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<b>Site Information:</b>	<b>Page 1:</b>
Customer: W. B. Howland Company, Inc.	Job Number: 20-4510
Job Description: Lot 41 JL	
Address: LOT 41 JEWELL LAKE, LAKE CITY, FL	

<b>Job Engineering Criteria:</b>			
Design Code: FBC 2017 RES		IntelliVIEW Version: 19.02.02B	JRef #: 1WXX2150001
Wind Standard: ASCE 7-10	Wind Speed (mph): 130	Roof Load (psf): 20.00-10.00- 0.00-10.00	Floor Load (psf): None
Building Type: Closed			

This package contains general notes pages, 25 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	231.20.1145.03259	A01
3	231.20.1145.03291	A03
5	231.20.1149.52686	B01
7	231.20.1149.52827	B03
9	231.20.1149.52717	B05
11	231.20.1149.52670	B07
13	231.20.1149.52779	C02
15	231.20.1153.36390	D01
17	231.20.1153.59759	G01
19	231.20.1153.59823	HJ1
21	231.20.1153.59870	J2
23	231.20.1153.59760	J3
25	231.20.1153.59714	P02
27	BRCLBSUB0119	
29	PB160101014	

Item	Drawing Number	Truss
2	231.20.1145.03305	A02
4	231.20.1149.53013	A04
6	231.20.1149.52530	B02
8	231.20.1149.52701	B04
10	231.20.1149.52967	B06
12	231.20.1149.52826	C01
14	231.20.1153.33710	C03
16	231.20.1153.59699	D02
18	231.20.1153.59713	G02
20	231.20.1153.59869	J1
22	231.20.1153.59698	J2A
24	231.20.1153.59838	P01
26	A14015ENC101014	
28	GBLLETIN0118	
30	A14030ENC101014	

## **General Notes**

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

## **General Notes** (continued)

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

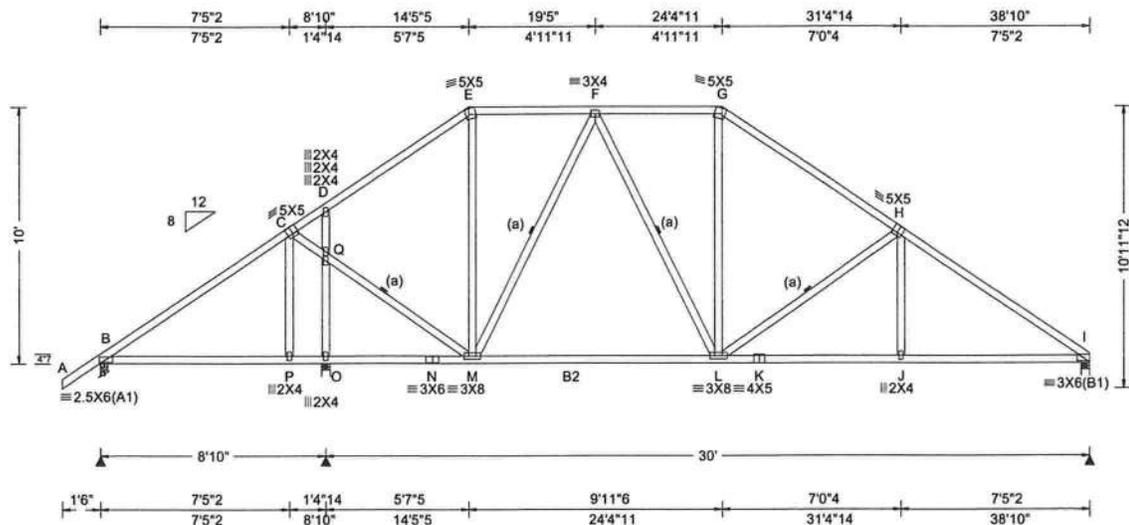
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

**References:**

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcindustry.com](http://www.sbcindustry.com).



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.88 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg, Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.091 L 999 360 VERT(CL): 0.179 L 999 240 HORZ(LL): 0.043 J - - HORZ(TL): 0.084 J - - Creep Factor: 2.0 Max TC CSI: 0.857 Max BC CSI: 0.785 Max Web CSI: 0.711  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1277 /- /- /717 /191 /316 O 757 /- /- /445 /131 /- I 1574 /- /- /901 /236 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 O Brg Width = 4.0 Min Req = 1.5 I Brg Width = 4.0 Min Req = 1.9 Bearings B, O, & I are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 303 -1623 F - G 454 -1470 C - D 336 -1491 G - H 481 -1890 D - E 433 -1660 H - I 507 -2388 E - F 406 -1285
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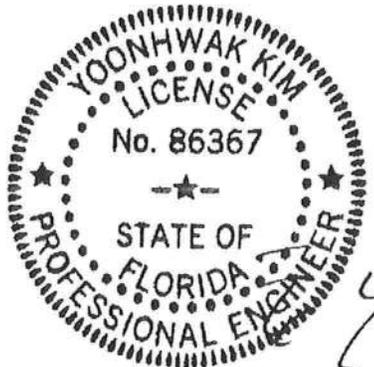
**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 10-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

**Maximum Bot Chord Forces Per Ply (lbs)**

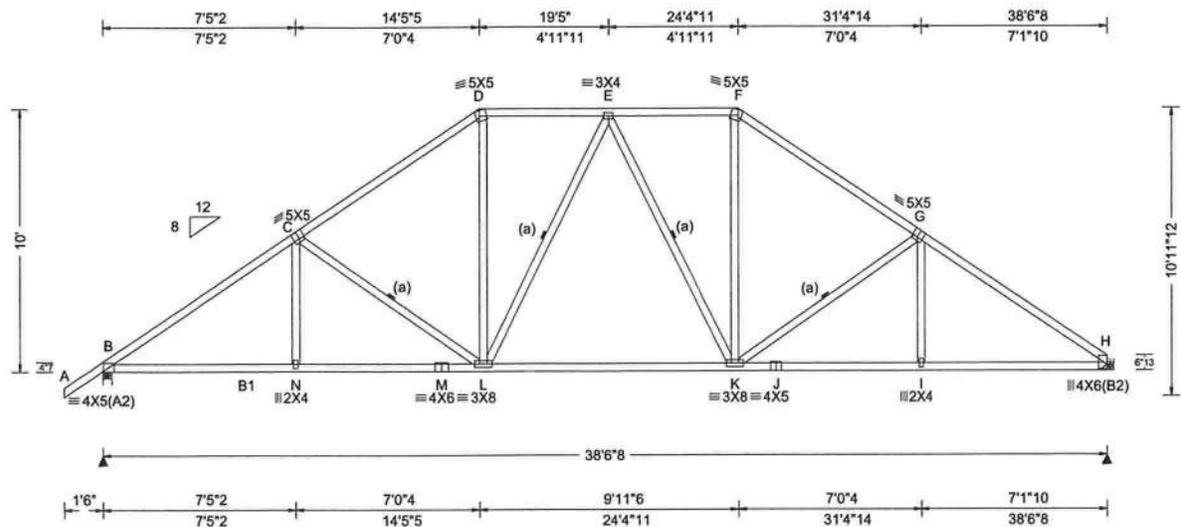
Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	1239 -149	M - L	1454 -147
P - O	1240 -150	L - K	1887 -319
O - N	1240 -150	K - J	1887 -319
N - M	1240 -150	J - I	1888 -318

**Maximum Web Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - D	140 -477	M - F	123 -424
O - Q	155 -574	L - G	596 -113
E - M	559 -100	L - H	218 -525

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
**\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**  
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.  
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org





<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.85 ft Loc. from endwall: not in 9.00 ft Gcpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Def/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.125 E 999 360 VERT(CL): 0.245 E 999 240 HORZ(LL): 0.061 I - - HORZ(TL): 0.119 I - - Creep Factor: 2.0 Max TC CSI: 0.692 Max BC CSI: 0.785 Max Web CSI: 0.280  VIEW Ver: 19.02.02B.0122.15	<b>Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1839</td> <td>-</td> <td>-</td> <td>1057</td> <td>291</td> <td>312</td> </tr> <tr> <td>H</td> <td>1725</td> <td>-</td> <td>-</td> <td>1957</td> <td>264</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS  B Brg Width = 4.0 Min Req = 2.2  H Brg Width = - Min Req = -  Bearing B is a rigid surface.  Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th></th> <th></th> <th></th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>558</td> <td>-2648</td> <td>E - F</td> <td>509</td> <td>-1698</td> </tr> <tr> <td>C - D</td> <td>542</td> <td>-2163</td> <td>F - G</td> <td>552</td> <td>-2156</td> </tr> <tr> <td>D - E</td> <td>514</td> <td>-1701</td> <td>G - H</td> <td>565</td> <td>-2599</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1839	-	-	1057	291	312	H	1725	-	-	1957	264	-	Chords	Tens.Comp.		Chords	Tens. Comp.						B - C	558	-2648	E - F	509	-1698	C - D	542	-2163	F - G	552	-2156	D - E	514	-1701	G - H	565	-2599
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Hangers / Ties**  
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.  
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.  
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.  
Bearing at location x=38'3"8 uses the following support conditions: 38'3"8  
Bearing H (38'3"8, 9'1"2) HUS26  
Supporting Member: (3)2x8 SP 2400f-2.0E  
(14) 0.148"x3" nails into supporting member,  
(4) 0.148"x3" nails into supported member.

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Refer to DWG PB160101011 for piggyback details.  
The overall height of the truss including diaphragm is 10-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.		Chords	Tens. Comp.	
B - N	2099	-377	K - J	2051	-368
N - M	2098	-377	J - I	2051	-368
M - L	2098	-377	I - H	2051	-367
L - K	1766	-234			

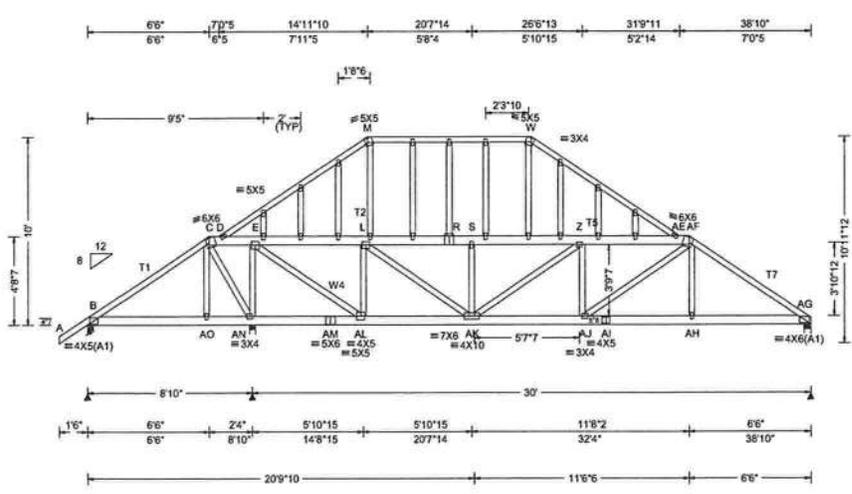
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.		Webs	Tens. Comp.	
C - L	206	-500	K - F	728	-149
D - L	736	-140	K - G	203	-447

**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



2 Complete Trusses Required



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.88 ft Loc. from endwall: not in 5.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.085 AC 999 360 VERT(CL): 0.194 AC 999 240 HORZ(LL): 0.018 M - - HORZ(TL): 0.040 M - - Creep Factor: 2.0 Max TC CSI: 0.241 Max BC CSI: 0.183 Max Web CSI: 0.631  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 119 /-171 /- /- /22 /- AN 6312 /- /- /- /1079 /- AG 3452 /- /- /- /582 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 AN Brg Width = 4.0 Min Req = 2.2 AG Brg Width = 4.0 Min Req = 1.5 Bearings B, AN, & AG are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.
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**Lumber**  
Top chord: 2x4 SP #2; T1,T7 2x4 SP M-31; T2, T5 2x6 SP 2400f-2.0E;  
Bot chord: 2x6 SP 2400f-2.0E;  
Webs: 2x4 SP #3; W4 2x4 SP #2;

**Nailnote**  
Nail Schedule: 0.128"x3", min. nails  
Top Chord: 1 Row @11.75" o.c.  
Bot Chord: 1 Row @12.00" o.c.  
Webs : 1 Row @ 4" o.c.  
Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From 64 plf at -1.50 to 64 plf at 6.50  
TC: From 32 plf at 6.50 to 32 plf at 32.33  
TC: From 64 plf at 32.33 to 64 plf at 38.83  
BC: From 5 plf at -1.50 to 5 plf at 0.00  
BC: From 20 plf at 0.00 to 20 plf at 6.53  
BC: From 10 plf at 6.53 to 10 plf at 32.30  
BC: From 20 plf at 32.30 to 20 plf at 38.83  
TC: 234 lb Conc. Load at 6.53,32.30  
TC: 178 lb Conc. Load at 8.56,10.56,12.56,14.56  
16.56,18.56,20.27,22.27,24.27,26.27,28.27,30.27  
BC: 416 lb Conc. Load at 6.53,32.30  
BC: 121 lb Conc. Load at 8.56,10.56,12.56,14.56  
16.56,18.56,20.27,22.27,24.27,26.27,28.27,30.27

**Plating Notes**  
All plates are 2X4 except as noted.  
Laterally brace top chord below filler at 2'0" O.C..

**Loading**  
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

**Wind**  
Wind loads and reactions based on MWFRS.

**Additional Notes**  
See DWGS A14015ENC 10.014 & GBL ETM 118 for gable wind bracing and other requirements.  
The overall height of this truss excluding overhangs 10-0-0.

B - C	508	-84	R - S	232	-1435
C - D	908	-151	S - Z	233	-1442
D - E	1656	-284	W -AE	185	-1025
D - M	180	-995	Z -AE	318	-1925
L - R	233	-1441	AE-AF	445	-2650
M - W	133	-754	AF-AG	499	-2879

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp. Chords Tens. Comp.  
AO-AN 56 -376 AK-AJ 2677 -454  
AN-AM 123 -762 AJ-AI 2330 -401  
AM-AL 123 -762 AI-AH 2330 -401  
AL-AK 969 -163 AH-AG 2339 -401

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
C -AN 200 -1134 L -AK 1514 -251  
AN -E 356 -1897 AK -Z 110 -608  
E -AL 2078 -338 AJ-AF 434 -63  
AL -L 227 -1070

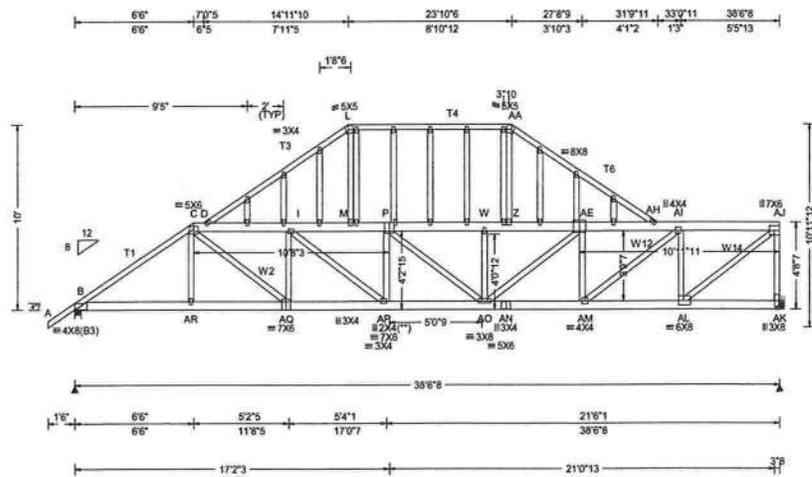


FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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**ALPINE**  
AN ITW COMPANY  
6750 Forum Drive  
Suite 305  
Orlando FL, 32821

2 Complete Trusses Required



**Loading Criteria (psf)**

TCLL: 20.00  
 TCDDL: 10.00  
 BCLL: 0.00  
 BCDL: 10.00  
 Des Ld: 40.00  
 NCBCLL: 0.00  
 Soffit: 2.00  
 Load Duration: 1.25  
 Spacing: 24.0"

**Wind Criteria**

Wind Std: ASCE 7-10  
 Speed: 130 mph  
 Enclosure: Closed  
 Risk Category: II  
 EXP: C Kzt: NA  
 Mean Height: 15.00 ft  
 TCDDL: 5.0 psf  
 BCDL: 5.0 psf  
 MWFRS Parallel Dist: 0 to h/2  
 C&C Dist a: 3.85 ft  
 Loc. from endwall: not in 5.00 ft  
 GCpi: 0.18  
 Wind Duration: 1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg: NA Ct: NA CAT: NA  
 Pf: NA Ce: NA  
 Lu: NA Cs: NA  
 Snow Duration: NA

**Building Code:**  
 FBC 2017 RES  
 TPI Std: 2014  
 Rep Fac: No  
 FT/RT:20(0)/10(0)  
 Plate Type(s):  
 WAVE

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#  
 VERT(LL): 0.166 S 999 360  
 VERT(CL): 0.373 S 999 240  
 HORZ(LL): 0.051 E - -  
 HORZ(TL): 0.114 E - -  
 Creep Factor: 2.0  
 Max TC CSI: 0.313  
 Max BC CSI: 0.260  
 Max Web CSI: 0.816

VIEW Ver: 19.02.02B.0122.15

**Maximum Reactions (lbs)**

Loc	Gravity		Non-Gravity	
	R+	/R-	/Rh	/Rw / U / RL
B 4787	-	-	-	/850 -
AK 5023	-	-	-	/863 -

Wind reactions based on MWFRS  
 B Brg Width = 4.0 Min Req = 2.0  
 AK Brg Width = - Min Req = -  
 Bearing B is a rigid surface.  
 Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	697 -3966	P - W	525 -3129
C - D	697 -4073	W - Z	525 -3129
D - L	276 -1557	Z - AE	527 -3144
D - I	499 -2932	AA-AH	274 -1543
I - M	536 -3192	AE-AH	487 -2869
L - AA	206 -1189	AH-AI	690 -4036
M - P	533 -3178	AI-AJ	505 -2926

**Lumber**

Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP M-31; T3, T4, T6 2x4 SP #2;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W2, W12 2x4 SP #2; W14 2x4 SP M-31;

**Plating Notes**

All plates are 2X4 except as noted.  
 (\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - AR	3256 -565	AO-AN	4080 -700
AR-AQ	3248 -565	AN-AM	4080 -700
AQ-AP	4141 -712	AM-AL	3072 -534
AP-AO	4364 -742		

**Nailnote**

Nail Schedule: 0.128"x3", min. nails  
 Top Chord: 1 Row @ 11.00" o.c.  
 Bot Chord: 1 Row @ 12.00" o.c.  
 Webs: 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Loading**

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specifically noted.  
 Laterally brace top chord to building.

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C - AQ	1124 -180	AM-AI	1259 -203
AQ - I	125 -509	AI-AL	421 -2135
L - M	532 -84	AL-AJ	3738 -645
Z - AA	547 -89	AJ-AK	435 -2453
AE - AM	146 -632		

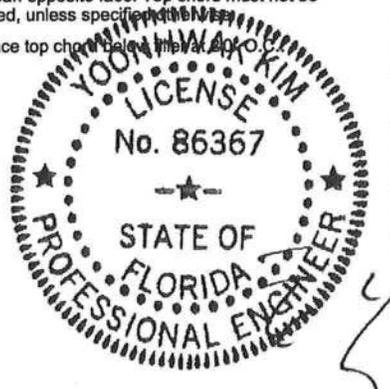
**Special Loads**

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 64 plf at -1.50 to 64 plf at 6.50  
 TC: From 32 plf at 6.50 to 32 plf at 38.54  
 BC: From 5 plf at -1.50 to 5 plf at 0.00  
 BC: From 20 plf at 0.00 to 20 plf at 6.53  
 BC: From 10 plf at 6.53 to 10 plf at 38.54  
 TC: 234 lb Conc. Load at 6.53  
 TC: 178 lb Conc. Load at 8.56, 10.56, 12.56, 14.56, 16.56, 18.56, 20.56, 22.56, 24.56, 26.56, 28.56, 30.56, 32.56, 34.56, 36.56  
 BC: 416 lb Conc. Load at 6.53  
 BC: 121 lb Conc. Load at 8.56, 10.56, 12.56, 14.56, 16.56, 18.56, 20.56, 22.56, 24.56, 26.56, 28.56, 30.56, 32.56, 34.56, 36.56

**Wind**

Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
 08/18/2020

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**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=38'3"8 uses the following support conditions: 38'3"8

Bearing AK (38'3"8, 9'1"2) HGUS28-2

Supporting Member: (3)2x8 SP 2400f-2.0E

(36) 0.148"x3" nails into supporting

member,

(12) 0.148"x3" nails into supported

member.

**Additional Notes**

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 10-0-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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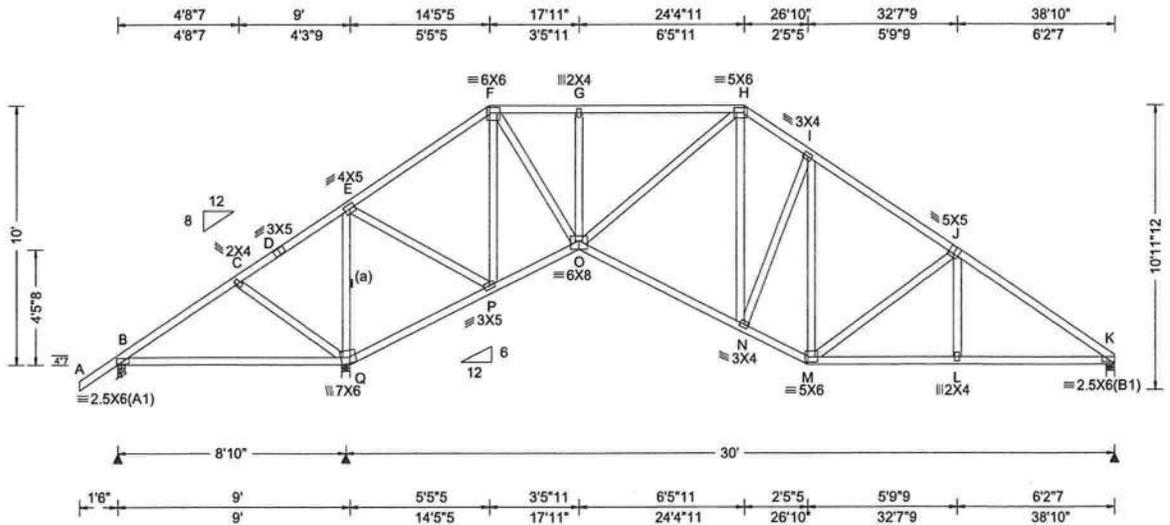
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6750 Forum Drive  
Suite 305  
Orlando FL, 32821



<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.88 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg, Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.055 N 999 360 VERT(CL): 0.118 N 999 240 HORZ(LL): 0.039 L - - HORZ(TL): 0.082 L - - Creep Factor: 2.0 Max TC CSI: 0.541 Max BC CSI: 0.540 Max Web CSI: 0.946  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 243 /-180 /- /58 /23 /316 Q 2232 /- /- /1331 /75 /- K 1144 /- /- /760 /44 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 Q Brg Width = 4.0 Min Req = 2.6 K Brg Width = 4.0 Min Req = 1.5 Bearings B, Q, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 645 -86 G - H 284 -1053 C - D 733 -97 H - I 391 -1146 D - E 827 -86 I - J 372 -1254 E - F 194 -578 J - K 391 -1695 F - G 284 -1053  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - Q 128 -497 N - M 1084 -108 Q - P 272 -756 M - L 1325 -234 P - O 453 -68 L - K 1327 -234 O - N 1062 -39  <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. Webs Tens. Comp. E - Q 306 -1573 F - O 1200 -142 E - P 1167 -122 M - J 182 -464 F - P 122 -1008
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

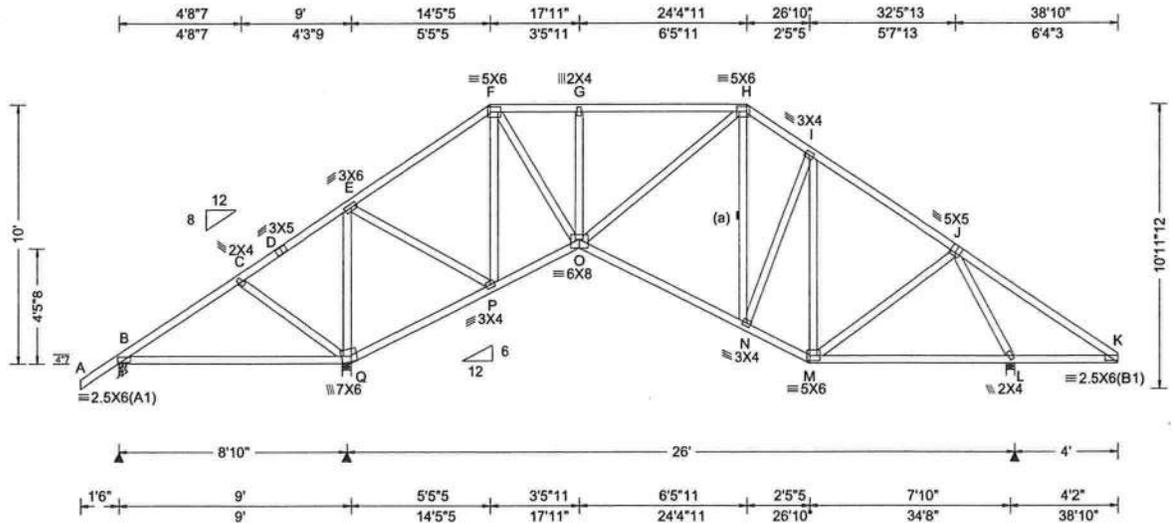
**Additional Notes**  
Negative reaction(s) of -180# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 10'-0-0.



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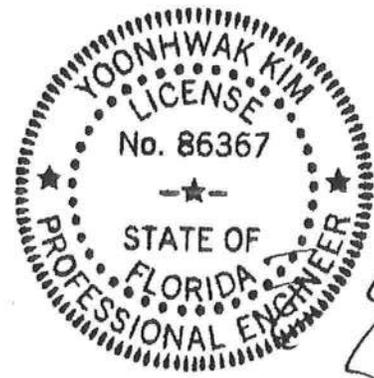
<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.88 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.037 G 999 360 VERT(CL): 0.079 G 999 240 HORZ(LL): 0.029 L - - HORZ(TL): 0.063 L - - Creep Factor: 2.0 Max TC CSI: 0.681 Max BC CSI: 0.548 Max Web CSI: 0.932  VIEW Ver: 19.02.02B.0122.15	<b>Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 331 /-14 /- /125 /77 /316 Q 1847 /- /- /1233 /- /- L 1380 /- /- /1030 /- /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 Q Brg Width = 4.0 Min Req = 2.2 L Brg Width = 4.0 Min Req = 1.5 Bearings B, Q, & L are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right cantilever is exposed to wind

**Additional Notes**  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 10'-0-0.



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C - D	448	-78	G - H	0	-929
D - E	542	-67	H - I	155	-852
E - F	68	-606	I - J	138	-869
F - G	0	-929	J - K	425	-228

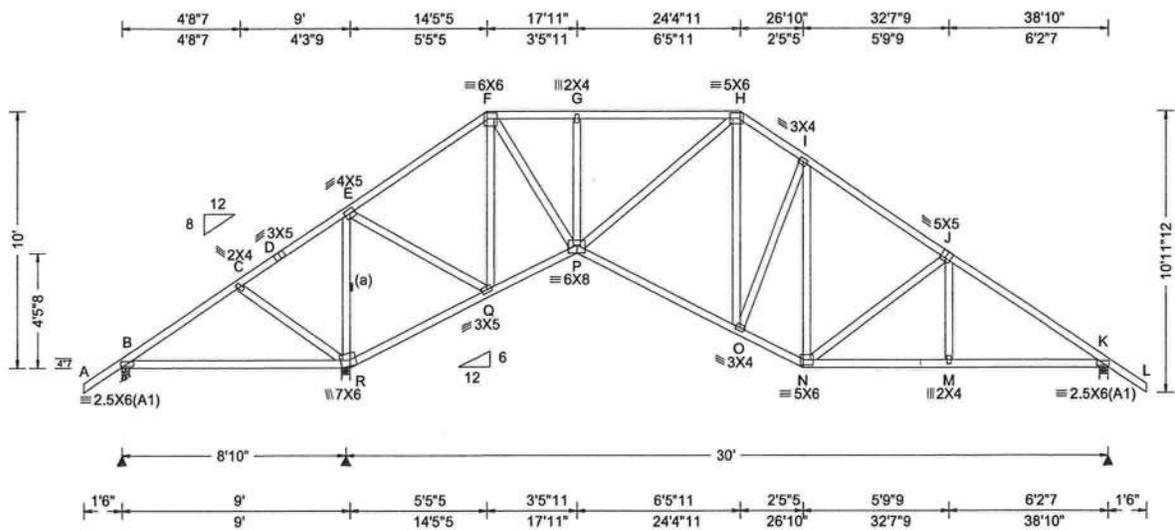
<b>Maximum Bot Chord Forces Per Ply (lbs)</b>					
Chords Tens.Comp.		Chords Tens. Comp.			
Q - P	88	-486	N - M	718	0
P - O	481	-54	M - L	401	-1
O - N	785	0			

<b>Maximum Web Forces Per Ply (lbs)</b>					
Webs Tens.Comp.		Webs Tens. Comp.			
E - Q	0	-1317	F - O	951	0
E - P	925	0	M - I	0	-402
F - P	0	-783	J - L	243	-1342

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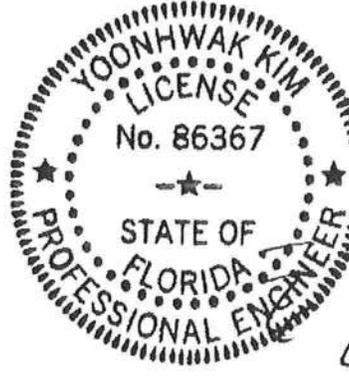
<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.88 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.056 O 999 360 VERT(CL): 0.119 O 999 240 HORZ(LL): 0.038 M - - HORZ(TL): 0.081 M - - Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.540 Max Web CSI: 0.945  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 241 /-183 /- /54 /32 /335 R 2233 /- /- /1343 /56 /- K 1250 /- /- /857 /58 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 R Brg Width = 4.0 Min Req = 2.6 K Brg Width = 4.0 Min Req = 1.5 Bearings B, R, & K are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 651 -81 G - H 259 -1044 C - D 739 -72 H - I 388 -1138 D - E 833 -61 I - J 372 -1243 E - F 195 -571 J - K 372 -1671 F - G 259 -1044
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Negative reaction(s) of -183# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 10-0-0.

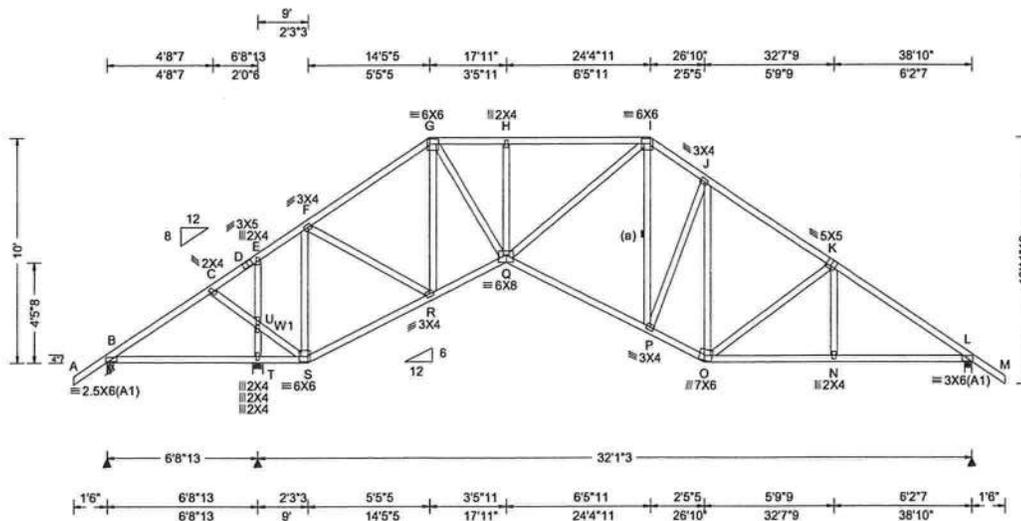


FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

<b>Maximum Bot Chord Forces Per Ply (lbs)</b>			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	138 -502	O - N	1075 -78
R - Q	282 -762	N - M	1301 -195
Q - P	446 -108	M - K	1302 -195
P - O	1053 -5		
<b>Maximum Web Forces Per Ply (lbs)</b>			
Webs	Tens.Comp.	Webs	Tens. Comp.
E - R	275 -1571	F - P	1194 -119
E - Q	1166 -100	N - J	157 -443
F - Q	102 -1007		

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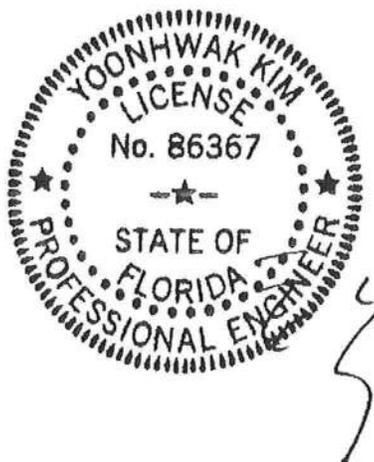
<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.88 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.144 H 999 360 VERT(CL): 0.294 H 999 240 HORIZ(LL): 0.093 N - - HORIZ(TL): 0.196 N - - Creep Factor: 2.0 Max TC CSI: 0.743 Max BC CSI: 0.696 Max Web CSI: 0.878  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 939 /- /- /546 /26 /335 T 987 /- /- /648 /15 /- L 1586 /- /- /998 /45 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 T Brg Width = 5.7 Min Req = 1.5 L Brg Width = 4.0 Min Req = 1.9 Bearings B, T, & L are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3; W1 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 10'-0-0.

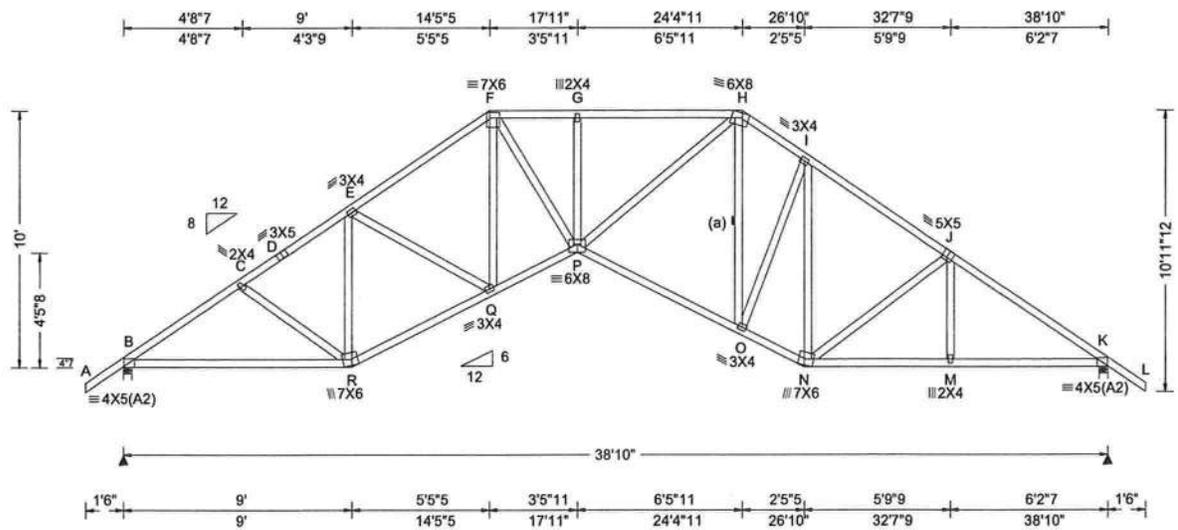


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<b>Maximum Bot Chord Forces Per Ply (lbs)</b>			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - T	854 -176	Q - P	1688 -137
T - S	854 -176	P - O	1627 -194
S - R	1153 -137	O - N	1780 -295
R - Q	1760 -92	N - L	1782 -295
<b>Maximum Web Forces Per Ply (lbs)</b>			
Webs	Tens.Comp.	Webs	Tens. Comp.
U - E	112 -500	G - Q	1452 -175
T - U	163 -752	Q - I	1138 -89
F - S	102 -818	O - J	36 -398
F - R	676 0	O - K	153 -422
G - R	0 -488		

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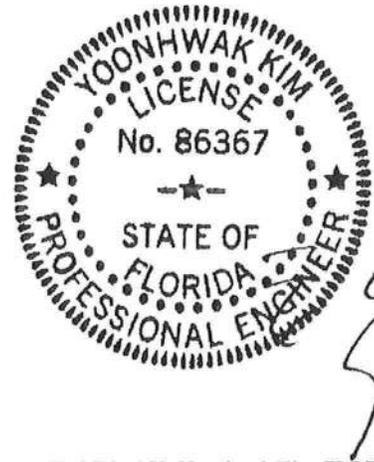
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**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Wind**  
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**Additional Notes**  
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 The overall height of this truss excluding overhang is 10-0-0.



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**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	2042 - 334	O - N	1904 - 241
R - Q	2124 - 291	N - M	2021 - 337
Q - P	2410 - 203	M - K	2023 - 336
P - O	2007 - 192		

**Maximum Web Forces Per Ply (lbs)**

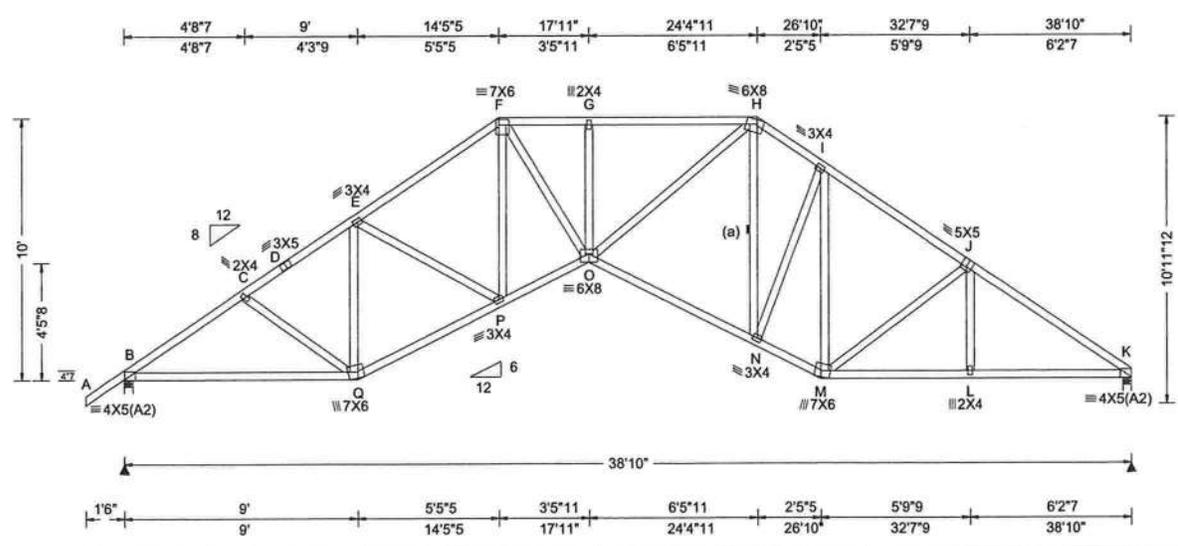
Webs	Tens.Comp.	Webs	Tens. Comp.
E - R	103 - 684	N - I	30 - 525
F - P	1621 - 203	N - J	151 - 411
P - H	1633 - 174		

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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
Refer to DWG PB160101014 for piggyback details.  
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**Maximum Bot Chord Forces Per Ply (lbs)**

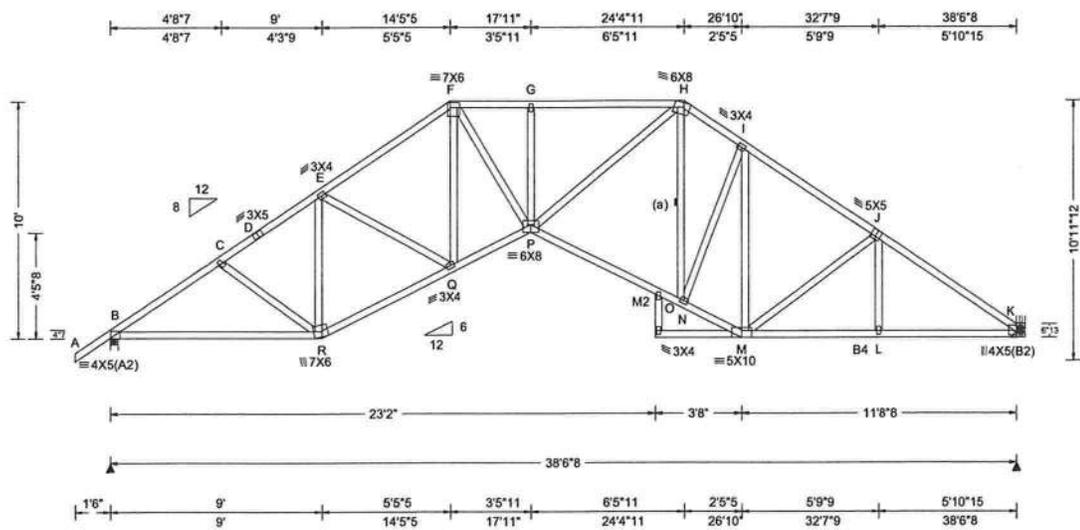
Chords	Tens.Comp.	Chords	Tens. Comp.
B - Q	2045 -391	N - M	1913 -278
Q - P	2128 -353	M - L	2045 -381
P - O	2415 -272	L - K	2047 -381
O - N	2014 -234		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
E - Q	128 -686	M - I	41 -519
F - O	1627 -230	M - J	176 -432
O - H	1634 -216		

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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg, Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.85 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.193 G 999 360 VERT(CL): 0.408 G 999 240 HORZ(LL): 0.126 L - - HORZ(TL): 0.266 L - - Creep Factor: 2.0 Max TC CSI: 0.740 Max BC CSI: 0.827 Max Web CSI: 0.663  VIEW Ver: 19.02.02B.0122.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1751 /- /- /1075 /38 /324 K 1633 /- /- /970 /27 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 2.1 K Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 541 -2538 G - H 696 -3020 C - D 519 -2317 H - I 597 -2150 D - E 536 -2258 I - J 547 -2111 E - F 624 -2668 J - K 547 -2474 F - G 696 -3020

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2; B4 2x4 SP M-31;  
Webs: 2x4 SP #3; M2 2x4 SP #2;  
Filler: 2x4 SP #2;

**Bracing**  
(a) Continuous lateral restraint equally spaced on member.

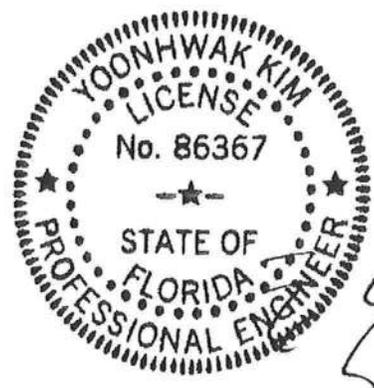
**Plating Notes**  
All plates are 2X4 except as noted.

**Hangers / Ties**  
(J) Hanger Support Required, by others

**Purlins**  
Laterally brace BC at 24" oc in lieu of rigid ceiling.  
Laterally brace BC above filler at 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
The overall height of this truss excluding overhang is 10'-0-0.



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**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - R	2033 -403	O - N	1917 -247
R - Q	2114 -367	N - M	1874 -273
Q - P	2396 -294	M - L	1958 -369
P - O	1986 -246	L - K	1958 -368

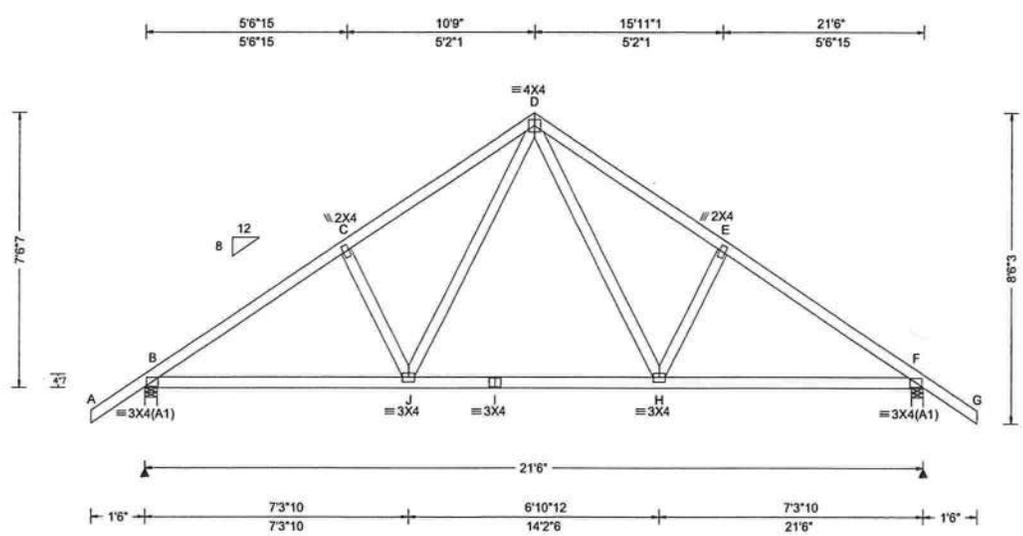
**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
E - R	135 -679	P - H	1631 -227
F - P	1608 -238	M - I	48 -532

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SEQN: 362220 / FROM: CDM	COMN Ply: 1 Qty: 11	Job Number: 20-4510 Lot 41 JL Truss Label: C01	Cust: R 215 JRef: 1WXX2150001 T1 DrawNo: 231.20.1149.52826 KD / YK 08/18/2020
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.039 J 999 360 VERT(CL): 0.076 J 999 240 HORZ(LL): 0.017 H - - HORZ(TL): 0.033 H - - Creep Factor: 2.0 Max TC CSI: 0.302 Max BC CSI: 0.583 Max Web CSI: 0.206  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1068 /- /- /622 /166 /251 F 1068 /- /- /511 /166 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 F Brg Width = 4.0 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 340 -1374 D - E 400 -1227 C - D 401 -1228 E - F 339 -1373
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
The overall height of this truss excluding overhang is 7-6-7.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	1065 -153	I - H	724 -7
J - I	724 -7	H - F	1065 -162

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
J - D	511 -167	D - H	510 -167

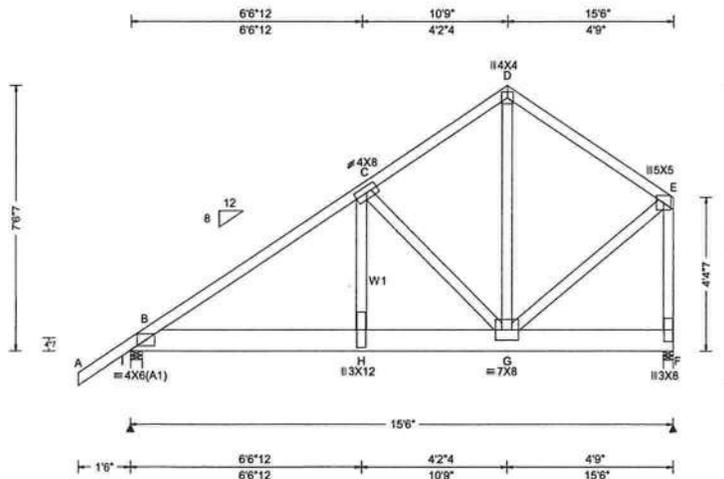


FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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3 Complete Trusses Required



<b>Loading Criteria (psf)</b> TCELL: 20.00 TCDDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.055 H 999 360 VERT(CL): 0.110 H 999 240 HORZ(LL): -0.012 D - - HORZ(TL): 0.024 D - - Creep Factor: 2.0 Max TC CSI: 0.345 Max BC CSI: 0.231 Max Web CSI: 0.652  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b>																																																																																														
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>I</td> <td>5428</td> <td>-</td> <td>-</td> <td>-</td> <td>/914</td> <td>-</td> </tr> <tr> <td>F</td> <td>7719</td> <td>-</td> <td>-</td> <td>-</td> <td>/1056</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS            I Brg Width = 4.0 Min Req = 1.5            F Brg Width = 3.5 Min Req = 2.1            Bearings I &amp; F are a rigid surface.            Members not listed have forces less than 375#  <b>Maximum Top Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th>B - C</th> <th>D - E</th> <th>C - D</th> <th></th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>503</td> <td>-3013</td> <td>D - E</td> <td>261</td> <td>-1620</td> </tr> <tr> <td>C - D</td> <td>261</td> <td>-1618</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p><b>Maximum Bot Chord Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th rowspan="2">Chords</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Chords</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th>B - H</th> <th>H - G</th> <th>H - G</th> <th></th> </tr> </thead> <tbody> <tr> <td>B - H</td> <td>2477</td> <td>-408</td> <td>H - G</td> <td>2449</td> <td>-404</td> </tr> </tbody> </table> <p><b>Maximum Web Forces Per Ply (lbs)</b></p> <table border="1"> <thead> <tr> <th rowspan="2">Webs</th> <th colspan="2">Tens.Comp.</th> <th rowspan="2">Webs</th> <th colspan="2">Tens. Comp.</th> </tr> <tr> <th>H - C</th> <th>G - E</th> <th>C - G</th> <th>E - F</th> </tr> </thead> <tbody> <tr> <td>H - C</td> <td>1839</td> <td>-297</td> <td>G - E</td> <td>1711</td> <td>-270</td> </tr> <tr> <td>C - G</td> <td>280</td> <td>-1623</td> <td>E - F</td> <td>327</td> <td>-2013</td> </tr> <tr> <td>D - G</td> <td>1664</td> <td>-243</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	I	5428	-	-	-	/914	-	F	7719	-	-	-	/1056	-	Chords	Tens.Comp.		Chords	Tens. Comp.		B - C	D - E	C - D		B - C	503	-3013	D - E	261	-1620	C - D	261	-1618				Chords	Tens.Comp.		Chords	Tens. Comp.		B - H	H - G	H - G		B - H	2477	-408	H - G	2449	-404	Webs	Tens.Comp.		Webs	Tens. Comp.		H - C	G - E	C - G	E - F	H - C	1839	-297	G - E	1711	-270	C - G	280	-1623	E - F	327	-2013	D - G	1664
Loc	Gravity			Non-Gravity																																																																																														
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C - G	280	-1623	E - F	327	-2013																																																																																													
D - G	1664	-243																																																																																																

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x8 SP 2400f-2.0E;  
 Webs: 2x4 SP #3; W1 2x4 SP #2;

**Nailnote**  
 Nail Schedule: 0.128"x3", min. nails  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 2 Rows @ 4.00" o.c. (Each Row)  
 Webs : 1 Row @ 4" o.c.  
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 --- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
 TC: From 64 plf at -1.50 to 64 plf at 15.50  
 BC: From 5 plf at -1.50 to 5 plf at 0.00  
 BC: From 20 plf at 0.00 to 20 plf at 6.56  
 BC: From 10 plf at 6.56 to 10 plf at 15.50  
 BC: 5023 lb Conc. Load at 6.56 +  
 BC: 1725 lb Conc. Load at 8.56, 10.56, 12.56  
 BC: 1633 lb Conc. Load at 14.56

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Right end vertical not exposed to wind pressure.

**Additional Notes**  
 The overall height of this truss excluding overhang is 7'-6".

+ PROVIDE (2) 0.131"x3.0" GUN NAILS IN AREA OF CONCENTRATED LOAD OPPOSITE HANGER, WITHOUT SPLITTING LUMBER.



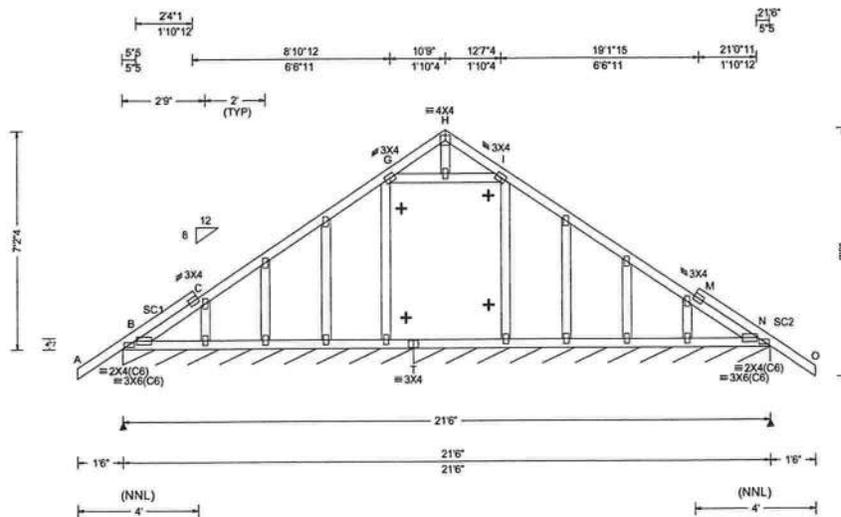
FL REG# 278, Yoonhwak Kim, FL PE #86367  
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *PLF</b>
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00  Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 H 999 360 VERT(CL): 0.005 H 999 240 HORZ(LL): 0.001 L - - HORZ(TL): 0.002 L - - Creep Factor: 2.0 Max TC CSI: 0.273 Max BC CSI: 0.108 Max Web CSI: 0.117  VIEW Ver: 19.02.02B.0122.15	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL  B* 129 /- /- /71 /- /6 T* 105 /- /- /56 /- /- Wind reactions based on MWFRS B Brg Width = 116 Min Req = - T Brg Width = 142 Min Req = - Bearings B & T are a rigid surface. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

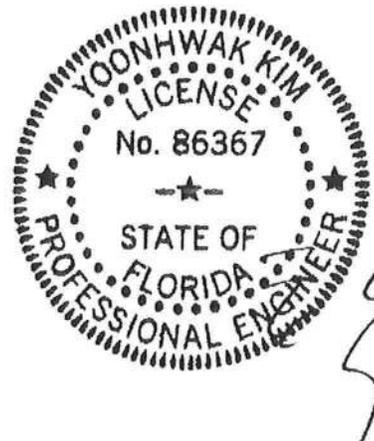
**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
See DWGS A14015ENC101014 & GBLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
The overall height of this truss excluding overhang is 7-2-4.

+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

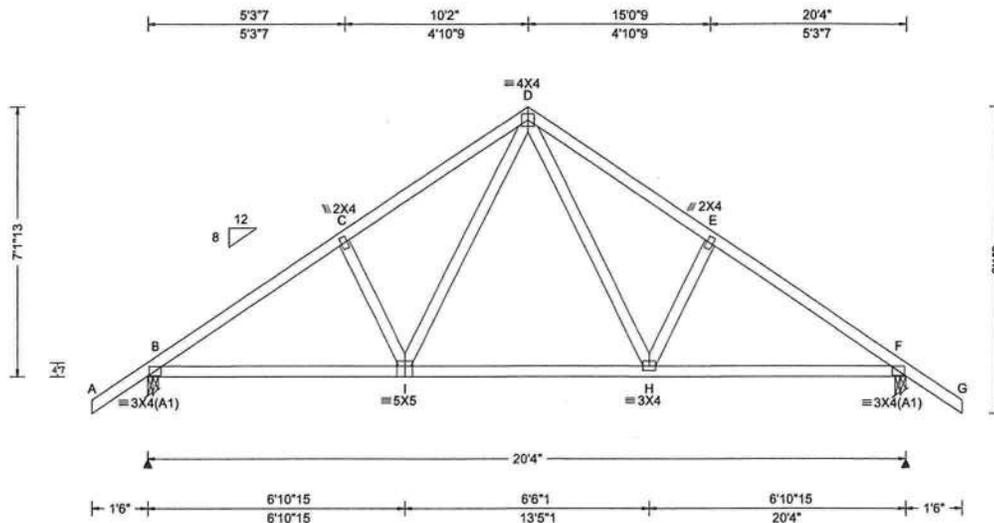


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SEQN: 362242 FROM: CDM	COMN Ply: 1 Qty: 2	Job Number: 20-4510 Lot 41 JL Truss Label: D01	Cust: R215 JRef:1WXX2150001 T6 DrwNo: 231.20.1153.36390 KD / YK 08/18/2020
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Def/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.035 H 999 360 VERT(CL): 0.068 H 999 240 HORZ(LL): 0.015 H - - HORZ(TL): 0.029 H - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.526 Max Web CSI: 0.180  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b>																																																																																								
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I - D	471	-157	D - H	473	-156																																																																																							

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Loading**  
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
The overall height of this truss excluding overhang is 7'-11.3".



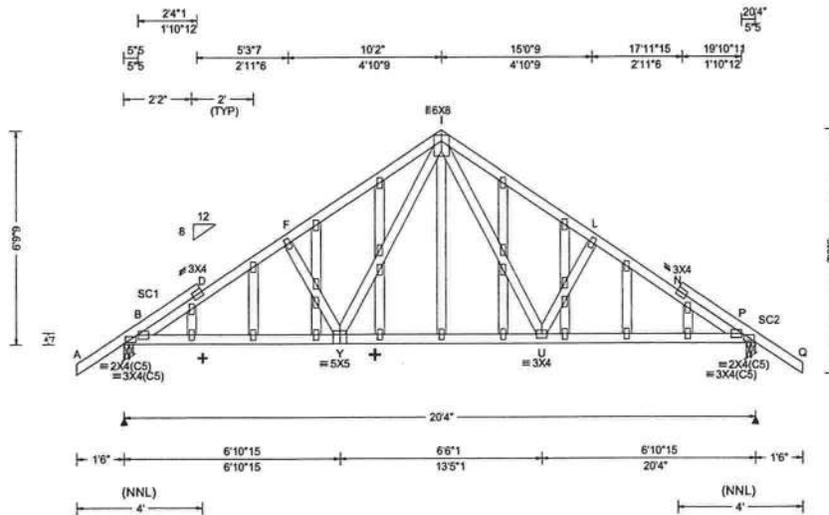
FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.047 AA 999 360 VERT(CL): 0.105 AA 999 240 HORZ(LL): 0.023 D - - HORZ(TL): 0.053 D - - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.584 Max Web CSI: 0.333  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ /R- /Rh /Rw /U /RL Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1176 /- /- /649 /- /60 P 1176 /- /- /649 /- /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 P Brg Width = 3.5 Min Req = 1.5 Bearings B & P are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - D 60 -442 I - L 229 -1392 B - D 49 -1165 L - N 132 -1513 D - F 71 -1513 N - P 0 -442 F - I 85 -1391 N - P 156 -1165
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;  
Stack Chord: SC1 2x4 SP #2;  
Stack Chord: SC2 2x4 SP #2;

**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

**Purlins**  
In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

**Additional Notes**  
See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.  
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

The overall height of this truss including overhang is 6'-9"-9".



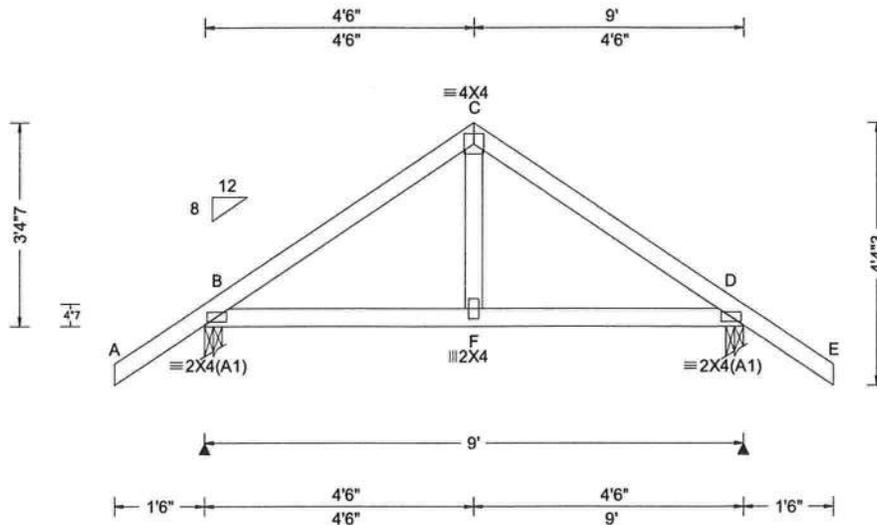
FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Def/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.004 F 999 360 VERT(CL): 0.008 F 999 240 HORZ(LL): 0.002 F - - HORZ(TL): 0.003 F - - Creep Factor: 2.0 Max TC CSI: 0.188 Max BC CSI: 0.186 Max Web CSI: 0.072  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 481 /- /- /319 /82 /135 D 481 /- /- /319 /82 /- Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 D Brg Width = 3.5 Min Req = 1.5 Bearings B & D are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 114 -395 C - D 115 -395
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

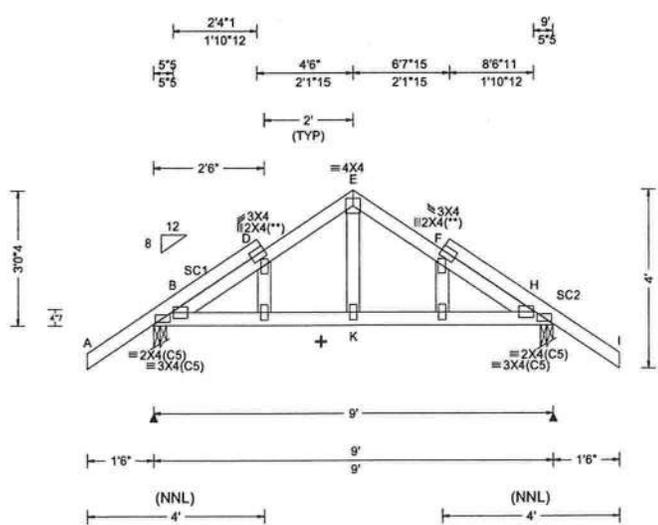
**Additional Notes**  
The overall height of this truss excluding overhang is 3-4-7.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
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<b>Loading Criteria (psf)</b> TCELL: 20.00 TCDDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.012 F 999 360 VERT(CL): 0.024 F 999 240 HORZ(LL): 0.006 D - - HORZ(TL): 0.012 D - - Creep Factor: 2.0 Max TC CSI: 0.304 Max BC CSI: 0.222 Max Web CSI: 0.069  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="2">Gravity</th> <th colspan="2">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw /U /RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>545</td> <td>-</td> <td>-</td> <td>/352 /10 /60</td> </tr> <tr> <td>H</td> <td>545</td> <td>-</td> <td>-</td> <td>/352 /10 /-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 3.5 Min Req = 1.5 H Brg Width = 3.5 Min Req = 1.5 Bearings B & H are a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>D - E</td> <td>91 -403</td> <td>E - F</td> <td>38 -403</td> </tr> </tbody> </table>	Loc	Gravity		Non-Gravity		R+	/R-	/Rh	/Rw /U /RL	B	545	-	-	/352 /10 /60	H	545	-	-	/352 /10 /-	Chords	Tens.Comp.	Chords	Tens. Comp.	D - E	91 -403	E - F	38 -403
Loc	Gravity		Non-Gravity																												
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D - E	91 -403	E - F	38 -403																												

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #3;  
 Stack Chord: SC1 2x4 SP #2;  
 Stack Chord: SC2 2x4 SP #2;

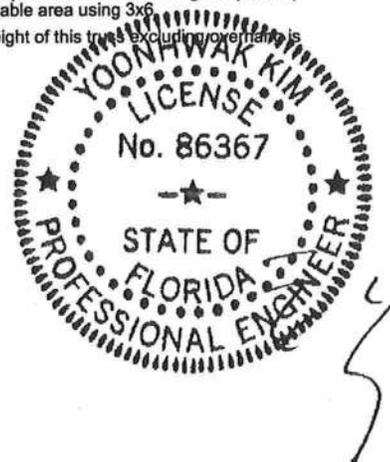
**Plating Notes**  
 All plates are 2X4 except as noted.  
 (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Loading**  
 Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

**Purlins**  
 In lieu of structural panels use purlins to brace TC @ 24" oc.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 + Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

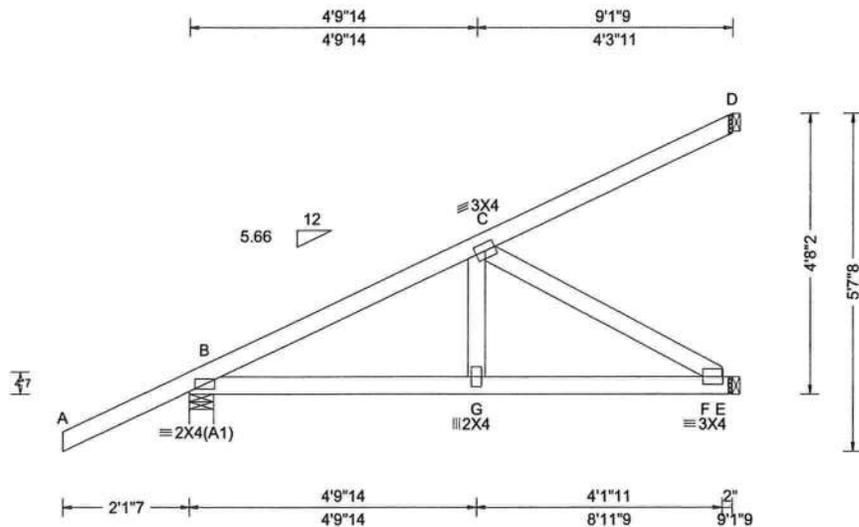
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 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.  
 The overall height of this truss excluding overhang is 3-0-4.



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.010 G 999 360 VERT(CL): 0.022 G 999 240 HORZ(LL): 0.003 F - - HORZ(TL): 0.007 F - - Creep Factor: 2.0 Max TC CSI: 0.414 Max BC CSI: 0.432 Max Web CSI: 0.245  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 372 /- /- /- /59 /- E 295 /- /- /- /52 /- D 57 /- /- /- /22 /- Wind reactions based on MWFRS B Brg Width = 4.9 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 104 -520
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Special Loads**  
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)  
TC: From -0 plf at -2.12 to 62 plf at 0.00  
TC: From 2 plf at 0.00 to 2 plf at 9.13  
BC: From 0 plf at -2.12 to 4 plf at 0.00  
BC: From 20 plf at 0.00 to 20 plf at 3.50  
BC: From 2 plf at 3.50 to 2 plf at 9.13  
TC: 86 lb Conc. Load at 3.50  
TC: 229 lb Conc. Load at 6.33  
BC: 78 lb Conc. Load at 3.50  
BC: 160 lb Conc. Load at 6.33

**Wind**  
Wind loads and reactions based on MWFRS.

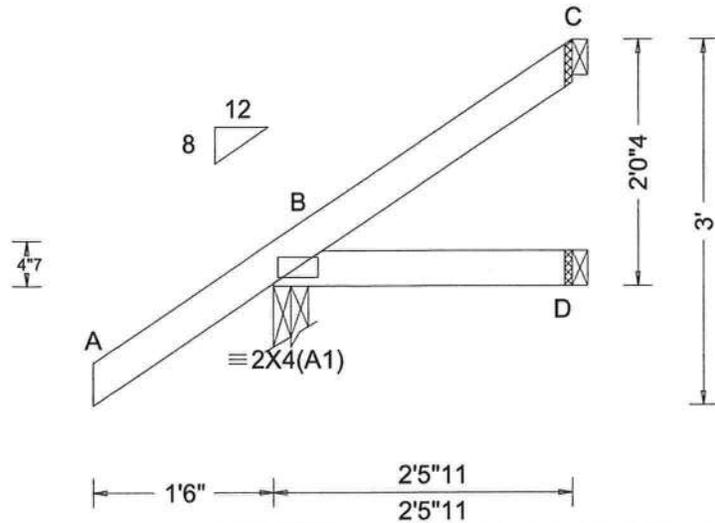
**Additional Notes**  
The overall height of this truss excluding overhang is 4-8-2.



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Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	253	/-	/-	/197	/36	/74																																
D	39	/-	/-	/33	/5	/-																																
C	43	/-	/-	/24	/23	/-																																

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
 The overall height of this truss excluding overhang is 2'-0-4.

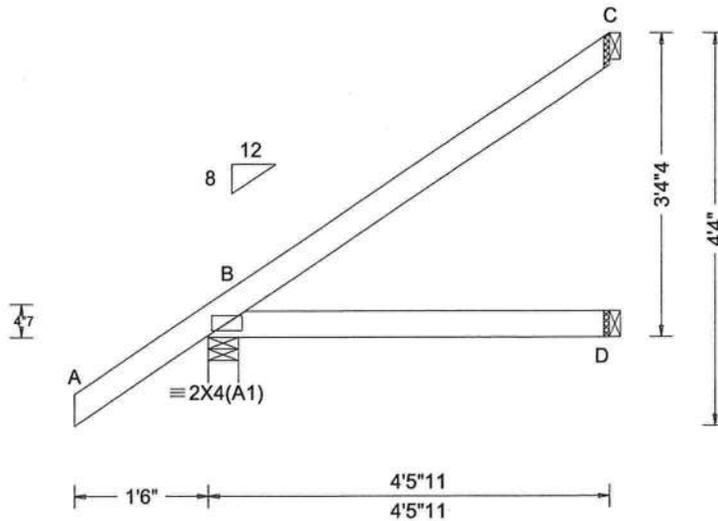


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SEQN: 362142 / FROM: CDM	JACK Ply: 1 Qty: 5	Job Number: 20-4510 Lot 41 JL Truss Label: J2	Cust: R215 JRef: 1WXX2150001 T12 DrwNo: 231.20.1153.59870 KD / YK 08/18/2020
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 D - - HORZ(TL): 0.005 D - - Creep Factor: 2.0 Max TC CSI: 0.244 Max BC CSI: 0.199 Max Web CSI: 0.000  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs)</b>																															
				<table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>319</td> <td>/-</td> <td>/-</td> <td>/235</td> <td>/30</td> <td>/113</td> </tr> <tr> <td>D</td> <td>80</td> <td>/-</td> <td>/-</td> <td>/58</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>115</td> <td>/-</td> <td>/-</td> <td>/64</td> <td>/52</td> <td>/-</td> </tr> </tbody> </table>		Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	319	/-	/-	/235	/30	/113	D	80	/-	/-	/58	/-	/-	C	115	/-
Loc	Gravity			Non-Gravity																															
	R+	/R-	/Rh	/Rw	/U	/RL																													
B	319	/-	/-	/235	/30	/113																													
D	80	/-	/-	/58	/-	/-																													
C	115	/-	/-	/64	/52	/-																													

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

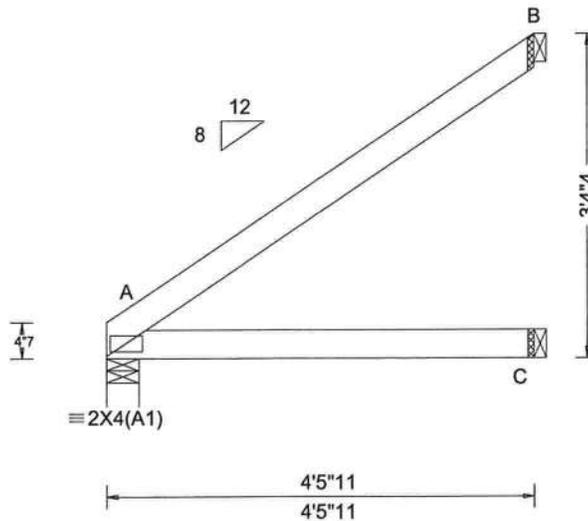
**Additional Notes**  
The overall height of this truss excluding overhang is 3-4-4.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
Loc	R+	/R-	/Rh	/Rw	/U	/RL				
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	A	194	-	-	127	-	184
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	C	84	-	-	162	12	-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B	127	-	-	176	157	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 C - -	Wind reactions based on MWFRS						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.009 C - -	A Brg Width = 4.0			Min Req = 1.5			
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 2017 RES	Creep Factor: 2.0	C Brg Width = 1.5			Min Req = -			
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.298	B Brg Width = 1.5			Min Req = -			
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.219	Bearing A is a rigid surface.						
Spacing: 24.0"	MWFRS Parallel Dist: 0 to h/2	FT/RT: 20(0)/10(0)	Max Web CSI: 0.000	Members not listed have forces less than 375#						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 19.02.02B.0122.15							
	Loc. from endwall: Any	WAVE								
	GCpi: 0.18									
	Wind Duration: 1.60									

**Lumber**

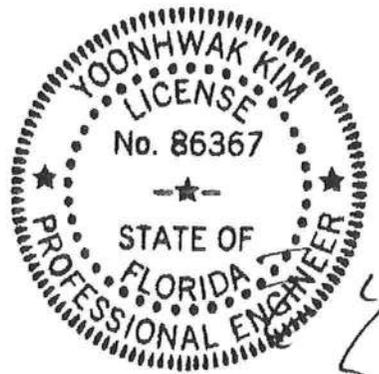
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**

The overall height of this truss excluding overhang is 3'-4".



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08/18/2020

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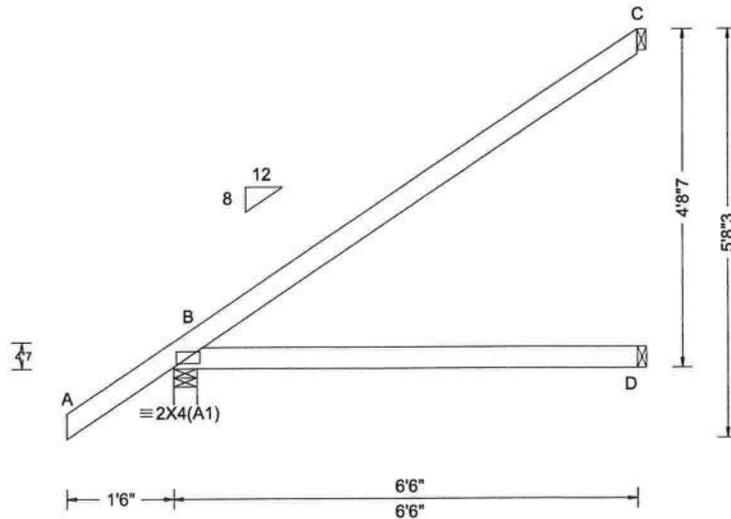
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6750 Forum Drive  
Suite 305  
Orlando FL, 32821



<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>																																		
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.010 D - - HORZ(TL): 0.021 D - - Creep Factor: 2.0 Max TC CSI: 0.626 Max BC CSI: 0.450 Max Web CSI: 0.000  VIEW Ver: 19.02.02B.0122.15	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>397</td> <td>-</td> <td>-</td> <td>/285</td> <td>/29</td> <td>/151</td> </tr> <tr> <td>D</td> <td>121</td> <td>-</td> <td>-</td> <td>/84</td> <td>/1</td> <td>-</td> </tr> <tr> <td>C</td> <td>178</td> <td>-</td> <td>-</td> <td>/104</td> <td>/79</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS            B Brg Width = 4.0 Min Req = 1.5            D Brg Width = 1.5 Min Req = -            C Brg Width = 1.5 Min Req = -            Bearing B is a rigid surface.            Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	397	-	-	/285	/29	/151	D	121	-	-	/84	/1	-	C	178	-	-	/104	/79	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	397	-	-	/285	/29	/151																																
D	121	-	-	/84	/1	-																																
C	178	-	-	/104	/79	-																																

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
 The overall height of this truss excluding overhang is 4-8-7.

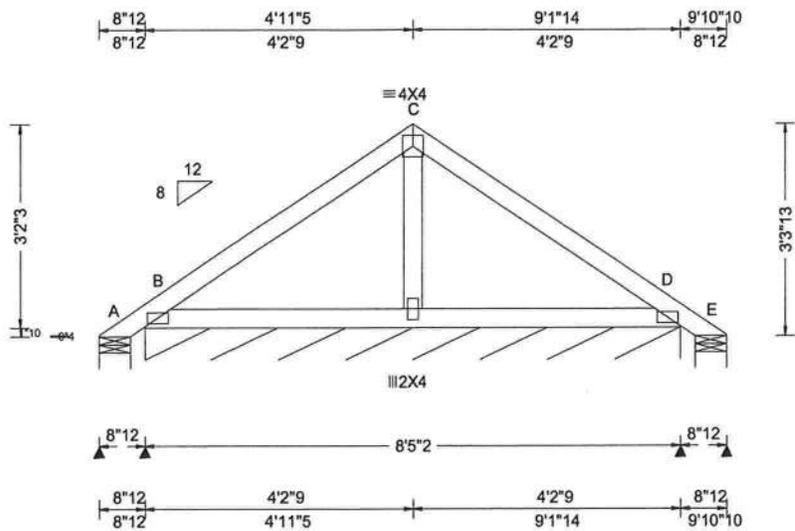


FL REG# 278, Yoonhwak Kim, FL PE #86367  
 08/18/2020

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SEQN: 362284 / FROM: CDM	GABL Ply: 1 Qty: 20	Job Number: 20-4510 Lot 41 JL Truss Label: P01	Cust: R 215 JRef: 1WXX2150001 T23 DrwNo: 231.20.1153.59838 KD / YK 08/18/2020
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pr: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Def/CSI Criteria</b> PP Deflection in loc L/def L/# VERT(LL): 0.001 F 999 360 VERT(CL): 0.002 F 999 240 HORZ(LL): -0.001 F - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.194 Max BC CSI: 0.090 Max Web CSI: 0.025  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-99</td> <td>/-</td> <td>/81</td> <td>/126</td> <td>/92</td> </tr> <tr> <td>B*</td> <td>100</td> <td>/-</td> <td>/-</td> <td>/61</td> <td>/4</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-99</td> <td>/-</td> <td>/31</td> <td>/75</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS  A Brg Width = 5.9 Min Req = 1.5  B Brg Width = 101 Min Req = -  E Brg Width = 5.9 Min Req = 1.5  Bearings A, B, &amp; E are a rigid surface.  Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-99	/-	/81	/126	/92	B*	100	/-	/-	/61	/4	/-	E	-	/-99	/-	/31	/75	/-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
A	-	/-99	/-	/81	/126	/92																																
B*	100	/-	/-	/61	/4	/-																																
E	-	/-99	/-	/31	/75	/-																																

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4(A1) except as noted.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
See DWGS A14030ENC101014 & GBULLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 13-3-13.

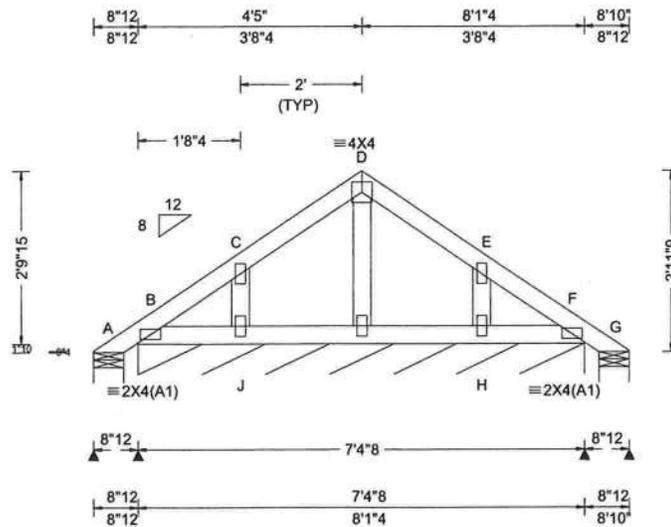


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SEQN: 362256 / FROM: CDM	GABL Ply: 1 Qty: 2	Job Number: 20-4510 Lot 41 JL Truss Label: P02	Cust: R 215 JRef: 1WXX2150001 T73 DrwNo: 231.20.1153.59714 KD / YK 08/18/2020
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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	<b>Wind Criteria</b> Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 360 VERT(CL): 0.000 D 999 240 HORZ(LL): 0.000 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.050 Max BC CSI: 0.019 Max Web CSI: 0.026  VIEW Ver: 19.02.02B.0122.15	<b>▲ Maximum Reactions (lbs), or *PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>17</td> <td>-</td> <td>-</td> <td>/54</td> <td>/40</td> <td>/82</td> </tr> <tr> <td>B*</td> <td>73</td> <td>-</td> <td>-</td> <td>/51</td> <td>-</td> <td>-</td> </tr> <tr> <td>G</td> <td>17</td> <td>-</td> <td>-</td> <td>/13</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Width = 5.9 Min Req = 1.5 B Brg Width = 88.5 Min Req = - G Brg Width = 5.9 Min Req = 1.5 Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	17	-	-	/54	/40	/82	B*	73	-	-	/51	-	-	G	17	-	-	/13	-	-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
A	17	-	-	/54	/40	/82																																
B*	73	-	-	/51	-	-																																
G	17	-	-	/13	-	-																																

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #3;

**Plating Notes**  
All plates are 2X4 except as noted.

**Loading**  
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.

**Additional Notes**  
See DWGS A14030ENC101014 & GBLETIN0118 for gable wind bracing and other requirements.  
Refer to DWG PB160101014 for piggyback details.  
The overall height of this truss excluding overhang is 12-11-9.



FL REG# 278, Yoonhwak Kim, FL PE #86367  
08/18/2020

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# Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Vertical Spacing	Gable Vertical Species	Brace		(1) 1x4 "L" Brace		(2) 2x4 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace	
			Grade	No Braces	Group A	Group B						
12" o.c.	SPF	#1 / #2	4' 3"	7' 3"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			4' 1"	6' 7"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
	HF	Stud	4' 1"	6' 7"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			4' 1"	5' 8"	7' 0"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
24" o.c.	SP	#1	4' 6"	7' 4"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
			4' 3"	7' 3"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
	DFL	Stud	4' 2"	6' 0"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			4' 2"	6' 0"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	4' 11"	8' 4"	9' 10"	10' 3"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 8"	8' 1"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1	5' 1"	8' 5"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			4' 11"	8' 4"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	Stud	4' 9"	7' 4"	8' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"
			4' 8"	6' 5"	8' 7"	9' 2"	11' 3"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	5' 1"	9' 0"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			5' 1"	9' 0"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	5' 1"	9' 0"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			5' 1"	9' 0"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SP	#1	5' 8"	9' 3"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			5' 3"	9' 2"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	Stud	5' 3"	8' 5"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			5' 3"	8' 5"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"

**Bracing Group Species and Grades:**

**Group A:**

- Spruce-Pine-Fir: #1 / #2 Standard, #3 Stud
- Hem-Fir: #2 Standard, #3 Stud
- Douglas Fir-Larch: #3 Standard, #3 Stud
- Southern Pinebeam: #3 Standard, #3 Stud

**Group B:**

- Hem-Fir: #1 & #2
- Douglas Fir-Larch: #1, #2
- Southern Pinebeam: #1, #2

1x4 Braces shall be SRB (Stress-Rated Board).  
 For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

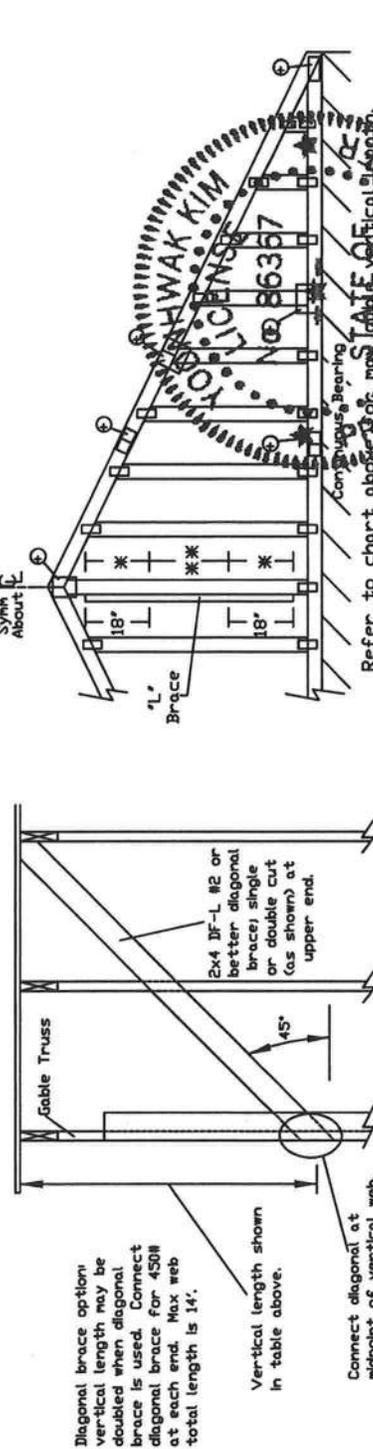
**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 55 psf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

**Gable Vertical Plate Sizes**

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



**ALPINE**  
 AN ITW COMPANY  
 514 Earth City Expressway  
 Suite 242  
 Earth City, MO 63045

Diagonal brace option vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.  
 Vertical length shown in table above.  
 Connect diagonal at midpoint of vertical web.

**IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ICCI Building Component Safety Information, by TPI and SCSA for safety practices prior to performing these functions. Installers shall provide temporary bracing per ICCI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing. ICCI details 833.17 or 810, as applicable, shall be used for truss and position as shown above and on the Joint Details, unless noted otherwise.  
 Refer to drawings 150M-2 for standard plate positions.  
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses.  
 The user of this drawing shall be responsible for obtaining the necessary engineering and engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see this job's general notes page and these web sites: 8/20/20  
 ALPINE: www.alpine.com TPI: www.tpi.org SCSA: www.scsa.org ICCI: www.icci.com

Refer to chart above for maximum allowable diagonal bearing.

MAX. TOT. L.D. 60 PSF  
 MAX. SPACING 24.0"

REF: ASCE7-10-GABI4015  
 DATE: 10/01/14  
 DRWG: A14015ENC101014

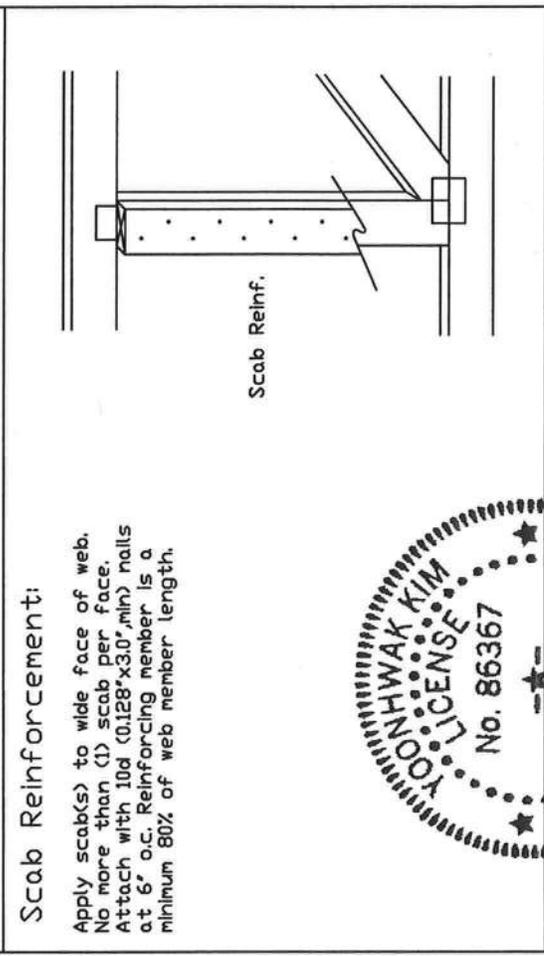
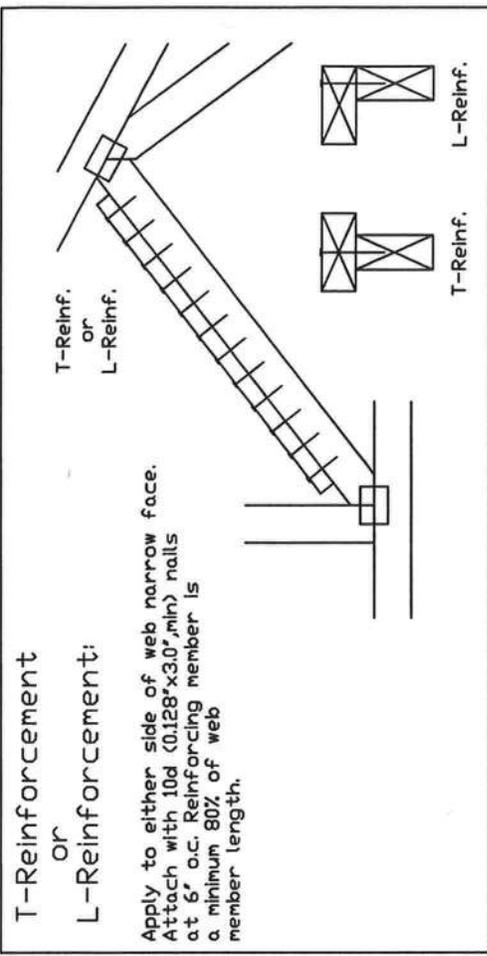
# CLR Reinforcing

# Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

### Notes:

- This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.
- Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.
- Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.



Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(Ⓢ)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(Ⓢ)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(Ⓢ) Center scab on wide face of web. Apply (1) scab to each face of web.



**IMPORTANT! READ AND FOLLOW ALL NOTES ON THIS DRAWING INCLUDING THE INSTALLER'S INSTRUCTIONS.**

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP Building Component Safety Information, by TPI and SCSA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached roof ceiling. Scabbing shall be performed on the lateral restraint of webs and shall be done in accordance with the instructions on the joint details, unless noted otherwise. Refer to drawings 150M-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses.

A seal on this drawing certifies that the design shown is the responsibility and use of the professional engineer whose name and seal are on this drawing. The seal shall be placed on the drawing for any structure. For more information see this job's general notes page and these web sites: [www.alpine.com](http://www.alpine.com) TPI: [www.tpiinc.org](http://www.tpiinc.org) SCSA: [www.scsa.com](http://www.scsa.com)

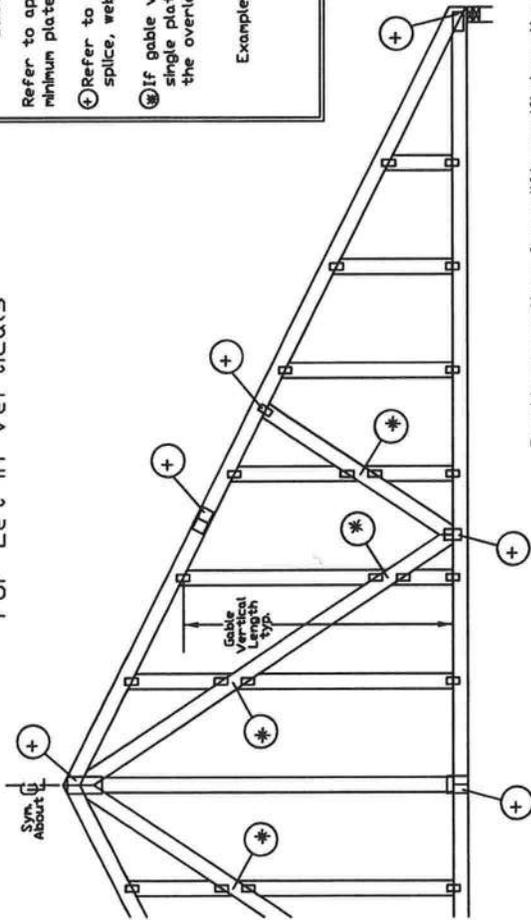
PSF	PSF	PSF	PSF	PSF
TC LL				
TZ DL				
BC DL				
SC LL				
TOT. LD.				
DUR. FAC.				
SPACING	SPACING	SPACING	SPACING	SPACING

REF CLR Subst.  
DATE 01/02/19  
DRWG BRCLBSUB0119



Yoonhwak Kim, FL PE #86367

# Gable Detail For Let-in Verticals



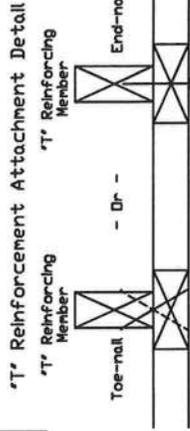
**Gable Truss Plate Sizes**

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X8



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

**Web Length Increase w/ 'T' Brace**

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:  
 ASCE 7-10 Wind Speed = 120 mph  
 Mean Roof Height = 30 ft, Kzt = 1.00  
 Gable Vertical = 24' o.c. SP #3  
 'T' Reinforcing Member Size = 2x4  
 'T' Brace Increase (From Above) = 30% = 1.30  
 (1) 2x4 'L' Brace Length = 8' 7"  
 Maximum 'T' Reinforced Gable Vertical Length = 1.30 x 8' 7" = 11' 2"

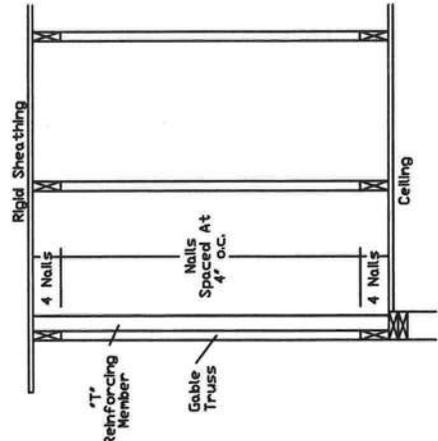
Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

**End Driven Nails:**  
 (1) Common (0.148" x 3", min) Nails at 4' o.c. plus  
 (4) nails in the top and bottom chords.

**Toenailed Nails:**  
 (1) Common (0.148" x 3", min) Toenails at 4' o.c. plus  
 (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.



- ASCE 7-05 Gable Detail Drawings  
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings  
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A20015ENC100118, A1530ENC100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A20030ENC100118, A18030ENC100118, A20030ENC100118, A20030ENC100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015ENC100118, S20015ENC100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030ENC100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

**STATE OF FLORIDA**  
**PROFESSIONAL ENGINEER**  
 Yoonhwak Kim, EIT #86367

**IMPORTANT! READ AND FOLLOW ALL NOTES ON THIS DRAWING TO THE INSTALLER.**

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of BC31 Building Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BC31. Unless noted otherwise, top chord shall have property attached structural sheathing and bottom chord shall have background structural sheathing. BC31 sections 35, 37 or 38, as applicable, apply plates to each face of truss and position as shown above and on the Job's Details, unless noted otherwise.

Refer to drawings 150A-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing or trusses.

Alpine and its representatives shall not be held responsible for the engineering, design or construction of any structure. The responsibility for the design and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: 8/20/20  
 ALPINE: www.alpinehwy.com | TPI: www.tpinet.com | SBCA: www.sbcaindustry.org | ICD: www.icd.com

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118
MAX. TOT. LD. 60 PSF	
DUR. FAC.	ANY
MAX. SPACING	24.0"

**ALPINE**  
 AN ITW COMPANY

514 Earth City Expressway  
 Suite 242  
 Earth City, MO 63045

# Piggyback Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

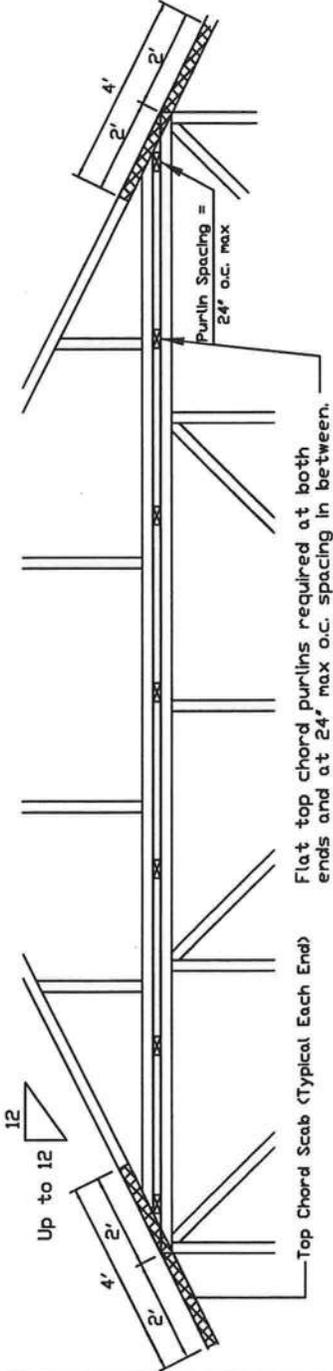
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0, Dr 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg, located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

## Detail A : Purlin Spacing = 24" o.c. or less

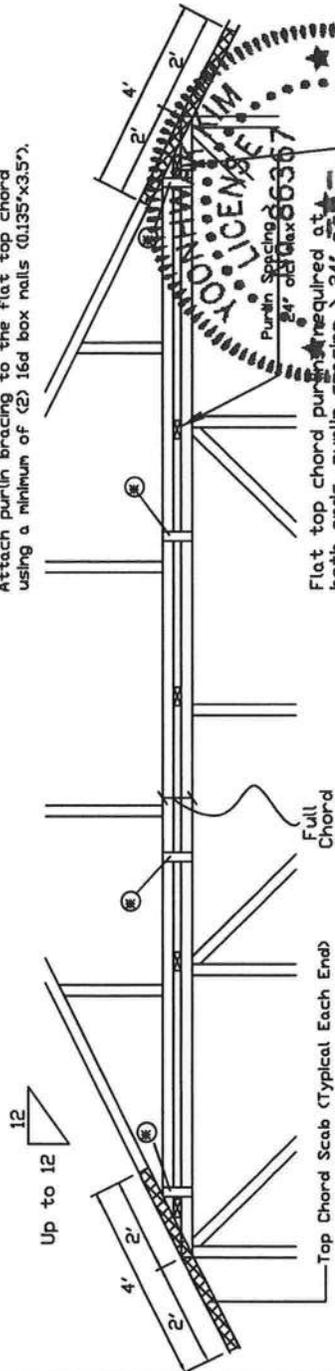


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 2x4 #3 wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

## Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

In addition, provide connection with one of the following methods:

**Trulox**  
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

**APA Rated Gusset**  
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

**2x4 Vertical Scabs**  
2x4 SPF #2, Full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

**2x4 Wave Piggyback Plate**  
One 2x4 wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

REF PIGGYBACK

DATE 10/01/14

DRWG PB160101014

SPACING 24.0'

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING BEFORE INSTALLING THE TRUSSES.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ICCI Guiding Component Safety Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per ICCI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per ICCI sections 4.1, 4.2 or 4.3, as applicable. Apply plates to each face per drawings 160A-2 for standard plate positions. Refer to drawings 160A-2 for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of trusses. A seal on this drawing solely for the use of the building designer and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: 8/2020 ALPINE: www.alpine.com TPI: www.tpinet.org SBCA: www.sbcaindustry.org ICD: www.icdinc.com



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Suite 242  
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Yoonhwak Kim, FL PE #86367

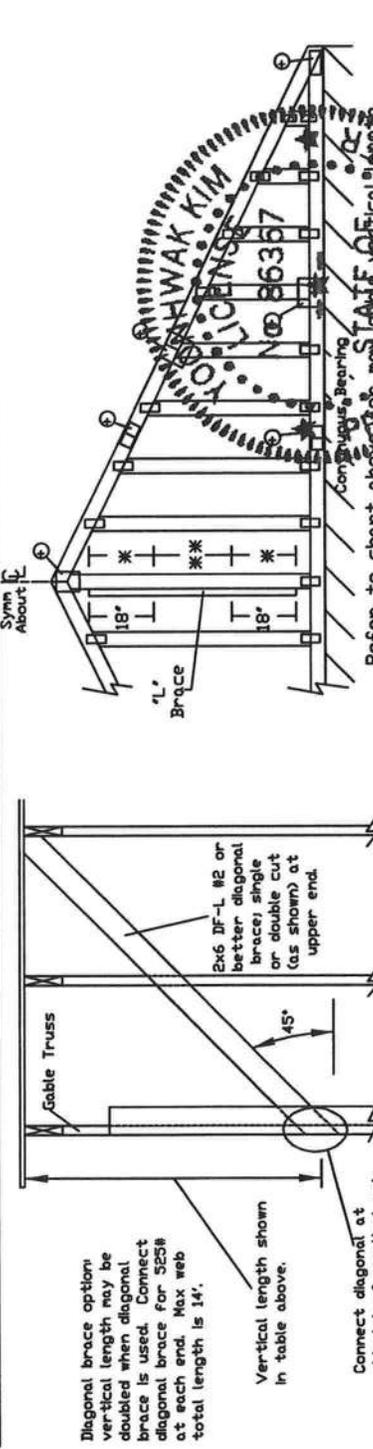


# Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00  
 Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00  
 Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	Gable Vertical Spacing	Species	Grade	No Braces		(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace			
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
24" O.C.	SPF	#1 / #2	Standard	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	
				3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"
				3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"
24" O.C.	HF	Standard	4' 2"	7' 0"	7' 3"	7' 0"	7' 6"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	
			4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	
			4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	
24" O.C.	SP	Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"	14' 0"	
			4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
16" O.C.	HF	Standard	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
16" O.C.	SP	Standard	4' 7"	6' 10"	7' 3"	8' 1"	8' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 5"	6' 10"	7' 3"	8' 1"	8' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	HF	Standard	4' 5"	6' 5"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"	14' 0"	
			5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	SP	Standard	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	DFL	Standard	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			6' 11"	7' 4"	7' 4"	9' 3"	9' 3"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			4' 10"	8' 4"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	



**Bracing Group Species and Grades:**

**Group A:**  
 Spruce-Pine-Fir: #1 / #2 Standard Stud, #3 Standard Stud  
 Hem-Fir: #2 Standard Stud, #3 Standard Stud  
 Douglas Fir-Larch: #3 Standard Stud, #3 Standard Stud

**Group B:**  
 Hem-Fir: #1, #2  
 Douglas Fir-Larch: #1, #2  
 Southern Pine: #1, #2

1x4 Braces shall be SRB (Stress-Rated Board).  
 For 1x4 So. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outcrookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x30" min) nails.  
 \* For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.  
 \* \* For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

**Gable Vertical Plate Sizes**

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF: ASCE7-10-GABI4030  
 DATE: 10/01/14  
 DRWG: A14030ENC101014

MAX. TOT. LD. 60 PSF  
 MAX. SPACING 24.0'

**ALPINE**  
 AN ITW COMPANY  
 514 Earth City Expressway  
 Suite 242  
 Earth City, MO 63045

Refer to chart above for maximum bearing. Refer to chart above for maximum bearing.

**IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING. THE INSTALLER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL CONNECTIONS INCLUDING THE INSTALLATION OF ALL BRACING. THE INSTALLER SHALL BE RESPONSIBLE FOR THE CORRECTNESS OF ALL CONNECTIONS INCLUDING THE INSTALLATION OF ALL BRACING.**

Trusses require erection by a qualified contractor. Refer to and follow the erection instructions, erection drawings, and bracing details. Refer to and follow the erection instructions, erection drawings, and bracing details. Refer to and follow the erection instructions, erection drawings, and bracing details.

Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per ICC sections 23.17 or 23.18, as applicable. Apply plates to each face of bracing. Refer to drawings 150A-2 for standard plate positions.

Alpha, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses.

A seal on this drawing or cover page listing the name of the professional engineer is required for any structure for which the engineer is responsible. The seal shall be placed on the drawing or cover page listing the name of the professional engineer.

For more information, visit [www.alpineinc.com](http://www.alpineinc.com) or call 1-800-368-2299.

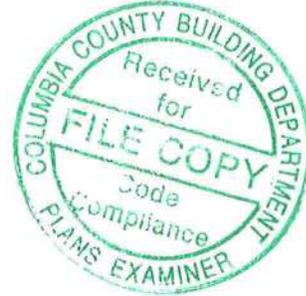
Yoonhwak Kim, PE #86367

Total Truss Quantity = 109.

W.B. Howland Truss Co.  
610 11th St. SW  
Live Oak, FL 32064  
(386) 362-1235  
(386) 362-7124 (Fax)  
howlandtruss@aol.com

ROOF PITCH: 8/12  
OVERHANG: 18" Plumb Typ  
12" ON GABLE ENDS  
CEILING: 6/12 @ GREAT ROOM & KITCHEN  
EXT. WALLS: 2 X 4 X 9'  
LOADING: 40 PSF  
WIND LOAD: 130 MPH  
EXPOSURE: C  
DATE: 8/18/20

5 - TRUSS TO TRUSS CONNECTION:  
4 HUS26  
1 HGUS28-2



JOB NO:  
20-4510

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1 OF 1

