ABBREVIATIONS

STRUCTURAL ABBREVIATIONS

ABOVE FINISHED FLOOR	- A.F.F.	KIP PER SQUARE FOOT	- KSF
ADDITIONAL	- ADDN'L.	KIP PER SQUARE INCH	- KSI
ADJACENT	- ADJ.		
AIR CONDITIONER	- A/C	LENGTH	- L.
AIR HANDLING UNIT	- AHU	LIGHTWEIGHT CONCRETE	- L.W.C.
ALTERNATE	- ALT.	LIVE LOAD	- L.L.
AMERICAN CONCRETE INSTITUTE	- A.C.I.	LONGITUDINAL	- LONG.
AMERICAN INSTITUTE OF STEEL	- A.I.S.C.	LONG LEG HORIZONTAL	- LLH
ANCHOR BOLT	- A.B.	LONG LEG VERTICAL	- LLV
AND	- &	LONG SIDE HORIZONTAL	- LSH
ANGLE	- L	LONG SIDE VERTICAL	- LSV
APPROXIMATE	- APPROX.		
ARCHITECT	- ARCH.	MANUFACTURE(R)	- MFR.
ARCHITECTURAL	- ARCH'L.	MAXIMUM	- MAX.
ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	- A.E.S.S.	MECHANICAL	- MECH.
		METAL	- MTL.
ARCHITECTURALLY EXPOSED CONCRETE	- A.E.C.	MEZZANINE	- MEZZ.
		MINIMUM	- MIN.
AT	- @	MISCELLANEOUS	- MISC.
		MOMENT	- M
BASE PLATE	- B.P.	MOMENT CONNECTION	- M.C.
BEAM	- BM.		
BEARING	- BRG.	NON-SHRINK	- N.S.
BETWEEN	- BTWN.	NORMAL WEIGHT CONCRETE	- N.W.C.
BLOCKING	- BLKG.	NOT TO SCALE	- N.T.S.
BOTTOM	- BOT.	NUMBER	- NO. or #
BOTTOM OF	- B.O.		
BOTTOM OF STEEL	- B.O.S.	ON CENTER	- O.C.
BRIDGING	- BRDG.	OPENING(S)	- OPNG(S)
BUILDING	- BLDG.	OPPOSITE	- OPP.
		OPPOSITE HAND	- O.H.
CAMBER	- C	OUTSIDE FACE	- O.F.
CAST-IN-PLACE	- C.I.P.	OUTSIDE DIAMETER	- O.D.
CEILING	- CLG.		
CENTER LINE	- C.L. or ℄	PENETRATION	- PEN.
CLEAR OR CLEARANCE	- CLR.	PERIMETER	- PERIM.
COLD FORMED STEEL	- CFS	PERPENDICULAR	- PERP.
COLUMN	- COL.	PLATE	- PL. or P
COMPRESSION	- C or COMP.	POINT	- PT.
CONCRETE	- CONC.	POST-TENSION(ED)	- P-T
CONCRETE MASONRY UNIT	- CMU	POUNDS	- # or LBS.
CONNECTION(S)	- CONN(S)	POUNDS PER CUBIC FOOT	- PCF
CONSTRUCTION JOINT	- C.J.	POUNDS PER LINEAR FOOT	- PLF
CONTINUOUS	- CONT.	POUNDS PER SQUARE FOOT	- PSF
COORDINATE	- COORD.	POUNDS PER SQUARE INCH	- PSI
		PRECAST CONCRETE	- P/C
DETAIL	- DTL.	PRE-ENGINEERED METAL BUILDING	- P.E.M.B.
DEAD LOAD	- D.L.		
DEFORMED BAR ANCHOR	- D.B.A.	PREFABRICATED	- PREFAB.
DIAGONAL	- DIAG.	PROJECTION	- PROJ.
DIAMETER	- DIA. or Ø		
DIMENSION(S)	- DIM(S)	RADIUS	- R
DOUBLE	- DBL.	REINFORCE(ING)(ED)(MENT)	- REINF.
DOUBLE EXTRA STRONG	- XX-STR	REQUIRED	- REQ'D.
DOWEL(S)	- DWL(S)	ROOF DRAIN	- R.D.
DRAWING(S)	- DWG(S)	ROOF LIVE LOAD	- R.L.L.
		ROOF TOP UNIT	- R.T.U.
EACH	- EA.		
EACH FACE	- E.F.	SAWN JOINT	- S.J.
EACH WAY	- E.W.	SCHEDULE(D)	- SCHED.
ELEVATION	- EL.	SHEAR	- V
ELEVATOR	- ELEV.	SHEET	- SHT.
EMBEDMENT	- EMBED.	SIMILAR	- SIM.
ENGINEER	- ENGR.	SLAB ON GRADE	- S.O.G.
EQUAL OR EQUALLY	- EQ.	SNOW LOAD	- S.L.
EQUIPMENT	- EQUIP.	SQUARE	- SQ.
EXISTING	- EXIST.	SQUARE FOOT	- S.F.
EXPANSION	- EXP.	STANDARD	- STD.
EXPANSION JOINT	- E.J.	STEEL	- STL.
EXTERIOR	- EXT.	STEEL JOIST INSTITUTE	- S.J.I.
EXTRA STRONG	- X-STR	STIFFENER	- STIFF.
		STRUCTURAL	- STRUCT'L
FAR SIDE	- F.S.	SUPPORT(S)	- SUPT(S).
FIELD VERIFY	- F.V.		
FINISHED FLOOR	- FIN. FL.	TEMPERATURE	- TEMP.
FIXED CONNECTION	- F.C.	TENSION	- T
FLOOR	- FL.	THICK	- THK.
FLOOR DRAIN	- F.D.	TONGUE AND GROOVE	- T&G
FOOTING	- FTG.	TOP AND BOTTOM	- T&B
FOUNDATION	- FDN.	TOP OF	- T.O.
		TOP OF BEAM	- T.O.B.
GAGE OR GAUGE	- GA.	TOP OF CONCRETE	- T.O.C.
GALVANIZED	- GALV.	TOP OF FOOTING	- T.O.F.
GENERAL	- GEN.	TOP OF JOIST	- T.O.J.
GLULAM	- GL.	TOP OF PIER	- T.O.P.
GRADE	- GR.	TOP OF PIER (PILE) CAP	- T.O.P.C.
GYPSUM BOARD	- GYP. BD.	TOP OF STEEL	- T.O.S.
		TOP OF WALL	- T.O.W.
HEADED STUD ANCHOR	- H.S.A. or H.S.	TYPICAL	- TYP.
HEIGHT	- HT.		
		UNLESS NOTED OTHERWISE	- U.N.O.
INFORMATION	- INFO.		
INSIDE DIAMETER	- I.D.	VERTICAL	- VERT.
INSIDE FACE	- I.F.		
INTERIOR	- INT.	WATERSTOP	- W.S.
INTERMEDIATE	- INTERM.	WEIGHT	- WT.
		WELDED WIRE MESH (FABRIC)	- W.W.F.
JOINT	- JT.	WIND LOAD	- W.L.
JOIST(S)	- JST(S)	WITH	- W/
		WITHOUT	- W/O
KIPS (1000 LBS)	- K	WOOD	- WD.
KIP PER LINEAR FOOT	- KLF	WORK POINT	- W.P.

GENERAL NOTES

GENERAL NOTES

I. GENERAL CONDITIONS & DESIGN LOADS

1. Building Code: 7th Edition of the Florida Building Code
2. Risk Category II
3. Roof Dead Loads

Roofing	1.5 psf
Decking	1.8 psf
Structure	6.0 psf
Insulation	4.0 psf
Mechanical and Electrical	1.0 psf
Sprinklers	2.0 psf
Miscellaneous	1.0 psf

4. Live Loads

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (pounds)
Roofs		
All roof surfaces subject to maintenance workers		300
Ordinary flat, pitched, and curved roofs (that are not occupiable)	20	

5. Wind Design Data

Ultimate design wind speed, V_{ult}	118 mph
Nominal design wind speed, V_{asd}	95 mph
Wind Importance Factor, I_w	1.0
Wind exposure	C

Windward Wall Pressures at "z"	Combined WW + LW	
	Normal to Ridge	Parallel to Ridge
0 to 15'	19.0	16
20.0 ft	19.6	16
25.0 ft	20.2	16
30.0 ft	20.7	16
h = 35.0 ft	21.1	16.2
ridge = 35.3 ft	21.1	16.2

6. General
   a. Consult structural engineer if locations or design weights of roof top units differ from those on plans.
   b. All existing conditions shall be field verified.
   c. Contractor shall verify all dimensions prior to start of construction.
   d. General Contractor shall be responsible for coordination of other trades (mech'l, elec'l, etc.) prior to fabrication and installation of materials.
   e. General Contractor shall be responsible for coordination of architectural and structural drawings prior to fabrication, forming, or placement of materials. General Contractor shall report discrepancies immediately to Architect and shall proceed with construction only after discrepancy has been resolved.
   f. The details designated as "Typical Details" (TYP) apply generally to the drawings in all areas where conditions are similar.
   g. Contractor shall review architectural and mechanical sheets for miscellaneous steel.
   h. Contractor shall review mechanical sheets for miscellaneous concrete.
   i. Hangers supporting piping, ceilings, light fixtures, and suspended framing shall not be attached to roof deck. Hangers shall be connected to roof joist, beams, or headers provided to span between roof framing members. [Hanger spacing shall not exceed 2 times the spacing of the roof framing members. Hanger load shall not exceed a uniform load of (10/15/20) psf.]
   j. The design represented by these plans provides for stability of the completed structure. It is the contractor's responsibility to maintain structure stability and safety during construction per requirements of SEI/ASCE 37-14 Design Loads on Structures During Construction.

II. FOUNDATION & EARTHWORK

1. Design based on Geotechnical Report No. 458560 by Universal Engineering Services, dated March 29, 2006.
2. Foundation construction shall be monitored by a representative of the Geotechnical Engineer to observe excavation, identify bearing strata, and provide recommendations if seepage is encountered.
3. Depths or elevations of bottoms footings shown on Structural Drawings are for estimating purposes only. Final bearing elevations shall be determined in the field by a representative of the Geotechnical Engineer to satisfy assumed design bearing values.
4. Footing design loadbearing values:

Isolated spread footing	2,500 psf
Continuous footing	2,500 psf
Bearing stratum	prepared subgrade

5. Slab on grade
   a. Contractor shall read and be familiar with the project geotechnical report for soil pad preparation prior to beginning earthwork.
   b. A 15-mil vapor barrier (0.01 perm maximum per ASTM E 1745) shall be provided above the fine insulation. Substitutions shall be approved in writing by engineer of record.
   c. Contractor shall brace wall or grade beam while placing backfill or fill material.

III. STRUCTURAL CONCRETE

1. Concrete shall have the following properties:

Description of Use	Unit Weight (pcf)	Min. 28 Day Strength (psi)	Max w/c Ratio	Slump	Air Content
Pilasters and foundation wall	145 (NW)	4,000	-	3"-5"	3-6%
Slab-on-Grade	145 (NW)	3,000	0.45	2"-4"	-
Spread Footings	145 (NW)		-	3"-6"	-

2. Reinforcing steel shall be ASTM A615 Grade 60 unless noted otherwise.
3. Lap reinforcing splices #6 and smaller bars - 57 bar diameters with 24" minimum unless noted otherwise. Lap reinforcing splices #7 and larger - 71 bar diameters with 24" minimum unless noted otherwise.
4. Provide corner bars with 30 bar diameter lap to match horizontal reinforcement at all grade beam and wall intersections. 2'-0" lap minimum.
5. All reinforcement shall be detailed in accordance with the latest ACI Detailing Manual.
6. Contractor shall coordinate all penetrations, conduit, chamfers and embedded items prior to concrete placement.
7. Provide concrete coverage for reinforcing steel as follows unless noted otherwise:

Location	Concrete Cover (in)
Concrete cast against and permanently exposed to earth	3
Concrete exposed to earth or weather:	
No. 6 through No. 18 bars	2
No. 5 bar and smaller	1-1/2
Concrete not exposed to weather or in contact with ground:	
No. 14 and No. 18 bars	1-1/2
No. 11 bar and smaller	3/4
Beams, columns:	
Primary reinforcement, ties, stirrups, spirals	1-1/2

8. Set anchor rods to have at least 3" concrete cover and to allow reinforcement to pass outside of rods.

IV. STRUCTURAL STEEL

1. Structural steel properties:

Shape	ASTM Designation	F_y (ksi)	F_u (ksi)
W shapes	A992	50	65
Channels	A36	36	58
Angles	A36	36	58
Plates & Bars	A36	36	58
HSS Rectangular	A500 Gr. C	50	62

2. All bolted connections shall use ASTM F3125 Grade A325 bolts unless noted otherwise.
3. All welds shall be made using E70 electrodes.
4. Temporary construction bracing of the structural steel frame shall remain in place until the roof deck attachments have been completed and all permanent bracing has been installed.
5. Welding shall be in accordance with "Structural Welding Code" by American Welding Society (AWS D1.1 – Latest Edition). Welding shall be performed by prequalified operators only.

V. STRUCTURAL STEEL CONNECTIONS

1. Submittals
   a. Preliminary Connection Review: Fabricator shall submit representative samples of the required substantiating connection information (details and calculations) no less than 14 days prior to the beginning of approval drawing preparation. Approval drawing preparation shall not commence until representative sample substantiating connection information have been approved as consistent with the requirements of contract drawings or modifications to the representative samples has been received.
   b. Connection Design Submittal: The Fabricator's licensed professional engineer responsible for the structural steel connection design shall submit complete sealed connection design calculations to accompany the approval drawing submittal. All approval drawings submitted without complete sealed connection design calculations will be rejected.
   c. Connection Design Confirmation Letter: The Fabricator's licensed professional engineer in responsible charge of the connection design shall review the approval drawings and confirm and attest in writing as part of the substantiating connection information that his connection designs have been properly incorporated into the approval drawings.
2. Qualifications
   a. The Fabricator's licensed professional engineer in responsible charge of the structural steel connection design shall be experienced in the specific area of structural steel connection design and with not less the 5 years of experience in the connection design of similar projects of similar complexity.
3. Performance Requirements (Option 3B)
   a. Connections for this project are a delegated design and shall be designed, detailed, and fabricated in accordance with Option 3B in the AISC Code of Standard Practice, Sections 3.1.1(3) and 3.1.2(2)(b)
   b. Connections shall be designed by a Connection Design Specialty Engineer (CDSE) working for the Fabricator. The CDSE shall be licensed to practice engineering in the state where the project is located and shall be experienced in the design of structural steel connections.
   c. Connections shall be designed for the reactions shown on the Contract Documents. Where shear reactions are not specified, connections shall be designed for the reaction specified in the Beam Reaction Schedule.
   d. Loads provided on the drawings are **service-load level forces to be used with Allowable Strength Design.**
   e. Connections shall be designed in accordance with the Specification for Structural Steel Buildings (ANSI/AISC 360-**10**) and the AISC Steel Construction Manual (**14th** edition).
   f. The CDSE shall review the contract documents, including drawings and specifications, to determine the delegated connection scope of work and connection design criteria.
   g. The Contract Documents should provide sufficient information for the CDSE to perform the delegated connection design. If the CDSE determines that there are conflicts between the framing plans and the conceptual connection details specified, the CDSE shall notify the Structural Engineer of Record in a timely manner so that these conflicts may be resolved.
   h. The connection details contained in the Contract Documents represent conceptual details and represent the intended connection arrangement. Final arrangement of the connection, including plate or angle thicknesses, bolt grade, bolt diameter and quantity, and weld sizes shall be determined by the CDSE.
   i. Column stiffeners, web doubler plates, beam bearing stiffeners, and all other member reinforcement required to satisfy strength and equilibrium of forces through the connection shall be designed by the CDSE. The Contract Documents provide bidding quantities and project-specific conceptual details for each of these types of member reinforcement; this information shall be used for bidding purposes.
   j. If the final design of member reinforcement differs from the bidding quantities and/or conceptual details shown, the contract and/or schedule will be adjusted in accordance with the AISC Code of Standard Practice Sections 9.4 and 9.5.
   k. The CDSE shall submit in a timely manner representative samples of the connection details and calculations applicable to the project. The Structural Engineer of Record will respond in a timely manner indicating if this sample information is consistent with the requirements in the Contract Documents or will advise what modifications to the sample information are required for compliance.
   l. The CDSE shall submit sealed calculations for structural steel connections which are delegated for connection design. These include, but are not limited to, beam shear connections (including those with axial loads and transfer forces), column splices, moment connections, vertical and horizontal bracing connections, girder and truss splices, truss-to-column and truss-to-truss connections, and truss web-to-chord and web-to-gusset connections, column stiffeners, web doubler plates, and beam bearing stiffeners.
   m. All approval drawings which include connections that have been delegated to the Fabricator for design shall be reviewed by the CDSE for conformance with the CDSE's connection design. The CDSE shall submit a sealed letter to attest and confirm in writing that the approval drawings have been reviewed and properly incorporate the CDSE's sealed connection calculations. The sealed letter shall accompany all approval drawings submitted for approval and shall include an index of applicable approval drawings, including erection and shop drawing numbers, dates, and revision numbers.
   n. The approval drawings shall clearly identify the CDSE's connection calculation identification and connection capacity on each end of each member for each connection designed by the CDSE.
   o. If alternate connections to the specified conceptual connection details are desired by the Fabricator, the Fabricator shall request written authorization from the Structural Engineer of Record. If accepted, the CDSE shall provide sealed calculations indicating that the alternate connections satisfy the requirements of the Contract Documents.

VI. ROOF DECK

1. Metal deck schedule:

Deck Gauge	Deck Type	Deck Depth	Sheet Width	S_p (in^3/ft)	S_n (in^3/ft)	I_p (in^4/ft)	I_n (in^4/ft)	Finish
22	WR	1.5"	36"	0.186	0.192	0.155	0.183	Galvanized

2. Roof deck shall be continuous over three (3) spans minimum.
3. Suspended ceilings, light fixtures, ducts or other utilities shall not be supported by the steel deck.
4. Metal deck connection schedule:

Location	Connection at Supports	Connection at Parallel Edges	Sidelap Connection No./Span	Required Shear Capacity (plf)
All Roof	36/4	12" o.c.	5	

5. Support and parallel edge connections shall be:
   a. Hilti X-ENP 19 for steel beam of flange thickness greater than 1/4"
6. Sidelap connections shall be Hilti SLC-01.
7. Provide 18" wide 20 gage filler sheets centered on non-nesting side laps as required. Connect as per note 4 above.

VII. SHOP DRAWINGS AND SUBMITTALS

1. Shop drawings shall be prepared and submitted for review to the Architect/Engineer for each structural building material as specified in the structural General Notes and the Project Specifications. See the contract specifications for submittal procedures and additional information.
2. Shop drawings shall use drafting line work and lettering that is clearly legible. Shop drawings shall not contain reproductions of the contract drawing plans or details. Shop drawings shall not show materials for more than one level on the same plan.
3. Shop drawings shall show clear and complete information for the fabrication (detail sheets and/or material list) and installation (erection and/or placing drawings) of materials. Erection and placing drawing shall provide every note and detail required to correctly install materials; they shall not refer to the contract documents for installation information.
4. Erection and placing drawing plans shall be oriented and referenced in the same manner as plans on the contract documents.
5. Shop drawings shall list the grade, class and/or strength of materials.
6. Submittals received by the Architect/Engineer which are insufficient or incomplete may be rejected and returned to the Contractor without review. The Contractor shall be responsible for any delays in the construction schedule due to rejected, insufficient, or incomplete shop drawing submittals.
7. Shop drawings shall be checked and initialed by the supplier prior to submittal to the Contractor.
8. The Contractor shall thoroughly check, coordinate, stamp, date and initial each and every shop drawing print prior to submittal to the Architect/Engineer for review. Shop drawings, which do not meet these requirements, shall be considered incomplete and returned to the Contractor without review.
9. Contractor's request to deviate from the contract documents shall be clearly identified by "clouding" on the shop drawings.
10. Resubmitted shop drawings shall clearly identify items that have been corrected. Shop Drawings that are resubmitted with revisions that are not clouded shall be rejected and returned to the Contractor without review.

VIII. DEFERRED SUBMITTALS

1. The following items are portions of the design that are not included in the Contract Documents. Design of these elements shall be the responsibility of Specialty Contractors and shall be designed and sealed by a registered Professional Engineer licensed in the State of Florida.
   a. Structural Steel Connections

IX. SPECIAL INSPECTIONS

1. The Owner shall employ one or more Special Inspectors to perform this work. The Special Inspector(s) shall keep records of all inspections and furnish interim inspection reports to the Building Official and to the Registered Design Professional in Responsible Charge. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge prior to completion of that phase of work. A Final Report of Special Inspections documenting required Special Inspection and correction of any discrepancies noted in the inspections shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge at the conclusion of the project.
2. The Special Inspection program does not relieve the Contractor of responsibility to comply with the Contract Documents.

Inspection of Fabricators
Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, special inspection of the fabricated items shall be required by Section 1704.2.5.

Inspection of Welding
Welding inspection shall be in compliance with AWS D1.1. The basis for welding inspector qualification shall be AWS D1.1.

Inspection of Structural Steel
Special inspection of structural steel shall be in accordance with the quality assurance inspection requirements of AISC 360.

Concrete Construction (IBC 1705.3)

TYPE	Continuous Special Inspection	Periodic Special Inspection
1. Inspect anchors cast in concrete.	—	X
2. Verify use of required design mix.	—	X
3. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	—
4. Inspect formwork for shape, location and dimensions of the concrete member being formed.	—	X

Soils (IBC 1705.6)

TYPE	Continuous Special Inspection	Periodic Special Inspection
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	—	X
2. Verify excavations are extended to proper depth and have reached proper material.	—	X
3. Perform classification and testing of compacted fill materials.	—	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	—
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	—	X

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PROFESSIONAL ENGINEER

MAY 28 2021

Project Number: 221207

Issue Date: 05/28/21

Revisions:

United States Cold Storage
211 MCCLOSKEY AVE
LAKE CITY, FLORIDA 32055

Project Engineer: F.N.

Project Draftsman: R.H.

Sheet Title
GENERAL NOTES

S0.1

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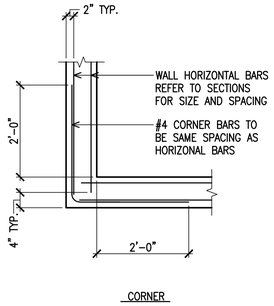
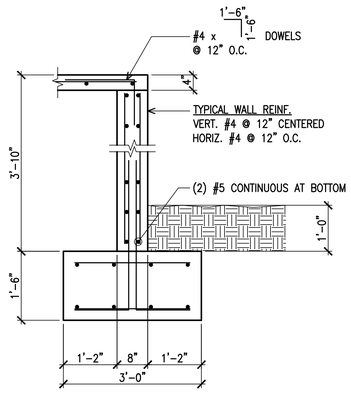
May 28, 2021 - 3:21pm



1 DETAIL
3/4" = 1'-0" | CONCRETE WALL REINFORCING BARS



2 DETAIL
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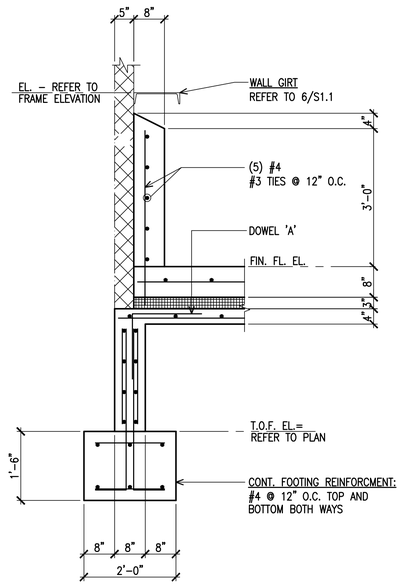
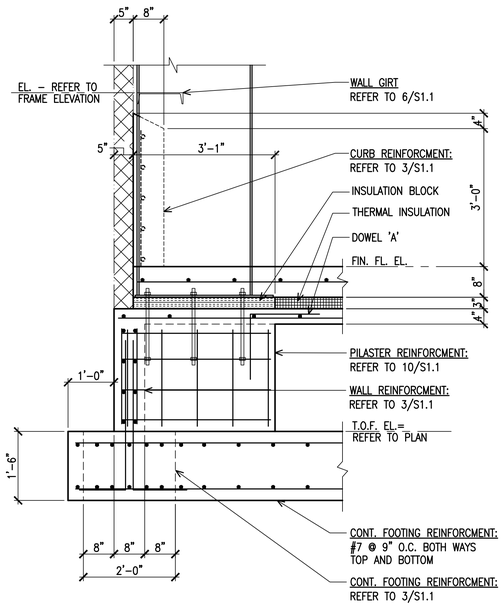
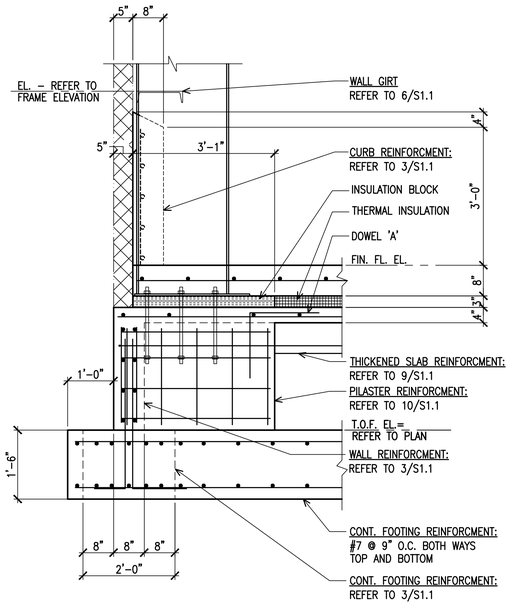


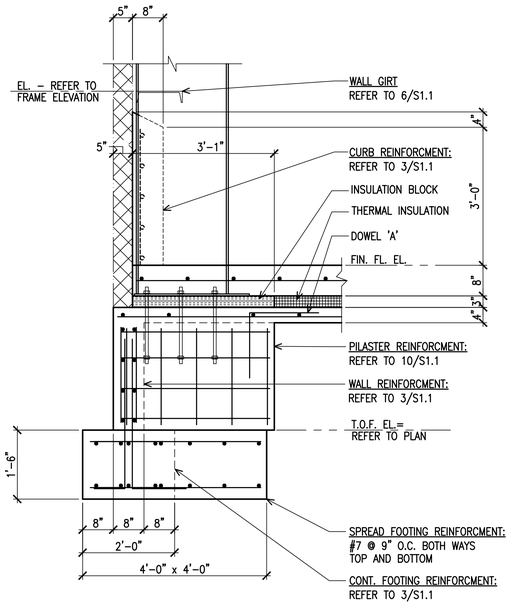
3 DETAIL
3/4" = 1'-0" | PERIMETER GRADE BEAM WITH LEDGE



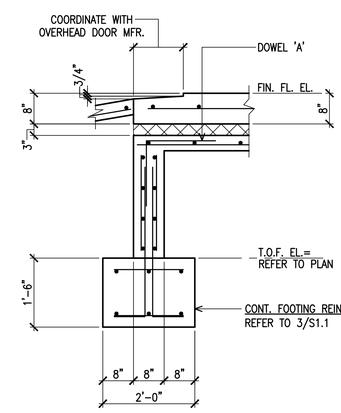
4 DETAIL
3/4" = 1'-0" | PERIMETER GRADE BEAM WITH LEDGE



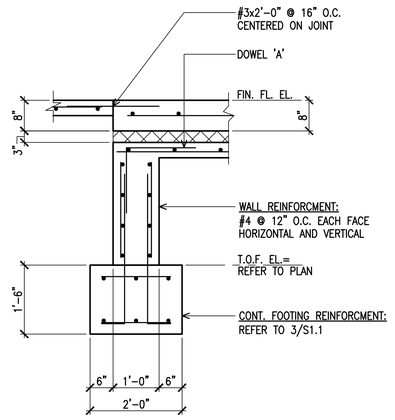
5 DETAIL
3/4" = 1'-0" | PERIMETER GRADE BEAM WITH ADJACENT THICKENED SLAB



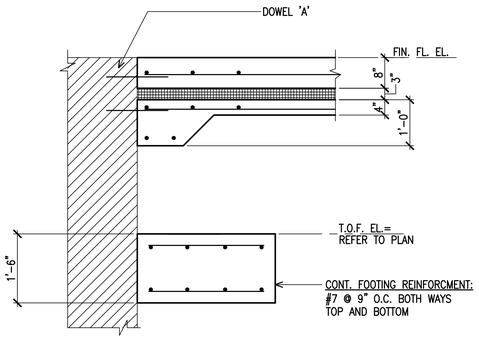
6 DETAIL
3/4" = 1'-0" | PERIMETER GRADE BEAM WITH LEDGE



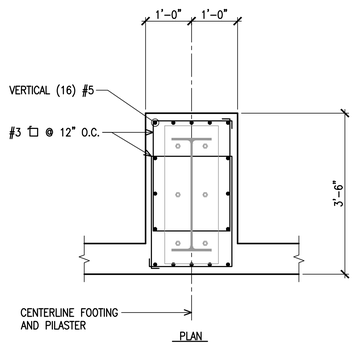
7 DETAIL
3/4" = 1'-0" | GRADE BEAM WITH DOOR POCKET



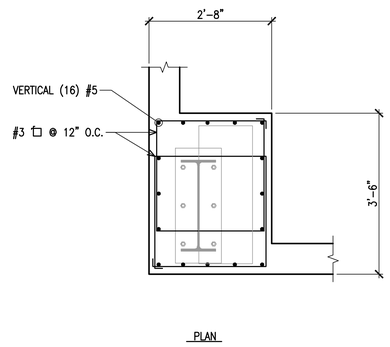
8 DETAIL
3/4" = 1'-0" | GRADE BEAM WITH DOOR POCKET



9 DETAIL
3/4" = 1'-0" | PERIMETER GRADE BEAM AT STEPS



10 DETAIL
3/4" = 1'-0" | ADDITIONAL REINFORCING AT PILASTER



11 DETAIL
3/4" = 1'-0" | ADDITIONAL REINFORCING AT CORNER PILASTER

DOWEL SCHEDULE
SCHEDULED DOWELS ARE MARKED 'DWL' ON THE SECTIONS AND DETAILS.

B
A

MARK	SIZE	A	B	SPACING
'A'	#4	2'-0"	2'-0"	18" O.C.

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MAY 28 2021

Project Number: 221207

Issue Date: 05/28/21

Revisions:

United States Cold Storage
211 MCCLOSKEY AVE
LAKE CITY, FLORIDA 32055

Project Engineer: F.N.

Project Draftsman: R.H.

Sheet Title
Foundation Details

S1.1

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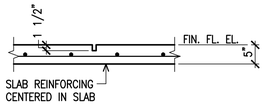
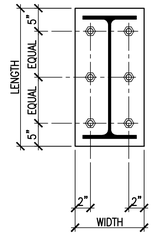
May 28, 2021 - 3:21pm



NOTES:
1. PROVIDE JOINTS IN SLAB AT MAXIMUM SPACING OF 25 FEET O.C. EACH WAY.
2. TYPICAL JOINT IS A SAWED JOINT. CONTRACTOR TO LOCATE CONSTRUCTION JOINTS AT HIS DISCRETION.
3. SAWN JOINT SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT.

1 DETAIL

3/4" = 1'-0" | SAW JOINT (S.J.)



PLAN

2 DETAIL

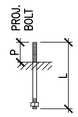
NOT TO SCALE | BASE PLATE – WIDE FLANGE COLUMN

BASE PLATE SCHEDULE

MARK	DIMENSIONS L	DIMENSIONS W	THICK.	ANCHOR RODS	COLUMN SIZE	DETAIL
BP-1	36"	14"	1 1/2"	(4) AR1	W30	REFER TO 2/S1.2
BP-2	30"	12"	1 1/2"	(6) AR1	W24	REFER TO 2/S1.2

ANCHOR ROD SCHEDULE

MARK	DIA.	DIMENSIONS L	DIMENSIONS H	DIMENSIONS P	MATERIAL
AB1	1"ø	20"	–	4"	F1554, GRADE 36



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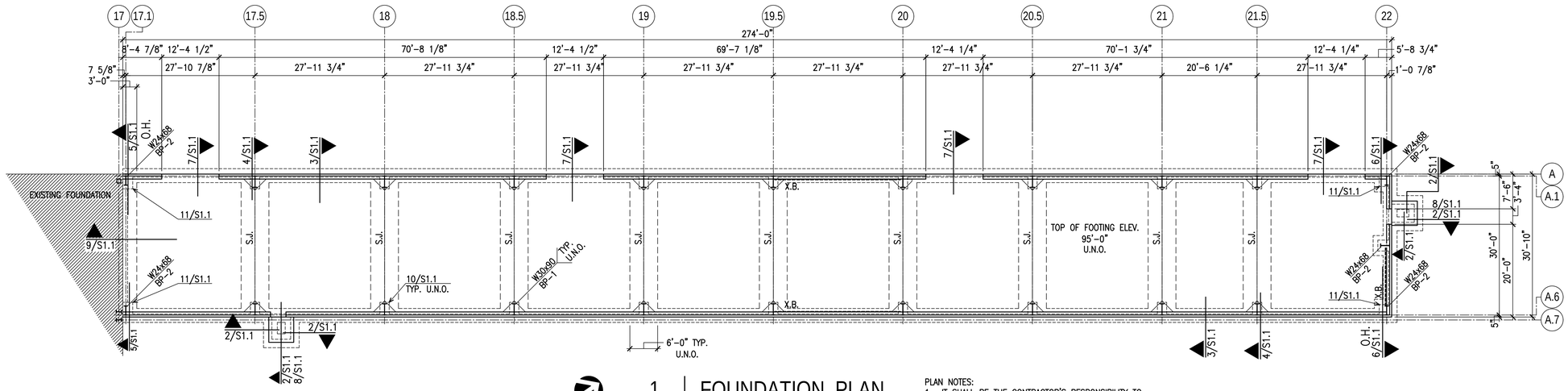
Sheet Title
Foundation Details

S1.2



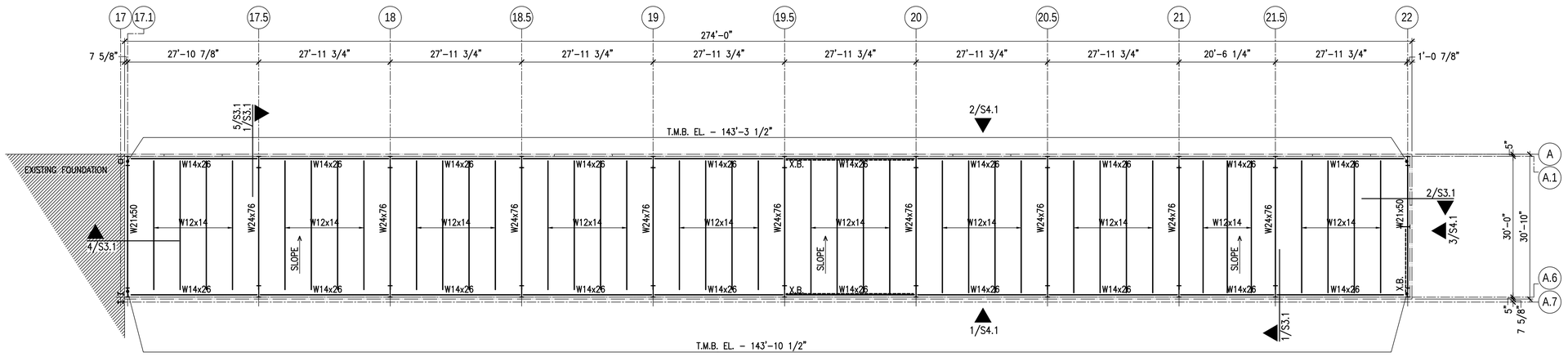
NORTH

1 FOUNDATION PLAN

3/32" = 1'-0"

PLAN NOTES:

1. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION.
2. REFER TO SHEET S0.1 FOR GENERAL NOTES.
3. REFER TO SHEET S1.1 FOR DETAILS NOT NOTED ON PLAN.
4. FINISH FLOOR = DATUM ELEVATION 100'-0", ACTUAL =
5. COLUMNS ARE CENTERED ON GRIDLINE INTERSECTIONS UNLESS DIMENSIONED OTHERWISE ON PLAN.
6. COLUMNS ARE NOTED ON PLAN THUS:
   W_x_ ← COLUMN SIZE (STD. PIPE U.N.O.)
   BP-1 ← BASE PLATE MARK. REFER TO BASE PLATE SCHEDULE FOR INFORMATION.
7. FLOOR SLAB SHALL BE 8" THICK CONCRETE REINFORCED WITH #4 @ 12" O.C.B.W. CENTERED IN SLAB.
8. PROVIDE (2) #4X3'-0" DOWELS CENTERED IN SLAB AT ALL INTERIOR CORNERS WITHOUT A CONSTRUCTION OR SAWN JOINT.
9. REFER TO ARCHITECTURAL FOR LOCATION AND EXTENT OF ALL LEDGES, POCKETS OR DEPRESSIONS.
10. S.J. – INDICATES SAWN JOINT ON PLAN. REFER TO 1/S1.2.



NORTH

2 ROOF FRAMING PLAN

3/32" = 1'-0"

PLAN NOTES:

1. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION.
2. REFER TO SHEET S0.1 FOR GENERAL NOTES.
3. REFER TO SHEET S3.1 FOR TYPICAL DETAILS NOT NOTED ON PLAN.
4. REFER TO PLAN FOR TOP OF METAL DECK (T.M.D.) ELEVATIONS.
5. ▭ INDICATES ALLOWABLE AREAS TO RECEIVE FUTURE ROOF TOP UNITS. ASSUMED DESIGN WEIGHT = . UNITS MUST BE SUPPORTED BY A MINIMUM OF 2 JOISTS. REFER TO 7/S3.1 FOR TYPICAL MECHANICAL UNIT CURB SUPPORT DETAIL.
6. STEEL JOIST BRIDGING SHALL BE PER THE STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS.

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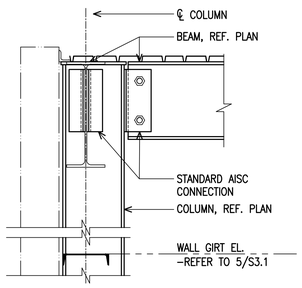
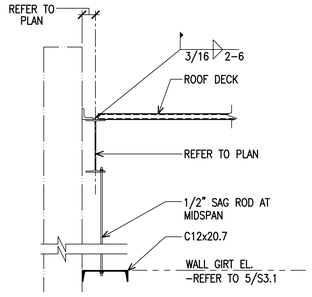
Project Draftsman: R.H.

Sheet Title
FOUNDATION AND ROOF FRAMING PLAN

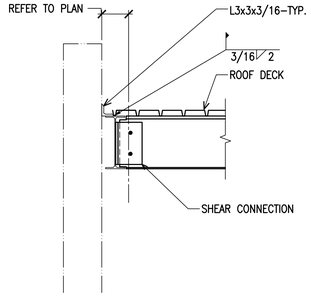
S2.1



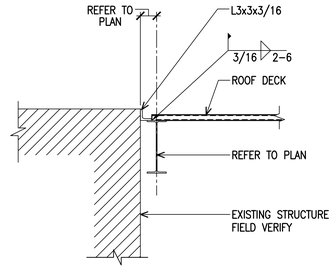
1 DETAIL
SCALE: N.T.S. | BEAM TO COLUMN CONNECTION



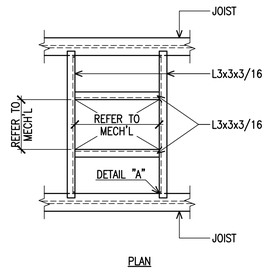
2 DETAIL
NOT TO SCALE | DECK TO BEAM CONNECTION



3 DETAIL
NOT TO SCALE | STEEL BEAM CONNECTION

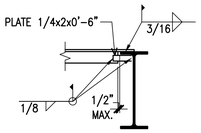


4 DETAIL
NOT TO SCALE | DECK TO BEAM CONNECTION



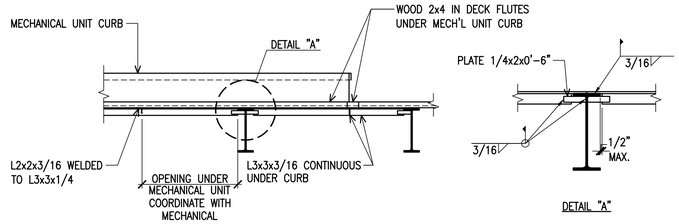
PLAN

NOTE:
WHERE MECHANICAL FRAME BEARS ON ROOF DECK, PLACE HSS 1 1/2 x 2 1/2 x 14 GA. x 0'–6" BLOCKING IN DECK FLUTES FOR DIRECT BEARING ON STRUCTURE.



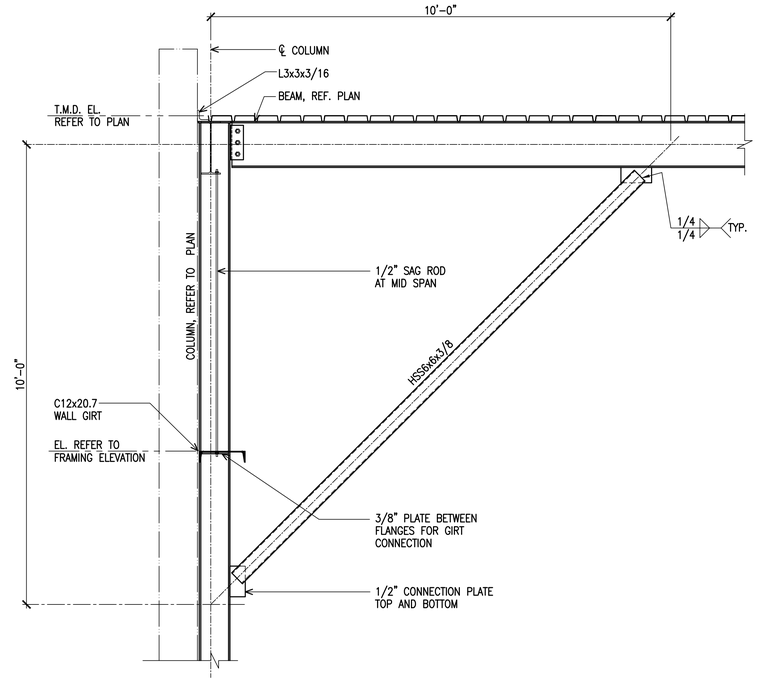
DETAIL "A"

6 DETAIL
NOT TO SCALE | FRAMING FOR ROOF OPENING



DETAIL "A"

7 DETAIL
NOT TO SCALE | MECHANICAL UNIT CURB SUPPORT



5 DETAIL
SCALE: N.T.S. | BEAM TO COLUMN CONNECTION

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Sheet Title
Framing Details

S3.1

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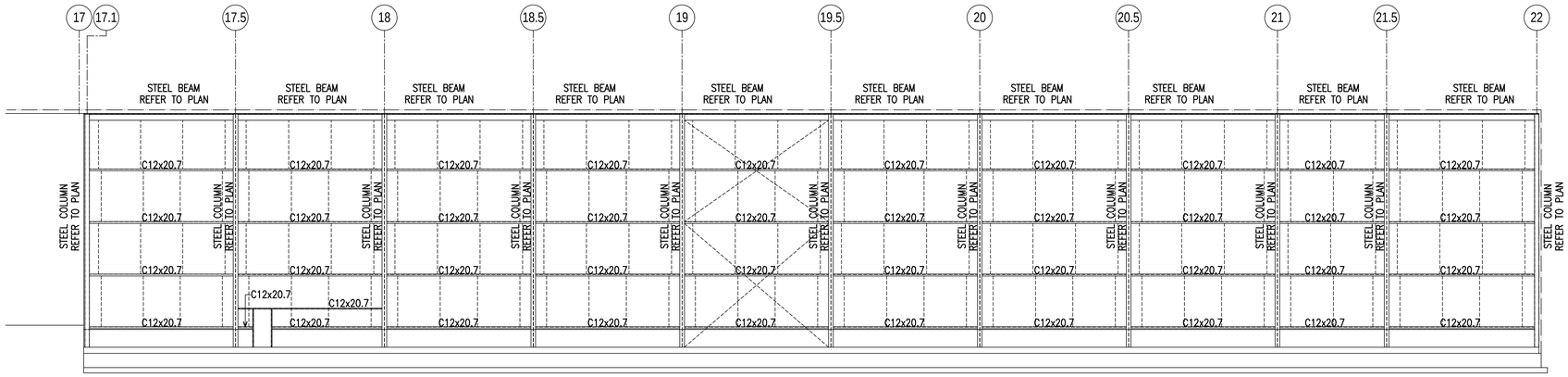
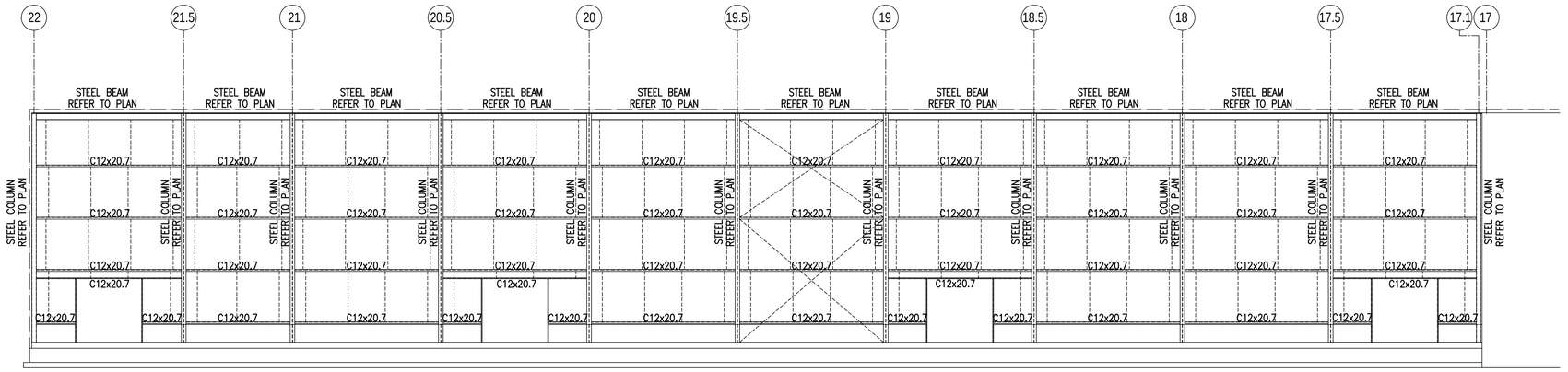
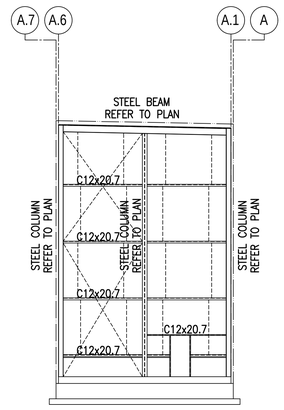
May 28, 2021 - 3:25pm



1 FRAMING ELEVATION
3/32" = 1'-0"



2 FRAMING ELEVATION
3/32" = 1'-0"



3 FRAMING ELEVATION
3/32" = 1'-0"

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Sheet Title
FRAMING ELEVATIONS

S4.1

T:\2021-WIP\221207 - Rail Dock addition to USCS - Lake City, FL - FWN\DRAWINGS\221207-S4.1.dwg
May 28, 2021 - 3:28pm