

Reviewed Columbianos Columbianos Code Compilanos

SINGLE FAMILY DWELLING FOR I WILMOTH RESIDENCE

ARCHITECTURAL DESIGN
2109 W U.S. HWY 90 SUITE 110-144
LAKE CITY, FL. 32055
(386)-155-8881

ELEYATION PLAN 6CALE: 1/4"=1'-0"

SHEET NUMBER

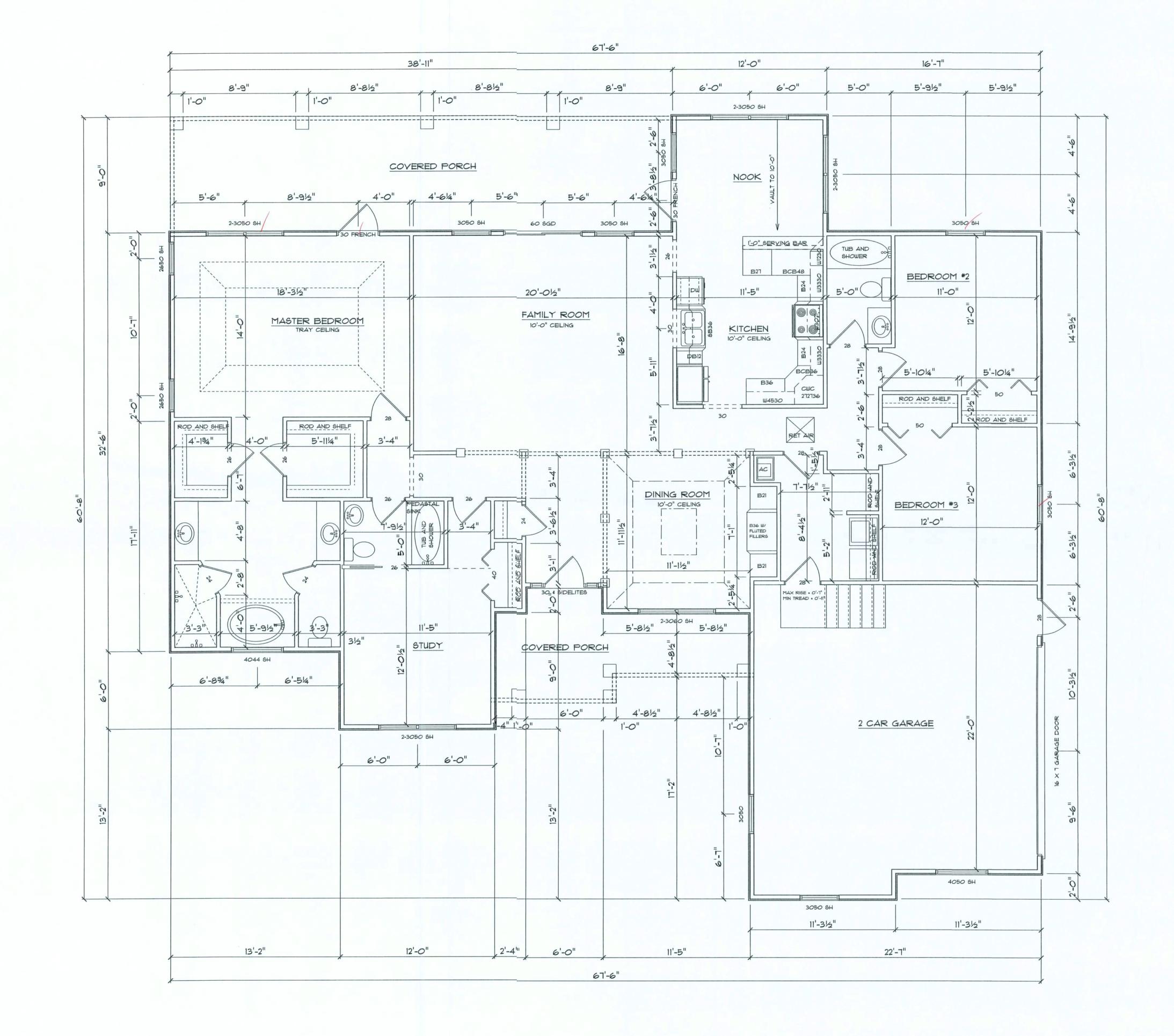
A-1
OF 4 SHEETS

AR0007005

LIVING AREA - 2180.5 SF GARAGE - 532.6 SF PORCHES - 479.7 SF

AREA SUMMARY

TOTAL AREA - 3192.8 SF



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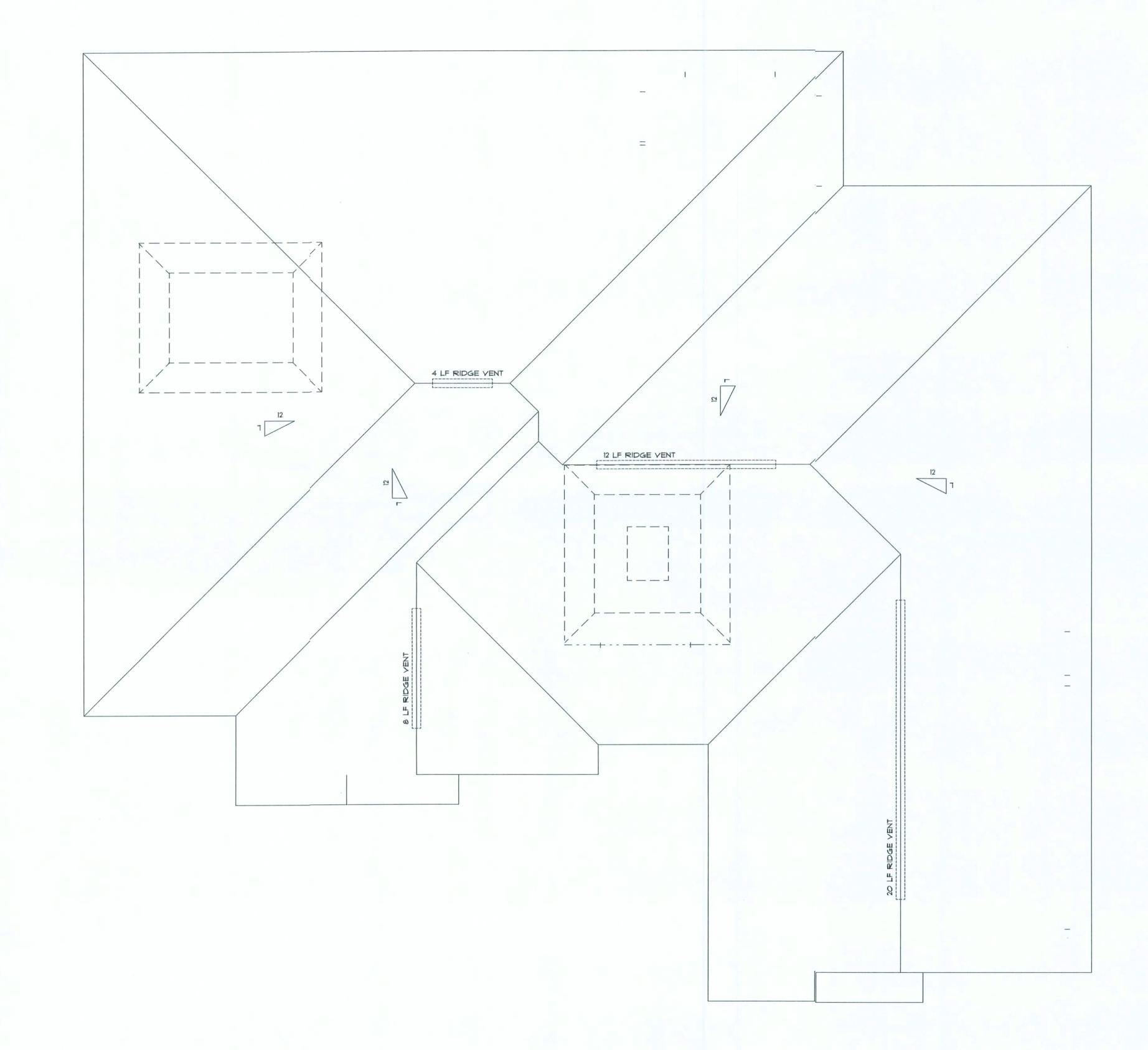
532.6 SF

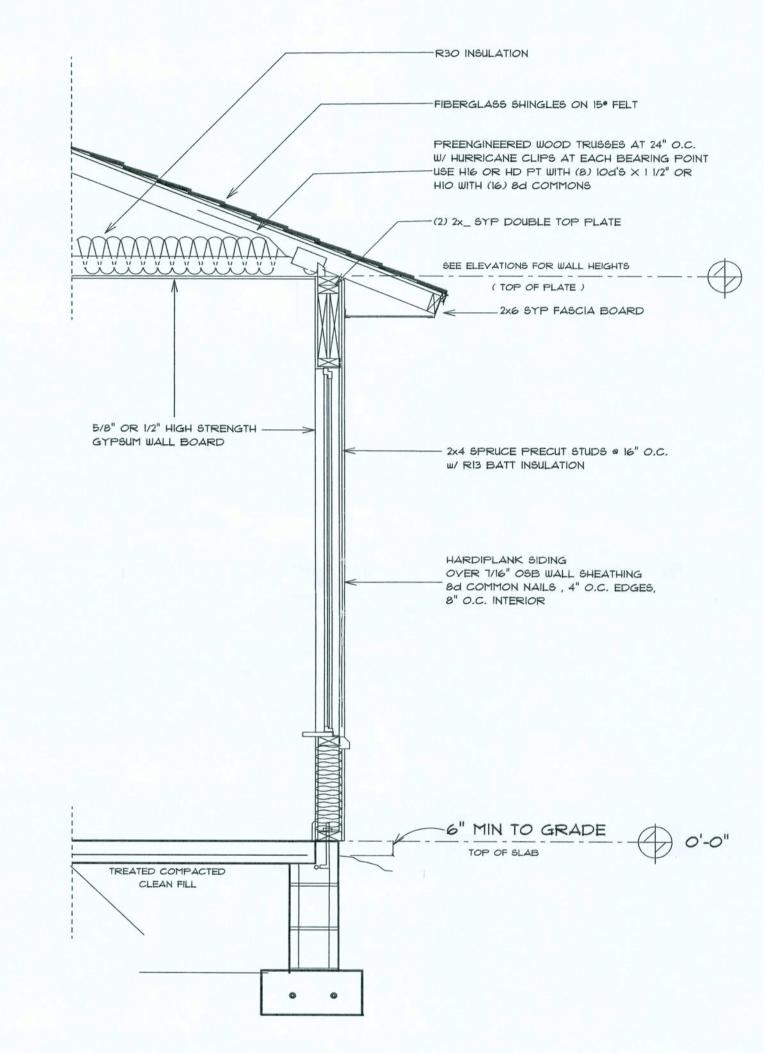
479,7 SF

GARAGE -

PORCHES -

AR0007005

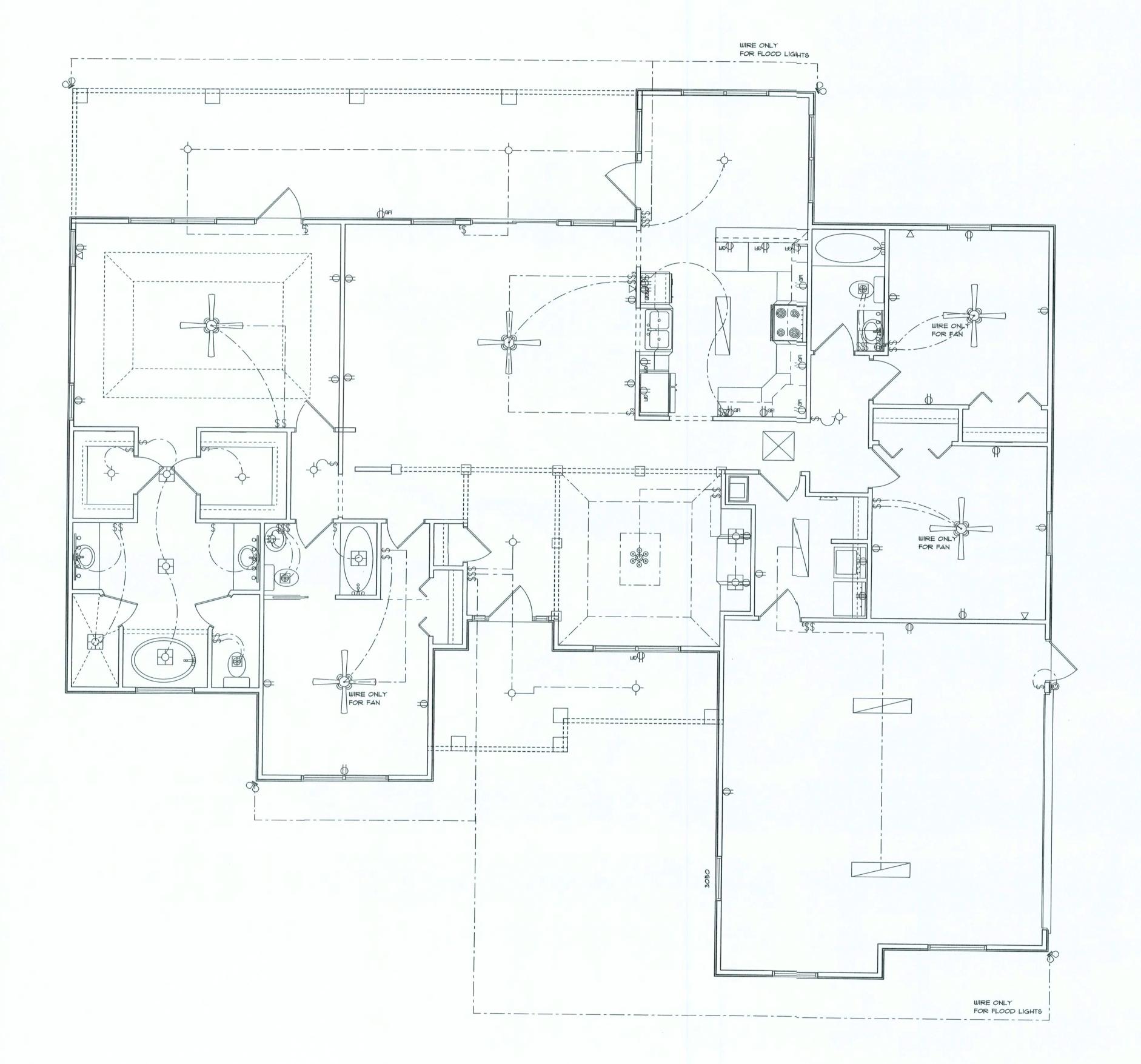




TYPICAL WALL SECTION

2 × 4 STUD WALL W/ SIDING

AR0007005



ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	5	
can light	9	
chandelier	1	9 <u>8</u> 9 989
double spotlight	4	Ø5
fluorescent fixture	6	
vanity bar light	1	<u> </u>
wall mount 1	1	©
electrical panel	1	t1
fan with light	3	(4)
light	8	
outlet	24	Ф
outlet 220v	2	#
outlet gfi	19	Фан
switch	36	\$
switch 3 way	8	\$3
telephone	5	∇

ELECTRICAL PLAN NOTES

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFIC CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

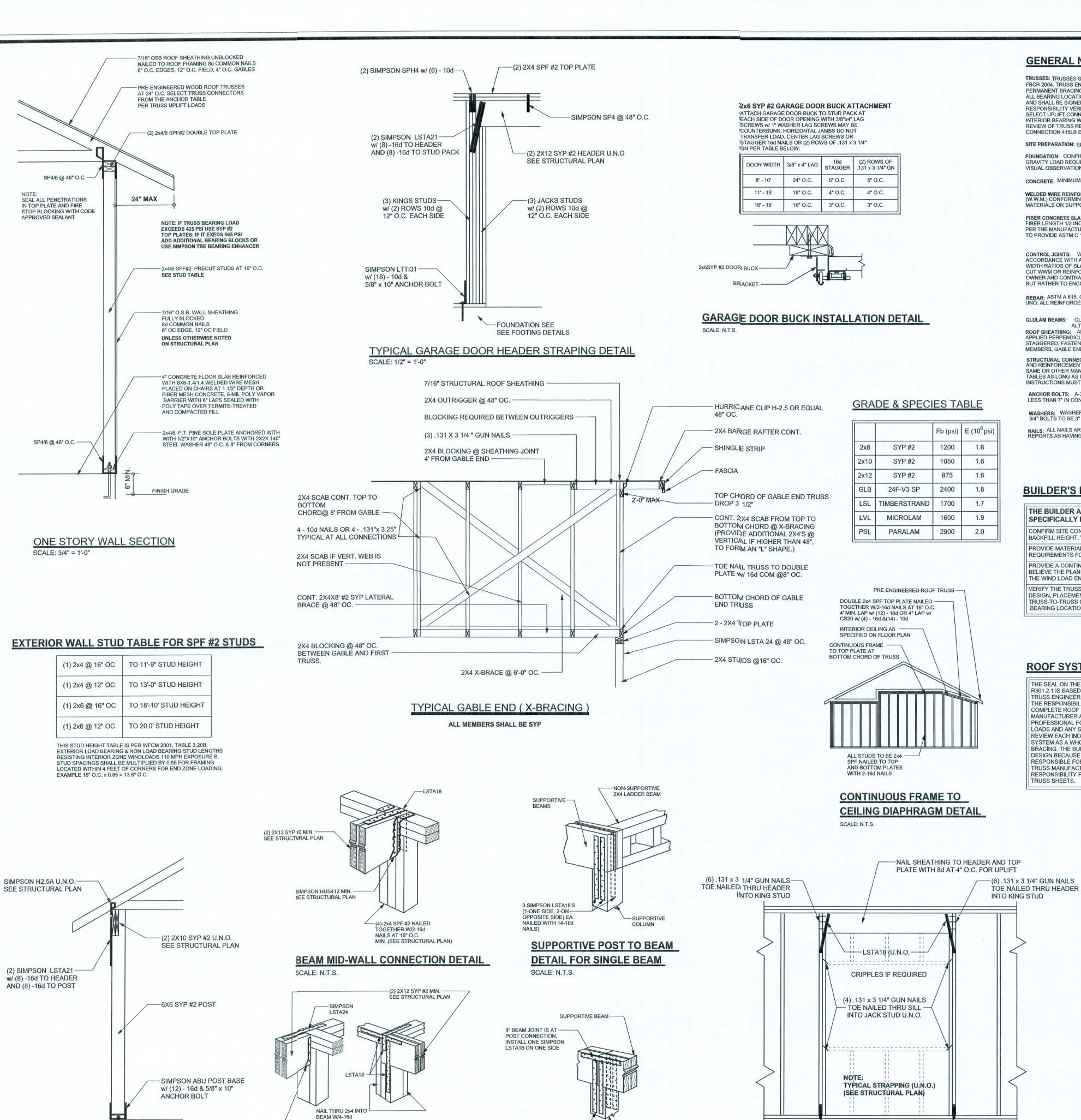
TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

AREA SUMMARY

LIVING AREA - 2180.5 SF GARAGE - 532.6 SF PORCHES - 479.7 SF TOTAL AREA - 3192.8 SF



(2-ONE SIDE, 2-ON

SUPPORTIVE CENTER POST TO BEAM DETAIL

OTHER SIDE)

SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIET AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR NTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END,

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO. STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOAD ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

 ${\bf NAILS}:$ ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

DESIGN DATA

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 265

< 235

< 320

< 365

< 535

< 565

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 600

< 760

< 1065

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR*

H4

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWN

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS*

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU66

TO PLATES TO RAFTER/TRUSS

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

22 -10d

16 -10d

16 -10d

16 -10d

TO FOUNDATION

-5/8" THREADED ROL

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROL

12" EMBEDMENT

2-5/8" THREADED ROD

TOSTUDS

4-10d

4-10d

8-10d

8-10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

12" EMBEDMENT

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

6 -10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

18-10d, 1 1/

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION .) BASIC WIND SPEED = 110 MPH

.) WIND EXPOSURE = B

) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II

ROOF ANGLE = 10-45 DEGREES

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

6.) MEAN ROOF HEIGHT = <30 FT

) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) Zone Effective Wind Area (ft2) 19.9 -21.8 18.1 -18.1 19.9 -25.5 18.1 -21.8 -40.6 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows | 21.8 | -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9 16x7 Garage Door | 18.5 | -21.0

DESIGN	LOADS	
FLOOR		
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	
	12 PSF (12:12 AND GREATER)	
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)	

REVISIONS

/INDLOAD ENGINEER: Mark Disosway PE No.53915, POB 868, Lake City, FL DIMENSIONS:

imensions. Refer all questions to Mark Disosway, P.E. for resolution Do not proceed without clarification OPYRIGHTS AND PROPERTY RIGHTS lark Disosway, P.E. hereby expressly rese common law copyrights and property right ese instruments of service. This document i not to be reproduced, altered or copied in an orm or manner without first the express writter

mission and consent of Mark Disosway CERTIFICATION: I hereby certify that I have amined this plan, and that the applicable ortions of the plan, relating to wind engine mply with section R301.2.1, florida building de residential 2004, to the best of my

IMITATION: This design is valid for one uilding, at specified location.

Stanley Crawford Construction

> Larry Wilmoth Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056

Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: January 08, 2007

FINALS DATE:

08 / Jan / 07

CHECKED BY

JOB NUMBER: 701081 DRAWING NUMBER

> **S-1** OF 3 SHEETS

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

-SP4 OR (2) H2.5A OR (2) SSP-

ALL OPENINGS (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL HEADER STRAPING DETAIL

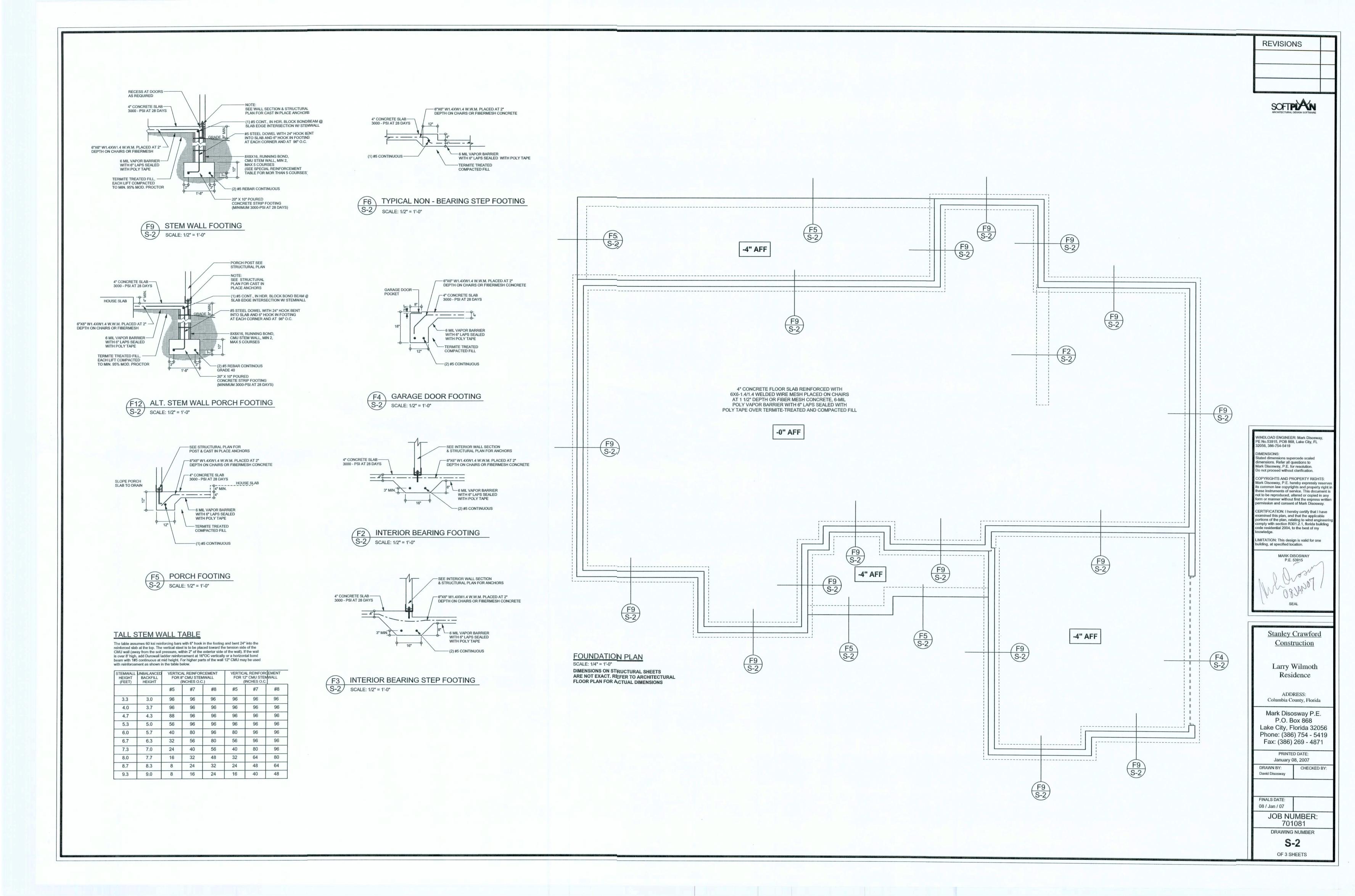
ACI530.1-02 Section 8" block bearing walls F'm = 1500 psi ASTM C 270, Type N, UNO 2.2 Grout ASTM C 476, admixtures require approval ASTM C 90-02, Normal weight, Hollow, CMU standard medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column ASTM C 216-02, Grade SW, Type FBS, 2.3 Clay brick standard 5.5"x2.75"x11.5" ASTM 615, Grade 60, Fy = 60 ksi, Lap Reinforcing bars, #3 - #11 splices min 48 bar dia. (30" for #5)

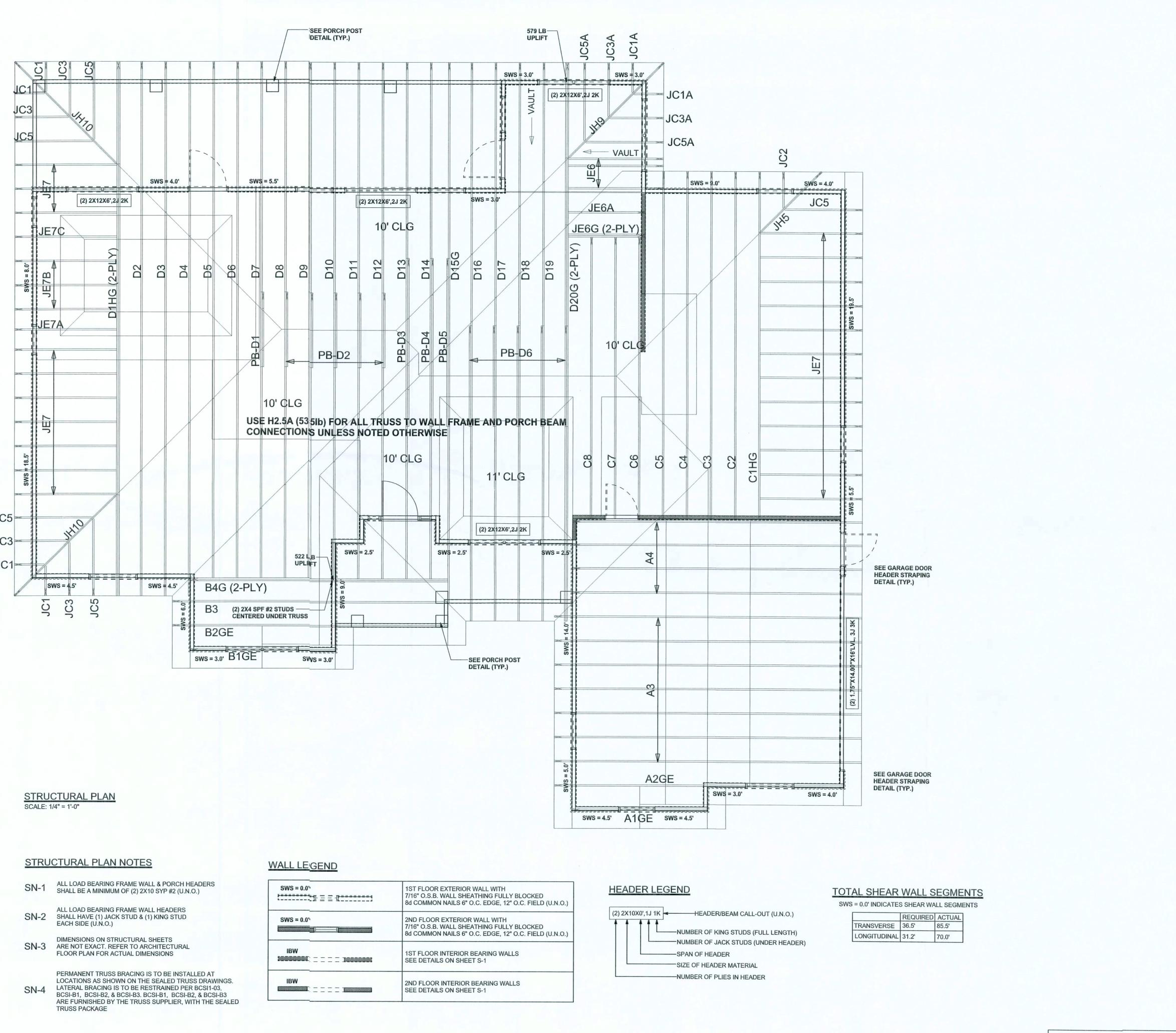
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS 2.4F | Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. 3.3.E.7 | Movement joints Contractor assumes responsibility for type and location of movement joints if not

detailed on project drawings.





SOFTPIA

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Stanley Crawford
Construction

Larry Wilmoth Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
January 08, 2007
WN BY: CHECKED BY:

David Disosway

FINALS DATE: 08 / Jan / 07

JOB NUMBER: 701081 DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS JOB #3499