

GENERAL NOTES

FABRICATION SHALL BE IN ACCORDANCE WITH ELITE STRUCTURES STANDARD PRACTICES AND IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE 'AWS STRUCTURAL WELDING CODE D 1.1 AND D1.3'

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
STEEL PIPES	A572	FY = 50 KSI
STRUCTURAL TUBING	A500	FY = 48 KSI
STRUCTURAL STEEL WEB PLATE	A572/A1011	FY = 46 KSI
STRUCTURAL STEEL FLANGE PLATES/BARS	A572/A572	FY = 50 KSI
COLD FORM LIGHT GAUGE	A653/A1011	FY = 55 KSI
ROOF AND WALL SHEETS	A792/A693	FY = 50, 55 KSI
CABLE BRACE	A475 - TYPE I	EXTRA HIGH STRENGTH
ROD BRACE	A36	FY = 36 KSI
HILL SECTIONS	A36	FY = 36 KSI
MACHINE BOLTS & NUTS	A307	MIN. TENSILE STRENGTH
HIGH STRENGTH BOLTS (1" DIA. & LESS)	A325 - TYPE 1	FU = 60 KSI
HIGH STRENGTH BOLTS (1" TO 1-1/2")	A325 - TYPE 1	FU = 80 KSI
ANCHOR BOLTS (IF SUPPLIED)	F1554 - GR55	FU = 105 KSI

CLOSURE STRIPS ARE FURNISHED ONLY IF NOTED ON SHIPPING DOCUMENTS

INSIDE - UNDER ROOF PANELS AT EAVE
OUTSIDE - BETWEEN END WALL PANELS AND RAKE TRIM
UNDER CONTINUOUS RIDGE VENT SKIRTS

ERECTION NOTE:

ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY ELITE STRUCTURES FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROVIDE AND INSTALL AS NEEDED.

ERECTION AND UNLOADING NOT BY ELITE STRUCTURES

SHORTAGES:

ANY CLAIMS OR SHORTAGES BY BUYERS MUST BE MADE TO ELITE STRUCTURES AT TIME OF DELIVERY, OR SUCH CLAIMS SHALL BE CONSIDERED WAIVED BY THE CUSTOMER AND DISALLOWED IF MATERIAL IS SHOWN DELIVERED

CORRECTIONS OF ERRORS AND REPAIRS (MRMA 6.102)

CLAIMS FOR CORRECTIONS OF ALLEGED MISFITS WILL BE DISALLOWED UNLESS ELITE STRUCTURES SHALL HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH MISFITS. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

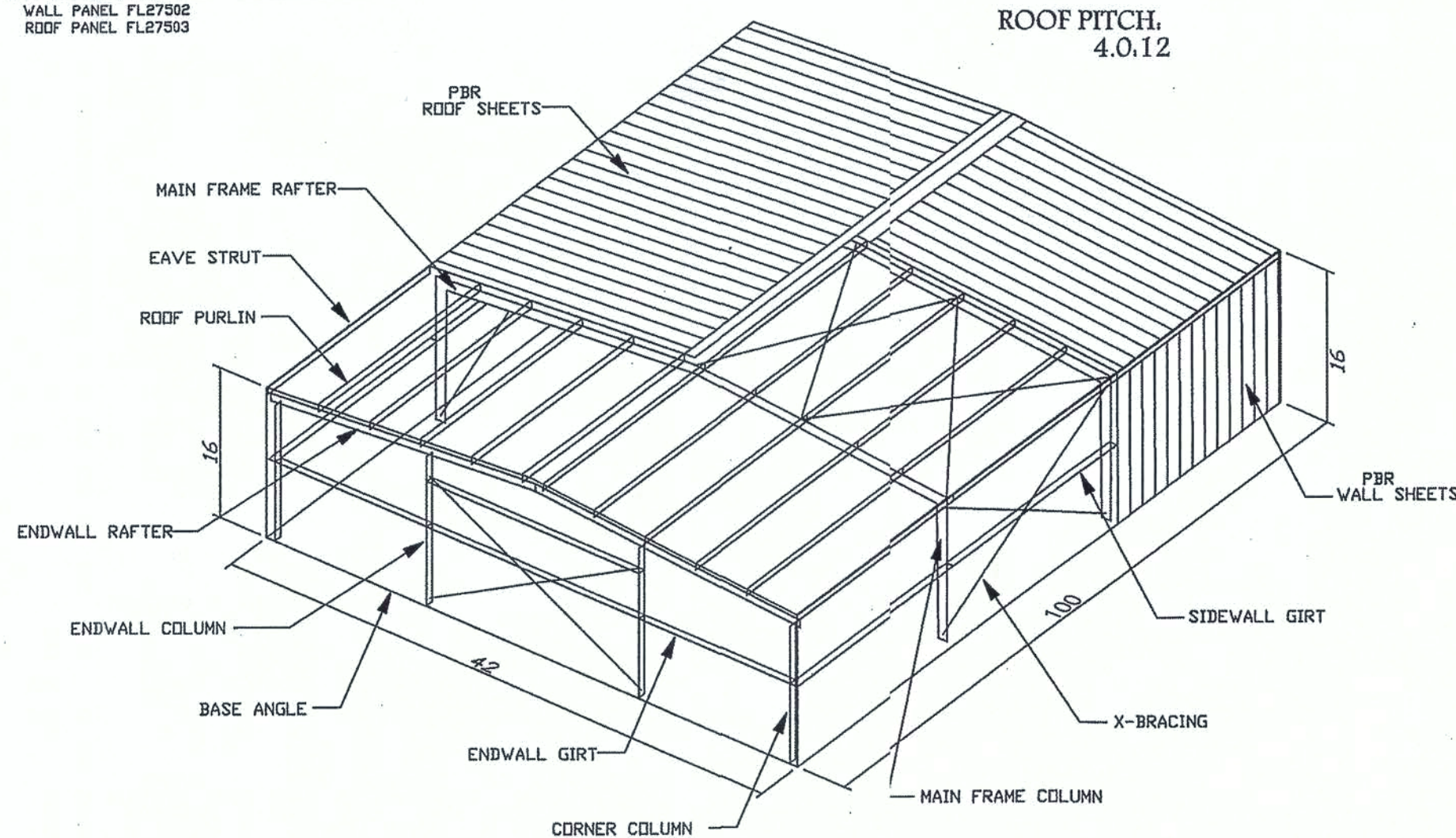
A-325 BOLT TIGHTENING REQUIREMENTS

ALL BOLTED JOINTS WITH A325 OR TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR TYPE 1 BOLTS, AISC 308M, PRETENSIONING METHODS, INCLUDING TURN-OF-NUT/CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL. BOLTS OR BUREST TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT (SPECIFICATION FOR STRUCTURAL JOINTS 9.1) IS SUGGESTED.

REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AISC)

FLORIDA PRODUCT APPROVAL NUMBER - ELITE STRUCTURES 26 GAUGE PBR PANEL

WALL PANEL FL27502
ROOF PANEL FL27503



BUILDING DRAWING FOR ILLUSTRATION ONLY



401 OLD QUITMAN RD.
ADEL, GA. 31620
229-896-7569



CONSTRUCTION MANUAL AVAILABLE ONLINE
WWW.ELITE-STRUCTURES.COM

PROJECT:

MARK SMITH

PROJECT MANAGER

JERRY RAGLAND

LOCATION:

LAKE CITY, FL.

DATE: 4/4/19



BUYER/CUSTOMER RESPONSIBILITIES

APPROVAL OF ELITE STRUCTURES DRAWINGS AND CALCULATIONS INDICATES THAT ELITE STRUCTURES HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNER ACCEPTANCE OF THE ELITE STRUCTURES DESIGN CONCEPTS, ASSUMPTIONS, AND LOADS.

ONCE THE BUYER/END USER CUSTOMER HAS SIGNED ELITE STRUCTURES APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/END USER CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND SHIPPING SCHEDULE.

THE BUYER/END USER CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION, ALL INTERFACE, COMPATIBILITY AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIAL NOT FURNISHED BY ELITE STRUCTURES ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USER CUSTOMER. SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR ELITE STRUCTURES ASSUMPTIONS WILL GOVERN SECTION 4 AND COMMENTARY, AISC CODE OF STANDARD 9TH EDITION

IT IS THE RESPONSIBILITY OF THE BUYER/END CUSTOMER TO INSURE THAT ELITE STRUCTURES PLANS COMPLIES WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT ELITE STRUCTURES OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY ELITE STRUCTURES

THE BUYER/END CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF THE STEEL IN ACCORDANCE WITH ELITE STRUCTURES CONSTRUCTION DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS BUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR ERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS, USE ONLY FINAL AISC CONSTRUCTION DRAWINGS FOR THE USE.

ELITE STRUCTURES IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE CONCRETE.

UNLESS OTHERWISE PROVIDED IN THE ORDER DOCUMENT, ELITE STRUCTURES DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR ERECTION DOCUMENT, THE END USER CUSTOMER SHALL ASSUME FULL RESPONSIBILITY FOR THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, VELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY REAMING, CHIPPING OR VELDING REQUIRE MAJOR CHANGES IN WORKING CONSIDERATION ARE TO REPORT IMMEDIATELY TO ELITE STRUCTURES BY THE BUYER/END USER CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS.

BUILDING DESCRIPTION:
42 x 100 x 16

LOCAL BUILDING CODE:
ENCLOSURE TYPE:
BUILDING RISK CATAGORY
LOAD REDUCTION USED:
LIVE LOAD TO FRAMES:
LIVE LOAD TO PURLINS:
DEAD LOAD:
COLLATERAL LOAD:
SNOW LOAD (GROUND):
SNOW LOAD (ROOF):
WIND SPEED:
WIND EXPOSURE:
SEISMIC ZONE:
SEISMIC IMPORTANCE:
SPECTRAL RESPONSE Ss
SPECTRAL RESPONSE S1
RAIN INTENSITY: 5 yr.
25 yr.

FBC 17
Closed
II - Normal
Yes
12
20.00
2,500
1
0
0
130
B
B
1.00
0.09
0.05
10.0000
11.0000

Gregory S. Barfield, P.E.
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P.E. # 54419

ENGINEERING SEAL
THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY ELITE STRUCTURES ONLY

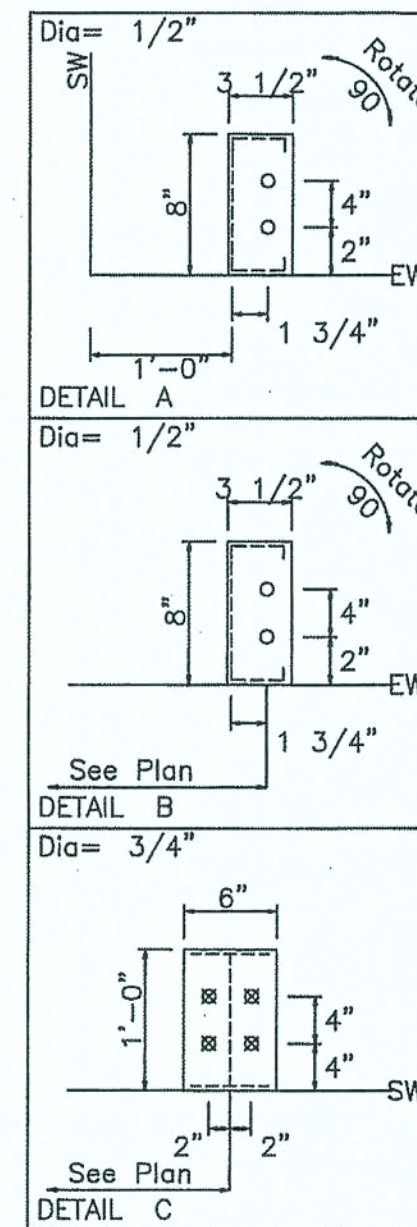
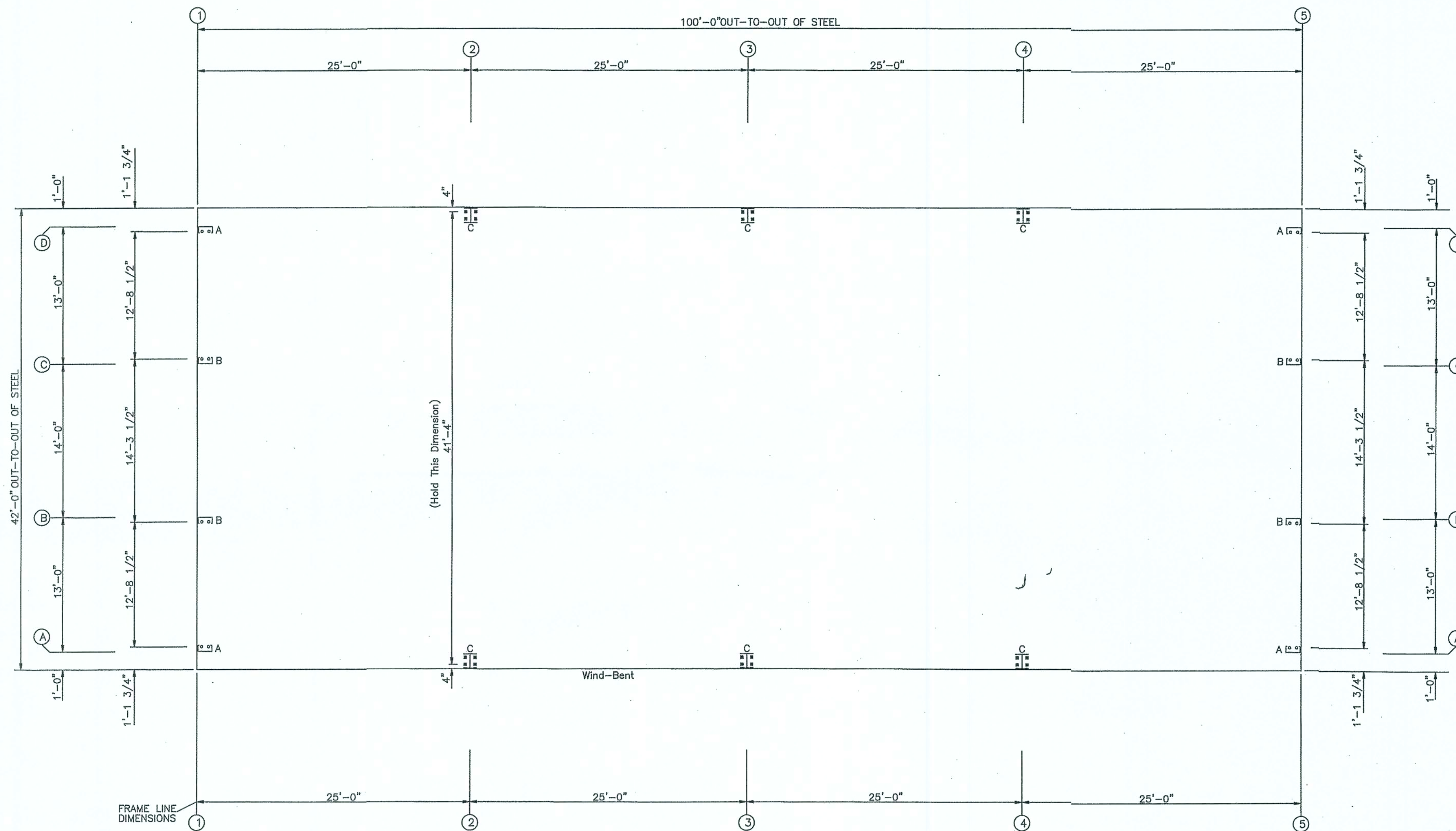


SEALING OF THESE DRAWINGS DOES NOT IMPLY OR CONSTITUTE THAT ELITE STRUCTURES ENGINEER IS THE ENGINEER OF RECORD IN THE DESIGN PROFESSIONAL FOR THE THIS PROJECT. ONLY THE DESIGN PART OF THE METAL BUILDING SYSTEM AS FURNISHED BY ELITE STRUCTURES IS INCLUDED. FOUNDATIONS ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS AND OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ELITE STRUCTURES ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

DESIGN SPECIFICATIONS

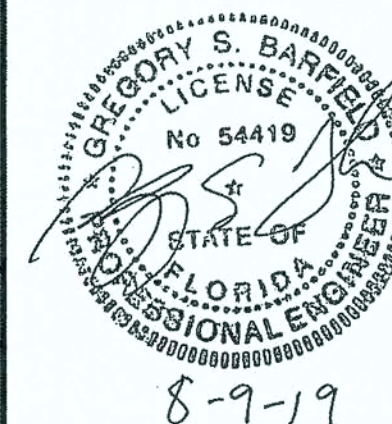
FOR

JOB ID: 11162



Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	10 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	20 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1.0
WIND EXPOSURE:	1
BUILDING OCCUPANCY CATEGORY:	II
SEISMIC USE GROUP:	ENCLOSURE CLASSIFICATION



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P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S.	DATE: 4-4-19	REVISION
DRAWN BY: R. WALKER	SALES: JERRY RAGLAND	DESCRIPTION
BUILDING SIZE: 42X100X16		

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

PLAN:
ANCHOR BOLT PLAN

JOB # 11162-AB
SHEET 1 OF 11

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bot(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	D	3	3.8	4.1	4	-4.1	-3.4	4	0.750	6.000	12.00	0.500	0.0
		1	3.0	8.8	7	-1.0	-6.3						
2*	A	5	4.1	-3.4	2	-3.8	2.1	4	0.750	6.000	12.00	0.500	0.0
		1	-3.0	8.8	8	1.0	-9.0						

2* Frame lines: 2 3 4

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	D	0.6	1.9	0.2	0.6	2.2	6.3	-7.0	-12.4	1.7	-8.0	-7.4	-7.5
2*	A	-0.6	1.9	-0.2	0.6	-2.2	6.3	-1.7	-8.0	7.0	-12.4	-1.3	-3.1

Frame Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right		Seismic_Long	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	D	1.3	-3.1	-2.3	-12.4	2.9	-6.3	-0.1	-0.1	0.1	0.1	-0.4	-0.3
2*	A	7.4	-7.5	-2.9	-11.3	2.3	-16.9	-0.1	0.1	0.1	-0.1	-0.4	-0.3

2* Frame lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

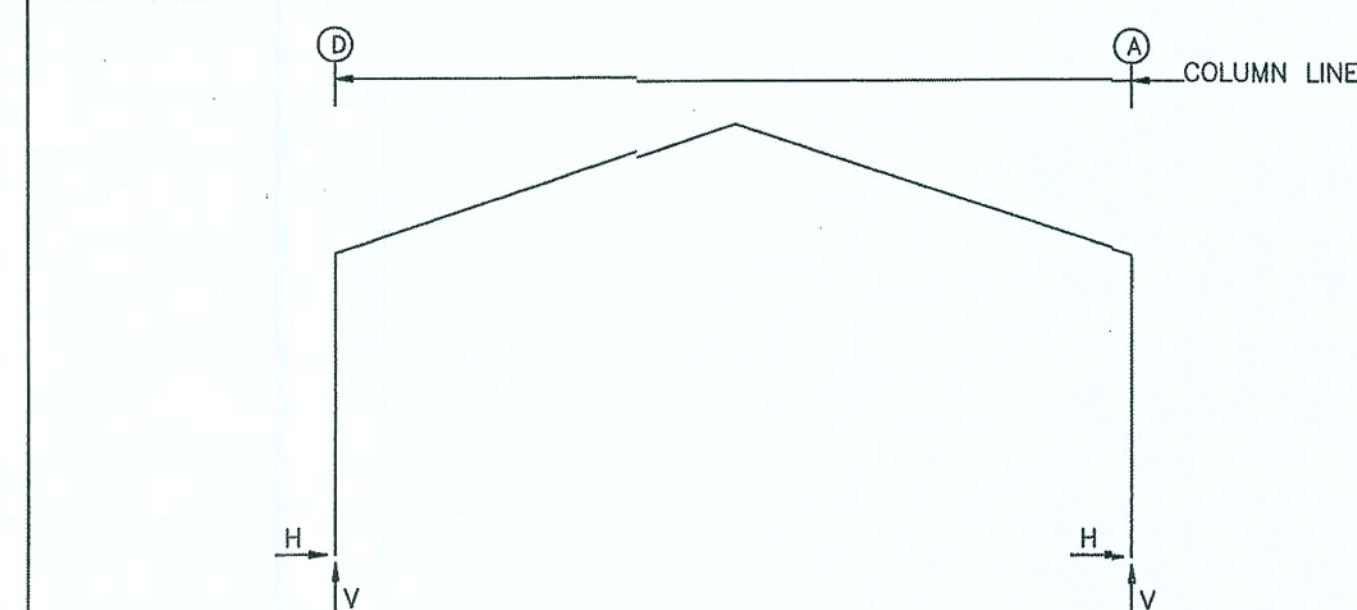
Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Wind Left1		Wind Right1		Wind Press Horiz	Wind Suct Long1		Wind Long2		Seis Left Vert
					Vert	Vert	Vert	Vert		Horiz	Vert	Vert	Vert	
1	D	0.3	0.1	1.5	-1.9	-1.5	-0.8	-0.3	0.0	0.0	-2.0	-1.4	0.0	0.0
1	C	0.7	0.2	3.8	-5.3	-3.7	-4.0	-2.5	-3.2	3.5	-5.2	-3.1	0.0	0.0
1	B	0.7	0.2	3.8	-3.7	-5.3	-2.5	-4.0	-3.2	3.5	-3.1	-5.2	-0.1	0.0
1	A	0.3	0.1	1.5	-1.5	-1.9	-0.3	-0.8	0.0	0.0	-1.4	-2.0	0.1	0.0
5	A	0.3	0.1	1.5	-1.9	-1.5	-0.8	-0.3	0.0	0.0	-2.0	-1.4	0.0	0.0
5	B	0.7	0.2	3.8	-5.3	-3.7	-4.0	-2.5	-3.2	3.5	-5.2	-3.1	0.0	0.0
5	C	0.7	0.2	3.8	-3.7	-5.3	-2.5	-4.0	-3.2	3.5	-3.1	-5.2	-0.1	0.0
5	D	0.3	0.1	1.5	-1.5	-1.9	-0.3	-0.8	0.0	0.0	-1.4	-2.0	0.1	0.0

Frm Line	Col Line	Seis Right Vert
1	D	0.1
1	C	-0.1
1	B	0.0
1	A	0.0
5	A	0.1
5	B	-0.1
5	C	0.0
5	D	0.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bot(in) Qty Dia		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	D	6 1	0.0 0.0	-1.1 1.8	6	0.0	-1.1	2	0.500	3.500	8.000	0.250	0.0
1	C	9 1	2.1 0.0	-2.8 4.7	10 9	-1.9 2.1	-2.7 -2.8	2	0.500	3.500	8.000	0.250	0.0
1	B	11 1	2.1 0.0	-2.8 4.7	12 11	-1.9 2.1	-2.7 -2.8	2	0.500	3.500	8.000	0.250	0.0
1	A	8 1	0.0 0.0	-1.1 1.8	8	0.0	-1.1	2	0.500	3.500	8.000	0.250	0.0
5	A	6 1	0.0 0.0	-1.1 1.8	6	0.0	-1.1	2	0.500	3.500	8.000	0.250	0.0
5	B	9 1	2.1 0.0	-2.8 4.7	10 9	-1.9 2.1	-2.7 -2.8	2	0.500	3.500	8.000	0.250	0.0
5	C	11 1	2.1 0.0	-2.8 4.7	12 11	-1.9 2.1	-2.7 -2.8	2	0.500	3.500	8.000	0.250	0.0
5	D	8 1	0.0 0.0	-1.1 1.8	8	0.0	-1.1	2	0.500	3.500	8.000	0.250	0.0

FRAME LINES: 2 3 4



BUILDING BRACING REACTIONS

Wall Loc	Col Line	Reactions(k)				Panel Shear (lb/ft)		Note
		Wind Horiz	Seismic Horiz	Wind Vert	Seismic Vert	Wind	Seis	
L_SW	1	4.0	4.5	0.5	0.5	62	11	(b)
F_SW	A							
R_SW	5					57	10	
B_SW	D							

(b) Wind bent in bay, base above finish floor.

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
16	Endwall	1/2"	GR36	1.50
24	Frame	3/4"	GR36	2.50

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	42.0
Length (ft)	=	100.0
Eave Height (ft)	=	16.0 / 16.0
Roof Slope (rise/12)	=	4.0 / 4.0
Dead Load (psf)	=	2.5
Collateral Load (psf)	=	1.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	130.0
Wind Code	=	FBC 17 (IBC 15)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa/Ss)	=	0.15

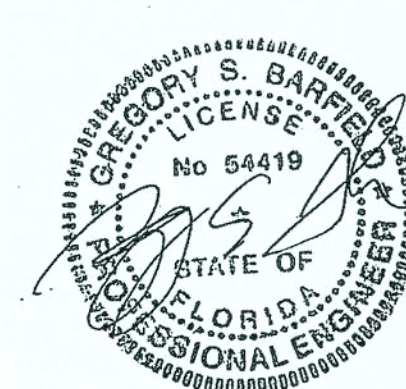
ID Description

1	Dead+Collateral+Live
2	Dead+Collateral+0.75Live+0.45Wind_Long1L
3	Dead+Collateral+0.75Live+0.45Wind_Long2R
4	0.6Dead+0.6Wind_Left2
5	0.6Dead+0.6Wind_Right2
6	0.6Dead+0.6Wind_Long1L
7	0.6Dead+0.6Wind_Long1R
8	0.6Dead+0.6Wind_Long2L
9	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
10	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
11	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
12	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L

Gregory S. Barfield, P.E.
2149 Nell Purvis Road
Adel, GA 31620
P.E. # 54419

DESIGN CRITERIA

BUILDING CODE	FBC 17
LIVE LOAD TO FRAMES	12 PSF
LIVE LOAD TO PURLINS	20 PSF
COLLATERAL LOAD	1.0 PSF
SNOW LOAD (GROUND)	0 PSF
WIND LOAD (ULTIMATE)	130 MPH
WIND IMPORTANCE FACTOR	1
WIND EXPOSURE	B
BUILDING OCCUPANCY CATEGORY	II
SEISMIC USE GROUP	I
ENCLOSURE CLASSIFICATION	ENCLOSED



PH 229-896-7569
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P.O. BOX 207
ADEL, GA 31620

SCALE: N.T.S. DATE: 4-4-19
DRAWN BY: R WALKER SALES: JERRY RAGLAND
BUILDING SIZE: 42X100X16

REVISION

DATE	DESCRIPTION

TITLE/LOCATION

MARK SMITH
LAKE CITY, FL.

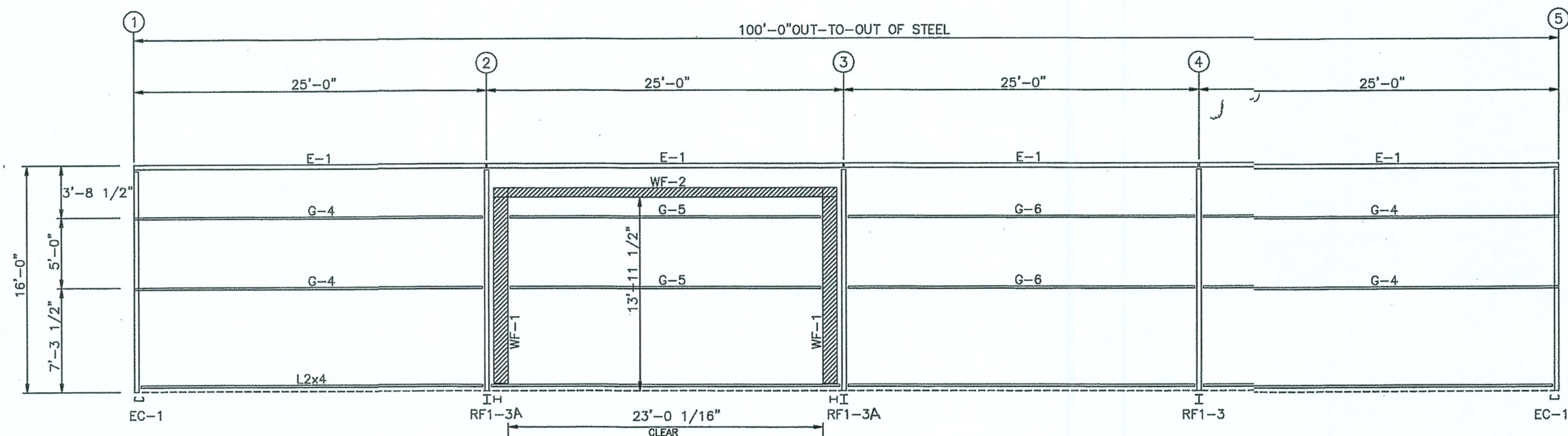
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COLUMN REACTIONS

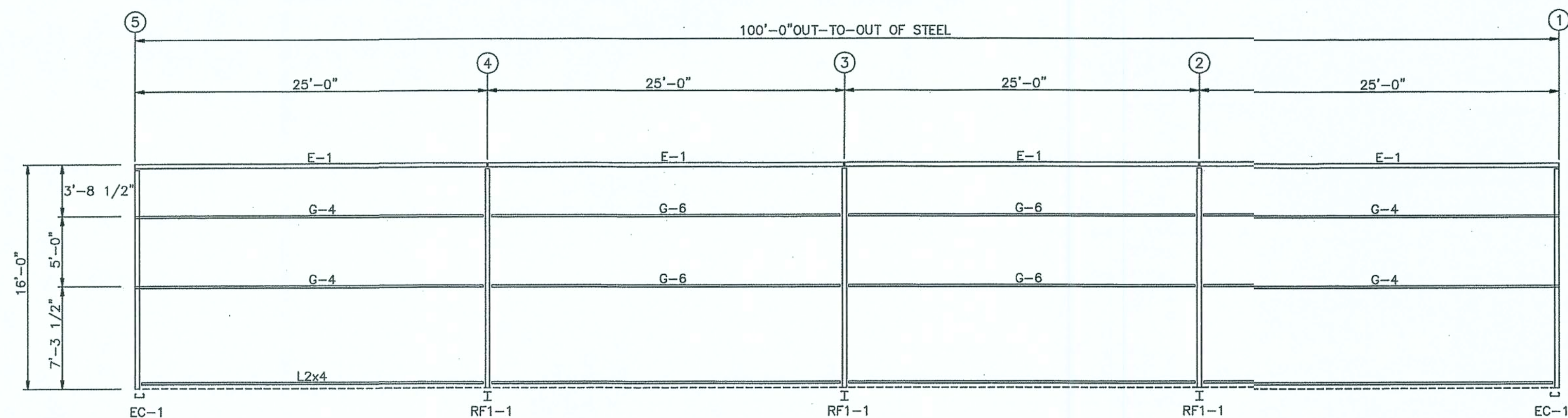
JOB # 11162-CR

SHEET 2 OF 11

8-9-19



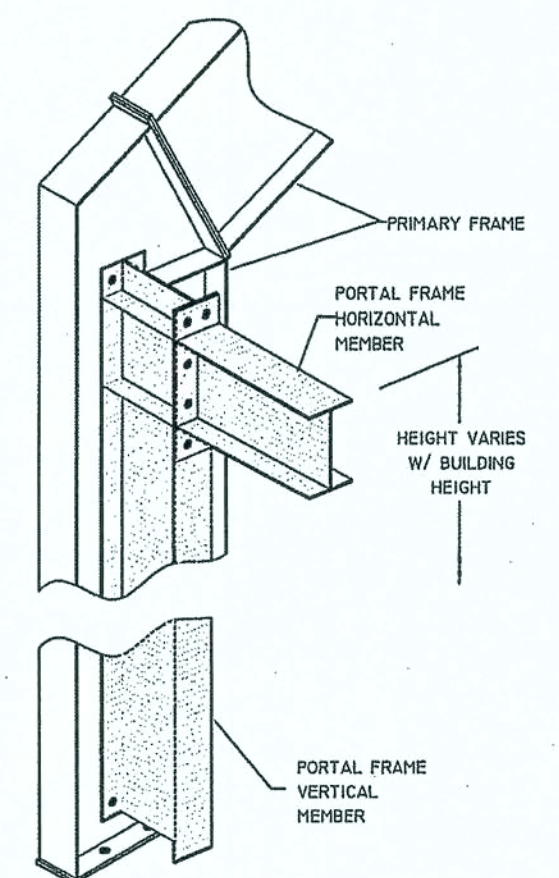
SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D

BOLT TABLE				
FRAME LINE A & D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	3"
WF-1 - RF1-3	14	A325	3/4"	3"

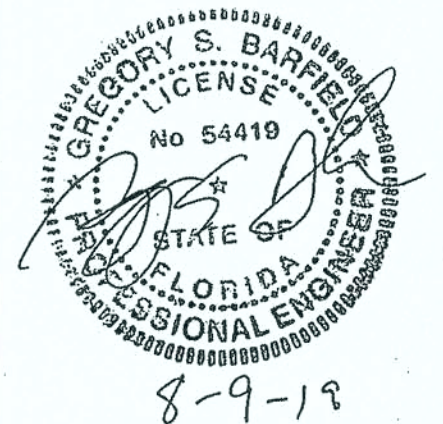
MEMBER TABLE			
FRAME LINE A & D			
MARK	PART	LENGTH	
WF-1	W12X14	14'-1 7/16"	
WF-2	W8X24	22'-11 1/16"	
E-1	10E214L4	24'-11 1/2"	
G-4	12X35C12	24'-11 1/2"	
G-5	12X35C12	22'-11 1/2"	
G-6	12X35C12	24'-7 1/2"	



PORTAL FRAME CONNECTION DETAIL

Gregory S. Barfield, P.E.,
2149 Nail Purvis Road,
Adel, GA 31620,
P.E. # 54419

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	10 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1.0
WIND EXPOSURE:	B
BUILDING OCCUPANCY CATEGORY:	II
SEISMIC USE GROUP:	ENCLOSURE CLASSIFICATION



PH 229-896-7569
FAX 229-896-7561
P.O. BOX 207
ADEL, GA 31620

SCALE:	N.T.S.	DATE:	4-4-19
DRAWN BY:	R. WALKER	SALES:	JERRY RAGLAND
BUILDING SIZE:	42X100X16		

REVISION

DATE	DESCRIPTION

TITLE/LOCATION

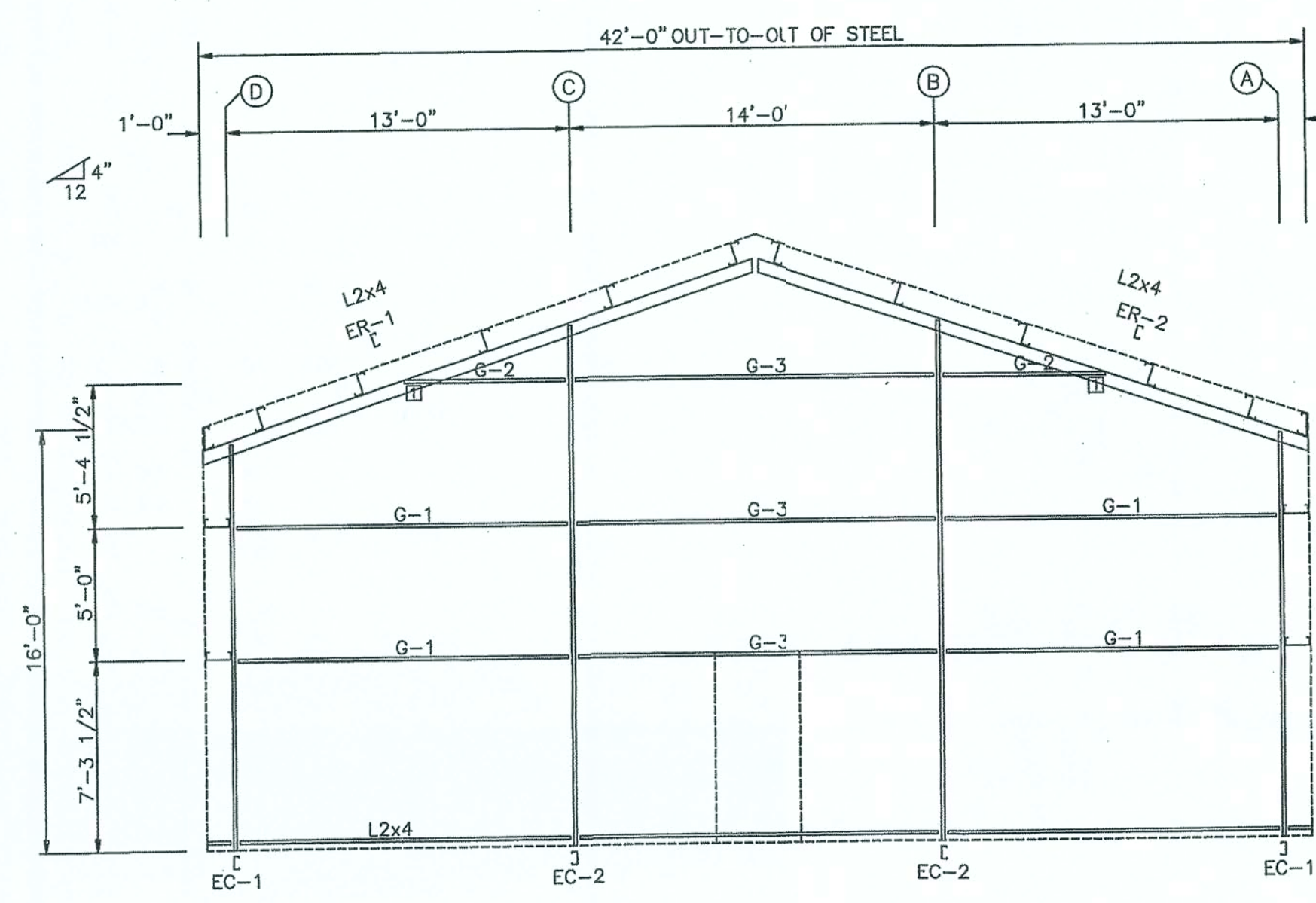
MARK SMITH
LAKE CITY, FL.

PLAN:

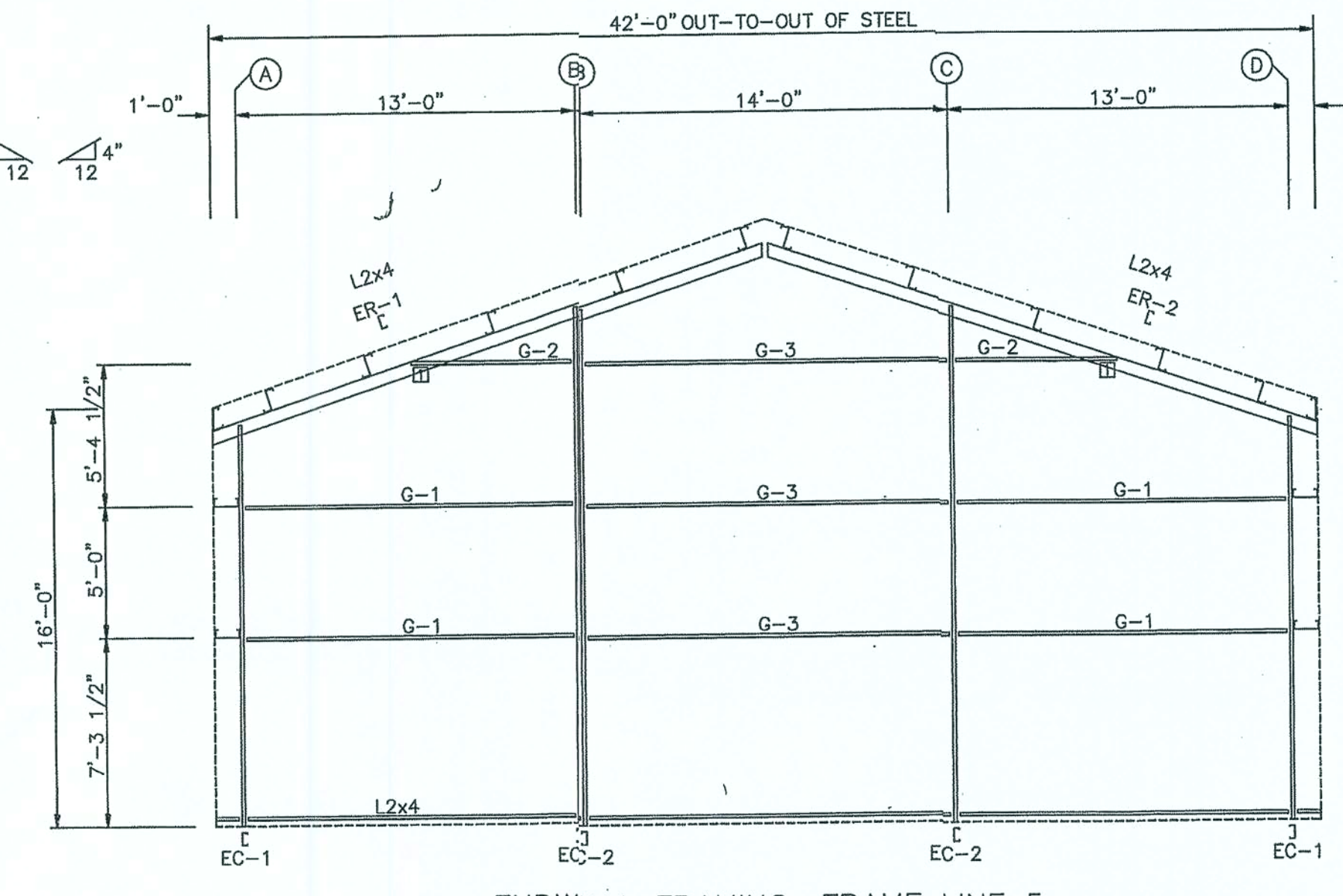
SIDEWALL FRAMING

JOB # 11162-SF

SHEET 3 OF 11



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 5

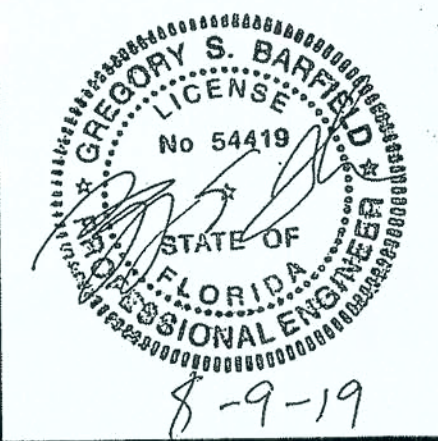
MEMBER TABLE FRAME LINE 1 & 5		
MARK	PART	LENGTH
EC-1	8X35C14	15'-5 1/16"
EC-2	8X35C12	19'-7 7/8"
ER-1	10X35C14	21'-4"
ER-2	10X35C14	21'-4"
G-1	8X25C16	12'-4 1/2"
G-2	8X25C16	6'-0 5/8"
G-3	8X25C16	13'-11 1/2"

BOLT TABLE FRAME LINE 1 & 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	1/2"	1 1/2"
Columns/Raf	2	A325	1/2"	1 1/2"

CONNECTION PLATES FRAME LINE 1 & 5	
☐ ID	MARK/PART
☒	1 d1

DESIGN CRITERIA		
BUILDING CODE:	FBC	17
LIVE LOAD TO FRAMES:	12	PSF
LIVE LOAD TO PURLINS:	20	PSF
COLLATERAL LOAD:	1	PSF
SNOW LOAD (GROUND):	0	PSF
WIND LOAD (ULTIMATE):	130	MPH
WIND EXPOSURE:	I	
WIND IMPORTANCE FACTOR:	1	
BUILDING OCCUPANCY CATEGORY:	II	
SEISMIC USE GROUP:	I	
ENCLOSURE CLASSIFICATION:	ENCLOSED	

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Adel, GA 31620
P.E. # 54419



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ADEL, GA 31620

SCALE: N.T.S. DATE: 4-4-19
DRAWN BY: R WALKER SALES: JERRY RAGLAND
BUILDING SIZE: 42X100X16

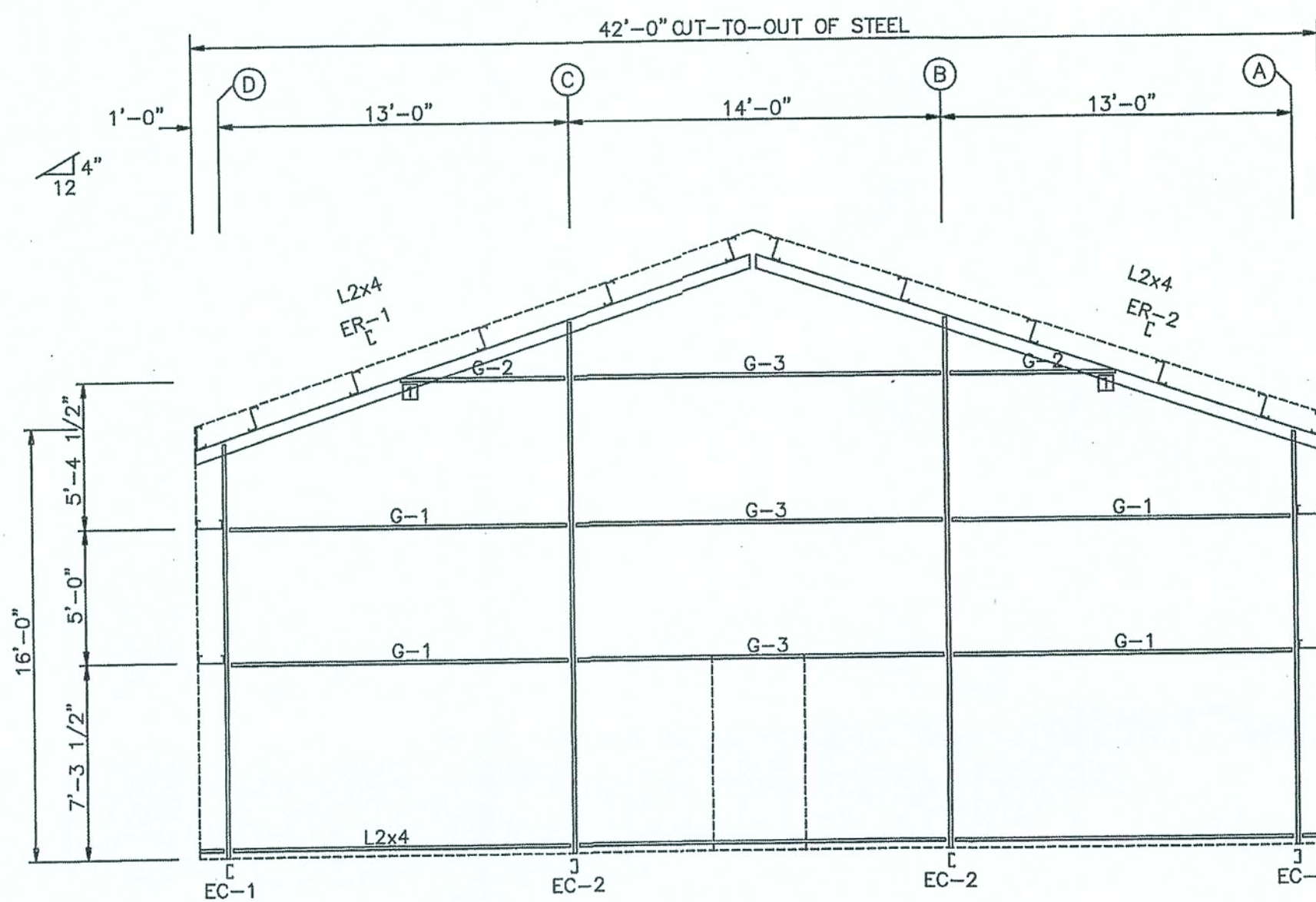
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

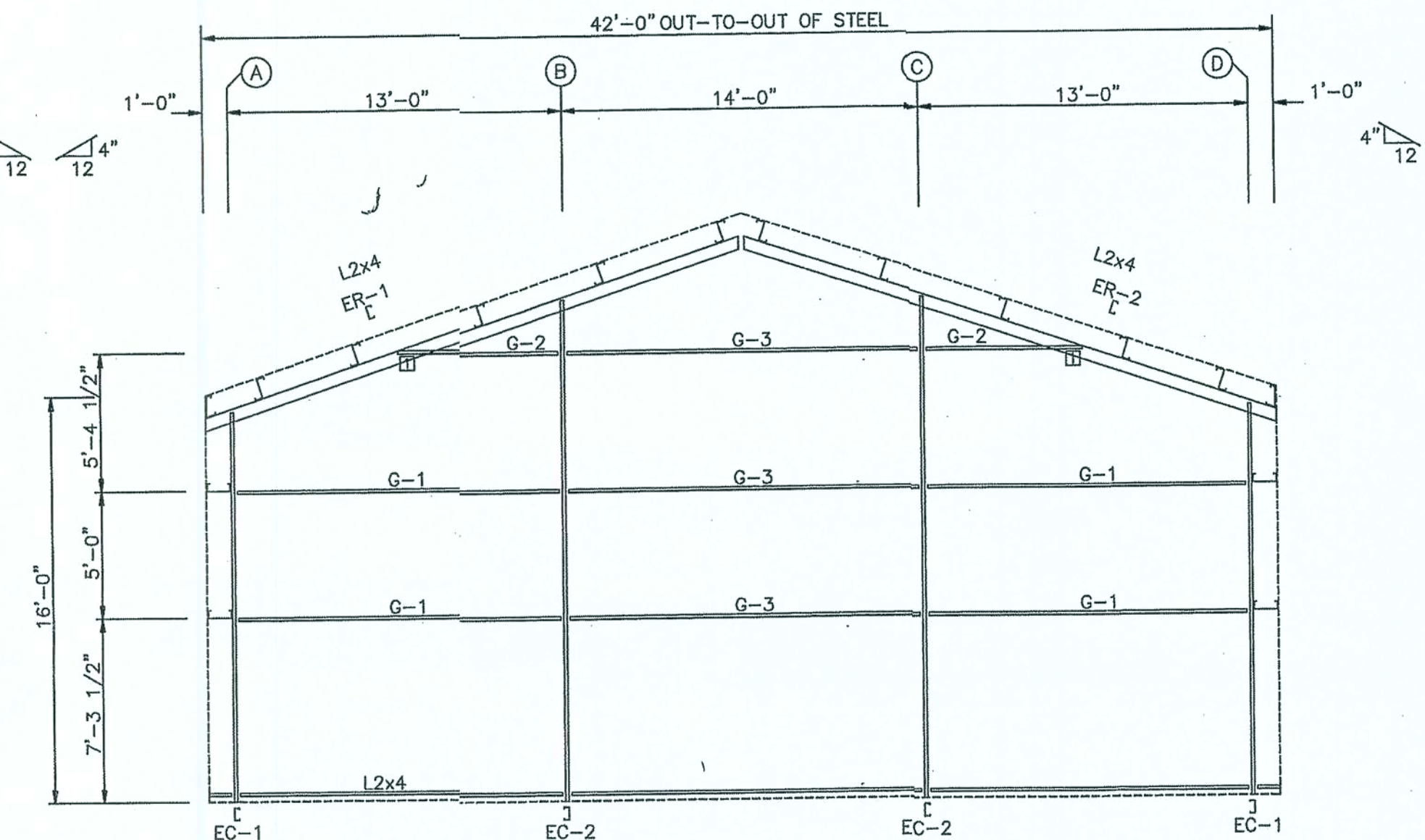
PLAN:
ENDWALL FRAMING

JOB # 11162-SF
SHEET 4 OF 11

8-9-19



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 5

MEMBER TABLE FRAME LINE 1 & 5		
MARK	PART	LENGTH
EC-1	8X35C14	15'-5 1/16"
EC-2	8X35C12	19'-7 7/8"
ER-1	10X35C14	21'-4"
ER-2	10X35C14	21'-4"
G-1	8X25C16	12'-4 1/2"
G-2	8X25C16	6'-0 5/8"
G-3	8X25C16	13'-11 1/2"

BOLT TABLE FRAME LINE 1 & 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	1/2"	1 1/2"
Columns/Raf	2	A325	1/2"	1 1/2"

CONNECTION PLATES FRAME LINE 1 & 5	
ID	MARK/PART
1	d1

DESIGN CRITERIA		
BUILDING CODE:	FBC	17
LIVE LOAD TO FRAMES:	12	PSF
LIVE LOAD TO PURLINS:	20	PSF
COLLATERAL LOAD:	1	PSF
SNOW LOAD (GROUND):	0	PSF
WIND LOAD (ULTIMATE):	130	MPH
WIND IMPORTANCE FACTOR:	1	
WIND EXPOSURE:	1	
BUILDING OCCUPANCY CATEGORY:	II	
SEISMIC USE GROUP:	ENCLOSED	

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SCALE: N.T.S.	DATE: 4-4-19
DRAWN BY: R WALKER	SALES: JERRY RAGLAND
BUILDING SIZE:	42X100X16

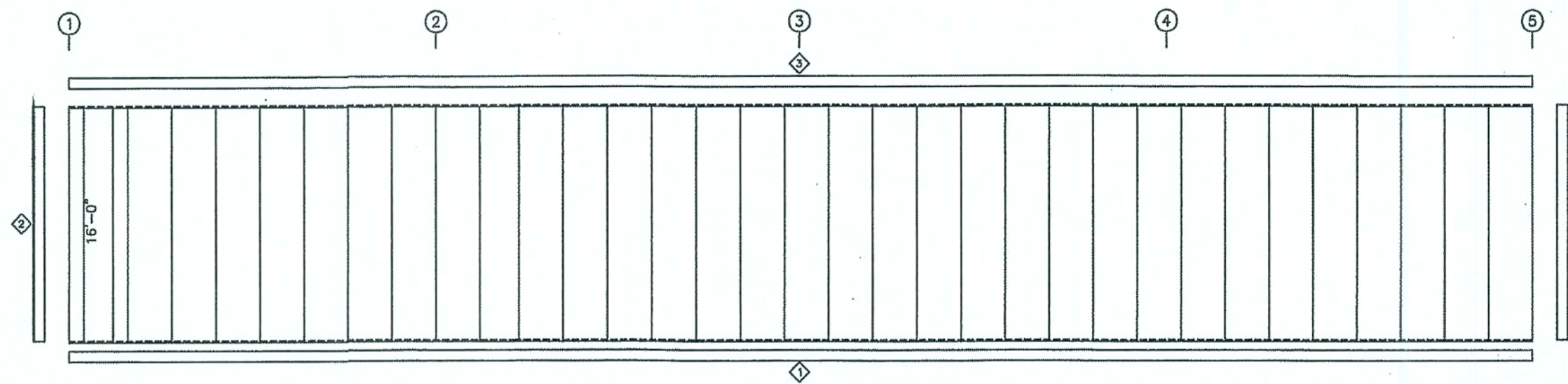
REVISION	
DATE:	DESCRIPTION

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

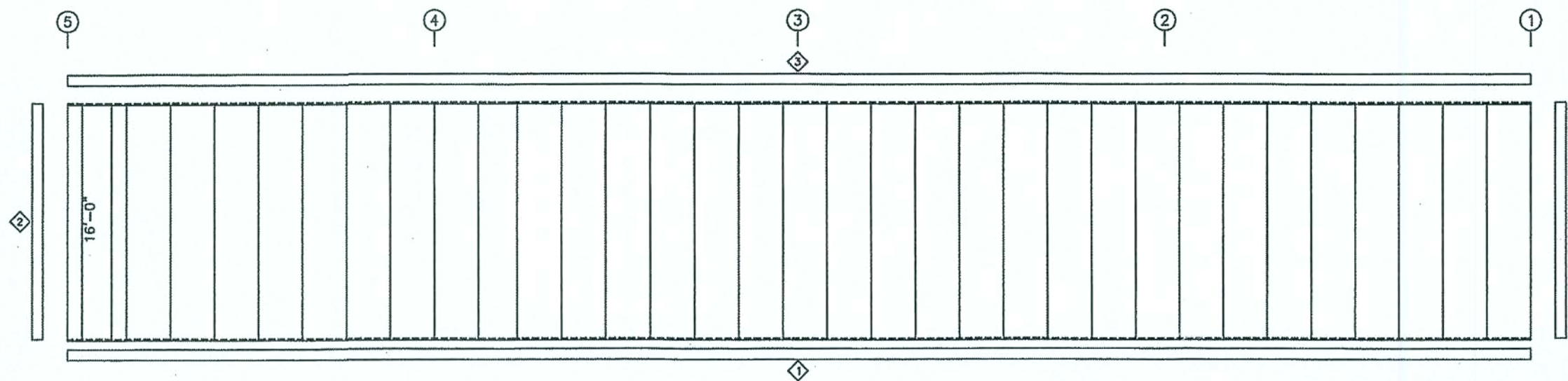
PLAN:
ENDWALL FRAMING

JOB # 11162-SF
SHEET 4 OF 11

8-9-19



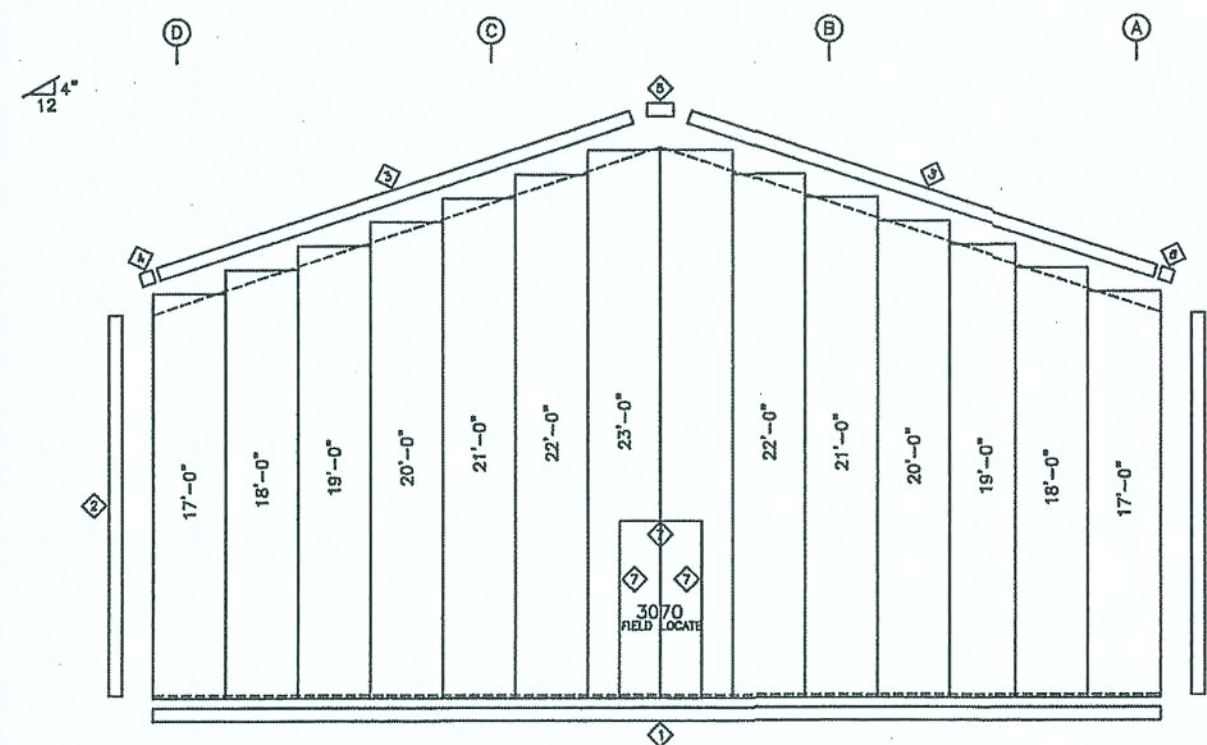
SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. PBR - NEED COLOR



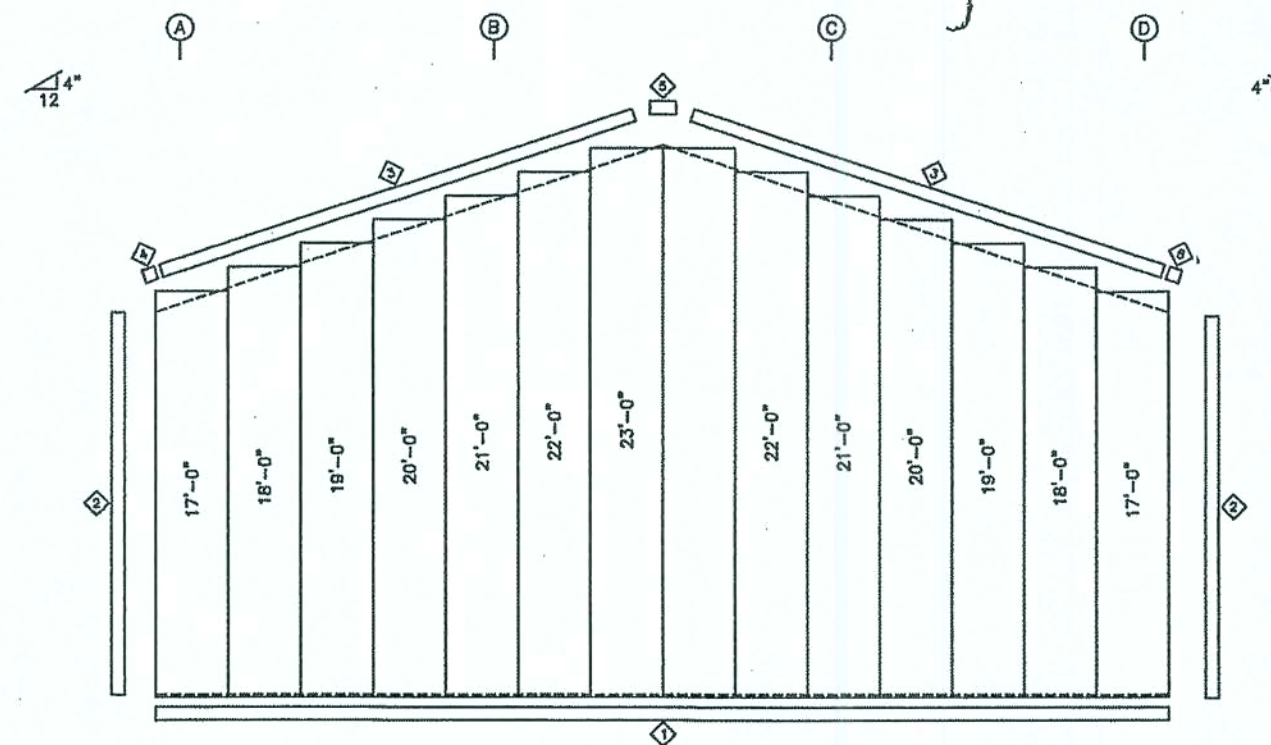
SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 Ga. PBR - NEED COLOR

TRIM TABLE FRAME LINE A & D		
ID	PART	DETAIL
1	E-108	TRIM_43
2	FL-830	TRIM_47
3	FL-19	TRIM_5

TRIM TABLE FRAME LINE 1 & 5		
ID	PART	DETAIL
1	E-108	TRIM_43
2	FL-830	TRIM_47
3	E-101	TRIM_16
4	E-145	TRIM_7
5	E-146	TRIM_23
6	E-105	TRIM_7
7	FL-26	TRIM_49



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. PBR - NEED COLOR



ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 Ga. PBR - NEED COLOR

DESIGN CRITERIA		
BUILDING CODE:	FBC	17
LIVE LOAD TO FRAMES:	12	PSF
LIVE LOAD TO PURLINS:	20	PSF
COLLATERAL LOAD:	1	PSF
SNOW LOAD (GROUND):	0	PSF
WIND LOAD (ULTIMATE):	130	MPH
WIND IMPORTANCE FACTOR:	1	
WIND EXPOSURE:	B	
BUILDING OCCUPANCY CATEGORY:	II	
SEISMIC USE GROUP:	I	
ENCLOSURE CLASSIFICATION:	ENCLOSED	

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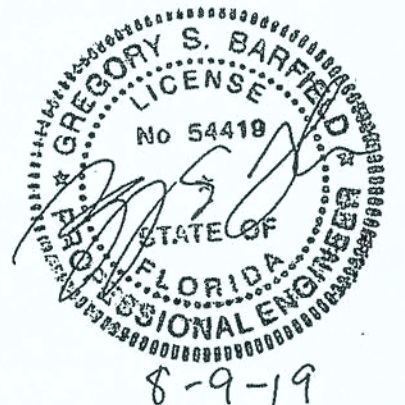
SCALE: N.I.S. DATE: 4-4-19
DRAWN BY: R. WALKER SALES: JERRY RAGLAND
BUILDING SIZE: 42X100X16

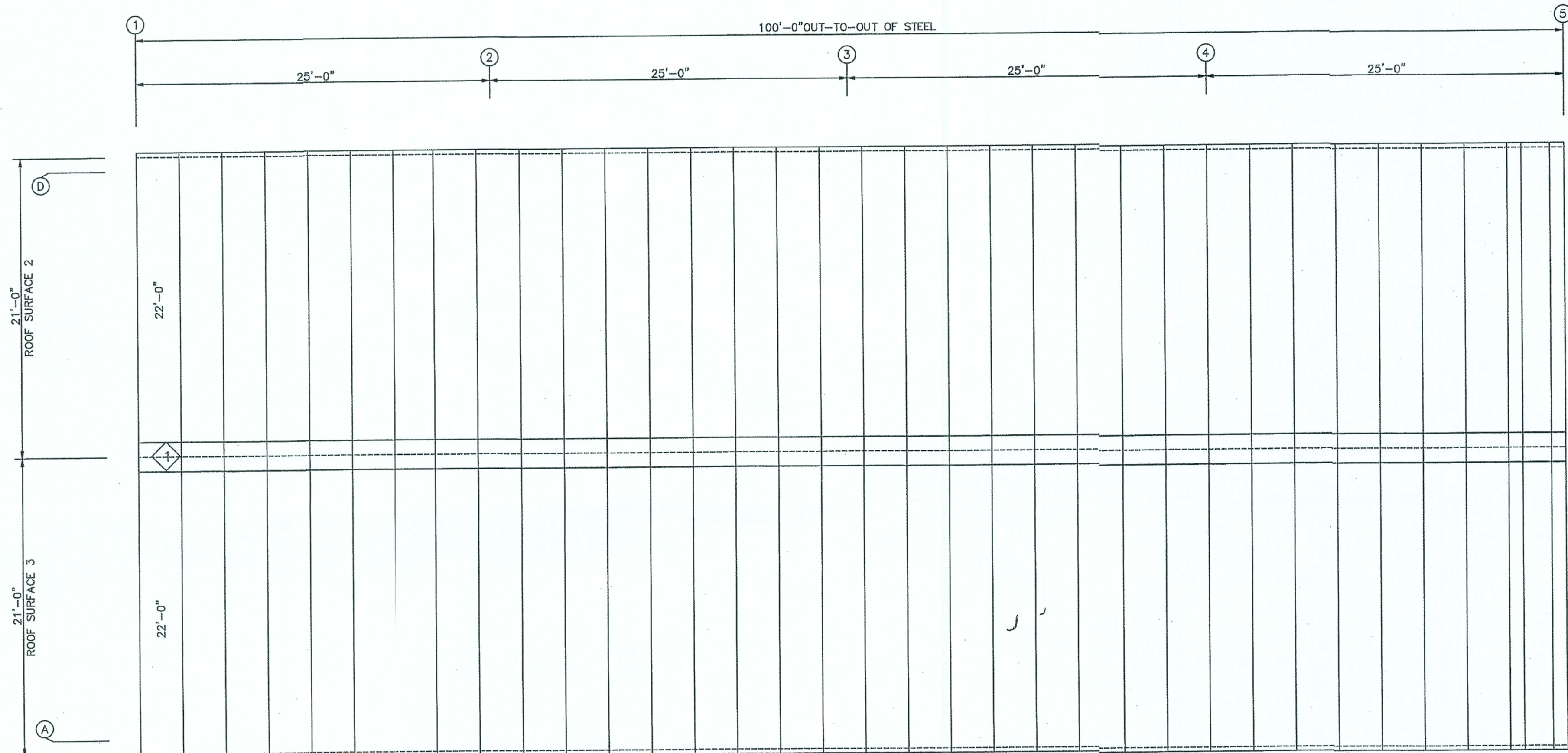
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

PLAN:
WALL SHEETING

JOB # 11162-W5
SHEET 6 OF 11



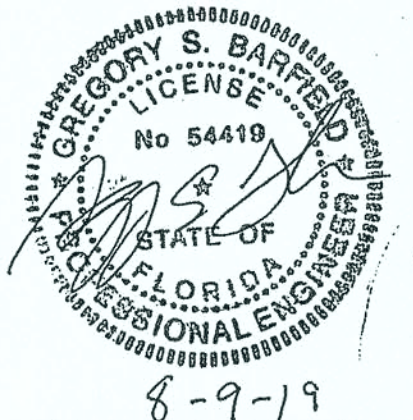


PANELS: 26 Ga. PBR - Galvalume

TRIM TABLE
ROOF PLAN
◇ ID PART
1 FL-51

DESIGN CRITERIA		
BUILDING CODE:	FBC	17
LIVE LOAD TO FRAMES:	12	PSF
LIVE LOAD TO PURLINS:	20	PSF
COLLATERAL LOAD:	1	PSF
SNOW LOAD (GROUND):	0	PSF
WIND LOAD (ULTIMATE):	130	MPH
WIND IMPORTANCE FACTOR:	1	
WIND EXPOSURE:	B	
BUILDING OCCUPANCY CATEGORY:	II	
SEISMIC USE GROUP:	I	
ENCLOSURE CLASSIFICATION:	ENCLOSED	

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ADEL, GA 31620

SCALE: I.T.S.	DATE: 4-4-19
DRAWN BY: R WALKER	SALES: JERRY RAGLAND
BUILDING SIZE:	42X100X16

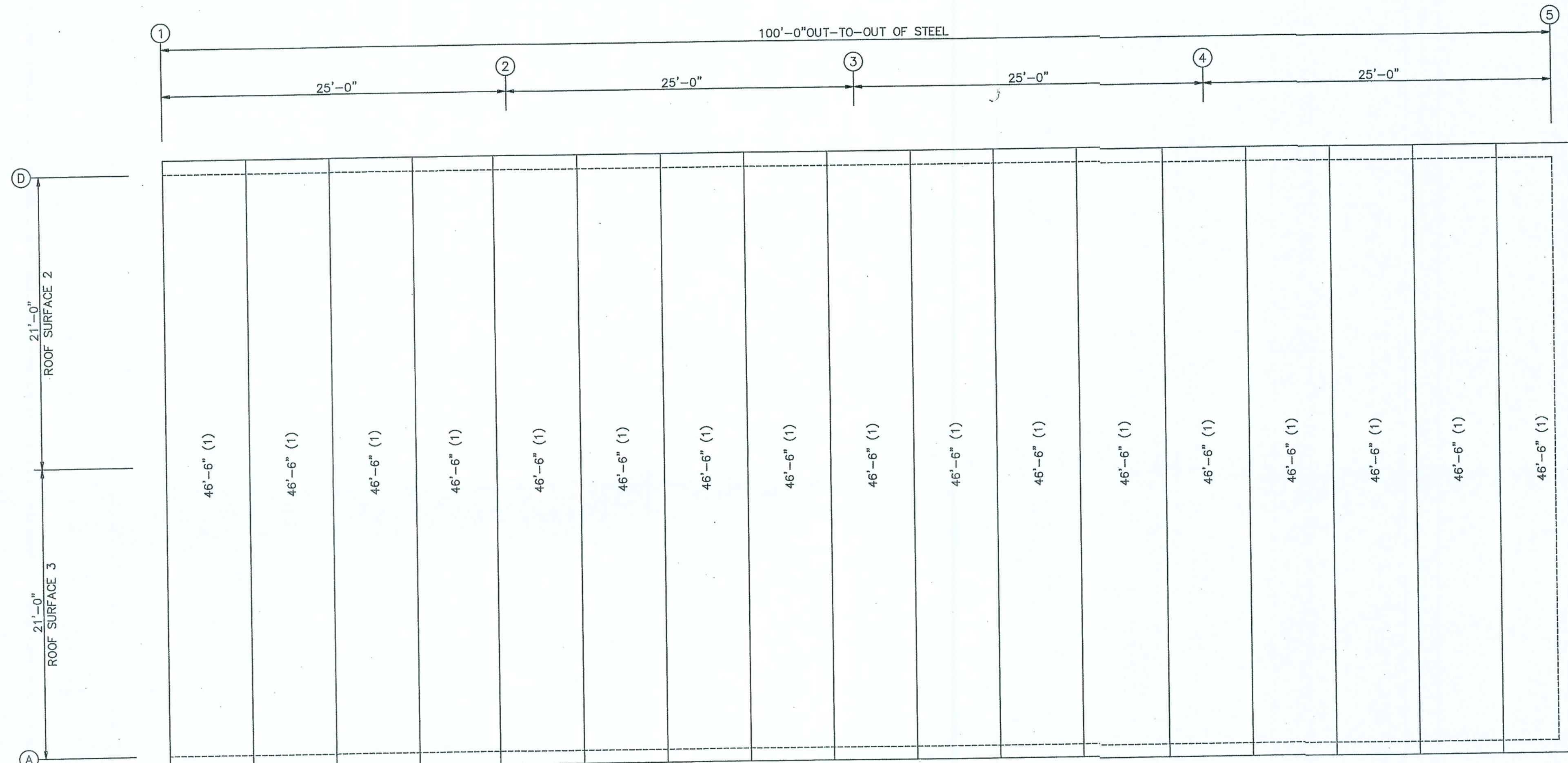
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

PLAN:
ROOF SHEETING

JOB # 11162-RS
SHEET 7 OF 11

8-9-19

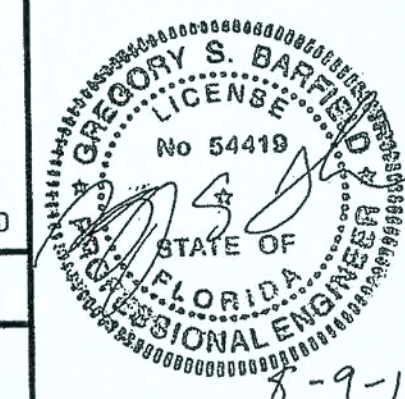


INSULATION: R-30

INSULATION TABLE				
ROLL	QUAN	MARK	WIDTH	LENGTH
1	17	RI-1	6'-0"	46'-6"

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	0 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1.0
WIND EXPOSURE:	II
BUILDING OCCUPANCY CATEGORY:	II
SEISMIC USE GROUP:	ENCLOSED
ENCLOSURE CLASSIFICATION:	ENCLOSED

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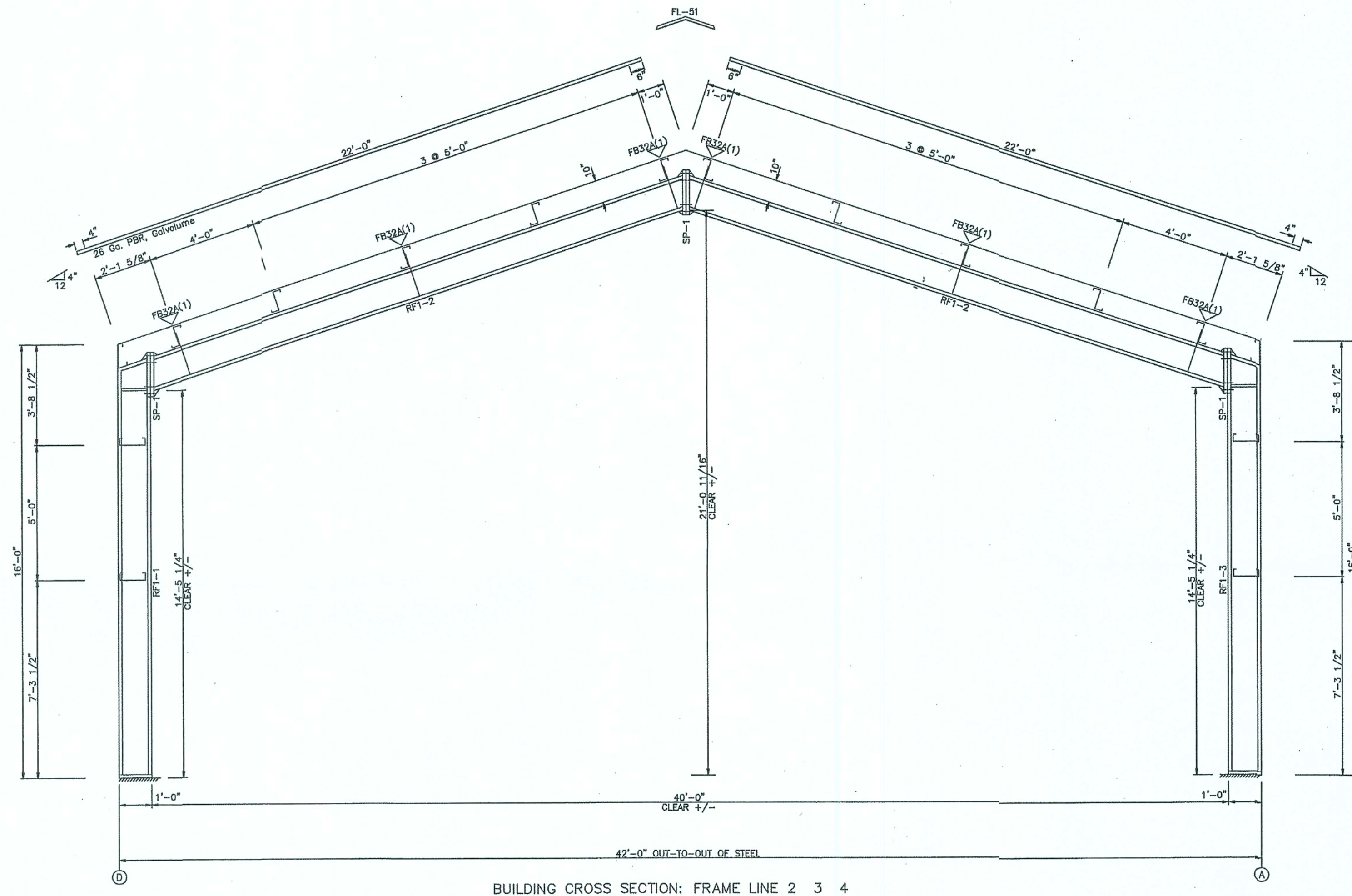
SCALE: N.T.S.	DATE: 4-4-19	REVISION	
DRAWN BY: R. WALKER	SALES: JERRY RAGLAND	DATE:	DESCRIPTION
BUILDING SIZE:	42X100X16		

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

PLAN:
ROOF INSULATION

JOB # 11162-RI
SHEET 8 OF 11

8-9-19



BUILDING CROSS SECTION: FRAME LINE 2 3 4

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GENERAL NOTES:

BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325-09 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, JUNE 30, 2004. PRETENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST OFF TYPE TENSION CONTROL BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT (SPECIFICATION FOR STRUCTURAL JOINTS 9.1) IS SUGGESTED.

REFERENCE: STEEL CONSTRUCTION MANUAL
AMERICAN INSTITUTE OF STEEL CONSTRUCTION INC.
THIRTEENTH EDITION (AISC)

MEMBER SIZE TABLE				
MARK	MEMBER	LENGTH	WEIGHT	
RF1-1	W12X16	15'-1 7/16"	291	
RF1-2	W12X14	21'-0 7/16"	347	
RF1-3	W12X16	15'-1 7/16"	287	

SPICE BOLT TABLE					
Mark	Qty	Top	Bot	Inch	Type
SP-1	4	4	0	A325	0.750 3.00

FLANGE BRACES: Both Sides (U.N.)
FBxxA(1): xx=length(in)
A - L2X2X14G

DESIGN CRITERIA

BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	1 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	I B
WIND EXPOSURE:	I
BUILDING OCCUPANCY CATEGORY:	II
SEISMIC USE GROUP:	I
ENCLOSURE CLASSIFICATION:	ENCLOSED



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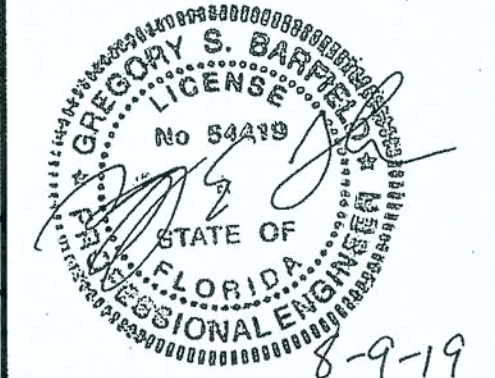
SCALE	N.T.S.	DATE	4-4-19
DRAWN BY:	R WALKER	SALES:	JERRY RAGLAND
BUILDING SIZE:	42X100X16		

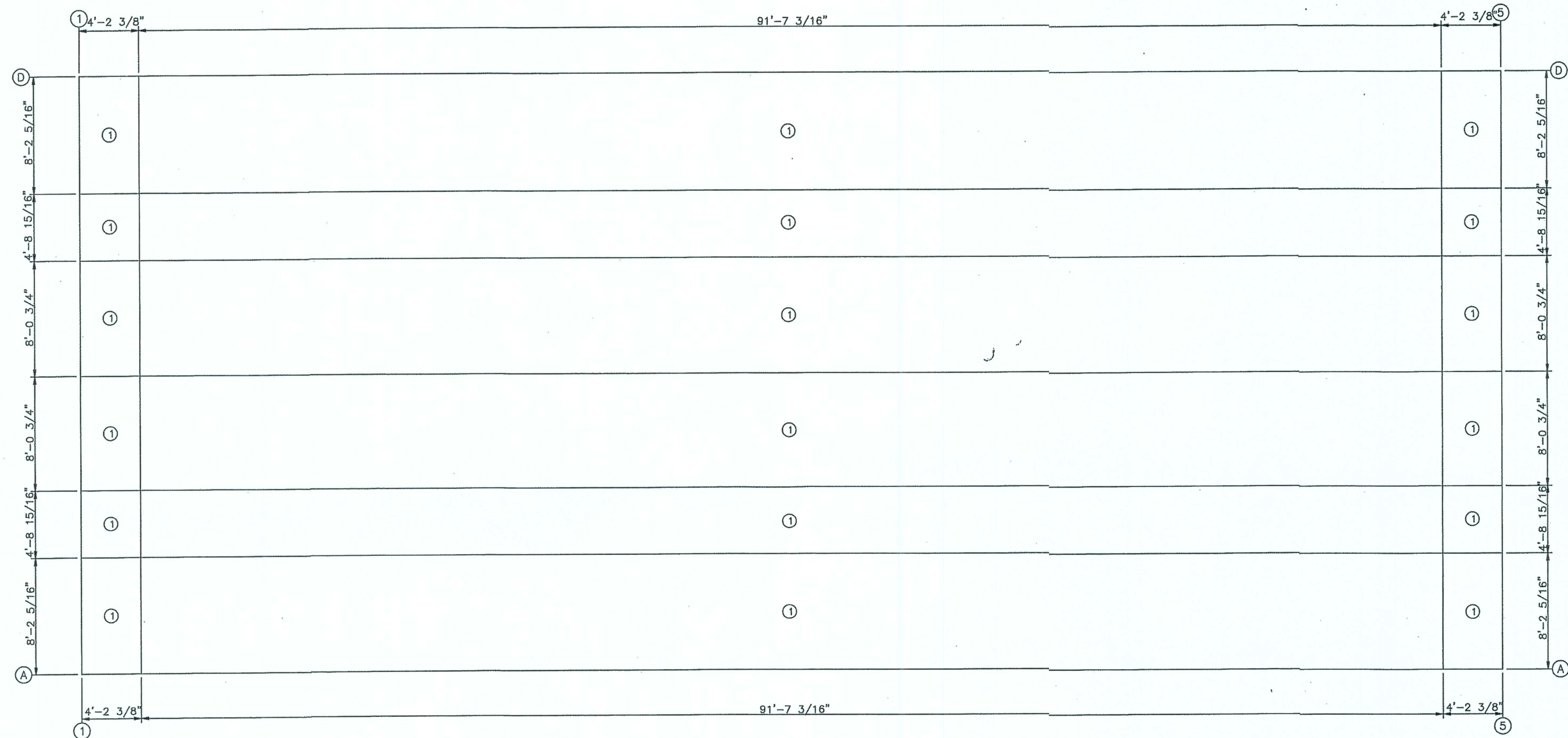
REVISION	
DATE	DESCRIPTION

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

PLAN:
ROOF SHEETING

JOB # 11162-RS
SHEET 9 OF 11





PANEL ZONE LAYOUT
(Minimum Fastener Spacing)

FASTENER TABLE		
○ ID	SCREW PART	SPACE (in)
1	12x1.25	12.0

DESIGN CRITERIA	
BUILDING CODE:	FBC 17
LIVE LOAD TO FRAMES:	12 PSF
LIVE LOAD TO PURLINS:	20 PSF
COLLATERAL LOAD:	10 PSF
SNOW LOAD (GROUND):	0 PSF
WIND LOAD (ULTIMATE):	130 MPH
WIND IMPORTANCE FACTOR:	1.0
WIND EXPOSURE:	II
BUILDING OCCUPANCY CATEGORY:	II
SEISMIC USE GROUP:	ENCLOSED

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ADEL, GA 31620

SCALE: N.T.S.	DATE: 4-4-19	REVISION
DRAWN BY: R WALKER	SALES: JERRY RAGLAND	DATE: DESCRIPTION
BUILDING SIZE: 42X100X16		

TITLE/LOCATION
MARK SMITH
LAKE CITY, FL.

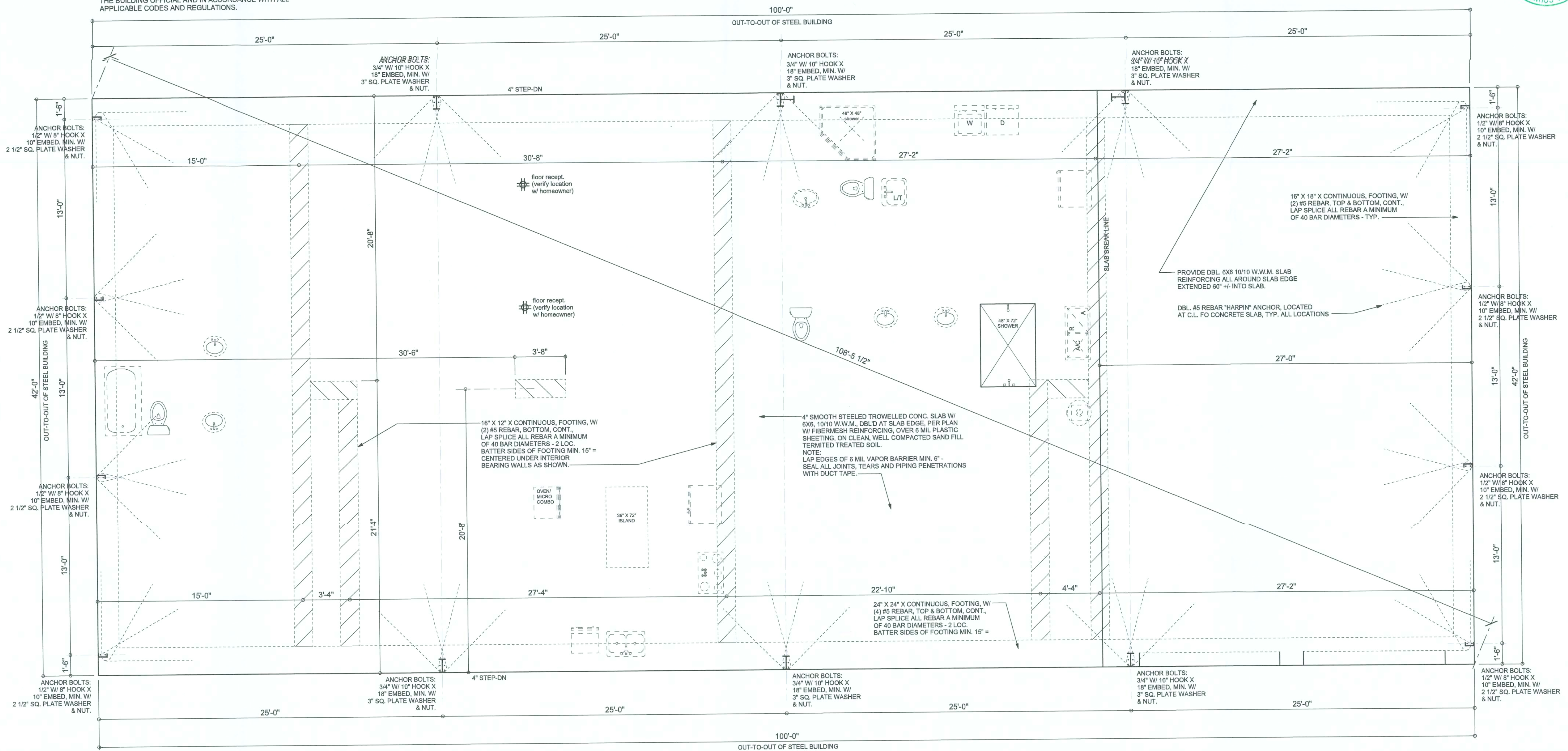
PLAN:
ZONE LAYOUT

JOB # 11162-ZL
SHEET 10 OF 11

6-9-19

INTERIOR BEARING WALLS:
IT IS THE BUILDING CONTRACTOR'S RESPONSIBILITY TO VERIFY WITH THE TRUSS ENGINEERING ANY AND ALL INTERIOR BEARING WALL LOCATIONS AND FURNISH THE ENGINEER OR ARCHITECT OF RECORD TRUSS INFO SO THICKENED FOOTINGS CAN BE SIZED AND LOCATED ON THE FOUNDATION PLAN.

PLUMBING - ALL WORK SHALL COMPLY WITH THE STANDARD PLUMBING CODE. PLUMBING CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWING INDICATING ALL PLUMBING WORK INCLUDING RISER DIAGRAM, BACK FLOW PROTECTION, PIPE LOCATIONS, WATER SUPPLY TO THE FACILITY IS TO BE CONNECTED THROUGH A REDUCED PRESSURE ZONE BACKFLOW PREVENTER ACCEPTABLE TO THE BUILDING OFFICIAL AND IN ACCORDANCE WITH ALL APPLICABLE CODES AND REGULATIONS.



FOUNDATION PLAN
SCALE 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

NOTE: THIS FOUNDATION DESIGN MEETS ALL REQUIREMENTS FOR WIND LOADS PER PROJECT IS 130 MPH PER 2017 PER R301.2.1.1 BASIC WIND SPEED, EXPOSURE B, 1.0 USE FACTOR. COLUMN PAD LOCATIONS ARE TYPICAL, EXACT ANCHOR BOLT LOCATIONS AND SIZES ARE PER METAL BUILDING SEALED ENGINEERING ANCHOR BOLT PLAN. CONTRACTOR SHALL FURNISH ARCHITECT SEALED METAL BUILDING PLANS FOR FOUNDATION SIZING AND VERIFICATION PRIOR TO SUBMITTING TO PERMIT OFFICE.

- ANCHOR BOLTS AND REINFORCEMENT - SEE ANCHOR BOLT NOTES AT EACH COLUMN LINE. TIE ANCHOR BOLTS TO BOTTOM REINFORCING STEEL.
REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI DETAILING MANUAL, SP-66, AND ACI318. REINFORCING SHALL NOT BE HEATED OR WELDED. REINFORCING SHALL BE APPROVED BY ENGINEER OR HIS REPRESENTATIVE BEFORE CONCRETE IS PLACED. PROVIDE 4" COVER FOR EXPOSED FOOTING SURFACES, 2" COVER FOR FORMED EXPOSED SURFACES, 3/4" COVER FOR NOT EXPOSED SURFACES. LAP SPLICES SHALL BE 40 BAR DIAMETERS.

- CONCRETE - MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE $F'_c = 3,000$ PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

- CONTROL JOINTS - SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

- FOUNDATION - THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 2000 PSF SHALL BE CONFIRMED IN THE FIELD BY A REGISTERED GEOTECHNICAL ENGINEER OR SHALL BE APPROVED BY THE OWNER. FOOTINGS AND SLABS ARE TO BEAR ON FIRM UNDISTURBED EARTH OR APPROVED CONTROLLED FILL. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL.

- UNLESS OTHERWISE SPECIFIED ALL MATERIALS AND CONSTRUCTION ARE TO MEET LOCAL BUILDING CODES.

- CONCRETE - MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE $F'_c = 3,000$ PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

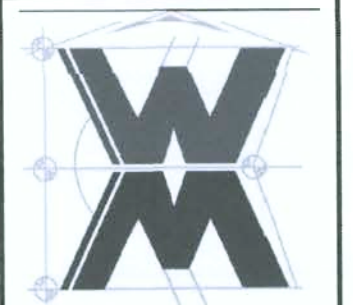


A CUSTOM HOME BUILT-OUT FOR:
MARK & ANGIE SMITH
PROJECT ADDRESS: COLUMBIA COUNTY, FLORIDA

AR0007005

NICHOLAS PAUL GEISLER
ARCHITECT
N.C.A.R.B. Certified

JOINT VENTURED WITH
WM DESIGN & ASSOCIATES, INC.
428 SW COMMERCE DR., STE 130
LAKE CITY, FL 33505
(386) 758-8406
will@willmayers.net



JOB NUMBER
20190801
DATE:
August 31, 2019

SHEET NUMBER
S.1