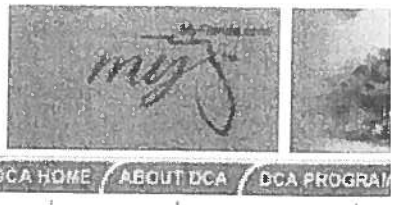


Product Approval
USER: Public User

FL #	FL1214-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer Address/Phone/Email	Alenco 615 Carson Bryan, TX 77802 (979) 779-7770 ext 343 mkoppers@alenco.com
Authorized Signature	Martin Koppers mkoppers@alenco.com
Technical Representative Address/Phone/Email	Martin Koppers 615 Carson St. Bryan, TX 77802 mkoppers@alenco.com
Quality Assurance Representative Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	National Accreditation & Management Institute,
Referenced Standard and Year (of	**Standard**

Standard)	AAMA/NWWDA 101/I.S.2
Equivalence of Product Standards Certified By	
Sections from the Code	1707.4.2.1
Product Approval Method	Method 1 Option A
Date Submitted	06/08/2005
Date Validated	08/04/2005
Date Pending FBC Approval	06/18/2005
Date Approved	08/05/2005

Summary of Products

FL #	Model, Number or Name	Description
1214.1	1111	Vinyl Tilt Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 1111: 48X72 R(35) Tested with DS annealed,44X72 R(40)Tested with SS annealed. For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** PTID 1214 R1 I FL INSTALLATION INSTRUCTIONS - Aluminum B.pdf PTID 1214 R1 I INSTALLATION INSTRUCTIONS - Vinyl B.pdf Verified By:
1214.2	3753	Aluminum Tilt Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 3753:44X72 R(40) Tested with Tested with DS annealed.For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** Verified By:
1214.3	4710F	Aluminum Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 4710F:48X72 R(40)/DP(50), Tested with DS annealed glass.For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** Verified By:

Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

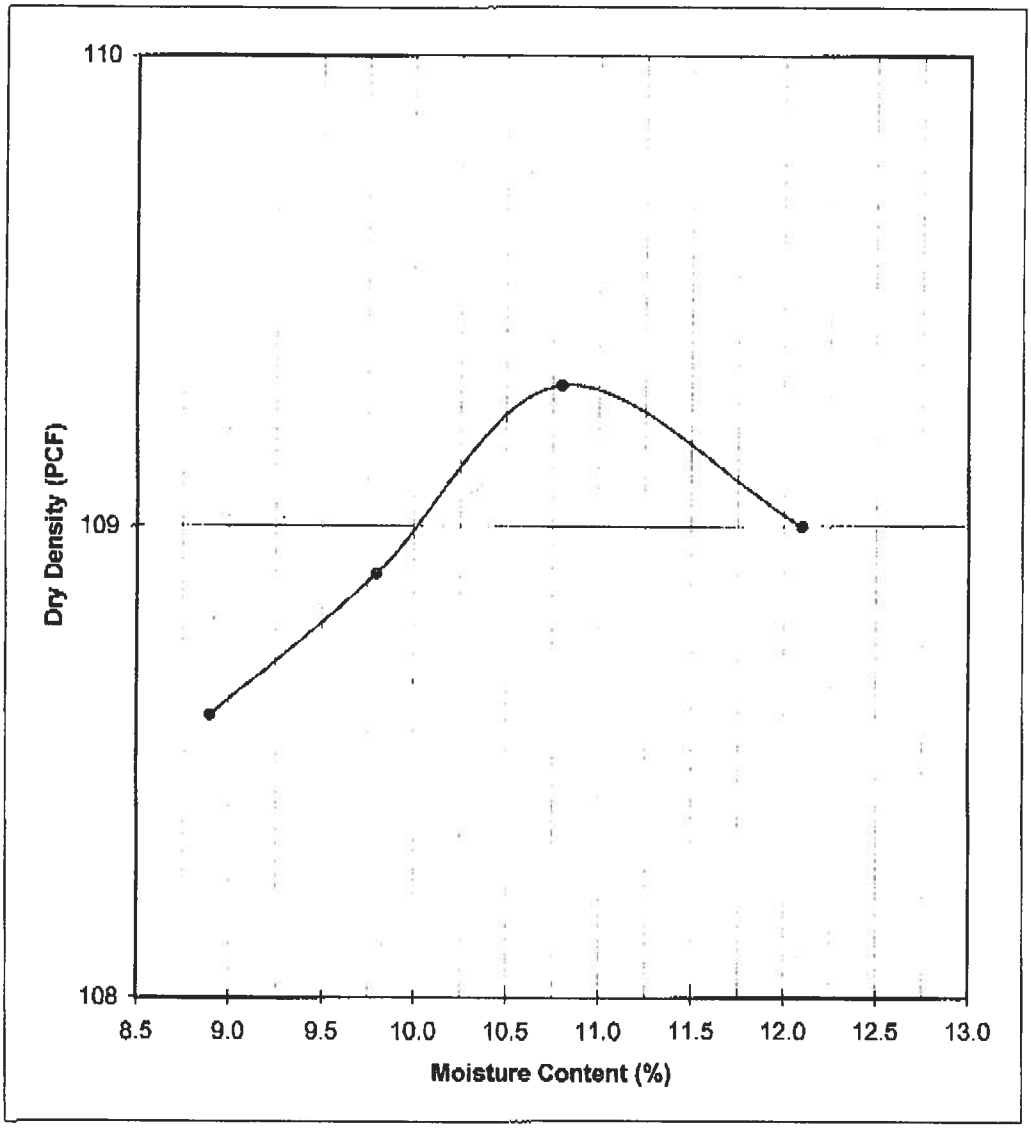
P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
6919 Distribution Ave. S., Unit #5, Jacksonville, FL 32257 • Tel(904)262-4046 • Fax(904)4047
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

#26563

REPORT OF LABORATORY COMPACTION TEST

Client: Mike Roberts Framing, 657 SW Catherine Lane, Lake City, FL 32025
Project Name: Lot 12, Crosswinds Subdivision
Project Location: Lake City, FL
Contractor: Mike Roberts Framing

File No: 08-00044-01
Date: 1/14/2008
Lab No: 10644



PROCTOR DATA

Proctor No.: 1

Modified Proctor (ASTM D-1557) ☑

Standard Proctor (ASTM D-698) ☐

Maximum Dry Dens. Pcf: 109.2

Optimum Moisture Percent: 10.9

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Sample Description: Tan Sand
Sample Location: Existing Building Pad
Proposed Use: House Pad
Sampled By: Chad Day **Date:** 1/11/2008
Tested By: Tim Cassidy **Date:** 1/14/2008
Remarks: 1cc: Client
1cc: File

Linda Creamer, CEO, DBE
Linda M. Creamer
President - CEO
Reviewed By:
Date: 1/16/08
Licensed, Florida No.: 57842

ee

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00044-01
DATE TESTED: 1/11/08
DATE REPORTED: 1/14/08

PROJECT:	Lot 12, Crosswinds Subdivision, Lake City
CLIENT:	Mike Roberts Framing, 657 SW Catherine Lane, Lake City, FL 32025
GENERAL CONTRACTOR:	Mike Roberts Framing
EARTHWORK CONTRACTOR:	Mike Roberts Framing
INSPECTOR:	Chad Day

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	BUILDING FILL

SPECIFICATION REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		SW Corner, 12' East x 10' North	12"	114.4	7.0	106.9	1	109.2	98%
2		NW Corner, 14' East x 8' South	12"	113.5	8.6	104.5	1	109.2	96%
3		East Side Approx. Center, 8' West	12"	114.3	8.0	105.8	1	109.2	97%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
1		Tan Sand	109.2	10.9	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 1/14/08
Licensed, Florida No: 57842

ee

Cal-Tech Testing, Inc.
• Engineering
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REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00044-01
DATE TESTED: 1/11/08
DATE REPORTED: 1/14/08

PROJECT:	Lot 12, Crosswinds Subdivision, Lake City
CLIENT:	Mike Roberts Framing, 657 SW Catherine Lane, Lake City, FL 32025
GENERAL CONTRACTOR:	Mike Roberts Framing
EARTHWORK CONTRACTOR:	Mike Roberts Framing
INSPECTOR:	Chad Day

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	BUILDING FILL

SPECIFICATION REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
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3		East Side Approx. Center, 8' West	12"	114.3	8.0	105.8	1	109.2	97%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Tan Sand	109.2	10.9	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 1/14/08
Licensed, Florida No: 57842

ee

[redacted]

Short Form
Entire House

Ahlbrandt Ref. Inc.

Job:
Date: May 16, 2007
By:

P O Box 1945, Alachua, Fl 32616 Phone: 352-225-1308 Fax: 386-418-0549

Project Information

For: Mike Roberts

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

HEATING EQUIPMENT

Make York
Trade Guarden
Model HP030X1321

Efficiency	8 HSPF	
Heating input		
Heating output	30000	Btuh @ 47°F
Temperature rise	29	°F
Actual air flow	933	cfm
Air flow factor	0.035	cfm/Btuh
Static pressure	0.10	in H2O
Space thermostat		

COOLING EQUIPMENT

Make York
Trade Guarden
Cond HP030X1321
Coil G2FD036S17+1TV0701

Efficiency	13 SEER	
Sensible cooling	19600	Btuh
Latent cooling	8400	Btuh
Total cooling	28000	Btuh
Actual air flow	933	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.10	in H2O
Load sensible heat ratio	0.69	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Dining rm	131	3391	1820	119	87
Kitchen	108	1408	2284	50	109
Bath	52	975	495	34	24
Bedrm 2	182	5143	3325	181	159
Bedrm 3	186	3195	2343	112	112
Fam rm	357	4398	4219	155	202
Master bedrm	219	3379	3010	119	144
Master bath	78	2926	1128	103	54
W.I.c.	48	1531	569	54	27
core	132	189	314	7	15

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House Other equip loads Equip. @ 0.97 RSM Latent cooling	1494	26536 5651	19507 2596 21440 9979	933	933
TOTALS	1494	32187	31419	933	933

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

TAMKO ROOFING PRODUCTS

• Glass-Seal • Glass-Seal AR • Elite Glass-Seal® • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Nailing Pattern.

6. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

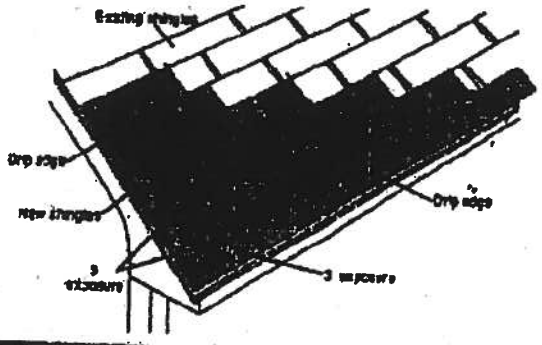
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the



rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.

8. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50-lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

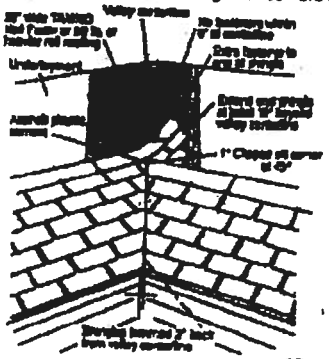
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- *CAUTION: Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

TAMKO
ROOFING PRODUCTS

FEB - 4 REC'D

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

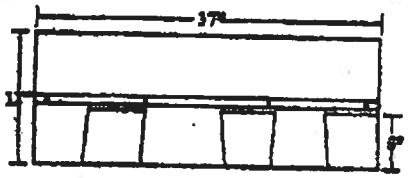
TAMKO Roofing Products, Inc.

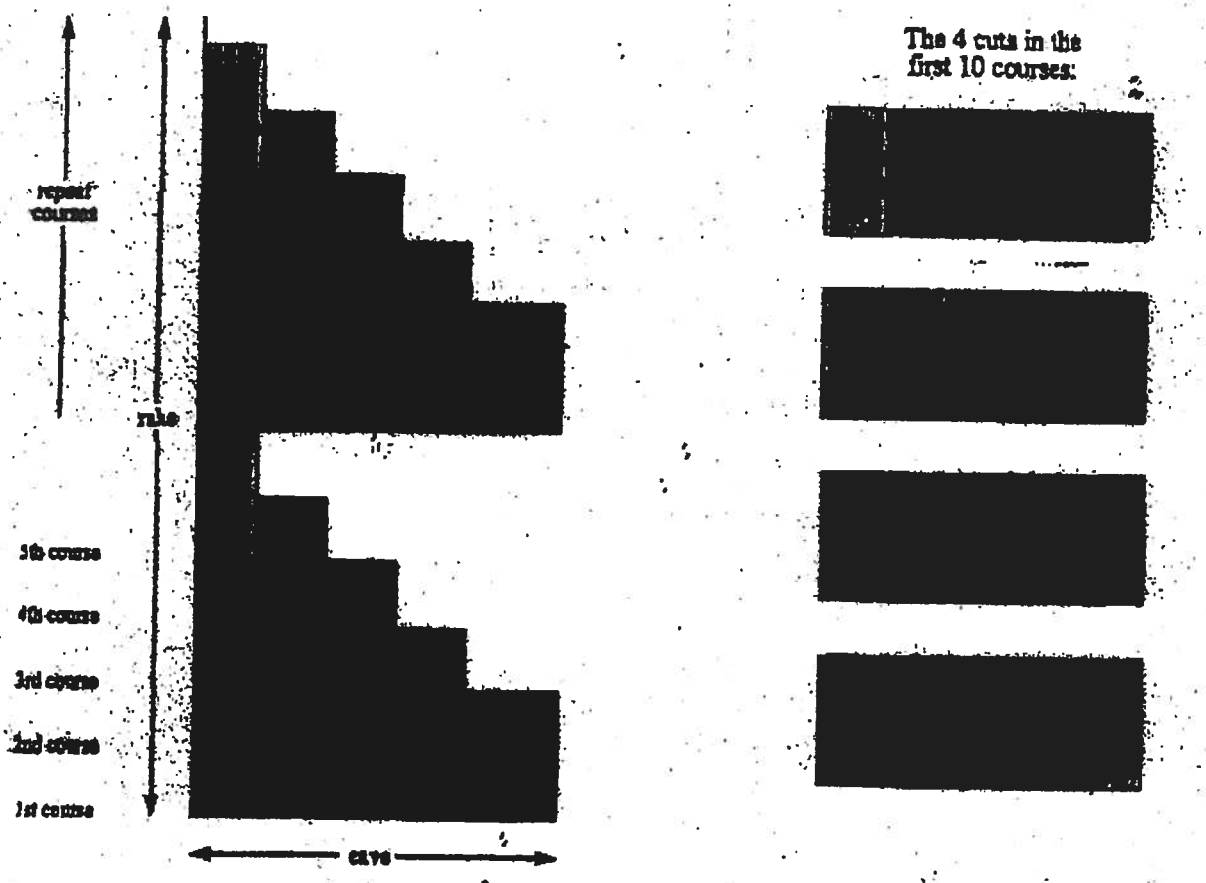
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1926

TAMKO
ROOFING PRODUCTS

Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"





In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO ROOFING PRODUCTS

Application Instructions for

• Glass-Seal • Elite Glass-Seal®
• Glass-Seal AR • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.
THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.
IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 8 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

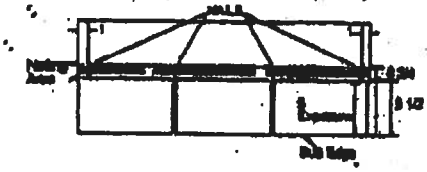
3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

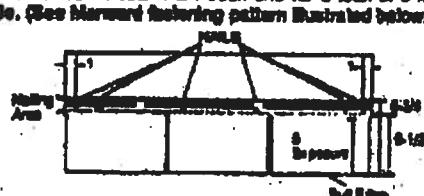
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) Stanpard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)



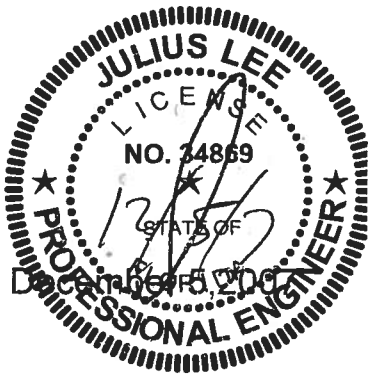
NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

Visit Our Web Site at www.tamko.com

Central District	220 West 4th St., Joplin, MO 64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD 21701	800-368-2065
Southeast District	2300 36th St., Tuscaloosa, AL 35401	800-228-2369
Southwest District	7910 S. Central Exp., Dallas, TX 75216	800-443-1534
Western District	5300 East 43rd Ave., Denver, CO 80216	800-530-8569

Builders FirstSource



Project Information for: **L262673**

Address:	474 Southwest Chesterfield Road
	Lake City, Florida
County:	Columbia
Truss Count:	28
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date
Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Seal Date
1	J1915406	CJ1	12/5/07
2	J1915407	CJ3	12/5/07
3	J1915408	CJ5	12/5/07
4	J1915409	EJ7	12/5/07
5	J1915410	EJ7A	12/5/07
6	J1915411	EJ7B	12/5/07
7	J1915412	HJ3	12/5/07
8	J1915413	HJ7	12/5/07
9	J1915414	HJ9	12/5/07
10	J1915415	T01	12/5/07
11	J1915416	T02	12/5/07
12	J1915417	T03	12/5/07
13	J1915418	T04	12/5/07
14	J1915419	T05	12/5/07
15	J1915420	T06	12/5/07
16	J1915421	T07	12/5/07
17	J1915422	T08	12/5/07
18	J1915423	T09	12/5/07
19	J1915424	T10	12/5/07
20	J1915425	T11	12/5/07
21	J1915426	T12	12/5/07
22	J1915427	T13	12/5/07
23	J1915428	T14	12/5/07
24	J1915429	T15	12/5/07
25	J1915430	T16	12/5/07
26	J1915431	T17	12/5/07
27	J1915432	T18	12/5/07
28	J1915433	T19	12/5/07

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915406
L262673	CJ1	JACK	16	1	Job Reference (optional)

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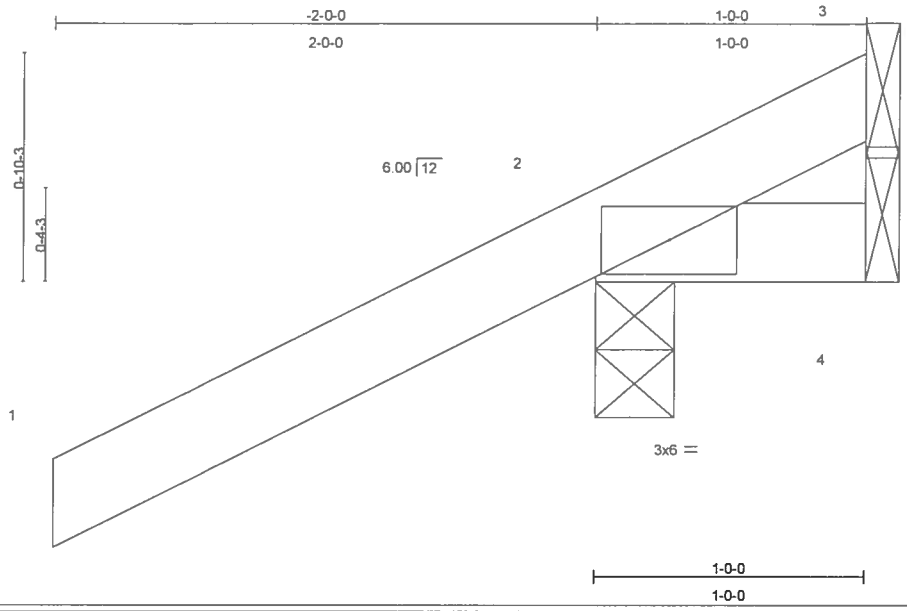

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)
Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31781
1005 Crandall Bay Blvd
Jacksonville Beach, FL 32250

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915406
L262673	CJ1	JACK	16	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1061 Crandall Bay Blvd
Jacksonville, FL 32218

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915407
L262673	CJ3	JACK	14	1	Job Reference (optional)

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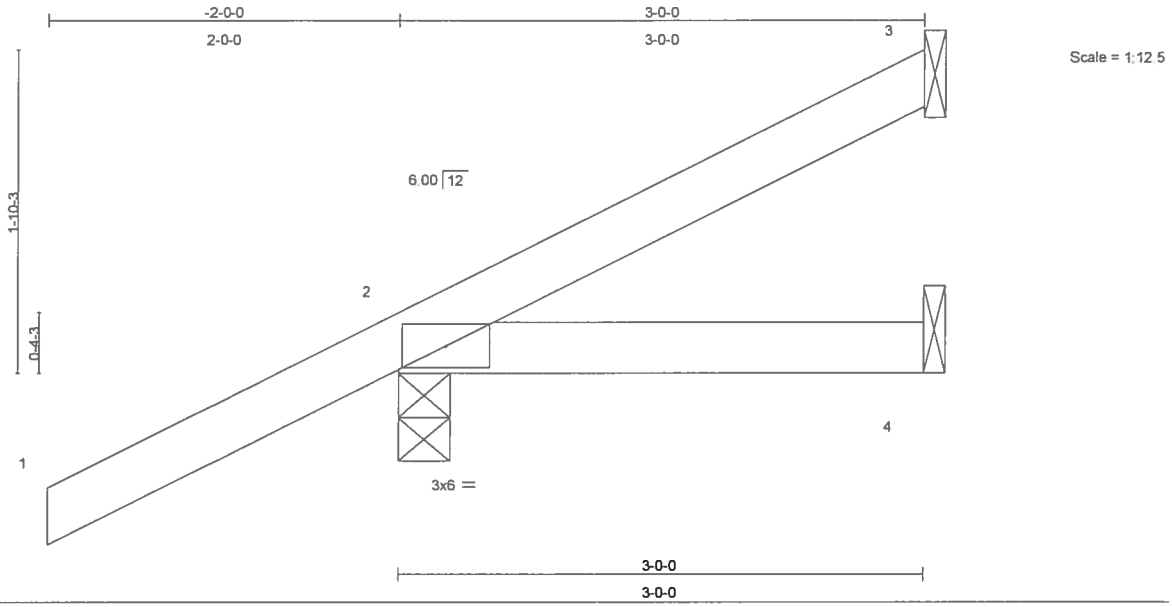

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31801
1103 Corneal Bay Blvd
Ocoee, FL 32761

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915407
L262673	CJ3	JACK	14	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34931
1104 Emerald Bay Run
Destin, FL 32541

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915408
L262673	CJ5	JACK	14	1	Job Reference (optional)

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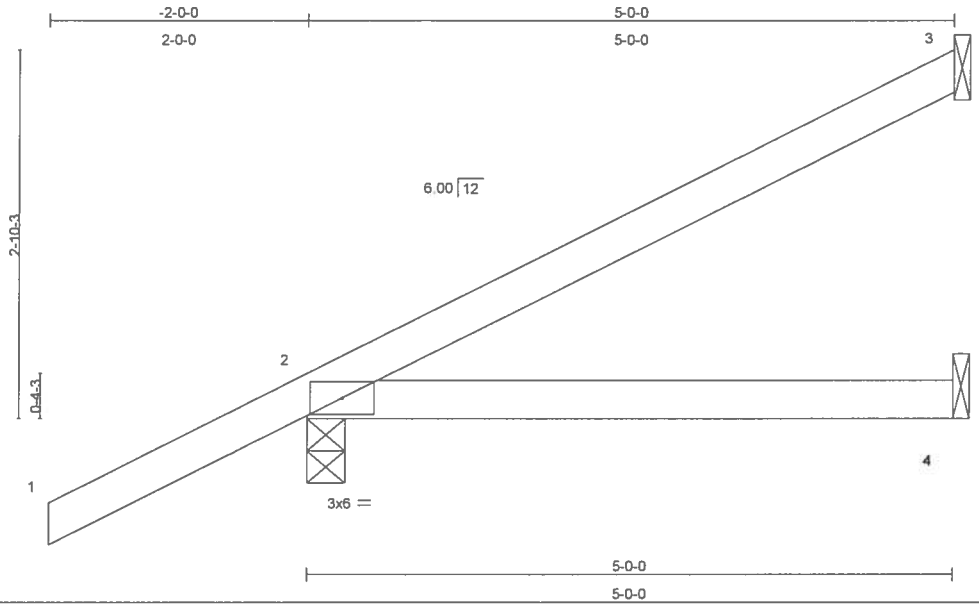

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No 34701
1150 Cromwell Ave #104
Bevron Beach, FL 33445

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915408
L262673	CJ5	JACK	14	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1104 Emmitt Bay Path
Ocynion Beach, FL 32435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915409
L262673	EJ7	JACK	30	1	Job Reference (optional)

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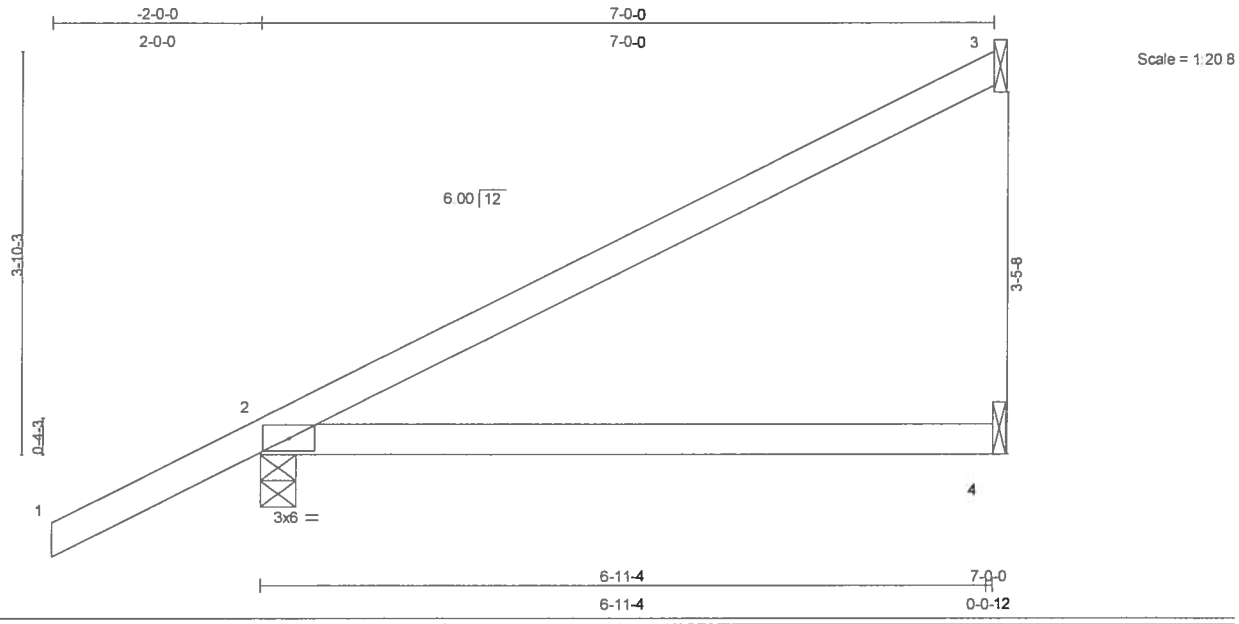


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.32	2-4	>253	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-94(load case 6), 2=-225(load case 6), 4=-64(load case 5)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-131/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Crandall Bay Blvd
Boynton Beach, FL 33426

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 225 lb uplift at joint 2 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard

December 5,2007

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Ocean Beach, FL 32435

December 5,2007

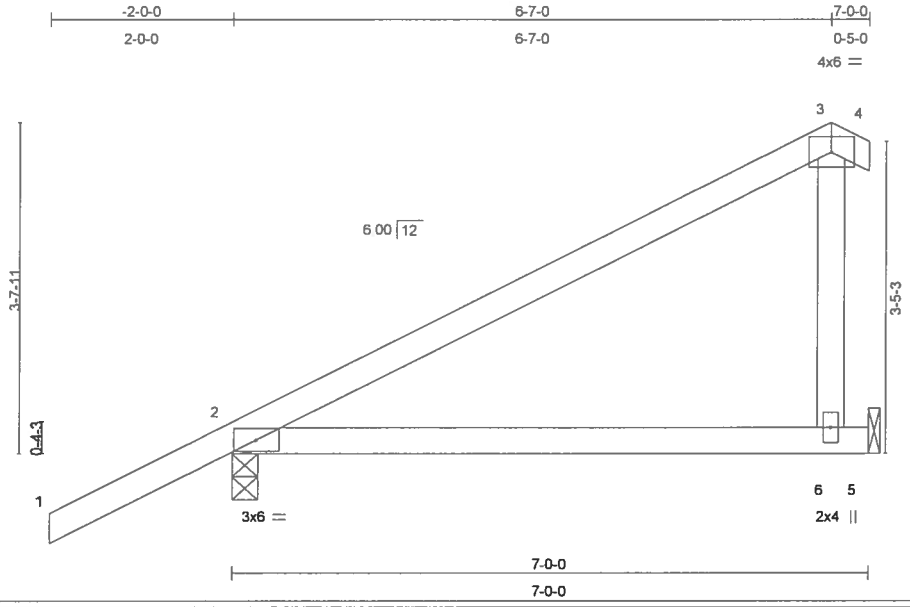
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915410
L262673	EJ7A	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:33 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.10	2-6	>812	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.17	2-6	>473	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=351/0-3-8, 5=202/Mechanical
Max Horz 2=147(load case 6)
Max Uplift 2=-146(load case 6), 5=-69(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-95/52, 3-4=0/10
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-171/217

JOINT STRESS INDEX
2 = 0.56, 3 = 0.09 and 6 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 and 69 lb uplift at joint 5.

Julius Lee
Truss Design Engineer
Florida PE No. 34164
1109 Crandall Bay Blvd
Jacksonville Beach, FL 32250

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915410
L262673	EJ7A	COMMON	1	1	Job Reference (optional)

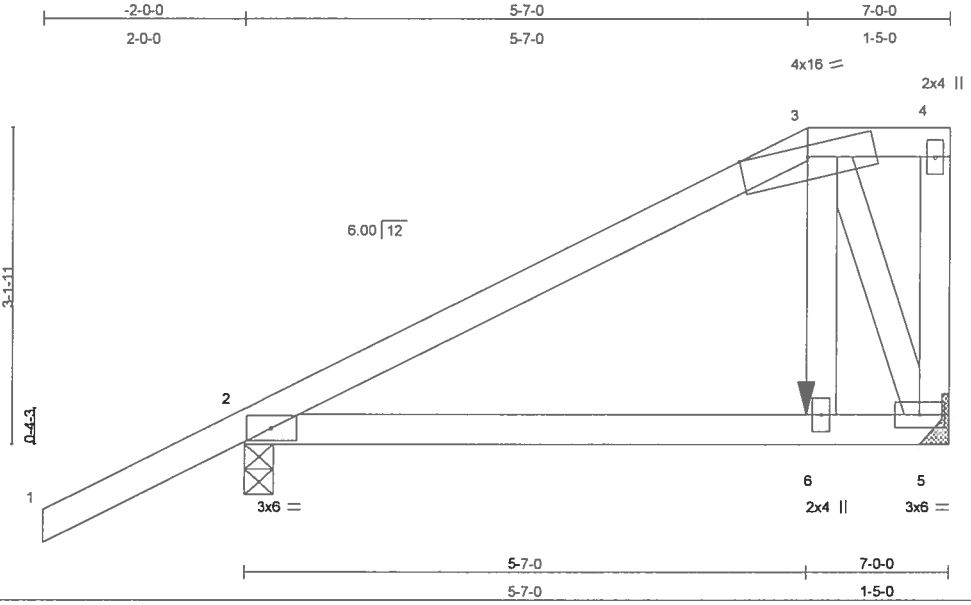
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34781
1109 Coastal Bay Blvd
Ocean Isle, NC 28469

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915411
L262673	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:33 2007 Page 1



Simpson HTU26

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.02	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.04	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.11	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=396/0-3-8, 5=458/Mechanical
Max Horz 2=140(load case 5)
Max Uplift 2=-166(load case 5), 5=-156(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-298/47, 3-4=-1/0
BOT CHORD 2-6=-66/204, 5-6=-69/219
WEBS 3-6=-65/301, 3-5=-536/168, 4-5=-36/33

JOINT STRESS INDEX
2 = 0.28, 3 = 0.69, 4 = 0.02, 5 = 0.25 and 6 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 2 and 156 lb uplift at joint 5.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34985
1 Lee Central Bay Blvd
Boynton Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915411
L262673	EJ7B	MONO HIP	1	1	Job Reference (optional)

NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-98(F=-44), 2-6=-10, 5-6=-18(F=-8)
   Concentrated Loads (lb)
   Vert: 6=-243(F)

Julius Lee
Truss Design Engineer
Florida PE No 34869
1061 Corbel Bay Run
Jacksonville, FL 32218

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915412
L262673	HJ3	MONO TRUSS	1	1	Job Reference (optional)

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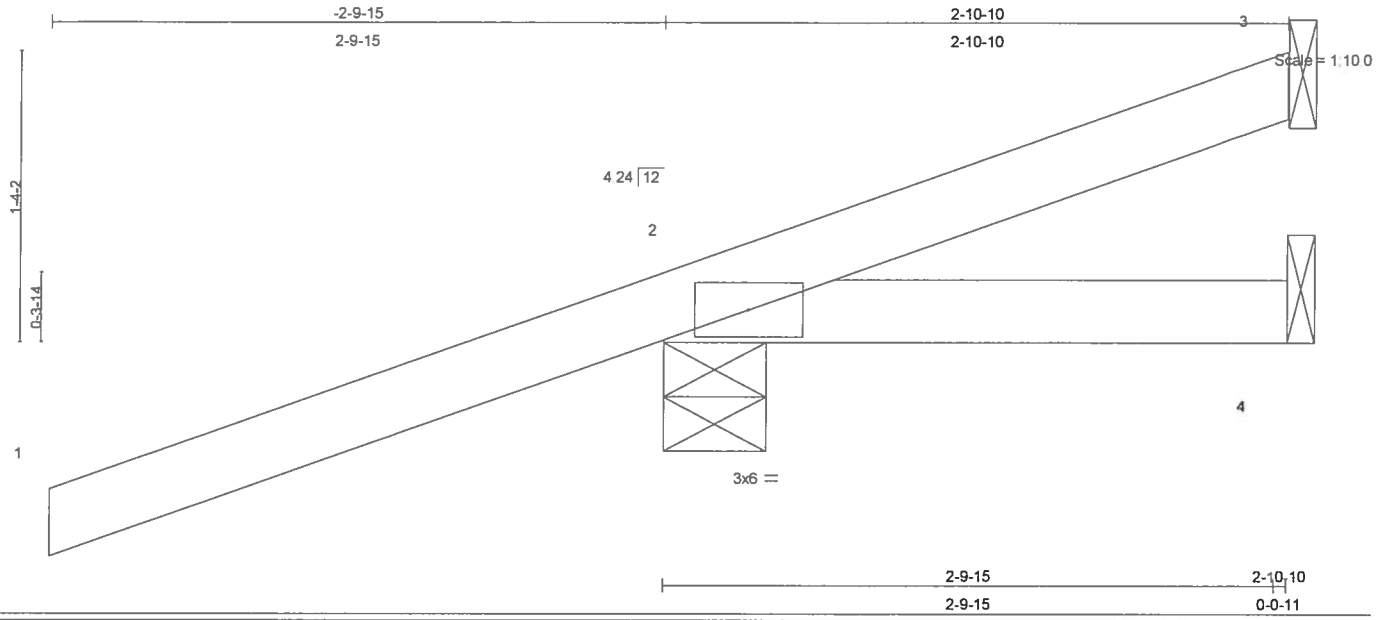

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-63/Mechanical, 2=286/0-5-11, 4=6/Mechanical
Max Horz 2=78(load case 3)
Max Uplift 3=-63(load case 1), 2=-289(load case 3)
Max Grav 3=89(load case 3), 2=286(load case 1), 4=32(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-43/32
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 289 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34985
1100 Crandall Bay Blvd
Jacksonville Beach, FL 32250

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915412
L262673	HJ3	MONO TRUSS	1	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-2=-54
   Trapezoidal Loads (plf)
      Vert: 2=-3(F=25, B=25)-to-3=-39(F=8, B=8), 2=0(F=5, B=5)-to-4=-7(F=1, B=1)

Julius Lee
Florida PE No. 34927
1808 Crestview Ave
Ocean Ocean, FL 32435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915413
L262673	HJ7	MONO TRUSS	1	1	Job Reference (optional)

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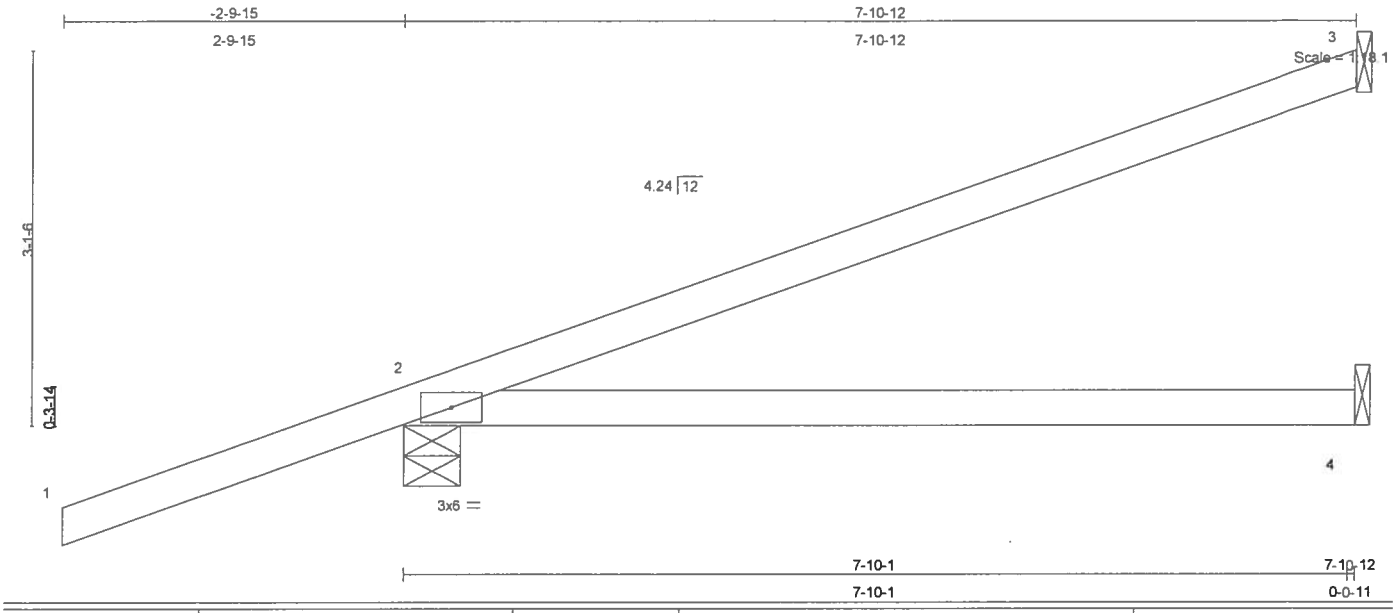

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.12	2-4	>733	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.24	2-4	>386	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=237/Mechanical, 2=367/0-5-11, 4=54/Mechanical
Max Horz 2=194(load case 3)
Max Uplift 3=-190(load case 3), 2=-257(load case 3)
Max Grav 3=237(load case 1), 2=367(load case 1), 4=115(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-88/55
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 3 and 257 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
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Florida PE No. 34869
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December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915413
L262673	HJ7	MONO TRUSS	1	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-107(F=-26, B=-26), 2=-0(F=5, B=5)-to-4=-20(F=-5, B=-5)

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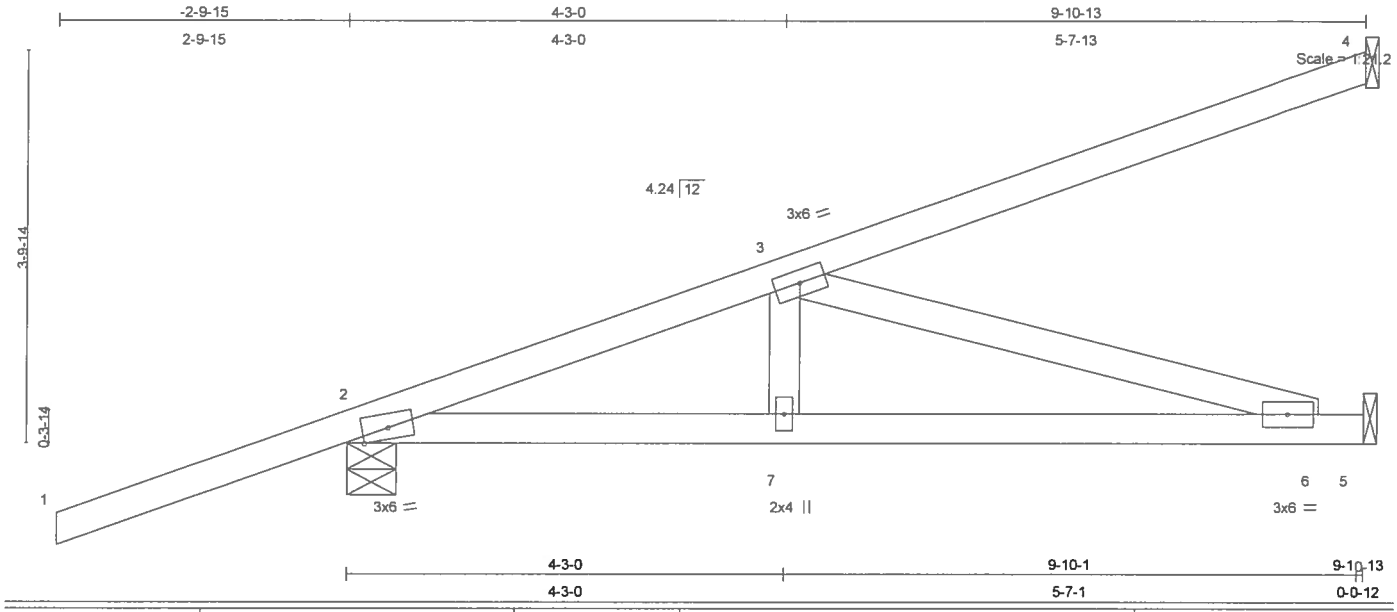
December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915414
L262673	HJ9	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:35 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.10	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>984	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift 4=-233(load case 3), 2=-401(load case 3), 5=-181(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-647/363, 3-4=-105/65
BOT CHORD 2-7=-535/599, 6-7=-535/599, 5-6=0/0
WEBS 3-7=-94/190, 3-6=-624/558

JOINT STRESS INDEX
2 = 0.77, 3 = 0.22, 6 = 0.17 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.

Julius Lee
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Florida PE No 34869
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Lehigh Acres, FL 33936

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915414
L262673	HJ9	MONO TRUSS	6	1	Job Reference (optional)

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

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Destin Beach, FL 32435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24
L262673	T01	SPECIAL	1	2	J1915415
					Job Reference (optional)

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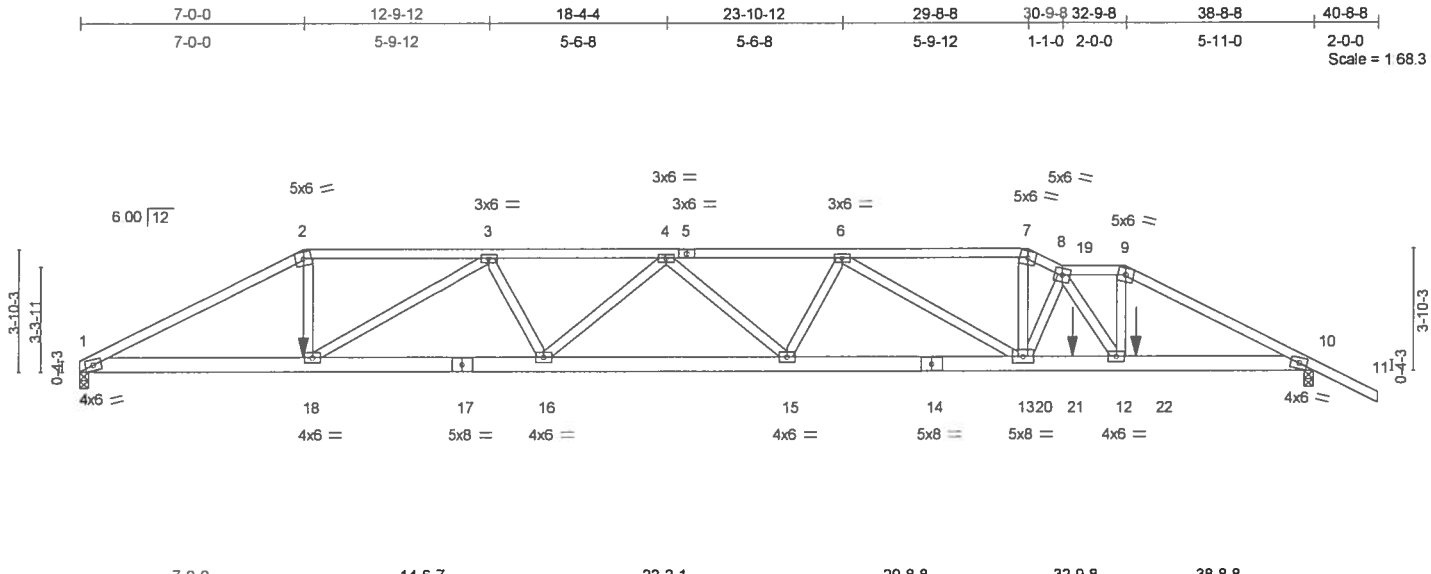

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.30	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.58	15-16	>795	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.12	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 440 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2571/0-3-8, 10=2814/0-3-8
Max Horz 1=-104(load case 6)
Max Uplift 1=-796(load case 4), 10=-816(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5370/1722, 2-3=-4804/1593, 3-4=-7233/2362, 4-5=-7537/2413, 5-6=-7537/2413, 6-7=-5750/1808, 7-19=-6085/1887, 8-19=-6117/1884, 8-9=-4983/1514, 9-10=-5538/1635, 10-11=0/51
BOT CHORD 1-18=-1495/4726, 17-18=-2243/6922, 16-17=-2243/6922, 15-16=-2493/7738, 14-15=-2320/7368, 13-14=-2320/7368, 13-20=-1738/5954, 20-21=-1738/5954, 12-21=-1738/5954, 12-22=-1398/4879, 10-22=-1398/4879
WEBS 2-18=-554/1860, 3-18=-2581/903, 3-16=-117/726, 4-16=-710/301, 4-15=-329/211, 6-15=-39/421, 6-13=-1974/731, 7-13=-468/1898, 8-13=-734/183, 8-12=-1821/633, 9-12=-639/2079

JOINT STRESS INDEX
1 = 0.81, 2 = 0.55, 3 = 0.45, 4 = 0.35, 5 = 0.50, 6 = 0.45, 7 = 0.64, 8 = 0.59, 9 = 0.52, 10 = 0.85, 12 = 0.48, 13 = 0.30, 14 = 0.77, 15 = 0.31, 16 = 0.31, 17 = 0.73 and 18 = 0.43

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Florida PE No 34927
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Ocoee, FL 34761

December 5,2007

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Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915415
L262673	T01	SPECIAL	1	2	Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 796 lb uplift at joint 1 and 816 lb uplift at joint 10.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-7=-118(F=-64), 7-19=-118(F=-64), 8-19=-54, 8-9=-54, 9-11=-54, 1-18=-10, 18-20=-22(F=-12), 10-20=-10
   Concentrated Loads (lb)
   Vert: 18=-411(F) 21=-202(F) 22=-458(F)

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1103 Emanuel Bay Blvd
Sevalon Beach, FL 32456

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915416
L262673	T02	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:37 2007 Page 1

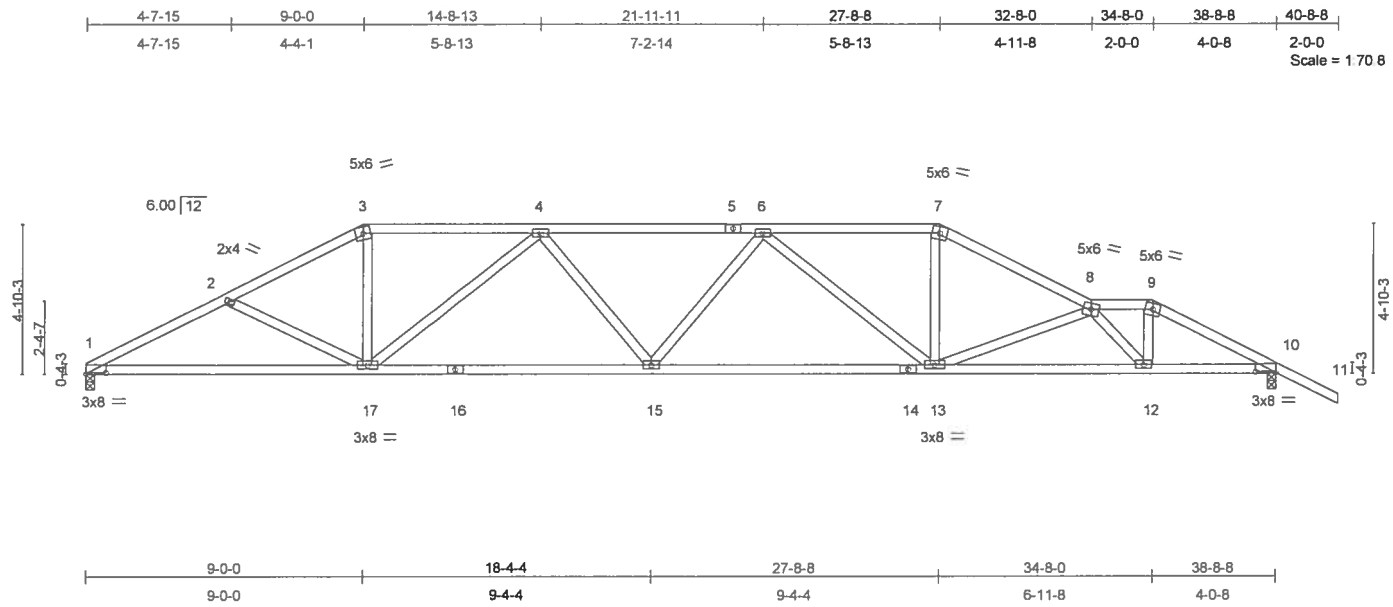

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [10:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.28	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.52	13-15	>888	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 194 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.

REACTIONS (lb/size) 1=1226/0-3-8, 10=1348/0-3-8
Max Horz 1=-102(load case 7)
Max Uplift 1=-237(load case 5), 10=-314(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2376/1296, 2-3=-2145/1165, 3-4=-1893/1103, 4-5=-2551/1420, 5-6=-2551/1420, 6-7=-2142/1224, 7-8=-2405/1290, 8-9=-2095/1096, 9-10=-2354/1166, 10-11=0/47
BOT CHORD 1-17=-1005/2071, 16-17=-1111/2468, 15-16=-1111/2468, 14-15=-1155/2561, 13-14=-1155/2561, 12-13=-1392/2907, 10-12=-880/2029
WEBS 2-17=-227/248, 3-17=-318/668, 4-17=-819/410, 4-15=0/229, 6-15=-68/126, 6-13=-652/311, 7-13=-350/754, 8-13=-851/527, 8-12=-1215/717, 9-12=-473/937

JOINT STRESS INDEX
1 = 0.70, 2 = 0.33, 3 = 0.42, 4 = 0.38, 5 = 0.49, 6 = 0.38, 7 = 0.51, 8 = 0.51, 9 = 0.41, 10 = 0.72, 12 = 0.59, 13 = 0.56, 14 = 0.86, 15 = 0.38, 16 = 0.83 and 17 = 0.56

Julius Lee
Truss Design Engineer
Florida PE No. 34824
1105 Crandall Parkway
Destin Beach, FL 32540

NOTES
1) Unbalanced roof live loads have been considered for this design.

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915416
L262673	T02	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 314 lb uplift at joint 10.

LOAD CASE(S) Standard

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Weston Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915417
L262673	T03	SPECIAL	1	1	Job Reference (optional)

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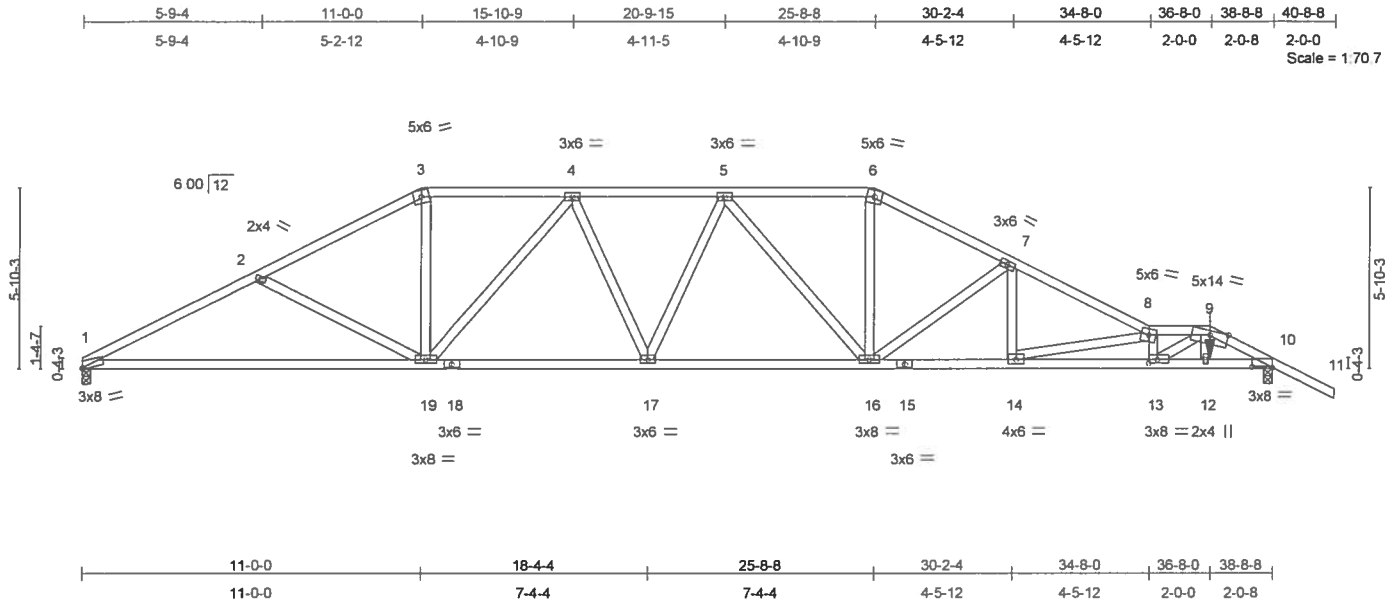

Plate Offsets (X,Y): [1:0-0-10,Edge], [10:0-8-0,0-0-6], [13:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.33	1-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.66	1-19	>700	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.15	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 209 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-0-12 oc bracing: 13-14.

REACTIONS (lb/size) 1=1227/0-3-8, 10=1360/0-3-8
Max Horz 1=-114(load case 6)
Max Uplift 1=-218(load case 5), 10=-333(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2340/438, 2-3=-2030/394, 3-4=-1770/377, 4-5=-2103/436, 5-6=-1914/410, 6-7=-2176/435, 7-8=-2772/500, 8-9=-3671/626, 9-10=-2290/378, 10-11=0/47
BOT CHORD 1-19=-388/2039, 18-19=-358/2064, 17-18=-358/2064, 16-17=-345/2116, 15-16=-376/2456, 14-15=-376/2456, 13-14=-614/3811, 12-13=-294/1961, 10-12=-293/1957
WEBS 2-19=-325/191, 3-19=-81/601, 4-19=-555/182, 4-17=-20/162, 5-17=-93/70, 5-16=-432/155, 6-16=-101/699, 7-16=-688/206, 7-14=-45/394, 8-14=-1393/278, 8-13=-1035/227, 9-13=-386/1973, 9-12=-3/58

Julius Lee
Truss Design Engineer
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Ocean Isle Beach, NC 28469

JOINT STRESS INDEX
1 = 0.87, 2 = 0.33, 3 = 0.43, 4 = 0.46, 5 = 0.46, 6 = 0.40, 7 = 0.39, 8 = 0.58, 9 = 0.60, 10 = 0.70, 12 = 0.33, 13 = 0.82, 14 = 0.38, 15 = 0.80, 16 = 0.56, 17 = 0.46, 18 = 0.81 and 19 = 0.56

December 5,2007

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Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915417
L262673	T03	SPECIAL	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 1 and 333 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-54, 6-8=-54, 8-9=-54, 9-11=-54, 1-10=-10
   Concentrated Loads (lb)
   Vert: 12=-12(F)

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December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915418
L262673	T04	SPECIAL	1	1	Job Reference (optional)

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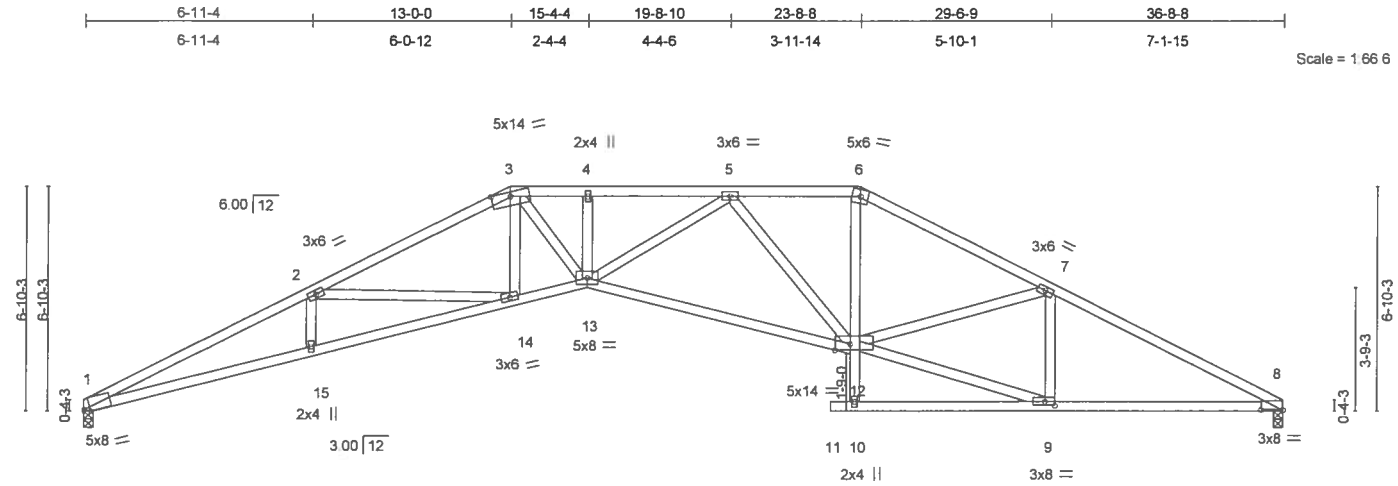

Plate Offsets (X,Y): [1:0-2-6,Edge], [8:0-8-4,0-0-6], [9:0-3-8,0-1-8], [12:0-5-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.50	13	>878	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.84	12-13	>523	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.50	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 6-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-13 oc bracing.

REACTIONS (lb/size) 1=1168/0-3-8, 8=1170/0-3-8
Max Horz 1=85(load case 5)
Max Uplift 1=-224(load case 6), 8=-222(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4123/2169, 2-3=-3345/1715, 3-4=-3724/1929, 4-5=-3723/1929, 5-6=-2072/1210, 6-7=-2357/1282, 7-8=-2224/1197
BOT CHORD 1-15=-1891/3705, 14-15=-1889/3704, 13-14=-1306/3025, 12-13=-1247/2840, 10-12=0/101, 6-12=-375/763, 10-11=0/0, 9-10=-21/28, 8-9=-964/1905
WEBS 2-15=0/208, 2-14=-683/566, 3-14=-160/250, 3-13=-504/1222, 4-13=-156/81, 5-13=-462/1154, 5-12=-1087/527, 9-12=-982/1956, 7-9=-461/297, 7-12=-77/290

JOINT STRESS INDEX
1 = 0.77, 2 = 0.39, 3 = 0.72, 4 = 0.33, 5 = 0.65, 6 = 0.43, 7 = 0.39, 8 = 0.72, 9 = 0.75, 10 = 0.69, 12 = 0.56, 13 = 0.92, 14 = 0.37 and 15 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.

Julius Lee
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December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915418
L262673	T04	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 222 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34863
1809 Crandall Avenue
Ocoee, FL 34761

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915419
L262673	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:40 2007 Page 1

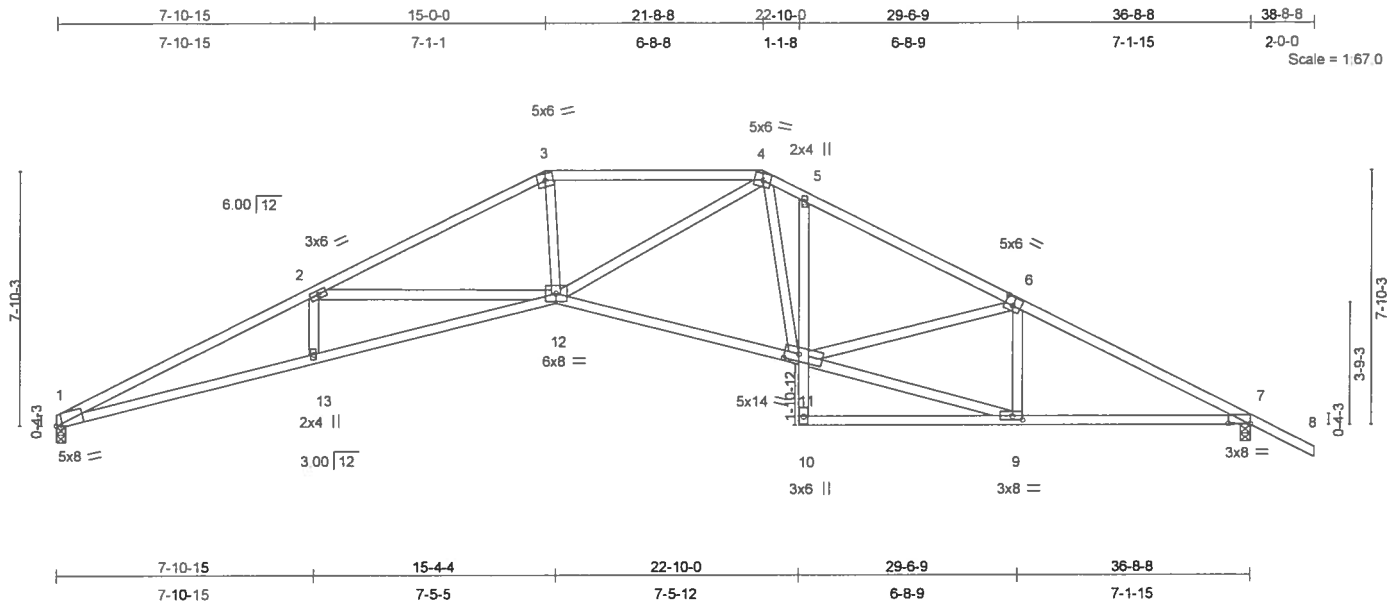


Plate Offsets (X,Y): [1:0-2-7,Edge], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-5-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	0.42	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.74	12-13	>592	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-6-3 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-137(load case 7)
Max Uplift 1=-237(load case 6), 7=-331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4060/2070, 2-3=-3083/1485, 3-4=-2769/1436, 4-5=-2218/1321, 5-6=-2310/1239, 6-7=-2181/1160, 7-8=0/47
BOT CHORD 1-13=-1711/3647, 12-13=-1707/3643, 11-12=-697/1998, 10-11=0/92, 5-11=-156/199, 9-10=-14/35, 7-9=-847/1864
WEBS 2-13=0/237, 2-12=-869/712, 3-12=-375/967, 4-12=-352/976, 4-11=-276/290, 9-11=-874/1915, 6-11=-57/241, 6-9=-457/291

JOINT STRESS INDEX
1 = 0.75, 2 = 0.39, 3 = 0.73, 4 = 0.51, 5 = 0.60, 6 = 0.73, 7 = 0.67, 9 = 0.73, 10 = 0.29, 11 = 0.44, 12 = 0.84 and 13 = 0.33

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1104 Emerald Bay Drive
Destin Beach, FL 32548

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915419
L262673	T05	SPECIAL	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 331 lb uplift at joint 7.

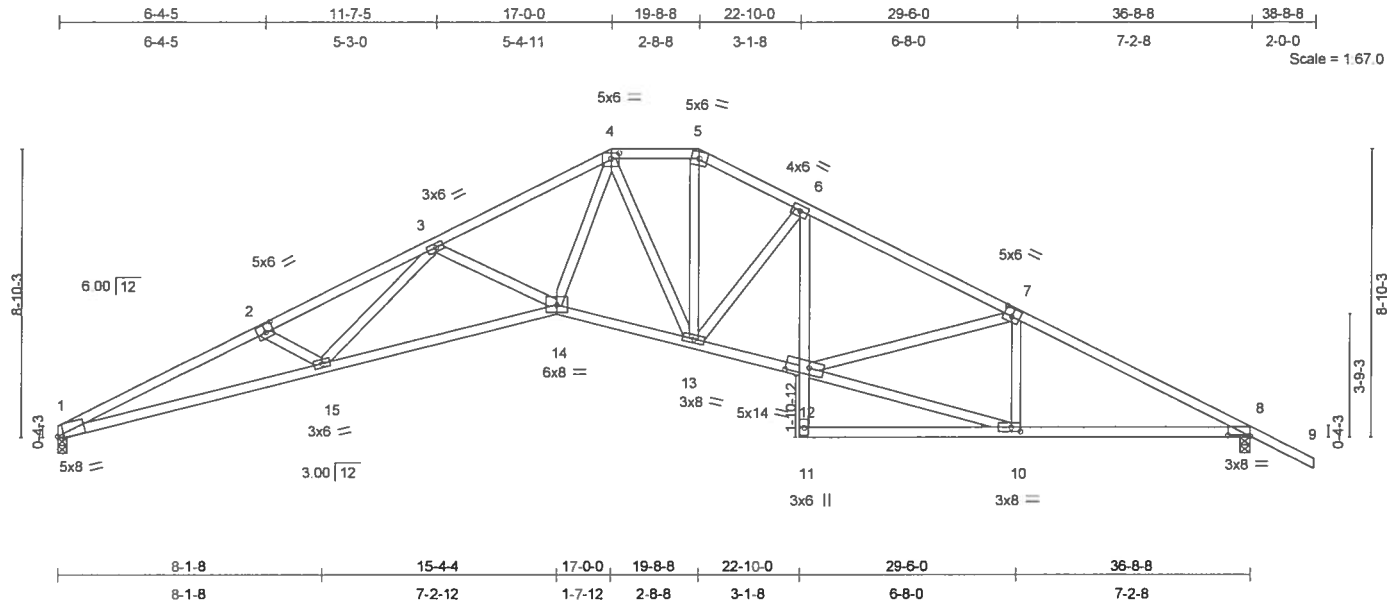
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
*Florida PE No. 34927
1106 Coastal Bay Blvd
Ocean Isle, NC 28469

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915420
L262673	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:41 2007 Page 1



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LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.44	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.75	14-15	>586	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.44	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 6-11 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-6 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 8=1285/0-3-8
Max Horz 1=-149(load case 7)
Max Uplift 1=-248(load case 6), 8=-342(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4094/2158, 2-3=-3828/2043, 3-4=-2958/1504, 4-5=-1810/1083, 5-6=-2052/1171, 6-7=-2284/1256, 7-8=-2178/1174, 8-9=0/47
BOT CHORD 1-15=-1805/3684, 14-15=-1343/3152, 13-14=-677/2106, 12-13=-780/2033, 11-12=0/93, 6-12=-49/124, 10-11=-17/78, 8-10=-859/1861
WEBS 2-15=-254/286, 3-15=-317/498, 3-14=-542/467, 4-14=-712/1672, 4-13=-610/216, 5-13=-455/781, 6-13=-320/305, 10-12=-882/1877, 7-12=-36/192, 7-10=-442/291

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1106 Emerald Bay Run
Dayton Beach, FL 32124

JOINT STRESS INDEX
1 = 0.76, 2 = 0.59, 3 = 0.39, 4 = 0.82, 5 = 0.36, 6 = 0.34, 7 = 0.72, 8 = 0.67, 10 = 0.71, 11 = 0.32, 12 = 0.81, 13 = 0.62, 14 = 0.72 and 15 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915420
L262673	T06	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 342 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Ocean Ridge, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915421
L262673	T07	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:42 2007 Page 1

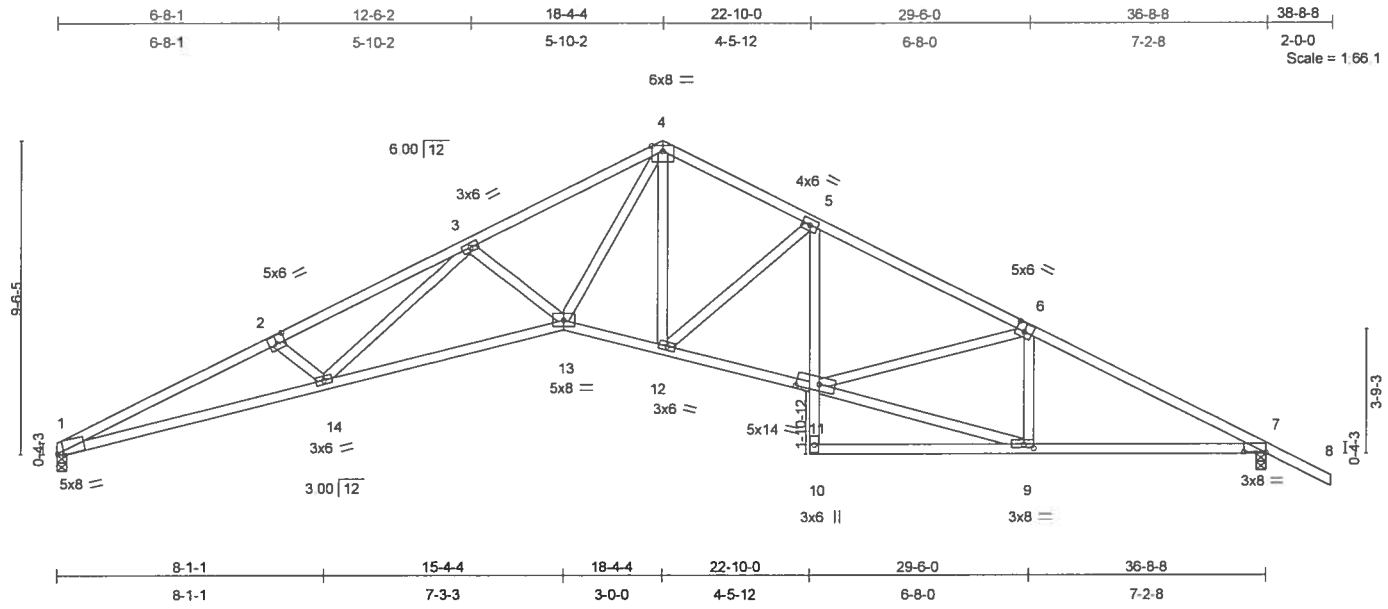


Plate Offsets (X,Y): [1:0-2-7,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8], [11:0-8-3,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.45	13-14	>975	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.76	13-14	>574	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.44	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 5-10 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-0 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-157(load case 7)
Max Uplift 1=-254(load case 6), 7=-348(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4095/2172, 2-3=-3849/2094, 3-4=-2957/1551, 4-5=-1986/1131, 5-6=-2286/1271, 6-7=-2178/1185, 7-8=0/47
BOT CHORD 1-14=-1817/3685, 13-14=-1285/3055, 12-13=-532/1783, 11-12=-794/2038, 10-11=0/92, 5-11=-46/154, 9-10=-18/64, 7-9=-868/1861
WEBS 2-14=-267/295, 3-14=-417/620, 3-13=-521/453, 4-13=-799/1765, 4-12=-268/382, 5-12=-395/324, 9-11=-891/1884, 6-11=-34/170, 6-9=-443/293

JOINT STRESS INDEX
1 = 0.76, 2 = 0.67, 3 = 0.39, 4 = 0.70, 5 = 0.29, 6 = 0.72, 7 = 0.67, 9 = 0.72, 10 = 0.31, 11 = 0.77, 12 = 0.37, 13 = 0.93 and 14 = 0.38

Julius Lee
Triad Design Engineer
Florida PE No. 34869
1104 Coastal Bay Blvd
Boynton Beach, FL 33435

NOTES
1) Unbalanced roof live loads have been considered for this design.

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915421
L262673	T07	SPECIAL	3	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 348 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34931
1109 Coastal Bay Blvd
Ocean Isle, NC 28469

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915422
L262673	T08	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:43 2007 Page 1

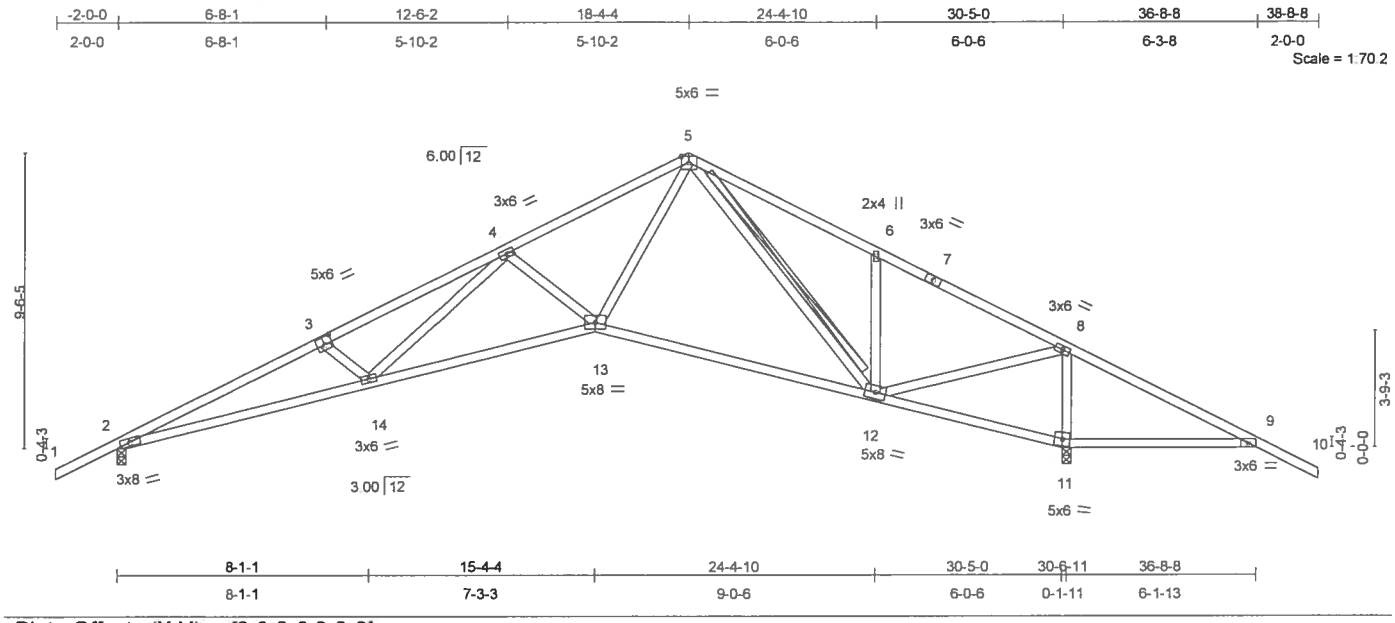


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.23	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.43	13-14	>835	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.26	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1021/0-3-8, 11=1543/0-3-8
Max Horz 2=-144(load case 7)
Max Uplift 2=-311(load case 6), 11=-588(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2959/1272, 3-4=-2708/1194, 4-5=-1805/680, 5-6=-839/334, 6-7=-709/199, 7-8=-843/188, 8-9=-842/708, 9-10=0/47
BOT CHORD 2-14=-983/2639, 13-14=-479/1999, 12-13=-101/988, 11-12=-647/974, 9-11=-561/891
WEBS 3-14=-266/293, 4-14=-387/600, 4-13=-520/442, 5-13=-393/1281, 5-12=-425/199, 6-12=-338/329, 8-12=-777/1348, 8-11=-1315/919

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1102 Coastal Bay Blvd
Ocean Isle, NC 28469

JOINT STRESS INDEX
2 = 0.68, 3 = 0.54, 4 = 0.39, 5 = 0.46, 6 = 0.33, 7 = 0.17, 8 = 0.68, 9 = 0.45, 11 = 0.64, 12 = 0.56, 13 = 0.90 and 14 = 0.37

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915422
L262673	T08	SPECIAL	4	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 588 lb uplift at joint 11.

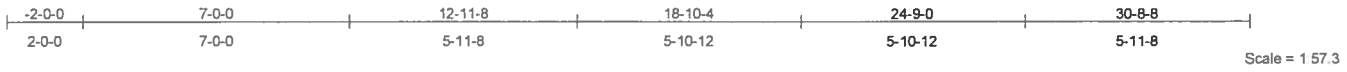
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34160
1106 Grandi Bay Blvd
Lovnion Beach, FL 33436

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915423
L262673	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:44 2007 Page 1



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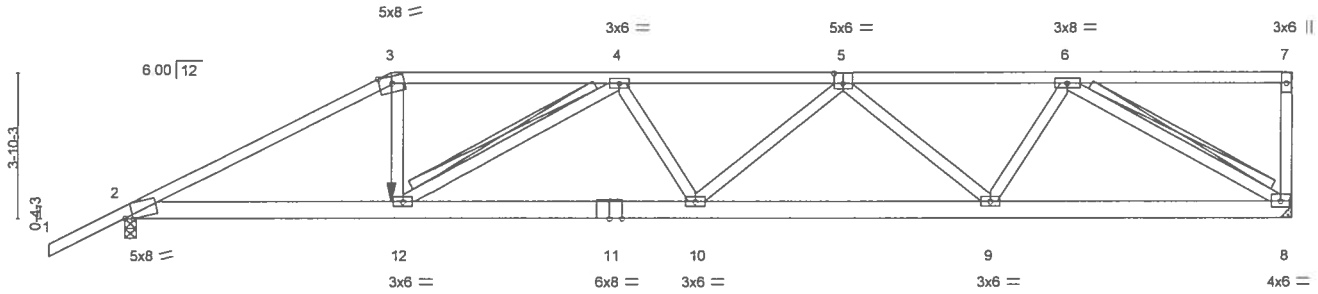


Simpson HTU26

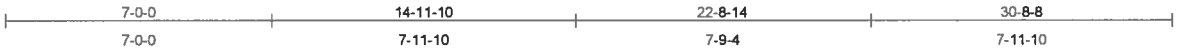


Plate Offsets (X,Y): [2:0-2-7,Edge], [5:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.26	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.49	10-12	>746	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2151/Mechanical, 2=2096/0-3-8
Max Horz 2=165(load case 5)
Max Uplift 8=-741(load case 4), 2=-654(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4021/1312, 3-4=-3574/1219, 4-5=-4758/1606, 5-6=-3585/1195, 6-7=-115/42, 7-8=-325/165
BOT CHORD 2-12=-1205/3521, 11-12=-1668/4758, 10-11=-1668/4758, 9-10=-1598/4555, 8-9=-1059/2998
WEBS 3-12=-360/1236, 4-12=-1380/571, 4-10=-1/178, 5-10=-19/272, 5-9=-1301/541, 6-9=-271/1171, 6-8=-3358/1185

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1061 Channel Bay Blvd
Davenport, Florida 33896

JOINT STRESS INDEX
2 = 0.76, 3 = 0.72, 4 = 0.42, 5 = 0.76, 6 = 0.88, 7 = 0.69, 8 = 0.85, 9 = 0.88, 10 = 0.42, 11 = 0.89 and 12 = 0.78

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915423
L262673	T09	HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 741 lb uplift at joint 8 and 654 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

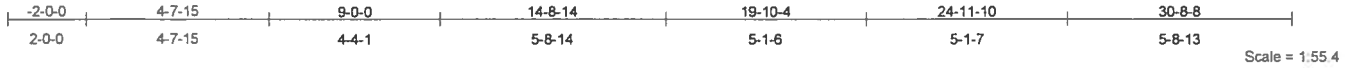
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 12=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34160
1105 Crandall Bay Drive
Boynton Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915424
L262673	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:45 2007 Page 1



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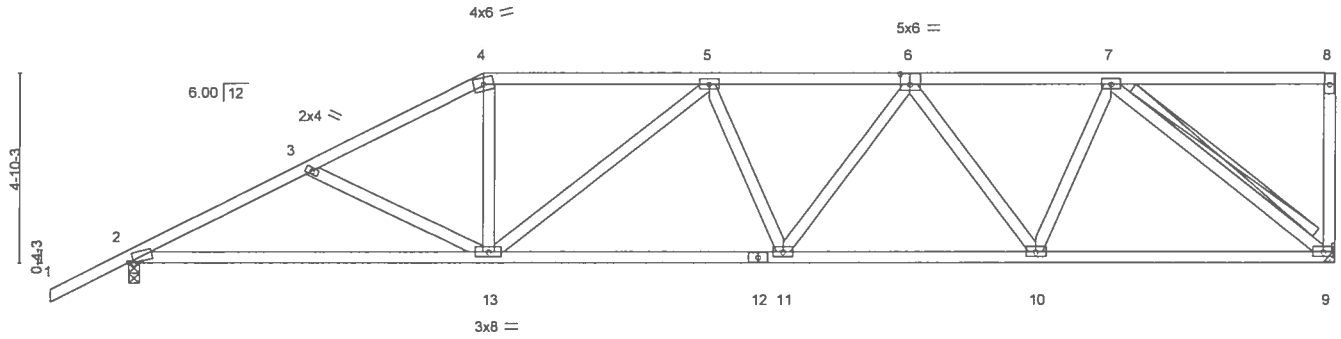

Simpson HTU26

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.27	2-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-15 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=969/Mechanical, 2=1093/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 9=-265(load case 5), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1801/904, 3-4=-1570/802, 4-5=-1374/777, 5-6=-1598/863, 6-7=-1198/630, 7-8=-32/12, 8-9=-142/100
BOT CHORD 2-13=-952/1544, 12-13=-906/1633, 11-12=-906/1633, 10-11=-813/1490, 9-10=-543/997
WEBS 3-13=-204/199, 4-13=-131/412, 5-13=-335/166, 5-11=-93/112, 6-11=-86/192, 6-10=-507/317, 7-10=-227/521, 7-9=-1245/685

JOINT STRESS INDEX
2 = 0.86, 3 = 0.33, 4 = 0.60, 5 = 0.46, 6 = 0.29, 7 = 0.46, 8 = 0.26, 9 = 0.46, 10 = 0.46, 11 = 0.46, 12 = 0.54 and 13 = 0.56

December 5,2007

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Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915424
L262673	T10	HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 9 and 262 lb uplift at joint 2.

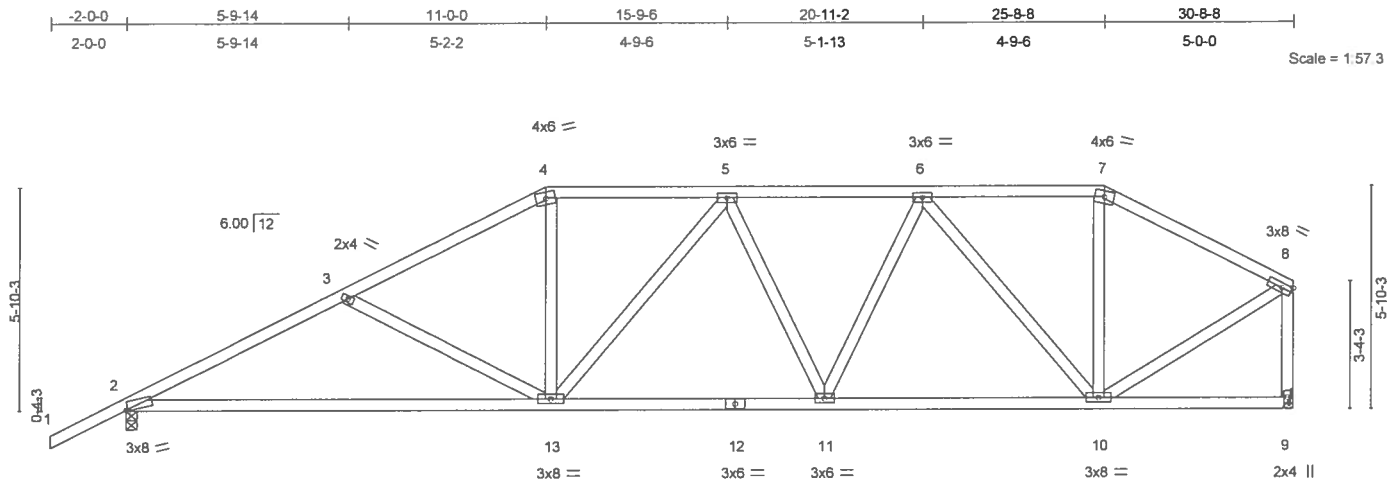
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
*Florida FIA No. 34801
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915425
L262673	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:46 2007 Page 1



Simpson HTU26

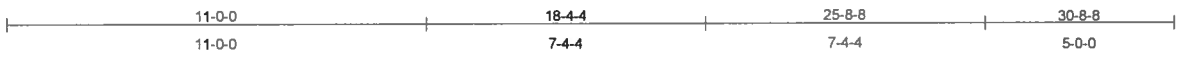


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.56	2-13	>650	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-7 oc bracing.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=177(load case 6)
Max Uplift 2=-280(load case 6), 9=-182(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1772/936, 3-4=-1455/788, 4-5=-1250/768, 5-6=-1264/764, 6-7=-752/497, 7-8=-893/493, 8-9=-944/530
BOT CHORD 2-13=-897/1516, 12-13=-695/1337, 11-12=-695/1337, 10-11=-600/1160, 9-10=-29/33
WEBS 3-13=-311/290, 4-13=-132/385, 5-13=-252/125, 5-11=-189/132, 6-11=-93/261, 6-10=-674/354, 7-10=-19/193, 8-10=-405/843

JOINT STRESS INDEX
2 = 0.89, 3 = 0.33, 4 = 0.59, 5 = 0.45, 6 = 0.45, 7 = 0.51, 8 = 0.94, 9 = 0.42, 10 = 0.78, 11 = 0.45, 12 = 0.44 and 13 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34801
1109 Coastal Bay Blvd
Ocean Isle, NC 28469

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915425
L262673	T11	HIP	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 182 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34701
1109 Comml Hwy 90
Lavon Lake, FL 32055

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915426
L262673	T12	HIP	1	1	Job Reference (optional)

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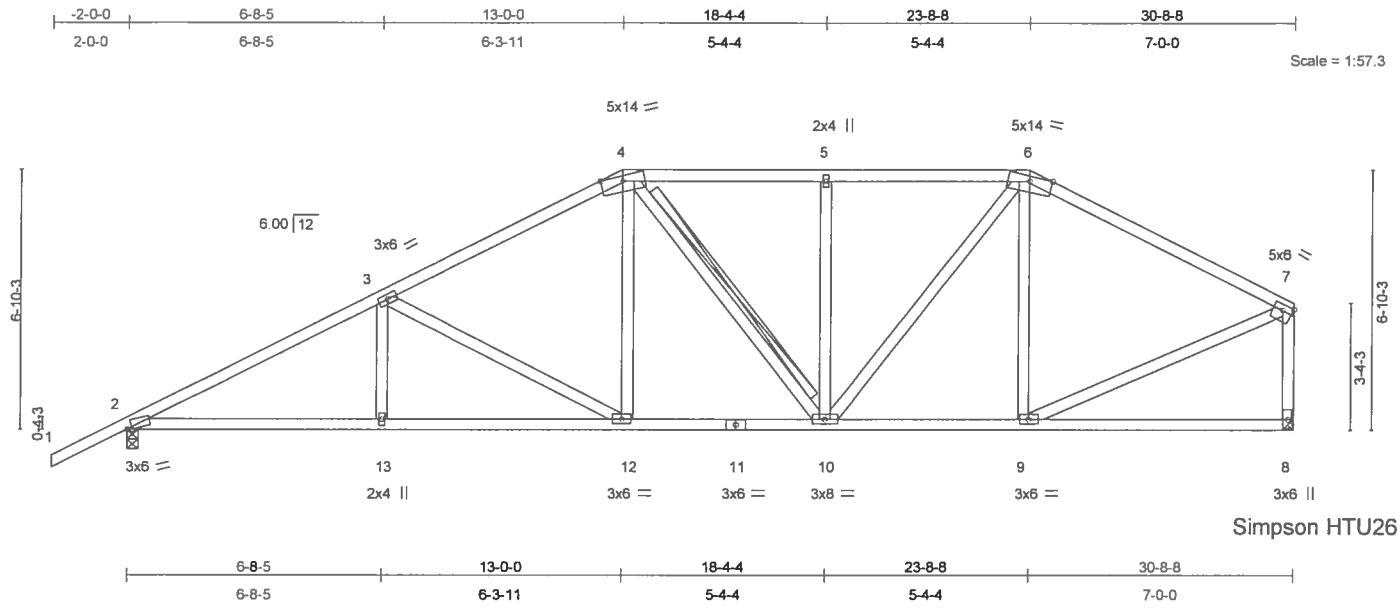

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	0.10	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.15	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=189(load case 6)
Max Uplift 2=-292(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1782/908, 3-4=-1326/762, 4-5=-1095/721, 5-6=-1095/721, 6-7=-1004/566, 7-8=-928/547
BOT CHORD 2-13=-866/1513, 12-13=-866/1513, 11-12=-576/1122, 10-11=-576/1122, 9-10=-404/819, 8-9=-57/74
WEBS 3-13=0/210, 3-12=-452/330, 4-12=-120/328, 4-10=-75/116, 5-10=-287/160, 6-10=-223/505, 6-9=-277/204, 7-9=-385/822

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1104 Emerald Bay Blvd
Destin, Destin, FL 32541

JOINT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.78, 5 = 0.33, 6 = 0.94, 7 = 0.69, 8 = 0.30, 9 = 0.46, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33

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Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915426
L262673	T12	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34-1906
1109 Grandi Ray Blvd
Boynton Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915427
L262673	T13	HIP	1	1	Job Reference (optional)

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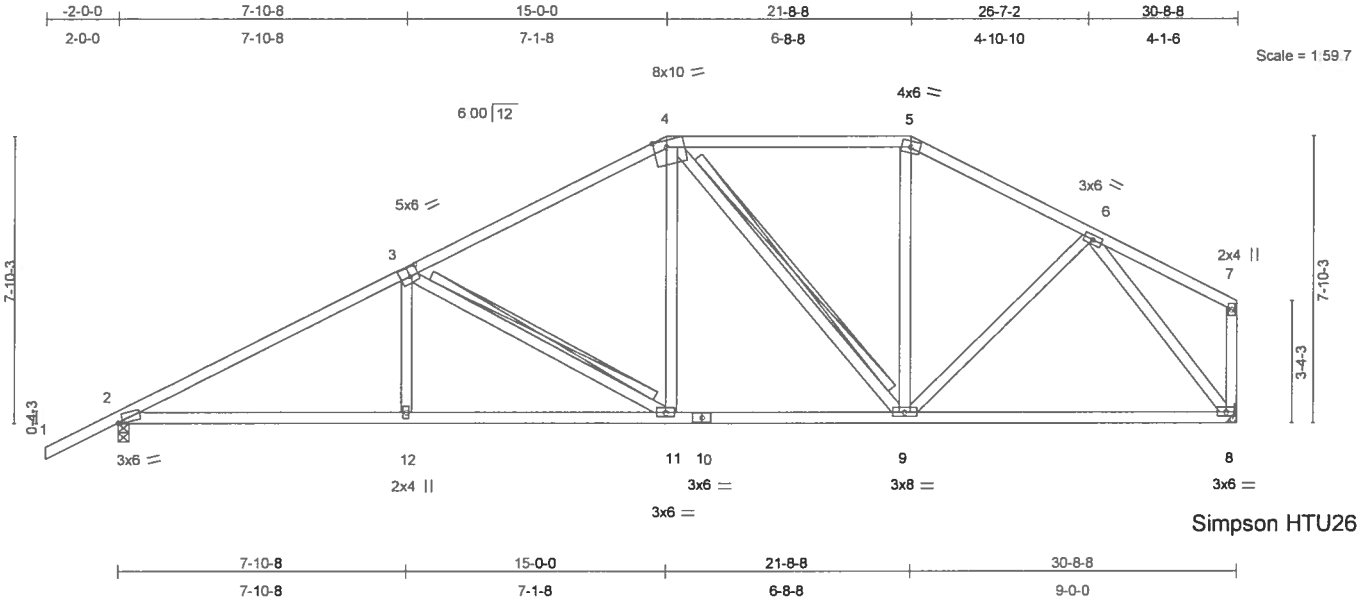

Simpson HTU26

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0], [4:0-4-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.13	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.23	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-12 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-11, 4-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=201(load case 6)
Max Uplift 2=-302(load case 6), 8=-172(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1742/902, 3-4=-1205/723, 4-5=-838/619, 5-6=-988/627, 6-7=-97/57, 7-8=-122/91
BOT CHORD 2-12=-846/1468, 11-12=-846/1468, 10-11=-498/1004, 9-10=-498/1004, 8-9=-367/654
WEBS 3-12=0/250, 3-11=-537/401, 4-11=-157/369, 4-9=-329/160, 5-9=-45/204, 6-9=-55/309, 6-8=-1009/600

Julius Lee
Truss Design Engineer
Florida FE No. 34801
1106 Shadel Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.76, 3 = 0.82, 4 = 0.60, 5 = 0.70, 6 = 0.38, 7 = 0.59, 8 = 0.77, 9 = 0.56, 10 = 0.33, 11 = 0.34 and 12 = 0.33

December 5,2007

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Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915427
L262673	T13	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 172 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 31935
1103 Crandal Bay Blvd
Boynton Beach, FL 33435

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915428
L262673	T14	HIP	1	1	Job Reference (optional)

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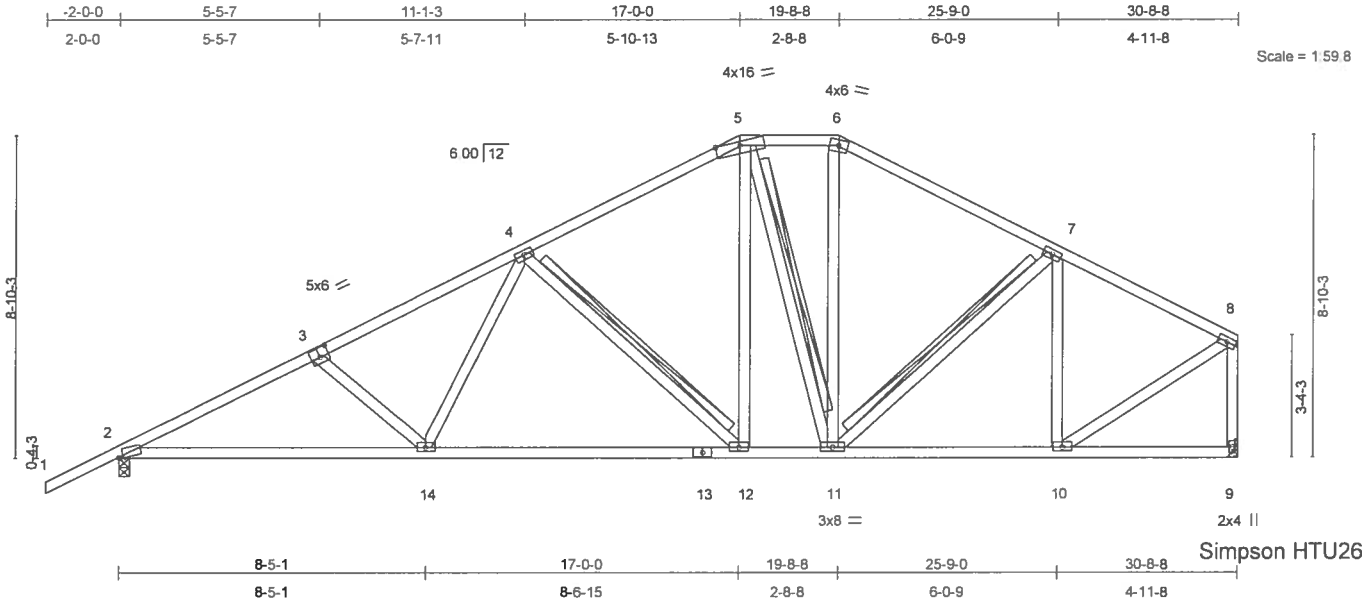

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.25	12-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.31	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 5-11, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=213(load case 6)
Max Uplift 2=-310(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1798/964, 3-4=-1580/901, 4-5=-1038/688, 5-6=-810/651, 6-7=-979/655, 7-8=-879/516, 8-9=-943/557
BOT CHORD 2-14=-928/1538, 13-14=-697/1231, 12-13=-697/1231, 11-12=-403/867, 10-11=-401/743, 9-10=-21/29
WEBS 3-14=-253/246, 4-14=-116/362, 4-12=-505/406, 5-12=-238/404, 5-11=-316/100, 6-11=-107/227, 7-11=-39/169, 7-10=-409/291, 8-10=-453/850

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1146 Coastal Bay Blvd
Ocean Isle Beach, NC 28469

JOINT STRESS INDEX
2 = 0.77, 3 = 0.39, 4 = 0.40, 5 = 0.72, 6 = 0.60, 7 = 0.39, 8 = 0.69, 9 = 0.41, 10 = 0.48, 11 = 0.66, 12 = 0.35, 13 = 0.39 and 14 = 0.45

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915428
L262673	T14	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 185 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34927
1100 Coastal Bay Blvd
Ocean Isle, NC 28459

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915429
L262673	T15	HIP	1	1	Job Reference (optional)

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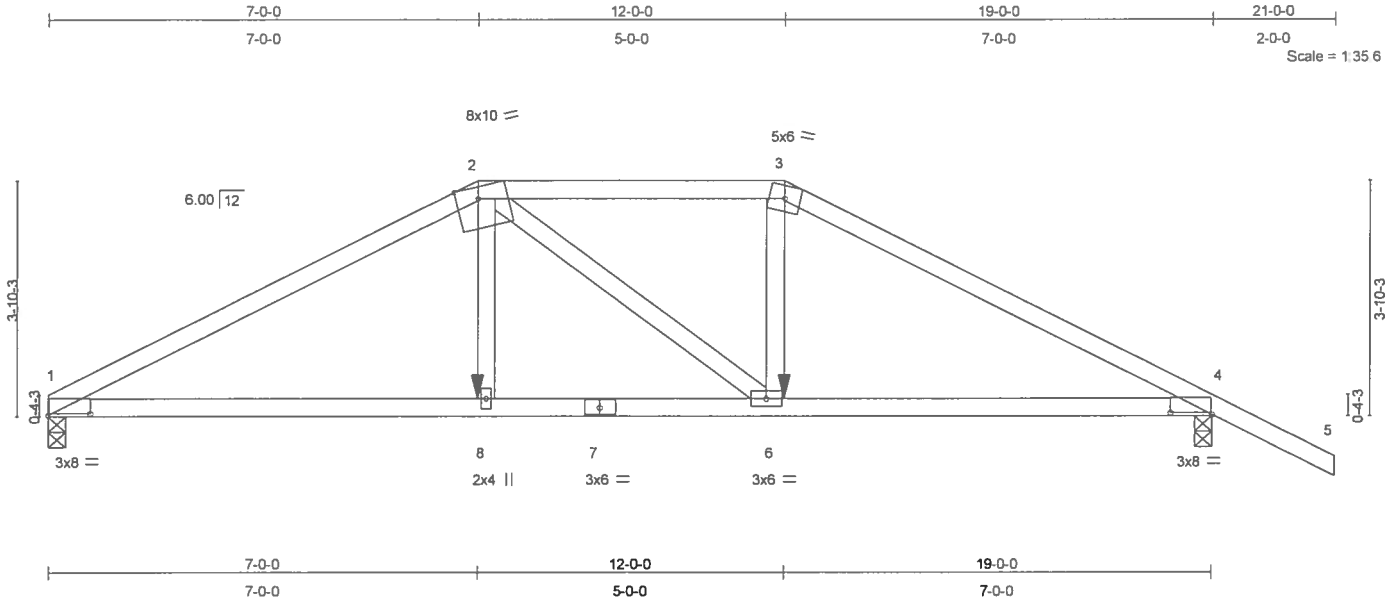

Plate Offsets (X,Y): [1:0-8-4,0-0-6], [2:0-4-3,Edge], [4:0-8-4,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.08	1-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.17	1-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-7 oc bracing.

REACTIONS (lb/size) 1=1189/0-3-8, 4=1318/0-3-8
Max Horz 1=-90(load case 6)
Max Uplift 1=-358(load case 5), 4=-456(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2278/694, 2-3=-1957/639, 3-4=-2260/676, 4-5=0/47
BOT CHORD 1-8=-580/1957, 7-8=-585/1978, 6-7=-585/1978, 4-6=-535/1936
WEBS 2-8=-126/524, 2-6=-149/118, 3-6=-145/576

JOINT STRESS INDEX
1 = 0.73, 2 = 0.73, 3 = 0.76, 4 = 0.75, 6 = 0.37, 7 = 0.65 and 8 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915429
L262673	T15	HIP	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 358 lb uplift at joint 1 and 456 lb uplift at joint 4.
7) Girder carries hip end with 7-0-0 end setback.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54, 2-3=-118(F=-64), 3-5=-54, 1-8=-10, 6-8=-22(F=-12), 4-6=-10
   Concentrated Loads (lb)
   Vert: 8=-411(F) 6=-411(F)

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December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915430
L262673	T16	HIP	1	1	Job Reference (optional)

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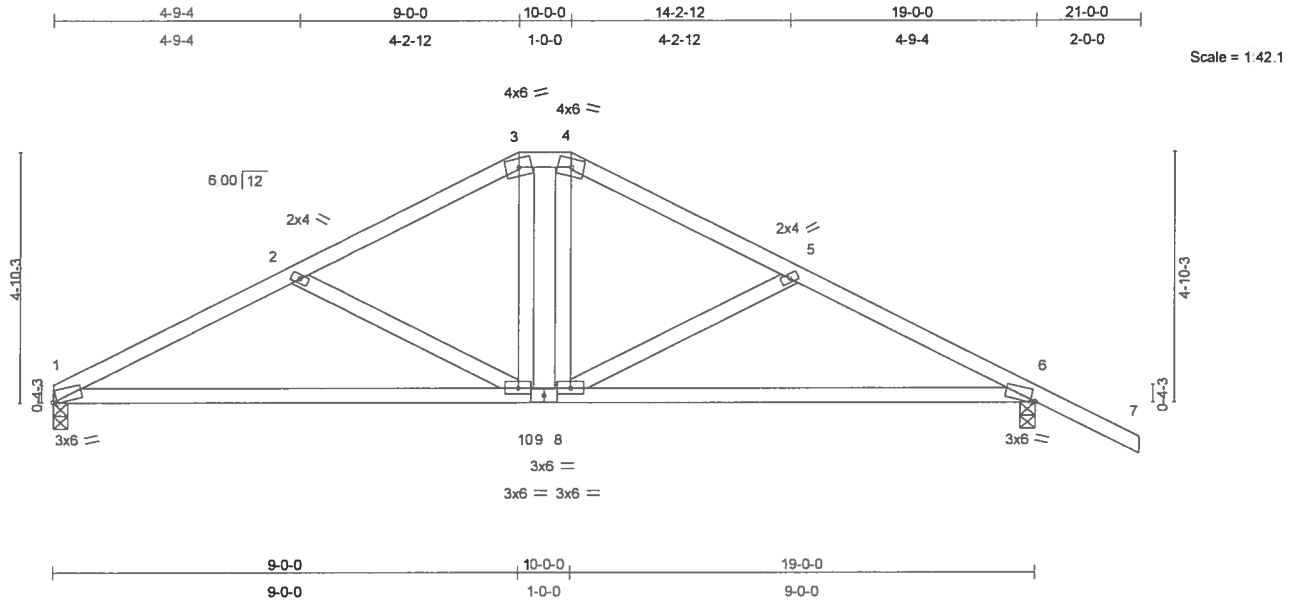


Plate Offsets (X,Y): [1:0-1-0,0-0-7], [6:0-1-0,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.12	1-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.23	1-10	>972	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=592/0-3-8, 6=721/0-3-8
Max Horz 1=-102(load case 7)
Max Uplift 1=-127(load case 6), 6=-225(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1021/590, 2-3=-756/444, 3-4=-630/445, 4-5=-753/439, 5-6=-1008/550, 6-7=0/47
BOT CHORD 1-10=-380/875, 9-10=-136/630, 8-9=-136/630, 6-8=-329/844
WEBS 2-10=-302/280, 3-10=-126/211, 4-8=-39/197, 5-8=-268/219

JOINT STRESS INDEX
1 = 0.85, 2 = 0.33, 3 = 0.45, 4 = 0.45, 5 = 0.33, 6 = 0.85, 8 = 0.34, 9 = 0.74 and 10 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34160
1109 Crandall Bay Run
Boynton Beach, FL 33436

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915430
L262673	T16	HIP	1	1	Job Reference (optional)

NOTES

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1 and 225 lb uplift at joint 6.

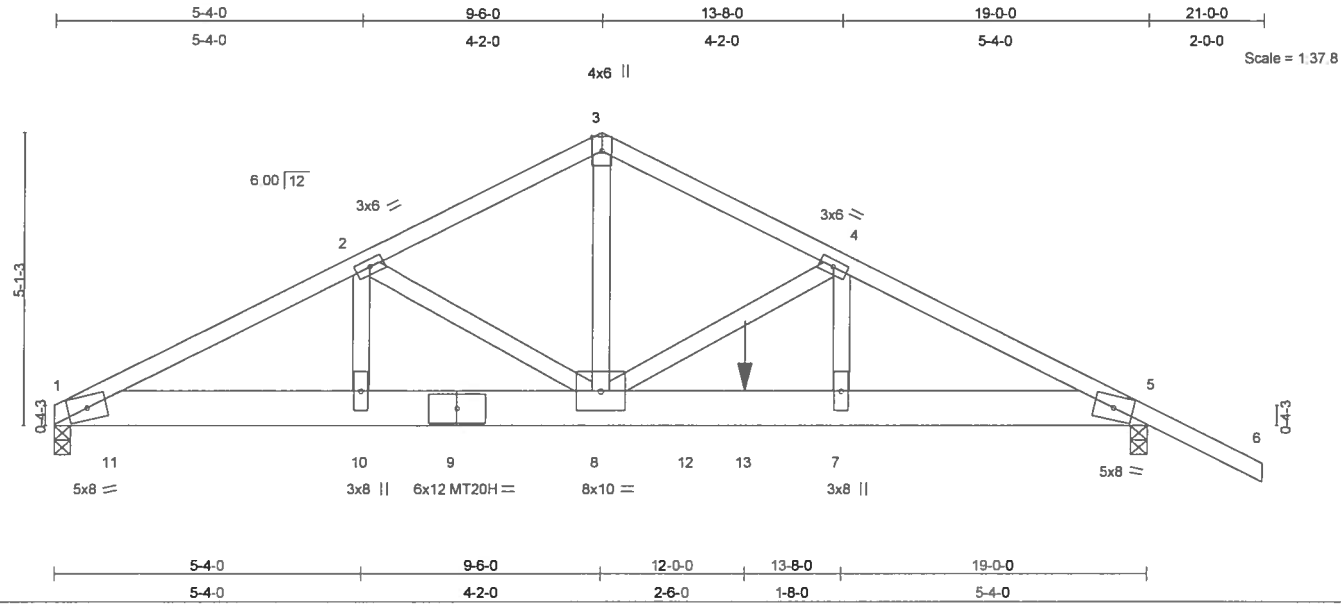
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34927
109 Coastal Bay Blvd
Ocean Beach, FL 32456

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915431
L262673	T17	HOWE	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Dec 05 12:31:51 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.12	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.22	7-8	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4504/0-3-8, 5=3507/0-3-8
Max Horz 1=-111(load case 5)
Max Uplift 1=-1212(load case 6), 5=-999(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8091/2169, 2-3=-5871/1589, 3-4=-5859/1598, 4-5=-7292/1908, 5-6=0/53
BOT CHORD 1-11=-1937/7218, 10-11=-1937/7218, 9-10=-1937/7218, 8-9=-1937/7218, 8-12=-1661/6477, 12-13=-1661/6477, 7-13=-1661/6477, 5-7=-1661/6477
WEBS 2-10=-538/1938, 2-8=-2367/701, 3-8=-1337/4946, 4-8=-1498/433, 4-7=-283/1170

JOINT STRESS INDEX
1 = 0.82, 2 = 0.70, 3 = 0.55, 4 = 0.70, 5 = 0.82, 7 = 0.31, 8 = 0.45, 9 = 0.72 and 10 = 0.31

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE No. 34701
1109 Comanche Bay Blvd
Davie Beach, FL 32435

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915431
L262673	T17	HOWE	1	2	Job Reference (optional)

NOTES

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1212 lb uplift at joint 1 and 999 lb uplift at joint 5.
9) Girder carries tie-in span(s): 30-8-8 from 8-0-0 to 18-0-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-6=-54, 1-11=-10, 11-12=-465(B=-455), 5-12=-10
   Concentrated Loads (lb)
   Vert: 13=-2151(B)

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Florida PE No. 34760
1102 Samuel Ray Blvd
Dayton Beach, FL 32114

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915432
L262673	T18	HIP	1	1	Job Reference (optional)

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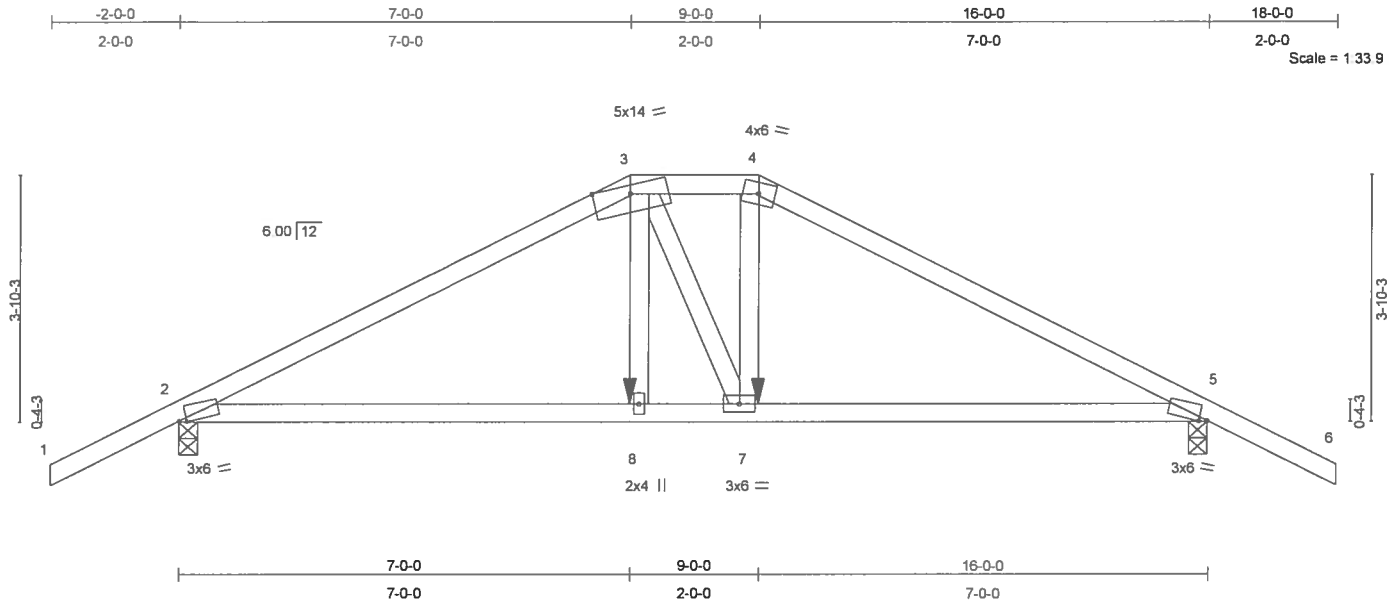

Plate Offsets (X,Y): [2:0-1-8,0-0-7], [5:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.12	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.14	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8
Max Horz 2=77(load case 5)
Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/805, 5-6=0/47
BOT CHORD 2-8=-674/1503, 7-8=-684/1523, 5-7=-658/1506
WEBS 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX
2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.38 and 8 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Ocoee, Florida 34761

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915432
L262673	T18	HIP	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
7) Girder carries hip end with 7-0-0 end setback.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10
   Concentrated Loads (lb)
   Vert: 8=-411(F) 7=-411(F)

December 5,2007

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915433
L262673	T19	QUEENPOST	2	1	Job Reference (optional)

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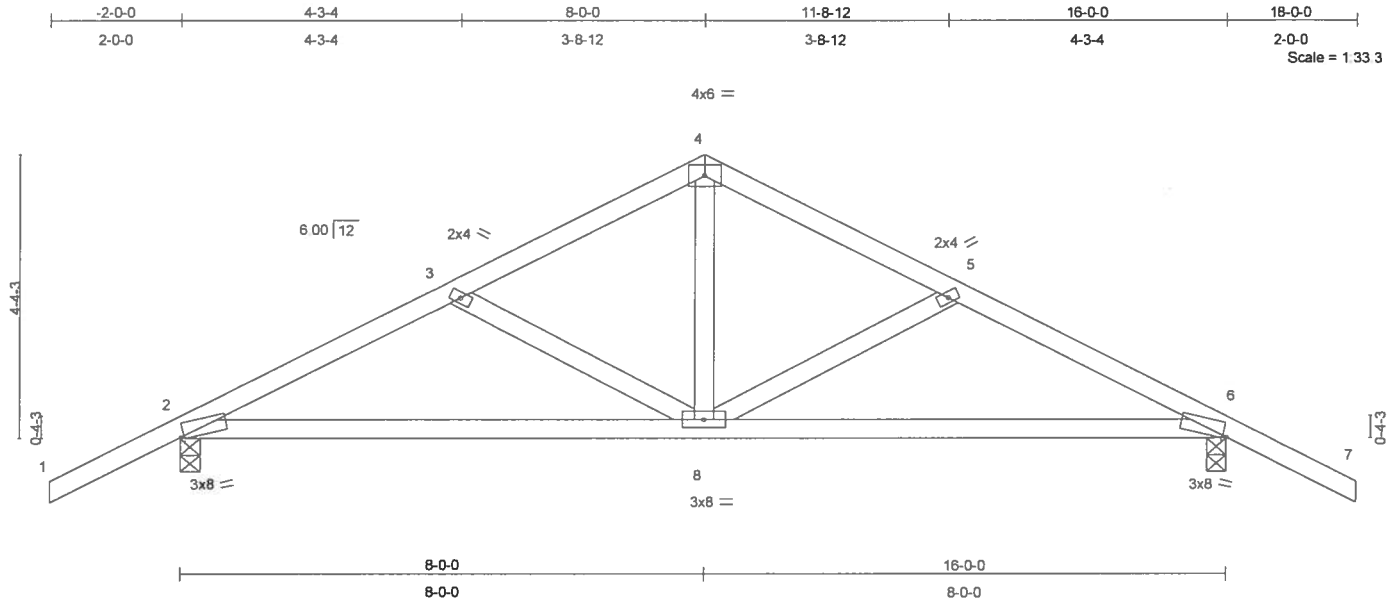

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.19	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	2-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-15 oc bracing.

REACTIONS (lb/size) 2=619/0-3-8, 6=619/0-3-8
Max Horz 2=83(load case 6)
Max Uplift 2=-404(load case 6), 6=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-802/1064, 3-4=-590/942, 4-5=-590/942, 5-6=-802/1064, 6-7=0/47
BOT CHORD 2-8=-810/663, 6-8=-810/663
WEBS 3-8=-217/243, 4-8=-670/331, 5-8=-217/243

JOINT STRESS INDEX
2 = 0.74, 3 = 0.13, 4 = 0.32, 5 = 0.13, 6 = 0.74 and 8 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Truss Design Engineer
Florida PE No. 34810
1109 Emerald Bay Blvd
Devion Beach, FL 32445

December 5,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CROSSWINDS LOT 24 J1915433
L262673	T19	QUEENPOST	2	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 6.

LOAD CASE(S) Standard

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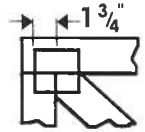
Truss Design Engineer
Florida PE No. 34627
1100 Coastal Bay Blvd
Ocean Beach, FL 32435

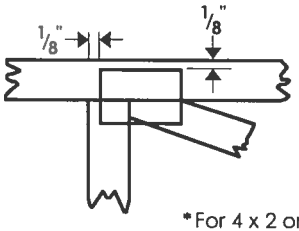
December 5,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

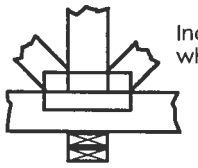
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

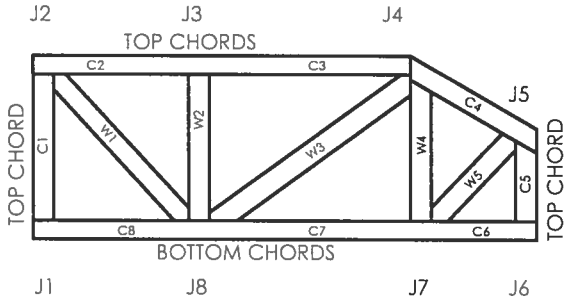
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

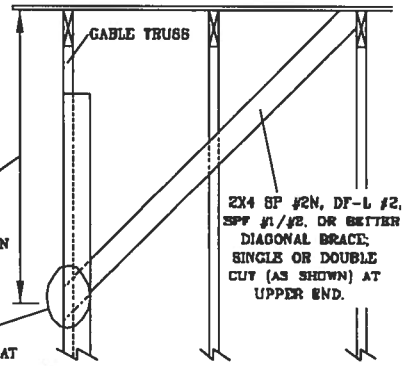
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

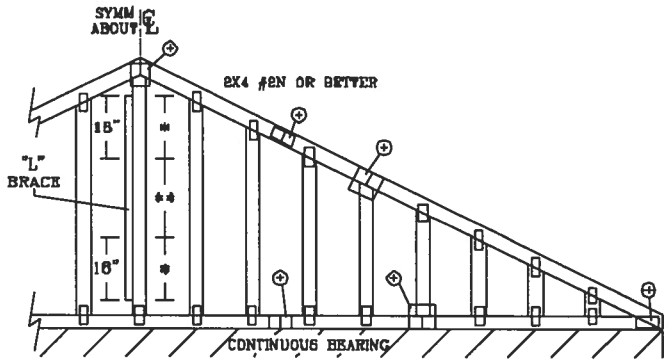
ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE / GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 6"	8' 6"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 8"	12' 8"	14' 0"	14' 0"

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 540# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#3	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 16 E HT
-ENG	

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 8"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	5' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

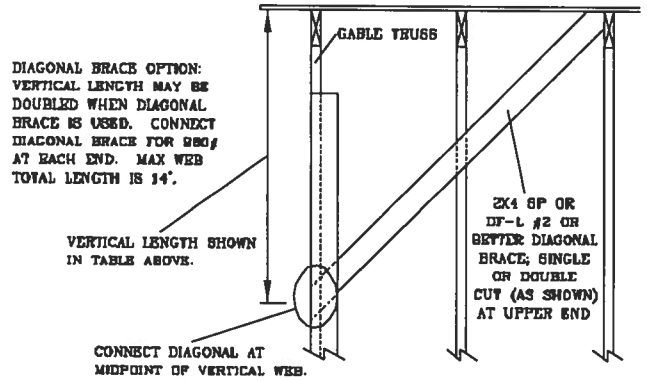
SOUTHERN PINE
#3
STUD
STANDARD

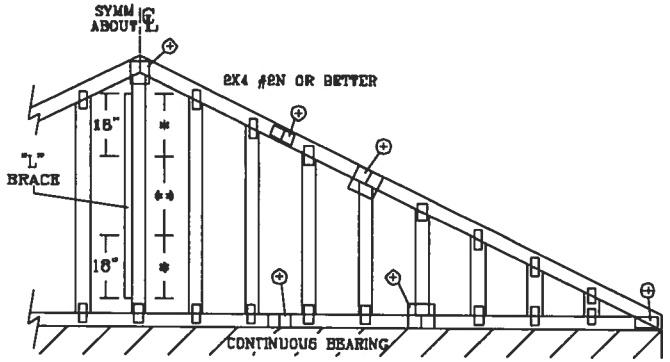
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#3
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

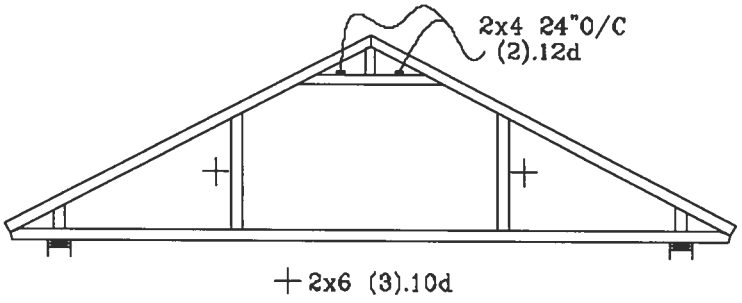
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

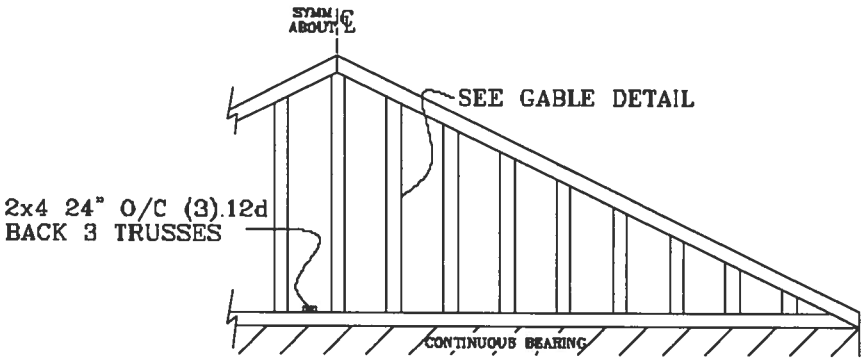
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 30' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

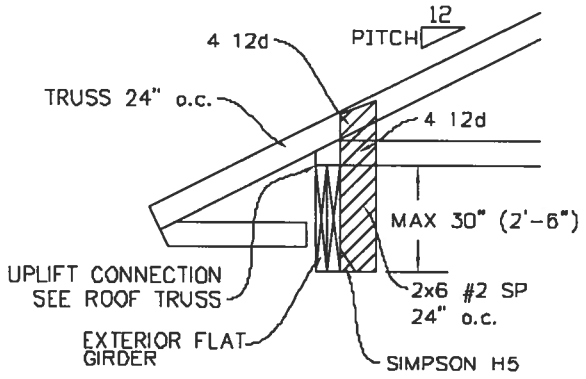


GABLE END TRUSS DETAIL

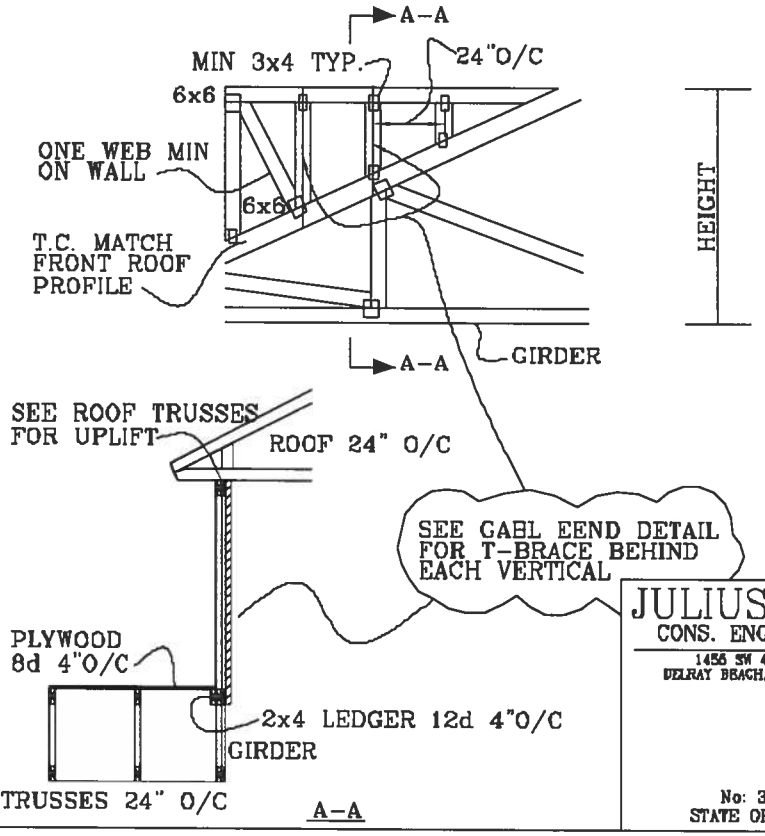


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



JULIUS LEE'S
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1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

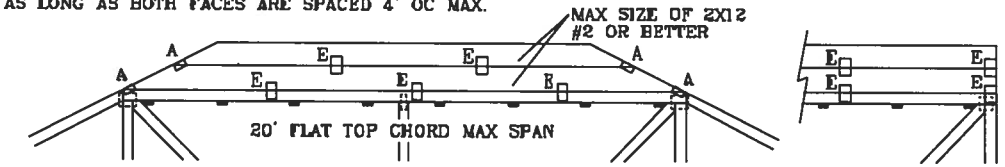
110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

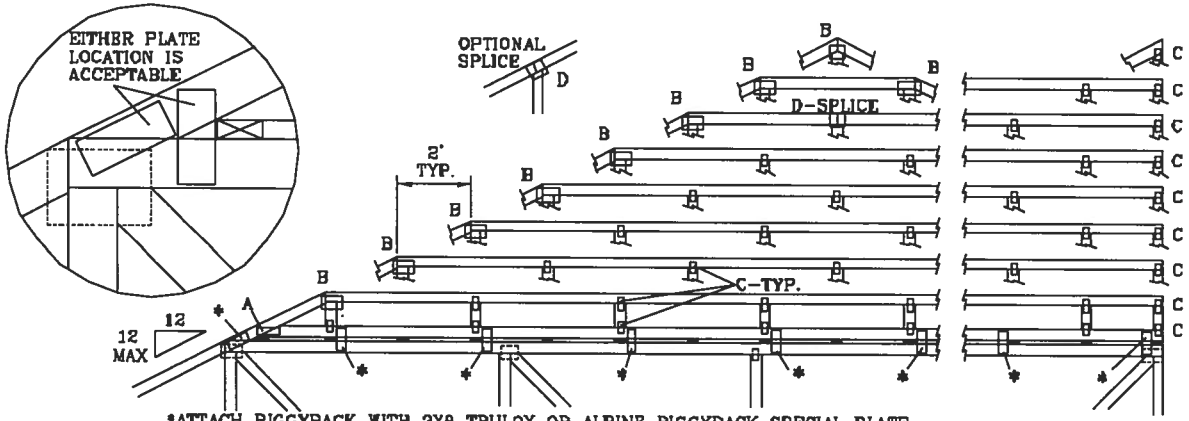
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

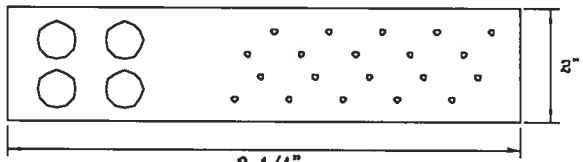




WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

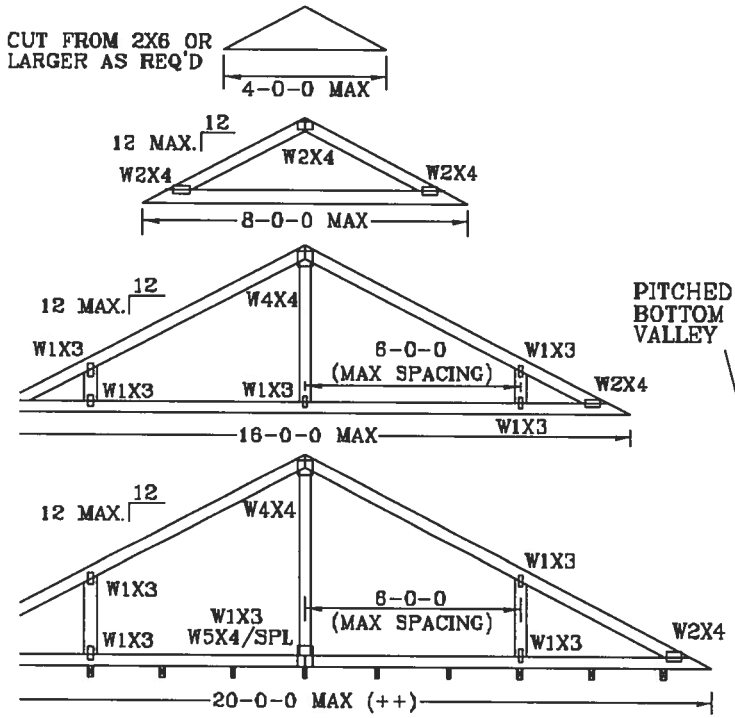
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

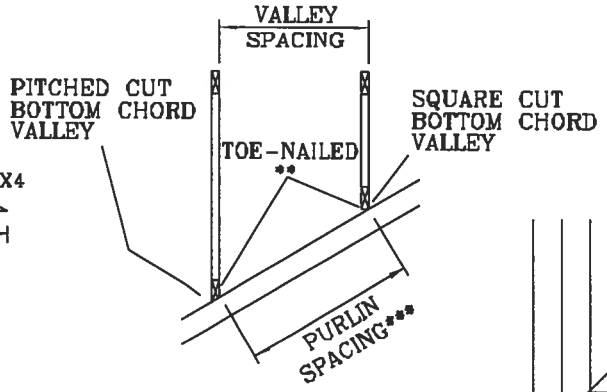
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

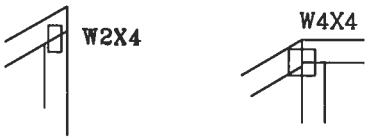
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



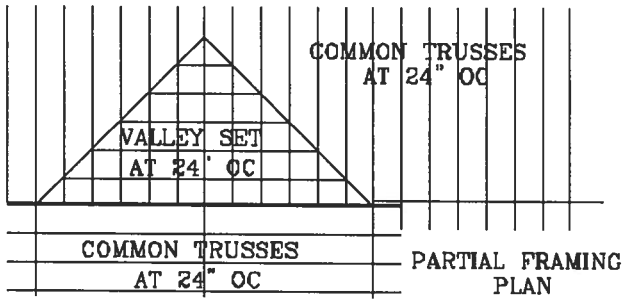
SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.





OPTIONAL STUB END DETAIL

OPTIONAL HIP JOINT DETAIL



PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

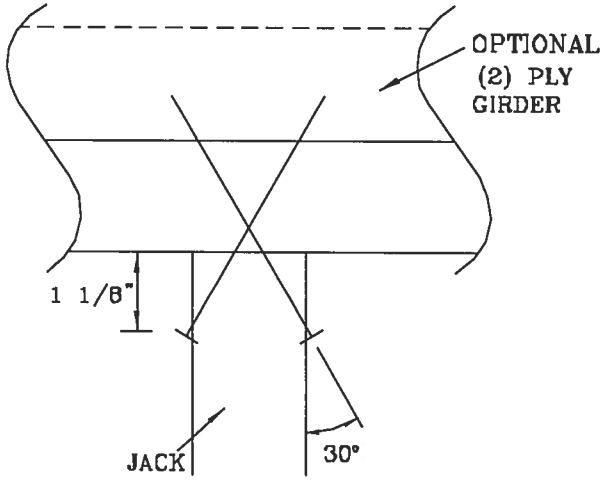
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

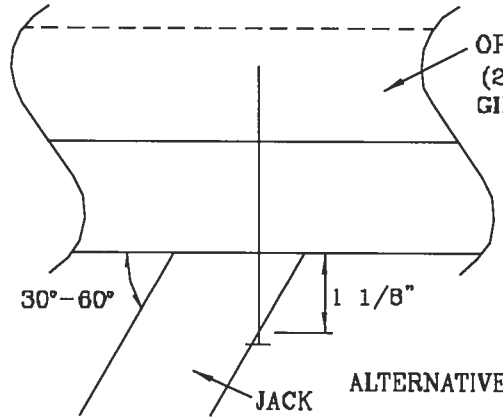
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

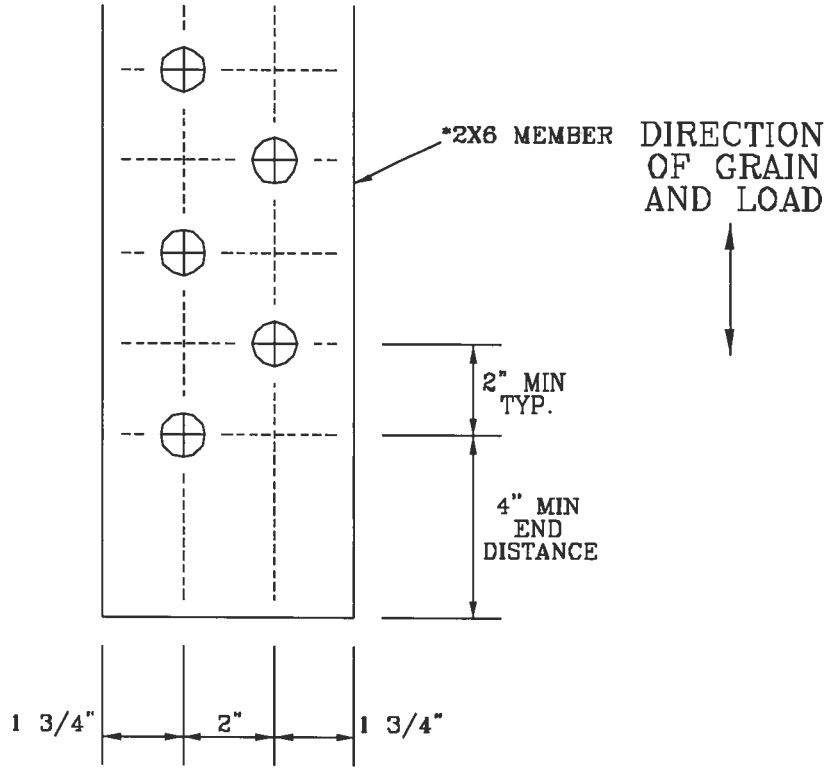
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

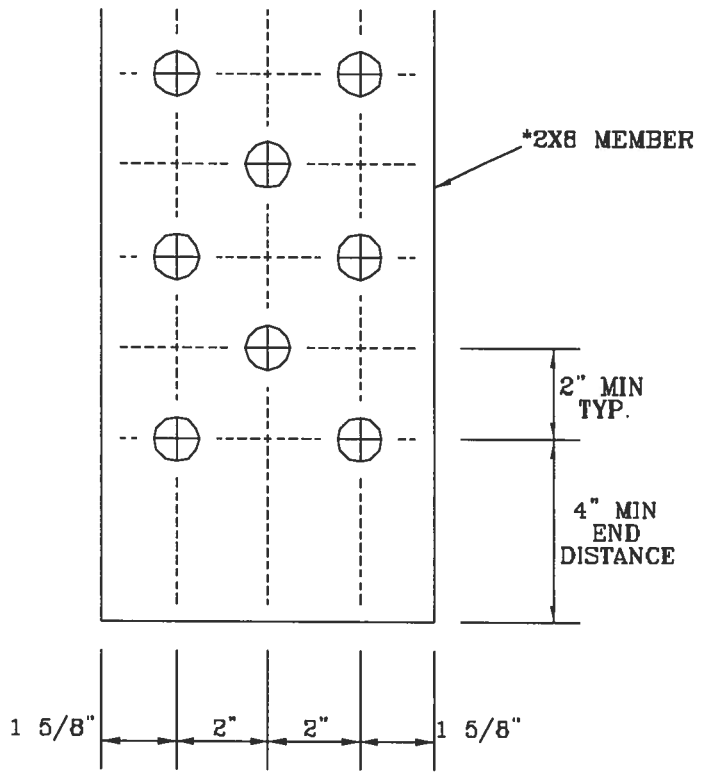
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

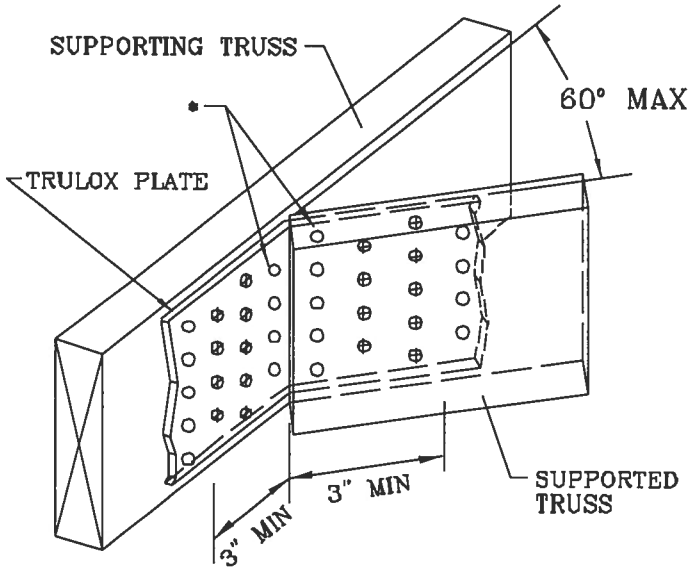
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

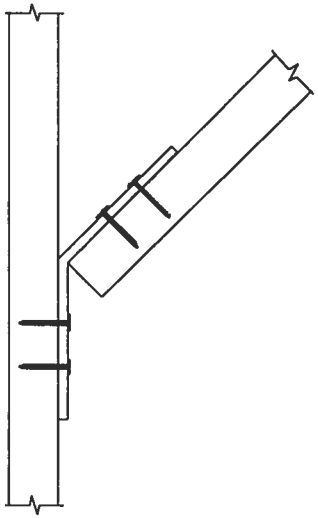
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

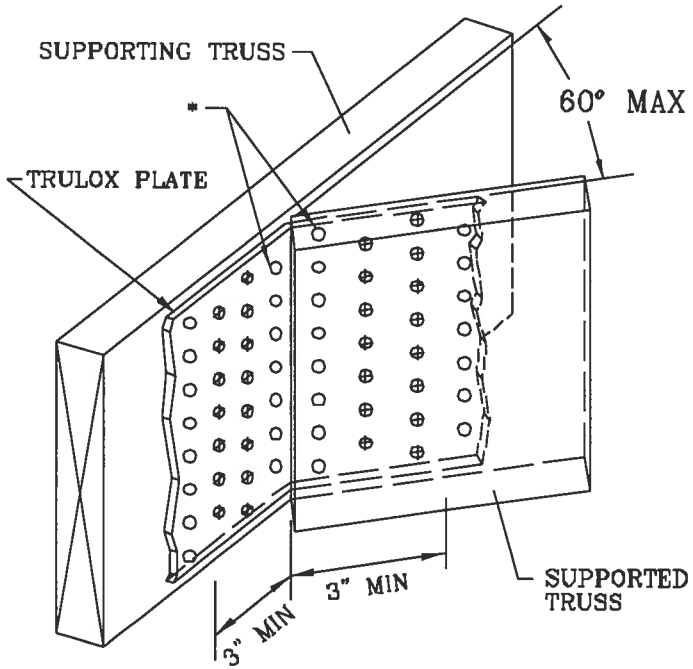
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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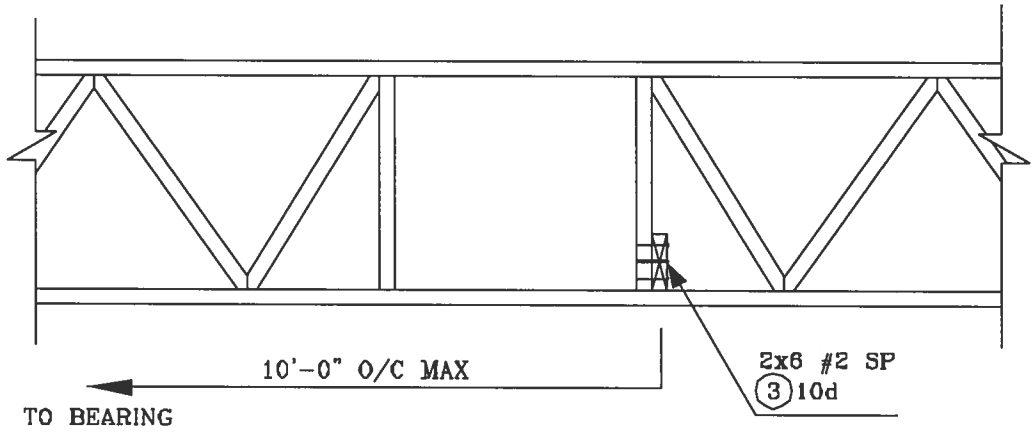
DELRAY BEACH, FL. 33444-2161

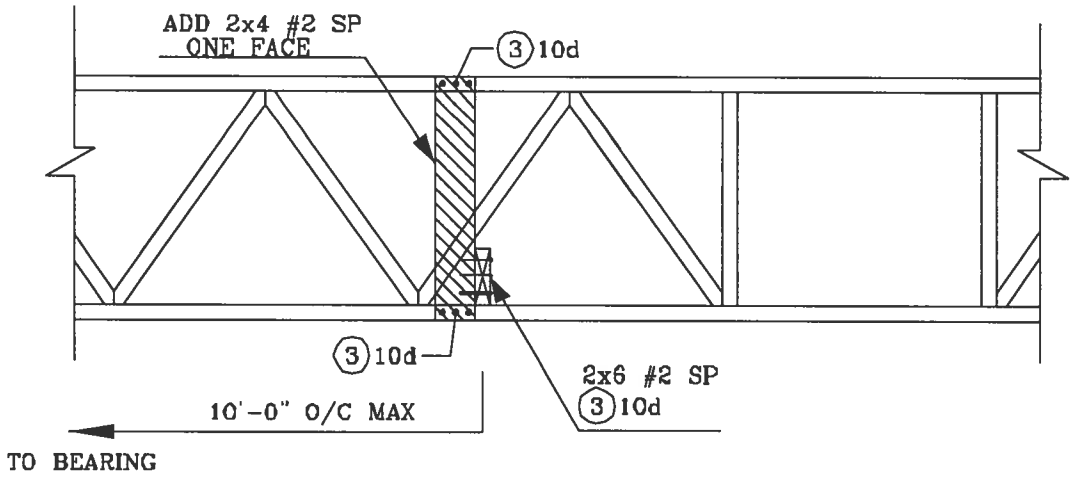
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

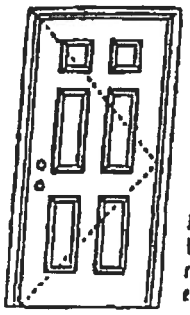
X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Design Pressure

+66.0/-66.0

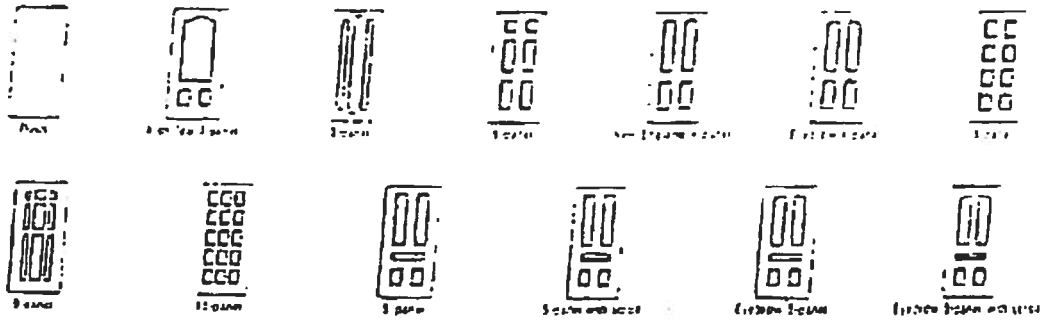
Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED

MINIMUM ASSEMBLY DETAIL:

MINIMUM INSTALLATION DETAIL:

APPROVED DOOR STYLES:



Johnson EntrySystems

PREMDOOR

Masonite

Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Baltz

State of Florida, Professional Engineer
Kurt Baltazor, P.E. – License Number 56533

Test Data Report Conforms #3025444-A and COP/Test Report Variations Matrix #3025444A-001 provides additional information - available from the IFS Web and Static Forms database. The Masonite criteria forms database can be the Masonite Intranet server.

Johnson EntrySystems

June 17, 2002

PRESTOOR

Masonite

Masonite International Corporation