



FL REG# 278, Yoonhwak Kim, FL PE #86367 Florida Certificate of Product Approval #FL 1999 11/18/2022 Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com



Site Information:	Page 1:	
Customer: W. B. Howland Company, Inc.	Job Number: 22-8515	
Job Description: Dave Blank		
Address: Little Rd. LAKE CITY		

Job Engineering Criteria:				
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.01			
	JRef #: 1XKQ2150005			
Wind Standard: ASCE 7-16 Wind Speed (mph): 150	Design Loading (psf): 47.00			
Building Type: Closed				

This package contains general notes pages, 17 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	322.22.0643.33020	A01
3	322.22.0643.37427	A03
5	322.22.0643.41827	B01
7	322.22.0643.45550	C01
9	322.22.0644.00757	C03G
11	321.22.1656.48079	C05G
13	321.22.1656.48017	HJ01
15	321.22.1656.48096	J02
17	321.22.1656.48049	J04
19	GBLLETIN0118	

Item	Drawing Number	Truss
2	322.22.0643.34853	A02
4	322.22.0643.39703	A04
6	322.22.0643.43600	B02
8	322.22.0643.48570	C02
10	322.22.0644.03080	C04
12	322.22.0644.21477	G01
14	321.22.1656.48032	J01
16	321.22.1656.48018	J03
18	A16015ENC160118	

# **General Notes**

# Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

## **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

## **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

### Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

# **General Notes** (continued)

# **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

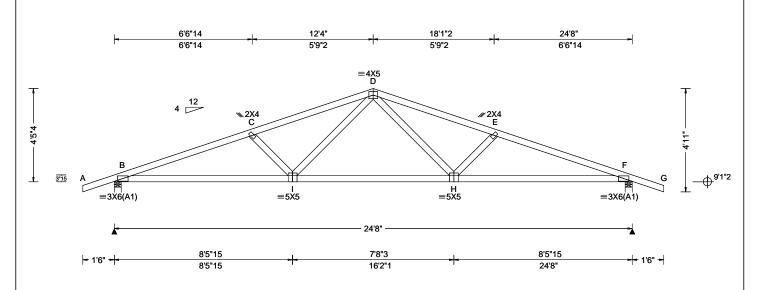
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

### References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; <a href="https://www.alpineitw.com">www.alpineitw.com</a>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 448842 COMN Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T6 FROM: CDM Qty: 7 Dave Blank DrwNo: 322.22.0643.33020 Truss Label: A01 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
Loading Criteria (psf)   TCLL: 20.00   TCDL: 17.00   BCLL: 0.00   BCDL: 10.00   Des Ld: 47.00   NCBCLL: 10.00   Soffit: 2.00   Load Duration: 1.25   Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.122 D 999 360 VERT(CL): 0.286 D 999 298 HORZ(LL): 0.037 F HORZ(TL): 0.088 F Creep Factor: 2.0 Max TC CSI: 0.518 Max BC CSI: 0.854 Max Web CSI: 0.271	
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 21.02.01.1214.12	-   -
Lumber	•	•	•	- (

<b>▲</b> N	▲ Maximum Reactions (lbs)						
	G	ravity			lon-Grav	vity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	1302	/-	/-	/880	/367	/152	
F	1302	/-	/-	/880	/367	/-	
Wi	nd read	ctions b	ased or	MWFRS	;		
В	Brg V	Vid = 4.	.0 Mir	n Req = 1	.5 (Trus	s)	
F	Brg V	Vid = 4.	.0 Mir	n Req = 1	.5 (Trus:	s)	
Bea	arings	B&Fa	re a rig	id surface		•	
Me	mbers	not list	ed have	forces les	ss than 3	375#	
Ma	ximun	Top C	hord F	orces Pe	r Ply (lb	s)	
Ch	ords 7	Tens.Co	omp.	Chords	Tens.	Ćomp.	
В-	С	1452 -	2724	D-E	1309	- 2397	
٦Ē-	-	1309 -		E-F	1453		

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

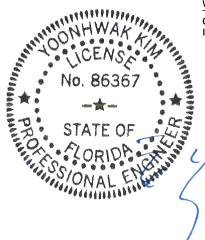
The overall height of this truss excluding overhang is



noras	rens.comp.	Choras	rens. Comp.
3 - I	2525 - 1285	H-F	2525 - 1261
- H	1723 - 798		

### Maximum Web Forces Per Ply (lbs)

vvebs	rens.comp.	webs	rens. C	Jomp.
C - I	412 - 479	D-H	710	- 312
I - D	710 - 312	H - E	412	- 479



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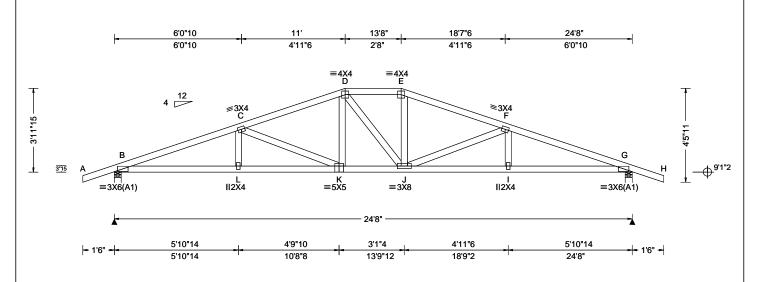
\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.



SEQN: 448840 HIPS Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T5 FROM: CDM Qty: 1 Dave Blank DrwNo: 322,22,0643,34853 Truss Label: A02 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.125 K 999 360 VERT(CL): 0.295 K 990 298 HORZ(LL): 0.041 G HORZ(TL): 0.097 G Creep Factor: 2.0 Max TC CSI: 0.413 Max BC CSI: 0.671 Max Web CSI: 0.394
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
Lumber	•	•	

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 1302 /-/881 /368 /139 1302 /-/-/881 /368 /-Wind reactions based on MWFRS Brg Wid = 4.0Min Reg = 1.5 (Truss) Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1337 - 2786 1106 - 2086 C-D 1112 - 2097 1336 D-E 1095 - 1903

#### Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

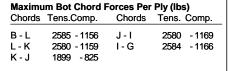
### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

The overall height of this truss excluding overhang is



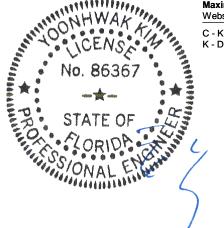
#### Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K 377 354 - 711 E - J - 118

J - F

358

- 718

376 - 109



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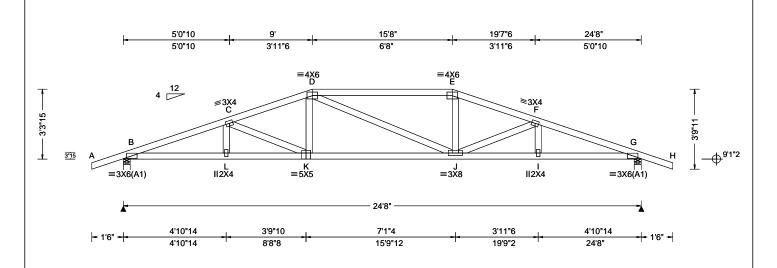
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SEQN: 448838 HIPS Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T4 FROM: CDM Qty: 1 Dave Blank DrwNo: 322.22.0643.37427 Truss Label: A03 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.127 D 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.300 D 975 298
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.041 G
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.097 G
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.785
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.754
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.150
'	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
Lumber		•	•

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 1302 /-/878 /369 /119 1302 /-/878 /369 /-Wind reactions based on MWFRS Brg Wid = 4.0Min Reg = 1.5 (Truss) Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1587 - 2806 1490 - 2390 C - D 1494 - 2400 1587 - 2806 D-E 1471 - 2219

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

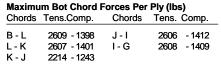
### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

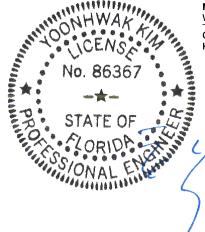
## **Additional Notes**

The overall height of this truss excluding overhang is



#### Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs

Tens. Comp. C - K - 400 384 172 E - J - 17 K - D 384 - 20 J-F 176 - 407



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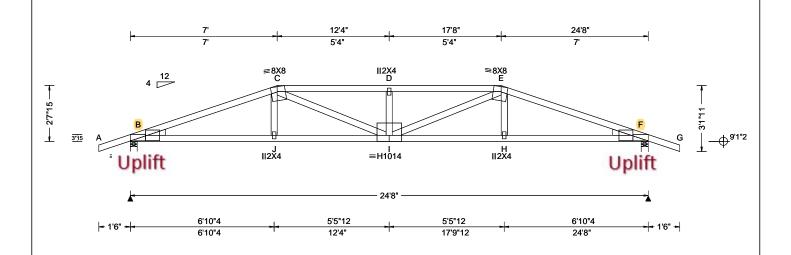
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SEQN: 448847 HIPS Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T8 FROM: CDM DrwNo: 322.22.0643.39703 Qty: 1 Dave Blank Truss Label: A04 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.345 D 846 360	ı
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.811 D 360 298	1
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.082 F	К
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.193 F	
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.966	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.881	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.742	
' '	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	HS, WAVE	VIEW Ver: 21.02.01.1214.12	
Lumber	•	•		_

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 2749 /-/819 /-2749 /-/-Wind reactions based on MWFRS Brg Wid = 4.0Min Reg = 2.3 (Truss) Brg Wid = 4.0 Min Req = 2.3 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 2087 - 7109 2464 - 8406 2464 - 8406 E-F 2087

Maximum Bot Chord Forces Per Ply (lbs)

Chords

H-F

Webs

D-I

E-H

Tens. Comp.

6666

506

649

6626 - 1944

Tens. Comp.

- 1947

- 996

- 46

Chords Tens.Comp.

J - I

6666 - 1947

6626 - 1944

Tens.Comp.

649 - 46

1947 - 569

1947 - 569

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;

Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

### Special Loads

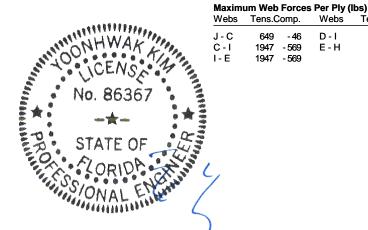
Opcolar Load				
(Lumber	Dur.Fac.=1.	25 / Plate [	Dur.Fac.=1.2	25)
TC: From	76 plf at	-1.50 to	76 plf at	7.00
TC: From	38 plf at	7.00 to	38 plf at	17.67
TC: From	76 plf at	17.67 to	76 plf at	26.17
BC: From	4 plf at	-1.50 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	7.03
BC: From	10 plf at		10 plf at	17.64
BC: From	20 plf at		20 plf at	24.67
BC: From		24.67 to	4 plf at	26.17
	Conc. Load			
	Conc. Load	at 9.06,11	.06,12.33,13	3.60
15.60				
	Conc. Load			
	Conc. Load	at 9.06,11	.06,12.33,1	3.60
15.60				

### Wind

Wind loads and reactions based on MWFRS Wind loading based on both gable and hip roof types.

## **Additional Notes**

The overall height of this truss excluding overhang is 2-7-15.



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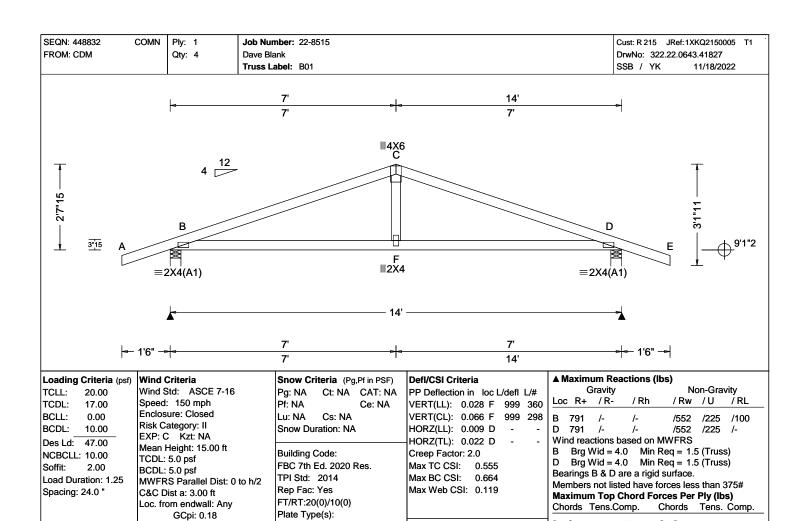
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025



#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

### Wind

Wind loads based on MWFRS with additional C&C

Wind Duration: 1.60

Wind loading based on both gable and hip roof types.

## **Additional Notes**

The overall height of this truss excluding overhang is



C-D

1016 - 1197

1016 - 1197

B - C



VIEW Ver: 21.02.01.1214.12

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WAVE

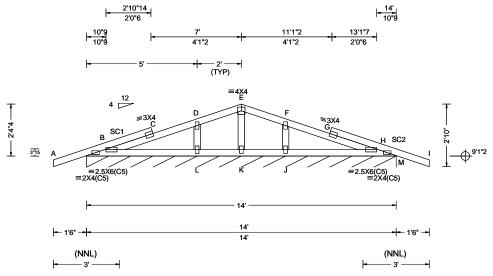
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SEQN: 448830 GABL Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T2 FROM: CDM Qty: 1 DrwNo: 322.22.0643.43600 Dave Blank Truss Label: B02 SSB / YK 11/18/2022



TCLL: 20.00						
TCDL: 17.00   Speed: 150 mph   Pf: NA   Ce: NA   VERT(LL): 0.013 C   999   360   Loc R	Loading Criteria (psf)	ding Criter	osf) Wind Criteria	Snow Criteria (Pg,Pf i	in PSF) Defl/CSI Criteria	▲ Maximum
Spacing: 24.0   C&C Dist a: 3.00 ft	TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	L: 20.00 DL: 17.00 L: 0.00 DL: 10.00 Ld: 47.00 BCLL: 10.00 it: 2.00 d Duration:	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18	Pg: NA Ct: NA CAPF: NA CAPF: NA Cs: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	AT: NA e: NA PP Deflection in loc L/defl L/2 VERT(LL): 0.013 C 999 3 VERT(CL): 0.031 C 999 2 HORZ(LL): 0.004 C - HORZ(TL): 0.010 C - Creep Factor: 2.0 Max TC CSI: 0.319 Max BC CSI: 0.143 Max Web CSI: 0.084	# Grave Base Base Base Base Base Base Base Bas

	▲ Maximum Reactions (lbs), or *=PLF							
		Gravity		N	on-Gra	vity		
)	Loc R+	· /R-	/ Rh	/ Rw	/ U	/ RL		
,	M* 113	/-	/-	/65	/31	/7		
	Wind re	actions b	ased or	MWFRS				
	M Brg	Wid = 1	68 Mir	n Req = -				
	Bearing	B is a rig	gid surfa	ice.				
	Member	s not list	ed have	forces les	s than	375#		
	Maximu	ım Gable	Force	s Per Ply (	lbs)			
	Gables	Tens.Co	omp.	Gables	Tens.	Comp.		
	D-L	393	- 280	J-F	394	- 280		

### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2;

Stack Chord: SC2 2x4 SP #2;

## **Plating Notes**

All plates are 2X4 except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

See DWGS A16015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 2-4-4.



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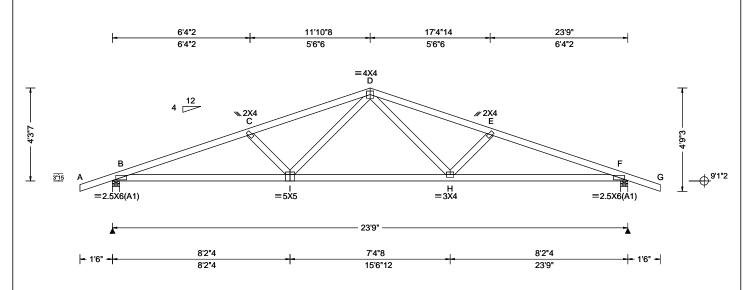
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SEQN: 448850 COMN Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T3 FROM: CDM Qty: 8 Dave Blank DrwNo: 322.22.0643.45550 Truss Label: C01 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	4
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	Defl/CSI Criteria	
Lumber	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s):  WAVE	VIEW Ver: 21.02.01.1214.12	] [

▲ Maximum Reactions (lbs)							
	Gravity		No	on-Grav	vity		
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
B 125	8 /-	/-	/852	/355	/147		
F 125	8 /-	/-	/852	/355	/-		
Wind re	actions b	ased on	MWFRS				
B Brg	Wid = 4.	0 Min	Req = 1.5	(Trus	s)		
F Brg	Wid = 4.	0 Min	Req = 1.5	(Trus	s)		
Bearing	sB&Fa	re a rigio	surface.	•	•		
Member	s not list	ed have t	forces less	s than 3	375#		
Maximum Top Chord Forces Per Ply (lbs)							
Chords	Tens.Co	omp.	Chords	Tens.	Ćomp.		
B-C	1450 -	2610	D-E	1307	- 2299		
C-D	1307 -		E-F	1450	- 2611		

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

## **Additional Notes**

The overall height of this truss excluding overhang is

# Maximum Bot Chord Forces Per Ply (lbs)

Cnoras	rens.comp.	Cnoras	rens. Comp.
B-I	2418 - 1279	H-F	2419 - 1257
I - H	1655 - 793		

## Maximum Web Forces Per Ply (lbs)

webs rens.comp. webs rens.	
	- 314 - 457



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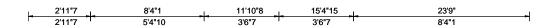
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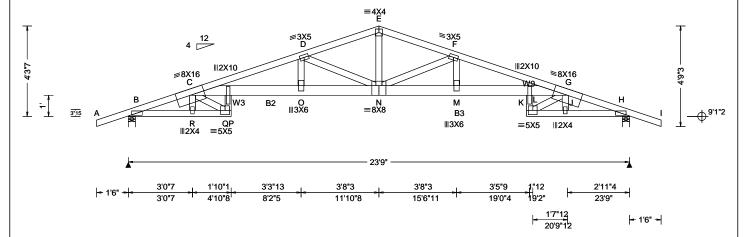
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SEQN: 448859 COMN Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T16 FROM: CDM Qty: 6 DrwNo: 322.22.0643.48570 Dave Blank Truss Label: C02 SSB / YK 11/18/2022





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	T
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.329 L 854 360	ı
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.775 L 363 298	ı
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.174 H	ı
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.408 H	
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.443	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.813	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.571	
'	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12	
Lumbor		•		_

#### Loc R+ /Rh /Rw / U /RL В 1258 /-/851 /354 /184 1258 /851 /356 /-Wind reactions based on MWFRS Brg Wid = 4.0Min Reg = 1.5 (Truss) Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 1087 - 2557 1062 - 2380 C-D 1698 - 4062 F-G 1652 - 4062

Non-Gravity

1096 - 2557

▲ Maximum Reactions (lbs) Gravity

D-E

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP #2; B2,B3 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W3,W9 2x4 SP #2;

### **Plating Notes**

All plates are 2.5X6(A1) except as noted.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is 4-3-7



# Maximum Bot Chord Forces Per Ply (lbs)

1053 - 2380

Chords	Tens.Comp.	Chords	Tens. Comp.	
B-R	2370 - 1000	N - M	3761 - 1443	
C - Q	3903 - 1557	M - K	3810 - 1457	
R-P	2356 - 986	L-J	2356 - 950	
Q - O	3810 - 1518	K-G	3903 - 1472	
O - N	3761 - 1503	J - H	2370 - 964	

G-H

#### Maximum Web Forces Per Ply (lbs) Tens Comp Webs Tens Comp

******	rono.comp.	******	rono. Comp.	
C-P	1165 - 2785	N - F	755 - 1722	_
Q - P	1340 - 551	F-M	844 - 238	
O - D	844 - 256	L-K	1340 - 522	
D - N	781 - 1722	L-G	1097 - 2785	
E - N	1266 - 506			

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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 450673 COMN Ply: 3 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T14 Qty: 2 FROM: CDM DrwNo: 322.22.0644.00757 Dave Blank Truss Label: C03G SSB / YK 11/18/2022 3 Complete Trusses Required 11'10"8 7'0"1 9'9"12 13'11"4 17'0"7 23'9' 2'0"12 7'0"1 2'9"11 2'0"12 3'1"3 6'8"9 =4X4 ≢2.5X6 D ≥2.5X6 **∥2X10 ||2X10** ≝H1<u>,</u>021 ≥H1021 s R вз 0 ⊪3X6 **⊕**9'1"2 B2 **∥3**X6 V ⊪2X4(\*\*) ≡5X5 =5X5 Uplift Uplift 1'8"5 2'9"11 2'0"12 2'0"12 1'10"1 2'11"4 4'10"8 11'10"8 13'11"4 18'10"8 3'2"3 9'9"12 17'0"7 23'9" 2'1"9 1'11"4 **→** 1'6" <del>→</del> 7'0"1 20'9"12 Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs) Non-Gravity Wind Std: ASCE 7-16 Ct: NA CAT: NA Gravity Pg: NA TCLL: 20.00 PP Deflection in loc L/defl L/# Loc R+ /R /Rh /Rw / U /RL Speed: 150 mph TCDL: 17.00 Pf: NA VERT(LL): 0.279 Q 999 360 Ce: NA Enclosure: Closed BCII: 0.00 Lu: NA Cs: NA VERT(CL): 0.680 Q 413 298 В 2654 /-Risk Category: II BCDL: 10.00 Snow Duration: NA HORZ(LL): 0.136 J /-2654 /-/663 EXP: C Kzt: NA Wind reactions based on MWFRS HORZ(TL): 0.332 J Des Ld: 47.00 Mean Height: 15.00 ft Brg Wid = 4.0Min Reg = 1.5 (Truss) В NCBCLL: 0.00 **Building Code:** Creep Factor: 2.0 TCDL: 5.0 psf Brg Wid = 4.0 Min Req = 1.5 (Truss) FBC 7th Ed. 2020 Res. Max TC CSI: 0.636 Soffit: 2.00 BCDL: 5.0 psf Bearings B & J are a rigid surface. TPI Std: 2014 Max BC CSI: 0.651 Load Duration: 1.25 MWFRS Parallel Dist: 0 to h/2 Members not listed have forces less than 375# Rep Fac: Yes Max Web CSI: 0.518 Spacing: 24.0 ' C&C Dist a: 3.00 ft Maximum Top Chord Forces Per Ply (lbs) Loc. from endwall: not in 9.00 ft FT/RT:20(0)/10(0) Chords Tens.Comp. Chords Tens. Comp. Plate Type(s): GCpi: 0.18 B - C 495 - 2072 580 - 2509 VIEW Ver: 21.02.01.1214.12 Wind Duration: 1.60 WAVE. HS C - D 993 - 4203 G-H 728 - 3115 Lumber Wind D-E 728 - 3115 H - I 993 - 4203 Top chord: 2x4 SP #2; Wind loads and reactions based on MWFRS. 580 - 2509 495 - 2072 Bot chord: 2x4 SP #2; B2,B3 2x6 SP 2400f-2.0E; Wind loading based on both gable and hip roof types. Webs: 2x4 SP #3; Maximum Bot Chord Forces Per Ply (lbs) **Additional Notes Nailnote** Chords Tens.Comp. Chords Tens. Comp. The overall height of this truss excluding overhang is Nail Schedule:0.128"x3", min. nails B - V 1933 Q - P - 460 2873 -668 4-3-7. It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conformed the shottererural plans/specifications and fabricators truss layout.

No. 86367

STATE OF Top Chord: 1 Row @ 8.50" o.c. Bot Chord: 1 Row @12.00" o.c. 4066 P - O C - U - 957 3927 - 926 O - M V - T 1917 - 455 3981 - 938 :1 Row @ 4" o.c. U-S 3981 - 938 N-I 1917 - 455 Repeat nailing as each layer is applied. Use equal S - R 3927 - 926 4066 M - I - 957 spacing between rows and stagger nails in each row R - Q 2873 - 668 1933 - 460 to avoid splitting Special Loads Maximum Web Forces Per Ply (lbs) --(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) Webs Tens.Comp. Webs Tens. Comp. -1.50 to TC: From 76 plf at 76 plf at C - T 538 - 2270 Q - G 187 - 780 TC: From 138 plf at 9.67 to 138 plf at 9.69 U - T G-P 1090 - 256 665 - 154 TC: From 266 plf at 9.69 to 266 plf at 14.06 S - D P - H TC: From 688 - 1084 138 plf at 14.06 to 14.08 - 148 266 138 plf at TC: From 76 plf at 76 plf at 14.08 to 25.25 H-O D-R 266 - 1084 688 - 148 BC: From 4 plf at -1.50 to 4 plf at 0.00 R-E 665 - 154 N - M 1090 - 256 BC: From 20 plf at 0.00 to 20 plf at 23.75 E - Q 187 - 780 N - I 538 - 2270 BC: From 4 plf at 23.75 to 4 plf at 25.25 / F - Q 1360 - 317 BC: 978 lb Conc. Load at 9.81,13.94 **Plating Notes** " The state of the All plates are 3X4 except as noted. (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning

requirements.

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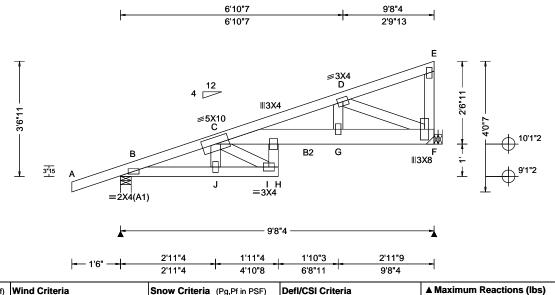
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SEQN: 448865 MONO Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T15 FROM: CDM DrwNo: 322.22.0644.03080 Qty: 4 Dave Blank Truss Label: C04 SSB / YK 11/18/2022



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.054 H 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.126 H 910 298
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 F
Des Ld: 47.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.035 F
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.266
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.536
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.227
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
Lumber		Wind	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2 2x6 SP #2; Webs: 2x4 SP #3;

### **Plating Notes**

All plates are 2X4 except as noted.

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating

Bearing at location x=9'5"4 uses the following support conditions: 9'5"4 Bearing F (95"4, 10"1"2) LUS26 Supporting Member: (2)2x6 SP #2 (4) 0.148"x3" nails into supporting

member,
(3) 0.148"x3" nails into supported

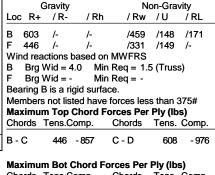
member.

### Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

The overall height of this truss excluding overhang is 3-6-11



Maximum Bot Chord Forces Per Ply (lbs)						
Chords	Tens.Comp.		Chords	hords Tens. (		
B-J		- 648	I - G	887	- 718	
C - I	922	- 746	G-F	858	- 702	
J - H	783	- 645				

#### Maximum Web Forces Per Ply (lbs) Tens. Comp. Webs Tens.Comp. Webs C-H 762 - 927 G - D 380 - 189 - 356 D-F 761

- 931

458



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SEQN: 448872 / COMN Ply: 3 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T7 FROM: CDM DrwNo: 321,22,1656,48079 Qty: 1 Dave Blank Truss Label: C05G SSB / YK 11/17/2022 3 Complete Trusses Required 4'10" 8'4"4 11'10"8 15'4"12 18'11" 23'9" 4'10' 3'6"4 3'6"4 3'6"4 3'6"4 4'10' ∥4X6 ≢4X6 D ≥4X6 <sup>≷</sup>2X4 C 4'3"7 W4 =4X6(A4) B =4X6(A4 ⊕<sup>9'1"2</sup> 3"15 ⊩ ⊪5X6 =10X10 ∭5X6 23'9' 6'7"2 5'3"6 5'3"6 6'7"2 <del>|-</del> 1'6" <del>-|</del> 6'7"2 11'10"8 17'1"14 23'9' Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs) Non-Gravity Wind Std: ASCE 7-16 Ct: NA CAT: NA Gravity TCLL: 20.00 Pg: NA PP Deflection in loc L/defl L/# Loc R+ /R /Rh /Rw /U / RL Speed: 150 mph TCDL: 17.00 Pf: NA VERT(LL): 0.248 K 999 Ce: NA 360 Enclosure: Closed VERT(CL): 0.586 K BCII: 0.00 Lu: NA Cs: NA 480 298 M 9023 /-/2400 /-Risk Category: II BCDL: 10.00 Snow Duration: NA HORZ(LL): 0.048 H 9023 /2400 /-EXP: C Kzt: NA Wind reactions based on MWFRS HORZ(TL): 0.114 H Des Ld: 47.00 Mean Height: 15.00 ft Brg Wid = 4.0Min Reg = 2.5 (Truss) **Building Code:** Creep Factor: 2.0 NCBCLL: 0.00 TCDL: 5.0 psf Brg Wid = 4.0 Min Req = 2.5 (Truss) FBC 7th Ed. 2020 Res. Max TC CSI: 0.848 Soffit: 2.00 BCDL: 5.0 psf Bearings M & H are a rigid surface. TPI Std: 2014 Max BC CSI: 0.730 Load Duration: 1.25 MWFRS Parallel Dist: 0 to h/2 Members not listed have forces less than 375# Rep Fac: Yes Max Web CSI: 0.654 Spacing: 24.0 ' C&C Dist a: 3.00 ft Maximum Top Chord Forces Per Ply (lbs) Loc. from endwall: NA FT/RT:20(0)/10(0) Tens. Comp.

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W4 2x4 SP #2;

### **Nailnote**

Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @ 3.50" o.c. Webs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

GCpi: 0.18

Wind Duration: 1.60

## Loading

Girder supports 30-0-0 spans to BC one face.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is 4-3-7.

VIEW Ver: 21.02.01.1214.12

## Chords Tens.Comp. Chords

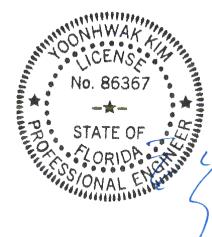
1256 B - C 1839 - 7001 C - D 1813 - 6895 F-G 1813 - 6895 D-E 1256 - 4777 1839 - 7001 G-H

# Maximum Bot Chord Forces Per Ply (lbs)

Cilolus	rens.comp.	Ciloius	rens. Comp.	
B-L	6643 - 1745	K-J	5624 - 1478	
L-K	5624 - 1478	J - H	6643 - 1745	

# Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Tens. Comp.		
L-D	1693 - 446	K-F	363 - 1385	
D-K	363 - 1385	F-J	1693 - 446	
	2007 704			



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Plate Type(s):

<u>WA</u>VE

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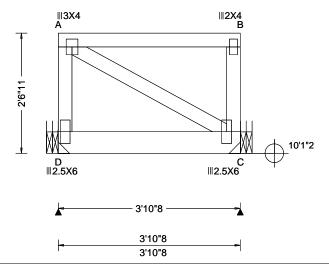
\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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SEQN: 450661 FLAT Ply: 2 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T19 FROM: CDM Qty: 2 DrwNo: 322.22.0644.21477 Dave Blank Truss Label: G01 SSB / YK 11/18/2022

# 2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.000 B 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 B 999 298
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B
Des Ld: 47.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.001 B
NCBCLL: 0.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.580
	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.439
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: No	Max Web CSI: 0.111
	Loc. from endwall: not in 10.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
I		A dalista and Massa	

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 60 D 978 /239 978 /-/-/-/239 Wind reactions based on MWFRS Brg Wid = -Min Reg = -D Brg Wid = -Min Reg = -Members not listed have forces less than 375#

### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x6 SP #2; Webs: 2x4 SP #3;

### **Nailnote**

Nail Schedule:0.128"x3", min. nails Top Chord: 1 Row @11.50" o.c. Bot Chord: 1 Row @ 6.50" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails

in each row to avoid splitting.

### **Special Loads**

--(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 264 plf at 0.00 to 264 plf at BC: From 10 plf at 0.00 to 10 plf at 3.88 BC: 446 lb Conc. Load at 1.81, 2.06

### Hangers / Ties

(J) Hanger Support Required, by others

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

### **Additional Notes**

Truss must be installed as shown with top chord up. Wall girder loading on this truss.

The overall height of this truss excluding overhang is 2-6-11.



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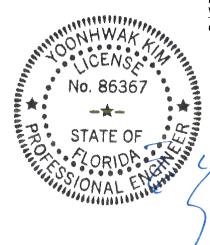
SEQN: 448843 / HIP\_ Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T13 Ply: 1 FROM: CDM DrwNo: 321.22.1656.48017 Qty: 2 Dave Blank Truss Label: HJ01 SSB / YK 11/17/2022 5'8"3 9'10"13 5'8"3 4'2"9 D 11'5"6 2.83 **≤3X4** С В 3"12 9'1"2 G ∥2X4 FΕ **≡4X4**  $\equiv$ 2X4(A1) 5'6"7 3'9"8 6"14 9'10"13 5'6"7 9'3"15 Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs) Gravity Non-Gravity Wind Std: ASCE 7-16 Pg: NA Ct: NA CAT: NA TCLL: 20.00 PP Deflection in loc L/defl L/# Loc R+ /R /Rh /Rw / U /RL Speed: 150 mph TCDL: 17.00 Pf: NA Ce: NA VERT(LL): 0.034 G 999 360 Enclosure: Closed VERT(CL): 0.075 G BCII: 0.00 Lu: NA Cs: NA 999 298 Н 470 /-/270 /-Risk Category: II BCDL: 10.00 Snow Duration: NA HORZ(LL): 0.007 F 371 /-/-Е /118 EXP: C Kzt: NA 84 /30 HORZ(TL): 0.016 F Des Ld: 47.00 Mean Height: 15.00 ft Wind reactions based on MWFRS **Building Code:** Creep Factor: 2.0 NCBCLL: 10.00 TCDL: 5.0 psf Brg Wid = 5.7 Min Req = 1.5 (Truss) FBC 7th Ed. 2020 Res. Max TC CSI: 0.602 Soffit: 2.00 Brg Wid = 1.5 BCDL: 5.0 psf Min Req = TPI Std: 2014 Max BC CSI: 0.734 Load Duration: 1.25 MWFRS Parallel Dist: 0 to h/2 Brg Wid = 1.5 Min Reg = -Rep Fac: Varies by Ld Case Max Web CSI: 0.411 Spacing: 24.0 " C&C Dist a: 3.00 ft Bearing H is a rigid surface. FT/RT:20(0)/10(0) Loc. from endwall: not in 4.50 ft Members not listed have forces less than 375# GCpi: 0.18 Plate Type(s): Maximum Top Chord Forces Per Ply (lbs) VIEW Ver: 21.02.01.1214.12 Wind Duration: 1.60 WAVE Chords Tens.Comp. Lumber B - C 439 - 1040 Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Maximum Bot Chord Forces Per Ply (lbs) Webs: 2x4 SP #3; Chords Tens.Comp. Chords Tens. Comp. **Special Loads** B - G 1016 - 403 G-F 996 - 402 --(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) 0.00 TC: From 0 plf at -2.12 to 0.00 to 75 plf at 2 plf at Maximum Web Forces Per Ply (lbs) TC: From 2 plf at 0 plf at 9.90 Tens.Comp. Webs BC: From -2.12 to 4 plf at 0.00 2 plf at 0.00 to BC: From 2 plf at C-F 425 - 1053 -55 lb Conc. Load at 1.48 148 lb Conc. Load at 4.31 304 lb Conc. Load at 7.13 TC: TC: BC: -8 lb Conc. Load at 1.48

95 lb Conc. Load at 4.31 182 lb Conc. Load at 7.13

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

## **Additional Notes**

The overall height of this truss excluding overhang is 2-7-12



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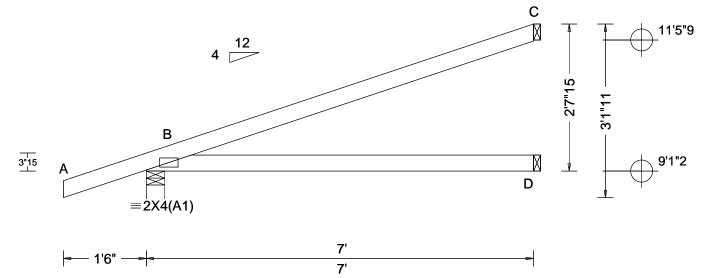
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SEQN: 448836 / **EJAC** Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T12 FROM: CDM Qty: 7 Dave Blank DrwNo: 321.22.1656.48032 Truss Label: J01 SSB / YK 11/17/2022



Loading Criteria	(psf) Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (	bs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1. Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 B HORZ(TL): 0.034 B Creep Factor: 2.0 Max TC CSI: 0.850 Max BC CSI: 0.538 Max Web CSI: 0.000	Gravity Loc R+ /R- /Rh  B 478 /- /- D 132 /- /- C 224 /- Wind reactions based on B Brg Wid = 4.0 Min D Brg Wid = 1.5 Min	Non-Gravity / Rw / U / RL /371 /121 /128 /77 /- /- /154 /116 /- MWFRS Req = 1.5 (Truss) Req = - Req = - e.
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s):	VIEW Ver: 21.02.01.1214.12		

### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is



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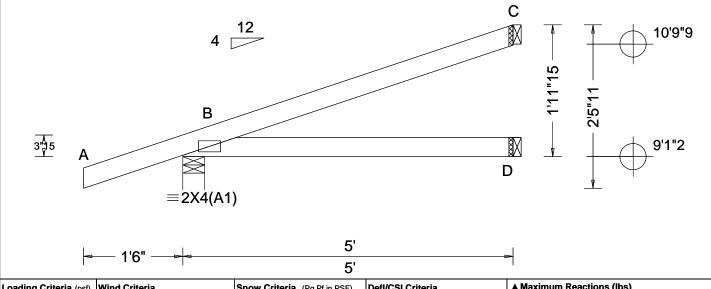
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 448835 / JACK Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T9 FROM: CDM Qty: 4 Dave Blank DrwNo: 321.22.1656.48096 Truss Label: J02 SSB / YK 11/17/2022



Defl/CSI Criteria

Loading Criteria (psi)	Willia Criteria	Show Cinteria (Pg,Pi in PSF)	Deli/Coi Ciliteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.010 B
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.367
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.239
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000
'	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
Lametra			

Snow Criteria (Pa Pf in PSE)

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 389 /309 /102 /97 D 91 /-/51 /103 152 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 1.5 Min Req = -Brg Wid = 1.5 Min Req = -Bearing B is a rigid surface. Members not listed have forces less than 375#

### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is



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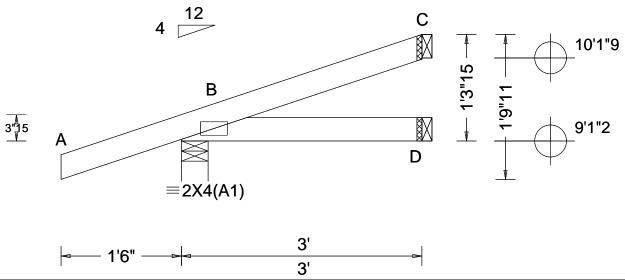
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 SEQN: 448834 / JACK Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T10 FROM: CDM Qty: 4 Dave Blank DrwNo: 321.22.1656.48018 Truss Label: J03 SSB / YK 11/17/2022



Defl/CSI Criteria

Luading Criteria (psi)	Willu Cillella	SHOW CITIETIA (FG,FI III FSF)	Deli/Col Cillella
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 B
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.001 B
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.209
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.059
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12
Lumber	•		

Snow Criteria (Pa Pf in PSE)

▲ Maximum Reactions (lbs)								
Gravity Non-Gravity								
Loc	: R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
В	309	/-	/-	/256	/96	/65		
D	48	/-	/-	/27	/-	/-		
С	74	/-	/-	/46	/40	/-		
Wir	nd read	ctions b	ased on N	<b>MWFRS</b>				
В	Brg V	Vid = 4	.0 Min F	Req = 1.5	(Trus	s)		
D	Brg V	Vid = 1	.5 Min F	eq = -	•	•		
С	Brg V	Vid = 1	.5 Min F	?eq = -				
Bearing B is a rigid surface.								
Mei	mbers	not list	ed have fo	rces les	s than	375#		

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Loading Criteria (nef) Wind Criteria

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is 1-3-15.



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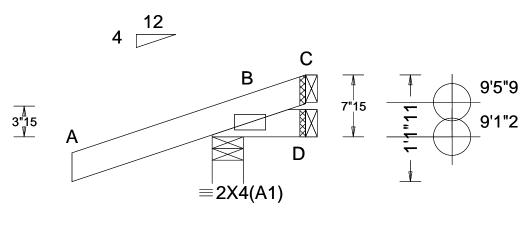
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 448833 / JACK Ply: 1 Job Number: 22-8515 Cust: R 215 JRef: 1XKQ2150005 T11 FROM: CDM Qty: 4 Dave Blank DrwNo: 321.22.1656.48049 Truss Label: J04 SSB / YK 11/17/2022



<del></del> 1'6"	_ا_	1'	_
10		1'	

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	Defi/CSI Criteria
GCpi: 0.18 Wind Duration: 1.60		Plate Type(s): WAVE	VIEW Ver: 21.02.01.1214.12
Lumbor	•		•

▲ Maxim	um Rea	ctions (II	os)			
G	ravity		No	on-Grav	vity	
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
B 302	/-	/-	/272	/141	/37	
D -	/-25	/-	/27	/27	/-	
C -	/-61	/-	/48	/63	/-	
Wind read	ctions b	ased on N	/WFRS			
B Brg V	Vid = 4.	0 Min F	Req = 1.5	(Trus	s)	
D Brg V	Vid = 1.	5 Min F	Req = -	•	•	
		5 Min F				
Bearing B is a rigid surface.						
Members	not liste	ed have fo	orces les	s than 3	375#	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.

### **Additional Notes**

The overall height of this truss excluding overhang is



FL REG# 278, Yoonhwak Kim, FL PE #86367 Flotidis@@2fficate of Product Approval #FL 1999

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# Gable Stud Reinforcement Detail

ASCE 7-16: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D. Kzt = 1.00

					Or		Wind Spee							)
		2x4 Vertico	Brace	No	(1) 1×4 *L	" Brace *	(1) 2×4 *L	." Brace *	(2) 2×4 *L	" Brace **	(1) 2x6 <b>'</b> L	" Brace *	(2) 2x6 <b>"</b> L	Brace **
ے	Spacing	Species	Grade	Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
字		CDE	#1 / #2	3′ 10 <b>″</b>	6′ 7 <b>″</b>	6′ 10 <b>″</b>	7′ 9″	8′ 1 <b>″</b>	9′ 3″	9′ 7″	12′ 2″	12′ 8″	14′ 0″	14′ 0″
'a	, l	SPF	#3	3′ 8″	5′ 9″	6′ 2 <b>″</b>	7′ 8″	7′ 11″	9′ 1″	9′ 6″	12′ 0″	12′ 6″	14′ 0″	14′ 0″
	기 및	HF	Stud	3′ 8″	5′ 9″	6′ 1″	7′ 8″	7′ 11″	9′ 1″	9′ 6″	12′ 0 <b>″</b>	12′ 6″	14′ 0″	14′ 0″
Ì	10		Standard	3′ 8″	4′ 11″	5′ 3 <b>″</b>	6′ 7″	7′ 1″	8′ 11″	9′ 6″	10′ 4″	11′ 1″	14′ 0″	14′ 0″
به	-		#1	4′ 0″	6′ 8″	6′ 11 <b>″</b>	7′ 10″	8′ 2 <b>″</b>	9′ 4″	9′ 8″	12′ 4″	12′ 9 <b>″</b>	14′ 0″	14′ 0″
$  \bot  $	*	ISP	#2	3′ 10″	6′ 7″	6′ 10 <b>″</b>	7′ 9″	8′ 1″	9′ 3″	9′ 7″	12′ 2″	12′ 8″	14′ 0″	14′ 0″
	4	l	#3	3′ 9″	5′ 3″	5′ 7 <b>″</b>	6′ 11″	7′ 5″	9′ 2″	9′ 7″	10′ 11″	11′ 8″	14′ 0″	14′ 0″
1 =	$L^{Q}$		Stud	3′ 9″	5′ 3″	5′ 7 <b>″</b>	6′ 11″	7′ 5″	9′ 2″	9′ 7″	10′ 11″	11′ 8″	14′ 0″	14′ 0″
전			Standard	3′ 6″	4′ 7″	4′ 11″	6′ 2″	6′ 7″	8′ 4″	8′ 11″	9′ 8″	10′ 4″	13′ 1″	14′ 0″
<u>  .U</u>		CDE	#1 / #2	4′ 5″	7′ 6″	7′ 9″	8′ 10 <b>″</b>	9′ 3″	10′ 7″	11′ 0″	13′ 11″	14′ 0″	14′ 0″	14′ 0″
1	-	SPF	#3	4′ 2″	7′ 1″	7′ 9″	8′ 9 <b>″</b>	9′ 1″	10′ 5 <b>″</b>	10′ 10″	13′ 9″	14′ 0″	14′ 0″	14′ 0″
1 6	U	HF	Stud	4′ 2″	7′ 1″	7′ 6″	8′ 9 <b>″</b>	9′ 1″	10′ 5″	10′ 10″	13′ 9″	14′ 0″	14′ 0″	14′ 0″
Ιà	ا م	1 11	Standard	4′ 2″	6′ 1″	6′ 5 <b>″</b>	8′ 1″	8′ 8 <b>″</b>	10′ 5 <b>″</b>	10' 10"	12′ 8″	13′ 7″	14′ 0″	14′ 0″
1 🖑			#1	4′ 7″	7′ 7″	7′ 11″	9′ 0″	9′ 4″	10′ 8″	11′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
>		ISP	#2	4′ 5″	7′ 6″	7′ 9″	8′ 10″	9′ 3″	10′ 7″	11' 0"	13′ 11″	14′ 0″	14′ 0″	14′ 0″
	1 %		#3	4′ 4″	6′ 5 <b>″</b>	6′ 10 <b>″</b>	8′ 6 <b>″</b>	9′ 1″	10′ 6″	10′ 11″	13′ 4″	14′ 0″	14′ 0″	14′ 0″
Ιω	1,6	IDFL	Stud	4′ 4″	6′ 5 <b>″</b>	6′ 10 <b>″</b>	8′ 6 <b>″</b>	9′ 1″	10′ 6″	10' 11"	13′ 4″	14′ 0″	14′ 0″	14′ 0″
I —			Standard	4′ 2″	5′ 8″	6′ 0″	7′ 6 <b>″</b>	8′ 0 <b>″</b>	10′ 2″	10′ 10″	11′ 10″	12′ 7″	14′ 0″	14′ 0″
		SPF	#1 / #2	4′ 10″	8′ 3″	8′ 7 <b>″</b>	9′ 9″	10′ 2″	10′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
1 ~	1	$I \setminus P \cap F$	#3	4' 7"	8' 2"	8' 5"	9' 8"	10/0"	11' 6"	12′ 0″	1 14′ ∩″	14' 0"	14' ∩"	l 14′ ∩″ l

9' 8"

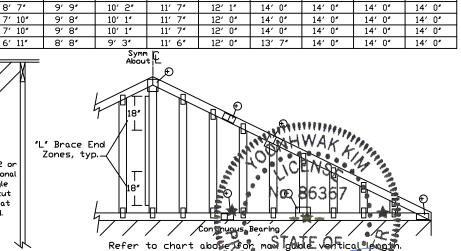
9' 4"

9' 11"

10' 0"

10' 0"

10' 3"



12' 0"

12' 0"

12' 3"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

14' 0"

11' 6"

11' 6"

11' 9"

Bracing Group Species and Grades: Group A: Spruce-Pine-Fir Hem-Fir #1 / #2 Standard #2 Stud #3 Stud #3 Standard Douglas Fir-Larch Southern Pine\*\*\* #3 #3 Stud Stud Standard Standard Group B Hem-Fir

#1 & Btr Douglas Fir-Larch Southern Pine\*\*\* #1 #1 #2

1x4 Braces shall be SRB (Stress-Rated Board) \*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ★ For (1) "L" brace: space nalls at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes				
Vertical Length	No Splice			
Less than 4' 0"	2X3			
Greater than 4' 0", but	3X4			
less than 11' 6"	3,4			
Greater than 11' 6"	4X4			
+ Pefen to common trues design for				

Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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For more information see this Job's general notes page and these web sites 18/2022 ALPINE: www.alpineltw.com, TPI: www.tpinstorg, SBCA: www.sbcacomponents.com, ICC: wind.cessFelorg, 78, Yoonhwak Kim, FL PE #86367

ASCE7-16-GAB16015 01/26/2018 

MAX, TOT, LD, 60 PSF

MAX. SPACING 24.0"

Vertical length shown

Connect diagonal at

midpoint of vertical web.

in table above.

 $\bigcirc$ 

 $\Omega$ 

Diagonal brace option:

vertical length may be doubled when diagonal

brace is used. Connect diagonal brace for 600# at each end. Max web

total length is 14'.

SP

X

Q

Stud

Standard

#1

#2

#3

Stud

Standard

4' 7"

4' 7"

5′ 1″

4' 10"

4' 9"

4' 9"

4′ 7″

Gable Truss

8' 2"

7′ 0″

8′ 5″

8' 3"

7' 4"

7' 4"

6' 6"

2x6 DF-L #2 or better diagonal

brace; single or double cut

(as shown) at

upper end.

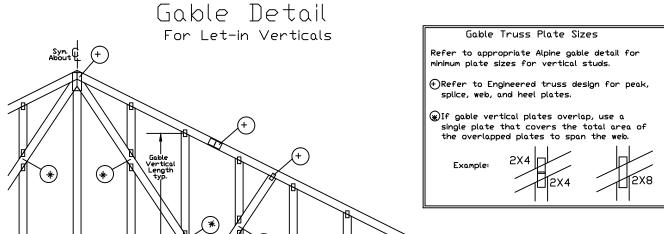
8' 5"

7′ 5″

8' 8"

8' 7"

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A1403ENC100118

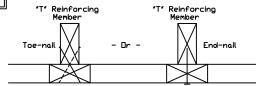
A18015ENC100118, A12015ENC100118, A12015ENC100118, A12015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A12003ENC100118, A12003ENC100118, A120030ENC100118, A120030ENC100118,

\$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015PED100118 \$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$18030ENC100118)

\$18030ENC100118, \$20030ENC100118, \$20030END100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical

#### "T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced aable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

### Web Length Increase w/ "T" Brace

"T" Reinf.	"T"
Mbr. Size	Increase
2×4	30 %
2x6	20 %

#### Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30 (1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length  $1.30 \times 8' \ 7'' = 11' \ 2''$ 

REF

LET-IN VERT

01/02/2018

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DATE DRWG GBLLETIN0118 MAX. TOT. LD. 60 PSF

DUR. FAC. ANY MAX. SPACING 24.0"



Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing

Member

Gable

Truss

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

