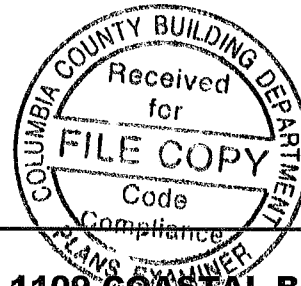


JULIUS LEE PE.



RE: 566946 - MIKE ROBERTS - LOT 1 CC

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer Mike Roberts Project Name: 566946 Model Custom
Lot/Block: 1 Subdivision: Cannon Creek
Address:
City: Columbia Cty State FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown at time of seal License # Unknown at time of seal
Address: Unknown at time of seal
City: Unknown at time of seal State: Unknown at time of seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet
conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

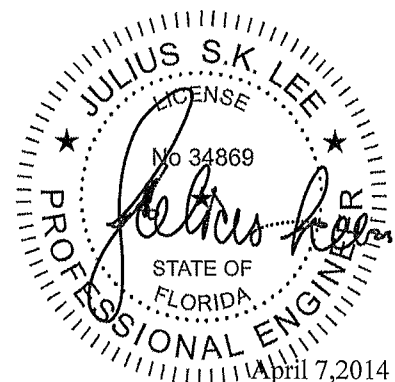
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I8035476	CJ01	4/7/014	18	I8035493	T05	4/7/014
2	I8035477	CJ02	4/7/014	19	I8035494	T06	4/7/014
3	I8035478	CJ03	4/7/014	20	I8035495	T07	4/7/014
4	I8035479	CJ04	4/7/014	21	I8035496	T08	4/7/014
5	I8035480	CJ05	4/7/014	22	I8035497	T09	4/7/014
6	I8035481	EJ01	4/7/014	23	I8035498	T10	4/7/014
7	I8035482	EJ02	4/7/014	24	I8035499	T11	4/7/014
8	I8035483	EJ03	4/7/014	25	I8035500	T12	4/7/014
9	I8035484	EJ04	4/7/014	26	I8035501	T13	4/7/014
10	I8035485	EJ05	4/7/014	27	I8035502	T14	4/7/014
11	I8035486	HJ01	4/7/014	28	I8035503	T15	4/7/014
12	I8035487	HJ02	4/7/014	29	I8035504	T16	4/7/014
13	I8035488	T01	4/7/014				
14	I8035489	T01G	4/7/014				
15	I8035490	T02	4/7/014				
16	I8035491	T03	4/7/014				
17	I8035492	T04	4/7/014				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 566946	Truss CJ01	Truss Type Jack-Open Truss	Qty 8	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional)	I8035476
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Builders FirstSource Lake City FL 32055
7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31 37 2014 Page 1

ID_ N8W8OLQbT8ydtVouale7z6MfY-dpmZ6g_2N?Wuu1wDRXMuGzpMhihNZWia1RDSiQzTAhK

-2-0-0 1-0-0
2-0-0 1-0-0

Scale = 1:3.0

Plate Offsets (X,Y) [2-0-0-12,0-1-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.32	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(LL) 0.00 8 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Vert(TL) 0.00 8 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.00 3 n/a n/a		
				Weight: 8 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=205/0-3-8 (min 0-1 8) 5=-52/Mechanical 3=-13/Mechanical
 Max Horz 2=67(LC 12)
 Max Uplift 2=-146(LC 12) 5=-85(LC 2) 3=-17(LC 2)
 Max Grav 2=250(LC 2) 5=44(LC 12) 3=13(LC 8)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-9)

- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C Encl , GCpf=0.18* MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed porch left and right exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 65 lb uplift at joint 5 and 17 lb uplift at joint 3.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 8) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 566946	Truss CJ03	Truss Type Jack-Open Truss	Qty 8	Ply 1	MIKE ROBERTS LOT 1 CC	18035478					
Builders FirstSource, Lake City FL 32055					Job Reference (optional) 7.350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31:40 2014 Page 1 ID_ N8W8OLQbT8ydtVoouale7z6MIY-2NSiki1wfwuTIVfo6gw bucRrTve5mtS0kPR6JizTAht						
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:25%;"> LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 0.0 * BCDL 5.0 </td> <td style="width:25%;"> SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007 </td> <td style="width:15%;"> CSI TC 0.42 BC 0.37 WB 0.00 (Matrix-M) </td> <td style="width:20%;"> DEFL in (loc) Vert(LL) 0 12 5-8 >475 240 Vert(TL) 0 11 5-8 >538 180 Horz(TL) -0.03 4 n/a n/a </td> <td style="width:15%;"> PLATES GRIP MT20 244/190 Weight: 22 lb FT = 20% </td> </tr> </table>							LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.42 BC 0.37 WB 0.00 (Matrix-M)	DEFL in (loc) Vert(LL) 0 12 5-8 >475 240 Vert(TL) 0 11 5-8 >538 180 Horz(TL) -0.03 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 22 lb FT = 20%
LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.42 BC 0.37 WB 0.00 (Matrix-M)	DEFL in (loc) Vert(LL) 0 12 5-8 >475 240 Vert(TL) 0 11 5-8 >538 180 Horz(TL) -0.03 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 22 lb FT = 20%							
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 SLIDER Left 2x4 SP No.3 1-6-0											
BRACING TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>											
REACTIONS (lb/size) 4=85/Mechanical, 2=239/0-3-8 (min. 0-1-8) 5=31/Mechanical Max Horz 2=162(LC 12) Max Uplift 4=102(LC 12) 2=127(LC 12) 5=47(LC 9) Max Grav 4=103(LC 2), 2=288(LC 2), 5=65(LC 3)											
FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=245/265											
NOTES (7-9) 1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II Exp C, Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 4, 127 lb uplift at joint 2 and 47 lb uplift at joint 5. 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB. 9) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435											
LOAD CASE(S) Standard											



April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 566946	Truss CJ05	Truss Type Jack-Open Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional)	I8035480																																																																								
Builders FirstSource, Lake City FL 32055		<div style="display: flex; justify-content: space-between;"> ID_ N8W8OLQbT8ydtVoouale7z6MIY-Sy7qNj3pyrG2cyNMooTIWE3NA6lmzEBSQNgmw4zTAHf 7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31:43 2014 Page 1 </div>																																																																												
<div style="display: flex; justify-content: space-around; margin-bottom: 10px;"> -2-0-0 2-0-0 1-4-15 1-4-15 </div>																																																																														
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td colspan="2">Plate Offsets (X,Y) [2.0-0-12,0-1-8]</td> <td colspan="2"></td> <td colspan="2"></td> <td colspan="2"></td> </tr> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">2-0-0</td> <td style="width:15%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">in (loc)</td> <td style="width:15%;">l/defl</td> <td style="width:15%;">L/d</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC 0.32</td> <td>Vert(LL)</td> <td>0.00 8</td> <td>>999</td> <td>240</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC 0.05</td> <td>Vert(TL)</td> <td>0.00 8</td> <td>>999</td> <td>180</td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr</td> <td>YES</td> <td>WB 0.00</td> <td>Horz(TL)</td> <td>0.00 2</td> <td>n/a</td> <td>n/a</td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td></td> <td>(Matrix-M)</td> <td colspan="4"></td> </tr> <tr> <td colspan="4"></td> <td colspan="2">PLATES</td> <td colspan="2">GRIP</td> </tr> <tr> <td colspan="4"></td> <td colspan="2">MT20</td> <td colspan="2">244/190</td> </tr> <tr> <td colspan="4"></td> <td colspan="2">Weight: 9 lb</td> <td colspan="2">FT = 20%</td> </tr> </table>							Plate Offsets (X,Y) [2.0-0-12,0-1-8]								LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	0.00 8	>999	240	TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	0.00 8	>999	180	BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00 2	n/a	n/a	BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)									PLATES		GRIP						MT20		244/190						Weight: 9 lb		FT = 20%	
Plate Offsets (X,Y) [2.0-0-12,0-1-8]																																																																														
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d																																																																							
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	0.00 8	>999	240																																																																							
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	0.00 8	>999	180																																																																							
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00 2	n/a	n/a																																																																							
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)																																																																											
				PLATES		GRIP																																																																								
				MT20		244/190																																																																								
				Weight: 9 lb		FT = 20%																																																																								
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2				BRACING TOP CHORD Structural wood sheathing directly applied or 1-4-15 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing			MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.																																																																							
REACTIONS (lb/size) 2=188/0-3-8 (min. 0-1-8) 5=-33/Mechanical 3=7/Mechanical Max Horz 2=77(LC 12) Max Uplift 2=127(LC 12), 5=-42(LC 2) 3=-20(LC 12) Max Grav 2=229(LC 2) 5=32(LC 16) 3=8(LC 2)																																																																														
FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown																																																																														
NOTES (7-9) 1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 2, 42 lb uplift at joint 5 and 20 lb uplift at joint 3. 6) "Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 9) Truss Design Engineer Julius Lee PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435																																																																														
LOAD CASE(S) Standard																																																																														



April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.
 Design valid for use only with MITTEK connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 566946	Truss EJ02	Truss Type Jack-Partial Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional)	18035482
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Builders FirstSource, Lake City FL 32055
7.350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31:46 2014 Page 1

ID: N8W8OLQbT8ydtVoouale7z6MiY-tXpz7l5hFmfdTQ6xTw1?8thpVKedAaxu6LuQWOzTAH8

-2-0-0
2-0-0
6-8-3
6-8-3
7-0-0
0-3-13

Scale = 1/24.5

Plate Offsets (X,Y) [2-0-3-0-0-1-3]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.70	Vert(LL) 0.22	5-8 >383 240	MT20 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.56	Vert(TL) -0.21	5-8 >387 180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL) -0.05	4 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)			Weight: 28 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

SLIDER Left 2x4 SP No.3 1-6-0

REACTIONS (lb/size) 4=127/Mechanical, 2=288/0-3-8 (min 0-1-8) 5=48/Mechanical

Max Horz 2=144(LC 12)

Max Uplift 4=96(LC 12) 2=75(LC 12) 5=-1(LC 12)

Max Grav 4=155(LC 2), 2=346(LC 2), 5=94(LC 3)

FORCES (lb) Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-721/394

BOT CHORD 2-5=-380/266

NOTES (7-9)

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C, End GCPI=0.18, MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SP No.2 crushing capacity of 585 psi.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 4, 75 lb uplift at joint 2 and 1 lb uplift at joint 5.

6) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

8) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.

9) Truss Design Engineer Julius Lee PE: Florida P.E. License No. 34869; Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

LOAD CASE(S) Standard

April 7, 2014

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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 566946	Truss EJ04	Truss Type Half Hip Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC	I8035484
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31:49 2014 Page 1 ID_N8W8OLQbT8ydtVoouale7z6MIY-H6V6dn8aYh1CKlrW83ailVJPfXkhNwLLol747jzTAh8				
Plate Offsets (X,Y): [2-0-4-0-0-1-3]						
LOADING (psf) TCCL 20.0 TCCL 7.0 BCLL 0.0 * BCDL 5.0		SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007		CSI TC 0.32 BC 0.28 WB 0.09 (Matrix-M)		DEFL in (loc) l/defl L/d Vert(LL) -0.06 6-9 >999 240 Vert(TL) -0.10 6-9 >850 180 Horz(TL) 0.01 2 n/a n/a
				PLATES MT20 Weight: 36 lb		GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-6-0						
BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>						
REACTIONS (lb/size) 2=288/0-3-8 (min. 0-1 8) 6=172/Mechanical Max Horz 2=74(LC 12) Max Uplift 2=87(LC 12) 6=64(LC 9) Max Grav 2=343(LC 2), 6=204(LC 2)						
FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-375/0						
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II, Exp C, Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 2 and 64 lb uplift at joint 6 8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) Note: Visually graded lumber designation SFP, represents new lumber design values as per SPIB. 11) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435						
LOAD CASE(S) Standard						



April 7, 2014

Job 569346	Truss HJ01	Truss Type Diagonal Hip Girder	Qty 4	Ply 1	MIKE ROBERTS LOT 1 CC	18035486
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31:53 2014 Page 1				
ID_ N8W8OLQbT8ydtVouuale7z6MIY-AukcT8B4bwXdpU8HNufewLTOW82zJhVwjw5IGUzTAh4						

Scale = 1/24

Plate Offsets (X,Y): [2:0-1-8,0-1 12]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.65	in (loc)	MT20	244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.51	Vert(LL) 0 13 7-8 >904 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.27	Vert(TL) -0 12 7-8 >957 180		
BCCL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0 02 5 n/a n/a		
				Weight: 47 lb FT = 20%	

LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No 3 1-6-0	BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 7-4-9 oc bracing. <div style="border: 1px solid black; padding: 5px; margin-top: 5px;"> MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS (lb/size) 5=130/Mechanical 2=368/0-4-15 (min 0-1-8) 6=192/Mechanical
 Max Horz 2=234(LC 4)
 Max Uplift 5=-152(LC 4) 2=-409(LC 4), 6=-280(LC 4)
 Max Grav 5=158(LC 2) 2=454(LC 2) 6=241(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-215/331 3-13=-497/476 4-13=-446/468
 BOT CHORD 2-15=-536/439 15-16=-536/439 8-16=-536/439 8-17=-536/439 7 17=-536/439
 WEBS 4-7=-476/581

NOTES (9-11)
 1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph: TCCL=4.2psf: BCCL=3.0psf: h=18ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) gable end zone: porch left and right exposed: Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tail by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 5, 409 lb uplift at joint 2 and 280 lb uplift at joint 6.
 6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 36 lb down and 46 lb up at 1-5-12, 36 lb down and 46 lb up at 1-5-12, 6 lb down and 53 lb up at 4-3-11 6 lb down and 53 lb up at 4-3-11 and 49 lb down and 103 lb up at 7 1 10 and 49 lb down and 103 lb up at 7-1 10 on top chord and 19 lb down and 41 lb up at 1-5-12 19 lb down and 41 lb up at 1-5-12 5 lb down and 36 lb up at 4-3-11 5 lb down and 36 lb up at 4-3-11 and 35 lb down and 57 lb up at 7 1 10, and 35 lb down and 57 lb up at 7 1-10 on bottom chord The design/selection of such connection device(s) is the responsibility of others
 8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 11) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25 Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=-44 6-9=-10



Continued on page 2

April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 566946	Truss HJ02	Truss Type Special Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC	18035487
Builders FirstSource, Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:31 55 2014 Page 1 ID_ N8W8OLQbt8ydtVooaule7z6MiY-6GsNuqCL7XnL2olgVJh6?mZKxygSnf9DAEaPLNzTAh2				

Scale = 1:15.4

Plate Offsets (X,Y): [2:0-0-7,0-1-8], [2:0-1 13,0-7-9]				
LOADING (psf) TCLL 20.0 TCDD 7 0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TPI2007	CSI TC 0.78 BC 0.13 WB 0.00 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.02 4-7 >999 240 Vert(TL) 0.02 4-7 >999 180 Horz(TL) -0.00 2 n/a n/a	PLATES MT20 GRIP 244/190 Weight 23 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3

REACTIONS (lb/size) 4=22/0-4-1 (min. 0-1-8), 2=294/0-6-5 (min. 0-1-8)

Max Horz 2=120(LC 4)

Max Uplift 4=71(LC 5) 2=362(LC 4)

Max Grav 4=58(LC 3) 2=357(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=-367/346

BOT CHORD 2-10=-336/333

NOTES (9-11)

1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDD=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C Encl GCpl=0.18, MWFRS (envelope) gable end zone; end vertical left exposed porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 4 and 362 lb uplift at joint 2.

6) "Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 35 lb up at 2-1-2 and 21 lb down and 46 lb up at 2-10-15 on top chord and 1 lb down and 26 lb up at 2-1-2 and 12 lb down and 35 lb up at 2-10-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.

11) Truss Design Engineer: Julius Lee PE; Florida P E License No 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert. 1-3=-44 4-5=-10

Concentrated Loads (lb)

Vert. 8=14(F) 9=37(B) 10=3(F) 11=16(B)

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
1109 Coastal Bay
Boynton Beach FL 33435

Job 566946	Truss T01G	Truss Type Common Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional) 7 350 s Sep 27 2012 MITEK Industries, Inc. Mon Apr 07 07:32:00 2014 Page 1 ID_N8WBOLQbT8ydtVooa9e7z6MIY-SEfGxXGTy3Pe8ZBeHtHhIqGlbzYpSwXyKwHA7bzTAgz	(8035489
Builders FirstSource, Lake City FL 32055						

Scale = 1:40.3

Plate Offsets (X,Y) [2 0-3-8,Edge], [14 0-3-8,Edge], [21 0-3-0,0-3-0]								
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.02	15	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL) -0.04	15	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(TL) 0.00	14	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)						
						Weight: 112 lb FT = 20%		

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS All bearings 20-0-0.

(lb) - Max Horz 2=119(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21 22 23, 24 19 18, 17 16

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20 21 22 23, 24 19, 18, 17 16

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

NOTES (13-15)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C; Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21 22 23, 24, 19 18, 17 16.

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

12) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.

15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

April 7, 2014

Job 566946	Truss T03	Truss Type Common Truss	Qty 2	Ply 1	MIKE ROBERTS LOT 1 CC	18036491
Builders FirstSource Lake City FL 32055		Job Reference (optional) 7.350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:32 04 2014 Page 1 ID_ N8W8OLQbT8ydtVoouale7z6MIY-L7vnnvJ_0lw3dBUPWILDsgRumamfOgKYF8FN8MzTAgy				

Scale = 1:32.9

Plate Offsets (X,Y): [2-0-3-0-0-1 7], [8-0-3-12-0-1 7]					
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.85	Vert(LL) 0.27	8-15	>717
TCDL 7.0	Lumber Increase 1.25	BC 0.63	Vert(TL) 0.23	8-15	>833
BCLL 0.0 *	Rep Stress Incr YES	WB 0.20	Horz(TL) -0.04	2	n/a
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)			
			PLATES GRIP MT20 244/190 Weight: 68 lb FT = 20%		

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 1-6-0 Right 2x4 SP No.3 1-6-0

REACTIONS (lb/size) 2=520/0-3-8 (min. 0-1-8) 6=520/0-3-8 (min. 0-1-8)

Max Horz 2=-68(LC 10)

Max Uplift 2=-247(LC 9) 6=-247(LC 8)

Max Grav 2=620(LC 2), 6=620(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-303/357 3-4=-645/999 4-5=-645/999 5-6=-303/357

BOT CHORD 2-8=-716/504 6-8=-716/504

WEBS 4-8=-527/258

NOTES (8-10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left and right exposed; porch left and right exposed C-C for members and forces & MWFRS for reactions shown. Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=247 6=247

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

10) Truss Design Engineer Julius Lee PE, Florida P.E. License No 34869 Address 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-1-3 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 568946	Truss T04	Truss Type Hip Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional)	18035492
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITEK Industries, Inc. Mon Apr 07 07:32:08 2014 Page 2
 ID_N8W8QLQbT8ydtVooa1e7z6MIY-Dm8HcGMU3XQV6ooAIYQ91WbXMB4fKJ88AmDbH7zTAg

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 12) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 13) Truss Design Engineer Julius Lee PE: Florida P.E. License No. 34869 Address, 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert. 1-3=-44, 3-6=-44, 6-7=-44 7-9=-44 8-16=-10

Concentrated Loads (lb)

Vert: 3=-83(F) 6=-83(F) 15=-220(F) 11=-162(F) 21=-83(F) 22=-83(F) 23=-83(F) 24=-83(F) 25=-83(F) 26=-83(F) 27=-38(F) 28=-38(F) 29=-38(F) 30=-38(F) 31=-38(F) 32=-38(F) 33=-38(F) 34=-162(F)



WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

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Julius Lee PE,
1109 Coastal Bay
Boynton Beach, FL 33435

Job 566946	Truss T06	Truss Type Hip Truss	Qty 1	Ply 1	MIKE ROBERTS - LOT 1 CC	18035494																																																												
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries Inc. Mon Apr 07 07:32:13 2014 Page 1																																																																
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<div style="display: flex; justify-content: space-between;"> -2-0-0 5-10-14 11-0-0 16-9-11 21-9-2 24-10-0 30-3-8 32-3-8 </div> <div style="display: flex; justify-content: space-between;"> 2-0-0 5-10-14 5-1-2 5-9-11 4-11-7 3-0-15 5-5-8 2-0-0 </div>																																																																		
		Scale 1:57.6																																																																
<div style="display: flex; justify-content: space-between;"> 5-10-14 11-0-0 16-9-11 24-10-0 25-10-4 30-3-8 </div> <div style="display: flex; justify-content: space-between;"> 5-10-14 5-1-2 5-9-11 8-0-6 1-0-4 4-5-4 </div>																																																																		
Plate Offsets (X,Y) [2:0-4-0,0-1-3], [6:0-6-0,0-2-8]																																																																		
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REACTIONS (lb/size) 2=753/0-3-8 (min. 0-1 8), 11=1059/0-3-8 (min. 0-1-8) Max Horz 2=-88(LC 10) Max Uplift 2=-224(LC 12) 11=-390(LC 9) Max Grav 2=896(LC 2) 11=1259(LC 2)																																																																		
FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown TOP CHORD 2-3=-458/45, 3-4=-1319/662, 4-5=-1046/592, 5-6=-910/586, 6-7=-945/505, 7-8=-594/267 8-9=-1062/835 BOT CHORD 2-16=-459/1109, 15-16=-459/1109 14-15=-161/742, 13-14=-161/742, 12-13=-95/641 11 12=-149/679, 9-11=-762/1114 WEBS 4-15=-341/283, 7 12=-1062/1021 8-12=-505/761 8-11=-1350/862																																																																		
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C' Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed and vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=224, 11=390 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 10) Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB. 11) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																																																		
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April 7,2014

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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 566946	Truss T08	Truss Type Special Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC	18035496																																																						
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Mon Apr 07 07:32:18 2014 Page 1																																																										
		ID_N8WBOLQbT8ydtVouuale7z6MIY-xh13jhUmjbh4JKZ5LebVRd0KIDYRguFcTJe7eYzTAgh																																																										
<div style="display: flex; justify-content: space-between;"> <div style="width: 40%;"> <p>2-0-0 7-0-9 13-10-13 20-9-2 24-10-0 30-3-8 32-3-8 2-0-0</p> <p>2-0-0 7-0-9 6-10-5 6-10-5 4-0-14 5-5-8</p> </div> <div style="width: 50%; text-align: right;"> <p>Scale = 1/57.8</p> </div> </div>																																																												
<div style="display: flex; justify-content: space-between;"> <div style="width: 40%;"> <p>10-2-12 10-2-12 17-6-15 24-10-0 25-10-4 30-3-8</p> <p>10-2-12 7-4-3 7-3-1 1-0-4 4-5-4</p> </div> <div style="width: 50%; text-align: right;"> <p>Plate Offsets (X,Y): [2:0-4-0-0-1 7], [4:0-3-0-0-3-4], [8:0-0-9,Edge]</p> </div> </div>																																																												
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REACTIONS (lb/size) 2=800/0-3-8 (min 0-1 8), 10=1113/0-3-8 (min. 0-1 10) Max Horz 2=107(LC 10) Max Uplift 2=238(LC 12) 10=343(LC 9) Max Grav 2=898(LC 2) 10=1259(LC 2)																																																												
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NOTES (8-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpl=0 18' MWFRS (envelope) and C-C Exterior(2) zone cantilever right exposed end vertical left exposed C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (It=lb) 2=238, 10=343. 7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 10) Truss Design Engineer: Julius Lee, PE, Florida P E License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435																																																												
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Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 566946	Truss T10	Truss Type Special Truss	Qty 3	Ply 1	MIKE ROBERTS LOT 1 CC	18035498																																				
Builders FirstSource, Lake City FL 32055					7.350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:32:23 2014 Page 1 ID_N8WBOLQbT8ydIVouale7z6MIY-HfYymPYvX8JNP6R28CBg8gj54E9yLCGLcbMJlZTAge																																					
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REACTIONS (lb/size) 2=743/0-3-8 (min. 0-1 B) 11=1069/0-3-8 (min. 0-1-9) Max Horz 2=107(LC 10) Max Uplift 2=238(LC 12) 11=350(LC 9) Max Grav 2=884(LC 2), 11=1271(LC 2)																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-683/161 3-4=-2178/1058, 4-5=-1578/675, 5-6=-1357/618 6-7=-1302/474 7-8=-1022/805, 8-9=-1081/829 BOT CHORD 2-14=-803/1917 13-14=-805/1933 12-13=-244/1137 11 12=-402/884 9-11=-752/1113 WEBS 4-13=-607/555 5-13=-295/905, 6-12=-377/370, 7-12=-953/1338, 7-11=-1108/669																																										
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Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 566946	Truss T11	Truss Type Half Hip Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC Job Reference (optional)	18035499
Builders FirstSource, Lake City FL 32055						
7.350 s Sep 27 2012 MiTek Industries, Inc. Mon Apr 07 07:32:26 2014 Page 2 ID_N8W8OLQbT8ydtVooaale7z6MIY-IEE5OQanq3hyGZAdpKINIJLe3SBCYX_nJZaYw4zTAgz						
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code						
13) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.						
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435						
LOAD CASE(S) Standard						
1) Regular: Lumber Increase=1.25, Plate Increase=1.25						
Uniform Loads (plf)						
Vert: 1-4=-44 4-9=-44 10-15=-10						
Concentrated Loads (lb)						
Vert: 4=-83(B) 7=-83(B) 9=-127(B) 14=-220(B) 8=-83(B) 19=-83(B) 20=-83(B) 21=-83(B) 22=-83(B) 23=-83(B) 24=-83(B) 25=-83(B) 26=-38(B) 27=-38(B) 28=-38(B) 29=-38(B) 30=-38(B) 31=-38(B) 32=-38(B) 33=-38(B) 34=-38(B)						



WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 566946	Truss T13	Truss Type Hip Truss	Qty 1	Ply 1	MIKE ROBERTS LOT 1 CC	18035501
Builders FirstSource, Lake City FL 32055		7 350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:32:30 2014 Page 1				
ID_N8W8OLQbT8ydtVooaale7z6MIY-a?TcEodluHBNIATP2ApJw9WOx3feULaNDByI3rzTAgV						
<div style="display: flex; justify-content: space-between;"> 5-9-3 11-0-0 16-9-11 21-7-5 26-0-0 </div> <div style="display: flex; justify-content: space-between;"> 5-9-3 5-2-14 5-9-11 4-9-10 4-4-11 </div>						
Scale = 1:44.8						
<div style="display: flex; justify-content: space-between;"> 5-9-3 11-0-0 16-9-11 26-0-0 </div> <div style="display: flex; justify-content: space-between;"> 5-9-3 5-2-14 5-9-11 9-2-5 </div>						
Plate Offsets (X,Y): [1.0-2-12,0-1 7], [4.0-6-0,0-2-8]						
LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0		SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TP12007		CSI TC 0.41 BC 0.57 WB 0.59 (Matrix-M)		DEFL in (loc) l/defl L/d Vert(LL) -0 19 8-9 >999 240 Vert(TL) -0 35 8-9 >885 180 Horiz(TL) 0 05 8 n/a n/a
				PLATES MT20 GRIP 244/190 Weight: 143 lb FT = 20%		
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No 3 1-6-0			BRACING TOP CHORD Structural wood sheathing directly applied or 4-5-9 oc purlins except end verticals. BOT CHORD Rigid ceiling directly applied or 7 1-3 oc bracing <div style="border: 1px solid black; padding: 2px; margin-top: 5px;"> MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>			
REACTIONS (lb/size) 1=698/0-3-8 (min. 0-1-8) 8=698/0-3-8 (min. 0-1-8) Max Horiz 1=90(LC 12) Max Uplift 1=-189(LC 12), 8=-176(LC 13) Max Grav 1=827(LC 2), 8=827(LC 2)						
FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown TOP CHORD 1-2=-560/222, 2-3=-1426/824 3-4=-1134/707 4-5=-931/647 5-6=-1070/661 BOT CHORD 1 12=-695/1222 11 12=-695/1222 10-11=-448/926, 9-10=-448/926, 8-9=-515/893 WEBS 3-11=-353/292, 6-8=-1072/652						
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C End GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed, C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 1=189 8=176. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code 10) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 11) Truss Design Engineer Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435						
LOAD CASE(S) Standard						



April 7, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
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 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 566948	Truss T15	Truss Type Special Truss	Qty 3	Ply 1	MIKE ROBERTS LOT 1 CC	18035503
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries, Inc. Mon Apr 07 07:32:33 2014 Page 1 ID_N8W8OLQbT8ydtVooaule7z6MIY-7a9kspgABCayceCzjINOXn8unGgmhdwpw8nPgAzTAgS 20-9-1 26-0-0 5-2-15 6-10-4 6-10-4 6-10-4				
Plate Offsets (X,Y) [1.0-3-8,Edge]						
LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.50 BC 0.61 WB 0.94 (Matrix-M)	DEFL in (loc) Vert(LL) -0.19 10-13 Vert(TL) -0.35 10-13 Horz(TL) 0.05 7	GRIP 244/190 Weight: 133 lb FT = 20%		
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-8-0						
BRACING TOP CHORD Structural wood sheathing directly applied or 4-4-4 oc purlins except end verticals BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing. MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.						
REACTIONS (lb/size) 1=746/0-3-8 (min. 0-1-8) 7=752/0-3-8 (min. 0-1-8) Max Horz 1=107(LC 12) Max Uplift 1=204(LC 12), 7=193(LC 13) Max Grav 1=827(LC 2) 7=827(LC 2)						
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-788/101 2-3=-1516/850 3-4=-1309/785 4-5=-1200/723 BOT CHORD 1 10=-703/1296 10-15=-337/827 9-15=-337/827 9-16=-337/827 8-16=-337/827 7-8=-557/1039 WEBS 3-10=-387/380 4-10=-268/487 4-8=-146/270 5-7=-1241/679						
NOTES (8-10) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=204 7=193. 7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB. 10) Truss Design Engineer Julius Lee, PE, Florida P.E. License No. 34869 Address, 1109 Coastal Bay Blvd. Boynton Beach FL 33435						
LOAD CASE(S) Standard						



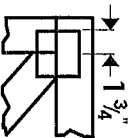
April 7, 2014

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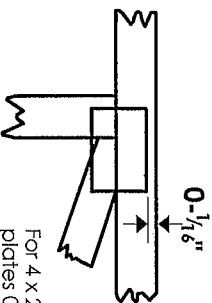
Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated
Dimensions are in ft-in-sixteenths
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss

This symbol indicates the required direction of slots in connector plates

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

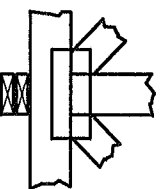
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated

BEARING

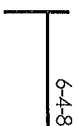


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur

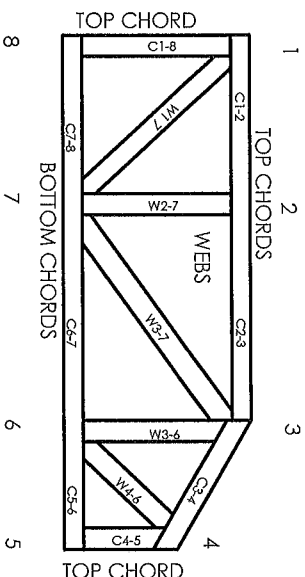
Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89 Design Standard for Bracing
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate
Connected Wood Trusses

Numbering System



6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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Boynton Beach, FL 33435



General Safety Notes

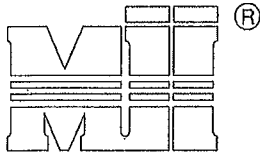
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11
2. Truss bracing must be designed by an engineer. For wide truss spacing individual lateral braces themselves may require bracing or alternative T, I, or Eliminator bracing should be considered
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TP11
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements
12. Lumber used shall be of the species and size and in all respects, equal to or better than that specified
13. Top chords must be sheathed or purlins provided at spacing indicated on design
14. Bottom chords require lateral bracing at 10 ft spacing or less, if no ceiling is installed, unless otherwise noted
15. Connections not shown are the responsibility of others
16. Do not cut or alter truss member or plate without prior approval of an engineer
17. Install and load vertically unless indicated otherwise
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
20. Design assumes manufacture in accordance with ANSI/TP11 Quality Criteria

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MITek Industries, Inc

MITek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern

T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

Brace Size for One-Ply Truss

Specified Continuous Rows of Lateral Bracing

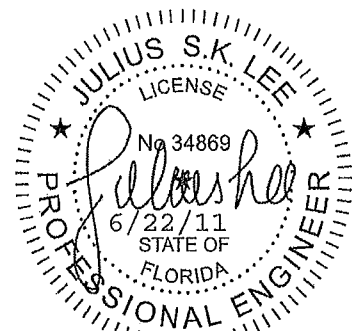
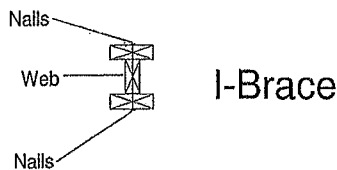
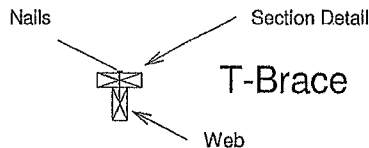
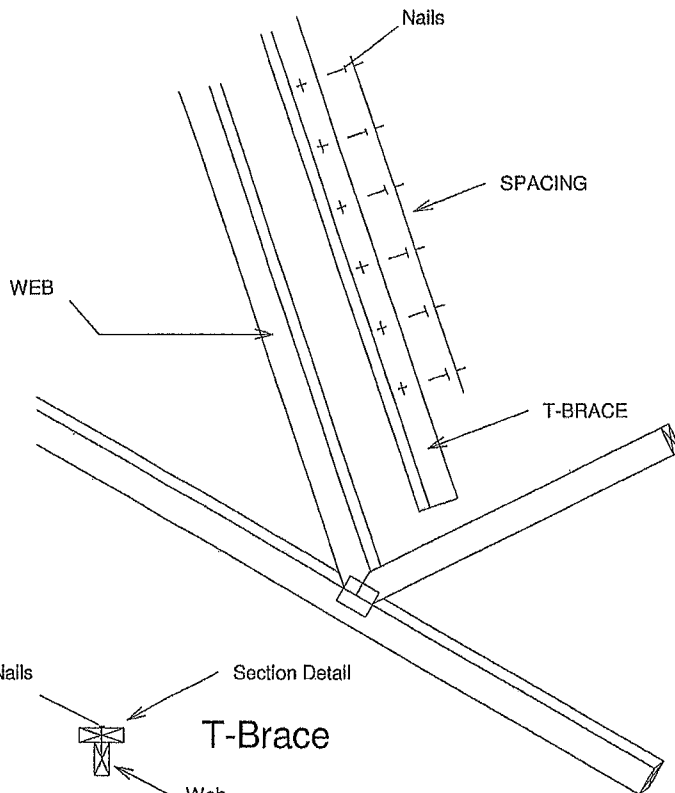
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss

Specified Continuous Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

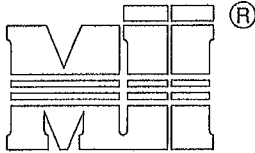


1109 COASTAL BAY
BOYNTON BC, FL 33435

JANUARY 1, 2009

LATERAL TOE-NAIL DETAIL

ST-TOENAIL_SP



MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO Page 1 of 1

NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

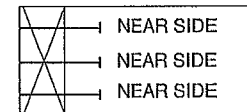
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88.0	80 6	69 9	68.4	59 7
	135	93.5	85 6	74.2	72.6	63 4
	162	108.8	99 6	86.4	84.5	73.8
3.25" LONG	128	74.2	67.9	58.9	57 6	50.3
	131	75.9	69.5	60.3	59 0	51.1
	148	81 4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

SIDE VIEW

3 NAILS



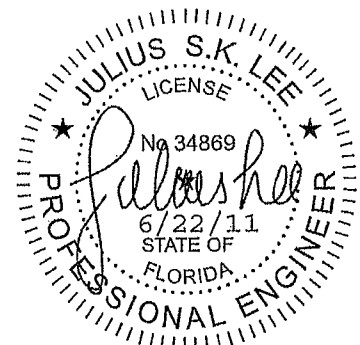
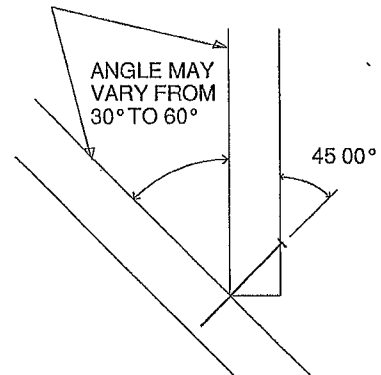
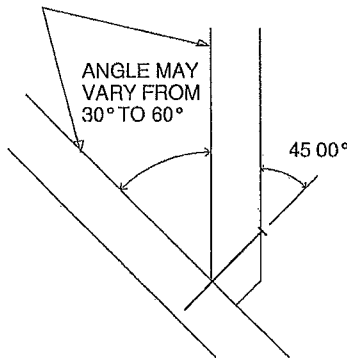
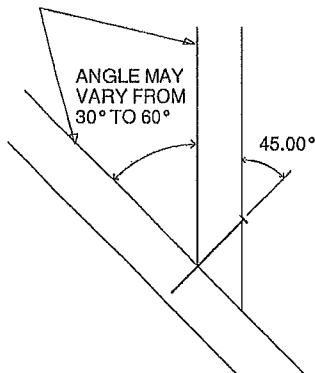
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

EXAMPLE

(3) - 16d NAILS (162" diam. x 3 5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84 5 (lb/nail) X 1 15 (DOL) = 291 5 lb Maximum Capacity



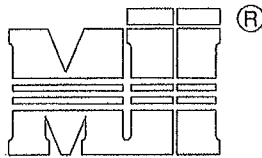
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 BOYNTON BC, FL 33435

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MITek Industries, Chesterfield, MO

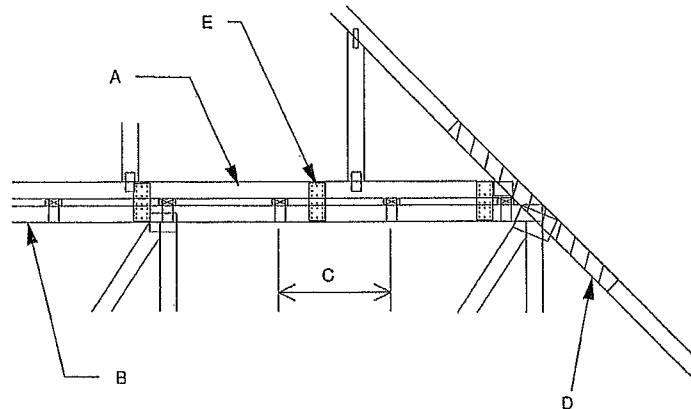


MITek Industries, Inc

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES)
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED

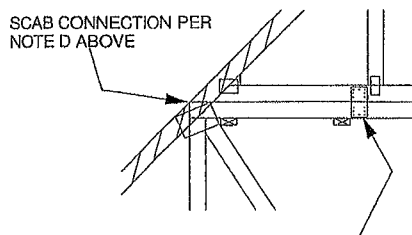
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 115 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 125 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



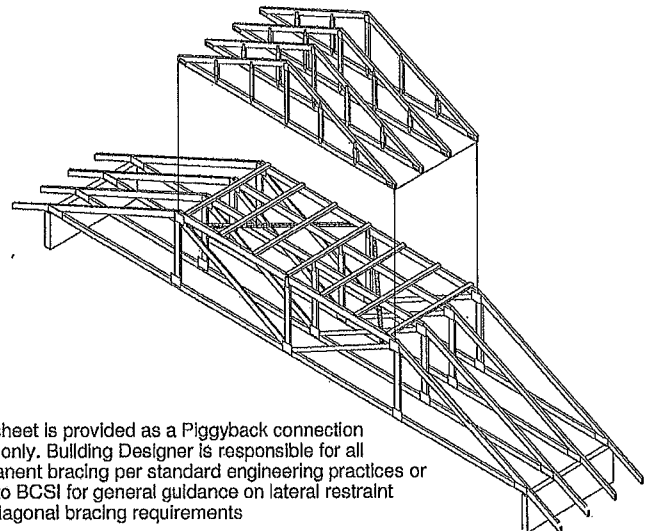
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER
NOTE D ABOVE

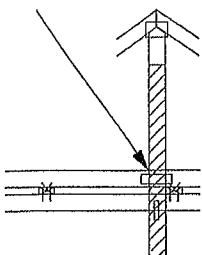


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



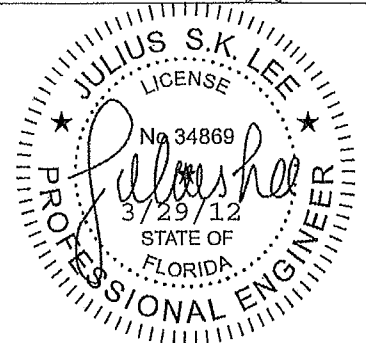
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK

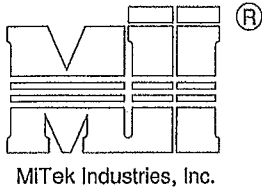


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB.

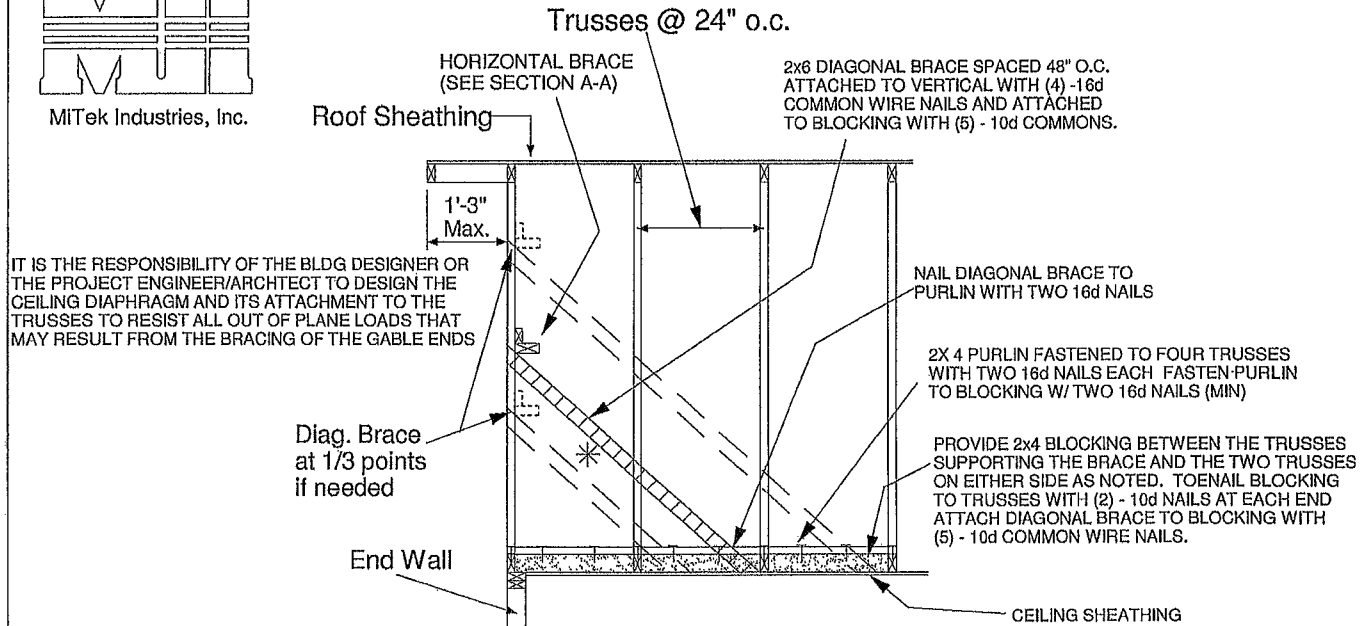
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@15'). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

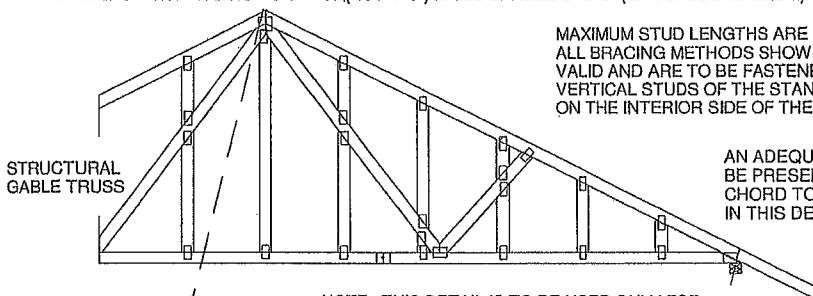
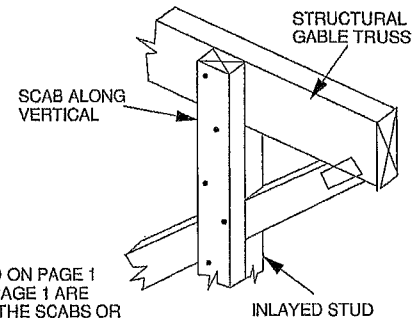
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

METHOD 1. ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2. ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

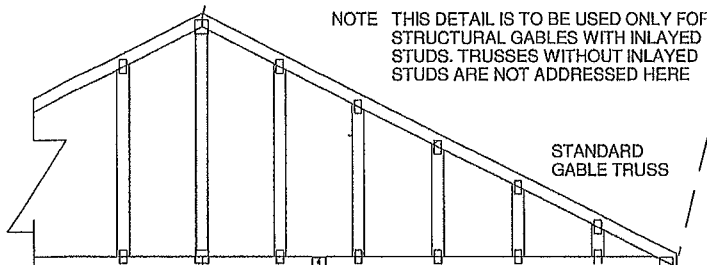
NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O C
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O C (2X 4 STUDS MINIMUM)

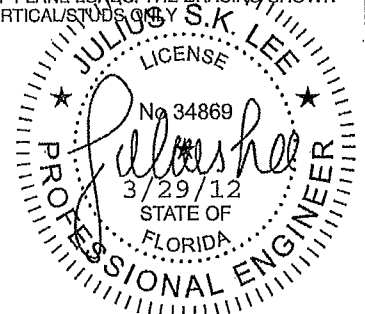


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1
ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



NOTE THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAVED STUDS. TRUSSES WITHOUT INLAVED STUDS ARE NOT ADDRESSED HERE



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6/12 - 4/12 PITCH =

