		uilding Permit on Premises During Con	struction	PERMIT 000027450
APPLICANT JERRY CUNNINGHAM	4	PHONE	813 690-2523	
ADDRESS 11505 MELLOW CREEK I	ANE	RIVERVEW		FL 33569
OWNER JERRY CUNNINGHAM		PHONE	813 690-2523	
ADDRESS 529 HAMMOCK HILL C	CIRCLE	LAKE CITY		FL 32024
CONTRACTOR OWNER BUILDER		PHONE		
LOCATION OF PROPERTY 441S, TR C	ON HAMMOCK HILL O	CIRCLE, BACK OF CIRC	CLE,	
THROUGH	I GATE 1/8 OF A MILI	3		
TYPE DEVELOPMENT SFD,UTILITY	EST	IMATED COST OF CO	NSTRUCTION	148900.00
HEATED FLOOR AREA 2128.00	TOTAL ARE	A 2978.00	HEIGHT 17	.80 STORIES 1
FOUNDATION CONCRETE WALL	S FRAMED R	OOF PITCH 5/12	FLO	OOR SLAB
LAND USE & ZONING AG-3		MAX	HEIGHT 35	5
Minimum Set Back Requirments: STREET-H	FRONT 30.00	REAR	25.00	SIDE 25.00
NO. EX,D.U. 0 FLOOD ZONE	X	DEVELOPMENT PERM	MIT NO.	
PARCEL ID 16-6S-17-09710-000	SUBDIVISION	J		
LOT BLOCK PHASE	UNIT	-	L ACRES 28.0	20
LOI BLOCK FHASE _	UNIT		LACKES _28.0	<i>G</i>
Culvert Permit No. Culvert Waiver Comments: Comments: Comments: Comments of Co		/ / <u>H</u>	Applicant/Owner/OD roved for Issuance	N
			Check # or Ca	sh 2064
FOR BU	ILDING & ZONIN	G DEPARTMENT	ONLY	(6 + (6) 1)
Temporary Power	Foundation		Monolithic	(footer/Slab)
date/app. by		date/app. by	_ '''ononana	date/app. by
Under slab rough-in plumbing	Slab		Sheathing/N	Nailing
date/app	estimate	date/app. by		date/app. by
Framing date/app. by	Rough-in plumbing ab	ove slab and below wood	floor	
Electrical rough-in				date/app. by
date/app. by	Heat & Air Duct	date/app. by	Peri. beam (Lintel	date/app. by
Permanent power	C.O. Final	ашо арр. бу	Culvert	dute upp. by
date/app. by	d	ate/app. by	жановия. «Э.4.YVA — II	date/app. by
M/H tie downs, blocking, electricity and plumbing	date/app.	hv	Pool	
Reconnection	Pump pole	Utility Pol		date/app. by
date/app. by M/H Pole Trav	date/s	app. by	date/app. by Re-roof	
date/app by		ate/ann hy	1001	date/app by

FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ _____ CULVERT FEE \$ _____ TOTAL FEE _ 849.78

INSPECTORS OFFICE _____ CLERKS OFFICE _____

ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00

CERTIFICATION FEE \$

14.89

SURCHARGE FEE \$

WASTE FEE \$

14.89

745.00

BUILDING PERMIT FEE \$

0.00

MISC. FEES \$

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

THIS INSTRUMENT WAS PREPARED BY:

05-61 TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056 32056-1328

Property Appraiser's Identification Number R09710-000 Inst:2005002669 Date:02/04/2005 Time:12:56

Doc Stamp-Deed: 581.00
DC,P.DeWitt Cason,Columbia County B:1037 P:676

WARRANTY DEED

THIS INDENTURE, made this 26th day of January, 2005, BETWEEN RICHARD MOORE and SABRINA MOORE, Husband and Wife whose post office address is 781 Metalski Road, Gaylord, MI 49735, of the State of Michigan, grantor*, and JERRY E. CUNNINGHAM and BELINDA CUNNINGHAM, Husband and Wife whose post office address is 11505 Mellow Creek Lane, Riverview, FL 33569, of the State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year. Restrictions, easements and outstanding

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Inst:2005002669 Date:02/04/2005 Time:12:56

Doc Stamp-Deed :

581.00

_DC,P.PeWitt Cason,Columbia County B:1037 P:677

Signed, sealed and delivered in our presence:

(Signature of First Witness)

STEPHEN P. ALLEN

(Typed Name of First Witness)

(Signature of Second Witness)

(Typed Name of Second Witness)

MOORE RICHARD Printed Name

(SEAL)

Grantor

SABRINA MOORE Printed Name

STATE OF Michigan COUNTY OF BEGO

The foregoing instrument was acknowledged before me this 260 day of January, 2005, by RICHARD MOORE and SABRINA MOORE, Husband and Wife who are personally known to me or who have produced as identification and who did not take an oath.

My Commission Expires:

Notary Public

Printed, typed, or stamped name:

CYNTHIA G. FITZGERALD Notary Public, Otsego County, MI My Commission Expires Mar 19, 2008

ALL OF THE FOLLOWING LYING AND BEING IN TOWNSHIP 6 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA:

Section 21: The Northeast Quarter of the Northeast Quarter (NE 1/4 of NE 1/4). (40+ acres)

Section 22: The Northwest Quarter of the Northwest Quarter (NW 1/4 of NW 1/4) and the South Half of the Northeast Quarter of the Northwest Quarter (S 1/2 of NE 1/4 of NW 1/4) West of State Road No. 2 and begin at a point located at the Northwest corner of the Southwest Quarter of the Northwest Quarter (SW 1/4 of NW 1/4) of Section 22, run thence South 285 feet, thence run East to a point 420 feet West of a road, thence run North 210 feet, thence run East to a road, thence run North to the Quarter Section Line and thence run West to the point of beginning, LESS four (4) acres conveyed to Jasper L. Rowe and more particularly described in the Deed recorded in Official Records Book 5, at page 241.

LESS AND EXCEPT the Pine Oak Hammock subdivision more particularly described as:

A portion of the Northeast 1/4 of the Northeast 1/4 of Section 21 TOGETHER WITH a portion of Section 22, all being in Township 6 South, Range 17 East, Columbia County, Florida and being more particularly described as follows: Begin at the Northwest corner of the Southwest 1/4 of the Northwest 1/4 of said Section 22; thence South 01°45'29" East along the West line of the Southwest 1/4 of the Northwest 1/4, a , distance of 285.00 feet; thence North 88°06'25" East, parallel to the North line of the Southwest 1/4 of the Northwest 1/4 of said Section 22, a distance of 1040.10 feet to a point, 420.00 feet Westerly of, as measured along the Easterly prolongation of the last said line from the Westerly right-of-way line of State Road No. 25 (U. S. Highway 41 and 441, as per S.R.D. Right-of-Way Map Section 2903-204, dated January 13, 1958); thence North 01°45'29" West, 210.00 feet; thence North 88°06'26" East parallel to the North line of the Southwest 1/4 of the Northwest 1/4 of said Section 22, a distance of 15.03 feet to an intersection with the Westerly line of the apparent location of those lands described and recorded in Deed Book 5, page 251 of the Public Records of said Columbia County; thence North 08°34'43" East along last said line, 382.32 feet to the Northerly line of the apparent location of said lands described and recorded in Deed Book 5, page 251; thence South 86°58'23" East along last said line, 437.89 feet to the Westerly right-of-way line of aforesaid State Road No. 25; thence North 08°34'17" East along said Westerly right-of-way line of State Road No. 25, a distance of 407.24 feet to the North line of the South 1/2 of the Northeast 1/4 of the Northwest 1/4 of said Section 22; thence South 88°08'10" West, 303.53 feet to the East line of the Northwest 1/4 of the Northwest 1/4 of said Section 22; thence North 01°40'33" West along last said line, 664.03. feet, to the North line of said Section 22; thence South 88°09'54" West along last said line, 1310 feet, more or less, to its intersection with the centerline of Hammock Branch (a 10 foot wide, more or less, creek); thence Southerly along said centerline run 2090 feet, more or less to its intersection with the South line of the Northeast 1/4 of the Northeast 1/4 of said Section 21; thence North 88°17'02" East along last said line, 462 feet, more or less, to the POINT OF BEGINNING.

Inst:2005002669 Date:02/04/2005 Time:12:56

Doc Stamp-Deed : 581.00

_____DC,P.DeWitt Cason,Columbia County B:1037 P:678

STATE OF FLORIDA W

COUNTY OF COLUMBIA HIS BOTTUGH

day of _

Sworn to (or affirmed) and subscribed before me

or Produced Identification

Ti onles 10, 11,09

Contractors License Number

Competency Card Number

NOTARY STAMP/SEAL

che MESSIZ

Columbia County Building Permit Application

<u>TIME LIMITATIONS OF APPLICATION</u>: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this

written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)	Contractor's License Number Columbia County Competency Card Number	_
Affirmed under penalty of perjury to by the <u>Contractor</u> and Personally known or Produced Identification		20
State of Florida Notary Signature (For the Contractor)	SEAL:	

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE GAINESVILLE FL 32606 PH: 386-462-1340 - 352-375-6329

October 28, 2008

Job: Cunningham Residence

Re: Garage door header strapping

Dear Sir:

Install Simpson LSTA24 on each end of the garage door opening attaching the header to the jack studs. Install HTT22 for attachment of the jack studs to the foundation.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

Bruce Schafer, P.E. #48984

7104 N. W. 42nd Lane

Gainesville, Florida 32606



RE: CUNRES -

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Project Customer: JERRY CUNNINGHAM Project Name: CUNNINGHAM RES

Lot/Block: 0

Subdivision: HAMMOCK HILL

Address:

City: FT WHITE State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 7.0

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 40.0 psf

This package includes 45 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E5145988	A1	12/10/08	18	E5146005	A7	12/10/08
2	E5145989	A10	12/10/08	19	E5146006	A8	12/10/08
3	E5145990	A11	12/10/08	20	E5146007	A9	12/10/08
4	E5145991	A12	12/10/08	21	E5146008	В	12/10/08
5	E5145992	A13	12/10/08	22	E5146009	B1	12/10/08
6	E5145993	A14	12/10/08	23	E5146010	B2	12/10/08
7	E5145994	A15	12/10/08	24	E5146011	B3	12/10/08
8	E5145995	A16	12/10/08	25	E5146012	BET	12/10/08
9	E5145996	A17	12/10/08	26	E5146013	C	12/10/08
10	E5145997	A18	12/10/08	27	E5146014	C1	12/10/08
11	E5145998	A19	12/10/08	28	E5146015	C2	12/10/08
12	E5145999	A2	12/10/08	29	E5146016	CJ01	12/10/08
13	E5146000	A20	12/10/08	30	E5146017	CJ09	12/10/08
14	E5146001	A3	12/10/08	31	E5146018	D	12/10/08
15	E5146002	A4	12/10/08	32	E5146019	DET	12/10/08
16	E5146003	A5	12/10/08	33	E5146020	EET	12/10/08
17	F5146004	A6	12/10/08	34	F5146021	FGT	12/10/08

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

No 43144

No 43144

No 43144

Marvin A. Strzyzewski, FL Lic. #43144

Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

December 10,2008

RE: CUNRES -

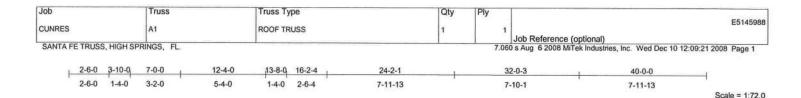
Site Information:

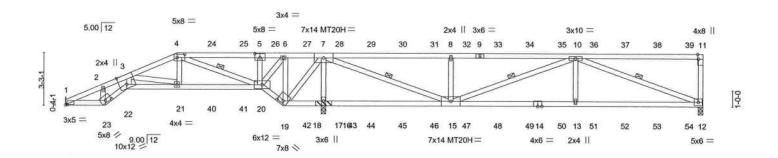
Project Customer: JERRY CUNNINGHAM Project Name: CUNNINGHAM RES Lot/Block: 0 Subdivision: HAMMOCK HILL

Address:

State: FL City: FT WHITE

No.	Seal#	Truss Name	Date
35	E5146022	EJ7	12/10/08
36	E5146023	EJ7A	12/10/08
37	E5146024	J01	12/10/08
38	E5146025	J01A	12/10/08
39	E5146026	J01B	12/10/08
40	E5146027	J07	12/10/08
41	E5146028	J07A	12/10/08
42	E5146029	J07B	12/10/08
43	E5146030	J1	12/10/08
44	E5146031	J3	12/10/08
45	E5146032	J5	12/10/08





	2-6-0 1-4-0	3-2-0 5-4-	0 1-4-0	2-6-4		7-11-13		7-10-1		7-11-13	•
Plate Of	fsets (X,Y): [1:0-1-6,0-0-2], [4:0-5-12,	0-2-8], [5:0-4-0	,0-3-0], [1	1:0-3-8,Edge	e], [19:0-4-8,0-1-	12], [22:0-5-8,0)-7-0], [2:	3:0-4-8,0-1-1	2]	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.99	Vert(LL)	-0.16 13-15	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.51 13-15	>559	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.17 12	n/a	n/a	1150,77750	
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	ix)	A CONTRACTOR OF THE PARTY OF TH		N. C. C.	/07URD	Weight: 206	lb

BRACING

WEBS

TOP CHORD

BOT CHORD

32-0-3

end verticals.

1 Row at midpt

Installation guide.

24-2-1

LUMBER TOP CHORD 2 X 4 SYP No.2 *Except*

2-6-0 3-10-0

5-9: 2 X 4 SYP 2400F 2.0E BOT CHORD 2 X 4 SYP No.2 *Except*

14-19.12-14: 2 X 4 SYP No.2D

7-0-0

2 X 4 SYP No.3 *Except* WEBS

11-12: 2 X 4 SYP No.2D, 7-15: 2 X 4 SYP No.2

REACTIONS (lb/size) 1=528/Mechanical, 12=1561/0-3-8, 17=4427/0-4-8 (0-3-8 + bearing block)

13-8-0 16-2-4

Max Horz 1=95(LC 5)

Max Uplift1=-63(LC 5), 12=-430(LC 3), 17=-984(LC 4)

12-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1502/201, 2-3=-1551/234, 3-4=-1076/152, 4-24=-304/1397, 24-25=-304/1397, 5-25=-304/1397, 5-26=-304/1397, TOP CHORD

6-26=-304/1397, 6-27=-392/1939, 7-27=-392/1939, 7-28=-1557/551, 28-29=-1557/551, 29-30=-1557/551,

30-31=-1557/551, 8-31=-1557/551, 8-32=-1557/551, 9-32=-1557/551, 9-33=-1557/551, 33-34=-1557/551, 34-35=-1557/551, 10-35=-1557/551, 10-36=-268/70, 36-37=-268/70, 37-38=-268/70, 38-39=-268/70, 11-39=-268/70,

11-12=-480/203

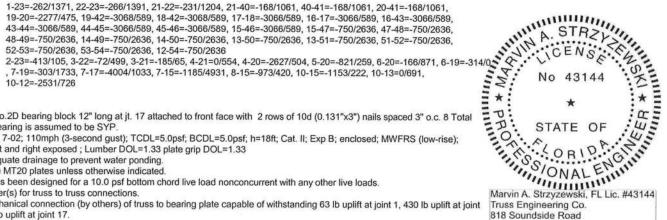
BOT CHORD 1-23=-262/1371, 22-23=-266/1391, 21-22=-231/1204, 21-40=-168/1061, 40-41=-168/1061, 20-41=-168/1061,

NOTES

WEBS

- 1) 2 X 4 SYP No.2D bearing block 12" long at jt. 17 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1, 430 lb uplift at joint 12 and 984 lb uplift at joint 17.

Continued on page 2



Edenton, NC 27932

FL COA #7239

40-0-0

Structural wood sheathing directly applied or 4-1-3 oc purlins, except

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

4-20, 7-15, 10-15, 10-12

Rigid ceiling directly applied or 3-0-9 oc bracing

December 10,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



nton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
CUNRES	A1	ROOF TRUSS	1	1	E514598
		7			Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL.

7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:22 2008 Page 2

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 326 lb down and 105 lb up at 7-0-0, 164 lb down and 56 lb up at 9-0-12, 164 lb down and 56 lb up at 11-0-12, 164 lb down and 56 lb up at 13-0-12, 125 lb down and 73 lb up at 15-0-12, 125 lb down and 73 lb up at 17-0-12, 125 lb down and 73 lb up at 19-0-12, 125 lb down and 73 lb up at 21-0-12, 125 lb down and 73 lb up at 27-0-12, 125 lb down and 73 lb up at 25-0-12, 125 lb down and 73 lb up at 29-0-12, 125 lb down and 73 lb up at 31-0-12, 125 lb down and 73 lb up at 33-0-12, 125 lb down and 73 lb up at 35-0-12, and 125 lb down and 73 lb up at 37-0-12, and 125 lb down and 73 lb up at 39-0-12 on top chord, and 254 lb down and 13 lb up at 7-0-0, 22 lb down at 9-0-12, 22 lb down at 11-0-12, 94 lb down at 15-0-12, 94 lb down at 17-0-12, 94 lb down at 19-0-12, 94 lb down at 21-0-12, 94 lb down at 23-0-12, 94 lb down at 25-0-12, 94 lb down at 27-0-12, 94 lb down at 29-0-12, 94 lb down at 31-0-12, 94 lb down at 33-0-12, 94 lb down at 35-0-12, and 94 lb down at 37-0-12, and 94 lb down at 39-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-11=-60, 1-23=-20, 22-23=-20, 20-22=-20, 19-20=-20, 12-19=-20

Concentrated Loads (lb)

Vert: 4=-326(F) 21=-254(F) 24=-164(F) 25=-164(F) 26=-164(F) 27=-125(F) 28=-125(F) 29=-125(F) 30=-125(F) 31=-125(F) 32=-125(F) 33=-125(F) 33=-125(F) 34=-125(F) 35=-125(F) 36=-125(F) 37=-125(F) 38=-125(F) 39=-125(F) 40=-11(F) 41=-11(F) 42=-47(F) 43=-47(F) 44=-47(F) 45=-47(F) 46=-47(F) 4 50=-47(F) 51=-47(F) 52=-47(F) 53=-47(F) 54=-47(F)

818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Ply E5145989 CUNRES A10 **ROOF TRUSS** Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:23 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 12-1-0 17-6-0 22-2-3 28-7-1 37-0-0 6-8-0 5-5-0 5-5-0 4-8-3 6-4-13 8-4-15 Scale = 1:68.0 5x5 = 5.00 12 5x5 = 4x12 = 4x6 = 3x4 = 5 6 7 3 5x5 = 7x8 = 12 14 4x5 = 11 5x8 = 10 2.25 12 3x10 = 3x6 = 2x4 || 5x6 = 3x4 || 9-8-0 17-6-0 22-2-3 29-5-6 37-0-0 9-8-0 7-10-0 4-8-3 7-3-2 2-5-8 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [14:0-4-0,0-3-4] LOADING (psf) SPACING CSI DEFL PLATES in (loc) I/defl L/d GRIP TCLL 20.0 1.25 TC 0.89 -0.44 13-14 Plates Increase Vert(LL) >999 240 MT20 244/190 TCDL 10.0 1.25 BC 0.87 Lumber Increase -1.15 13-14 Vert(TL) >382 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.77 Horz(TL) 0.59 8 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 193 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-1-15 oc purlins, except BOT CHORD 2 X 4 SYP No.2 *Except* end verticals 1-14: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 8-5-15 oc bracing. **WEBS** 2 X 4 SYP No.3 WFBS 1 Row at midpt 7-8, 6-9 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1465/0-5-8, 1=1465/0-5-8 Max Horz 1=197(LC 5) Max Uplift8=-178(LC 6), 1=-144(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

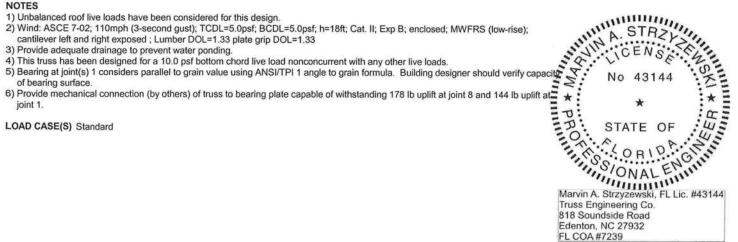
TOP CHORD 1-2=-5236/650, 2-3=-4836/518, 3-4=-3623/352, 4-5=-3608/364, 5-6=-4176/426, 6-7=-564/54, 7-8=-1407/164

BOT CHORD 1-14=-753/4839, 13-14=-544/4181, 12-13=-446/4317, 11-12=-272/2340, 10-11=-278/2321, 9-10=-277/2345, 8-9=-21/61 **WEBS**

2-14=-383/205, 3-14=-20/596, 3-13=-888/228, 4-13=-190/2410, 5-13=-1048/179, 5-12=-1206/175, 6-12=-197/2101,

6-10=0/268, 6-9=-2330/287, 7-9=-84/1275

NOTES



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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



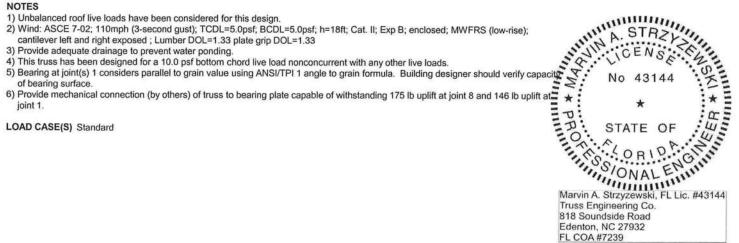
Job Truss Truss Type Qty E5145990 CUNRES A11 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:24 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 12-1-0 17-6-0 23-7-15 29-6-12 37-0-0 6-8-0 5-5-0 5-5-0 6-1-15 5-10-12 Scale = 1:68.0 5x5 = 5.00 12 4 3x4 = 5x5 = 4x12 = 4x5 = 5 6 5x5 = 7-7-9 5-0-12 13 7x8 = 12 11 14 0-4-1 5x8 = 3x6 = 10 2.25 12 4x6 = 3x10 = 2x4 || 8 5x6 = 2x4 || 9-8-0 17-6-0 23-7-15 30-2-4 34-6-8 37-0-0 9-8-0 6-1-15 6-6-4 4-4-4 2-5-8 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [14:0-4-0,0-3-4] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.78 -0.45 Vert(LL) 13 >966 240 244/190 MT20 TCDL 10.0 1.25 BC 0.90 Lumber Increase Vert(TL) -1.18 13-14 >373 180 BCLL 0.0 Rep Stress Incr YES WB 0.97 0.61 Horz(TL) 8 n/a n/a BCDL Code FBC2004/TPI2002 10.0 (Matrix) Weight: 190 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2 *Except* 1-14: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 8-7-8 oc bracing. WEBS 2 X 4 SYP No.3 WEBS 1 Row at midpt 7-8.6-9 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1465/0-5-8, 1=1465/0-5-8 Max Horz 1=185(LC 5) Max Uplift8=-175(LC 6), 1=-146(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-5236/648, 2-3=-4832/516, 3-4=-3637/349, 4-5=-3645/359, 5-6=-4509/464, 6-7=-638/62, 7-8=-1408/167 **BOT CHORD** 1-14=-739/4839, 13-14=-527/4177, 12-13=-489/4675, 11-12=-266/2360, 10-11=-275/2352, 9-10=-275/2363, 8-9=-19/56

WEBS

2-14=-385/206, 3-14=-22/590, 3-13=-880/226, 4-13=-168/2370, 5-13=-1316/229, 5-12=-1377/196, 6-12=-220/2435,

6-10=0/230, 6-9=-2318/281, 7-9=-100/1333

NOTES



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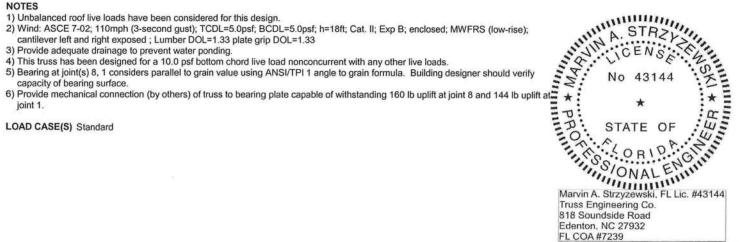
Job Truss Truss Type Qty E5145991 CUNRES A12 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:25 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 12-1-0 17-6-0 25-1-11 30-6-5 35-4-0 6-8-0 5-5-0 5-5-0 7-7-11 5-4-10 4-9-11 Scale: 3/16"=1" 5x5 = 5.00 12 3x4 = 5x5 =4x8 = 2x4 || 5 6 5x5 = 7-7-9 11 7x8 = 10 12 6x6 = 4x6 = 2.25 12 3x6 = 3x10 = 8 4x6 = 9-8-0 17-6-0 25-1-11 35-4-0 9-8-0 7-10-0 7-7-11 10-2-5 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [12:0-3-0,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d PLATES GRIP TC BC TCLL 20.0 Plates Increase 1 25 0.97 Vert(LL) -0.41 11-12 >999 240 MT20 244/190 TCDL 10.0 0.91 Lumber Increase 1.25 Vert(TL) -1.06 11-12 >394 180 BCLL 0.0 Rep Stress Incr YES WB 0.91 Horz(TL) 0.60 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 172 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2 *Except* end verticals. 1-12: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc bracing. WEBS 2 X 4 SYP No.3 WEBS 1 Row at midpt 5-11, 6-8 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1398/0-7-12, 1=1398/0-5-8 Max Horz 1=172(LC 5) Max Uplift8=-160(LC 6), 1=-144(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=-4958/632, 2-3=-4539/499, 3-4=-3349/334, 4-5=-3382/342, 5-6=-4225/417, 6-7=-82/0, 7-8=-139/45 TOP CHORD **BOT CHORD**

1-12=-712/4581, 11-12=-497/3893, 10-11=-447/4406, 9-10=-204/1706, 8-9=-220/1688

2-12=-394/208, 3-12=-24/596, 3-11=-877/221, 4-11=-145/2097, 5-11=-1315/229, 5-10=-1510/236, 6-10=-230/2825, WEBS

6-8=-2170/297

NOTES



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 $WARNING \cdot \textit{Verify design parameters and READ NOTES ON THIS AND INCLUDED \textit{BITEK REFERENCE PAGE MII-7473 BEFORE USE}. \\$

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5145992 CUNRES A13 **ROOF TRUSS** 3 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:26 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 12-1-0 17-6-0 22-11-0 35-0-0 6-8-0 5-5-0 5-5-0 5-5-0 5-5-0 6-8-0 Scale = 1:62.9 4x5 = 4 5.00 12 3x4 = 3x4 > 5 5x5 = 5x5 < 5x8 = 8 10 14 6x6 = 6x6 = 2.25 12 3x10 = 3x10 \\ 3x10 = 1-6-12 9-8-0 17-6-0 25-4-0 35-0-0 7-10-0 7-10-0 9-8-0 Plate Offsets (X,Y): [1:0-0-4,3-6-6], [1:0-1-4,Edge], [1:0-0-8,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [7:0-4-12,Edge], [8:0-3-0,Edge], [10:0-3-0,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL **PLATES** (loc) L/d GRIP TCLL 20.0 Plates Increase 1.25 TC 0.79 Vert(LL) -0.45 >926 240 MT20 244/190 TCDL 10.0 1.25 BC 0.81 Lumber Increase Vert(TL) -1.14 9-10 >362 180 BCLL Rep Stress Incr YES WB 0.68 0.65 Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 167 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-2-2 oc purlins. BOT CHORD 2 X 4 SYP No.2 *Except* **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing 1-10,7-8: 2 X 4 SYP 2400F 2.0E MiTek recommends that Stabilizers and required cross bracing **WEBS** 2 X 4 SYP No.3 be installed during truss erection, in accordance with Stabilizer WEDGE Installation guide. Left: 2 X 10 SYP No.2

REACTIONS (lb/size) 7=1382/0-5-8, 1=1382/0-5-8 Max Horz 1=87(LC 5)

Max Uplift7=-149(LC 6), 1=-149(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD

1-2=-4884/583, 2-3=-4471/450, 3-4=-3253/276, 4-5=-3253/288, 5-6=-4471/380, 6-7=-4884/515

BOT CHORD 1-10=-581/4512, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4512

WEBS 2-10=-393/208, 3-10=-25/595, 3-9=-889/231, 4-9=-120/2113, 5-9=-889/233, 5-8=-28/595, 6-8=-393/212

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 7 and 149 lb uplift at

6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard



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Job Truss Truss Type Qty E5145993 CUNRES ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MITek Industries, Inc. Wed Dec 10 12:09:27 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 12-1-0 17-6-0 22-11-0 35-0-0 6-8-0 5-5-0 5-5-0 5-5-0 5-5-0 6-8-0 Scale = 1:61.7 4x5 = Δ 5.00 12 3x4 = 3x4 > 3 5x5 = 5x5 > 5x8 = 8 10 6x6 = 6x6 = 2.25 12 3x10 = 3x10 =

		9-8-0		7-10-	0		7-10-0)			9-8-0	22	
Plate O	Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [7:0-4-12,Edge], [8:0-3-0,Edge], [10:0-3-0,Edge]												
LOADIN	NG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.45	9	>926	240	MT20	244/190	
TCDL	10.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-1.14	9-10	>362	180	2010/25/200		
BCLL	0.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.65	7	n/a	n/a			

BRACING

TOP CHORD

BOT CHORD

25-4-0

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*

1-10,7-8: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

10.0

REACTIONS (lb/size) 1=1382/0-5-8, 7=1382/0-5-8

Max Horz 1=87(LC 5)

Max Uplift1=-149(LC 5), 7=-149(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

9-8-0

TOP CHORD 1-2=-4884/583, 2-3=-4471/450, 3-4=-3253/276, 4-5=-3253/288, 5-6=-4471/380, 6-7=-4884/515

Code FBC2004/TPI2002

BOT CHORD 1-10=-580/4512, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4512

WEBS 2-10=-393/208, 3-10=-25/595, 3-9=-889/231, 4-9=-120/2113, 5-9=-889/233, 5-8=-28/595, 6-8=-393/212

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 1 and 149 lb uplift at joint 7.

17-6-0

(Matrix)

LOAD CASE(S) Standard



35-0-0

Weight: 153 lb

Structural wood sheathing directly applied or 2-2-2 oc purlins.

MiTek recommends that Stabilizers and required cross bracing

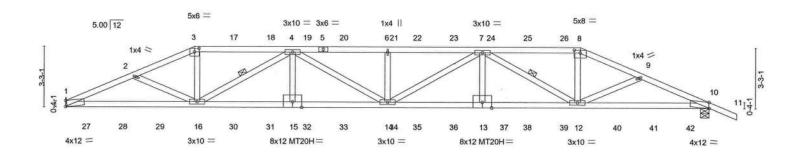
be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

December 10,2008

Job Truss Truss Type Qty E5145994 CUNRES A15 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:29 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 3-9-14 7-0-0 12-3-14 17-6-0 22-8-2 28-0-0 31-2-2 35-0-0 36-6-0 3-9-14 3-2-2 5-3-14 5-2-2 5-2-2 5-3-14



	7-0	0-0	5-3-14		5-2-2	5-2-2			5-3-1	4	7-0-0	
Plate Offse		:Edge,0-3-12], [3:0-3-0,		-0,0-1-13], [7-0-0	
LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.54	14	>766	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-1.40 1	14-15	>297	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.34	10	n/a	n/a		1000100
BCDL	10.0	Code FBC2004/TF	PI2002	(Matr	ix)			4.2.020.0			Weight: 1721	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

7-0-0

3-5: 2 X 4 SYP No.2D, 5-8: 2 X 4 SYP 2400F 2.0E

12-3-14

BOT CHORD 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** WEBS

22-8-2

Structural wood sheathing directly applied or 1-7-10 oc purlins.

35-0-0

Rigid ceiling directly applied or 5-11-13 oc bracing.

1 Row at midpt 4-16, 7-12

28-0-0

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=2685/Mechanical, 10=2863/0-5-8

Max Horz 1=-65(LC 6)

Max Uplift1=-578(LC 5), 10=-667(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6130/1429, 2-3=-6000/1453, 3-17=-5613/1376, 17-18=-5612/1376, 4-18=-5612/1376, 4-19=-8605/2175,

5-19=-8605/2175, 5-20=-8605/2175, 20-21=-8605/2175, 6-21=-8605/2175, 6-22=-8605/2175, 22-23=-8605/2175,

17-6-0

7-23=-8605/2175, 7-24=-5634/1415, 24-25=-5634/1415, 25-26=-5634/1415, 8-26=-5634/1415, 8-9=-6024/1495,

9-10=-6021/1441, 10-11=0/34

BOT CHORD 1-27=-1285/5594, 27-28=-1285/5594, 28-29=-1285/5594, 16-29=-1285/5594, 16-30=-1900/7847, 30-31=-1900/7847

15-31=-1900/7847, 15-32=-1900/7847, 32-33=-1900/7847, 33-34=-1900/7847, 14-34=-1900/7847, 14-35=-1922/7867, 35-36=-1922/7867, 13-36=-1922/7867, 13-37=-1922/7867, 37-38=-1922/7867, 38-39=-1922/7867, 12-39=-1922/7867,

12-40=-1268/5443, 40-41=-1268/5443, 41-42=-1268/5443, 10-42=-1268/5443

2-16=-69/118, 3-16=-276/1624, 4-16=-2640/697, 4-15=0/401, 4-14=-223/926, 6-14=-619/264, 7-14=-201/882,

7-13=0/408, 7-12=-2614/683, 8-12=-249/1558, 9-12=-74/280

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 578 lb uplift at joint 1 and 667 lb uplift at joint 10.

Continued on page 2

A. STRZY
CENS
No 43144

*
D. STATE OF
ONALE

Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co. Truss Engineering Co. 818 Soundside Road

Edenton, NC 27932 FL COA #7239

December 10,2008

Scale = 1:62.3

Job	Truss	Truss Type	Qty	Ply	
CUNRES	A15	ROOF TRUSS	1	1	E5145994
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL.

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NOTES

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 257 lb down and 152 lb up at 7-0-0, 125 lb down and 73 lb up at 9-0-12, 125 lb down and 73 lb up at 11-0-12, 125 lb down and 73 lb up at 13-0-12, 125 lb down and 73 lb up at 15-0-12, 125 lb down and 73 lb up at 17-0-12, 125 lb down and 73 lb up at 19-0-12, 125 lb down and 73 lb up at 21-0-12, 125 lb down and 73 lb up at 23-0-12, 125 lb down and 73 lb up at 25-0-12, and 125 lb down and 73 lb up at 27-0-12, and 297 lb down and 152 lb up at 28-0-0 on top chord, and 98 lb down at 1-0-12, 107 lb down at 3-0-12, 94 lb down at 5-0-12, 94 lb down at 7-0-12, 94 lb down at 9-0-12, 94 lb down at 11-0-12, 94 lb down at 13-0-12, 94 lb down at 13-0-12, 94 lb down at 15-0-12, 94 lb down at 12-0-12, 94 lb down at 13-0-12, down at 25-0-12, 94 lb down at 27-0-12, 94 lb down at 27-11-4, 94 lb down at 29-11-4, and 107 lb down at 31-11-4, and 98 lb down at 33-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

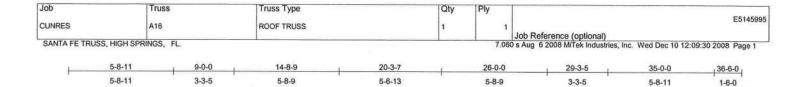
Uniform Loads (plf)

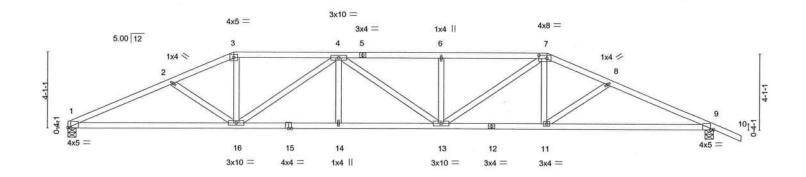
Vert: 1-3=-60, 3-8=-60, 8-11=-60, 1-10=-20

Concentrated Loads (lb)

Vert: 3=-257(B) 8=-257(B) 16=-47(B) 12=-47(B) 17=-125(B) 18=-125(B) 19=-125(B) 20=-125(B) 21=-125(B) 22=-125(B) 23=-125(B) 24=-125(B) 25=-125(B) 26=-125(B) 27=-49(B) 28=-72(B) 29=-47(B) 30=-47(B) 31=-47(B) 32=-47(B) 33=-47(B) 35=-47(B) 35=-47(B) 36=-47(B) 36=-47(B) 38=-47(B) 39=-47(B) 30=-47(B) 30=-4 41=-72(B) 42=-49(B)







-		9-0-0	1 1	4-8-9		20-3-7	26-0-0	3		35-0-0	
	9-0-0			5-8-9		5-6-13	5-8-9		9-0-0		1
Plate Of	fsets (X,Y):	[1:0-1-14,0-0-2], [7:0-5-4,	0-2-0], [9:0-1	-14,0-0-2]							
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.22 13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.58 13-14	>720	180	10.0000000	.075.0000.000.00
BCLL	0.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.17 9	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	rix)			10,000		Weight: 170	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-2-3 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 1=1379/0-5-8, 9=1488/0-5-8

Max Horz 1=-75(LC 6)

Max Uplift1=-135(LC 4), 9=-190(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2928/347, 2-3=-2676/327, 3-4=-2462/317, 4-5=-3094/430, 5-6=-3094/430, 6-7=-3095/430, 7-8=-2660/327,

8-9=-2914/346, 9-10=0/34

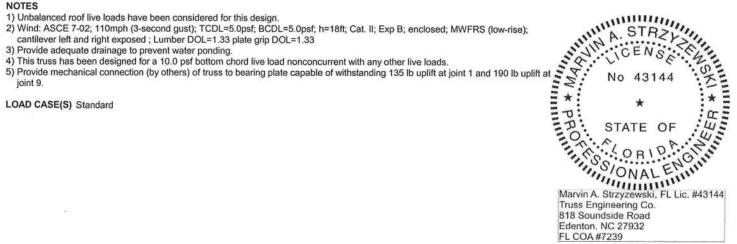
BOT CHORD 1-16=-264/2640, 15-16=-330/3097, 14-15=-330/3097, 13-14=-330/3097, 12-13=-214/2431, 11-12=-214/2431,

2-16=-252/133, 3-16=-34/746, 4-16=-856/138, 4-14=0/193, 4-13=-78/73, 6-13=-344/126, 7-13=-144/879, 7-11=0/380,

8-11=-233/117

NOTES

WEBS



December 10,2008

Scale = 1:62.2

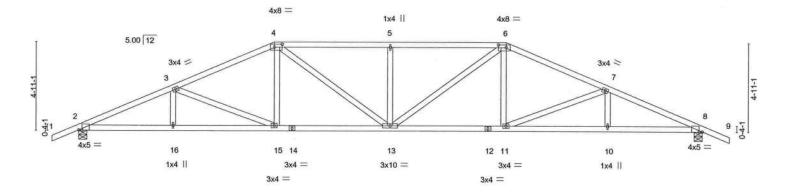


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED ALL INCLUDED AND INCLUDED Design Valid for use drifty with without Continue Costs. This design is obsess only upon parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty Ply E5145996 CUNRES A17 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:31 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL





	1	5-3-10 1	1-0-0	1	17-6-0		24-0-0	-	29-8-6	35-0	0-0
		5-3-10 5-8-6		6-6-0		6-6-0		2.7	5-8-6	5-3-	-10
Plate Of	fsets (X,Y):	[2:0-2-2,0-0-2], [4:0-5-4,0	-2-0], [6:0-5-	4,0-2-0], [8:0-	2-2,0-0-2]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.18 13	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.46 11-13	>909	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.16 8	n/a	n/a		
BCDL	10.0	Code FBC2004/T	PI2002	(Matrix	()					Weight: 177	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-5-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 2=1485/0-5-8, 8=1485/0-5-8

Max Horz 2=-76(LC 6) Max Uplift2=-181(LC 5), 8=-181(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=0/34, 2-3=-2990/277, 3-4=-2494/289, 4-5=-2613/341, 5-6=-2613/341, 6-7=-2494/289, 7-8=-2990/276, 8-9=0/34 2-16=-194/2680, 15-16=-194/2680, 14-15=-160/2256, 13-14=-160/2256, 12-13=-165/2256, 11-12=-165/2256,

10-11=-207/2680, 8-10=-207/2680 **WEBS**

3-16=0/217, 3-15=-473/114, 4-15=0/385, 4-13=-74/572, 5-13=-401/148, 6-13=-74/572, 6-11=0/385, 7-11=-473/115,

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 181 lb uplift at joint 8.

LOAD CASE(S) Standard



December 10,2008



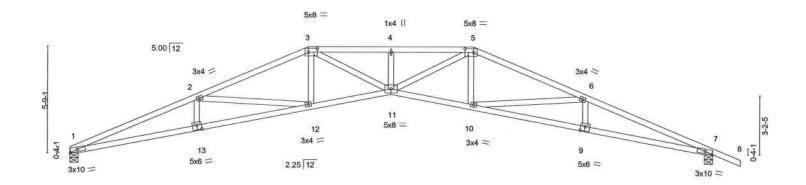


Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Design Valid for use only with writers continectors. This design is added only upon parameters snown, and is for an inanimal obliging component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consort. AMSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty E5145997 CUNRES A18 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:32 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-11-0 13-0-0 17-6-0 22-0-0 28-1-0 35-0-0 36-6-0 6-11-0 6-1-0 4-6-0 4-6-0 6-1-0 6-11-0 1-6-0



-	- 6	-11-0	13-0-0		17-6-0	22-0-0	-		28-1-0)	35-0-0	
	6	-11-0	6-1-0		4-6-0	4-6-0			6-1-0		6-11-0	
Plate O	ffsets (X,Y):	[1:0-4-12,Edge], [3:0-5-1]	2,0-2-8], [5:0-	5-12,0-2-8],	[7:0-4-12,Ed	ige], [9:0-3-0,0-3	-0], [13:	0-3-0,0	-3-0]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.56	11	>740	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.40	11	>296	180	19791(0)/407525	
BCLL	0.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.79	7	n/a	n/a		

BRACING

TOP CHORD

BOT CHORD

Installation guide.

Weight: 158 lb

Structural wood sheathing directly applied or 2-2-0 oc purlins.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 2-2-0 oc bracing.

BCDL LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except* 1-13,7-9: 2 X 4 SYP No.2D

WEBS

10.0

2 X 4 SYP No.3

REACTIONS (lb/size) 1=1379/0-5-8, 7=1488/0-5-8

Max Horz 1=-93(LC 6)

Max Uplift1=-123(LC 5), 7=-195(LC 6)

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4955/415, 2-3=-4011/339, 3-4=-5220/450, 4-5=-5220/450, 5-6=-4005/342, 6-7=-4909/369, 7-8=0/32 **BOT CHORD**

1-13=-392/4552, 12-13=-391/4555, 11-12=-197/3719, 10-11=-200/3714, 9-10=-290/4509, 7-9=-291/4504 WEBS

2-13=0/264, 2-12=-860/239, 3-12=-11/412, 3-11=-134/1751, 4-11=-210/88, 5-11=-131/1757, 5-10=-6/409,

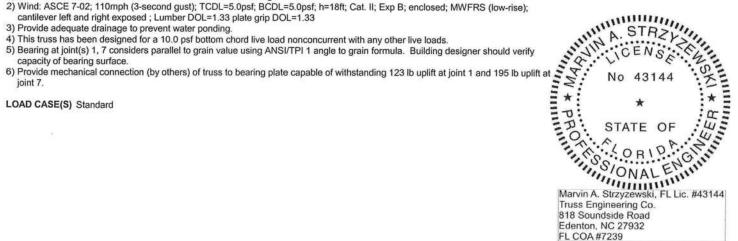
(Matrix)

6-10=-820/215, 6-9=0/260

NOTES

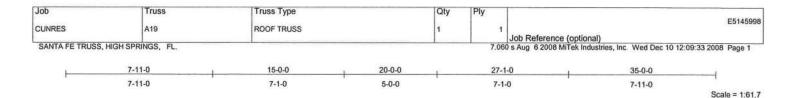
1) Unbalanced roof live loads have been considered for this design.

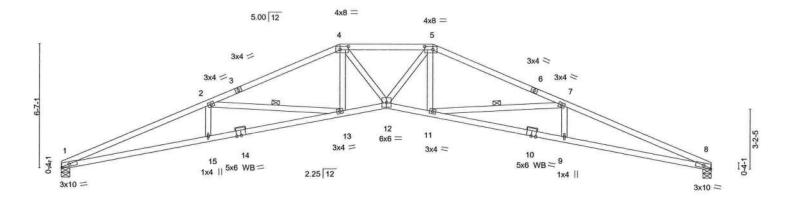
2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise);



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Scale = 1:62.5





1		7-11-0	7-	1-0	2-6-0	2-6-0	7	-1-0	,	7-11-0	1		
Plate Offsets (X,Y): [1:0-4-12,Edge], [4:0-5-4,0-2-0], [5:0-5-4,0-2-0], [8:0-4-12,Edge]													
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.45 12	>925	240	MT20	244/190		
TCDL	10.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-1.12 13-15	>369	180	0000 PGTV EV			
BCLL	0.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.67 8	n/a	n/a				
BCDL	10.0	Code FBC2004/TF	PI2002	(Matr	ix)					Weight: 155	lb		

20-0-0

BRACING

WEBS

TOP CHORD

BOT CHORD

27-1-0

1 Row at midpt

Installation guide.

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 9-5-3 oc bracing.

2-13, 7-11

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

17-6-0

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

1-3,6-8: 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2 *Except*

1-14,8-10: 2 X 4 SYP 2400F 2.0E

7-11-0

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1382/0-5-8, 8=1382/0-5-8

Max Horz 1=-75(LC 6)

Max Uplift1=-135(LC 5), 8=-135(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4936/469, 2-3=-3701/244, 3-4=-3615/269, 4-5=-3834/281, 5-6=-3615/260, 6-7=-3701/235, 7-8=-4936/410

15-0-0

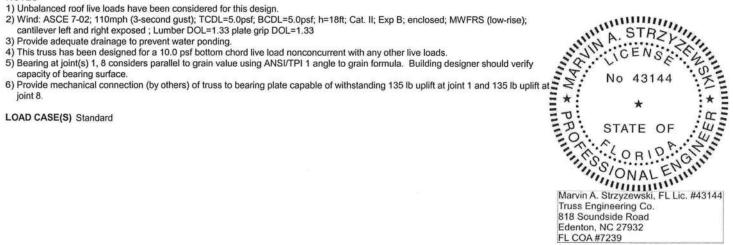
BOT CHORD 1-15=-453/4538, 14-15=-453/4514, 13-14=-446/4534, 12-13=-159/3406, 11-12=-118/3406, 10-11=-315/4534.

9-10=-322/4514, 8-9=-322/4538

2-15=0/320, 2-13=-1134/288, 4-13=-13/494, 4-12=-61/785, 5-12=-92/785, 5-11=-13/494, 7-11=-1134/292, 7-9=0/320

WEBS NOTES

1) Unbalanced roof live loads have been considered for this design.



35-0-0

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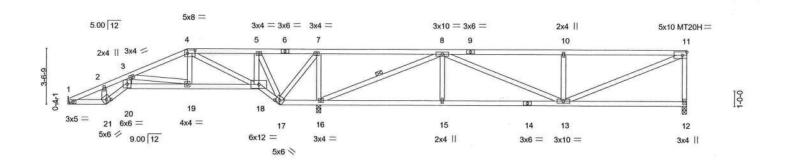
WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

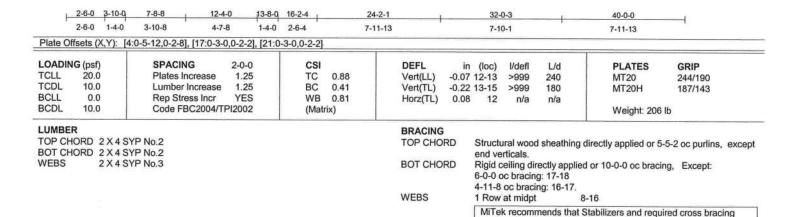
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Design Valid to see Only will be will write commercios. This design is obsequently applicability of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719,



			Ply	Qty		Truss Type		E.	Truss		
E5145999			1	1		ROOF TRUSS			A2		
0 12:09:34 2008 Page 1	(optional) iTek Industries, Inc. Wed Dec 10 1	Job Reference (opt s Aug 6 2008 MiTek						FL.	PRINGS, F	S, HIGH SI	E TRUSS
	40-0-0	3	32-0-		24-2-1	16-2-4	12-4-0		7-8-8	3-10-0	2-6-0
	7-11-13	1	7-10-		7-11-13	3-10-4	4-7-8		3-10-8	1-4-0	2-6-0





be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 1=327/Mechanical, 12=730/0-3-8, 16=2127/0-3-8

Max Horz 1=104(LC 5) Max Uplift1=-31(LC 5), 12=-117(LC 3), 16=-300(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-803/99, 2-3=-804/123, 3-4=-168/15, 4-5=-104/850, 5-6=-104/923, 6-7=-104/923, 7-8=-178/1454, 8-9=-1123/185, TOP CHORD

9-10=-1123/185, 10-11=-1123/185, 11-12=-647/151

BOT CHORD 1-21=-176/719, 20-21=-178/748, 19-20=-159/616, 18-19=-36/160, 17-18=-1030/147, 16-17=-1455/178, 15-16=-125/618,

14-15=-125/618, 13-14=-125/618, 12-13=-17/108

2-21=-226/71, 3-20=-33/330, 3-19=-477/124, 4-19=0/280, 4-18=-1121/155, 5-18=-26/92, 5-17=-173/17, 7-17=-121/864,

7-16=-1091/243, 8-16=-2238/325, 8-15=0/316, 8-13=-66/547, 10-13=-479/179, 11-13=-183/1099

NOTES

WEBS

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

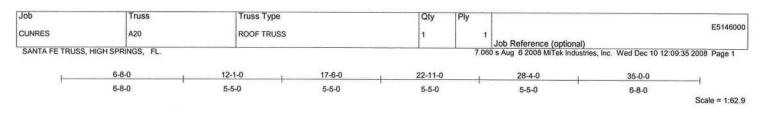
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 117 lb uplift at joint 12 and 300 lb uplift at joint 16.

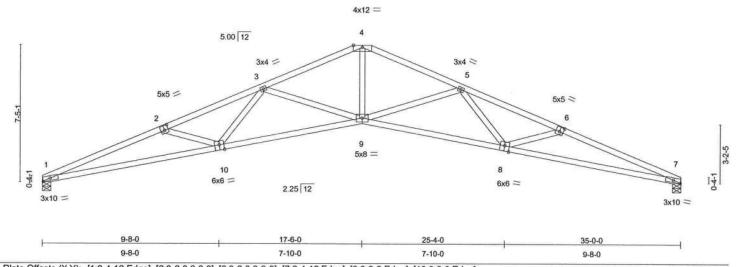
LOAD CASE(S) Standard



FL COA #7239

December 10,2008





LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.45	9	>926	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-1.14	9-10	>362	180	111111111111111111111111111111111111111	-111155
BCLL	0.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.65	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	rix)				0.00.000	1.0.00	Weight: 153 lt	1

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except* 1-10,7-8: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1382/0-5-8, 7=1382/0-5-8

Max Horz 1=87(LC 5)

Max Uplift1=-149(LC 5), 7=-149(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4884/583, 2-3=-4471/450, 3-4=-3253/276, 4-5=-3253/288, 5-6=-4471/380, 6-7=-4884/515

BOT CHORD 1-10=-580/4512, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4512

WEBS 2-10=-393/208, 3-10=-25/595, 3-9=-889/231, 4-9=-120/2113, 5-9=-889/233, 5-8=-28/595, 6-8=-393/212

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 1 and 149 lb uplift at joint 7.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-2-2 oc purlins.

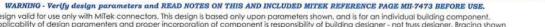
MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

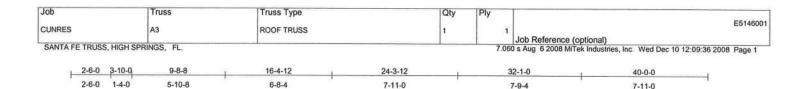
Installation guide.

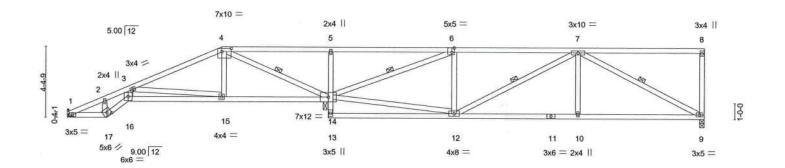
December 10,2008



Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fruss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing in building construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, VI 53719.







	7	10 1 0	10 11 16	21-0-12	JZ-1-0		40-0-0	
2-6-0 1-4-	0 5-10-8	6-4-8	0-3-12	7-11-0	7-9-4		7-11-0	,
Plate Offsets (X,Y):	[4:0-7-0,0-2-4], [6:0-1-12,	0-3-4], [17:0-3-	0,0-2-2]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.77	Vert(LL)	-0.13 15-16 >999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.35 15-16 >558	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.72	Horz(TL)	0.13 9 n/a	n/a		
BCDL 10.0	Code FBC2004/T	PI2002	(Matrix)				Weight: 216	lb
I A CONTRACTOR OF THE CONTRACT								

24-3-12

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* 5-13: 2 X 4 SYP No.3

2-6-0 3-10-0

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 14-15,12-13.

end verticals.

32-1-0

WEBS 1 Row at midpt 4-14, 6-14, 7-12, 7-9

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

40 0 0

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=337/Mechanical, 9=712/0-3-8, 14=2121/0-3-0

9-8-8

Max Horz 1=131(LC 5)

Max Uplift1=-18(LC 5), 9=-116(LC 3), 14=-291(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=-903/86, 2-3=-900/104, 3-4=-77/112, 4-5=-249/1550, 5-6=-234/1533, 6-7=-429/95, 7-8=-77/11, 8-9=-217/78 1-17=-189/806, 16-17=-182/825, 15-16=-181/701, 14-15=-11/21, 13-14=0/152, 5-14=-443/164, 12-13=-3/54,

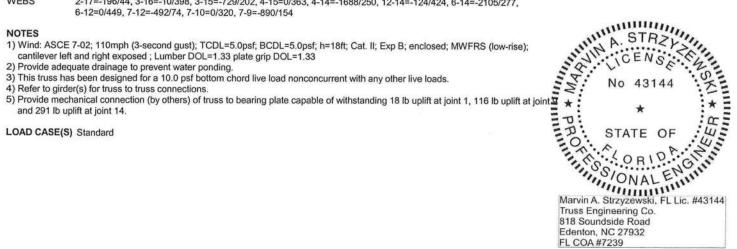
11-12=-148/864, 10-11=-148/864, 9-10=-148/864

WEBS 2-17=-196/44, 3-16=-10/398, 3-15=-729/202, 4-15=0/363, 4-14=-1688/250, 12-14=-124/424, 6-14=-2105/277,

16-1-0

16-4-12

6-12=0/449, 7-12=-492/74, 7-10=0/320, 7-9=-890/154



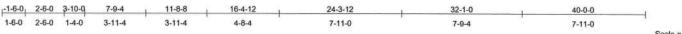
December 10,2008

Scale = 1:72.0

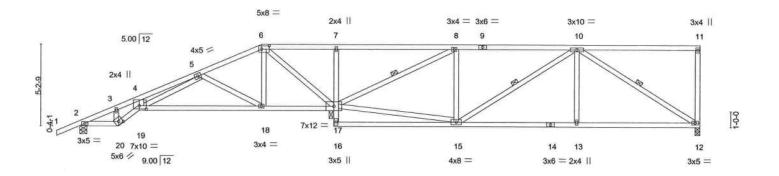
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

818 Soundside Road Edenton, NC 27932





Scale = 1:74.1



	2-6-0	3-10-0	11-8-8		16-1-0	16-4-12	24-3-12		32-1-0		40-0-0	
	2-6-0	1-4-0	7-10-8	•	4-4-8	0-3-12	7-11-0		7-9-4		7-11-0	
Plate Offse	ets (X,Y):	[6:0-5-	12,0-2-8], [19:0-5-6	3,0-2-8], [20	:0-3-0,0-2-	2]						
LOADING	(psf)		SPACING	2-0-0	CS	il .	DEFL	in (loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TO	0.73	Vert(LL)	-0.13 18-19	>999	240	MT20	244/190
TCDL	10.0		Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.38 18-19	>515	180	2534700	
BCLL	0.0		Rep Stress Incr	YES	W	3 0.82	Horz(TL)	0.09 12	n/a	n/a		
BCDL	10.0		Code FBC2004/T	PI2002	(M	atrix)	1,555,578,578				Weight: 230	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except*

7-16: 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 18-19,17-18.

end verticals.

1 Row at midpt 8-17, 10-15, 10-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

REACTIONS (lb/size) 12=683/0-3-8, 2=405/0-5-8, 17=2187/0-3-0

Max Horz 2=188(LC 5)

Max Uplift12=-123(LC 3), 2=-94(LC 5), 17=-274(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

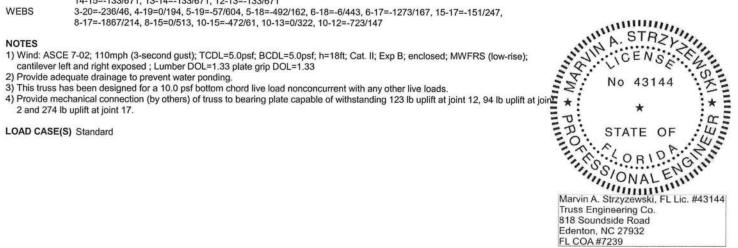
1-2=0/34, 2-3=-637/1, 3-4=-643/19, 4-5=-555/63, 5-6=-97/530, 6-7=-186/1425, 7-8=-178/1395, 8-9=-272/103, TOP CHORD

9-10=-272/103, 10-11=-60/9, 11-12=-216/77

BOT CHORD 2-20=-132/559, 19-20=-121/608, 18-19=-123/0, 17-18=-437/57, 16-17=0/151, 7-17=-412/148, 15-16=0/128, 14-15=-133/671, 13-14=-133/671, 12-13=-133/671

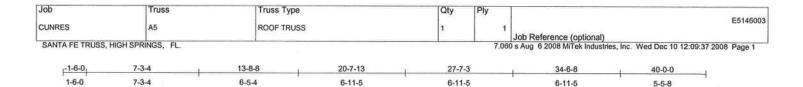
3-20=-236/46, 4-19=0/194, 5-19=-57/604, 5-18=-492/162, 6-18=-6/443, 6-17=-1273/167, 15-17=-151/247,

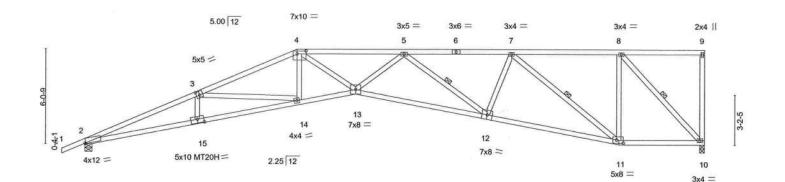
WEBS



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	-	7-3-4	13-8-8	17-1	5-0	26-0-4		-		34-6-8	40-0	-0
		7-3-4	6-5-4	3-9	-8	8-6-4		,		8-6-4	5-5-	8
Plate Of	fsets (X,Y): [2	2:0-0-11,Edge], [3:0-2-8,	0-3-0], [4:0-7	-0,0-2-4], [1	1:0-5-4,0-2-	8], [15:0-5-0,0-3-0)]					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.57	13	>836	240	MT20	244/190
CDL	10.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-1.48	12-13	>321	180	MT20H	187/143
3CLL	0.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.73	10	n/a	n/a	20002000000	
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	ix)	100000000000000000000000000000000000000				200000	Weight: 209	lb

BRACING

WEBS

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 1-3: 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2 *Except*

13-15,12-13: 2 X 4 SYP No.2D, 2-15: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 10=1583/0-3-8, 2=1691/0-5-8

Max Horz 2=214(LC 5)

Max Uplift10=-231(LC 4), 2=-196(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

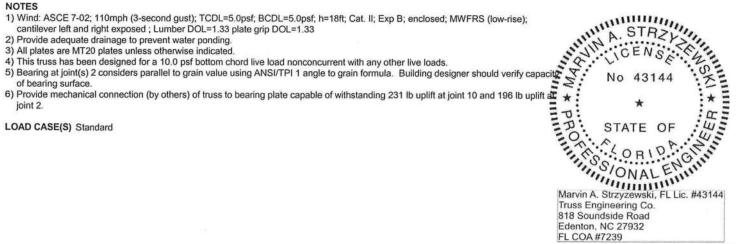
TOP CHORD 1-2=0/32, 2-3=-5757/626, 3-4=-4805/624, 4-5=-5968/794, 5-6=-3458/472, 6-7=-3458/472, 7-8=-1317/188, 8-9=-18/4,

9-10=-131/47

BOT CHORD 2-15=-678/5293, 14-15=-681/5290, 13-14=-609/4466, 12-13=-754/5092, 11-12=-464/3133, 10-11=-188/1317 WEBS

3-15=0/288, 3-14=-820/215, 4-14=-22/370, 4-13=-238/1909, 5-13=-82/1254, 5-12=-1947/332, 7-12=-65/1096, 7-11=-2268/338, 8-11=-55/1015, 8-10=-1915/273

NOTES



Structural wood sheathing directly applied, except end verticals.

5-12, 7-11, 8-10

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

Installation guide.

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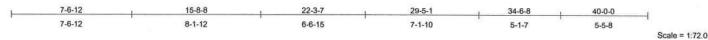
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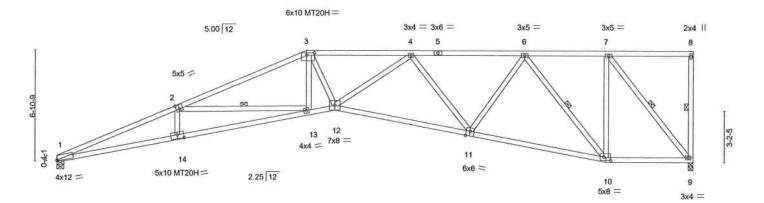
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component,

Design Valid on the Only with Miles Continectors. This design is acided only upon parameters anown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer. For an individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult MSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5146004 CUNRES A6 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:38 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL





H	7-0	-12	15-8-8	17-1	6-0	26-0-4			34-6-8	40-0-	0
	7-6	-12	8-1-12	1-9)-8	8-6-4			8-6-4	5-5-8	· '
Plate Of	ffsets (X,Y): [1:0-0-11,Edge], [2:0-2-8,	0-3-4], [3:0-6	-0,0-1-12], [1	0:0-5-4,0-2	-8], [11:0-3-0,Ed	ge], [14:0-5-0,0)-3-0]			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.44 13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.21 13-14	>393	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.60 9	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matrix	()				1000	Weight: 215	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

2-3: 2 X 4 SYP No.2D, 1-2: 2 X 4 SYP 2400F 2.0E

BOT CHORD 2 X 4 SYP No.2 *Except*

12-14: 2 X 4 SYP No.2D, 1-14: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8

Max Horz 1=213(LC 5)

Max Uplift9=-228(LC 4), 1=-135(LC 5)

BRACING

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt 8-9, 2-13, 6-10, 7-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5844/598, 2-3=-4539/562, 3-4=-4573/595, 4-5=-2857/375, 5-6=-2857/375, 6-7=-1140/158, 7-8=-19/4, 8-9=-149/54

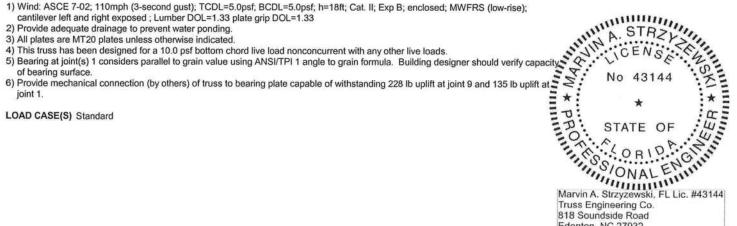
BOT CHORD 1-14=-703/5386, 13-14=-708/5380, 12-13=-554/4188, 11-12=-543/3722, 10-11=-337/2243, 9-10=-158/1140

WEBS 2-14=0/331, 2-13=-1169/312, 3-13=-27/422, 3-12=-111/969, 4-12=-148/1138, 4-11=-1320/252, 6-11=-91/1180,

6-10=-1709/270, 7-10=-75/1079, 7-9=-1786/245

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise);



Edenton, NC 27932 FL COA #7239

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Job Truss Truss Type Qty E5146005 CUNRES ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:39 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 28-3-7 34-6-8 40-0-0 6-8-0 5-3-8 5-3-8 5-0-11 5-11-10 6-3-1 5-5-8 Scale = 1:72.0 5x5 = 3x4 = 5x5 = 2x4 || 4x5 = 5.00 12 5 6 7 8 3x4 = 5x5 = 12 3-2-5 7x8 = 13 11 5x10 MT20H= 6x6 = 2.25 12 4x12 = 10 5x8 = 2x4 || 9-8-0 17-6-0 25-3-9 34-6-8 40-0-0 7-10-0 7-9-9 9-2-15 5-5-8 Plate Offsets (X,Y): [1:0-0-11,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [11:0-3-0,Edge], [13:0-5-0,0-3-0] LOADING (psf) SPACING 2-0-0 DEFL in (loc) **PLATES** I/defl L/d GRIP TCLL 20.0 1.25 TC Plates Increase 0.88 Vert(LL) -0.44 12-13 >999 240 MT20 244/190 TCDL 10.0 -1.14 12-13 Lumber Increase 1.25 BC 0.92 Vert(TL) >416 180 187/143 MT20H BCLL 0.0 Rep Stress Incr YES WB 0.75 0.56 Horz(TL) 9 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 221 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD 2 X 4 SYP No.2 *Except* **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc bracing. 1-13: 2 X 4 SYP 2400F 2.0E WEBS 1 Row at midpt 8-9, 6-10 **WEBS** 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8 Max Horz 1=233(LC 5)

Max Uplift9=-226(LC 4), 1=-142(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-5737/669, 2-3=-5357/540, 3-4=-4171/513, 4-5=-3871/497, 5-6=-2663/346, 6-7=-1048/143, 7-8=-1048/143,

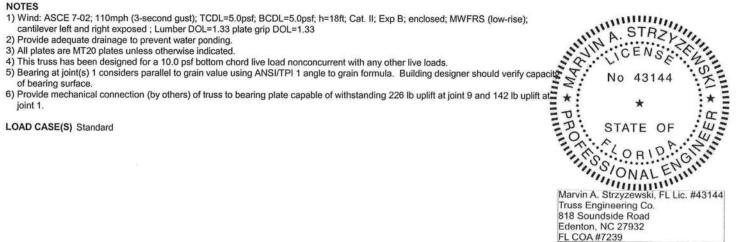
8-9=-1545/239

BOT CHORD 1-13=-808/5304, 12-13=-605/4699, 11-12=-453/3225, 10-11=-327/2218, 9-10=-4/14

2-13=-325/202, 3-13=-20/562, 3-12=-855/229, 4-12=-103/1270, 5-12=-160/931, 5-11=-1042/194, 6-11=-67/1040, **WEBS**

6-10=-1704/260, 7-10=-369/137, 8-10=-236/1747

NOTES



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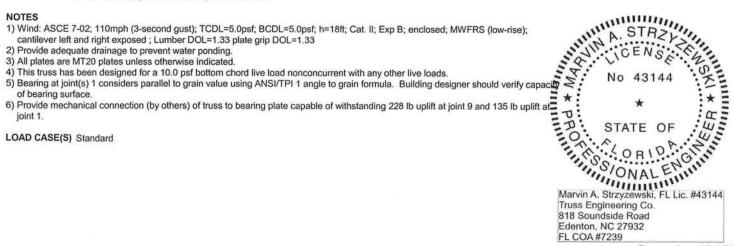
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component.

Design Valid up see they will write a commercial the assign is assess only upon parameters and will all a for an individual building component. Applicability of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5146006 CUNRES ROOF TRUSS **A8** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:40 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 8-3-10 15-9-5 22-7-8 29-5-8 40-0-0 8-3-10 7-5-11 6-10-3 6-10-0 5-1-0 5-5-8 Scale = 1:72.0 3x4 = 7x10 = 5.00 12 3x6 = 3x5 = 3x5 = 2x4 || 3 5 5x5 = 12 3-2-5 13 7x8 = 3x4 = 14 11 15 5x10 MT20H WB = 6x6 = 2.25 12 4x12 = 2x4 || 10 5x8 = 3x4 = 8-3-10 15-9-5 17-6-0 26-0-4 34-6-8 40-0-0 8-3-10 7-5-11 1-8-11 8-6-4 8-6-4 5-5-8 Plate Offsets (X,Y): [1:0-0-11,Edge], [2:0-2-8,0-3-4], [3:0-7-0,0-2-4], [10:0-5-4,0-2-8], [11:0-3-0,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP 20.0 TCLL Plates Increase 1.25 TC 0.83 Vert(LL) -0.44 >999 13 240 MT20 244/190 TCDL 10.0 BC -1.17 13-15 Lumber Increase 1.25 0.99 Vert(TL) >405 180 MT20H 187/143 BCLL 0.0 Rep Stress Incr YES WB 0.85 Horz(TL) 0.60 9 n/a n/a BCDL Code FBC2004/TPI2002 10.0 (Matrix) Weight: 215 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 *Except* TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 1-2: 2 X 4 SYP 2400F 2.0E end verticals BOT CHORD 2 X 4 SYP No.2 *Except* **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc bracing 1-14: 2 X 4 SYP 2400F 2.0E WEBS 1 Row at midpt 8-9, 2-13, 6-10, 7-9 **WEBS** 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8 Max Horz 1=214(LC 5) Max Uplift9=-228(LC 4), 1=-135(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-5776/594, 2-3=-4500/562, 3-4=-4527/587, 4-5=-2846/375, 5-6=-2846/375, 6-7=-1136/158, 7-8=-18/4, 8-9=-148/53 **BOT CHORD** 1-15=-683/5319, 14-15=-686/5280, 13-14=-679/5305, 12-13=-551/4161, 11-12=-530/3635, 10-11=-332/2220, 9-10=-158/1136 **WEBS** 2-15=0/343, 2-13=-1132/296, 3-13=-45/405, 3-12=-100/973, 4-12=-149/1164, 4-11=-1277/248, 6-11=-97/1197,

6-10=-1690/266, 7-10=-73/1075, 7-9=-1785/245



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Design Valid to see only winter Continuations. This design is added only upon parameters anown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer. For thus designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consort. ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty Ply E5146007 CUNRES A9 **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:41 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 12-1-0 17-6-0 20-8-7 28-10-14 37-0-0 6-8-0 5-5-0 5-5-0 3-2-7 8-2-7 8-1-2 Scale = 1:68.0 5x5 = 5.00 12 4 5x5 = 3x6 = 4x12 =4x5 = 5 6 8 3x4 = 5x5 = 14 7x8 = 13 4x5 = 15 12 5x8 = 3x6 = 2x4 || 2.25 12 3x10 = 5x6 2x4 11 9-8-0 17-6-0 20-8-7 28-8-8 34-6-8 37-0-0 9-8-0 3-2-7 8-0-0 5-10-0 2-5-8 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [15:0-4-0,0-3-4] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.86 Vert(LL) -0.43 14-15 >999 240 MT20 244/190 TCDL 10.0 1.25 BC 0.88 Lumber Increase Vert(TL) -1.13 14-15 >387 180 BCLL 0.0 Rep Stress Incr YES WB 0.80 Horz(TL) 0.56 9 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 198 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied, except end verticals. 2 X 4 SYP No.2 *Except* BOT CHORD **BOT CHORD** Rigid ceiling directly applied or 8-4-7 oc bracing. 1-15: 2 X 4 SYP 2400F 2.0E **WEBS** 1 Row at midpt 8-9, 7-10 **WEBS** 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 9=1465/0-5-8, 1=1465/0-5-8 Max Horz 1=210(LC 5)

Max Uplift9=-186(LC 4), 1=-142(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5235/650, 2-3=-4839/520, 3-4=-3611/370, 4-5=-3590/383, 5-6=-3907/422, 6-7=-3907/422, 7-8=-504/54,

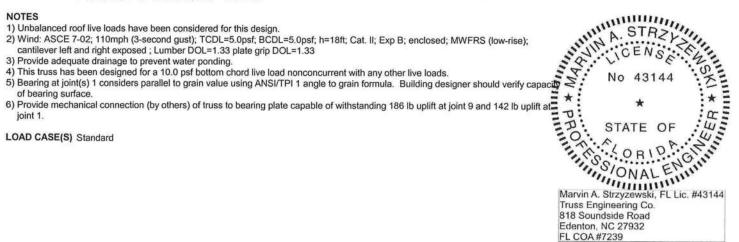
8-9=-1404/179

BOT CHORD 1-15=-767/4838, 14-15=-559/4184, 13-14=-440/4029, 12-13=-245/1992, 11-12=-252/1965, 10-11=-249/1992,

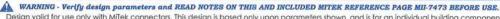
9-10=-15/44

WEBS 2-15=-382/205, 3-15=-18/602, 3-14=-893/229, 4-14=-243/2492, 5-14=-922/156, 5-13=-1107/187, 7-13=-221/2133,

7-11=0/312, 7-10=-2105/273, 8-10=-108/1278

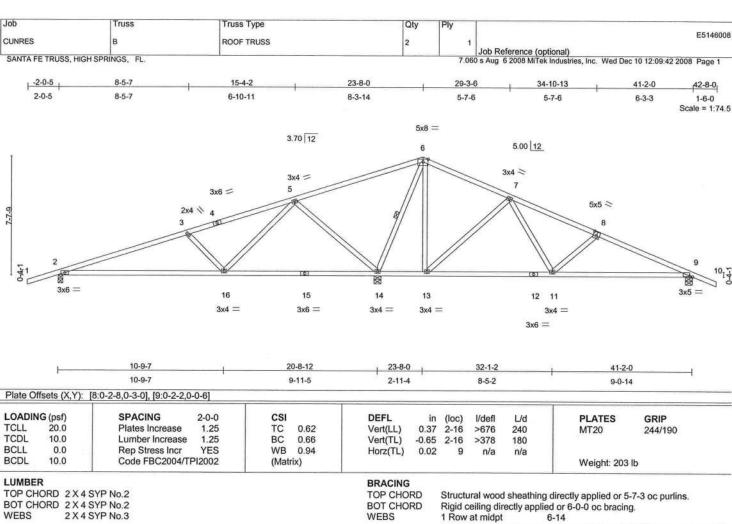


December 10,2008



Design valid for use only with MTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=651/0-3-8, 14=2246/0-5-8, 9=601/0-5-8

Max Horz 2=-108(LC 6)

Max Uplift2=-366(LC 5), 14=-650(LC 3), 9=-110(LC 6) Max Grav 2=701(LC 7), 14=2246(LC 1), 9=698(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-932/429, 3-4=-565/339, 4-5=-453/357, 5-6=-286/1122, 6-7=-194/524, 7-8=-670/54, 8-9=-966/37,

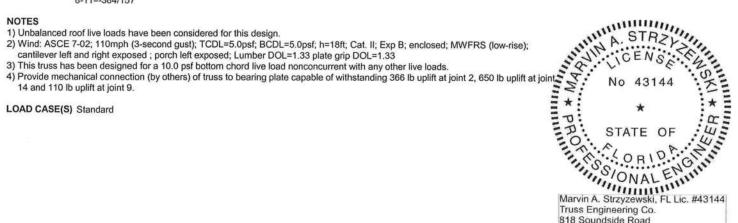
9-10=0/34

BOT CHORD **WEBS**

2-16=-437/823, 15-16=-315/159, 14-15=-315/159, 13-14=-423/310, 12-13=-208/304, 11-12=-208/304, 9-11=0/827

3-16=-506/212, 5-16=-452/839, 5-14=-999/475, 6-14=-1560/256, 6-13=-15/541, 7-13=-686/171, 7-11=-19/562,

8-11=-384/157



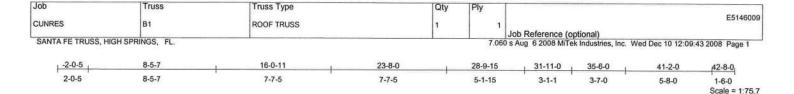
818 Soundside Road Edenton, NC 27932 FL COA #7239

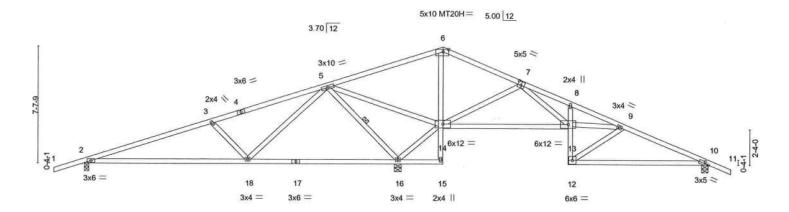
December 10,2008

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LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.37	2-18	>670	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.65	2-18	>379	180	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.04	10	n/a	n/a	3713,000,711	
BCDL 10.0	Code FBC2004/T	PI2002	(Matr	rix)				(A.M.)		Weight: 215	b

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except* 6-15,8-12: 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

23-8-0

2-11-4

TOP CHORD **BOT CHORD**

WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 12-13,10-12.

1 Row at midpt 5-16

31-11-0

8-3-0

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Structural wood sheathing directly applied or 5-6-0 oc purlins.

41-2-0

9-3-0

REACTIONS (lb/size) 2=590/0-3-8, 10=539/0-5-8, 16=2368/0-5-8

10-9-7

10-9-7

Max Horz 2=-108(LC 6)

Max Uplift2=-415(LC 5), 10=-137(LC 6), 16=-560(LC 3) Max Grav 2=713(LC 7), 10=594(LC 8), 16=2368(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-980/596, 3-4=-599/501, 4-5=-478/521, 5-6=-170/977, 6-7=-174/986, 7-8=-876/39, 8-9=-819/128,

9-10=-731/106, 10-11=0/34

BOT CHORD 2-18=-596/871, 17-18=-677/104, 16-17=-677/104, 15-16=-114/2, 14-15=-69/54, 6-14=-1029/238, 13-14=-370/277,

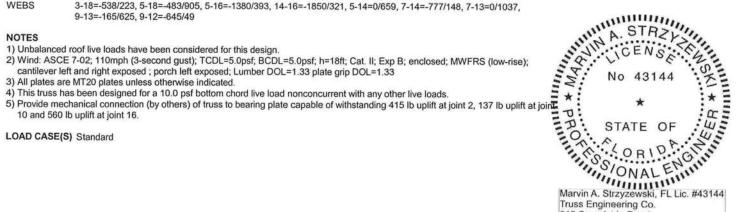
12-13=0/449, 8-13=-175/55, 10-12=-14/619

3-18=-538/223, 5-18=-483/905, 5-16=-1380/393, 14-16=-1850/321, 5-14=0/659, 7-14=-777/148, 7-13=0/1037,

20-8-12

9-11-5

WEBS



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December 10,2008

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Job Truss Truss Type Qty Ply E5146010 CUNRES B2 **ROOF TRUSS** Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:44 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -2-0-5 7-5-7 14-0-11 20-11-8 2-0-5 7-5-7 6-7-5 6-10-13 3x4 || Scale = 1:43.3 5 3.70 12 3x4 = 4 5x5 = X 3 3x6 = 8 7 3x4 = 3x5 = 3x6 = 10-9-1 20-11-8 10-9-1 10-2-7 Plate Offsets (X,Y): [2:0-1-1,0-0-3], [3:0-2-8,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d PLATES GRIP TCLL 20.0 Plates Increase 1.25 TC 0.70 Vert(LL) 0.35 2-8 >708 240 MT20 244/190 TCDL 10.0 1.25 BC Lumber Increase 0.79 Vert(TL) -0 64 >389 180 2-8 BCLL 0.0 Rep Stress Incr YES WB 0.33 Horz(TL) 0.05 6 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 102 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-1-2 oc purlins, except BOT CHORD 2 X 4 SYP No.2 end verticals. 2 X 4 SYP No.3 **BOT CHORD** WEBS Rigid ceiling directly applied or 6-7-6 oc bracing. **WEBS** 1 Row at midpt 5-6. 4-6 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 6=820/0-5-8, 2=964/0-3-8 Max Horz 2=237(LC 3) Max Uplift6=-411(LC 3), 2=-445(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-1859/716, 3-4=-1409/600, 4-5=-106/14, 5-6=-177/74

BOT CHORD 2-8=-844/1714, 7-8=-439/892, 6-7=-439/892

WEBS 3-8=-516/221, 4-8=-372/727, 4-6=-1014/495

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 6 and 445 lb uplift at joint 2.

LOAD CASE(S) Standard



December 10,2008

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Job Truss Truss Type Qty E5146011 CUNRES **B3 ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:44 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -2-0-5 7-11-5 15-4-0 2-0-5 7-11-5 7-4-11 3x4 || Scale = 1:32.4 3.70 12 5 3x4 = 7 1x4 || 3x5 = 7-11-5 15-4-0 7-11-5 7-4-11 LOADING (psf) SPACING CSI DEFL (loc) 2-0-0 I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.58 Vert(LL) 0.14 2-7 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.44 Vert(TL) -0.252-7 >731 180 BCLL 0.0 Rep Stress Incr YES WB 0.32 Horz(TL) 0.03 6 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 71 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 5-2-13 oc purlins, except BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS **BOT CHORD** Rigid ceiling directly applied or 8-6-10 oc bracing. **WEBS** 1 Row at midpt MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS (lb/size) 6=592/0-5-8, 2=742/0-3-8

Max Horz 2=181(LC 3)

Max Uplift6=-296(LC 3), 2=-351(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-1174/443, 3-4=-123/0, 4-5=-52/5, 5-6=-178/74

BOT CHORD

2-7=-525/1057, 6-7=-525/1057

WEBS 3-7=-130/338, 3-6=-1065/528

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 6 and 351 lb uplift at joint 2.

LOAD CASE(S) Standard



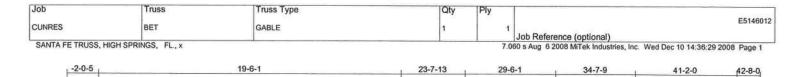
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December 10,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL 7473 BEFORE USE.

Installation guide

818 Soundside Road Edenton, NC 27932



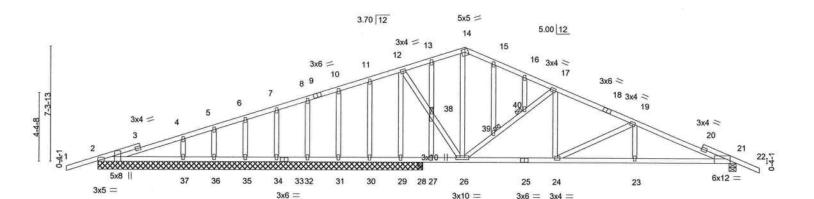
4-1-12

5-10-4

5-1-8

6-6-7

1-6-0 Scale = 1:73.9



1			20-11-1 23-7	13	29-6-1		34-7	-9	41-2-0		
		19-6-1			1-5-0 2-8-	13	5-10-4		5-1	-8	6-6-7
Plate Offsets (X,Y):	[2:0-5-2,Edge], [2:0-3-8,Ed	ge], [14:0-2-8	3,0-2-5], [21:	0-3-2,Edge	1						
LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.07	21-23	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.19	21-23	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	-0.02	28	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI	2002	(Matrix	x)						Weight: 244	1 lb
LUMBER					BRACING	17				445000000000000000000000000000000000000	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3

2-0-5

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-8-11 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 24-26,23-24,21-23.

1 Brace at Jt(s): 38, 39, 40

REACTIONS (lb/size) 2=215/20-11-8, 29=874/20-11-8, 37=352/20-11-8, 36=70/20-11-8, 35=183/20-11-8, 34=154/20-11-8, 32=166/20-11-8, 31=139/20-11-8, 30=215/20-11-8, 21=820/0-5-8, 28=312/0-3-8

Max Horz 2=133(LC 3)

Max Uplift 2=-150(LC 3), 29=-198(LC 6), 37=-96(LC 3), 36=-48(LC 5), 35=-64(LC 3), 34=-60(LC 3), 32=-62(LC 5), 31=-59(LC 3), 30=-66(LC 5), 21=-337(LC 6), 28=-83(LC 5)

Max Grav 2=269(LC 9), 29=874(LC 1), 37=374(LC 9), 36=70(LC 1), 35=185(LC 9), 34=154(LC 1), 32=166(LC 1), 31=148(LC 9), 30=215(LC 1), 21=820(LC 1), 28=312(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-208/486, 3-4=-199/539, 4-5=-151/502, 5-6=-129/519, 6-7=-97/515, 7-8=-68/516, 8-9=-38/483,

9-10=-35/515, 10-11=-9/519, 11-12=0/505, 12-13=-59/170, 13-14=-70/183, 14-15=-51/167, 15-16=-79/151,

16-17=-119/138, 17-18=-680/319, 18-19=-787/301, 19-20=-1381/458, 20-21=-1421/433, 21-22=0/34

2-37=-476/292, 36-37=-476/292, 35-36=-476/292, 34-35=-476/292, 33-34=-476/292, 32-33=-476/292, 32-32=-476/292,

31-32=-476/292, 30-31=-476/292, 29-30=-476/292, 28-29=-476/292, 27-28=-476/292, 26-27=-476/292,

25-26=-84/666, 24-25=-84/666, 23-24=-322/1275, 21-23=-322/1275

19-6-1

12-29=-968/252, 12-38=-207/908, 26-38=-216/938, 14-26=-173/24, 26-39=-791/331, 39-40=-768/319,

17-40=-752/308, 17-24=-57/443, 19-24=-674/264, 19-23=0/244, 4-37=-265/143, 5-36=-64/55, 6-35=-136/87, 7-34=-116/80, 8-32=-124/81, 10-31=-112/80, 11-30=-154/81, 13-38=-147/60, 27-38=-181/63, 15-39=-38/20,

16-40=-26/17

NOTES

WEBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; porch left exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 2, 198 lb uplift at joint 29, 96 lb uplift at joint 37, 48 lb uplift at joint 36, 64 lb uplift at joint 35, 60 lb uplift at joint 34, 62 lb uplift at joint 32, 59 lb uplift at joint 31, 66 lb uplift at joint 30, 337 lb uplift at joint 21 and 83 lb uplift at joint 28.



Marvin A. Strzyzewski, FL Lic. #4314 Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

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Job Qty Truss Truss Type E5146012 CUNRES BET GABLE Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 14:36:29 2008 Page 2

SANTA FE TRUSS, HIGH SPRINGS, FL., x

LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply E\$146013 CUNRES C **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:47 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -1-6-0 10-4-0 15-2-2 20-8-0 22-2-0 1-6-0 5-5-14 4-10-2 4-10-2 5-5-14 1-6-0 Scale = 1:39.4 4x4 = 5.00 12 1x4 \ 1x4 // 7 1 ф 10 3x4 = 3x4 > 3x4 = 3x4 = 7-1-4 13-6-12 20-8-0 7-1-4 6-5-7 7-1-4 Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8] LOADING (psf) SPACING 2-0-0 DEFL in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 1.25 TC Plates Increase 0.19 Vert(LL) -0.06 6-8 >999 240 244/190 MT20 TCDL 10.0 Lumber Increase 1.25 BC 0.38 Vert(TL) -0.176-8 >999 180 BCLL 0.0 Rep Stress Incr WB YES 0.15 Horz(TL) 0.04 6 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 92 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

REACTIONS (lb/size) 2=912/0-5-8, 6=912/0-5-8

Max Horz 2=73(LC 5)

Max Uplift2=-160(LC 5), 6=-160(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1550/154, 3-4=-1363/143, 4-5=-1363/144, 5-6=-1550/154, 6-7=0/34

BOT CHORD 2-10=-135/1365, 9-10=-38/941, 8-9=-38/941, 6-8=-63/1365

WEBS 3-10=-286/126, 4-10=-37/460, 4-8=-37/460, 5-8=-286/126

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 2 and 160 lb uplift at joint 6.

LOAD CASE(S) Standard

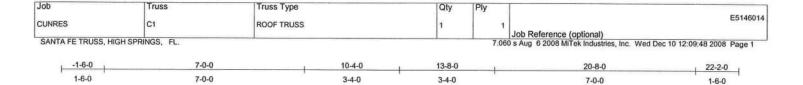


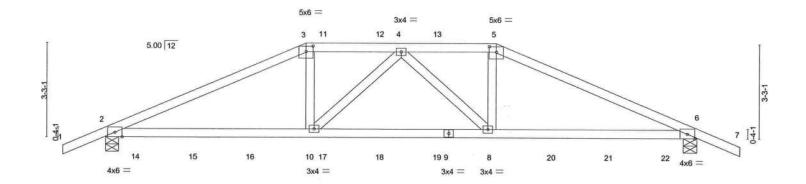
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	7-0-0				13-8-0		-			20-8-0			
		7-0-0			6-8-0					7-0-0			
Plate Of	fsets (X,Y): [[3:0-3-0,0-2-4], [5:0-3-0,0	-2-4]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.14	6-8	>999	240	MT20	244/190	
TCDL	10.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.41	6-8	>593	180	100000000		
BCLL	0.0	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.10	6	n/a	n/a			
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	ix)					1079075	Weight: 88 II)	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-11 oc purlins. Rigid ceiling directly applied or 7-7-13 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 2=1661/0-5-8, 6=1624/0-5-8

Max Horz 2=-57(LC 6)

Max Uplift2=-408(LC 5), 6=-393(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=0/34, 2-3=-3150/750, 3-11=-2845/719, 11-12=-2845/719, 4-12=-2844/719, 4-13=-2758/683, 5-13=-2759/683,

5-6=-3054/709, 6-7=0/34

2-14=-652/2822, 14-15=-652/2822, 15-16=-652/2822, 10-16=-652/2822, 10-17=-681/2944, 17-18=-681/2944,

18-19=-681/2944, 9-19=-681/2944, 8-9=-681/2944, 8-20=-575/2734, 20-21=-575/2734, 21-22=-575/2734,

6-22=-575/2734

WEBS 3-10=0/665, 4-10=-269/103, 4-8=-387/144, 5-8=-32/663

NOTES

BOT CHORD

WEBS

- NOTES

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

 3) Provide adequate drainage to prevent water ponding.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 2 and 393 lb uplift at joint 6.

 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 257 lb down and 152 lb up at 7-0-0, 125 lb down and 73 lb up at 7-7-4, 125 lb down and 73 lb up at 9-7-4, and 125 lb down and 73 lb up at 11-7-4, and 257 lb down and 152 lb up at 13-8-0 on top chord, and 98 lb down at 1-0-12, 107 lb down at 3-0-12, 94 lb down at 5-0-12, 94 lb down at 7-0-12, 94 lb down at 9-7-4, 94 lb down at 11-7-4, 94 lb down at 13-7-4, 94 lb down at 17-7-4, and 98 lb down at 19-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

 7) In the LOAD CASE(S) Standard

 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

 Marvin A. Strzyzewski, FL Lic. #43144

 Truss Engineering Co.

Continued on page 2



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

Scale = 1:40.1

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 REFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Design Valid for use only with writer connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer fracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	STEARSPENS
CUNRES	C1	ROOF TRUSS	1	1	E514601
SANTA FE TRUSS, I	HIGH CODINGS FI		(0)		Job Reference (optional)
SANTA PE TRUSS, I	HIGH SPRINGS, FL.			7.06	0 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:48 2008 Page 2

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 5-7=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 3=-257(B) 5=-257(B) 10=-47(B) 8=-47(B) 11=-125(B) 12=-125(B) 13=-125(B) 14=-49(B) 15=-72(B) 16=-47(B) 17=-47(B) 18=-47(B) 19=-47(B) 20=-47(B) 21=-72(B) 22=-49(B)



Job Truss Type Qty Ply E5146015 CUNRES C2 ROOF TRUSS Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:48 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -1-6-0 5-8-11 9-0-0 20-8-0 22-2-0 1-6-0 5-8-11 3-3-5 2-8-0 3-3-5 5-8-11 1-6-0 Scale = 1:40.1 4x4 = 4x8 = 4 5 5.00 12 1x4 > 1x4 / 11 10 9 3x5 = 3x5 > 3x5 = 3x4 = 3x10 = 9-0-0 11-8-0 20-8-0 9-0-0 2-8-0 9-0-0 Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-5-4,0-2-0], [7:0-3-0,0-1-8] LOADING (psf) SPACING 2-0-0 DEFL in (loc) I/defl L/d **PLATES** GRIP 1.25 TCLL 20.0 Plates Increase TC 0.29 Vert(LL) -0.147-9 >999 240 244/190 MT20 TCDL 10.0 Lumber Increase 1.25 BC 0.56 -0.39 Vert(TL) 7-9 >617 180 BCLL 0.0 Rep Stress Incr YES WB 0.11 0.05 Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 97 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-10-12 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **WEBS** 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing

REACTIONS (lb/size) 2=912/0-5-8, 7=912/0-5-8

Max Horz 2=-67(LC 6)

Max Uplift2=-153(LC 5), 7=-153(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34 2-3=-1490/146 3-4=-1203/99 4

DRD 1-2=0/34, 2-3=-1490/146, 3-4=-1203/99, 4-5=-1074/101, 5-6=-1201/99, 6-7=-1490/147, 7-8=0/34

BOT CHORD 2-11=-117/1322, 10-11=-0/1072, 9-10=-0/1072, 7-9=-66/1322

WEBS 3-11=-316/123, 4-11=-3/311, 5-11=-94/99, 5-9=-8/312, 6-9=-318/123

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 2 and 153 lb uplift at joint 7.

LOAD CASE(S) Standard



FL COA #7239

be installed during truss erection, in accordance with Stabilizer

Installation guide.

December 10,2008



Job Truss Type Truss Qty E5146016 CUNRES CJ01 **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:49 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 4-6-9 -2-1-7 9-10-1 2-1-7 1-4-4 3-2-5 5-3-8 1:20.3 0-3-14 3.54 12 6 0-4-1 9-10-1 8-5-13 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.63 Vert(LL) -0.00 2 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.02 Vert(TL) -0.00 2 >999 180 BCLL 0.0 Rep Stress Incr NO WB 0.00 Horz(TL) -0.00 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 Weight: 21 lb (Matrix) LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 1-4-4 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

REACTIONS (lb/size) 4=192/Mechanical, 2=252/0-7-12, 5=22/0-3-8, 3=720/0-3-8

Max Horz 2=124(LC 3)

Max Uplift4=-89(LC 3), 2=-82(LC 3), 3=-253(LC 4)

Max Grav 4=192(LC 1), 2=252(LC 1), 5=44(LC 2), 3=720(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-6=-90/0, 3-6=-73/45, 3-7=-80/2, 4-7=-33/45

BOT CHORD 2-5=0/0

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 4, 82 lb uplift at joint 2 and 253 lb uplift at joint 3.

5) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.

- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 28 lb down and 31 lb up at 4-2-8, 257 lb down and 79 lb up at 4-2-8, 88 lb down and 59 lb up at 7-0-7, 63 lb down and 42 lb up at 7-0-7, and 22 lb down at 1-4-9, and 22 lb down at 1-4-9 on top chord, and 19 lb down at 1-3-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 5=-9(F) 3=-284(F=-28, B=-257) 7=-151(F=-88, B=-63)

Marvin A. Strzyzewski, FL Lic. #4314 Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5146017 CUNRES CJ09 **ROOF TRUSS** Job Reference (optional) SANTA FE TRUSS, HIGH SPRINGS, FL. x 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 14:37:11 2008 Page 1 -2-1-7 3-6-7 5-5-1 9-10-13 2-1-7 3-6-7 1-10-10 4-5-12 Scale = 1:20.2 0-4-1 12 3.54 12 1x4 3 10 3x5 15 0-3-13 10x10 = 13 6.36 12 5x6 < 3-6-7 9-10-13 3-6-7 1-10-10 4-5-12 Plate Offsets (X,Y): [8:0-4-0,0-6-12], [9:0-3-0,0-2-6] LOADING (psf) SPACING CSI 2-0-0 DEFL I/defl **PLATES** GRIP (loc) L/d TCLL 20.0 1.25 TC Plates Increase 0.80 Vert(LL) -0.137-8 >885 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.85 -0.35 Vert(TL) 7-8 >324 180 BCLL 0.0 Rep Stress Incr NO WB 0.34 Horz(TL) 0.12 6 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 42 lb LUMBER BRACING TOP CHORD TOP CHORD

2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BOT CHORD

Structural wood sheathing directly applied or 5-5-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 5=222/0-1-8, 2=575/0-8-8, 6=261/Mechanical Max Horz 2=124(LC 3)

Max Uplift 5=-59(LC 3), 2=-109(LC 3), 6=-2(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-10=-1195/14, 3-10=-1143/29, 3-11=-1172/32, 4-11=-1134/40, 4-12=-59/0, 5-12=-24/55 BOT CHORD 2-13=-75/1107, 9-13=-75/1107, 9-14=-68/1092, 8-14=-60/1094, 8-15=-82/1011, 7-15=-82/1011, 6-7=0/0

WEBS 3-9=-229/18, 4-8=0/336, 4-7=-1024/83

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 5, 109 lb uplift at joint 2 and 2 lb uplift at joint 6.

6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 42 lb down at 4-3-4, 42 lb down at 4-3-4, 99 lb down and 24 lb up at 7-1-3, 99 lb down and 24 lb up at 7-1-3, and 22 lb down at 1-5-4, and 22 lb down at 1-5-4 on top chord, and 20 lb up at 1-5-4, 20 lb up at 1-5-4, 31 lb up at 4-3-4, 31 lb up at 4-3-4, and 17 lb up at 7-1-3, and 17 lb up at 7-1-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-60, 2-9=-20, 8-9=-20, 6-8=-20

Concentrated Loads (lb)

Vert: 11=-19(F=-10, B=-10) 12=-199(F=-99, B=-99) 13=20(F=10, B=10) 14=31(F=15, B=15) 15=17(F=9, B=9)



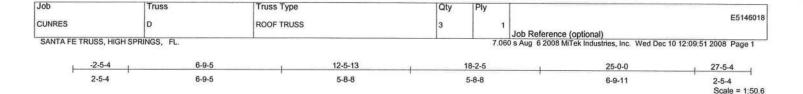
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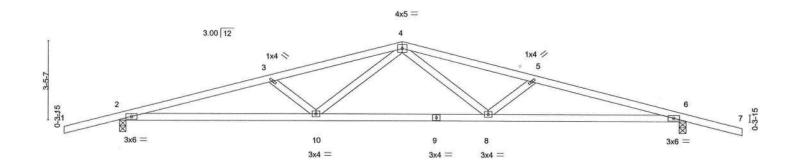
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	8-8-2		16-3-7		25-0-0	
	8-8-2	,	7-7-5	8-8-9		
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in ((loc) I/defl L/d	PLATES GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.37	Vert(LL) 0.29	6-8 >999 240	MT20 244/190	
TCDL 10.0	Lumber Increase 1.25	BC 0.64	Vert(TL) -0.51 8	8-10 >586 180	The state of the s	
BCLL 0.0	Rep Stress Incr YES	WB 0.22	Horz(TL) 0.11	6 n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)			Weight: 105 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-4-11 oc purlins. Rigid ceiling directly applied or 5-10-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1143/0-3-8, 6=1143/0-3-8

Max Horz 2=-59(LC 6)

Max Uplift2=-538(LC 3), 6=-538(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-2885/1183, 3-4=-2542/1098, 4-5=-2541/1098, 5-6=-2882/1182, 6-7=0/34

BOT CHORD 2-10=-1130/2745, 9-10=-728/1908, 8-9=-728/1908, 6-8=-1080/2742

3-10=-441/170, 4-10=-355/696, 4-8=-355/697, 5-8=-439/169 WEBS

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 538 lb uplift at joint 2 and 538 lb uplift at joint 6.

LOAD CASE(S) Standard



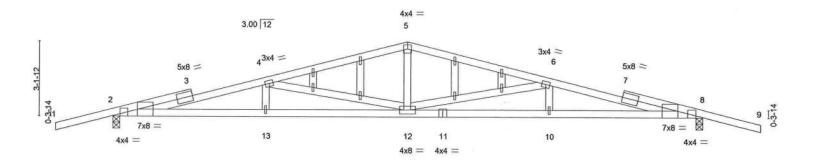
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December 10,2008

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	-	6-6-0		12-6-0 18-6-0		F	25-0-0					
		6-6-0		6-0	0-0		6-0-	0			6-6-0	,
Plate Of	fsets (X,Y):	[2:0-0-7,Edge], [2:0-5-6,E	dge], [8:0-0-7	,Edge], [8:0-5	-6,Edge]							
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	0.49	12-13	>610	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.74	10-12	>398	180	100000000	
BCLL	0.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.15	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	PI2002	(Matrix)						Weight: 121 I	b

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2

2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3 **OTHERS**

REACTIONS (lb/size) 2=1143/0-3-8, 8=1143/0-3-8

Max Horz 2=-71(LC 4)

Max Uplift 2=-733(LC 5), 8=-733(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-3546/2009, 3-4=-3512/2017, 4-5=-2276/1269, 5-6=-2276/1270, 6-7=-3512/2019, 7-8=-3546/2011,

8-9=0/34

BOT CHORD 2-13=-1963/3437, 12-13=-1963/3437, 11-12=-1902/3437, 10-11=-1902/3437, 8-10=-1902/3437

WEBS

4-13=-90/237, 4-12=-1305/841, 5-12=-439/730, 6-12=-1305/843, 6-10=-90/237

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) All plates are 1x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 733 lb uplift at joint 2 and 733 lb uplift at joint 8.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-9-11 oc purlins.

Rigid ceiling directly applied or 4-4-3 oc bracing.

December 10,2008



Job Truss Truss Type Qty Ply F5146020 CUNRES EET ROOF TRUSS 2 3x4 = Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:52 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 2-3-0 4-6-0 2-3-0 2-3-0 Scale = 1:8.1 5.00 12 2-3-0 4-6-0 2-3-0 2-3-0 Plate Offsets (X,Y): [2:0-2-0,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL in I/defl L/d **PLATES** GRIP 20.0 TC TCLL Plates Increase 1.25 0.07 Vert(LL) n/a 999 MT20 244/190 n/a TCDL 10.0 Lumber Increase 1.25 BC 0.20 Vert(TL) 999 n/a n/a BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=180/4-6-0, 3=180/4-6-0 Max Horz 1=11(LC 5)

Max Uplift1=-19(LC 5), 3=-19(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-192/53, 2-3=-192/53

BOT CHORD 1-3=-35/156

NOTES

Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 19 lb uplift at joint 3.

LOAD CASE(S) Standard



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December 10,2008

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Job Truss Truss Type Qtv E5146021 CUNRES EGT **ROOF TRUSS** 2 4x4 = Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:52 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 4-6-0 2-3-0 2-3-0 Scale = 1:8.1 5.00 12 3 2x4 || 5 2-3-0 4-6-0 2-3-0 2-3-0 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TC BC TCLL 20.0 0.07 Plates Increase 1.25 Vert(LL) -0.013-4 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 0.56 Vert(TL) -0.033-4 >999 180 BCLL 0.0 Rep Stress Incr NO WB 0.25 Horz(TL) 0.01 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 15 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BOT CHORD

Structural wood sheathing directly applied or 4-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=838/0-5-8, 3=646/0-5-8

Max Horz 1=-11(LC 6)

Max Uplift1=-82(LC 5), 3=-85(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1041/132, 2-3=-1041/131

BOT CHORD 1-4=-105/920, 4-5=-105/920, 3-5=-105/920

WEBS 2-4=-76/764

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 1 and 85 lb uplift at joint 3.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 340 lb down and 19 lb up at 0-2-12, and 308 lb down and 42 lb up at 2-1-8, and 513 lb down and 73 lb up at 2-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

Concentrated Loads (lb)

Vert: 1=-340(B) 4=-308(B) 5=-513(B)



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Design Valid for see Only will be Controlled to the design is observed to the proposed to the



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty E5146022 CUNRES EJ7 **ROOF TRUSS** 30 Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:53 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 7-0-0 1-6-0 7-0-0 3 Scale = 1:18.6 5.00 12 0-4-1 7-0-0

	f=0-0											
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.11	2-4	>757	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.27	2-4	>303	180	WW.	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	212002	(Mati			-100			1070	Weight: 24 lt	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=185/Mechanical, 2=383/0-5-8, 4=67/Mechanical

Max Horz 2=123(LC 5)

Max Uplift3=-83(LC 5), 2=-100(LC 5)

Max Grav 3=185(LC 1), 2=383(LC 1), 4=134(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-85/58

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3 and 100 lb uplift at joint 2.

LOAD CASE(S) Standard



Edenton, NC 27932 FL COA #7239

December 10,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Job Truss Truss Type Qty E5146023 CUNRES F.17A **ROOF TRUSS** Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:53 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -1-6-0 2-6-0 3-10-0 7-0-0 1-6-0 2-6-0 5 1-4-0 3-2-0 Scale = 1:18.6 1x4 || 5.00 12 1x4 || 90 0-4-1 5x5 = 9.00 12 8 2-6-0 3-10-0 7-0-0 2-6-0 1-4-0 Plate Offsets (X,Y): [8:0-2-8,0-2-4]

OADING (psf)	SPACING 2	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.17	7	>464	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.43	7	>186	180	100.000.00	
BCLL 0.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.02	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2	2002	(Matr	ix)				Visitatio	0.000	Weight: 27 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No 3 WEBS

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 5=224/0-1-8, 2=385/0-5-8, 6=31/Mechanical

Max Horz 2=124(LC 5)

Max Uplift5=-66(LC 5), 2=-101(LC 5)

Max Grav 5=224(LC 1), 2=385(LC 1), 6=62(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/34, 2-3=-118/0, 3-4=-58/0, 4-5=-36/79 2-8=0/15, 7-8=0/42, 6-7=-0/0 TOP CHORD

BOT CHORD

WEBS 3-8=0/56, 4-7=0/108

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 5 and 101 lb uplift at joint 2
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

LOAD CASE(S) Standard



December 10,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

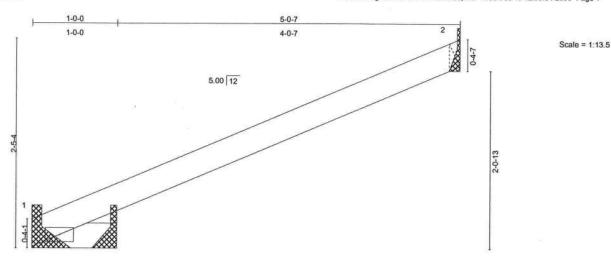
ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information

available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5146024 CUNRES J01 **ROOF TRUSS** Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:54 2008 Page 1

SANTA FE TRUSS, HIGH SPRINGS, FL



2x4 1=0-0 5-0-7 1-0-0 4-0-7

LOADING (psf)	SPACING 2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC	0.33	Vert(LL)	-0.00	1	>999	240	MT20 244/190
TCDL 10.0	Lumber Increase 1.25	BC (0.01	Vert(TL)	-0.00	1	>999	180	211100
BCLL 0.0	Rep Stress Incr YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002	(Matrix	()						Weight: 10 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=157/Mechanical, 2=148/Mechanical, 3=9/Mechanical

Max Horz 1=68(LC 5)

Max Uplift1=-35(LC 5), 2=-69(LC 5)

Max Grav 1=157(LC 1), 2=148(LC 1), 3=19(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-46/46 **BOT CHORD** 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3

- Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1 and 69 lb uplift at joint 2.

LOAD CASE(S) Standard



December 10,2008

Job Truss Truss Type Qty E5146025 CUNRES J01A ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:54 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, 1-0-0 3-0-7 1-0-0 2-0-7 5.00 12 1-2-13 1-0-0 3-0-7

LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	1	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	1	>999	180		2111100
BCLL 0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2004/TP	12002	(Matr	ix)						Weight: 7 It)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=97/0-5-8, 2=88/Mechanical, 3=9/0-3-8

Max Horz 1=41(LC 5)

Max Uplift1=-19(LC 5), 2=-41(LC 5)

Max Grav 1=97(LC 1), 2=88(LC 1), 3=19(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-28/28

BOT CHORD 1-3=0/0

NOTES

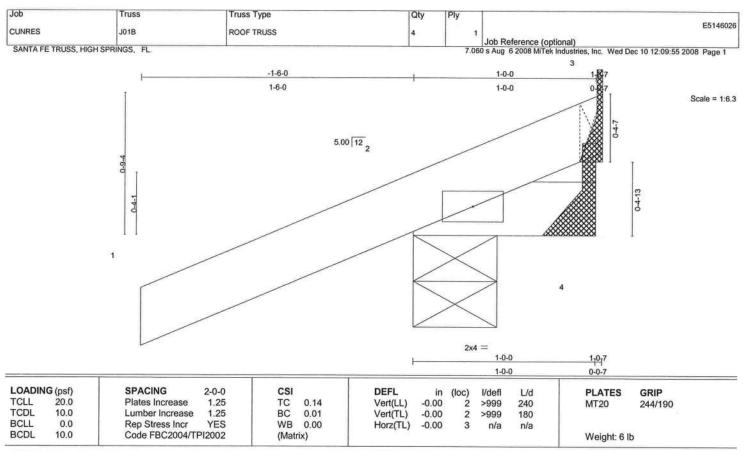
- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 41 lb uplift at joint 2.

LOAD CASE(S) Standard



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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=205/0-5-8, 4=9/Mechanical, 3=-44/Mechanical

Max Horz 2=45(LC 5)

Max Uplift2=-124(LC 5), 3=-44(LC 1)

Max Grav 2=205(LC 1), 4=19(LC 2), 3=44(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-44/18

BOT CHORD 2-4=0/0

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 2 and 44 lb uplift at joint 3.

LOAD CASE(S) Standard



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818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty E5146027 CUNRES J07 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:55 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -1-6-0 5-0-7 7-0-0 3 1-6-0 5-0-7 1-11-9 Scale = 1:14.7 5.00 12 0-4-1 2x4 = 5-0-7 7-0-0 5-0-7 1-11-9 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.23 Vert(LL) -0.11 2-4 >757 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.43 Vert(TL) -0.27>303 180 2-4 BCLL 0.0 Rep Stress Incr YES WB 0.00 -0.00Horz(TL) 3 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 21 lb LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=123/Mechanical, 2=332/0-5-8, 4=67/Mechanical

Max Horz 2=98(LC 5)

Max Uplift3=-53(LC 5), 2=-89(LC 5)

Max Grav 3=123(LC 1), 2=332(LC 1), 4=134(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-65/39

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 3 and 89 lb uplift at joint 2.

LOAD CASE(S) Standard

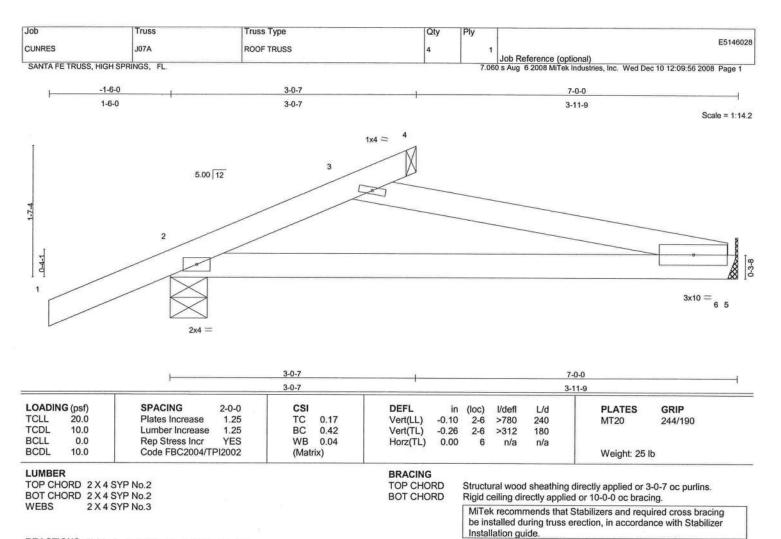


December 10,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





REACTIONS (lb/size) 2=317/0-5-8, 6=92/Mechanical

Max Horz 2=69(LC 5) Max Uplift2=-89(LC 5)

Max Oplitt2=-89(LC 5)

Max Grav 2=317(LC 1), 6=147(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-160/60, 3-4=-19/0

BOT CHORD 2-6=-75/107, 5-6=0/0 WEBS 3-6=-109/77

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

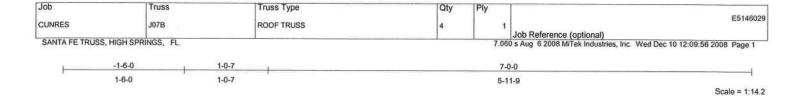
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2.

LOAD CASE(S) Standard



December 10,2008







	1-0-7	7-0-0						
	1-0-7	5-11-9						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES	CSI DEFL in (loc) l/defl L/d PLATES GRIP TC 0.14 Vert(LL) -0.12 2-4 >699 240 MT20 244/190 BC 0.45 Vert(TL) -0.30 2-4 >279 180 WB 0.00 Horz(TL) -0.00 3 n/a n/a						
BCDL 10.0	Code FBC2004/TPI2002	(Matrix) Weight: 15 lb						

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=264/0-5-8, 4=69/Mechanical, 3=-44/Mechanical

Max Horz 2=45(LC 5)

Max Uplift2=-94(LC 5), 3=-44(LC 1)

Max Grav 2=264(LC 1), 4=138(LC 2), 3=44(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-44/18

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 2 and 44 lb uplift at ioint 3.

LOAD CASE(S) Standard



818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5146030 CUNRES J1 **ROOF TRUSS** 2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industrie SANTA FE TRUSS, HIGH SPRINGS, FL. es, Inc. Wed Dec 10 12:09:57 2008 Page 1 3 -1-6-0 1-0-7 1-6-0 1-0-7 Scale = 1:6.3 5.00 12 2 0-4-1 2x4 = 1-0-7 1-0-7 LOADING (psf) SPACING 2-0-0 DEFL CSI (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.14 Vert(LL) -0.00 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.01 Vert(TL) -0.00 2 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 -0.00 3 Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 6 lb

BRACING

TOP CHORD

BOT CHORD

REACTIONS (lb/size) 2=206/0-5-8, 4=10/Mechanical, 3=-46/Mechanical Max Horz 2=44(LC 5) Max Uplift2=-125(LC 5), 3=-46(LC 1) Max Grav 2=206(LC 1), 4=20(LC 2), 3=46(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/33, 2-3=-44/19 TOP CHORD

BOT CHORD 2-4=0/0

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 2 and 46 lb uplift at joint 3.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-0-7 oc purlins.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

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Job Truss Truss Type Qty Ply E5146031 CUNRES J3 ROOF TRUSS 2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:58 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -1-6-0 3-0-7 1-6-0 3-0-7 1x4 || Scale = 1:10.1 3 5.00 12 2 0-4-1 0-3-11 9.00 12 6 4x4 = 2x4 = 2-6-0 2-6-0 0-6-7 Plate Offsets (X,Y): [6:0-2-8,0-2-4] LOADING (psf) SPACING 2-0-0 CSI DEFL in I/defl L/d PLATES GRIP (loc) 20.0 TCLL Plates Increase 1.25 TC 0.17 Vert(LL) -0.00 >999 6 240 MT20 244/190

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

10.0

0.0

10.0

BRACING

Vert(TL)

Horz(TL)

-0.01

-0.00

2-6

5

>999

n/a

180

n/a

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-7 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 14 lb

REACTIONS (lb/size) 4=70/Mechanical, 2=244/0-5-8, 5=5/Mechanical

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

Max Horz 2=70(LC 5)

Max Uplift4=-8(LC 4), 2=-103(LC 5)

Max Grav 4=70(LC 1), 2=244(LC 1), 5=9(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-47/6, 3-4=-5/26

BOT CHORD 2-6=-8/0, 5-6=-5/5

WEBS 3-6=0/55

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

BC

WB

(Matrix)

0.05

0.02

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 4 and 103 lb uplift at joint 2.

LOAD CASE(S) Standard



818 Soundside Road Edenton, NC 27932 FL COA #7239

December 10,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty F5146032 CUNRES **J**5 **ROOF TRUSS** 2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Dec 10 12:09:58 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -1-6-0 2-6-0 3-10-0 5-0-7 1-6-0 2-6-0 1-4-0 1-2-7 Scale = 1:14.8 1x4 || 5.00 12 1x4 || 3 75x5 = 9 2 0-4-1 9.00 12 4x4 = 2x4 = 2-6-0 3-10-0 5-0-7 2-6-0 1-4-0 1-2-7 Plate Offsets (X,Y): [8:0-2-8,0-2-4] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP 20.0 TCLL Plates Increase 1.25 TC 0.32 Vert(LL) -0.048 >999 240 244/190 MT20 BC TCDL 10.0 Lumber Increase 1.25 0.05 Vert(TL) -0.11 180 8 >521 BCLL 0.0 Rep Stress Incr YES WB 0.02 Horz(TL) -0.046 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 21 lb LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 5=159/0-1-8, 2=312/0-5-8, 6=11/Mechanical

Max Horz 2=98(LC 5)

Max Uplift5=-35(LC 5), 2=-98(LC 5)

Max Grav 5=159(LC 1), 2=312(LC 1), 6=23(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/34, 2-3=-82/0, 3-4=-31/17, 4-5=-17/59

BOT CHORD 2-8=0/4, 7-8=0/29, 6-7=-0/0

WEBS 3-8=0/63, 4-7=0/60

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 5 and 98 lb uplift at joint 2.
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

LOAD CASE(S) Standard



Edenton, NC 27932 FL COA #7239

December 10,2008

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Project Name:

Address:

JERRY CUNNINGHAM

COLUMBIA COUNTY

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Permitting Office:

CUNNINGHAM RESIDENCE

City, Sta Owner: Climate	JERRY CUN	NINGHAM	Permit Number: 276 Jurisdiction Number: 22	
2. Sing 3. Num 4. Num 5. Is thi 6. Conc 7. Glass a. U-fa (or S b. SHG (or S b. N/A c. N/A 9. Wall a. Fram b. Fram c. Fram d. N/A e. N/A 10. Ceili a. Unde b. N/A c. N/A 11. Duct	Single or Double DEFAULT) 7 iC: Clear or Tint DEFAULT) 7 r types -On-Grade Edge Insulation types ne, Wood, Adjacent ne, Wood, Exterior ne, Wood, Adjacent ne, Wood, Adjacent	Description Area	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 48.0 kBtu/hr SEER: 13.00
	Glass/Floor Area:	0.12 Total as-built p	points: 28739 points: 30215 PASS	

I hereby certify that the plans and specifications covered by Review of the plans and this calculation are in compliance with the Florida Energy specifications covered by this calculation indicates compliance Code. with the Florida Energy Code. PREPARED BY Farry Resmondo alc Before construction is completed DATE: Que 12 this building will be inspected for compliance with Section 553.908 I hereby certify that this building, as designed, is in compliance Florida Statutes. with the Florida Energy Code. BUILDING OFFICIAL: OWNER/AGENT: DATE: DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643-PERMIT #:

BASE		AS-BI	JILT		
GLASS TYPES .18 X Conditioned X BSPM = Points	0	verhang			
Floor Area	Type/SC Orn		t Area X SPM X	SOF	= Points
.18 2128.0 18.59 7121.0	1.Double,U=0.87,Clear E	8.0 8.0	14.0 42.06	0.52	304.0
1	2.Double,U=0.87,Clear		60.0 42.06	0.63	1591.0
	3.Double,U=0.87,Clear		30.0 42.06	0.91	1151.0
	4.Double,U=0.87,Clear	th contract the contract		0.94	540.0
	5.Single,U=0.60,Clear			0.92	352.0
	6.Double,U=0.87,Clear W			0.45	345.0
	7.Double,U=0.87,Clear W			0.91	527.0
	8.Double,U=0.57,Clear W		STATE OF THE STATE	0.47	728.0
	9.Double,U=0.87,Clear SW		The season of th	0.41	164.0
	10.Double,U=0.87,Clear	n riberi resine		0.86	460.0
	11.Double,U=0.87,Clear	3 1.5 2.0	3.0 35.87	0.57	60.0
	As-Built Total:		252.0		6222.0
WALL TYPES Area X BSPM = Points	Туре	R-Val	ue Area X SP	M =	Points
Adjacent 1679.0 0.70 1175.3	1. Frame, Wood, Adjacent	0.0	1540.0 2.20	0	3388.0
Exterior 1115.0 1.70 1895.5	2. Frame, Wood, Exterior	19.0	1115.0 0.90	0	1003.5
The second secon	3. Frame, Wood, Adjacent	19.0	139.0 0.40	0	55.6
Base Total: 2794.0 3070.8	As-Built Total:		2794.0		4447.1
DOOR TYPES Area X BSPM = Points	Туре	i.	Area X SP	M =	Points
Adjacent 21.0 2.40 50.4	1.Exterior Wood		21.0 6.10	0	128.1
Exterior 21.0 6.10 128.1	2.Adjacent Wood		21.0 2.40)	50.4
Base Total: 42.0 178.5	As-Built Total:		42.0		178.5
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X SPM X S	CM =	Points
Under Attic 2128.0 1.73 3681.4	1. Under Attic	30.0	2128.0 1.73 X 1.00)	3681.4
Base Total: 2128.0 3681.4	As-Built Total:		2128.0		3681.4
FLOOR TYPES Area X BSPM = Points	Туре	R-Val	ue Area X SP	M =	Points
Slab 173.5(p) -37.0 -6419.5 Raised 0.0 0.00 0.00	Slab-On-Grade Edge Insulation	5.0	173.5(p -36.20)	-6280.7
Base Total: -6419.5	As-Built Total:		173.5		-6280.7
INFILTRATION Area X BSPM = Points			Area X SP	M =	Points
2128.0 10.21 21726.9			2128.0 10.2	1	21726.9

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643-PERMIT #:

	BASE		AS-BUILT
Summer Ba	ase Points: 2	9359.1	Summer As-Built Points: 29975.2
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
29359.1	0.3250	9541.7	(sys 1: Central Unit 48000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS) 29975 1.00 (1.09 x 1.000 x 1.00) 0.260 1.000 8495.0 29975.2 1.00 1.090 0.260 1.000 8495.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643- PERMIT #:

BASE			AS-	BUI	LT				
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Type/SC		erhang Len	Hgt	Area X	WP	мх	WOI	= Points
.18 2128.0 20.17 7726.0	1.Double,U=0.87,Clear	Е	8.0	8.0	14.0	18.7	9	1.29	338.0
	2.Double,U=0.87,Clear	E	4.0	6.0	60.0	18.7		1.18	1332.0
	3.Double,U=0.87,Clear	E	1.5	6.0	30.0	18.7	9	1.04	583.0
	4.Double,U=0.87,Clear	N	1.5	6.0	30.0	24.5	8	1.00	739.0
	5.Single,U=0.60,Clear	N	1.5	5.0	16.0	16.4		1.00	263.0
	6.Double,U=0.87,Clear	W	9.0	6.0	20.0	20.7		1.20	499.0
	7.Double,U=0.87,Clear	W	1.5	6.0	15.0	20.7		1.02	318.0
_	8.Double,U=0.57,Clear	W	10.0	7.5	39.0	12.3		1.20	574.0
	9.Double,U=0.87,Clear 10.Double,U=0.87,Clear	SW S	10.0	6.0 6.0	10.0 15.0	16.7 13.3		1.87	312.0
	11.Double,U=0.87,Clear	S	1.5	2.0	3.0	13.3		2.27	222.0 90.0
		J	1.0	2.0	3.0	10.0		2.21	90.0
-	As-Built Total:				252.0				5270.0
WALL TYPES Area X BWPM = Points	Туре		R-	Value	Area	Χ	WPN	=	Points
Adjacent 1679.0 3.60 6044.4	1. Frame, Wood, Adjacent			0.0	1540.0		10.40		16016.0
Exterior 1115.0 3.70 4125.5	2. Frame, Wood, Exterior			19.0	1115.0		2.20		2453.0
	3. Frame, Wood, Adjacent			19.0	139.0		2.20		305.8
Base Total: 2794.0 10169.9	As-Built Total:				2794.0				18774.8
DOOR TYPES Area X BWPM = Points	Туре				Area	Х	WPM	=	Points
Adjacent 21.0 11.50 241.5	1.Exterior Wood				21.0	-	2.30		258.3
Exterior 21.0 12.30 258.3	2.Adjacent Wood				21.0		1.50		241.5
Base Total: 42.0 499.8	As-Built Total:				42.0				499.8
CEILING TYPES Area X BWPM = Points	Туре	R	-Value	Are	ea X W	PM)	(WC	M =	Points
Under Attic 2128.0 2.05 4362.4	1. Under Attic		;	30.0	2128.0 2	2.05 X	1.00		4362.4
Base Total: 2128.0 4362.4	As-Built Total:				2128.0				4362.4
FLOOR TYPES Area X BWPM = Points	Туре		R-\	/alue	Area	χV	NPM	=	Points
Slab 173.5(p) 8.9 1544.1 Raised 0.0 0.00 0.00	1. Slab-On-Grade Edge Insula	ation		5.0 1	173.5(p		7.60		1318.6
Base Total: 1544.1	As-Built Total:				173.5				1318.6
INFILTRATION Area X BWPM = Points					Area	χV	NPM	=	Points
2128.0 -0.59 -1255.5					2128.0)	-0.59		-1255.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643-PERMIT #:

	BASE		AS-BUILT			
Winter Base Points: 23046.7			Winter As-Built Points: 28970.1			
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)			
23046.7	0.5540	12767.9	(sys 1: Electric Heat Pump 48000 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 28970.1 1.000 (1.069 x 1.000 x 1.00) 0.401 1.000 12424.0 28970.1 1.00 1.069 0.401 1.000 12424.0			

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643-PERMIT #:

	E	BASE			AS-BUILT							
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank EF Number of X Tank X Multiplier X Credit = Tota Volume Bedrooms Ratio Multiplier							
3		2635.00		7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0
					As-Built To	otal:						7820.0

	CODE COMPLIANCE STATUS												
		BAS	SE							AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling + Heating + Hot Water = Total Points Points Points Points						
9542		12768		7905		30215	8495		12424		7820		28739

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , 32643- PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.4

The higher the score, the more efficient the home.

JERRY CUNNINGHAM, , , , 32643-

1.	New construction or existing	Ne		12.	Cooling systems		
2.	Single family or multi-family	Single fami	y	a.	Central Unit	Cap: 48.0 kBtu/hr	_
3.	Number of units, if multi-family		1			SEER: 13.00	
4.	Number of Bedrooms		3 _	b.	N/A		_
5.	Is this a worst case?	1	o				_
6.	Conditioned floor area (ft2)	2128	t²	c.	N/A		_
7.	Glass type 1 and area: (Label reqd.)	by 13-104.4.5 if not default)				
a.	U-factor:	Description Area		13.	Heating systems		
	(or Single or Double DEFAULT)		_	a.	Electric Heat Pump	Cap: 48,0 kBtu/hr	-
b.	SHGC:	Personal and Colors				HSPF: 8.50	100000 20000
	(or Clear or Tint DEFAULT)	7b. (Clear) 252.0 ft		b.	N/A		
8.	Floor types	,					10000
a.	Slab-On-Grade Edge Insulation	R=5.0, 173.5(p)	ft	c.	N/A		76
	N/A		-				7570
c.	N/A			14.	Hot water systems		
	Wall types		70.		Electric Resistance	Cap: 40.0 gallons	1
	Frame, Wood, Adjacent	R=0.0, 1540.0	t²			EF: 0.93	1
	Frame, Wood, Exterior	R=19.0, 1115.0	7	b.	N/A		
	Frame, Wood, Adjacent	R=19.0, 139.0	_				
	N/A		5,111	C.	Conservation credits		
	N/A		-		(HR-Heat recovery, Solar		
	Ceiling types				DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 2128.0	t²	15.	HVAC credits		
	N/A		_		(CF-Ceiling fan, CV-Cross ventilation,		-
	N/A				HF-Whole house fan,		
	Ducts(Leak Free)		-		PT-Programmable Thermostat,		
	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 350.0	ft		MZ-C-Multizone cooling,		
	N/A	24,11 010,0001	-		MZ-H-Multizone heating)		
	1111		-		in a manual manu		
T	rtify that this home has complie	nd with the Floride Ener	ov Eff	ioionos	Code For Building		
	istruction through the above en					OF THE STATE	à.
						3	B
	nis home before final inspection ed on installed Code compliant	50	Disbi	ay Cai	a will be completed	5	8
	발생하면 10명을 보는 11 시간에 보면 12 전 12		-				
Bui	lder Signature:		Dat	e:		13	X
						*	* #

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar TMdesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

Address of New Home:

ON BIA COUNTY

COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave.. Suite B-21

Lake City, FL 32055

Office: 386-758-1008 Fax: 386-758-2160

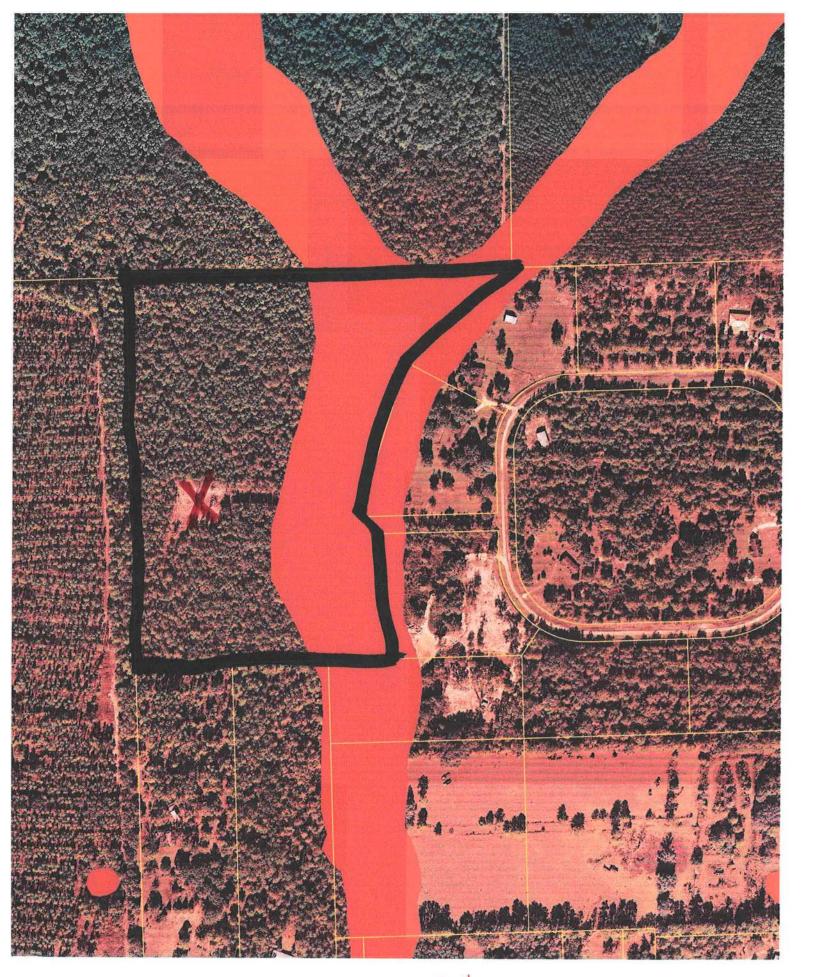
NOTARIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

	TYPE OF CONSTRUCTION		
(Y Single Family Dwelling	() Two-Family Residence	() Farm Outbui	lding
() Other	() Addition, Alteration, Modifica		_
from contractor licensing as an owner/builde	, have been advised of the a	above disclosure states ements provided for in	ment for exemption
ss.489.103(7) allowing this exception for the	construction permitted by Columb	ia County Building	
Permit Number	0 11	0	
FLORIDA NOTARY The above signer is pers Notary Signature	N# DD 805657 Ily 14, 2012 Public Underwriters Or produced identification	er's Libers	10/10/08 Date
Notary Signature	Date 10/10/08	C-552-42	-5-52-010-0
FOR BUILDING DEPARTMENT USE ONLY I hereby certify that the above listed owner/k ss 489.103(7). Date 10.10.08 Build		closure statement in F	lorida Statutes



0810-21

Columbia County Property Appraiser DB Last Updated: 8/5/2008

2008 Proposed Values

Search Result: 1 of 1

Property Card Interactive GIS Map Tax Record

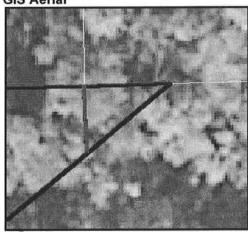
Print

Parcel: 21-6S-17-09710-000

Owner & Property Info

Owner's Name	CUNNINGHAM	1 JERRY E & BELINDA					
Site Address							
Mailing Address		11505 MELLOW CREEK LANE RIVERVIEW, FL 33569					
Use Desc. (code)	TIMBERLAND	(005500)					
Neighborhood	21617.00	Tax District	3				
UD Codes	MKTA02	Market Area	02				
Total Land Area	28.000 ACRES	5					
Description	OF NW1/4 OF HAMMOCK S/ 768-277, 792	NE1/4 OF NE1/4 EX PINE OAK HAMMOCK & NW1/4 OF NW1/4 OF SEC 22-6S-17 EX PINE OAK HAMMOCK S/D. ORB 745-555, 745-556, 755-086, 768-277, 792-1181, 863-716, 863-778, EASEMENT DEED RECORDED ORB 1026-984 & ORB 1037-676					

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (0)	\$0.00
Ag Land Value	cnt: (1)	\$3,332.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$3,332.00

Just Value	\$135,660.00
Class Value	\$3,332.00
Assessed Value	\$3,332.00
Exempt Value	\$0.00
Total Taxable Value	\$3,332.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
1/26/2005	1037/676	WD	V	Q		\$83,000.00
7/28/1998	863/778	WD	V	U	01	\$5,000.00
7/14/1998	863/778	СТ	V	U	01	\$1,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
			NONE			

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
				NONE		

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
005500	TIMBER 2 (AG)	28.000 AC	1.00/1.00/1.00/.50	\$119.00	\$3,332.00
009910	MKT.VAL.AG (MKT)	28.000 AC	1.00/1.00/1.00/1.00	\$0.00	\$135,660.00

SEEMING OFFICE BEFORE YOUR FIRST INSPECTION. ***

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 21-65-17-09710-000

Inst:200812019632 Date:10/27/2008 Time:3:08 PM _____DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1161 P:405

1. Description of property: (legal description of the p	property and street address or 911 address)
NE 14 OF NE 14 EXPINE DAK HAMIN	MACK & MILL MAN NOW IN THE SECOND AS AS AS
ENLINE UNK HAMMOCK SID. O	88745 EEF 745-CV
792-1181, 863-716, 863-778,	Easement Deed Recorded ORB
	No. of the control of
2. General description of improvement: Single	Family Dewelling
3. Owner Name & Address Jerry E Cunni	Interest in Property Duner
Riverview FL 23569	Interest in Property A.
4. Name & Address of Fee Simple Owner (If other than	an owner):
[4] - S. [4] - [4]	
5. Contractor Name Jer (4 E Cumin en Am	e Riverview FL 33569
Address 11505 Mellow Creek (pn.	e Riverview El 37507055
6. Surety Holders Name	Phone Number
Address	Frione Number
Amount of Bond	
7. Lender Name	Phone Number
Address	Frione Number
8. Persons within the State of Florida designated by served as provided by section 718.13 (1)(a) 7; Florida	the Owner upon whom notices or other documents may be Statutes:
Name	Phone Number
Address	
9. In addition to himself/herself the owner designate	
to receive a c	opy of the Lienor's Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee	
10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified)	the date of recording,
NOTICE AS PER CHAPTER 713, Florida Statutes:	
The owner must sign the notice of commencement and	d no one else may be permitted to sign in his/her stead.

Signature of Owner

Sworn to (or affirmed) and subscribed before day of 15th Legtember , 2018

Notary Public - State of Florida My Commission Express Sep 16, 2011

Commission if DD 689541

Bonded Through National Notary Assn.

Signature of Notary

21450



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number _ -- PART II - SITE PLAN --Scale: Each block represents 5 feet and 1 inch = 50 feet. water LIVE Drive WAY 250' 210' 500 To South Property Notes: Creek Ditch is Empty 90% of year This Property has A recorded easment Site Plan submitted by: Jung Cheming Plan Approved Not Approved Date

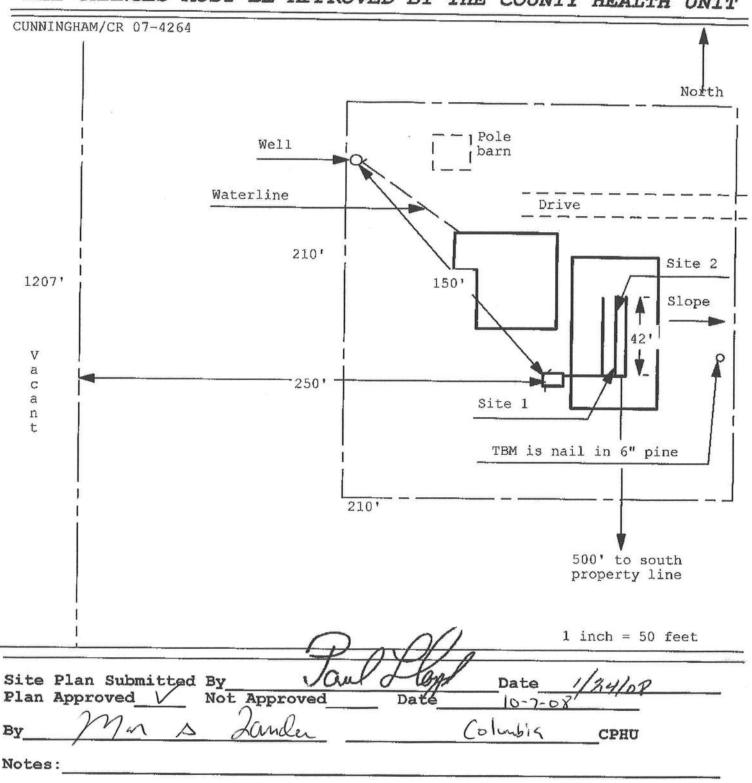
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

By_

County Health Department

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787 PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED:

10/2/2008

DATE ISSUED:

10/6/2008

ENHANCED 9-1-1 ADDRESS:

529

SW HAMMOCK HILL

CIR

LAKE CITY

FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

21-6S-17-09710-000

Remarks:

PINE OAK HAMMOCK S/D

Address Issued By:

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.





STATE OF FLORIDA REPAIR, MODIFY, OI	PERMIT APPLICATION TO CONSTRUCT, R ABANDON A WELL
☐ Southwest ☐ Northwest ☐ South Florida	THIS FORM MUST BE FILLED OUT COMPLETELY. The water well contractor is responsible for completing this form and forwarding the permit to the appropriate delegate

for completing this

Permit No. Florida Unique I.D. Permit Stipulations Required (See attached)

	WELL	⊠ Suwannee	LHAGI	ere applicable.				SUP Application	No	
_		CHECK BUX FOR APPR	IOPRIATE DISTRICT. ADDRESS	ON BACK OF PER	MIT FORM			ABOVE THIS	LIME 4 OF	OFFICIAL USE OBLY
	Jerry & Belinda (Owner, Legal Name of I	Entity if Corporation		11505 N Addres	dellow (reek Lane. C	Rivervi	iew, FL 33569	Zip	813-690-2523 Telephone Num
2.	SW Hammock Hill Well Location — Address	I Circle off Hwy s, Road Name or Nu	441 mber, City	Par	rcel# 21-	6S-17-0971	10-000			
3.	Chester Ray Sheff	ield		2665	Office	(386)454-0	355 Fe	v· (386)454_3	724 0	ell: (352) 215-9
	Well Drilling Contractor			License No.	011100	(300)1017	333 14	Telephon		NWNE
	PO Box 2662					NE	MC .		T. 07. T. 200	
	Address				_ *	(9ms/899) 1/4 OT	(political)	/4 of Section 2		
	High Springs	FL	32655					(Indicate Well o	n Chert)	1 1 1 1
	City	State	Zip		- 5.	Township 6	S	Range 17		
	a								_	
6.	Columbia		Subdivision Name							
	County .		Subdivision Name		Lot	Blo	ock	Unit	*	SW SE
7.	Number of proposed w	ells1 Checi	t the use of well: (See bad	k of permit for eddi	ional cholose)	X	Domestic	Monitor (type)	
	(See Back) Irrigation (t)	ype) Public Wa	iter Supply (type)			List Other				
	Distance from septic s	ystem 100 ft.	Description of facility	home		Estimated :	start of co	onstruction date	1/30/0	08
8.	Application for: X	New Construction	Repair/Modify	Abr	endonment	. 1005-11005-1100				Date Stamp
						(Re		endonment)	٦.,	
9.	Estimated: Well Depth	100.	Casing Depth	140'		Screen	Interval fo	om to	KE	CEIVED
	Casing Mate	rial: Blk-Steel / Gal /	PMC Casing Diamet	er <u>4"</u>						
0.	If applicable: Proposed	From 1401	to 31 Continued	L Dantonit					JAN	24 2008
	Grouting Interval	From 3'	to 0' Seal Materi	at Dentoniu		_				N - 2000
		From	o Seal Materia	el <u>Comano</u>						
4 7	Palasnana Casluu					roads and	landmarks	r; provide distances t	wan saa v watwoon v	vith un "X". Identily kno vell and landmarks.
•	Telescope Casing Blk-Steel / Galvanized / P			_				North	ı	
	Lint-Stool / Garvarnzed / P	vc Omer (spe	cify:			-		100		
2. 1	Method of Construction	: X Rotary	Cable Tool	Combi	nstion	1		N		
			y:)		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	- 1				
3 6					•	1				
	ndicate total No. of well					8	45			
4. k	this well or any other t	well or water withdo	awai on the owner's cor	ntiguous prop	erty cove	red ≸		20 1000 0		
u	nder a Consumptive/W	der Use Permit (CU	P/WUP) or CUP/WUP A	pplication? >	(No	Year		See Attacl	iment	
(1	f yes, complete the follow	ina) CUP/WUP	No.							
	istrict well I.D. No.									
	atitude					. 1				
D	ata obtained from GPS _		survey (map datum							
					NAD 63	_)		South		
pri ap go mh	erably cartily that I will comply will d that a water use permit or artilic or to commencement of well com- plication is accurate and that I will veryments, if applicable. I agree is ar drifting or the permit explication.	t the approache ruses of 1 to full recharge permit, if need fruction. I further certify that a between the completion of whichever occurs first.	140, Florida Administrative Code, ed, has been or will be obtained tell information provided on this from other lederal, state, or local report to the District within 30 day	l cersiy respen the age sponsit ra	y that I arm the sibilities under set for the over silities as allele	owner of the proper Chapter 373, Florid er, first the Informat of above. Owner co	rly, that the in de Statetes, t fon provided nsents to per	formation provided is at to maintain or properly a is accurate, and that it is sonnel of the WMD or a	ocumie, an bendon Sá ave inform representa	d that I am aware of my a well; or, I certily that I am of the comer of his re- sive access to the well sit
24	Chester K.	sugara	2665				-			
01	Service of Coulterior	00.1	License No.		- 252		Omer's or A	geni's Signeture		Date
		11-	OT WRITE BELOW T	HIS LINE -	FOR OF	FICIAL USE	ONLY	,		
Ą	oproval Granted By:	Anom Ol	asour		Issue Da	te: 81-20	4-08	Hydrold	glat App	oroval
										Initials

WELL COMPL	ETION REPO	RT (Please comple	te in black ink or type.)				da Cunningham		
PERMIT # 97443							Florida Unique I.D		
f permit is for multiple				WELL USE: DEP/	Public	Imig	ation Domestic X Monitor		
ndicate remaining we			,	HRSI	Limited _	62-	524 Other		
WATER WELL CONT				DRILL METHOD	X Rota	ry []	Cable Tool [] Combination		
SIGNATURE		Licen Licen	se # 2665	Charles and Allert Andrews and Allert	[] Jet		Auger Other		
		provided in this report is		Managerard Static Wa	ter I evel	106'	Measured Pumping Water Level		
Grout	No. of Bags	From (FI	L) To (Ft.)	After 1 Hours at	18 G.P.	Measu	ring Pt. (Describe): top of casing		
Neat Cement:	1	3	0		Which is 1 Ft. MAbove []B		elow Land Surface		
Bentonite:	11	156	3	Open Hole	Dec		í ·		
WELL LOCATION:	County Columbi	a		Screen	(FI		DRILL CUTTINGS LOG Examine cuttings every 20 ft. or at formation changes.		
NE 1/4 of NE 1/4	4 of Section 21	Twp: <u>6S</u>	Rge: 17	Casing Diameter & Depth (Ft.)	From	То	Note cavities, depth to producing zones. Color Grain Size Type of Material		
Latitude 29*57'27		Longitude 082*36		Diameter 4"	0	8	sand		
DATE ST	TAMP	Sketch of well location	n on property	From 0'	8	30	sandy clay		
		well located far	back of prop	то156'	30	48	dry pebble clay		
		X 00	12. 0		48	60	clay & rock mix		
				Diameter	60	78	jagged rock		
		No slab/No sept	tic at this time	From	78	84	" " lost circulation		
Official Us				То	84	90	" " hard rock		
CHEMICAL ANALYSIS					90	110			
Iron: ppm S				Liner [] or	110	170	limestone to b.o.h.		
Chloride: ppm				Casing [] Diameter					
[] Lab Test []		SW Hammock		From					
	FIEID TEST KIT	Give distances from se or other reference point	ptic tank and house	То			-		
Pump Type									
[] Centrifugal [
Horsepower 11/				Driller's Name:					
Pump Depth 135	rt. Intake	Debtu 130 F	T., Form 40B-3-3 Rev. 12/95		ster Sh	effield	assisted by Melissa Sheffield		



SANTAFETRUSS

October 20, 2008
Page 1 of 2
Columbia County Building Department
Plans Examiner
Re: Cunningham residence (Permit # 0810-21)

Dear Joe,

Jerry asked me to send you this design for his garage door header. This was produced using design software from Boise Wood Products, using the applied loads from the "C" trusses shown on our truss documents.

I hope this provides the information you need; if not, please contact me to let me know what else is needed.

Best Regards,

Tom Wootton President ouble 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP Roof Beam\...GARAGE HEADER

BC CALC® 9.5 Design Report - US Build 91

1 span | No cantilevers | 0/12 slope

Monday, October 20, 2008 17:03

Job Name: Address:

City, State, Zip: Customer:

JERRY CUNNINGHAM

Code reports: ESR-1040 File Name: cunres.BCC

Description: GARAGE HEADER Specifier: TOM WOOTTON Designer:

TOM WOOTTON Company: SANTA FE TRUSS

Misc:

16-09-00

B0, 3-1/2" 5299 lbs

B1, 3-1/2" 4607 lbs

Total Horizontal Product Length = 16-09-00

oad Summary ag Description	Load Type	Ref.	Start	F4	Live	Dead	Snow	Wind	Roof	ive
Standard Load	The state of the s		Start	End	100%	90%	115%	133%	125%	Trib
C1 TRUSS	Unf. Area (psf) Conc. Pt. (lbs)	Left Left	00 - 00-00 05 - 00-00	16-09-00 05-00-00		840			20 830	12-00-00 n/a

Controls Summary	Value	% Allowable	Duration	Load Case	Span Location
Pos. Moment	20540 ft-lbs	77.2%	125%	F	Transfer
End Shear	4669 lbs	47.3%	125%	5	1 - Internal 1 - Left
Total Load Defl.	L/194 (1.005")	92.6%	12070	5	1 - LOIL
Live Load Defl.	L/397 (0.493")	60.5%		5	1
Span / Depth	16.5	n/a		ŏ.	i
Bessing Supports			% Allow	% Allow	

	ing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
BO	Wall/Plate	3-1/2" x 3-1/2"	5299 lbs	101.8%	57.7%	Spruce-Pine-Fir
B1	Wall/Plate	3-1/2" x 3-1/2"	4607 lbs	88.5%	50.1%	Spruce-Pine-Fir

Cautions

Bearing length at bearing B0 should be at least 3-9/16".

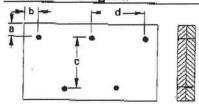
Bearing 80 cannot support a load of 5299 lbs.

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Member Slope = 0, consider drainage.

Connection Dlagram



a minimum = 2" c = 7-7/8"b minimum = 3" d = 12"

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record. . Member has no side loads,

Concentrated loads are not considered in side load analysis.

Pagedod are: 16d Common Nalls

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (888)234-0056 before installation.

BC CALC®, BC FRAMER® , AJS™ ALLJOIST®, BC RIM BOARDTM. BCI® BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Wood Products.



Prepared for:

C. K. CONTRACTING THE CUNNINGHAM RESIDENCE C.R. 18 \ WORTHINGTON SPRINGS

Ву:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED



SCHAFER ENGINEERING LLC

Trusses: Pre-engineered with manufacturer's required bracing system installed. Roof Sheathing: Type: Size: 7//C Fastener type nails: 8d/.113 Ring Shank
Interior zone spacing: Interior: 6 in. Periphery: f. in. Edge and end zone spacing: Interior: 6 in. Periphery: fin.
Top Double PI: Type: Spruce Grade: #1 #2 Size: 2 x 6 Nail spacing: 10 in.
Studs: Wood or Steel: Wood Type: Spruce Grade: #1 #2 Size: 2 x Interior Stud spacing: 16 in. Composite: (yes or no) Y End Stud spacing: 16 in. Composite: (yes or no) Y
Shear Wall Siding: Type: OSA Thickness: 7/16 in. 69 ft. Trans: Fastener: 80 / 131 Spacing: Int 8 in. Edge 4 in. 53 ft. Long: Fastener: 80 / 131 Spacing: Int 8 in. Edge 4 in.
Allowable Unit Shear on Shear Walls: 3/4 pounds per linear foot Unit Shear Transferred from Diaphragm: Trans: 86 Long: 83 Wall Tension Transferred by: Siding nails: 8d/131 @ 4 O.C. edges
Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2 in. Shape: L Washer: 2" Embedment: 7 in. Location of first anchor bolt from corner: 8 in.
Anchor Bolts @ 48" O.C. Model: A307 Loc. from corner: 8 in.
Type of Foundation: (1) - #5 rebar continuous required in bond beam. Floor Slab: 4 in. CMU: Size 8 x 16 in. Height: in. Reinf.: #5 at 72 in. Mionolithic Footing: Depth: 20 in. Bottom Width: 12 in. Reinf.: 2 # 5 pars
Footing: Width: Zo in. Depth: 10 in. Reinforcing: 2 # 5 pars Interior Footings: 16" W X 10" D Porch Columns: 6X6X 8 Syp# PT 684 Column Fasteners: Supro C866 CC66 OR Gara
Special Comments: Invall ceiling disphrage on front entry using same orde
NOTE: 1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail. 2. All trusses must bear on exterior walls & porch beams. 3. All walls to be nailed with same nailing pattern as shear walls. 4. This is a wind load only, NOT a structural analysis.
5. This wind load is not valid without a raised embosced soal.

8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.

6. It is assumed that ideal soil conditions and pad preparations are provided.

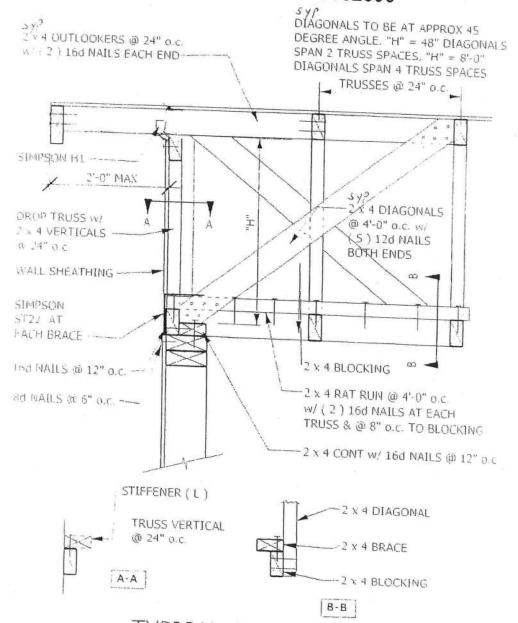
11. All headers over 12 feet to be pre-engineered.

7. Fiber mesh or WWM may be used in concrete slab.

48984 7104 NW 42ng Lr. Gainesville, Fi

SCHAFER ENGINEERING, LLC

7104 N. W. 42ND LANE GAINESVILLE, FLORIDA 32606



TYPICAL GABLE END BRACING

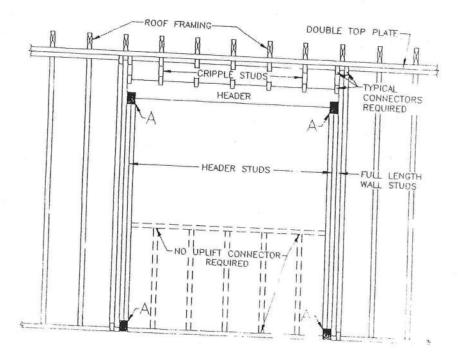
48984 7104 NW 42nd Ln Gainesville, FI

			Maximum Header Span (ft.)						
		3'	6'	9'	12'	15'	18		
		· Number o	of Heade	r Studs S	Supportir	ng End of	Неаде		
	7	11	1	2	2	2	2		
Unsupported Wall Height	Stud Spacing		Number of Full-Length Studs at Each End of Header						
	1 3			T	<u>T</u>		618		
10' or less greater	12 in. 16 in. 24 in.	2 2	2	3 3 2	3 2	3	3		

1 The header stud shall not be required if the header is supported by a suitable framing anchor.

Total connection
reconcerned at points A
for any borrow of
finater starts. Uplift
man per training
member apply the
finater from Table
in 4.1 or 30.14, as
interprete controlled
by the demoter of framing
members displaced

MEATE Tipliff connection is required at each end of treaties and at bottom in manual sturts in and from to connectors at Anii sturts and at top and and manual sturtes.



9

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.

Use proper bolt anchors.

Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.

Strap ridge beam at each end with minimum uplift resistance of 1000#.

It is the contractors' responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300

^{*}or per truss engineering

Use proper bolt anchors

All beams to be sheathed or strapped to Double Top Plate when applicable.

CRIPPLES | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS	
Wall sheathing nailing Adequate exterior walls bottom w/8d nails.	
Use SP1 & SP2 @32" O.C. on all interior non-sheathed bearing walls.	
Interior anchor bolts to be ½" x 8" A307 or ½" x 7" wedge anchor or equivalent.	

- ** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

 NOTE:
- 1. For nailing into SPF members, multiply table values by .86
- 2. See truss engineering for anchor tie-down values.

Wind Load Design per ASCE 7-02

User Input Data			
Structure Type	Building		
Basic Wind Speed (V)	110	mph	
Structural Category	11		
Exposure	В		
Struc Nat Frequency (n1)	1	Hz	
Slope of Roof (Theta)	22.6	Deg	
Type of Roof	Hipped		
Eave Height (Eht)	8.00	ft	
Ridge Height (RHt)	16.38	ft	
Mean Roof Height (Ht)	12.36	ft	
Width Perp. to Wind (B)	68.00	ft	
Width Parallel to Wind (L)	56.84	ft	
Damping Ratio (beta)	0.01		

Red values should be changed only through "N	vlain Menu"
--	-------------

Calculated Parameter	rs
Type of Structure	
Height/Least Horizontal Dim	0.22
Flexible Structure	No

Calculated Parameters			
Importance Factor	1		
Hurricane Prone F	Region (V>100 m	ph)	
Table Co	6-4 Values		
Alpha =	7.000		
zg =	1200.000		
At =	0.143		
117111111	0.143 0.840		
Bt =			
Bt = Am =	0.840		
Bt =	0.840 0.250		
Bt = Am = Bm =	0.840 0.250 0.450	ft	
Bt = Am = Bm =	0.840 0.250 0.450 0.300	ft	

	Gust Factor Category I: Rigid Structures - Simplified Me	thod	
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
	Gust Factor Category II: Rigid Structures - Complete Ana	lysis	
Zm	Zmin	30.00	ft
lzm	Cc * (33/z)^0.167	0.3048	
Lzm	I*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8877	
Gust2	0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	0.8587	
	Gust Factor Category III: Flexible or Dynamically Sensitive St	tructures	
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.80	
Nb	4.6*NatFreq*B/Vzm	4.41	
Nd	15.4*NatFreq*Depth/Vzm	12.35	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6258	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2010	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0777	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.6270	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*lzm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*lzm))	1.03	

Gust Factor Summary			
Main Wind-force resisting system: Components and Claddir			dding:
Gust Factor Category:	1	Gust Factor Category:	1
Gust Factor (G)	0.86	Gust Factor (G)	0.86

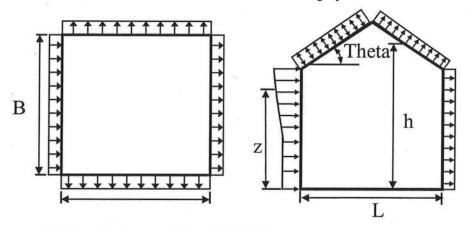
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz lb/ft^2	Pressure (lb/ft^2	
ft			1.00		Windwa +GCpi	rd Wall* -GCpi
16.38	0.70	1.00	1.00	21.70	11.70	18.11
15	0.70	1.00	1.00	21.70	11.70	18.11

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Ср
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coeffic	ients, Cp
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Ср	Pressure	(psf)
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 68 ft wall)	-0.50	-10.85	-4.44
Leeward Walls (Wind Dir Parallel to 56.84 ft wall)	-0.46	-10.25	-3.84
Side Walls	-0.70	-13.91	-7.50
Roof - Normal to Ridge (Ti	heta>=10)		
Windward - Max Negative	-0.25	-7.00	-0.59
Windward - Max Positive	0.25	0.65	7.06
Leeward Normal to Ridge	-0.60	-12.38	-5.97
Overhang Top	-0.25	-3.79	-3.79
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (A	All Theta)		-
Dist from Windward Edge: 0 ft to 6.18 ft	-0.90	-16.96	-10.55
Dist from Windward Edge: 6.18 ft to 12.36 ft	-0.90	-16.96	-10.55
Dist from Windward Edge: 12.36 ft to 24.72 ft	-0.50	-10.85	-4.44

Wind Load Design per ASCE 7-02

Dist from Windward Edge: > 24.72 ft

-0.30

-7.79 -1.38

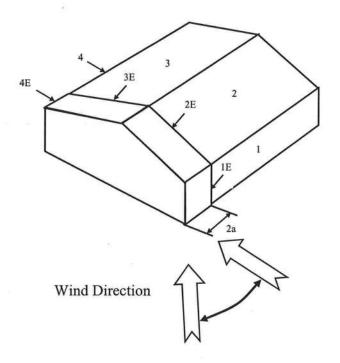
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	. =	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

	100		Case /	A	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.54	0.18	-0.18	21.70	7.76	15.58
2	-0.46	0.18	-0.18	21.70	-13.80	-5.99
3	-0.47	0.18	-0.18	21.70	-14.04	-6.23
4	-0.41	0.18	-0.18	21.70	-12.90	-5.09
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.77	0.18	-0.18	21.70	12.83	20.65
2E	-0.72	0.18	-0.18	21.70	-19.57	-11.75
3E	-0.65	0.18	-0.18	21.70	-17.98	-10.16
4E	-0.60	0.18	-0.18	21.70	-16.89	-9.08
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

^{*} p = qh * (GCpf - GCpi)



^{*} Horizontal distance from windward edge

Wind Load Design per ASCE 7-02

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

			Case	3			
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)	
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86	
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07	
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12	
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86	
5	0.40	0.18	-0.18	21.70	4.77	12.59	
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39	
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51	
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31	
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60	
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51	
5E	0.61	0.18	-0.18	21.70	9.33	17.14	
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43	

^{*} p = qh * (GCpf - GCpi)

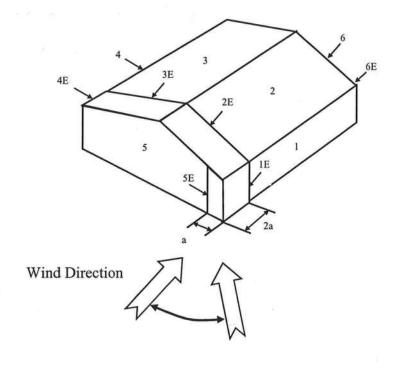
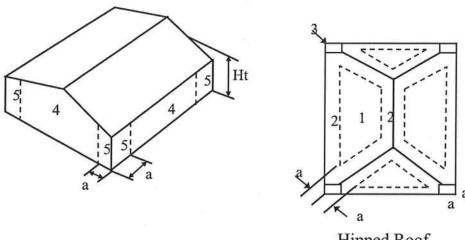


Figure 6-5 - External Pressure Coefficients, GCp

Wind Load Design per ASCE 7-02



Hipped Roof 10 < Theta <= 30

a = 4.944 ==> 4.94 ft

Component	Width	Length	Area	Zone	G	Ср	Wind Pres	ss (lb/ft^2
The Control of the property of the Control of the	(ft)	(ft)	(ft^2)	A 10.07 (10.00 (10.00)	Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00		-			
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
11 11 112	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -

Wind Load Design per ASCE 7-02

Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18
Partially Enclosed Buildings	0.55	-0.55
Open Buildings	0.00	0.00

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) =

0.3

100			Ср	
Condition	Variable	Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Ср	0.13	-1	-0.5
•	P (+GCpi) - psf	-1.29	-18.49	-10.85
	P (-GCpi) -psf	5.12	-12.08	-4.44
Roof Springing from Ground	Ср	0.42	-1	-0.5
	P (+GCpi) - psf	3.22	-18.49	-10.85
	P (-GCpi) -psf	3.22	-18.49	-10.85

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	56.84	ft
В	Roof dimension parallel to wind direction	68.00	ft
L/B	Ratio of L to B	0.836	
Theta	Slope of Roof	22.6	Deg
Cf	Force Coefficient	1.04	+
X	Distance to center of pressure from windward edge	0.35	ft

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Exam	MENTS: Two (2) complete sets of plans containing the following: niner
DZ		All drawings must be clear, concise and drawn to scale ("Optional"
9		details that are not used shall be marked void or crossed off). Square
/		footage of different areas shall be shown on plans.
B		Designers name and signature on document (FBC 106.1). If licensed
/		architect or engineer, official seal shall be affixed.
e		Site Plan including:
		a) Dimensions of lot
		 b) Dimensions of building set backs
		 Location of all other buildings on lot, well and septic tank if
		applicable, and all utility easements.
1		 d) Provide a full legal description of property.
Ø		Wind-load Engineering Summary, calculations and any details required
		Plans or specifications must state compliance with FBC Section 1609.
BUILDING D		The following information must be shown as per section 1603.1.4 FBC
ived	E.	 Basic wind speed (3-second gust), miles per hour (km/hr).
gece!	131	 Wind importance factor, Iw, and building classification from Table
Received of Code Complian	一一	1604.5 or Table 6-1, ASCE 7 and building classification in Table
TEU	731	1-1, ASCE 7.
Combijau	000	c. Wind exposure, if more than one wind exposure is utilized, the
nsilan	791	wind exposure and applicable wind direction shall be indicated.
COMMENT	SHIP!	d. The applicable enclosure classifications and, if designed with
PLANS EXP		ASCE 7, internal pressure coefficient.
		e. Components and Cladding. The design wind pressures in terms of
		psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design
		professional.
		Elevations including:
	0	a) All sides
	0	b) Roof pitch
D/	0	다 아이트 마스 아이트 마스트 마스트 이 사람들이 보고 있다. 그는 아이트 아이트 아이트 이 사람들이 되었다. 그는 이 아이트 아이트 아이트 아이트 아이트 아이트 아이트 아이트 아이트 아
2	П	 c) Overhang dimensions and detail with attic ventilation

3 2 3		
B'		d) Location, size and height above roof of chimneys.
		e) Location and size of skylights
1		f) Building height
W		e) Number of stories
/		Floor Plan including:
2		a) Rooms labeled and dimensioned.
		b) Shear walls identified.
©/		c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
e e		d) Show safety glazing of glass, where required by code.
e		e) Identify egress windows in bedrooms, and size.
ď		f) Fireplace (gas vented), (gas non-vented) of wood burning with hearth (Please circle applicable type).
B		 g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
Ħ		h) Must show and identify accessibility requirements (accessible bathroom)
		Foundation Plan including:
D/		a) Location of all load-bearing wall with required footings indicated as standard
,		or monolithic and dimensions and reinforcing.
ď	. 🛮	b) All posts and/or column footing including size and reinforcing
		c) Any special support required by soil analysis such as piling
		d) Location of any vertical steel.
,		Roof System:
Q		a) Truss package including:
		 Truss layout and truss details signed and sealed by Fl. Pro. Eng.
		Roof assembly (FBC 106.1.1.2)Roofing system, materials,
		manufacturer, fastening requirements and product evaluation with
/		wind resistance rating)
		b) Conventional Framing Layout including:
		 Rafter size, species and spacing
		2. Attachment to wall and uplift
		3. Ridge beam sized and valley framing and support details
		 Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
		Wall Sections including:
TB/		a) Masonry wall
G		All materials making up wall
		Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall be
		designed by a Windload engineer using the engineered roof truss
		plans.
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating)
		7. Fire resistant construction (if required)
		8. Fireproofing requirements9. Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
2		a. Vapor retarder (6mil. Polyethylene with joints lapped 6
E		inches and sealed)
		b. Must show control joints, synthetic fiber reinforcement or
		Welded fire fabric reinforcement and supports
		11. Indicate where pressure treated wood will be placed
		12. Provide insulation R value for the following:

		a. Attic spaceb. Exterior wall cavityc. Crawl space (if applicable)
r r	0	 b) Wood frame wall All materials making up wall Size and species of studs Sheathing size, type and nailing schedule Headers sized Gable end showing balloon framing detail or gable truss and wall hinge bracing detail All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
		 Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) Fire resistant construction (if applicable) Fireproofing requirements Show type of termite treatment (termiticide or alternative method) Slab on grade a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports Indicate where pressure treated wood will be placed Provide insulation R value for the following: a. Attic space b. Exterior wall cavity
0		 c. Crawl space (if applicable) c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)
		Floor Framing System:
		a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
		b) Floor joist size and spacing
		c) Girder size and spacing
		d) Attachment of joist to girder
		e) Wind load requirements where applicable
Ø		Plumbing Fixture layout Electrical layout including:
		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
		b) Ceiling fans
		c) Smoke detectors
		d) Service panel and sub-panel size and location(s)
		e) Meter location with type of service entrance (overhead or underground)
1		f) Appliances and HVAC equipment
	0	g) Arc Fault Circuits (AFCI) in bedrooms
M		h) Exhaust fans in bathroom
		HVAC information
E C	. 0	a) Energy Calculations (dimensions shall match plans)
D D		b) Manual J sizing equipment or equivalent computation
1		c)Gas System Type (LP or Natural) Location and BTU demand of equipment
M		Disclosure Statement for Owner Builders
<u>n</u>		*** Notice Of Commencement Required Before Any Inspections Will Be Done
M		Private Potable Water

Load Short Form Entire House LARRY RESMONDO AIR CONDITIONING

Job: CUNNINGHAM RESIDE...

Date: Aug 12, 2008

Project Information

For:

JERRY CUNNINGHAM

Design Information						
Htg Clg Infiltration						
Outside db (°F)	33	92	Method	Simplified		
Inside db (°F)	70	75	Construction quality	Average		
Design TD (°F)	37	17	Fireplaces	1 (Semi-tight)		
Daily range	-	M	6-11/2000	,		
Inside humidity (%)	-	50				
Moisture difference (gr/lb)	9	52				

HEATING EQUIPMENT

COOLING EQUIPMENT

Make Trade Model	Ruud Ruud UPNL Series UPNL-048J*Z			Make Trade Cond Coil	Ruud Ruud UPNL Series UPNL-048J*Z UHSL-HM4821+R		1A*
Efficiend	су	8.5 HSPF		Efficiency		13 SEER	
Heating	input			Sensible c	ooling	32900	Btuh
Heating	output	46500	Btuh @ 47°F	Latent coo	ling	14100	Btuh
Temper	ature rise	27	°F	Total cooli	ng	47000	Btuh
Actual a	ir flow	1567	cfm	Actual air t	flow	1567	cfm
Air flow	factor	0.042	cfm/Btuh	Air flow fac	ctor	0.050	cfm/Btuh
Static pr	ressure	0.10	in H2O	Static pres	sure	0.10	in H2O
	hermostat				ible heat ratio	0.82	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
LAUNDRY	83	1617	4637	68	234
PANTRY	26	578	196	24	10
MASTER BATH	163	2341	1345	99	68
W.I.C./HALL	69	834	354.0	UNTY BUY 35	18
M/BEDROOM	232	6488	3843	075	194
NOOK	48	2425	1656	Received 103	83
KITCHEN	299	490	4723	4D1	238
DINING	132	3283	2058	139	104
GREAT ROOM	504	4122	3616	00,175	182
ENTRY	42	1776	1216	0de 75/	61
STUDY	132	3087	2021	(lano)131	102
BEDROOM 3	167	4913	2828	208	142
BATH/HALL	66	952	474	MINER 40	24
BEDROOM 2	167	4101	2145	174	108

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



Entire House Other equip loads Equip. @ 0.97 RSM Latent cooling	d 2128	37006 1930	31112 887 31038 7099	1567	1567
TOTALS	2128	38936	38137	1567	1567

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Building Analysis Entire House LARRY RESMONDO AIR CONDITIONING

Job: CUNNINGHAM RESIDE...

Date: Aug 12, 2008

By:

Project Information

For:

JERRY CUNNINGHAM

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 0 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 1 (Semi-tight)	75 17 50 51.6

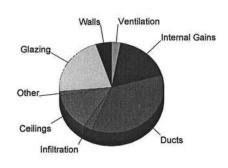
Heating

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	1.1 29.8 14.4 1.2 4.1 3.0	3155 7519 606 2520 8718 4225 10264 0 1930 0	8.1 19.3 1.6 6.5 22.4 10.9 26.4 0.0 5.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments	0.6 27.2 11.4 2.0 0.0 0.6	1599 6846 477 4266 0 849 11354 887 5720	5.0 21.4 1.5 13.3 0.0 2.7 35.5 2.8 17.9 0.0
Total		31998	100.0



Overall U-value = 0.136 Btuh/ft2-°F

Data entries checked.

Project Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: CUNNINGHAM RESIDE...

Date: Aug 12, 2008

By:

Project Information

For:

JERRY CUNNINGHAM FL

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Summer Design Conditions

Outside db Inside db Design TD	33 70 37	°F °F	Outside db Inside db Design TD	92 75 17	°F °F °F
			Daily range Relative humidity Moisture difference	M 50 52	% gr/lb

Heating Summary

Sensible Cooling Equipment Load Sizing

Structure	26742	Btuh	Structure	19758	Btuh
Ducts	10264	Btuh	Ducts	11354	Btuh
Central vent (47 cfm)	1930	Btuh	Central vent (47 cfm)	887	Btuh
Humidification	0	Btuh	Blower `	0	Btuh
Piping	0	Btuh			
Piping Equipment load	38 936	Btuh	Use manufacturer's data	n	
AND CONTROL OF CONTROL AND CON			Rate/swing multiplier	0.97	
Infiltration	on		Equipment sensible load	31038	Btuh

intiltration

Method Construction quality	Simplified Average	Latent Cooling E
Fireplaces	1 (Semi-tight)	Structure Ducts

5 125.7	Heating	Cooling
Area (ft²)	2128	2128
Area (ft²) Volume (ft³)	170 25	17025
Air changes/hour	0.37	0.16
Equiv. AVF (cfm)	104	45

Equipment Load Sizing

Structure Ducts Central vent (47 cfm) Equipment latent load	2392 3045 1662 7099		
Equipment total load	38137	Btuh	
Req. total capacity at 0.70 SHR	3.7	ton	

Heating Equipment Summary

Make	Ruud	Make
Trade	Ruud UPNL Series	Trade
Model	UPNL-048J*Z	Cond

Efficiency Heating input	8.5	HSPF
Heating output	46500	Btuh @ 47°F
Temperature rise Actual air flow	27 1567	°F cfm
Air flow factor	0.042	cfm/Btuh
Static pressure Space thermostat	0.10	in H2O

Cooling Equipment Summary

Make Trade Cond	Ruud Ruud UPNL Series UPNL-048J*Z		
Coil	UHSL-HM4821+RCS	SI -H*4821A*	
Efficient Sensible Latent of Total co Actual a Air flow	cy e cooling cooling poling air flow factor		BEER Btuh Btuh Btuh cfm cfm/Btuh in H2O
Load se	ressure ensible heat ratio	0.82	

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Duct System Summary Entire House LARRY RESMONDO AIR CONDITIONING

Job: CUNNINGHAM RESIDE...

Date: Aug 12, 2008

By:

Project Information

For:

JERRY CUNNINGHAM

	He	eating		C	ooling
External static pressure	0.10	in H2O		0.10	in H2O
Pressure losses	0.25	in H2O		0.25	in H2O
Available static pressure	-0.2	in H2O		-0.2	in H2O
Supply / return available pressure	-0.10 / -0.05	in H2O		-0.10 / -0.05	in H2O
Lowest friction rate	0.100	in/100ft		0.100	in/100ft
Actual air flow	1567	cfm		1567	cfm
Total effective length (TEL)			335	ft	

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
LAUNDRY-A	С	2318	34	117	0.100	7	12×4	VIFx	215.0	0.0	ST1
LAUNDRY	С	2318	34	117	0.100	7	12x4	VIFx	215.0	0.0	ST1
PANTRY	h	578	24	10	0.100	4	12 x 1	VIFx	215.0	0.0	ST1A
MASTER BATH	h	2341	99	68	0.100	7	12x3	VIFx	215.0	0.0	ST1
W.I.C./HALL	h	834	35	18	0.100	4	12x1	VIFx	215.0	0.0	ST1
M/BEDROOM	h	6488	275	194	0.100	10	12x7	VIFx	215.0	0.0	ST1
NOOK	h	2425	103	83	0.100	7	12×4	VIFx	215.0	0.0	ST1
KITCHEN-A	c	2362	10	119	0.100	7	12×4	VIFx	215.0	0.0	ST1
KITCHEN	С	2362	10	119	0.100	7	12×4	VIFx	215.0	0.0	ST1
DINING	h	3283	139	104	0.100	8	12x5	VIFx	215.0	0.0	ST1
GREAT ROOM	С	3616	175	182	0.100	8	12x5	VIFx	215.0	0.0	ST1
ENTRY	h	1776	75	61	0.100	6	12x3	VIFx	215.0	0.0	ST1
STUDY	h	3087	131	102	0.100	7	12×4	VIFx	215.0	0.0	ST1
BEDROOM 3	h	4913	208	142	0.100	9	12x6	VIFx	215.0	0.0	ST1
BATH/HALL	h	952	40	24	0.100	4	12x1	VIFx	215.0	0.0	ST1
BEDROOM 2	h	4101	174	108	0.100	8	12x5	VIFx	215.0	0.0	ST1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cím)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
ST1	Peak AVF	1567	1567	0.100	830	18	16 x 17	RectFbg	ST1
ST1A	Peak AVF	24	10	0.100	220	10	16 x 1	RectFbg	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSi (in)	ze	Stud/Joist Opening (in)	Duct Matl	Trunk
RB2 RB3 RB4 RB5	0x0 0x0 0x0 0x0	275 175 208 174	194 182 142 108	120.0 120.0 120.0 120.0	0.100 0.100 0.100 0.100	468 428	10 8 9 8	14x 14x 14x 14x	6 4 5 4		VIFx VIFx VIFx VIFx	



RE: CUNRES -

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Project Customer: JERRY CUNNINGHAM Project Name: CUNNINGHAM RES

Lot/Block: 0

Subdivision: HAMMOCK HILL

Address:

City: FT WHITE

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address: City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 7.0

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 40.0 psf

This package includes 45 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E5008799	A1	8/26/08	18	E5008816	A7	8/26/08
2	E5008800	A10	8/26/08	19	E5008817	A8	8/26/08
3	E5008801	A11	8/26/08	20	E5008818	A9	8/26/08
4	E5008802	A12	8/26/08	21	E5008819	В	8/26/08
5	E5008803	A13	8/26/08	22	E5008820	B1	8/26/08
6	E5008804	A14	8/26/08	23	E5008821	B2	8/26/08
7	E5008805	A15	8/26/08	24	E5008822	B3	8/26/08
8	E5008806	A16	8/26/08	25	E5008823	BET	8/26/08
9	E5008807	A17	8/26/08	26	E5008824	C	8/26/08
10	E5008808	A18	8/26/08	27	E5008825	C1	8/26/08
11	E5008809	A19	8/26/08	28	E5008826	C2	8/26/08
12	E5008810	A2	8/26/08	29	E5008827	CJ01	8/26/08
13	E5008811	A20	8/26/08	30	E5008828	CJ09	8/26/08
14	E5008812	A3	8/26/08	31	E5008829	D	8/26/08
15	E5008813	A4	8/26/08	32	E5008830	DET	8/26/08
16	E5008814	A5	8/26/08	33	E5008831	EET	8/26/08
17	E5008815	A6	8/26/08	34	E5008832	EGT	8/26/08



The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Marvin A. Strzyzewski, FL Lic. #43144 Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

RE: CUNRES -

Site Information:

Project Customer: JERRY CUNNINGHAM Project Name: CUNNINGHAM RES Lot/Block: 0 Subdivision: HAMMOCK HILL

Lot/Block: 0 Address:

State: FL City: FT WHITE

No.	Seal#	Truss Name	Date
35	E5008833	EJ7	8/26/08
36	E5008834	EJ7A	8/26/08
37	E5008835	J01	8/26/08
38	E5008836	J01A	8/26/08
39	E5008837	J01B	8/26/08
40	E5008838	J07	8/26/08
41	E5008839	J07A	8/26/08
42	E5008840	J07B	8/26/08
43	E5008841	J1	8/26/08
44	E5008842	J3	8/26/08
45	E5008843	J5	8/26/08

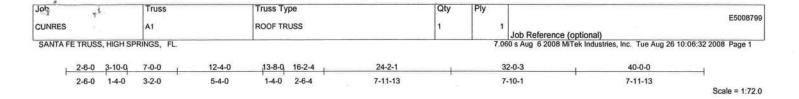
GEO-TECH, INC.

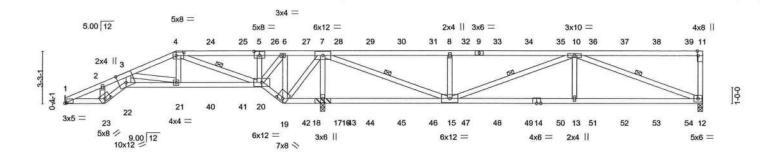
Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

#29450

FIELD DENSITY WORKSHEET

CLIENT JESTY CUNNING	Hans				DATE	12-10	0-08	
					PROJE	CT NO		
PROJECT NAME CUNNING HAM					PERMI	T NO		
EARTH CONTRACTOR					TESTE	D BY DI	1	
COMPACTION REQUIREMENT (%) 95		□ Sta	andard Fodified P					CONTACT
TOTAL ON-SITE TIME								OONTAGT
□ Limerock □ Subgrade □ Pipe Backfill ☑	/		/					
	LAB PR	OCTOR	TEOT	PROPE		WET	DRY	
TEST LOCATION	DENS.	ОМС	TEST DEPTH	PROBE DEPTH	MOIST.	DENSITY (PCF)	DENSITY (PCF)	COMP.
		H.	2745	0		2		
W Side of Pad	1090	10.0	16	12"	7-00°C	113.3	106.8	98.0
Content of N fty	1	1	1	1	8.8	117.6	108.1	99.2
11 Jan 11 5 11	1		28	1	717	115.2	107.5	98.6
cantes of gad	1	+	V		6.9	114.0	1066	97,8
			-9.					
	N = 22221 A.S							
	- 1							
REMARKS						mini requ ** Rete dens obta () Cliei	sity failed to mum project irement est indicated sity required ined. Int is aware atisfactory to	s minimum ment was





-	2-6-0	3-10-0	7-0-0 12-4	13-8-0	10-2-4		24-2-1		32-0-3		40-0-0	
20	2-6-0	1-4-0	3-2-0 5-4	-0 1-4-0	2-6-4		7-11-13		7-10-1	3	7-11-13	,
Plate Of	fsets (X	(,Y): [1:	0-1-6,0-0-2], [4:0-5-12	,0-2-8], [5:0-4-0	,0-3-0], [1	1:0-3-8,Edge	e], [19:0-4-8,0-1-1	2], [22:0-5-	8,0-7-0], [2	3:0-4-8,0-1-	12]	
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in (lo	c) I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.98	Vert(LL)	-0.16 13-1	5 >999	240	MT20	244/190
TCDL	10.0		Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.50 13-1	5 >569	180		
BCLL	0.0		Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.16	2 n/a	n/a	1	
BCDL	10.0		Code FBC2004/T	PI2002	(Mat	rix)					Weight: 206	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except* 5-9: 2 X 4 SYP 2400F 2.0E

BOT CHORD 2 X 4 SYP No.2D WEBS

2 X 4 SYP No.3 *Except* 11-12,7-15: 2 X 4 SYP No.2D BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 4-4-5 oc purlins, except end verticals.

Rigid ceiling directly applied or 3-0-14 oc bracing. 1 Row at midpt 4-20, 7-15, 10-15, 10-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=533/Mechanical, 12=1564/0-3-8, 17=4420/0-4-7 (0-3-8 + bearing block)

Max Horz 1=95(LC 5)

Max Uplift1=-63(LC 5), 12=-430(LC 3), 17=-984(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1517/203, 2-3=-1567/237, 3-4=-1091/154, 4-24=-300/1371, 24-25=-300/1371, 5-25=-300/1371, 5-26=-300/1371. TOP CHORD

6-26=-300/1371, 6-27=-389/1920, 7-27=-389/1920, 7-28=-1571/551, 28-29=-1571/551, 29-30=-1571/551, 30-31=-1571/551, 8-31=-1571/551, 8-32=-1571/551, 9-32=-1571/551, 9-33=-1571/551, 33-34=-1571/551.

34-35=-1571/551, 10-35=-1571/551, 10-36=-266/69, 36-37=-266/69, 37-38=-266/69, 38-39=-266/69, 11-39=-266/69,

11-12=-480/202

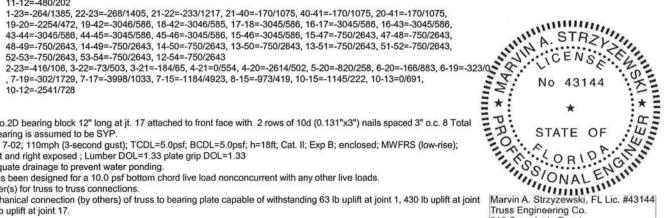
BOT CHORD 1-23=-264/1385, 22-23=-268/1405, 21-22=-233/1217, 21-40=-170/1075, 40-41=-170/1075, 20-41=-170/1075,

NOTES

WEBS

- 1) 2 X 4 SYP No.2D bearing block 12" long at jt. 17 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1, 430 lb uplift at joint 12 and 984 lb uplift at joint 17.

Continued on page 2



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

August 26,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI:7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Edenton, NC 27932

Jolé +	Truss	Truss Type	Qty	Ply	E5008799
CUNRES	A1	ROOF TRUSS	1	1	E0008/99
					Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL.

7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:32 2008 Page 2

NOTES

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 326 lb down and 105 lb up at 7-0-0, 164 lb down and 56 lb up at 9-0-12, 164 lb down and 56 lb up at 11-0-12, 164 lb down and 56 lb up at 13-0-12, 125 lb down and 73 lb up at 15-0-12, 125 lb down and 73 lb up at 19-0-12, 125 lb down and 73 lb up at 21-0-12, 125 lb down and 73 lb up at 29-0-12, 125 lb down and 73 lb up at 29-0-12, 125 lb down and 73 lb up at 29-0-12, 125 lb down and 73 lb up at 31-0-12, 125 lb down and 73 lb up at 33-0-12, 125 lb down and 73 lb up at 35-0-12, and 125 lb down and 73 lb up at 39-0-12 on top chord, and 254 lb down and 13 lb up at 7-0-0, 22 lb down at 9-0-12, 22 lb down at 11-0-12, 94 lb down at 15-0-12, 94 lb down at 17-0-12, 94 lb down at 13-0-12, 94 lb down at 33-0-12, 94 lb down at

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-11=-60, 1-23=-20, 22-23=-20, 20-22=-20, 19-20=-20, 12-19=-20

Concentrated Loads (lb)

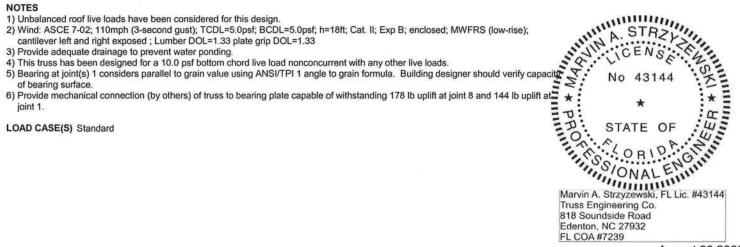
Vert: 4=-326(F) 21=-254(F) 24=-164(F) 25=-164(F) 26=-164(F) 27=-125(F) 28=-125(F) 29=-125(F) 30=-125(F) 31=-125(F) 32=-125(F) 33=-125(F) 34=-125(F) 35=-125(F) 36=-125(F) 36=-125(F) 37=-125(F) 39=-125(F) 39=-125(F) 40=-11(F) 41=-11(F) 42=-47(F) 43=-47(F) 44=-47(F) 45=-47(F) 46=-47(F) 47=-47(F) 48=-47(F) 49=-47(F) 50=-47(F) 51=-47(F) 52=-47(F) 53=-47(F) 54=-47(F) 54



Edenton, NC 27932

Truss Type Qty Joh Truss E5008800 CUNRES A10 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:33 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 17-6-0 6-8-0 37-0-0 6-8-0 5-5-0 5-5-0 4-8-3 6-4-13 8-4-15 Scale = 1:68.0 5x5 = 5.00 12 5x5 = 4x12 = 4x6 = 3x4 = 5 3 5x5 = 6-7-7 13 7x8 = 12 14 4x5 = 11 10 5x8 = 2.25 12 3x6 = 3x10 = 2x4 || 5x6 = 9-8-0 17-6-0 22-2-3 29-5-6 37-0-0 9-8-0 7-10-0 4-8-3 7-3-2 5-1-2 2-5-8 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [14:0-4-0,0-3-4] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl **PLATES** GRIP (loc) L/d TCLL 20.0 1.25 TC 0.88 Vert(LL) -0.42 13-14 >999 240 244/190 Plates Increase MT20 -1.10 13-14 TCDL 1.25 BC 0.80 180 10.0 Lumber Increase Vert(TL) >400 BCLL 0.0 Rep Stress Incr YES WB 0.77 Horz(TL) 0.56 8 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 193 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-3-6 oc purlins, except 2 X 4 SYP No.2D *Except* **BOT CHORD** 1-14: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 8-9-2 oc bracing. **WEBS** 2 X 4 SYP No.3 WEBS 7-8, 6-9 1 Row at midpt MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1465/0-5-8, 1=1465/0-5-8 Max Horz 1=197(LC 5) Max Uplift8=-178(LC 6), 1=-144(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-5236/649, 2-3=-4836/518, 3-4=-3623/352, 4-5=-3608/364, 5-6=-4175/426, 6-7=-564/54, 7-8=-1407/164 **BOT CHORD** 1-14=-753/4840, 13-14=-544/4181, 12-13=-446/4316, 11-12=-272/2340, 10-11=-278/2322, 9-10=-277/2345, 8-9=-21/60 WEBS 2-14=-384/205, 3-14=-20/597, 3-13=-888/228, 4-13=-190/2409, 5-13=-1047/178, 5-12=-1205/174, 6-12=-197/2100, 6-10=0/268, 6-9=-2331/288, 7-9=-84/1275

NOTES

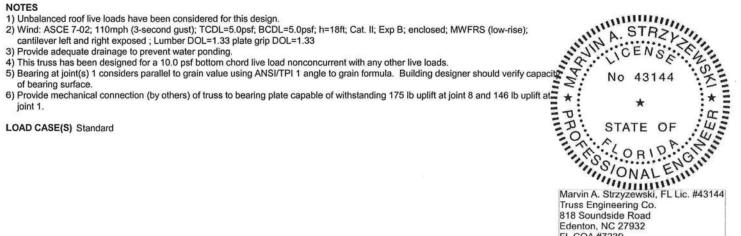


August 26,2008

818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty E5008801 CUNRES ROOF TRUSS A11 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:34 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 23-7-15 29-6-12 37-0-0 6-8-0 5-5-0 5-5-0 6-1-15 5-10-12 Scale = 1:68.0 5x5 = 5.00 12 4 3x4 = 5x5 = 4x12 = 4x5 = 5 6 7 5x5 = 6-7-7 5-0-12 13 7x8 = 12 11 14 5x8 = 3x6 = 10 2.25 12 3x10 = 2x4 || 5x6 = 34-6-8 9-8-0 17-6-0 23-7-15 30-2-4 37-0-0 9-8-0 7-10-0 6-1-15 6-6-4 4-4-4 2-5-8 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [14:0-4-0,0-3-4] LOADING (psf) SPACING 2-0-0 DEFL I/defl L/d **PLATES** GRIP (loc) 20.0 TC -0.43 TCLL Plates Increase 1.25 0.71 Vert(LL) 13 >999 240 MT20 244/190 TCDL 10.0 1.25 BC 0.84 Vert(TL) -1.12 13-14 >391 180 Lumber Increase BCLL 0.0 Rep Stress Incr YES WB 0.97 0.59 8 Horz(TL) n/a n/a Code FBC2004/TPI2002 (Matrix) BCDL 10.0 Weight: 190 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-3-6 oc purlins, except **BOT CHORD** 2 X 4 SYP No.2D *Except* end verticals. 1-14: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 8-10-14 oc bracing. WEBS 2 X 4 SYP No.3 WEBS 7-8, 6-9 1 Row at midpt MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1465/0-5-8, 1=1465/0-5-8 Max Horz 1=185(LC 5) Max Uplift8=-175(LC 6), 1=-146(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-5237/648, 2-3=-4832/516, 3-4=-3637/350, 4-5=-3645/359, 5-6=-4508/464, 6-7=-637/62, 7-8=-1408/167 **BOT CHORD** 1-14=-739/4840, 13-14=-527/4177, 12-13=-489/4674, 11-12=-266/2360, 10-11=-275/2353, 9-10=-275/2364, 8-9=-19/55 2-14=-386/206, 3-14=-22/590, 3-13=-880/226, 4-13=-168/2369, 5-13=-1315/228, 5-12=-1377/196, 6-12=-220/2434, **WEBS** 6-10=0/230, 6-9=-2319/282, 7-9=-100/1333

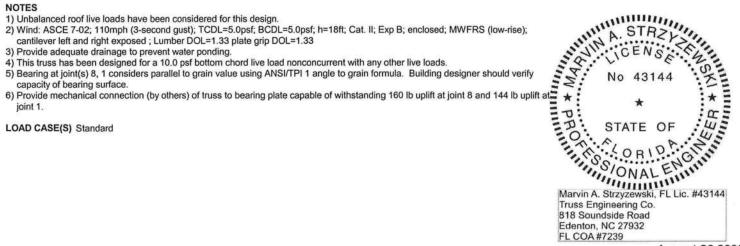
NOTES



FL COA #7239

Qty Job Truss Truss Type E5008802 ROOF TRUSS CUNRES A12 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:35 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 12-1-0 25-1-11 30-6-5 35-4-0 5-5-0 5-5-0 7-7-11 5-4-10 6-8-0 4-9-11 Scale: 3/16"=1" 5x5 = 4 5.00 12 3x4 = 5x5 = 3 4x8 = 2x4 || 6 7 5x5 = 11 7x8 = 10 12 6x6 = 4x6 = 2.25 12 3x6 = 3x10 = 8 4x6 = 9-8-0 17-6-0 25-1-11 35-4-0 9-8-0 7-10-0 7-7-11 10-2-5 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [12:0-3-0,Edge] SPACING **PLATES GRIP** LOADING (psf) 2-0-0 DEFL in (loc) I/defl L/d TC TCLL 20.0 Plates Increase 1.25 0.95 Vert(LL) -0.39 11-12 >999 240 MT20 244/190 TCDL 1.25 BC 0.84 Vert(TL) -1.02 11-12 >413 180 10.0 Lumber Increase BCLL 0.0 Rep Stress Incr YES WB 0.91 Horz(TL) 0.57 8 n/a n/a Code FBC2004/TPI2002 (Matrix) Weight: 172 lb BCDL 10.0 LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-4-13 oc purlins, except BOT CHORD 2 X 4 SYP No.2D *Except* 1-12: 2 X 4 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 9-2-1 oc bracing. **WEBS** 2 X 4 SYP No.3 WEBS 1 Row at midpt 5-11, 6-8 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 8=1398/0-7-12, 1=1398/0-5-8 Max Horz 1=172(LC 5) Max Uplift8=-160(LC 6), 1=-144(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=-4959/632, 2-3=-4540/498, 3-4=-3349/334, 4-5=-3383/342, 5-6=-4224/417, 6-7=-82/0, 7-8=-139/45 TOP CHORD 1-12=-712/4582, 11-12=-497/3893, 10-11=-447/4404, 9-10=-204/1706, 8-9=-220/1688 **BOT CHORD** WEBS 2-12=-395/208, 3-12=-24/597, 3-11=-877/221, 4-11=-145/2097, 5-11=-1314/229, 5-10=-1509/236, 6-10=-230/2824, 6-8=-2171/297

NOTES





Qty Job Truss Truss Type E5008803 CUNRES ROOF TRUSS 3 A13 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:36 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 17-6-0 35-0-0 6-8-0 5-5-0 5-5-0 5-5-0 5-5-0 6-8-0 Scale = 1:62.9 4x5 = 5.00 12 4 3x4 = 3x4 > 5 5x5 = 5x5 > 5x8 = 10 8 6x6 = 6x6 = 2.25 12 3x10 = 3x10 | 3x10 = 4x12 \\ 1-6-12 9-8-0 17-6-0 25-4-0 35-0-0 7-10-0 7-10-0 1-6-12 8-1-4 9-8-0 Plate Offsets (X,Y): [1:0-1-4,Edge], [1:0-1-4,Edge], [1:0-0-8,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [7:0-4-12,Edge], [8:0-3-0,Edge], [10:0-3-0,Edge] SPACING DEFL **PLATES** GRIP LOADING (psf) 2-0-0 (loc) l/defl L/d TCLL 20.0 TC 240 Plates Increase 1.25 1.00 Vert(LL) -0.43>969 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.75 Vert(TL) -1.09 9-10 >379 180 BCLL 0.0 Rep Stress Incr YES WB 0.68 Horz(TL) 0.63 n/a n/a Code FBC2004/TPI2002 10.0 (Matrix) Weight: 162 lb BCDL BRACING LUMBER TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-5-2 oc purlins. **BOT CHORD** 2 X 4 SYP No.2D *Except* **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. 1-10,7-8: 2 X 4 SYP 2400F 2.0E MiTek recommends that Stabilizers and required cross bracing WEBS 2 X 4 SYP No.3 be installed during truss erection, in accordance with Stabilizer WEDGE Installation guide.

Left: 2 X 8 SYP No.2

REACTIONS (lb/size) 7=1382/0-5-8, 1=1382/0-5-8

Max Horz 1=87(LC 5)

Max Uplift7=-149(LC 6), 1=-149(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-4885/583, 2-3=-4472/450, 3-4=-3253/277, 4-5=-3253/288, 5-6=-4472/379, 6-7=-4885/515

BOT CHORD

1-10=-581/4513, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4513

WEBS

2-10=-394/208, 3-10=-25/596, 3-9=-889/231, 4-9=-120/2112, 5-9=-889/233, 5-8=-28/596, 6-8=-394/212

NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 7 and 149 lb uplift at
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard



FL COA #7239



Joh Truss Truss Type Qty E5008804 CUNRES A14 ROOF TRUSS 2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:37 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 12-1-0 17-6-0 22-11-0 35-0-0 6-8-0 5-5-0 5-5-0 5-5-0 5-5-0 6-8-0 Scale = 1:61.7 4x5 = 5.00 12 3x4 > 3x4 = 5 5x5 = 5x5 > 5x8 8 10 6x6 = 6x6 = 2.25 12 3x10 = 3x10 = 17-6-0 9-8-0 35-0-0 9-8-0 7-10-0 7-10-0 9-8-0 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [7:0-4-12,Edge], [8:0-3-0,Edge], [10:0-3-0,Edge] LOADING (psf) SPACING CSI 2-0-0 DEFL **PLATES** in (loc) I/def L/d GRIP TCLL 20.0 1 25 TC Vert(LL) -0 43 >969 Plates Increase 0.61 9 240 MT20 244/190 10.0 1.25 BC 0.75 -1.099-10 TCDL Lumber Increase Vert(TL) >379 180 BCLL 0.0 Rep Stress Incr YES WB 0.68 Horz(TL) 0.63 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 153 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-5-2 oc purlins. BOT CHORD 2 X 4 SYP No.2D *Except* **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing 1-10,7-8: 2 X 4 SYP 2400F 2.0E MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1382/0-5-8, 7=1382/0-5-8

Max Horz 1=87(LC 5)

Max Uplift1=-149(LC 5), 7=-149(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-4885/583, 2-3=-4472/450, 3-4=-3253/277, 4-5=-3253/288, 5-6=-4472/379, 6-7=-4885/515 TOP CHORD **BOT CHORD** 1-10=-580/4513, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4513

2-10=-394/208, 3-10=-25/596, 3-9=-889/231, 4-9=-120/2112, 5-9=-889/233, 5-8=-28/596, 6-8=-394/212 **WEBS**

NOTES

Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

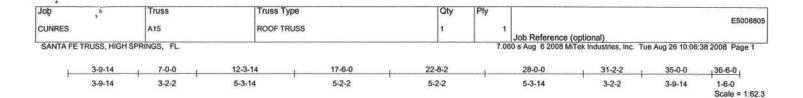
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

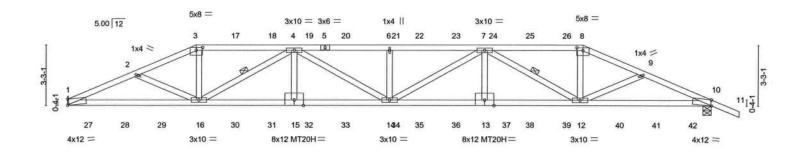
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 1 and 149 lb uplift at joint 7.

LOAD CASE(S) Standard









5-3-14	5-2-2	5-2-2	5-3-14	7-0-0
,0-3-12], [3:0-4-0,0-1-13], [8:0	4-0,0-1-13], [10:0-0-6,Edg	ge]		
SPACING 2-0-0	CSI	DEFL in (loc)	I/defl L/d	PLATES GRIP
Plates Increase 1.25	TC 0.94	Vert(LL) -0.54 14	>772 240	MT20 244/190
Lumber Increase 1.25	BC 0.87	Vert(TL) -1.39 14-15	>300 180	MT20H 187/143
Rep Stress Incr NO	WB 0.57	Horz(TL) 0.34 10	n/a n/a	II ANTONO ESPANO SERVICIO
Code FBC2004/TPI2002	(Matrix)	AND THE CONTROL OF TH	r vowers wasse	Weight: 172 lb
	e,0-3-12], [3:0-4-0,0-1-13], [8:0- SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25	\$PACING 2-0-0 CSI Plates Increase 1.25 TC 0.94 Lumber Increase 1.25 BC 0.87 Rep Stress Incr NO WB 0.57	SPACING 2-0-0 CSI DEFL in (loc)	1.0-3-12 , [3:0-4-0,0-1-13], [8:0-4-0,0-1-13], [10:0-0-6,Edge]

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except* 5-8: 2 X 4 SYP 2400F 2.0E

7.0.0

BOT CHORD 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 BRACING

22.8.2

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 1-7-10 oc purlins. Rigid ceiling directly applied or 5-11-13 oc bracing.

25 0 0

1 Row at midpt 4-16, 7-12

29 0 0

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=2685/Mechanical, 10=2863/0-5-8

Max Horz 1=-65(LC 6)

Max Uplift1=-578(LC 5), 10=-667(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6129/1428, 2-3=-6001/1454, 3-17=-5614/1377, 17-18=-5614/1377, 4-18=-5614/1377, 4-19=-8605/2175,

5-19=-8605/2175, 5-20=-8605/2175, 20-21=-8605/2175, 6-21=-8605/2175, 6-22=-8605/2175, 22-23=-8605/2175, 7-23=-8605/2175, 7-24=-5636/1416, 24-25=-5636/1416, 25-26=-5636/1416, 8-26=-5636/1416, 8-9=-6027/1496,

17.60

9-10=-6020/1440, 10-11=0/34

BOT CHORD 1-27=-1284/5593, 27-28=-1284/5593, 28-29=-1284/5593, 16-29=-1284/5593, 16-30=-1900/7847, 30-31=-1900/7847,

15-31=-1900/7847, 15-32=-1900/7847, 32-33=-1900/7847, 33-34=-1900/7847, 14-34=-1900/7847, 14-35=-1922/7866, 35-36=-1922/7866, 13-36=-1922/7866, 13-37=-1922/7866, 37-38=-1922/7866, 38-39=-1922/7866, 12-39=-1922/7866,

12-40=-1267/5442, 40-41=-1267/5442, 41-42=-1267/5442, 10-42=-1267/5442

12.3.14

2-16=-66/121, 3-16=-275/1622, 4-16=-2638/696, 4-15=0/401, 4-14=-223/927, 6-14=-619/264, 7-14=-201/883, **WEBS** 7-13=0/408, 7-12=-2611/682, 8-12=-248/1555, 9-12=-77/283

NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 578 lb uplift at joint 1 and 667 lb uplift at joint 10.

Continued on page 2



Edenton, NC 27932 FL COA #7239



Job	Truss	Truss Type	Qty	Ply		E5008805
CUNRES	A15	ROOF TRUSS	1	1	VI 1000 1000 1000 1000 1000 1000 1000 10	E5008805
		A STATE OF THE STA			Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS, FL.

NOTES

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 257 lb down and 152 lb up at 7-0-0, 125 lb down and 73 lb up at 9-0-12, 125 lb down and 73 lb up at 11-0-12, 125 lb down and 73 lb up at 13-0-12, 125 lb down and 73 lb up at 15-0-12, 125 lb down and 73 lb up at 17-0-12, 125 lb down and 73 lb up at 19-0-12, 125 lb down and 73 lb up at 21-0-12, 125 lb down and 73 lb up at 23-0-12, 125 lb down and 73 lb up at 25-0-12, and 125 lb down and 73 lb up at 27-0-12, and 297 lb down and 152 lb up at 28-0-0 on top chord, and 98 lb down at 1-0-12, 107 lb down at 3-0-12, 94 lb down at 5-0-12, 94 lb down at 7-0-12, 94 lb down at 9-0-12, 94 lb down at 11-0-12, 94 lb down at 13-0-12, 94 lb down at 15-0-12, 94 lb down at 17-0-12, 94 lb down at 19-0-12, 94 lb down at 21-0-12, 94 lb down at 23-0-12, 94 lb down at 25-0-12, 94 lb down at 27-0-12, 94 lb down at 27-11-4, 94 lb down at 29-11-4, and 107 lb down at 31-11-4, and 98 lb down at 33-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

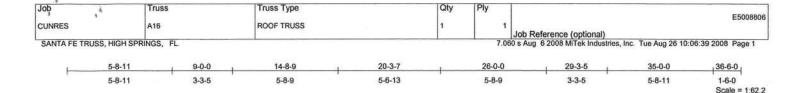
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

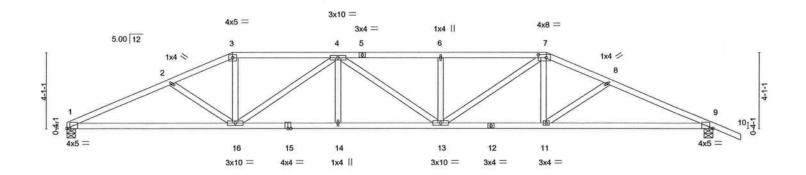
Uniform Loads (plf)

Vert: 1-3=-60, 3-8=-60, 8-11=-60, 1-10=-20

Concentrated Loads (lb)

Vert: 3=-257(B) 8=-257(B) 16=-47(B) 12=-47(B) 17=-125(B) 18=-125(B) 19=-125(B) 20=-125(B) 21=-125(B) 22=-125(B) 23=-125(B) 24=-125(B) 25=-125(B) 25=-125(B 26=-125(B) 27=-49(B) 28=-72(B) 29=-47(B) 30=-47(B) 31=-47(B) 32=-47(B) 33=-47(B) 35=-47(B) 35=-47(B) 36=-47(B) 37=-47(B) 38=-47(B) 39=-47(B) 40=-47(B) 41=-72(B) 42=-49(B)





1		9-0-0	1 1	1-8-9	-	20-3-7	26-0-0		-	35-0-0	
0.96	9-0-0		-8-9	1.5	5-6-13	5-8-9			9-0-0	90	
Plate Of	sets (X,Y):	[1:0-1-14,0-0-2], [7:0-5-4,	0-2-0], [9:0-1	-14,0-0-2]							
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.21 13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.54 13-14	>763	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.16 9	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	ix)					Weight: 170	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-4-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1379/0-5-8, 9=1488/0-5-8

Max Horz 1=-75(LC 6)

Max Uplift1=-135(LC 4), 9=-190(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2928/347, 2-3=-2676/327, 3-4=-2462/317, 4-5=-3094/430, 5-6=-3094/430, 6-7=-3094/430, 7-8=-2660/327,

8-9=-2914/346, 9-10=0/34

BOT CHORD 1-16=-264/2640, 15-16=-330/3097, 14-15=-330/3097, 13-14=-330/3097, 12-13=-214/2431, 11-12=-214/2431,

9-11=-264/2611

 $2 - 16 = -251/133, \ 3 - 16 = -33/746, \ 4 - 16 = -856/138, \ 4 - 14 = 0/193, \ 4 - 13 = -78/73, \ 6 - 13 = -344/126, \ 7 - 13 = -144/879, \ 7 - 11 = 0/37$

8-11=-232/117

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 1 and 190 lb uplift at joint 9.

LOAD CASE(S) Standard





Qty Job Truss Truss Type E5008807 CUNRES A17 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:40 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -1-6-0 5-3-10 11-0-0 17-6-0 24-0-0 29-8-6 35-0-0 36-6-0 1-6-0 5-3-10 5-8-6 6-6-0 6-6-0 5-8-6 5-3-10 1-6-0 Scale: 3/16"=1" 4x8 = 1x4 || 4x8 = 5 6 5.00 12 3x4 = 3x4 > 4x5 = 4x5 =16 15 14 13 12 11 10 1x4 || 3x4 = 3x10 = 3x4 = 1x4 || 3x4 = 3x4 = 11-0-0 17-6-0 24-0-0 5-3-10 29-8-6 35-0-0 5-8-6 6-6-0 6-6-0 5-8-6 5-3-10 5-3-10 Plate Offsets (X,Y): [2:0-2-2,0-0-2], [4:0-5-4,0-2-0], [6:0-5-4,0-2-0], [8:0-2-2,0-0-2] LOADING (psf) SPACING 2-0-0 CSI DEFL **PLATES** GRIP L/d in (loc) I/defl TCLL 20.0 1.25 TC 0.28 Vert(LL) -0.17>999 240 244/190 Plates Increase 13 MT20 10.0 1.25 BC 0.52 -0.43 11-13 >965 180 TCDL Lumber Increase Vert(TL) WB BCLL 0.0 Rep Stress Incr YES 0.32 Horz(TL) 0.15 8 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 177 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-7-10 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1485/0-5-8, 8=1485/0-5-8

Max Horz 2=-76(LC 6)

Max Uplift2=-181(LC 5), 8=-181(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/34, 2-3=-2989/276, 3-4=-2494/289, 4-5=-2613/341, 5-6=-2613/341, 6-7=-2494/289, 7-8=-2989/276, 8-9=0/34 2-16=-194/2680, 15-16=-194/2680, 14-15=-160/2256, 13-14=-160/2256, 12-13=-165/2256, 11-12=-165/2256,

2-16=-194/2680, 15-16=-194/2680, 1-10-11=-207/2680, 8-10=-207/2680

WEBS 3-16=0/21

3-16=0/217, 3-15=-473/114, 4-15=0/385, 4-13=-74/571, 5-13=-401/148, 6-13=-74/571, 6-11=0/385, 7-11=-473/115,

7-10=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

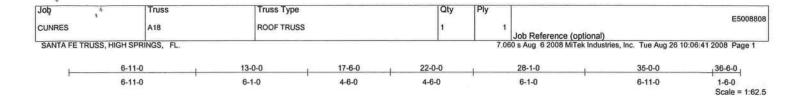
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2 and 181 lb uplift at joint 8.

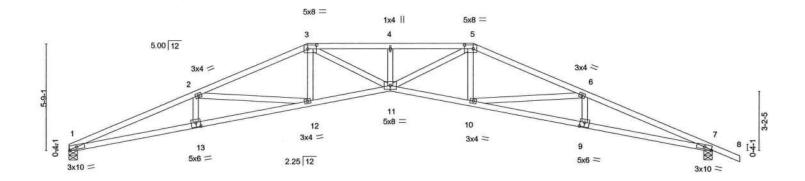
LOAD CASE(S) Standard



FL COA #7239







1-	6-11-0		13-0-0		17-6-0	22-0-0	-		28-1-0		35-0-0	
,	6-11-0		6-1-0		4-6-0	4-6-0			6-1-0		6-11-0	(M.).
Plate Offsets (X,Y): [1:0-4-12,Edge], [3:0-5		[1:0-4-12,Edge], [3:0-5-12	2,0-2-8], [5:0-	5-12,0-2-8],	[7:0-4-12,Ed	dge], [9:0-3-0,0-3	-0], [13:	0-3-0,0	-3-0]			
OADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.53	11	>777	240	MT20	244/190
CDL	10.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-1.34	11	>310	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.76	7	n/a	n/a		
BCDL	10.0	Code FBC2004/T	PI2002	(Mate	rix)						Weight: 158	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 2-4-1 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 1=1379/0-5-8, 7=1488/0-5-8

Max Horz 1=-93(LC 6)

Max Uplift1=-123(LC 5), 7=-195(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

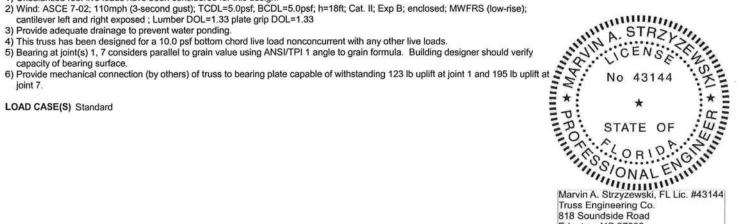
TOP CHORD **BOT CHORD** 1-2=-4955/415, 2-3=-4012/339, 3-4=-5218/449, 4-5=-5218/449, 5-6=-4006/342, 6-7=-4908/368, 7-8=0/32 1-13=-392/4552, 12-13=-391/4555, 11-12=-197/3720, 10-11=-200/3714, 9-10=-290/4509, 7-9=-291/4504 2-13=0/263, 2-12=-860/239, 3-12=-11/412, 3-11=-134/1748, 4-11=-209/88, 5-11=-130/1754, 5-10=-6/410,

WEBS 6-10=-819/215, 6-9=0/260

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise);

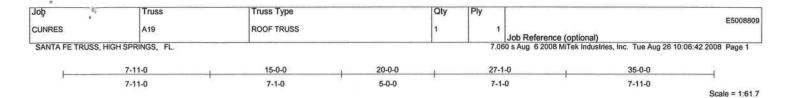


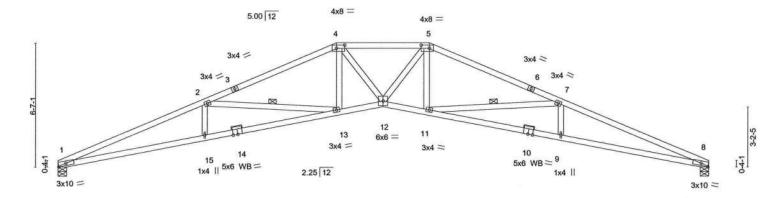
Edenton, NC 27932 FL COA #7239

August 26,2008

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ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.







1-		7-11-0	15-	-0-0	17-0-0	20-0-0		21-	1-0		35-0-0	
		7-11-0	7-	1-0	2-6-0	2-6-0		7-	1-0	1.0	7-11-0	
Plate Of	ffsets (X,Y): [1:0-4-12,Edge], [4:0-5-4,	0-2-0], [5:0-5	-4,0-2-0], [8:0	0-4-12,Edge]							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L∕d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.44	12	>951	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-1.09 1	3-15	>380	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.65	8	n/a	n/a	ZONOTZANA MINARA	
BCDL	10.0	Code FBC2004/TI	212002	(Matri:	x)						Weight: 155	b

LUMBER

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D *Except* 1-14,8-10: 2 X 4 SYP 2400F 2.0E

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied. Rigid ceiling directly applied or 9-8-12 oc bracing.

1 Row at midpt 2-13, 7-11

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1382/0-5-8, 8=1382/0-5-8

Max Horz 1=75(LC 5)

Max Uplift1=-135(LC 5), 8=-135(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-4935/469, 2-3=-3701/244, 3-4=-3615/269, 4-5=-3834/281, 5-6=-3615/260, 6-7=-3701/235, 7-8=-4935/410 TOP CHORD

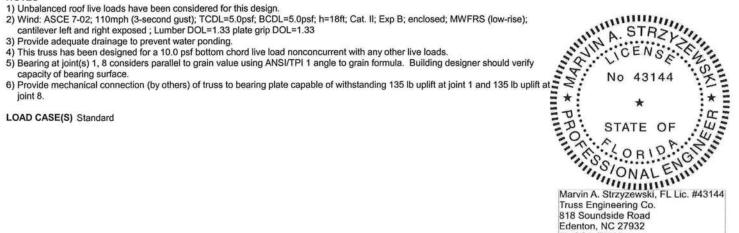
1-15=-453/4537, 14-15=-452/4512, 13-14=-445/4533, 12-13=-159/3406, 11-12=-118/3406, 10-11=-314/4533, **BOT CHORD**

9-10=-321/4512, 8-9=-322/4537

2-15=0/320, 2-13=-1133/288, 4-13=-13/494, 4-12=-62/785, 5-12=-92/785, 5-11=-13/494, 7-11=-1133/292, 7-9=0/320

WEBS NOTES

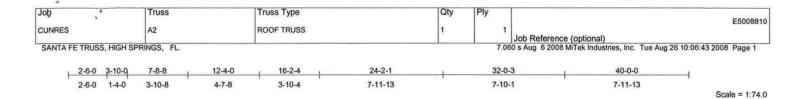
1) Unbalanced roof live loads have been considered for this design.

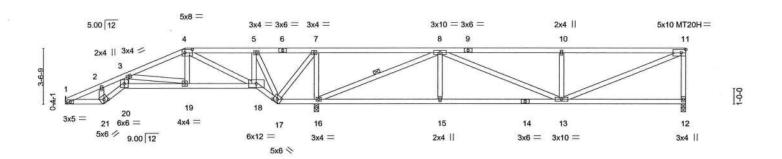


FL COA #7239

August 26,2008







2-6-0 3-10-0	7-8-8	12-4-0	13-8-0	16-2-4	24-2-1	32-0-3	40-0-0
2-6-0 1-4-0	3-10-8	4-7-8	1-4-0	2-6-4	7-11-13	7-10-1	7-11-13
Plate Offsets (X,Y):	[4:0-5-12,0-2-8]	, [17:0-3-0,0-2-2	2], [21:0	-3-0,0-2-2]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Rep Stre	Increase 1.25	5 5	BC 0	DEFL 0.87 Vert(LL) 0.38 Vert(TL) 0.81 Horz(TL)	in (loc) I/defl L/d -0.07 12-13 >999 240 -0.21 13-15 >999 180) 0.08 12 n/a n/a	PLATES GRIP MT20 244/190 MT20H 187/143 Weight: 206 lb
LUMBER TOP CHORD 2 X 4 S BOT CHORD 2 X 4 S WEBS 2 X 4 S					BRACIN TOP CH BOT CH	ORD Structural wood sheathing end verticals.	g directly applied or 5-8-1 oc purlins, except ed or 10-0-0 oc bracing, Except:

WEBS 1 Row at n

5-2-0 oc bracing: 16-17. 1 Row at midpt 8-16

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=328/Mechanical, 12=731/0-3-8, 16=2124/0-3-8

Max Horz 1=104(LC 5)

Max Uplift1=-32(LC 5), 12=-117(LC 3), 16=-299(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-808/100, 2-3=-810/124, 3-4=-174/16, 4-5=-103/840, 5-6=-103/916, 6-7=-103/916, 7-8=-177/1445, 8-9=-1126/186,

9-10=-1126/186, 10-11=-1126/186, 11-12=-648/151 BOT CHORD 1-21=-177/724, 20-21=-178/753, 19-20=-159/620, 1/2

1-21=-177/724, 20-21=-178/753, 19-20=-159/620, 18-19=-37/166, 17-18=-1017/145, 16-17=-1445/177, 15-16=-126/623,

14-15=-126/623, 13-14=-126/623, 12-13=-16/107

2-21=-227/71, 3-20=-33/331, 3-19=-476/124, 4-19=0/280, 4-18=-1117/155, 5-18=-18/97, 5-17=-180/18, 7-17=-120/860,

7-16=-1088/242, 8-16=-2234/324, 8-15=0/315, 8-13=-66/544, 10-13=-480/180, 11-13=-183/1103

NOTES

WEBS

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) Provide adequate drainage to prevent water ponding.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1, 117 lb uplift at joint 12 and 299 lb uplift at joint 16.

LOAD CASE(S) Standard



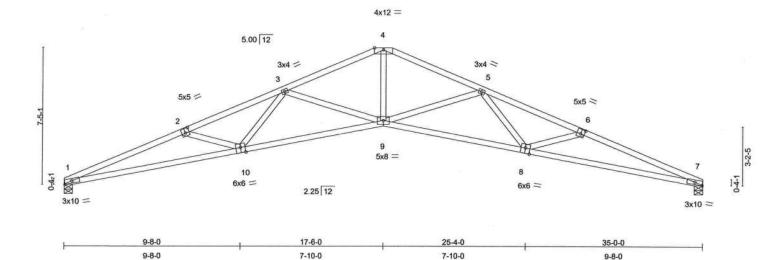
August 26,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job Truss Truss Type Qty E5008811 CUNRES A20 ROOF TRUSS Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:44 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 6-8-0 12-1-0 17-6-0 22-11-0 28-4-0 35-0-0 6-8-0 5-5-0 5-5-0 5-5-0 5-5-0 6-8-0 Scale = 1:62 9



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.43	9	>969	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-1.09	9-10	>379	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.63	7	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	PI2002	(Matr	ix)				1000	3000	Weight: 153	lb

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D *Except*

1-10,7-8: 2 X 4 SYP 2400F 2.0E

WERS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1382/0-5-8, 7=1382/0-5-8

Max Horz 1=87(LC 5)

Max Uplift1=-149(LC 5), 7=-149(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4885/583, 2-3=-4472/450, 3-4=-3253/277, 4-5=-3253/288, 5-6=-4472/379, 6-7=-4885/515

1-10=-580/4513, 9-10=-364/3834, 8-9=-208/3834, 7-8=-428/4513 BOT CHORD

2-10=-394/208, 3-10=-25/596, 3-9=-889/231, 4-9=-120/2112, 5-9=-889/233, 5-8=-28/596, 6-8=-394/212 WEBS

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 149 lb uplift at joint 1 and 149 lb uplift at joint 7.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-5-2 oc purlins.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

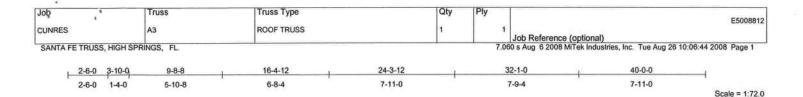
Installation guide

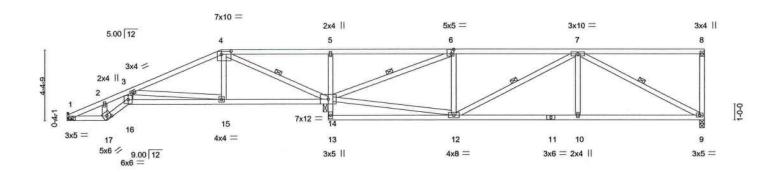
August 26,2008

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Design valid at a bed my with Miles Contributes. This design is a design is a design by port parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fives designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult AMS/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







9-4 7-11-0	
/defl L/d PLATES G	RIP
999 240 MT20 24	44/190
582 180	
n/a n/a	
Weight: 216 lb	
99	99 240 MT20 24 82 180 l/a n/a

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D *Except*

WEBS

5-13: 2 X 4 SYP No.3 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 14-15,12-13. 1 Row at midpt

WEBS

4-14, 6-14, 7-12, 7-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=340/Mechanical, 9=714/0-3-8, 14=2116/0-3-0 Max Horz 1=131(LC 5)

Max Uplift1=-19(LC 5), 9=-117(LC 3), 14=-291(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-913/87, 2-3=-910/106, 3-4=-75/102, 4-5=-246/1534, 5-6=-232/1519, 6-7=-436/96, 7-8=-76/11, 8-9=-217/77 TOP CHORD **BOT CHORD**

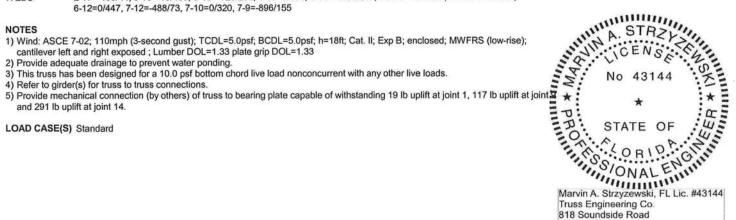
1-17=-191/815, 16-17=-183/833, 15-16=-182/708, 14-15=-4/20, 13-14=0/152, 5-14=-443/164, 12-13=-1/55,

11-12=-148/868, 10-11=-148/868, 9-10=-148/868

2-17=-198/44, 3-16=-10/400, 3-15=-728/201, 4-15=0/364, 4-14=-1681/248, 12-14=-124/429, 6-14=-2097/276,

6-12=0/447, 7-12=-488/73, 7-10=0/320, 7-9=-896/155

WEBS



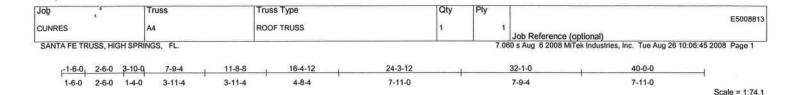
Edenton, NC 27932 FL COA #7239

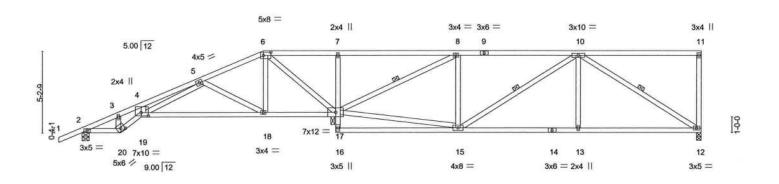
August 26,2008

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2-6	6-0 3-10-0	11-8-8		16-1-0	16-4-12	24-3-12	_1	32-1-0		40-0-0	
2-4	6-0 1-4-0	7-10-8		4-4-8	0-3-12	7-11-0		7-9-4		7-11-0	
Plate Offsets (X	K,Y): [6:0-	5-12,0-2-8], [19:0-5-8	3,0-2-8], [20	0-3-0,0-2-	2]					·	
LOADING (psf)		SPACING	2-0-0	CS		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
FCLL 20.0 FCDL 10.0		Plates Increase Lumber Increase	1.25 1.25	BC		Vert(LL) Vert(TL)	-0.12 18-19 -0.36 18-19		240 180	MT20	244/190
BCLL 0.0 BCDL 10.0		Rep Stress Incr Code FBC2004/T	YES PI2002	W (M	B 0.81 atrix)	Horz(TL)	0.08 12	n/a	n/a	Weight: 230	lb
UMBER						BRACING			200 300		PARK SEL MANY
OP CHORD 2		No.2D No.2D *Except*				TOP CHO		tural woo erticals.	d sheathing o	directly applied or 6	-0-0 oc purlins, excep
	7-16: 2 X 4 2 X 4 SYP	SYP No.3				BOT CHO			rectly applied	d or 10-0-0 oc braci 18.	ng, Except:

WEBS

1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

8-17, 10-15, 10-12

REACTIONS (lb/size) 12=684/0-3-8, 2=407/0-5-8, 17=2183/0-3-0 Max Horz 2=188(LC 5)

Max Uplift12=-123(LC 3), 2=-94(LC 5), 17=-274(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

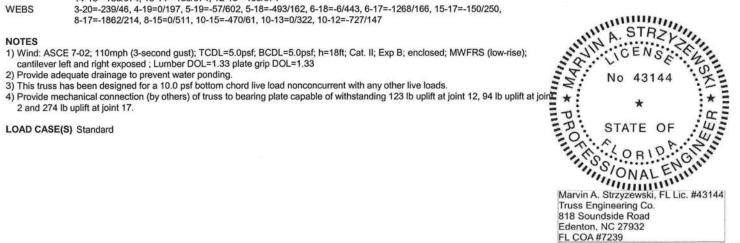
1-2=0/34, 2-3=-644/2, 3-4=-651/20, 4-5=-561/64, 5-6=-97/523, 6-7=-185/1415, 7-8=-177/1385, 8-9=-277/103, TOP CHORD

9-10=-277/103, 10-11=-59/9, 11-12=-216/77

 $2-20 = -133/566,\ 19-20 = -122/614,\ 18-19 = -116/0,\ 17-18 = -430/56,\ 16-17 = 0/151,\ 7-17 = -413/148,\ 15-16 = 0/128,\ 18-19 = -116/0,\ 1$ **BOT CHORD**

14-15=-133/674, 13-14=-133/674, 12-13=-133/674

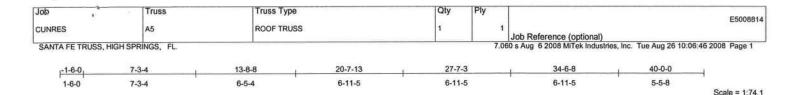
WEBS 3-20=-239/46, 4-19=0/197, 5-19=-57/602, 5-18=-493/162, 6-18=-6/443, 6-17=-1268/166, 15-17=-150/250,

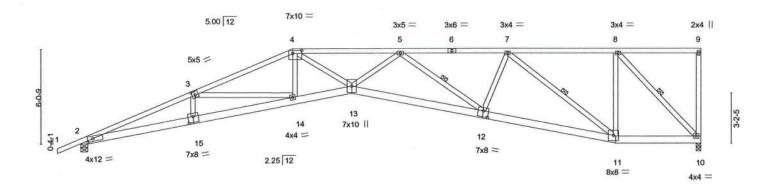


August 26,2008

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1		7-3-4	13-8-8	17-6	-0	26-0-4		-		34-6-8	40-0	-0
,		7-3-4	6-5-4	3-9	-8	8-6-4		8		8-6-4	5-5-	8
Plate Offse	ets (X,Y): [2:0-4-4,0-2-0], [3:0-2-8,0	-3-0], [4:0-7-0	0,0-2-4], [12:	0-4-0,0-4-8]	, [15:0-4-0,0-4-8]						
LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL :	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.51	13	>928	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-1.29	12-13	>368	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.61	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matri	x)	1/ 2					Weight: 241	lb
0000	10.0	0020.00200		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							,	

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 6 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 2-2-0 oc purlins, except

end verticals.

Rigid ceiling directly applied or 2-2-0 oc bracing.

5-12, 7-11, 8-10

1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1583/0-3-8, 2=1691/0-5-8

Max Horz 2=215(LC 5)

Max Uplift10=-230(LC 4), 2=-197(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

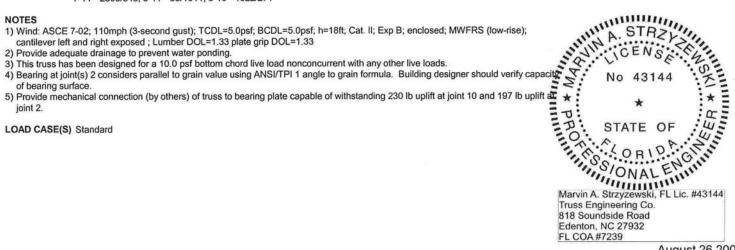
1-2=0/36, 2-3=-5861/637, 3-4=-4948/640, 4-5=-6109/810, 5-6=-3533/481, 6-7=-3533/481, 7-8=-1334/190, 8-9=-20/5, TOP CHORD

9-10=-132/47 BOT CHORD 2-15=-689/5407, 14-15=-693/5419, 13-14=-623/4594, 12-13=-768/5196, 11-12=-472/3192, 10-11=-190/1334

3-15=0/237, 3-14=-807/211, 4-14=-27/435, 4-13=-236/1894, 5-13=-82/1282, 5-12=-1972/335, 7-12=-67/1113,

7-11=-2306/343, 8-11=-53/1011, 8-10=-1922/271

WEBS



August 26,2008

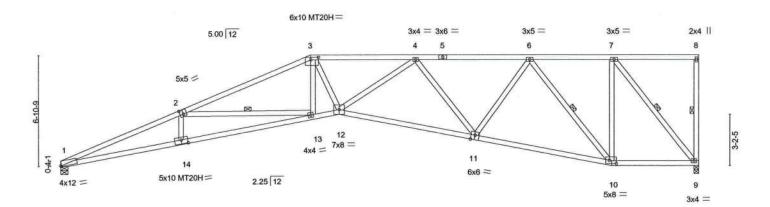


Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
ANSI/TP11 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



818 Soundside Road Edenton, NC 27932

Truss Truss Type Qty Ply Job E5008815 **ROOF TRUSS** CUNRES Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:47 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 40-0-0 7-6-12 15-8-8 22-3-7 29-5-1 34-6-8 6-6-15 7-1-10 5-1-7 5-5-8 7-6-12 8-1-12 Scale = 1:72.0



	7-6-	-12	15-8-8		8 17-6-0			34-6-8		34-6-8	40-0-	-0
	7-6-12		8-1-12		9-8	8-6-4		8-6-4			5-5-	8
Plate Off	fsets (X,Y): [1:0-0-11,Edge], [2:0-2-8,	0-3-4], [3:0-6-	0,0-1-12], [1	0:0-5-4,0-2	2-8], [11:0-3-0,Edg	je], [14:0	0-5-0,0	-3-0]			
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.44	13-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.20	13-14	>398	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.59	9	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matrix	()						Weight: 215	lb

BRACING

WFBS

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except*

1-2: 2 X 4 SYP 2400F 2.0E

BOT CHORD 2 X 4 SYP No.2D *Except*

1-14: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8

Max Horz 1=213(LC 5)

Max Uplift9=-228(LC 4), 1=-135(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-5844/598, 2-3=-4540/562, 3-4=-4573/596, 4-5=-2857/375, 5-6=-2857/375, 6-7=-1140/158, 7-8=-19/4, 8-9=-148/53 TOP CHORD

1-14=-703/5385, 13-14=-707/5380, 12-13=-554/4188, 11-12=-543/3722, 10-11=-337/2243, 9-10=-158/1140 **BOT CHORD**

2-14=0/331, 2-13=-1168/311, 3-13=-26/424, 3-12=-111/968, 4-12=-149/1138, 4-11=-1319/252, 6-11=-91/1180, **WEBS**

6-10=-1709/270, 7-10=-75/1079, 7-9=-1787/245

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise);

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 9 and 135 lb upl Truss Engineering Co. 818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

8-9, 2-13, 6-10, 7-9

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

Installation guide

August 26,2008

818 Soundside Road Edenton, NC 27932

FL COA #7239

Job Truss Truss Type Qty Ply E5008816 CUNRES A7 **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:48 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 28-3-7 17-3-1 22-3-12 34-6-8 40-0-0 6-8-0 6-8-0 5-3-8 5-3-8 5-0-11 5-11-10 6-3-1 5-5-8 Scale = 1:72.0 5x5 = 3x4 = 5x5 = 2x4 || 4x5 = 5.00 12 5 6 7 8 3x4 = 5x5 = 12 7x8 = 11 13 6x6 = 5x10 MT20H= 2.25 12 4x12 = 10 5x8 = 2x4 || 17-6-0 25-3-9 34-6-8 40-0-0 7-10-0 9-2-15 9-8-0 7-9-9 5-5-8 Plate Offsets (X,Y): [1:0-0-11,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [11:0-3-0,Edge], [13:0-5-0,0-3-0] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP TC 0.75 -0.42 12-13 >999 244/190 20.0 1 25 Vert(LL) 240 MT20 TCLL Plates Increase BC -1.09 12-13 0.86 >436 180 MT20H 187/143 TCDL 10.0 Lumber Increase 1.25 Vert(TL) BCLL 0.0 Rep Stress Incr YES WB 0.75 Horz(TL) 0.54 9 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 221 lb BRACING LUMBER Structural wood sheathing directly applied or 2-2-0 oc purlins, except

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D *Except*

1-13: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

TOP CHORD **BOT CHORD**

WEBS

end verticals.

Rigid ceiling directly applied or 8-3-0 oc bracing. 1 Row at midpt 8-9, 6-10

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8

Max Horz 1=233(LC 5)

Max Uplift9=-226(LC 4), 1=-142(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

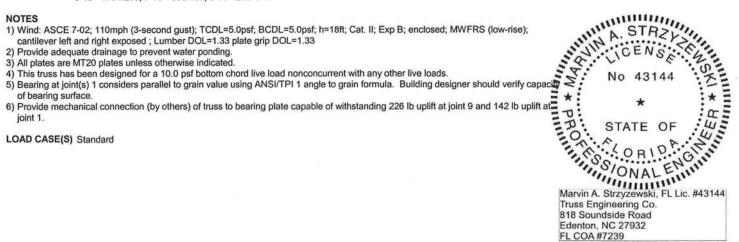
1-2=-5738/669, 2-3=-5357/540, 3-4=-4171/513, 4-5=-3871/497, 5-6=-2663/346, 6-7=-1048/143, 7-8=-1048/143, TOP CHORD 8-9=-1545/239

BOT CHORD

1-13=-808/5305, 12-13=-605/4699, 11-12=-453/3225, 10-11=-327/2218, 9-10=-3/14 2-13=-325/202, 3-13=-20/563, 3-12=-855/229, 4-12=-103/1270, 5-12=-160/932, 5-11=-1042/194, 6-11=-67/1040,

6-10=-1704/260, 7-10=-369/137, 8-10=-236/1747

WEBS



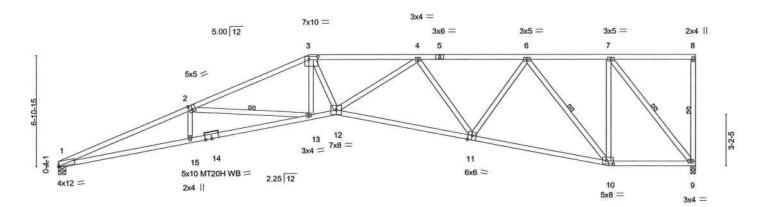
August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidonce regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Truss Type Qty Ply Job Truss E5008817 **ROOF TRUSS** CUNRES Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:49 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 40-0-0 8-3-10 15-9-5 22-7-8 29-5-8 34-6-8 6-10-3 6-10-0 5-1-0 5-5-8 8-3-10 7-5-11 Scale = 1:72.0



	3-3-10	7-5-11	1-8-11	8-6-4			8-6-4	5-5-	8
Plate Offsets (X,Y):	[1:0-0-11,Edge], [2:0-2-8,	0-3-4], [3:0-7-0	0,0-2-4], [10:0-5-4,0-2	2-8], [11:0-3-0,Edge	1				
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.83	Vert(LL)	-0.43 13	>999	240	MT20	244/190
TCDL 10.0	Lumber Increase	1.25	BC 0.93	Vert(TL)	-1.13 13-15	>420	180	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.85	Horz(TL)	0.58 9	n/a	n/a		
BCDL 10.0	Code FBC2004/TI	212002	(Matrix)					Weight: 215	lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except*

8-3-10

1-2: 2 X 4 SYP 2400F 2.0E

BOT CHORD 2 X 4 SYP No.2D *Except*

1-14: 2 X 4 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3 BRACING

WEBS

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 2-2-0 oc bracing. 8-9, 2-13, 6-10, 7-9

1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 9=1585/0-3-8, 1=1585/0-5-8

Max Horz 1=214(LC 5)

Max Uplift9=-228(LC 4), 1=-135(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=-5776/594, 2-3=-4501/562, 3-4=-4527/587, 4-5=-2846/375, 5-6=-2846/375, 6-7=-1136/158, 7-8=-18/4, 8-9=-147/53

1-15=-683/5319, 14-15=-687/5280, 13-14=-679/5306, 12-13=-551/4161, 11-12=-530/3635, 10-11=-332/2220, **BOT CHORD**

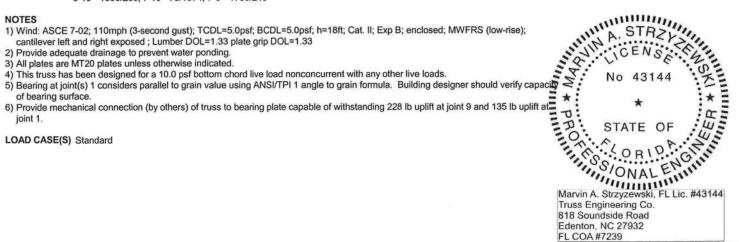
WEBS 2-15=0/342, 2-13=-1132/296, 3-13=-45/406, 3-12=-100/972, 4-12=-149/1164, 4-11=-1277/248, 6-11=-97/1197,

6-10=-1690/266, 7-10=-73/1074, 7-9=-1786/245

TOP CHORD

15-9-5

17-6-0



August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

818 Soundside Road Edenton, NC 27932

Truss Type Qty Ply Truss Job E5008818 ROOF TRUSS CUNRES Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:49 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 6-8-0 12-1-0 17-6-0 20-8-7 28-10-14 37-0-0 3-2-7 8-2-7 8-1-2 6-8-0 5-5-0 5-5-0 Scale = 1:68.0 5x5 = 5.00 12 4 5x5 = 3x6 = 4x12 =4x5 = 6 8 3x4 = 5x5 = 14 13 7x8 = 4x5 = 15 12 5x8 = 11 2.25 12 3x6 = 2x4 || 3x10 = 10 2x4 || 9-8-0 17-6-0 20-8-7 28-8-8 34-6-8 37-0-0 7-10-0 8-0-0 5-10-0 2-5-8 9-8-0 Plate Offsets (X,Y): [1:0-4-12,Edge], [2:0-2-8,0-3-0], [15:0-4-0,0-3-4] DEFL **PLATES** GRIP SPACING 2-0-0 CSI in (loc) I/defl L/d LOADING (psf) -0.41 14-15 MT20 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.73 Vert(LL) >999 240 TCDL 10.0 1.25 BC 0.81 Vert(TL) -1.08 14-15 >405 180 Lumber Increase YES WB 0.80 0.54 BCLL 0.0 Rep Stress Incr Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 198 lb BRACING LUMBER TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 2-3-6 oc purlins, except BOT CHORD 2 X 4 SYP No.2D *Except* end verticals. **BOT CHORD** Rigid ceiling directly applied or 8-7-12 oc bracing. 1-15: 2 X 4 SYP 2400F 2.0E **WEBS** WEBS 2 X 4 SYP No.3 1 Row at midpt 8-9, 7-10 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=1465/0-5-8, 1=1465/0-5-8

Max Horz 1=210(LC 5)

Max Uplift9=-186(LC 4), 1=-142(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-5236/650, 2-3=-4839/519, 3-4=-3611/370, 4-5=-3590/383, 5-6=-3906/421, 6-7=-3906/421, 7-8=-503/53, TOP CHORD

BOT CHORD 1-15=-767/4839, 14-15=-559/4184, 13-14=-440/4028, 12-13=-245/1992, 11-12=-252/1965, 10-11=-249/1993,

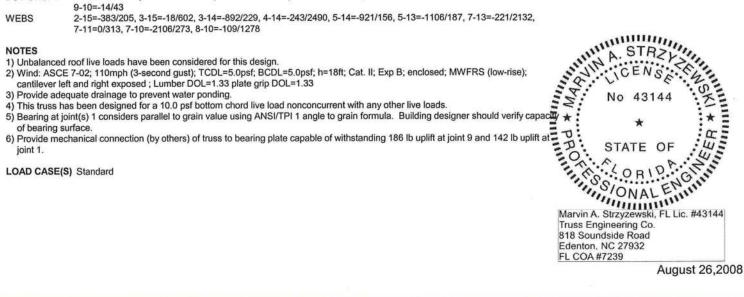
2-15=-383/205, 3-15=-18/602, 3-14=-892/229, 4-14=-243/2490, 5-14=-921/156, 5-13=-1106/187, 7-13=-221/2132,

7-11=0/313, 7-10=-2106/273, 8-10=-109/1278

NOTES

WEBS

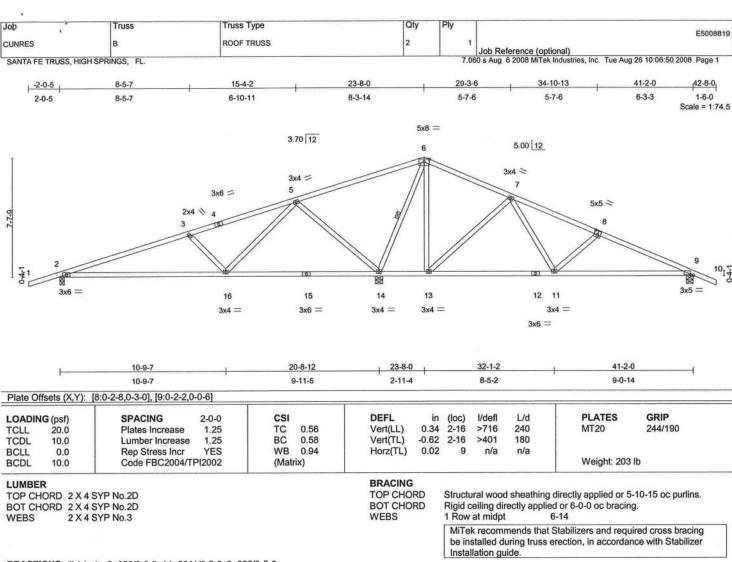
Unbalanced roof live loads have been considered for this design.





Design valid for use only with MTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





REACTIONS (lb/size) 2=653/0-3-8, 14=2241/0-5-8, 9=603/0-5-8

Max Horz 2=-108(LC 6)

Max Uplift2=-366(LC 5), 14=-650(LC 3), 9=-111(LC 6) Max Grav 2=703(LC 7), 14=2241(LC 1), 9=700(LC 8)

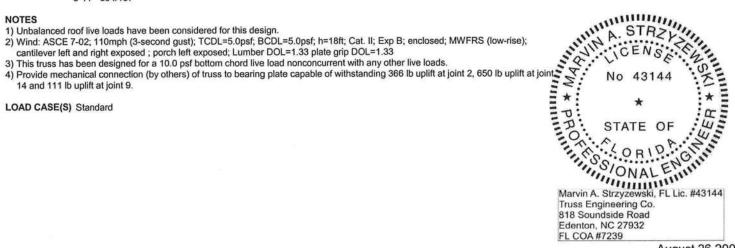
FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-938/430, 3-4=-571/340, 4-5=-459/359, 5-6=-284/1113, 6-7=-193/517, 7-8=-675/49, 8-9=-971/37, TOP CHORD

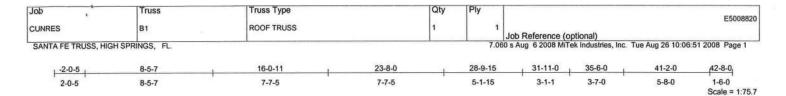
9-10=0/34

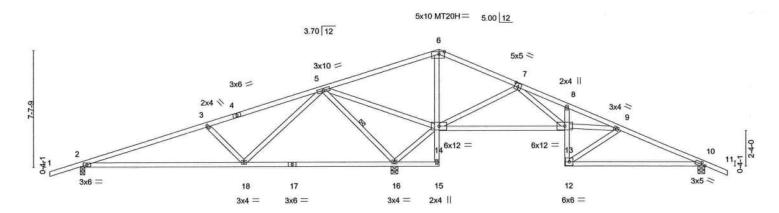
2-16=-438/829, 15-16=-309/158, 14-15=-309/158, 13-14=-419/309, 12-13=-204/309, 11-12=-204/309, 9-11=0/831 **BOT CHORD WEBS**

3-16=-506/212, 5-16=-452/839, 5-14=-999/475, 6-14=-1553/255, 6-13=-15/541, 7-13=-686/171, 7-11=-19/562,









		10-9-7		9-	11-5	2-11-4			8-3-0		9-3-0	
Plate Of	fsets (X,Y): [6:0-4-4,Edge], [7:0-2-8,0	-3-0], [10:0-3	-0,0-1-8]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.35	2-18	>711	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.62	2-18	>401	180	MT20H	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.04	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Matr	ix)	180.8					Weight: 215 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D *Except*

6-15,8-12: 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

23-8-0

TOP CHORD **BOT CHORD**

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 12-13,10-12.

5-16

WEBS 1 Row at midpt

31-11-0

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Structural wood sheathing directly applied or 5-9-11 oc purlins.

REACTIONS (lb/size) 2=590/0-3-8, 10=539/0-5-8, 16=2368/0-5-8

10-9-7

Max Horz 2=-108(LC 6)

Max Uplift2=-416(LC 5), 10=-137(LC 6), 16=-558(LC 3) Max Grav 2=714(LC 7), 10=594(LC 8), 16=2368(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-982/599, 3-4=-602/504, 4-5=-481/524, 5-6=-167/977, 6-7=-171/985, 7-8=-870/34, 8-9=-815/123,

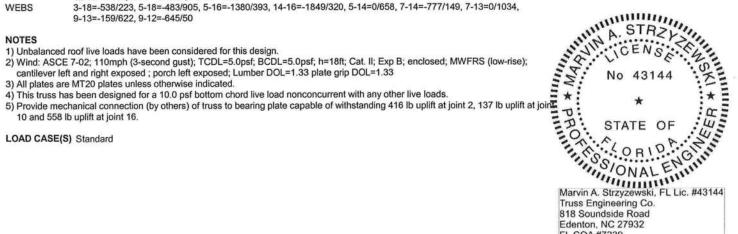
BOT CHORD 2-18=-599/873, 17-18=-679/102, 16-17=-679/102, 15-16=-114/2, 14-15=-70/54, 6-14=-1029/236, 13-14=-367/274,

12-13=0/448, 8-13=-174/55, 10-12=-15/617

WEBS

3-18=-538/223, 5-18=-483/905, 5-16=-1380/393, 14-16=-1849/320, 5-14=0/658, 7-14=-777/149, 7-13=0/1034,

20-8-12



FL COA #7239

August 26,2008

on, NC 27932

Truss Type Qty Ply Truss Job E5008821 **ROOF TRUSS** CUNRES B2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:52 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -2-0-5 7-5-7 14-0-11 20-11-8 6-10-13 2-0-5 7-5-7 6-7-5 3x4 || Scale = 1:43.3 5 3.70 12 3x4 = 5x5 = X 4 3x6 7 6 3x4 = 3x5 = 3x6 = 10-9-1 20-11-8 10-9-1 10-2-7 Plate Offsets (X,Y): [2:0-1-1,0-0-3], [3:0-2-8,0-3-0] GRIP DEFL **PLATES** LOADING (psf) SPACING CSI (loc) I/defl L/d 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.69 Vert(LL) 0.33 2-8 >752 240 MT20 BC TCDL 10.0 Lumber Increase 1.25 0.71 Vert(TL) -0.60 2-8 >413 180 YES WB 0.34 0.05 BCLL 0.0 Rep Stress Incr Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 102 lb BRACING LUMBER TOP CHORD TOP CHORD 2 X 4 SYP No.2D Structural wood sheathing directly applied or 4-3-12 oc purlins, except BOT CHORD 2 X 4 SYP No.2D end verticals. **WEBS** 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-10-5 oc bracing. WEBS 1 Row at midpt 5-6, 4-6 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

REACTIONS (lb/size) 6=820/0-5-8, 2=964/0-3-8

Max Horz 2=237(LC 3)

Max Uplift6=-411(LC 3), 2=-445(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-1859/716, 3-4=-1410/600, 4-5=-105/14, 5-6=-177/74 TOP CHORD **BOT CHORD** 2-8=-844/1714, 7-8=-439/892, 6-7=-439/892

WEBS 3-8=-516/221, 4-8=-372/727, 4-6=-1016/495

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 6 and 445 lb uplift at joint 2.

LOAD CASE(S) Standard





Truss Type Qty Ply Truss Job E5008822 ROOF TRUSS CUNRES вз Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:52 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL -2-0-5 7-11-5 15-4-0 7-4-11 2-0-5 7-11-5 3x4 ||Scale = 1:32.4 3.70 12 5 3x4 = 3x4 = 3 7 3x4 = 1x4 || 3x5 = 7-11-5 15-4-0 7-11-5 7-4-11 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP 1.25 0.57 Vert(LL) 2-7 >999 240 MT20 244/190 TCLL 20.0 Plates Increase TC 0.14 TCDL 10.0 1.25 BC 0.40 Vert(TL) -0.232-7 >775 180 Lumber Increase WB 0.32 0.03 BCII 00 Rep Stress Incr YES Horz(TL) 6 n/a n/a Code FBC2004/TPI2002 Weight: 71 lb BCDL 10.0 (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 5-6-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 8-10-1 oc bracing.

1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=592/0-5-8, 2=742/0-3-8 Max Horz 2=181(LC 3)

Max Uplift6=-296(LC 3), 2=-351(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-1174/443, 3-4=-122/0, 4-5=-51/6, 5-6=-178/74 TOP CHORD

BOT CHORD 2-7=-525/1057, 6-7=-525/1057 3-7=-130/338, 3-6=-1066/528 WEBS

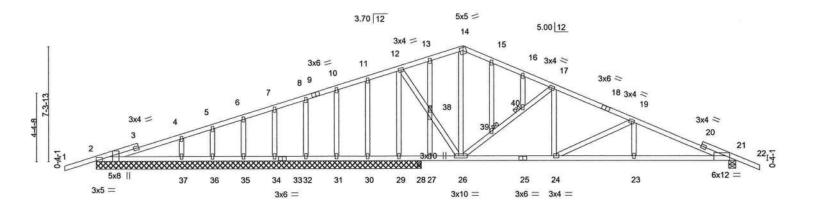
- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 6 and 351 lb uplift at joint 2.

LOAD CASE(S) Standard





CUNRES	Truss	Truss Type GABLE	Qty 1	Ply 1	Job Refer	rence (optional)		E500882
SANTA FE TRUSS, HIG	H SPRINGS, FL., x			7		2008 MiTek Industries, Ind	. Tue Aug 26 11:00:10	2008 Page 1
ı -2-0-5 ı		19-6-1	23-7-13	29-	-6-1	34-7-9	41-2-0	42-8-0
2-0-5		19-6-1	4-1-12	5-1	0-4	5-1-8	6-6-7	1-6-0



L	19-6-1		20-11-8 23-7-13	29-6-1	34-7-9	41-2-0	
	19-6-1		1-5-7 2-8-5	5-10-4	5-1-8	6-6-7	
late Offsets (X,Y): [2:0-5-2,Edge], [2:0-3-8,Edge], [14:0-2-	3,0-2-5], [21:0-3-2,Edge]					
OADING (psf) CLL 20.0 CDL 10.0 CLL 0.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES	CSI TC 0.33 BC 0.40 WB 0.56	Vert(LL) 0.0	n (loc) I/defl 7 21-23 >999 8 21-23 >999 2 28 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190	
CDL 10.0	Code FBC2004/TPI2002	(Matrix)	BRACING	LISE MITEL MI	II TI DDACE Jumb	Weight: 244 lb	
57, 1 1 5 7 7 7 7 7 1 1 5 1 1 1 1 1 1 1 1 1	SYP No.2D		BRACING	per BCSI.	LTI-BRACE, IUIII	der of other products for bracing	
	SYP No.2D		TOP CHORD	Structural wood sheathing directly applied or 4-11-11 oc pur			
	SYP No.3		BOT CHORD	HORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 24-26,23-24,21-23.			
THERS 2X4S	SYP No.3		JOINTS	1 Brace at Jt(s):		-23.	
					talled during truss	ers and required cross erection, in accordance with	

2=217/20-11-8, 29=862/20-11-8, 37=353/20-11-8, 36=70/20-11-8, 35=183/20-11-8, 34=154/20-11-8, REACTIONS (lb/size) 32=166/20-11-8, 31=138/20-11-8, 30=219/20-11-8, 21=822/0-5-8, 28=318/0-3-8

Max Horz 2=133(LC 3)

Max Uplift 2=-150(LC 3), 29=-195(LC 6), 37=-96(LC 3), 36=-48(LC 5), 35=-64(LC 3), 34=-60(LC 3), 32=-62(LC 5), 31=-59(LC 3), 30=-67(LC 5), 21=-337(LC 6), 28=-84(LC 5)

Max Grav 2=270(LC 9), 29=862(LC 1), 37=374(LC 9), 36=70(LC 1), 35=185(LC 9), 34=154(LC 1), 32=166(LC 1), 31=147(LC 9), 30=219(LC 1), 21=822(LC 1), 28=318(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-207/479, 3-4=-198/532, 4-5=-150/495, 5-6=-128/511, 6-7=-96/507, 7-8=-67/508, 8-9=-37/475,

9-10=-34/507, 10-11=-7/512, 11-12=0/497, 12-13=-61/171, 13-14=-73/184, 14-15=-53/168, 15-16=-81/152, 16-17=-122/139, 17-18=-683/320, 18-19=-790/302, 19-20=-1384/459, 20-21=-1424/433, 21-22=0/34

2-37=-469/290, 36-37=-469/290, 35-36=-469/290, 34-35=-469/290, 33-34=-469/290, 32-33=-469/290

31-32=-469/290, 30-31=-469/290, 29-30=-469/290, 28-29=-469/290, 27-28=-469/290, 26-27=-469/290,

25-26=-85/669, 24-25=-85/669, 23-24=-323/1277, 21-23=-323/1277

12-29=-954/249, 12-38=-206/900, 26-38=-215/930, 14-26=-169/24, 26-39=-791/331, 39-40=-768/319,

17-40=-753/309, 17-24=-56/443, 19-24=-674/264, 19-23=0/244, 4-37=-265/143, 5-36=-64/56, 6-35=-135/87,

7-34=-116/80, 8-32=-124/81, 10-31=-111/80, 11-30=-157/82, 13-38=-151/61, 27-38=-186/64, 15-39=-37/20,

16-40=-25/16

WEBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; porch left exposed; Lumber DOL=1.33 plate grip DOL=1.33

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard
- Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

August 26,2008

Scale = 1:73.9



Job '	Truss	Truss Type	Qty	Ply	ESO	00000
CUNRES	BET	GABLE	1	1	Job Reference (optional)	00023

SANTA FE TRUSS, HIGH SPRINGS, FL., X

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NOTES

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 2, 195 lb uplift at joint 29, 96 lb uplift at joint 37, 48 lb uplift at joint 36, 64 lb uplift at joint 35, 60 lb uplift at joint 34, 62 lb uplift at joint 32, 59 lb uplift at joint 31, 67 lb uplift at joint 30, 337 lb uplift at joint 21 and 84 lb uplift at

8) When using MiTek Multi-Brace, attach per Multi-Brace Installation Guide with 2-10d (0.131" X 3") nails. Cross bracing required per BCSI.

LOAD CASE(S) Standard

Job Truss Truss Type Qty Ply E5008824 CUNRES C **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:54 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 22-2-0 10-4-0 15-2-2 20-8-0 -1-6-0 5-5-14 5-5-14 4-10-2 4-10-2 5-5-14 1-6-0 1-6-0 Scale = 1:39.4 4x4 = 5.00 12 1x4 / 1x4 // 5 3 14 9 8 10 3x4 > 3x4 = 3x4 3x4 13-6-12 20-8-0 6-5-7 7-1-4 7-1-4 Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8] LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TC -0.056-8 >999 240 MT20 244/190 1 25 0.16 Vert(LL) TCLL 20.0 Plates Increase BC -0.16>999 180 0.37 Vert(TL) 6-8 TCDL 10.0 Lumber Increase 1 25 BCLL 0.0 Rep Stress Incr YES WB 0.15 Horz(TL) 0.04 6 n/a n/a Weight: 92 lb BCDL 10.0 Code FBC2004/TPI2002 (Matrix) LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 5-0-14 oc purlins. TOP CHORD 2 X 4 SYP No.2D **BOT CHORD** BOT CHORD 2 X 4 SYP No.2D Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 WEBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=912/0-5-8, 6=912/0-5-8

Max Horz 2=73(LC 5)

Max Uplift2=-160(LC 5), 6=-160(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-1550/154, 3-4=-1362/143, 4-5=-1362/144, 5-6=-1550/154, 6-7=0/34 TOP CHORD 2-10=-135/1365, 9-10=-38/941, 8-9=-38/941, 6-8=-63/1365

BOT CHORD

3-10=-286/126, 4-10=-37/460, 4-8=-37/460, 5-8=-286/126 WEBS

NOTES

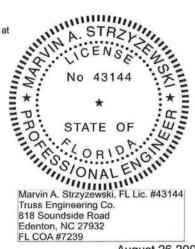
1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 2 and 160 lb uplift at joint 6.

LOAD CASE(S) Standard



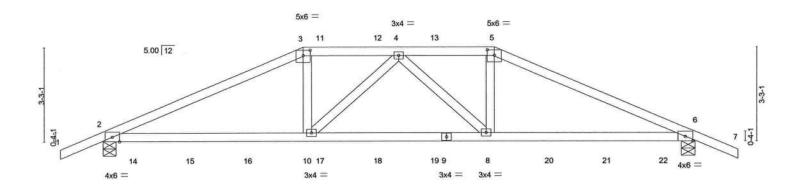
August 26,2008

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guldance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Qty Ply Job Truss Truss Type E5008825 CUNRES C1 ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:55 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 22-2-0 10-4-0 13-8-0 20-8-0 7-0-0 -1-6-0 1-6-0 7-0-0 3-4-0 3-4-0 7-0-0 1-6-0 Scale = 1:40.1



<u> </u>	7-0-0	13-8-0	20-8-0	-1
	7-0-0	6-8-0	7-0-0	

LOADING (ps	sf) SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.	.0 Plates Incre	ase 1.25	TC	0.49	Vert(LL)	-0.13	6-8	>999	240	MT20	244/190
TCDL 10.	.0 Lumber Incr	ease 1.25	BC	0.83	Vert(TL)	-0.38	6-8	>630	180	2.000	
BCLL 0.	.0 Rep Stress	ncr NO	WB	0.21	Horz(TL)	0.10	6	n/a	n/a		
BCDL 10.	.0 Code FBC20	004/TPI2002	(Matr	rix)						Weight: 88 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 3-3-2 oc purlins. Rigid ceiling directly applied or 7-10-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1661/0-5-8, 6=1624/0-5-8

Max Horz 2=57(LC 5)

Max Uplift2=-408(LC 5), 6=-393(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-3150/750, 3-11=-2845/719, 11-12=-2845/719, 4-12=-2844/719, 4-13=-2758/683, 5-13=-2759/683, TOP CHORD

5-6=-3054/709, 6-7=0/34

2-14=-652/2822, 14-15=-652/2822, 15-16=-652/2822, 10-16=-652/2822, 10-17=-681/2944, 17-18=-681/2944, BOT CHORD

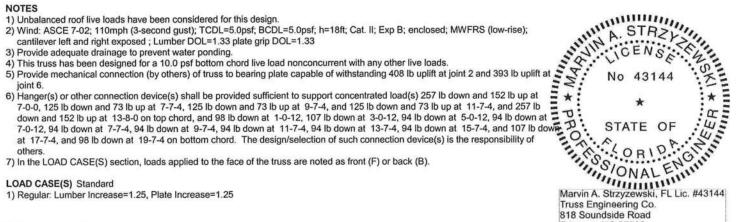
18-19=-681/2944, 9-19=-681/2944, 8-9=-681/2944, 8-20=-575/2734, 20-21=-575/2734, 21-22=-575/2734,

6-22=-575/2734

WEBS 3-10=0/665, 4-10=-269/103, 4-8=-386/144, 5-8=-32/663

NOTES

Continued on page 2



Edenton, NC 27932 FL COA #7239

August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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Job , *	Truss	Truss Type	Qty	Ply	Espagas
CUNRES	C1	ROOF TRUSS	1	1	E5006625
		p		1	Job Reference (optional)
CANTA EE TOUCE UI	CH CDDINGS EI			7.06	EO e Aug 6 2008 MiTok Industrias Inc. Tuo Aug 26 10:06:55 2008 Dags 2

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 5-7=-60, 2-6=-20

Concentrated Loads (lb)
Vert: 3=-257(B) 5=-257(B) 10=-47(B) 8=-47(B) 11=-125(B) 12=-125(B) 13=-125(B) 14=-49(B) 15=-72(B) 16=-47(B) 17=-47(B) 18=-47(B) 19=-47(B) 20=-47(B) 21=-72(B) 22=-49(B)

Truss Type Qty Ply Job Truss E5008826 ROOF TRUSS CUNRES C2 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:56 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 5-8-11 22-2-0 -1-6-0 3-3-5 2-8-0 3-3-5 5-8-11 1-6-0 5-8-11 Scale = 1:40.1 4x4 = 4x8 = 5 5.00 12 1x4 > 1x4 / 6 14 11 10 9 3x5 = 3x5 > 3x4 = 3x5 =3x10 = 9-0-0 11-8-0 20-8-0 2-8-0 9-0-0 9-0-0 Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-5-4,0-2-0], [7:0-3-0,0-1-8] 2-0-0 DEFL **PLATES** GRIP LOADING (psf) SPACING (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.25 Vert(LL) -0.147-9 >999 240 MT20 244/190 TCDL 10.0 1.25 BC 0.50 Vert(TL) -0.377-9 >655 180 Lumber Increase Rep Stress Incr WB 0.11 BCLL YES Horz(TL) 0.04 0.0 n/a n/a Code FBC2004/TPI2002 10.0 (Matrix) Weight: 97 lb BCDL BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 5-1-3 oc purlins.

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=912/0-5-8, 7=912/0-5-8

Max Horz 2=-67(LC 6)

Max Uplift2=-153(LC 5), 7=-153(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-1490/146, 3-4=-1203/99, 4-5=-1074/101, 5-6=-1201/99, 6-7=-1490/147, 7-8=0/34

BOT CHORD 2-11=-117/1322, 10-11=-0/1072, 9-10=-0/1072, 7-9=-66/1322

WEBS 3-11=-316/123, 4-11=-3/310, 5-11=-94/99, 5-9=-8/312, 6-9=-318/123

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 2 and 153 lb uplift at joint 7.

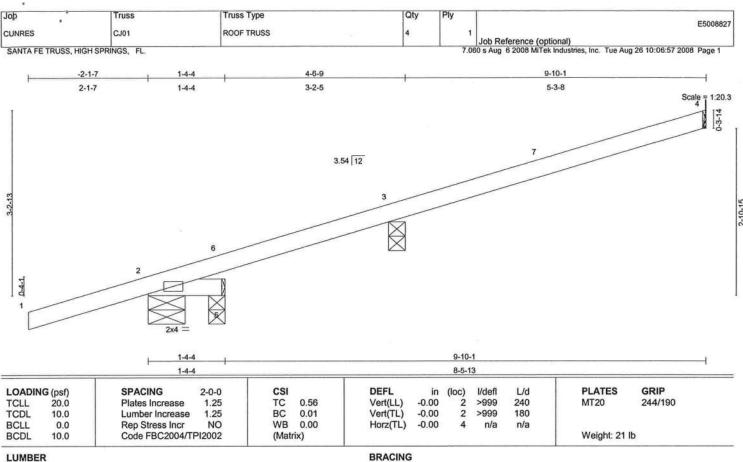
LOAD CASE(S) Standard



August 26,2008

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-4-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 4=192/Mechanical, 2=252/0-7-12, 5=22/0-3-8, 3=720/0-3-8

Max Horz 2=124(LC 3)

Max Uplift4=-89(LC 3), 2=-82(LC 3), 3=-253(LC 4)

Max Grav 4=192(LC 1), 2=252(LC 1), 5=44(LC 2), 3=720(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-6=-90/0, 3-6=-73/45, 3-7=-80/2, 4-7=-33/45

BOT CHORD 2-5=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 4, 82 lb uplift at joint 2 and 253 lb uplift at joint 3.

5) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 3.

- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 28 lb down and 31 lb up at 4-2-8, 257 lb down and 79 lb up at 4-2-8, 88 lb down and 59 lb up at 7-0-7, 63 lb down and 42 lb up at 7-0-7, and 22 lb down at 1-4-9, and 22 lb down at 1-4-9 on top chord, and 19 lb down at 1-3-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 5=-9(F) 3=-284(F=-28, B=-257) 7=-151(F=-88, B=-63)



Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

August 26,2008

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818 Soundside Road Edenton, NC 27932

Ply Job Truss Truss Type Qty E5008828 CUNRES C.109 **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 11:00:51 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL., x 9-10-13 -2-1-7 3-6-7 1-10-10 4-5-12 2-1-7 3-6-7 Scale = 1:20.2 0-4-1 3.54 12 11 3x5 =15 2 10x10 = 0-3-13 6.36 12 13 5x6 / 9-10-13 3-6-7 Plate Offsets (X,Y): [8:0-4-0,0-6-12], [9:0-3-0,0-2-6] **PLATES** GRIP SPACING 2-0-0 CSI DEFL I/defl L/d LOADING (psf) TCLL 20.0 1.25 TC Vert(LL) -0.127-8 >938 240 MT20 244/190 Plates Increase 0.71 BC 0.76 -0.33>343 180 TCDL 10.0 Lumber Increase 1.25 Vert(TL) 7-8 WB 0.34 NO n/a n/a BCLL 0.0 Rep Stress Incr Horz(TL) Code FBC2004/TPI2002 (Matrix) Weight: 42 lb BCDL 10.0 BRACING USE MITEK MULTI-BRACE, lumber or other products for bracing LUMBER TOP CHORD 2 X 4 SYP No.2D TOP CHORD BOT CHORD 2 X 4 SYP No.2D Structural wood sheathing directly applied or 5-8-3 oc purlins. 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. (lb/size) 5=222/0-1-8, 2=575/0-8-8, 6=261/Mechanical REACTIONS Max Horz 2=124(LC 3) Max Uplift 5=-60(LC 3), 2=-109(LC 3), 6=-2(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-10=-1194/14, 3-10=-1142/28, 3-11=-1171/32, 4-11=-1133/40, 4-12=-59/0, 5-12=-24/55

BOT CHORD 2-13=-75/1106, 9-13=-75/1106, 9-14=-68/1091, 8-14=-60/1092, 8-15=-82/1010, 7-15=-82/1010, 6-7=0/0

WEBS 3-9=-228/18, 4-8=0/336, 4-7=-1023/83

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 5, 109 lb uplift at joint 2 and 2 lb uplift at joint 6.
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 42 lb down at 4-3-4, 42 lb down at 4-3-4, 99 lb down and 24 lb up at 7-1-3, 99 lb down and 24 lb up at 7-1-3, and 22 lb down at 1-5-4, and 22 lb down at 1-5-4 on top chord, and 20 lb up at 1-5-4, 20 lb up at 1-5-4, 31 lb up at 4-3-4, 31 lb up at 4-3-4, and 17 lb up at 7-1-3, and 17 lb up at 7-1-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) When using MiTek Multi-Brace, attach per Multi-Brace Installation Guide with 2-10d (0.131" X 3") nails. Cross bracing required per BCSI.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2



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RENCO A MiTek Affiliate

Job .	Truss	Truss Type	Qty	Ply	E5008828
CUNRES	C109	ROOF TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS, FL., X

7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 11:00:51 2008 Page 2

LOAD CASE(S) Standard

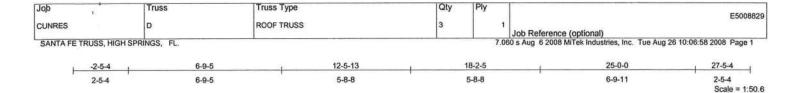
Uniform Loads (plf)

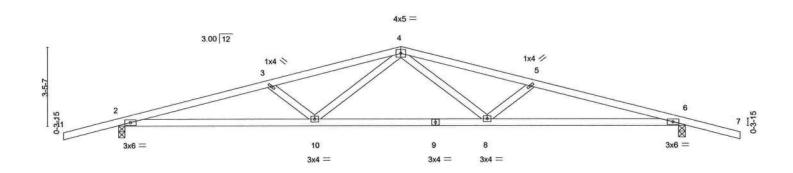
Vert: 1-5=-60, 2-9=-20, 8-9=-20, 6-8=-20

Concentrated Loads (lb)

Vert: 11=-19(F=-10, B=-10) 12=-199(F=-99, B=-99) 13=20(F=10, B=10) 14=31(F=15, B=15) 15=17(F=9, B=9)







	8-8-2		16-3-7			25-0-0				
	8-8-2		7-7-5		8-8-9					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL is	(loc)	I/defi	L/d	PLATES	GRIP		
TCLL 20.0	Plates Increase 1.25	TC 0.33	Vert(LL) 0.2	6-8	>999	240	MT20	244/190		
TCDL 10.0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0.48	8-10	>622	180				
BCLL 0.0	Rep Stress Incr YES	WB 0.22	Horz(TL) 0.10	6	n/a	n/a				
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)					Weight: 105 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-6-12 oc purlins. Rigid ceiling directly applied or 6-0-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1143/0-3-8, 6=1143/0-3-8

Max Horz 2=-59(LC 6)

Max Uplift2=-538(LC 3), 6=-538(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-2885/1183, 3-4=-2542/1098, 4-5=-2541/1098, 5-6=-2882/1182, 6-7=0/34

BOT CHORD 2-10=-1130/2745, 9-10=-728/1908, 8-9=-728/1908, 6-8=-1080/2742

WEBS

3-10=-441/170, 4-10=-355/696, 4-8=-355/696, 5-8=-439/169

NOTES

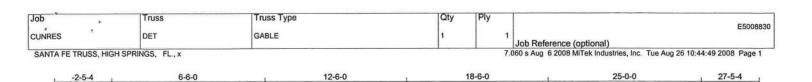
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 538 lb uplift at joint 2 and 538 lb uplift at

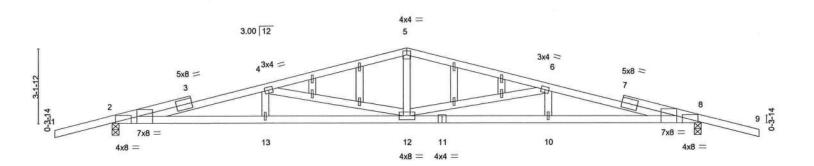
LOAD CASE(S) Standard





6-0-0

12-6-0



	1										
		6-6-0	8!	•	5-0-0		6-0-0		0.80	6-6-0	
Plate Offsets (X,Y): [2:0-0-7,Edge], [2:0-3-5,Edge], [8:0-0-7,Edge], [8:0-3-5,Edge]											
LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in (lo	c) I/de	1 L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	0.46 12-	13 >64	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.70 10-	12 >42	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.14	8 n/	a n/a	2002-00024-250-0004-00042-00	
BCDL	10.0	Code FBC2004/TI	PI2002	(Mati	rix)	.00000000000000000000000000000000000000				Weight: 121 lb)

LUMBER TOP CHORD 2 X 4 SYP No.2D

6-6-0

6-6-0

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WERS **OTHERS**

2-5-4

2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

18-6-0

USE MITEK MULTI-BRACE, lumber or other products for bracing

25-0-0

Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 4-6-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1143/0-3-8, 8=1143/0-3-8

Max Horz 2=71(LC 3)

Max Uplift 2=-733(LC 5), 8=-733(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/34, 2-3=-3545/2008, 3-4=-3510/2016, 4-5=-2276/1270, 5-6=-2276/1270, 6-7=-3510/2018, 7-8=-3545/2010, TOP CHORD

8-9=0/34

2-13=-1962/3436, 12-13=-1962/3436, 11-12=-1901/3436, 10-11=-1901/3436, 8-10=-1901/3436 **BOT CHORD**

4-13=-90/237, 4-12=-1302/839, 5-12=-439/730, 6-12=-1302/841, 6-10=-89/237 **WEBS**

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; porch left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- All plates are 1x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 733 lb uplift at joint 2 and 733 lb uplift at joint 8.
- 8) When using MiTek Multi-Brace, attach per Multi-Brace Installation Guide with 2-10d (0.131" X 3") nails. Cross bracing required per BCSI.

LOAD CASE(S) Standard



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Scale = 1:48.6



Truss Truss Type Qty Ply Job E5008831 EET **ROOF TRUSS** 2 CUNRES 3x4 = Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:59 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 4-6-0 2-3-0 2-3-0 2-3-0 5.00 12 3 2-3-0 2-3-0 2-3-0 Plate Offsets (X,Y): [2:0-2-0,Edge] DEFL I/defl L/d **PLATES** GRIP SPACING 2-0-0 CSI LOADING (psf) (loc) TC BC 244/190 0.06 999 MT20 TCLL 20.0 Plates Increase 1.25 Vert(LL) n/a n/a TCDL 10.0 Lumber Increase 1.25 0.18 Vert(TL) n/a n/a 999 BCLL Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 3 n/a n/a 0.0 BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb BRACING LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=180/4-6-0, 3=180/4-6-0 Max Horz 1=11(LC 5) Max Uplift1=-19(LC 5), 3=-19(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-192/53, 2-3=-192/53

BOT CHORD 1-3=-35/156

NOTES

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 19 lb uplift at joint 3.

LOAD CASE(S) Standard



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f the nt

Qty Ply Job Truss Truss Type E5008832 2 CUNRES FGT ROOF TRUSS 4x4 = Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:06:59 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 2 2-3-0 2-3-0 2-3-0 Scale = 1:8 1 5.00 12 3 2x4 || 5 2-3-0 4-6-0 2-3-0 2-3-0 DEFL **PLATES** GRIP LOADING (psf) CSI L/d SPACING 2-0-0 **V**defl in (loc) 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.06 Vert(LL) -0.01>999 240 MT20 TCDL 10.0 Lumber Increase 1.25 BC 0.39 Vert(TL) -0.023-4 >999 180 BCLL 0.0 Rep Stress Incr NO WB 0.21 Horz(TL) 0.00 3 n/a n/a Code FBC2004/TPI2002 Weight: 19 lb BCDL 10.0 (Matrix) **BRACING** LUMBER TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing

Installation guide

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.3 WEBS

REACTIONS (lb/size) 1=838/0-5-8, 3=646/0-5-8

Max Horz 1=-10(LC 6)

Max Uplift1=-83(LC 5), 3=-85(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-979/124, 2-3=-979/124 TOP CHORD

BOT CHORD 1-4=-101/878, 4-5=-101/878, 3-5=-101/878

WEBS 2-4=-63/656

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 1 and 85 lb uplift at ioint 3.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 340 lb down and 19 lb up at 0-2-12, and 308 lb down and 42 lb up at 2-1-8, and 513 lb down and 73 lb up at 2-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

Concentrated Loads (lb)

Vert: 1=-340(B) 4=-308(B) 5=-513(B)



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Design Valid for use only with Milek Connectors. This design is based only upon parameters snown, and is for an individual billiang Component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Fracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult MSIX/PII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Truss Type Qty Ply Truss Job E5008833 **ROOF TRUSS** 30 CUNRES EJ7 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:00 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 7-0-0 7-0-0 1-6-0 3 Scale = 1:18.6 5.00 12 0-4-1 2x4 = 7-0-0 7-0-0 PLATES GRIP LOADING (psf) SPACING CSI DEFL I/defl L/d TC 0.46 Vert(LL) -0.10 2-4 >805 240 MT20 244/190 TCLL 20.0 Plates Increase 1.25 0.38 -0.25 180 TCDL 10.0 Lumber Increase 1.25 BC Vert(TL) >322 WB 0.00 Horz(TL) -0.00 n/a n/a BCLL 00 Rep Stress Incr YES Code FBC2004/TPI2002 Weight: 24 lb BCDL 10.0 (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=185/Mechanical, 2=383/0-5-8, 4=67/Mechanical

Max Horz 2=123(LC 5)

Max Uplift3=-83(LC 5), 2=-100(LC 5)

Max Grav 3=185(LC 1), 2=383(LC 1), 4=134(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-85/58

BOT CHORD 2-4=0/0

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3 and 100 lb uplift at joint 2

LOAD CASE(S) Standard





Truss Type Qty Ply Job Truss E5008834 **ROOF TRUSS** CUNRES EJ7A Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:00 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -1-6-0 2-6-0 3-10-0 7-0-0 5 3-2-0 1-6-0 2-6-0 1-4-0 Scale = 1:18.6 1x4 || 5.00 12 1x4 || 0-4-1 5x5 = 9.00 12 4x4 = 2x4 = 2-6-0 3-10-0 7-0-0 3-2-0 1-4-0 Plate Offsets (X,Y): [8:0-2-8,0-2-4] **PLATES** GRIP DEFL I/defl L/d LOADING (psf) SPACING 2-0-0 CSI (loc) MT20 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.64 Vert(LL) -0.16>492 240 BC TCDL 1.25 0.08 Vert(TL) -0.417 >198 180 10.0 Lumber Increase WB 0.03 0.02 5 BCLL 0.0 Rep Stress Incr YES Horz(TL) n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=224/0-1-8, 2=385/0-5-8, 6=31/Mechanical

Max Horz 2=124(LC 5)

Max Uplift5=-66(LC 5), 2=-101(LC 5)

Max Grav 5=224(LC 1), 2=385(LC 1), 6=62(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-118/0, 3-4=-58/0, 4-5=-36/79

BOT CHORD 2-8=0/15, 7-8=0/42, 6-7=-0/0

WEBS

3-8=0/56, 4-7=0/108

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 5 and 101 lb uplift at joint 2.
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

LOAD CASE(S) Standard



August 26,2008



818 Soundside Road Edenton, NC 27932

Truss Type Qty Ply Job Truss E5008835 ROOF TRUSS J01 CUNRES Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:01 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 1-0-0 5-0-7 1-0-0 4-0-7 Scale = 1:13.5 5.00 12 3

		1-		4-0-7								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	1	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	1	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2004/TI	PI2002	(Mati	ix)	12.000000000000000000000000000000000000					Weight: 10 l	b

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD **BOT CHORD**

5-0-7

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=157/Mechanical, 2=148/Mechanical, 3=9/Mechanical

2x4 1=0-0

Max Horz 1=68(LC 5)

Max Uplift1=-35(LC 5), 2=-69(LC 5)

Max Grav 1=157(LC 1), 2=148(LC 1), 3=19(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-46/46

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1 and 69 lb uplift at ioint 2.

LOAD CASE(S) Standard



August 26,2008



Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Qty Ply Job Truss Truss Type E5008836 CUNRES J01A ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MITek Industries, Inc. Tue Aug 26 10:07:01 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 1-0-0 3-0-7 2-0-7 1-0-0 Scale = 1:9.7 5.00 12 1-2-_1-0-0 3-0-7 2-0-7 1-0-0 LOADING (psf) DEFL **PLATES** GRIP SPACING CSI L/d in I/defl 2-0-0 (loc) 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.10 Vert(LL) -0.00 >999 240 MT20 TCDL 10.0 Lumber Increase 1.25 BC 0.01 Vert(TL) -0.00 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.002 n/a n/a Code FBC2004/TPI2002 (Matrix) Weight: 7 lb BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 1=97/0-5-8, 2=88/Mechanical, 3=9/0-3-8

Max Horz 1=41(LC 5)

Max Uplift1=-19(LC 5), 2=-41(LC 5)

Max Grav 1=97(LC 1), 2=88(LC 1), 3=19(LC 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=-28/28 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1 and 41 lb uplift at joint 2.

LOAD CASE(S) Standard





Qty Joh Truss Truss Type Ply E5008837 CUNRES J01B ROOF TRUSS Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:02 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 3 1-0-0 -1-6-0 1-0-0 1-6-0 Scale = 1:6.3 5.00 12 4 2x4 = 1,0,7 1-0-0 0-0-7 1-0-0 **PLATES** GRIP CSI DEFL I/defl L/d LOADING (psf) SPACING 2-0-0 in (loc) 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.12 Vert(LL) -0.00 2 >999 240 MT20 TCDL 1.25 BC 0.01 Vert(TL) -0.002 >999 180 10.0 Lumber Increase YES WB 0.00 -0.00 3 n/a n/a BCLL 0.0 Rep Stress Incr Horz(TL) Weight: 6 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 2=205/0-5-8, 4=9/Mechanical, 3=-44/Mechanical

Max Horz 2=45(LC 5)

Max Uplift2=-124(LC 5), 3=-44(LC 1)

Max Grav 2=205(LC 1), 4=19(LC 2), 3=44(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-44/18

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 2 and 44 lb uplift at joint 3.

LOAD CASE(S) Standard



August 26,2008



Job ,	Truss	Truss Type	Qty	Ply	E5008838
CUNRES	J07	ROOF TRUSS	4	1 Job Reference (entional)	CONTRACT CONTRACT
SANTA FE TRUSS, H	IGH SPRINGS, FL.			7.060 s Aug 6 2008 MiTek Indust	ries, Inc. Tue Aug 26 10:07:02 2008 Page 1
	-1-6-0		5-0-7		7-0-0
I	1-6-0		5-0-7	3	1-11-9 Scale = 1:14.7
			2		
		5.00 12			
2-5-4					
2					
	2				ū
0.4-1					638
		1			
'					4
	2x4 =	=			
	<u> </u>		5-0-7		7-0-0
			5-0-7		1-11-9
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING Plates Increase Lumber Increase	2-0-0 CSI 1.25 TC 0.20 1.25 BC 0.38	DEFL in Vert(LL) -0.10 Vert(TL) -0.25	(loc) I/defl L/d 2-4 >805 240 2-4 >322 180	PLATES GRIP MT20 244/190
BCLL 0.0 BCDL 10.0	Rep Stress Incr Code FBC2004/TI	YES WB 0.00 Pl2002 (Matrix)	Horz(TL) -0.00	3 n/a n/a	Weight: 21 lb
LUMBER			BRACING		

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=123/Mechanical, 2=332/0-5-8, 4=67/Mechanical

Max Horz 2=98(LC 5)

Max Uplift3=-53(LC 5), 2=-89(LC 5)

Max Grav3=123(LC 1), 2=332(LC 1), 4=134(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-65/39

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 3 and 89 lb uplift at joint 2.

LOAD CASE(S) Standard



August 26,2008

Job Truss Truss Type Qty Ply E5008839 CUNRES J07A **ROOF TRUSS** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:03 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. -1-6-0 3-0-7 7-0-0 1-6-0 3-0-7 3-11-9 Scale = 1:14.2 1x4 = 5.00 12 04-1 3x10 = 6 5 2x4 = 3-0-7 7-0-0 3-0-7 3-11-9 DEFI. PLATES GRIP SPACING CSI L/d LOADING (psf) 2-0-0 in (loc) I/defl TCLL 20.0 Plates Increase 1.25 TC 0.15 Vert(LL) -0.102-6 >829 240 MT20 244/190 TCDL 10.0 1.25 BC 0.38 Vert(TL) -0.242-6 >332 180 Lumber Increase BCLL YES WB 0.04 0.00 n/a 0.0 Rep Stress Incr Horz(TL) n/a Weight: 25 lb BCDL 10.0 Code FBC2004/TPI2002 (Matrix) LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

REACTIONS (lb/size) 2=317/0-5-8, 6=92/Mechanical

Max Horz 2=69(LC 5) Max Uplift2=-89(LC 5)

Max Grav 2=317(LC 1), 6=147(LC 2)

TOP CHORD

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/34, 2-3=-160/60, 3-4=-19/0

BOT CHORD

2-6=-75/107, 5-6=0/0

WEBS

3-6=-109/77

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2.

LOAD CASE(S) Standard



August 26,2008

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobification, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job *	Truss	Truss Type	Qty	Ply		E5008840
CUNRES	J07B	ROOF TRUSS	4	1	Job Reference (optional)	
SANTA FE TRUSS, H	IGH SPRINGS. FL.			7.06	0 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:03 2008	Page 1
	-1-6-0	1-0-7		7-0	8	



	1-0-7		5-11-9
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I/defl L/d PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.12	Vert(LL) -0.11 2-4 >742 240 MT20 244/190
TCDL 10.0	Lumber Increase 1.25	BC 0.40	Vert(TL) -0.28 2-4 >297 180
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	Weight: 15 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=264/0-5-8, 4=69/Mechanical, 3=-44/Mechanical

Max Horz 2=45(LC 5)

Max Uplift2=-94(LC 5), 3=-44(LC 1)

Max Grav 2=264(LC 1), 4=138(LC 2), 3=44(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD

1-2=0/33, 2-3=-44/18 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 2 and 44 lb uplift at joint 3.

LOAD CASE(S) Standard



August 26,2008



Edenton, NC 27932

(Job Truss Truss Type Qty Ply E5008841 CUNRES J1 **ROOF TRUSS** 2 Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:03 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL 3 -1-6-0 1-0-7 1-6-0 1-0-7 Scale = 1:6.3 5.00 12 2x4 = LOADING (psf) SPACING CSI DEFL 2-0-0 in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.12 Vert(LL) -0.00 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.01 Vert(TL) -0.00 >999 180 BCLL 0.0 YES WB 0.00 -0.00 3 Rep Stress Incr Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 6 lb

BRACING

TOP CHORD

BOT CHORD

REACTIONS (lb/size) 2=206/0-5-8, 4=10/Mechanical, 3=-46/Mechanical Max Horz 2=44(LC 5)

Max Uplift2=-125(LC 5), 3=-46(LC 1)

Max Grav 2=206(LC 1), 4=20(LC 2), 3=46(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-44/19

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D

BOT CHORD 2-4=0/0

NOTES

LUMBER

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 2 and 46 lb uplift at joint 3.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-0-7 oc purlins.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

August 26,2008



Job Qty Truss Truss Type Ply E5008842 ROOF TRUSS 2 CUNRES **J3** Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:04 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 1-6-0 3-0-7 1-6-0 3-0-7 1x4 || Scale = 1:10.1 3 5.00 12 2 0-3-11 0-4-1 9.00 12 4x4 = 2x4 = 2-6-0 3-0-7 2-6-0 0-6-7 Plate Offsets (X,Y): [6:0-2-8,0-2-4] LOADING (psf) SPACING 2-0-0 DEFL I/defl **PLATES** GRIP (loc) L/d TCLL 20.0 Plates Increase 1.25 TC 0.15 Vert(LL) -0.00 6 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.04 Vert(TL) -0.01 2-6 >999 180 WB 0.02 -0.00 5 Rep Stress Incr YES n/a n/a Horz(TL)

0.0 BCLL

BCDL

LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3

10.0

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-7 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

Weight: 14 lb

REACTIONS (lb/size) 4=70/Mechanical, 2=244/0-5-8, 5=5/Mechanical

Code FBC2004/TPI2002

Max Horz 2=70(LC 5)

Max Uplift4=-8(LC 4), 2=-103(LC 5) Max Grav 4=70(LC 1), 2=244(LC 1), 5=9(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/34, 2-3=-47/6, 3-4=-5/26 2-6=-8/0, 5-6=-5/5

BOT CHORD WEBS

3-6=0/55

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 4 and 103 lb uplift at

LOAD CASE(S) Standard



August 26,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding (abrication, quality control, storage, delivery, erection and bracing, consult

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information

available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



818 Soundside Road Edenton, NC 27932

Qty Job Truss Type Ply Truss E5008843 ROOF TRUSS 2 CUNRES 15 Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Tue Aug 26 10:07:04 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS, FL. 2-6-0 5 1-6-0 2-6-0 1-4-0 1-2-7 Scale = 1:14.8 1x4 || 5.00 12 1x4 || 7 5x5 = 0-4-1 9.00 12 8 4×4 = 2-6-0 3-10-0 5-0-7 2-6-0 1-4-0 1-2-7 Plate Offsets (X,Y): [8:0-2-8,0-2-4] SPACING 2-0-0 DEFL l/defl **PLATES** GRIP LOADING (psf) (loc) TCLL 20.0 Plates Increase 1.25 TC 0.28 Vert(LL) -0.048 >999 240 MT20 244/190 TCDL 10.0 Lumber Increase 1.25 BC 0.04 Vert(TL) -0.10 8 >554 180 WB 0.02 -0.04BCLL 0.0 Rep Stress Incr YES 6 Horz(TL) n/a n/a Code FBC2004/TPI2002 10.0 (Matrix) Weight: 21 lb BCDL

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=159/0-1-8, 2=312/0-5-8, 6=11/Mechanical

Max Horz 2=98(LC 5)

Max Uplift5=-35(LC 5), 2=-98(LC 5)

Max Grav 5=159(LC 1), 2=312(LC 1), 6=23(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/34, 2-3=-82/0, 3-4=-31/17, 4-5=-17/59 **BOT CHORD**

2-8=0/4, 7-8=0/29, 6-7=-0/0 WEBS 3-8=0/63, 4-7=0/60

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); TCDL=5.0psf; BCDL=5.0psf; h=18ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 5 and 98 lb uplift at
- 6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.

LOAD CASE(S) Standard



August 26,2008



Edenton, NC 27932



COLUMBIA COUNTY, FLORIDA

epartment of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Building permit No. 000027450

36.66

Fire:

Parcel Number 16-6S-17-09710-000

Use Classification SFD, UTILITY

Permit Holder OWNER BUILDER

Owner of Building JERRY CUNNINGHAM

86.91

Total:

50.25

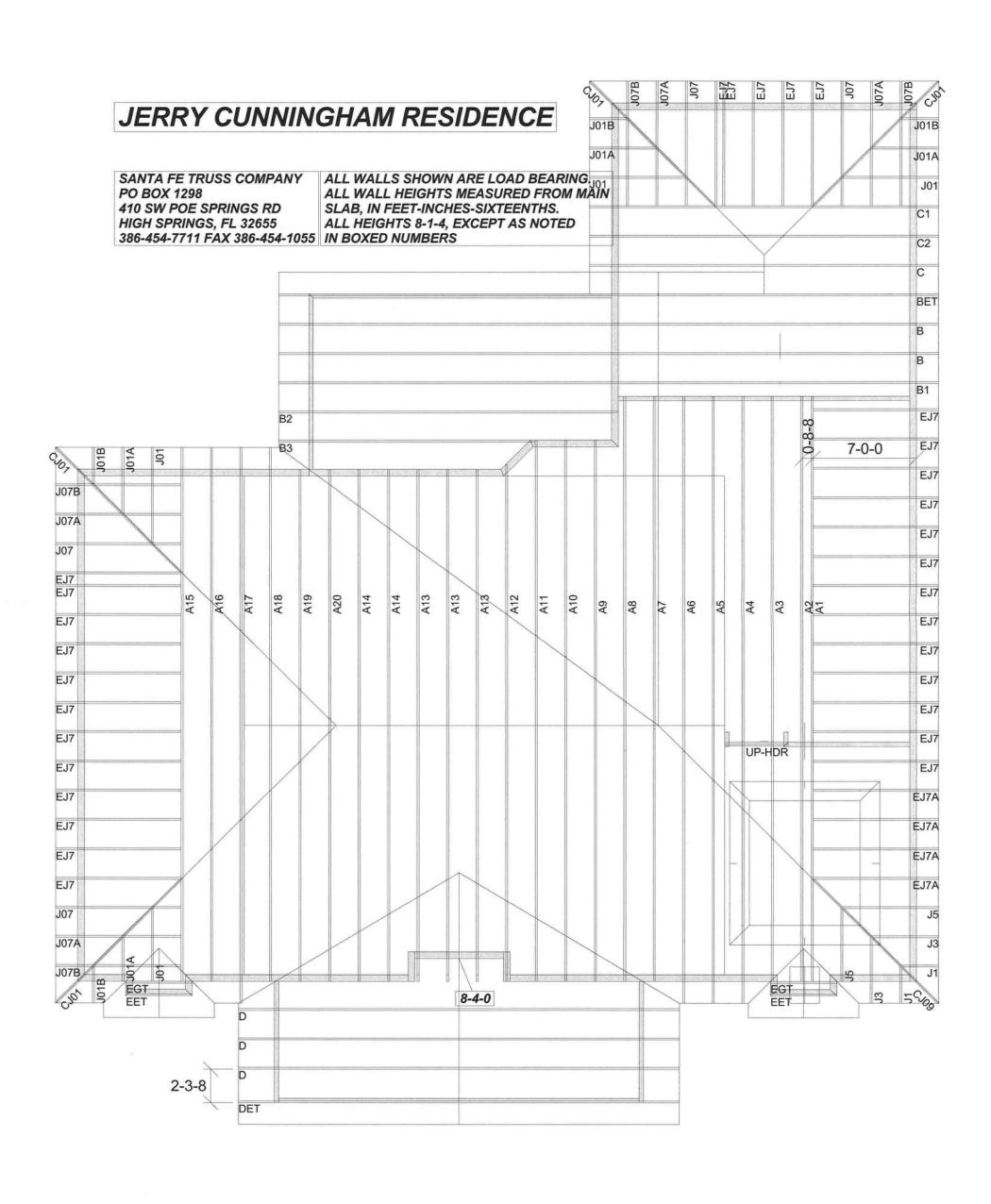
Waste:

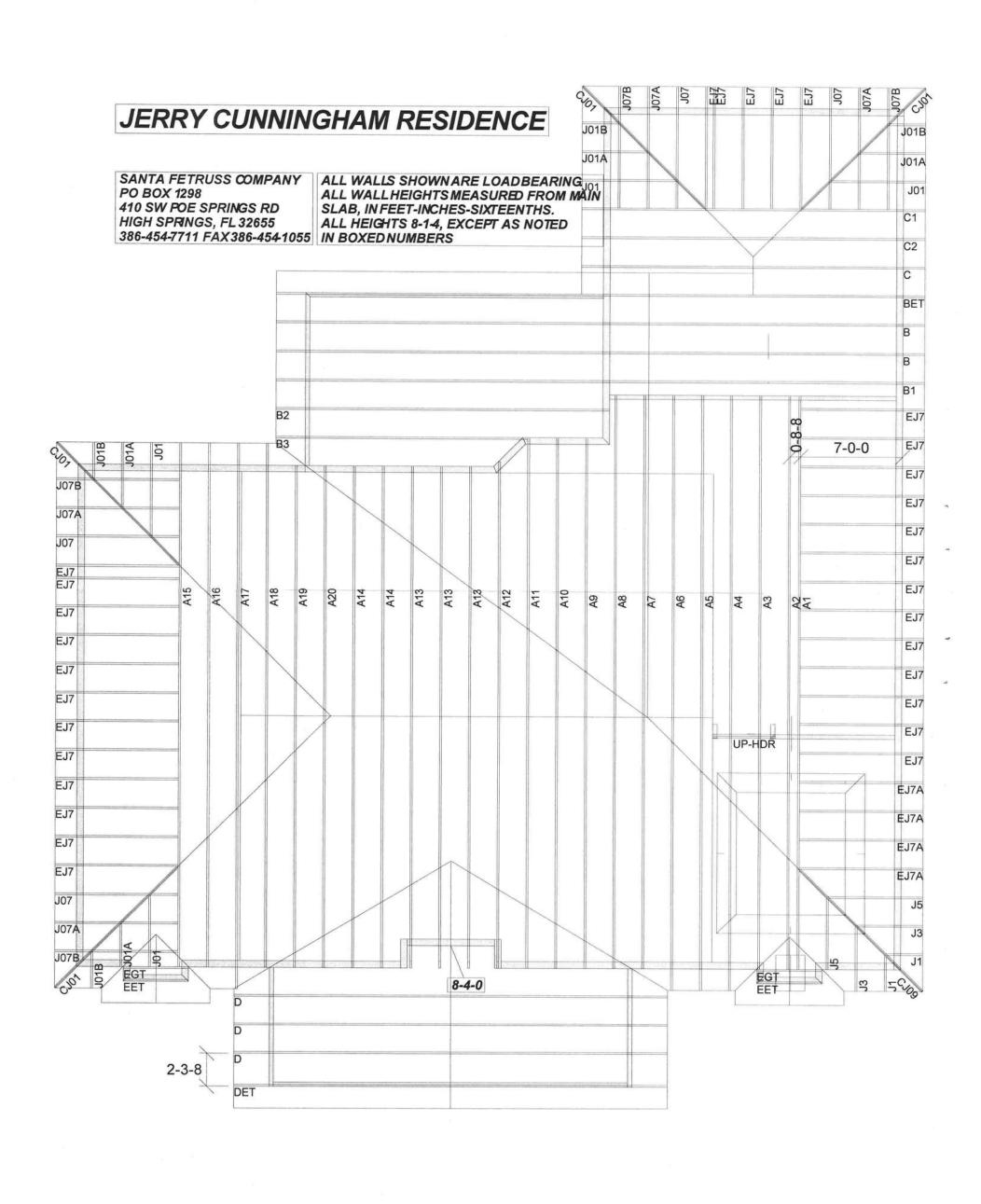
529 SW HAMMOCK HILL CIRCLE, LAKE CITY, FL Location:

Date: 07/27/2009

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)







5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642 FAX (352) 373-9037

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Applicator:	% Concentration:	Product Used:	Area Treated:	Site Location:	Date:	Builder:	
Tem	.06% # Gallons Used:	Talstar P. Chemical Used:	Living Entry Garage Porch	529 SW Hammax Hill Circle	13-10-08	erry Charinghan	CERTIFICATE OF PROTECTIVE TREATMENT
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