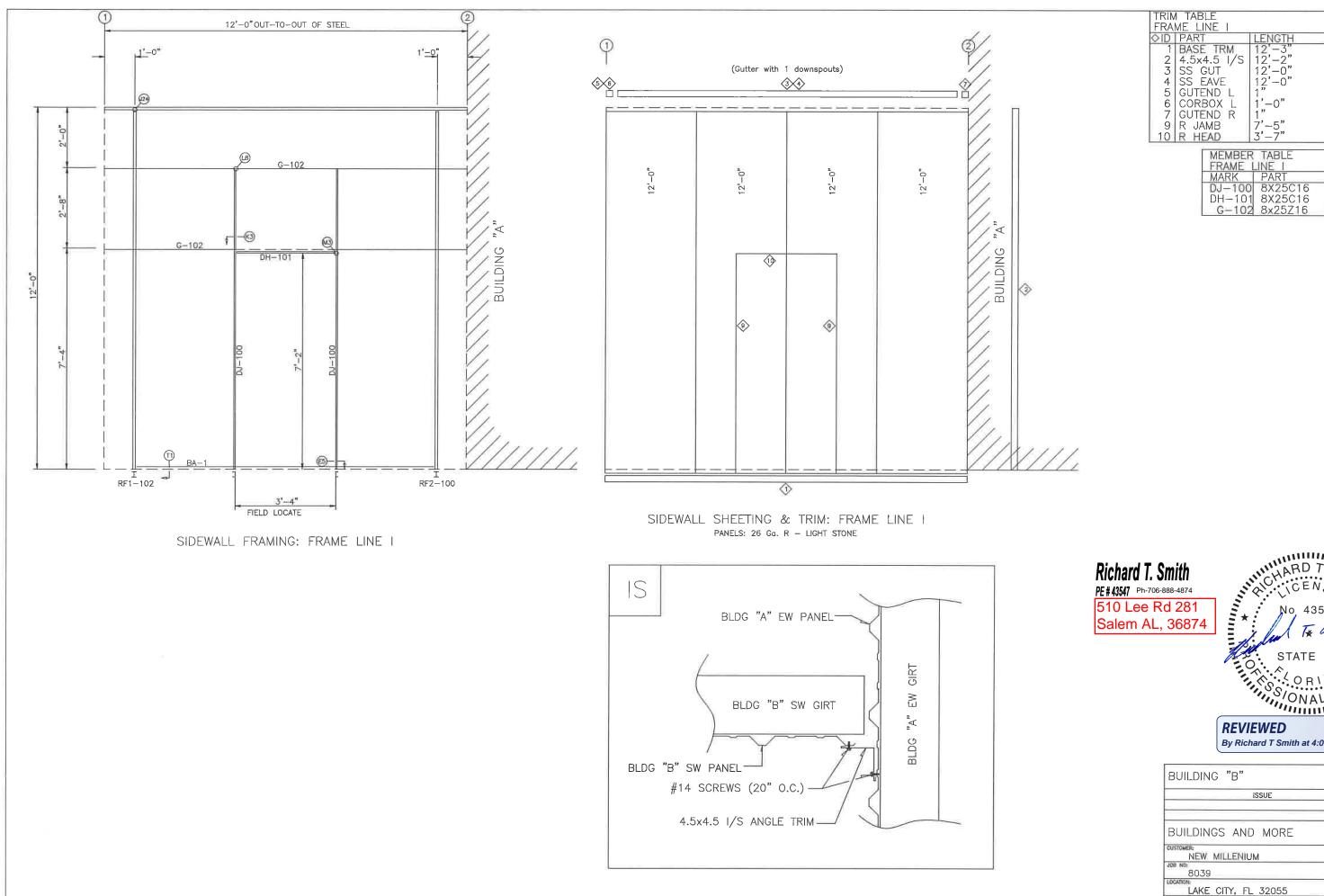
510 Lee Rd 281 Salem AL, 36874



 TRIM TABLE FRAME LINE A ◇ID PART LENGTH DETAIL 11 BASE TRM 20'-3" TRIM_16 3 LSS GUT 20'-3" X4 PG 6.2			
LFRA	ME LINE A		
◇ID	PART	LENGTH	DETAIL
1	BASE TRM		TRIM_16
3	SS GUT	20'-3"	X4 PG 6.2
4	SS EAVE	20'-3"	X4 PG 6.2
5	GUTEND L	1"	
6	CORBOX L	1'-0"	
7	GUTEND R	1"	
8	CORBOX R	1'-0"	
9	R JAMB	7'-5"	TRIM_8
10	R HEAD	3'-7"	TRIM_61

MEMBER	TABLE	
FRAME	LINE A	
MARK	PART	LENGTH
DJ-5	8X25C16	10'-0"
DH-1	8X25C16	3'-4"
G-13	8x25Z12	29'-7 1/2"
G-19	8x25Z16	29'-7 1/2"
G-20	8x25Z14	29'-7 1/2"
G-21	8x25Z16	32'-7 1/2"
G-22	8x25Z16	29'-11 1/2"
CB-5	5/16 CBL	29'-8" [']
CB-6	5/16 CBL	29'-7"

BUILDING "A"				
ISS	SUE	DET	СНК	DATE
BUILDINGS AND	MORE			
NEW MILLENIUM				
8 NO: 8039		DATE:	/24/2	3
LAKE CITY, FL	32055			
FRAMING & SHE	ETING LAYO	JT		
PAGE 4.1	DRAWN BY: DAR	SPW SPW		NONE



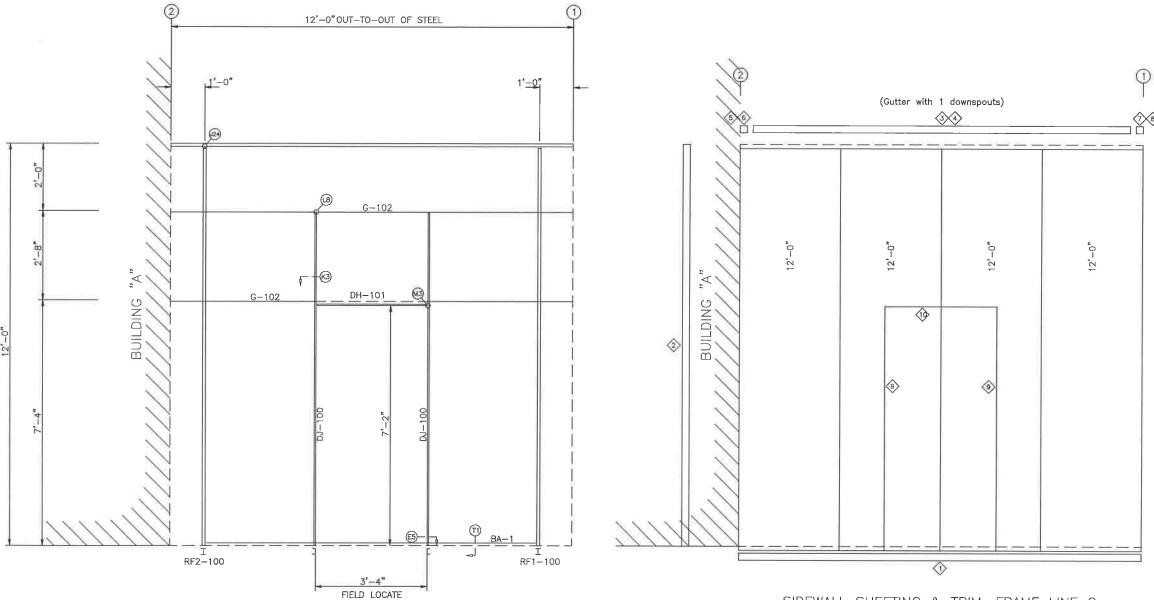
DETAIL TRIM_16 X4 PG 6.2 X4 PG 6.2 TRIM_8 TRIM_61

LENGTH 10'-0" 3'-4" 11'-11 1/2"



By Richard T Smith at 4:00 pm, Apr 26, 2023

DET CHK DATE 4/24/23 LAKE CITY, FL 32055 FRAMING & SHEETING LAYOUT DAR PAGE 4.2



SIDEWALL FRAMING: FRAME LINE G

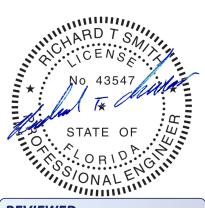
SIDEWALL SHEETING & TRIM: FRAME LINE G PANELS: 26 Ga. R - LIGHT STONE

TRIN	1 TABLE		
FRA	ME LINE G		
♦ID	PART	LENGTH	DETAIL
1	BASE TRM	12'-3"	TRIM_16
2 3	4.5x4.5 I/S	12'-2"	IS
3	SS GUT '	12'-0"	X4 PG 6.2
4	SS EAVE	12'-0"	X4 PG 6.2
5	GUTEND L	1"	
6	CORBOX L	1'-0"	
7	GUTEND R	1"	
8	CORBOX R	1'-0"	
9	R JAMB	7'-5"	TRIM_8
10	R HEAD	3'-7"	TRIM_61

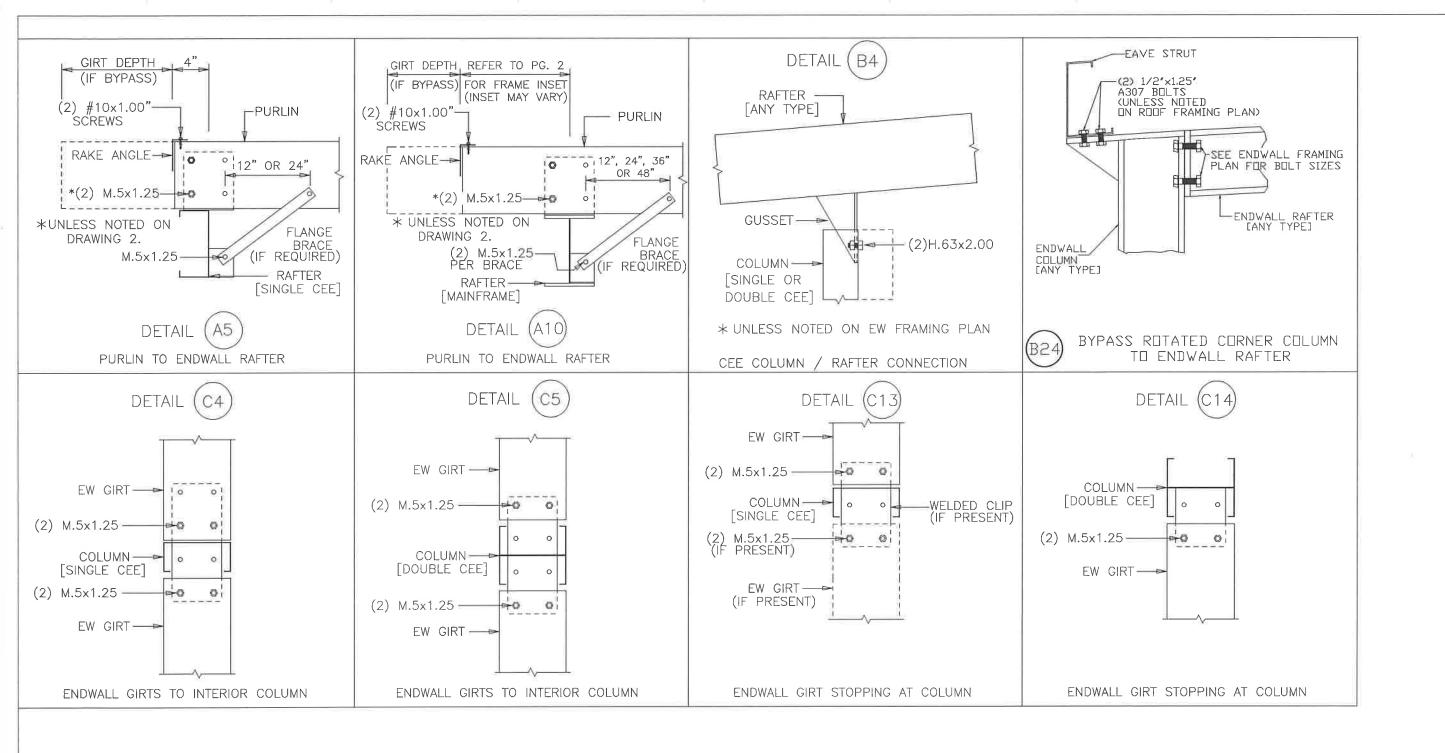
F	MEMBER	TABLE	
	FRAME L	INE G	
	MARK	PART	LENGTH
Ī	DJ-100	8X25C16	10'-0"
- 1	DH-101	8X25C16	3'-4"
- 1	G - 102	8x25716	11'-11 1/2"

Richard T. Smith
PE#43547 Ph-706-888-4874

510 Lee Rd 281 Salem AL, 36874



BUILDING "B"			
ISSUE		DET C	HK DATE
BUILDINGS AND M	ORE		
NEW MILLENIUM			
8039		DATE: 4/2	24/23
LAKE CITY, FL 32	055		
FRAMING & SHEET	ING LAYOUT		
PAGE 4.3	DAR	SPW	NONE.

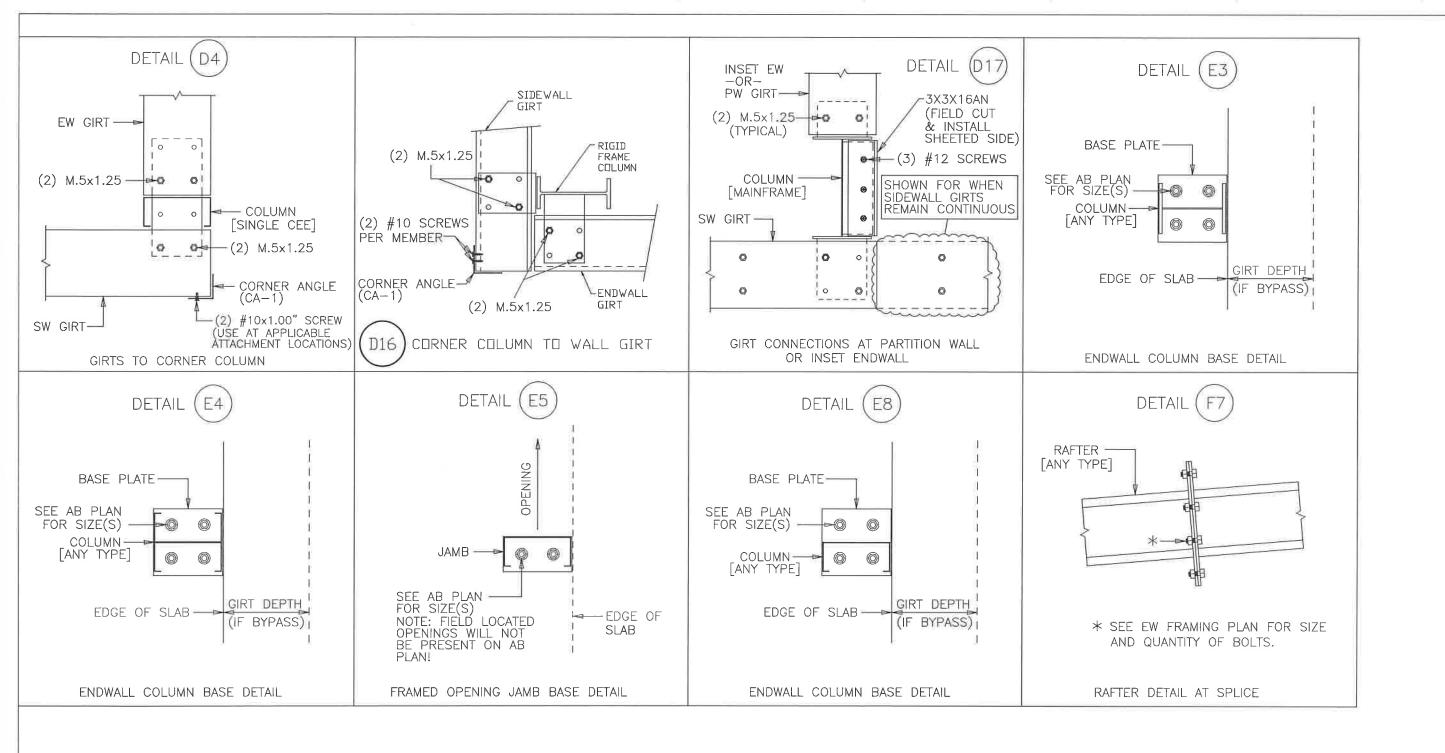


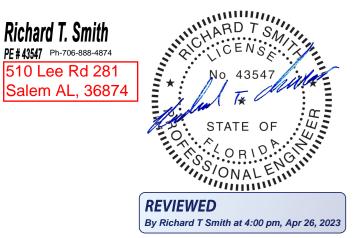




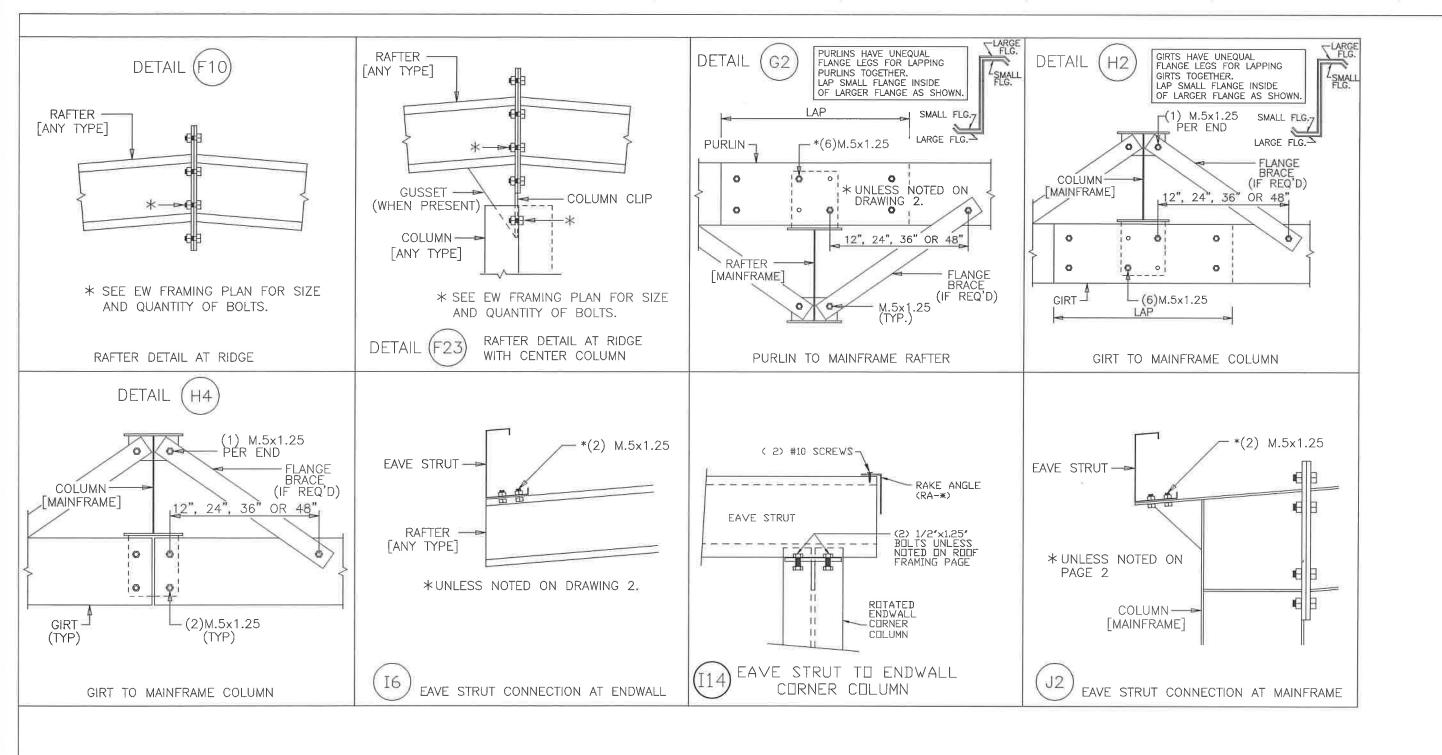
REVIEWED
By Richard T Smith at 4:00 pm, Apr 26, 2023

ISS	DET	снк	DATE	
JILDINGS AND	MORE			
TOMER: NEW MILLENIUM				
8039 DATE: 4/24/23				23
LAKE CITY, FL.	32055			
FRAMING DETAIL	S			
WING NO: PAGE 5	DRAWN BY: DAR	SPW	SC	NONE





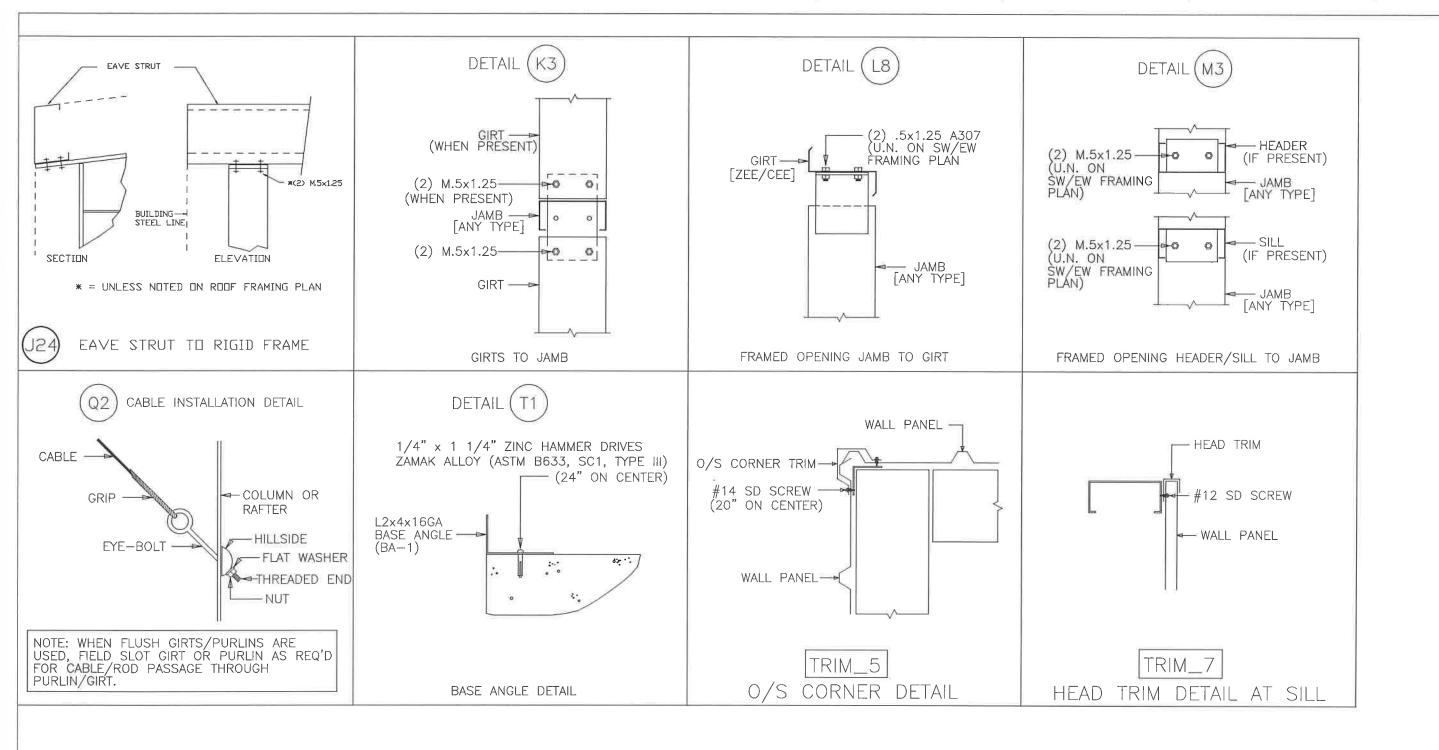
IS	DET	CHK	DATE	
BUILDINGS AND	MORE			
OUSTOMER: NEW MILLENIUM				
JOB NO: 8039		DATE:	/24/	23
LAKE CITY, FL	32055			
FRAMING DETAIL	S			
PAGE 5.1	DAR	SPW	SCA	NONE







	SSUE	DET	CHK	DATE
BUILDINGS AND	MORE			
DOILDINGS AND	WORL			
CUSTOMER				
NEW MILLENIUN	A			
JDB NO:		DATE:	nwent w	2000
8039		4	/24/	23
LOCATION:				
LAKE CITY, FL	32055			
DRAWING NAME:				
FRAMING DETAIL	LS			
DRAWING NO:	DRAWN BY:	CHECKED BY	SC/	V.E.
PAGE 5.2	DAR	SPW		NONE







By Richard T Smith at 4:00 pm, Apr 26, 2023

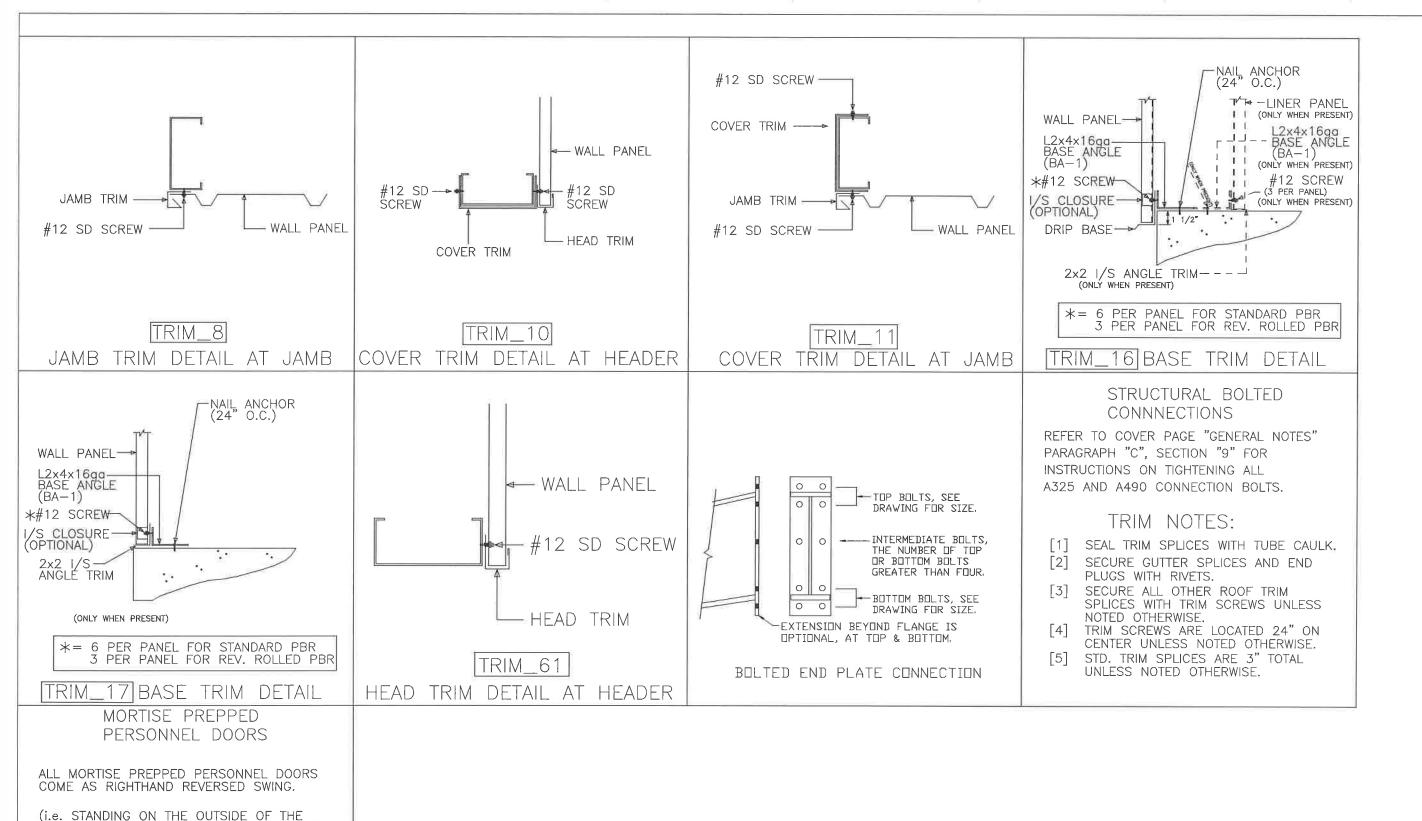
BUILDINGS AND MORE

CUSTOMER:
NEW MILLENIUM

JOB NO:
8039
LOCATION:
LAKE CITY, FL 32055

DIVANNING NAME:
FRAMING DETAILS

DRAWN BY:
PAGE 5.3
DRAWN BY:
PAGE 5.3
DRAWN BY:
SPW
NONE



BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF

THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

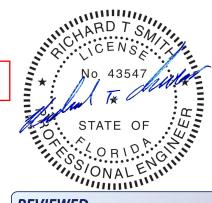
ANY FIELD MODIFICATIONS ARE THE RE— SPONSIBILITY OF THE ERECTOR AND MBM

DAMAGES DUE TO ERROR.

IS NOT LIABLE FOR LABOR CHARGES NOR

Richard T. Smith
PE # 43547 Ph-706-888-4874

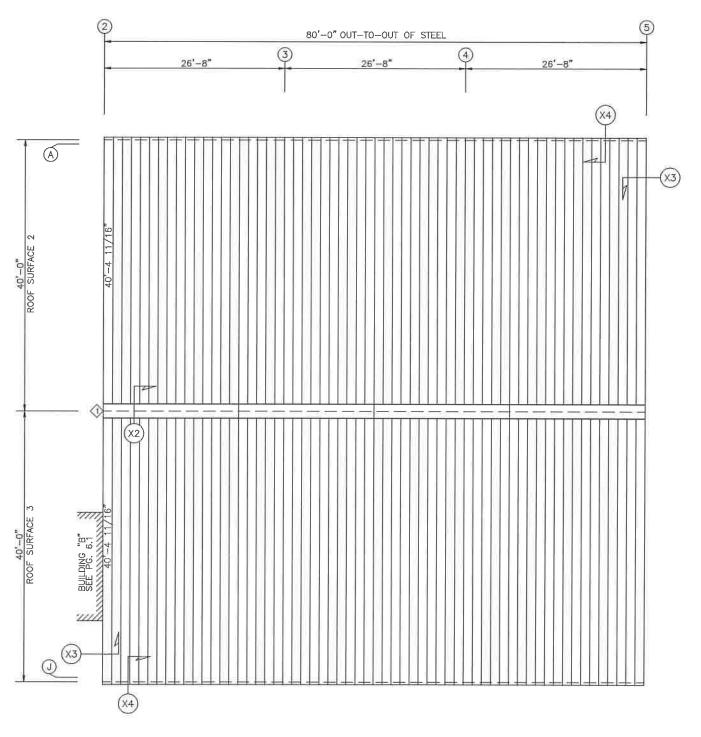
510 Lee Rd 281 Salem AL, 36874



BUILDINGS AND MORE

CUSTOMER:
NEW MILLENIUM
JOB NO:
BO39
LDCATION:
LAKE CITY, FL 32055
DEAMING NAME:
FRAMING DETAILS
DRAWING NAME:
PAGE 5.4
DAR
SPW
NONE

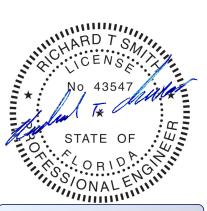




ROOF SHEETING PLAN
PANELS: 24 Ga. 16" CENTRAL SPAN (90" TRIPLE LOCK SEAM) — GALVALUME

Richard T. Smith

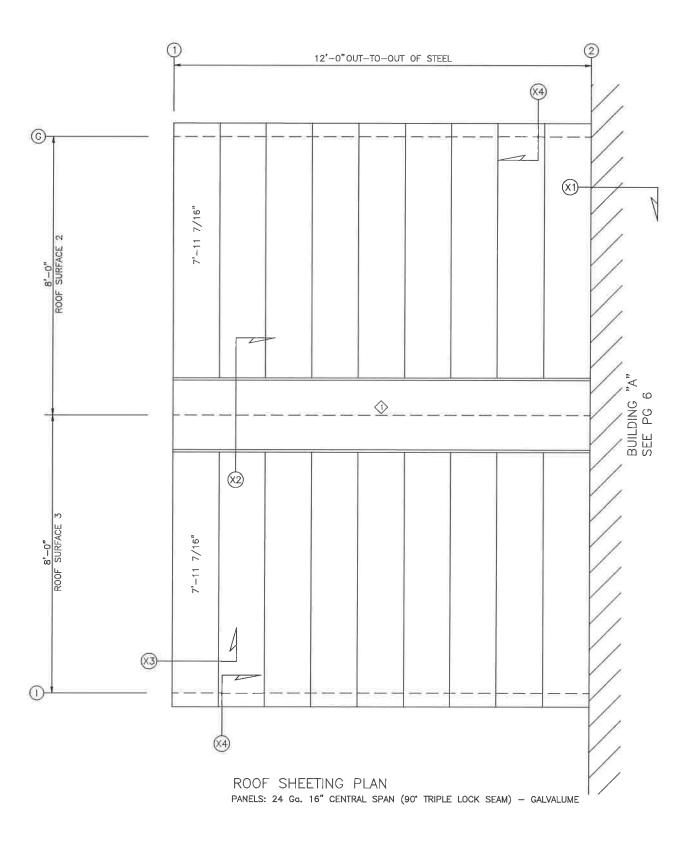
PE#43547 Ph-706-888-4874 510 Lee Rd 281 Salem AL, 36874



BUILDING "A"				
ISSUE		DET	СНК	DATE
BUILDINGS AND M	ORE			
USTOMER: NEW MILLENIUM				
08 NO: DATE: 4/24/2			23	
LAKE CITY, FL 32	055	-11		
ROOF PANELS &	TRIM			
PAGE 6	DAR	SPW	; SC	NONE

TRIM TABLE
ROOF PLAN

◇ID | PART | LENGTH
1 | RID | FLSH | 12'-0"



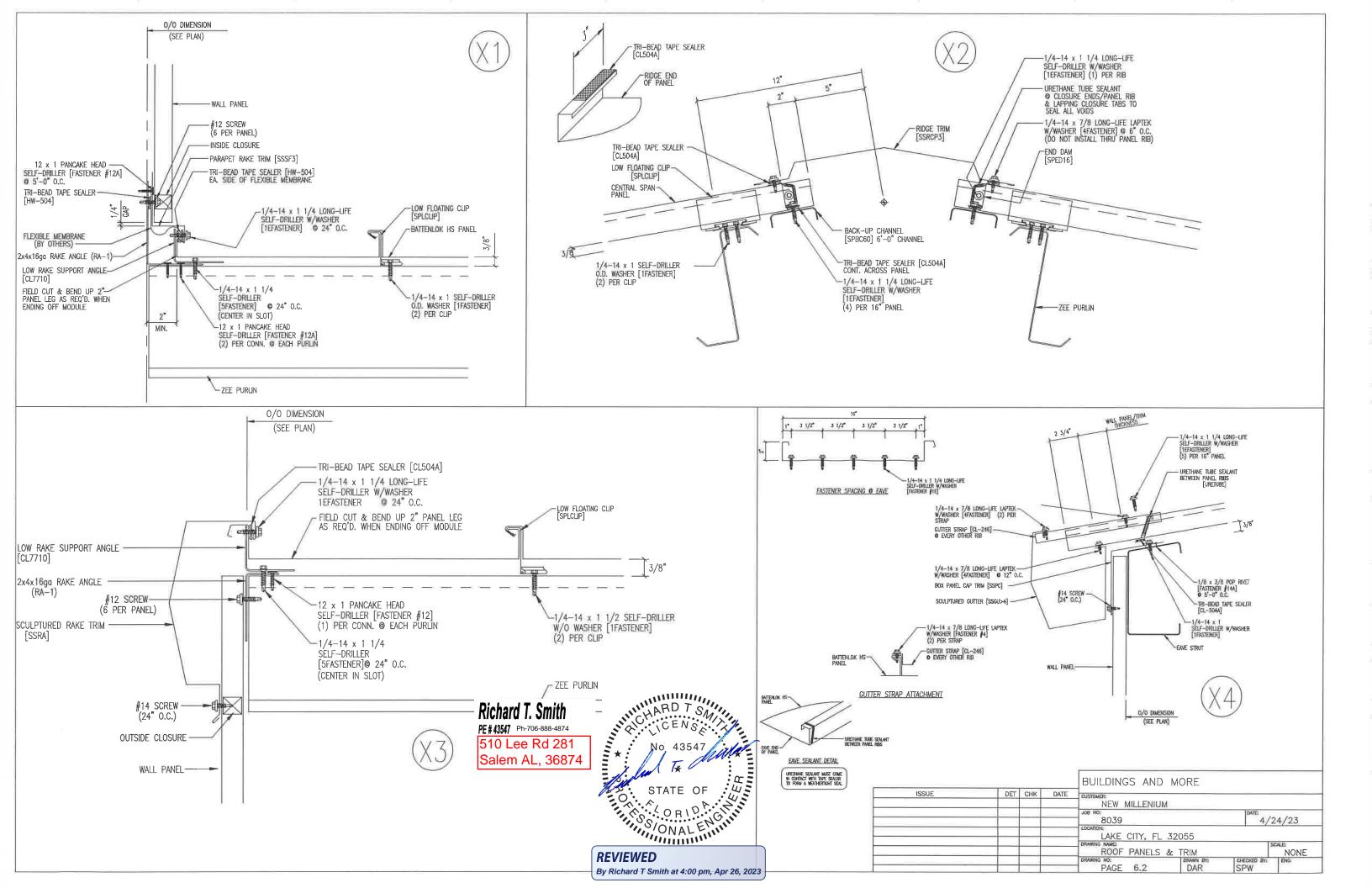
Richard T. Smith
PE#43547 Ph-706-888-4874
510 Lee Rd 281

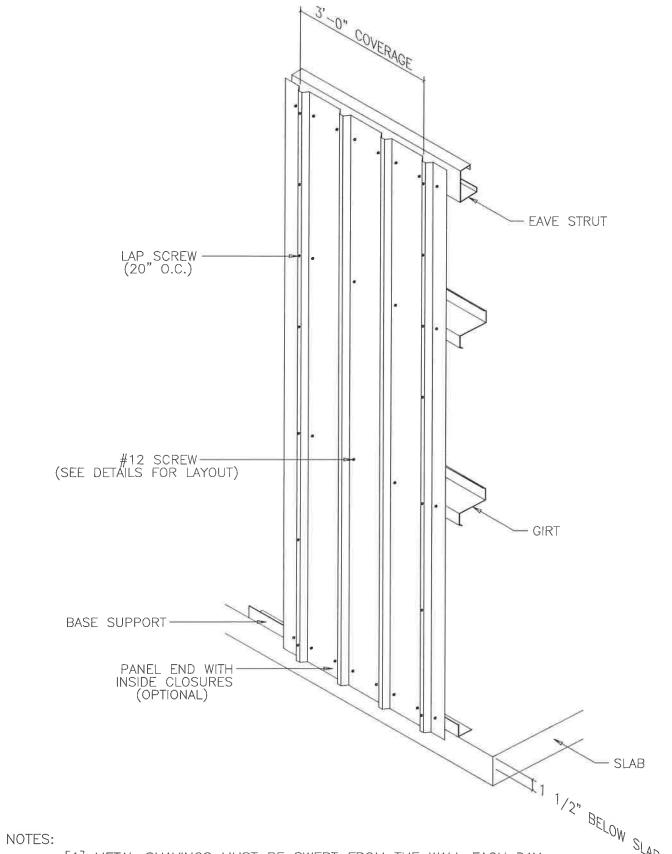
Salem AL, 36874



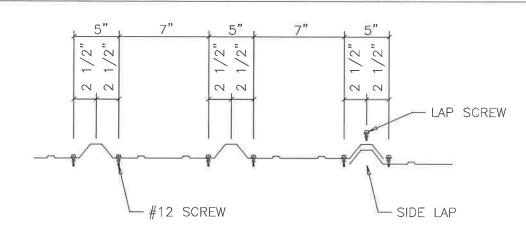
REVIEWED

ISSUE		СНК	DATE
1.5			
ORE			
DATE: 4/24/23			23
055			
TRIM			
DRAWN BY: DAR	SPW SPW	50	NONE
	ORE 055 TRIM DRAWN BY:	ORE DATE: 4, 055 TRIM DRAWN BY: CHECKED BY:	ORE DATE: 4/24/ 055 TRIM DRAWN BY: CHECKED BY: SQ

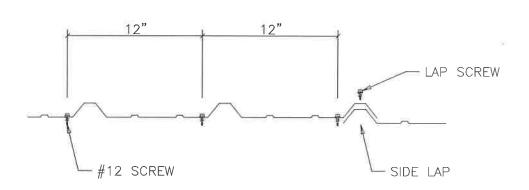




- [1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.



PANEL ATTACHMENT AT PANEL END (BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

Richard T. Smith
PE # 43547 Ph-706-888-4874

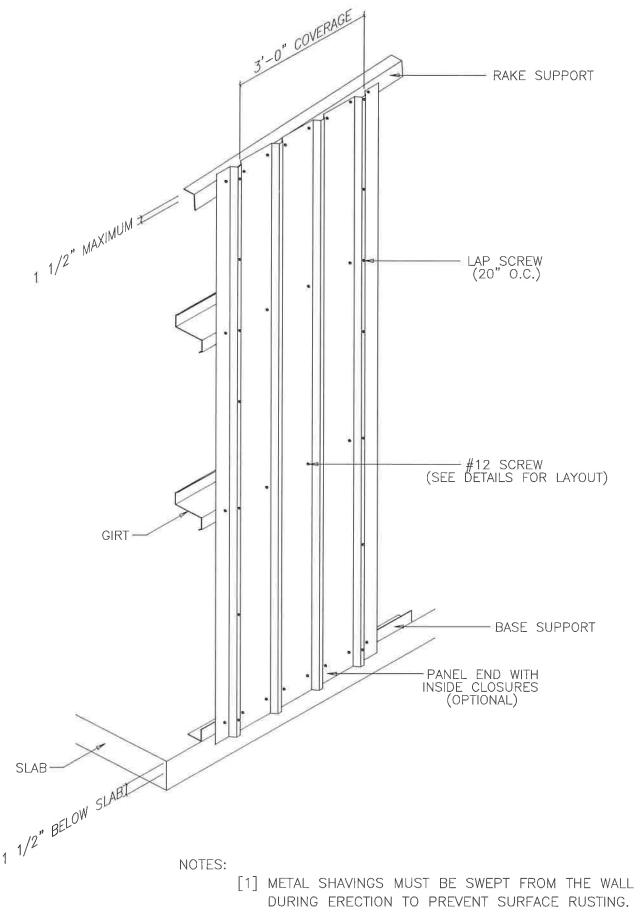
510 Lee Rd 281 Salem AL, 36874



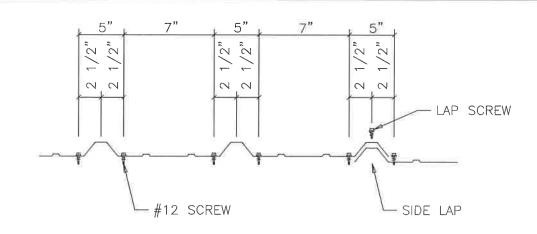
ISSUE		DET	СНК	DATE
BUILDINGS AND	MORE			
CUSTOMER: NEW MILLENIU	M			
JOB NO: 8039			4/24/23	
LAKE CITY, FL	32055			
SIDEWALL PAN	EL DETAILS			
PAGE 7	DRAWN BY: DAR	SPW BY	SCA	NONE

KEVIEWED

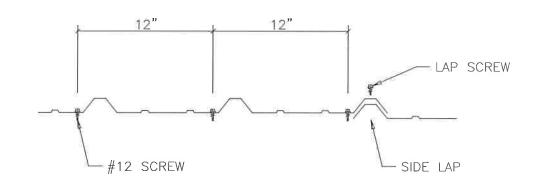
By Richard T Smith at 4:00 pm, Apr 26, 2023



- [1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.



PANEL ATTACHMENT AT PANEL END (BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)

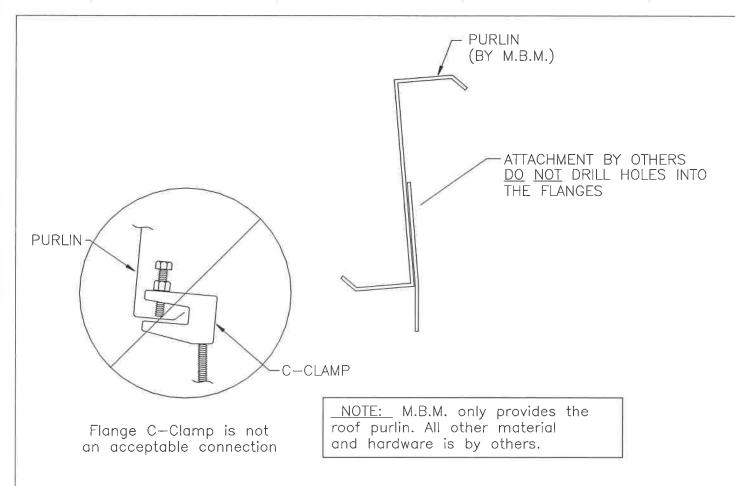


PANEL ATTACHMENT AT INTERMEDIATE MEMBERS





	ISSUE		СНК	DATE	
BUILDINGS AND	MORE				
CUSTOMER: NEW MILLENIU	М				
лов мо: 8039		DATE:	4/24/23		
LAKE CITY, FL	32055				
DRAWING NAME: ENDWALL PANE	EL DETAILS				
PAGE 8	DRAWN BY: DAR	SPW SPW	: SCA	NONE	



Recommended Connection Detail

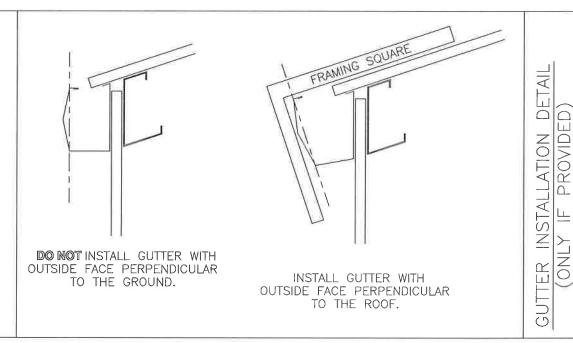
NOTE

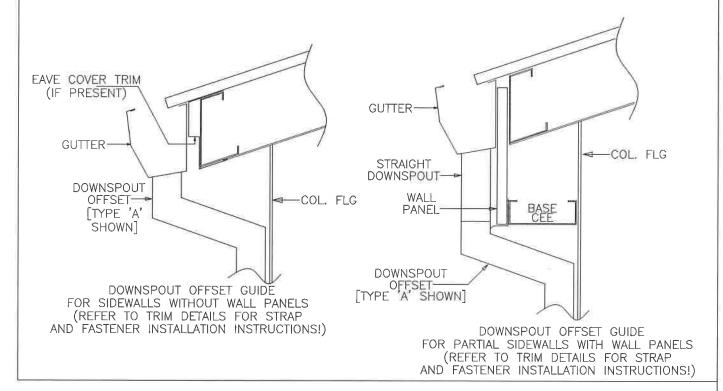
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS







510 Lee Rd 281 Salem AL, 36874



BUILDINGS AND	MORE		
USTOMER: NEW MILLENIUM			
OB NO:		DATE:	
8039		4/2	24/23
OCATION:	Series and		
LAKE CITY, FL 3	2055		
RAWING NAME:			
SPECIAL DETAILS			
RAWING NO:	DRAWN BY:	CHECKED BY:	SCALE
PAGE 9	DAR	SPW	NONE

DET CHK DATE

ISSUE