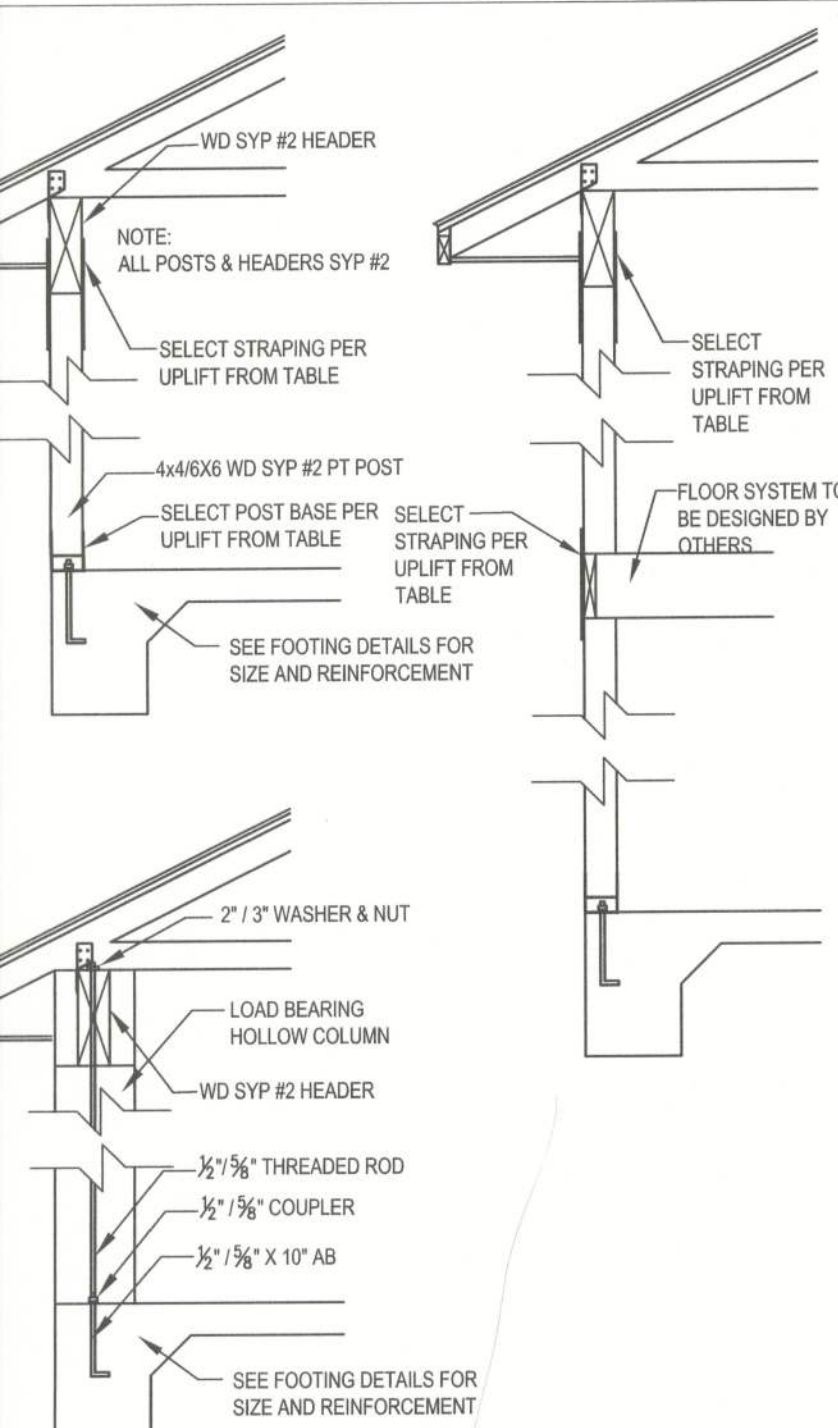


TYPICAL TRUSS UPLIFT & MAX 12" WALL HEIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE SP4/SP6 SPACING
770 LB	48" O.C.	48" O.C.	N/A
880 LB	48" O.C.	32" O.C.	N/A
1270 LB	32" O.C.	16" O.C.	32" O.C.
1500 LB	24" O.C.	16" O.C.	16" O.C.
2200 LB	LT103 W/ 5/8" X 7' WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4#6

NOTE:
MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.

W1 - SINGLE STORY EXT. WALL SECTION
SCALE: 1/2\"/>



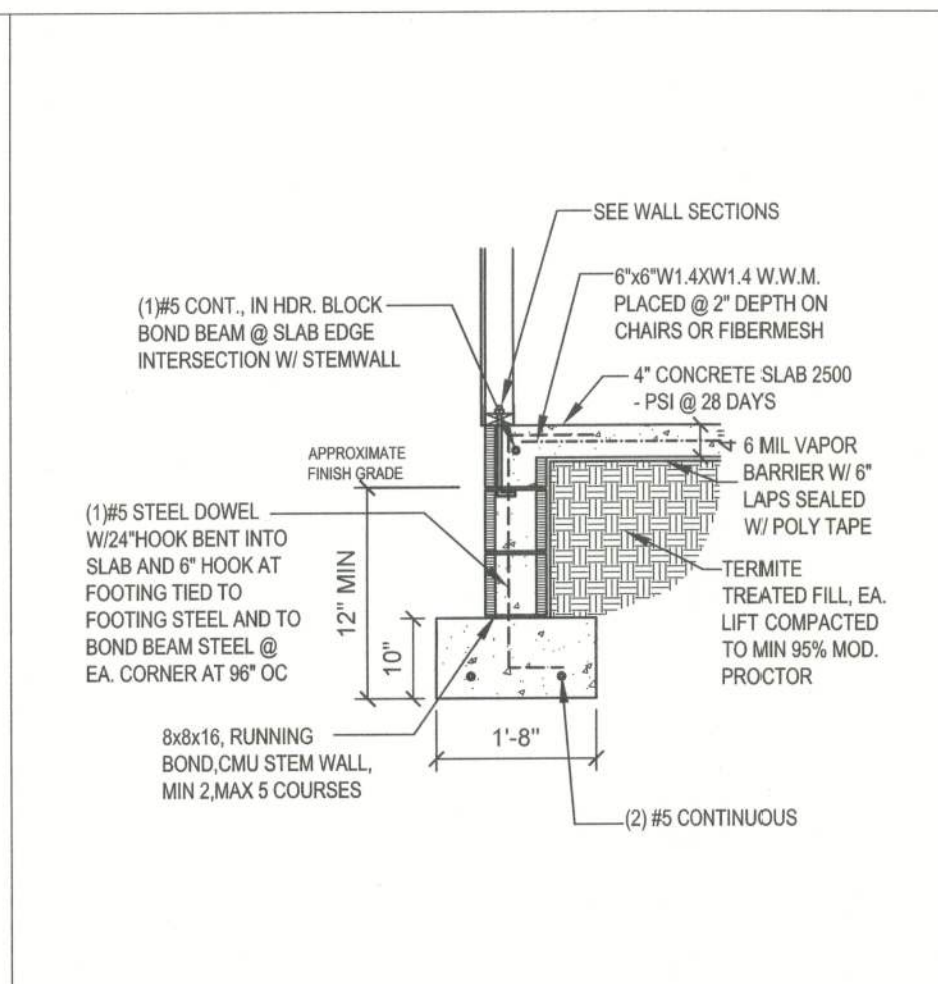
SYP #2 FT WD POSTS

TYPICAL POST UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPPING	HEADER STRAPPING
555 LB	AS404 W/ (6)-10d & 2\"/>	(2) LSTA21 W/ (6)-10d EA.	(2) LSTA21 W/ (6)-10d EA.
720 LB	AS404 W/ (8)-10d & 3\"/>	(2) LSTA21 W/ (6)-10d EA.	(2) LSTA21 W/ (6)-10d EA.
2200 LB	AS404 W/ (12)-10d & (3)-5/8\"/>	(2) LSTA21 W/ (10)-10d EA.	(2) LSTA21 W/ (10)-10d EA.
2300 LB	AS404 W/ (12)-10d & (3)-5/8\"/>	(2) LSTA21 W/ (10)-10d EA.	(2) LSTA21 W/ (10)-10d EA.

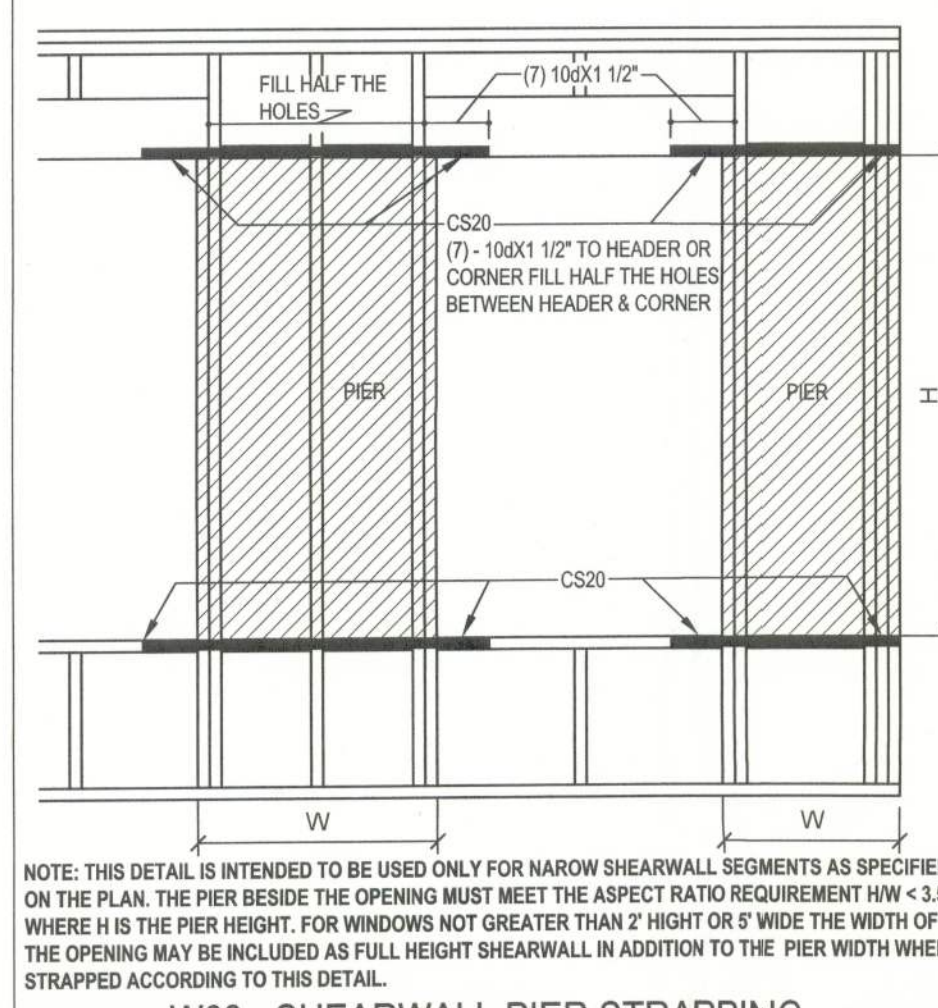
HOLLOW COLUMN

2\"/>

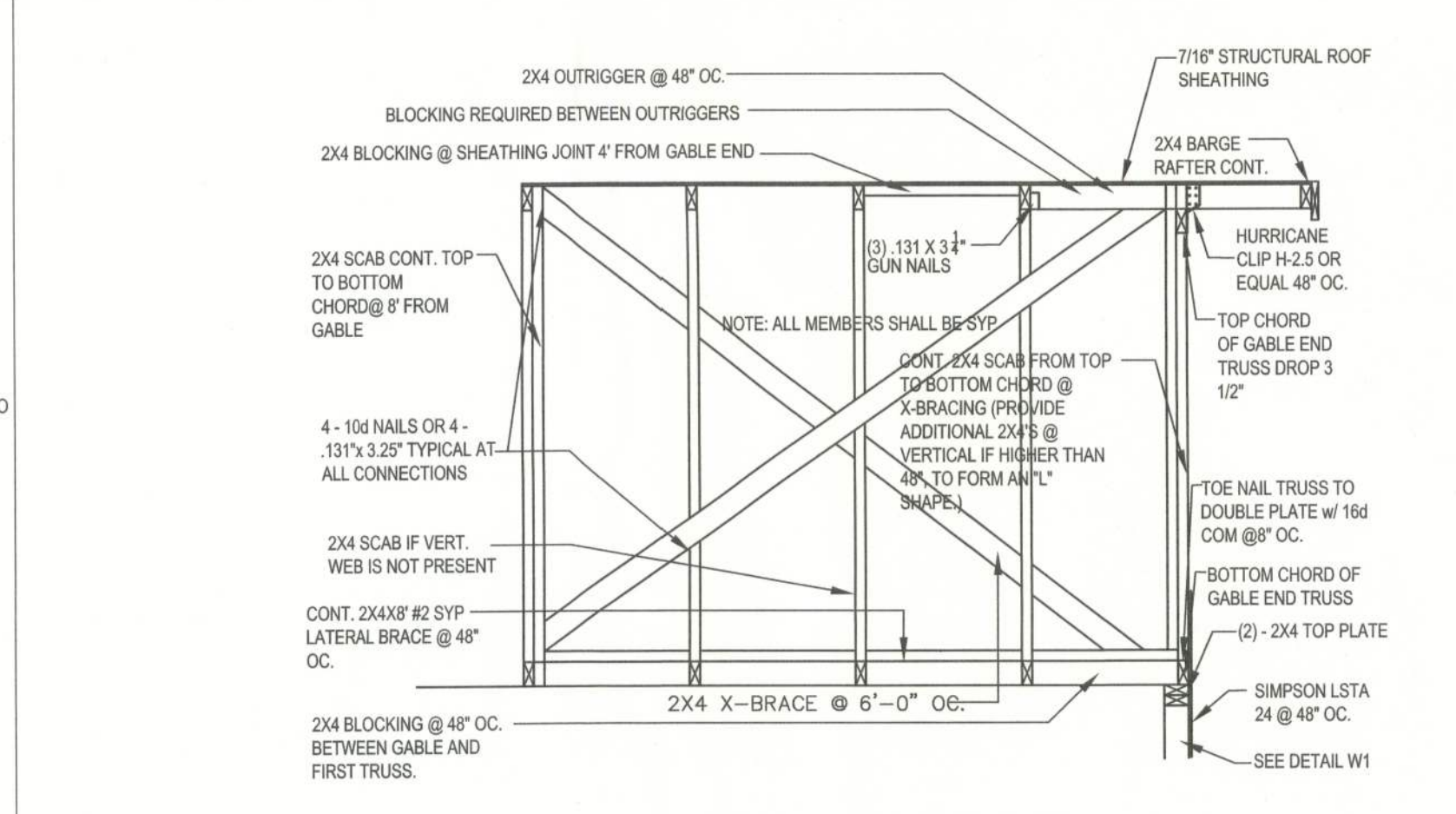
W12 - PORCH HEADER ANCHORS
SCALE: N.T.S. REV-18-JUL-03



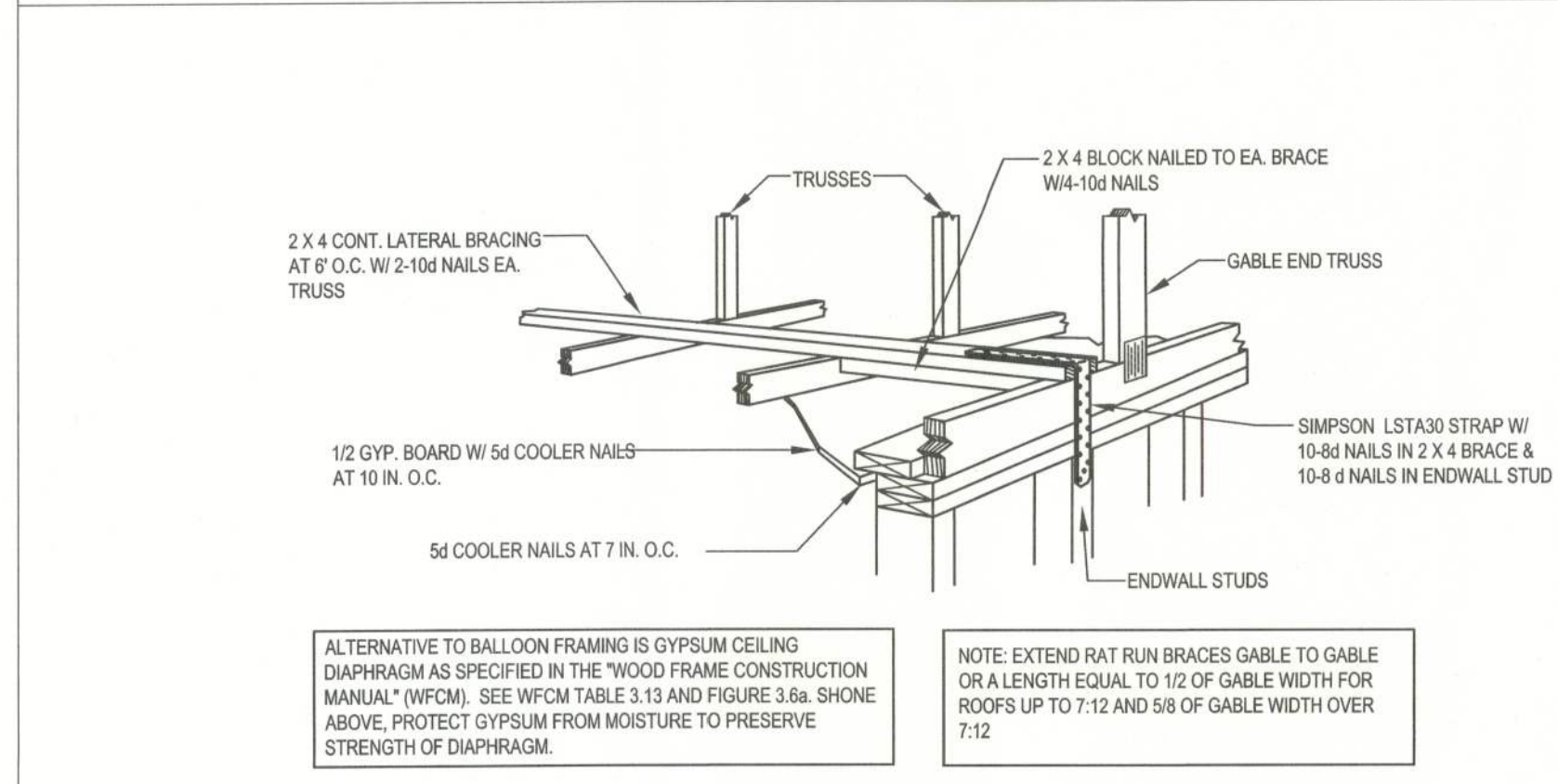
F1 - STEM WALL FOUNDATION
SCALE: 1/2\"/>



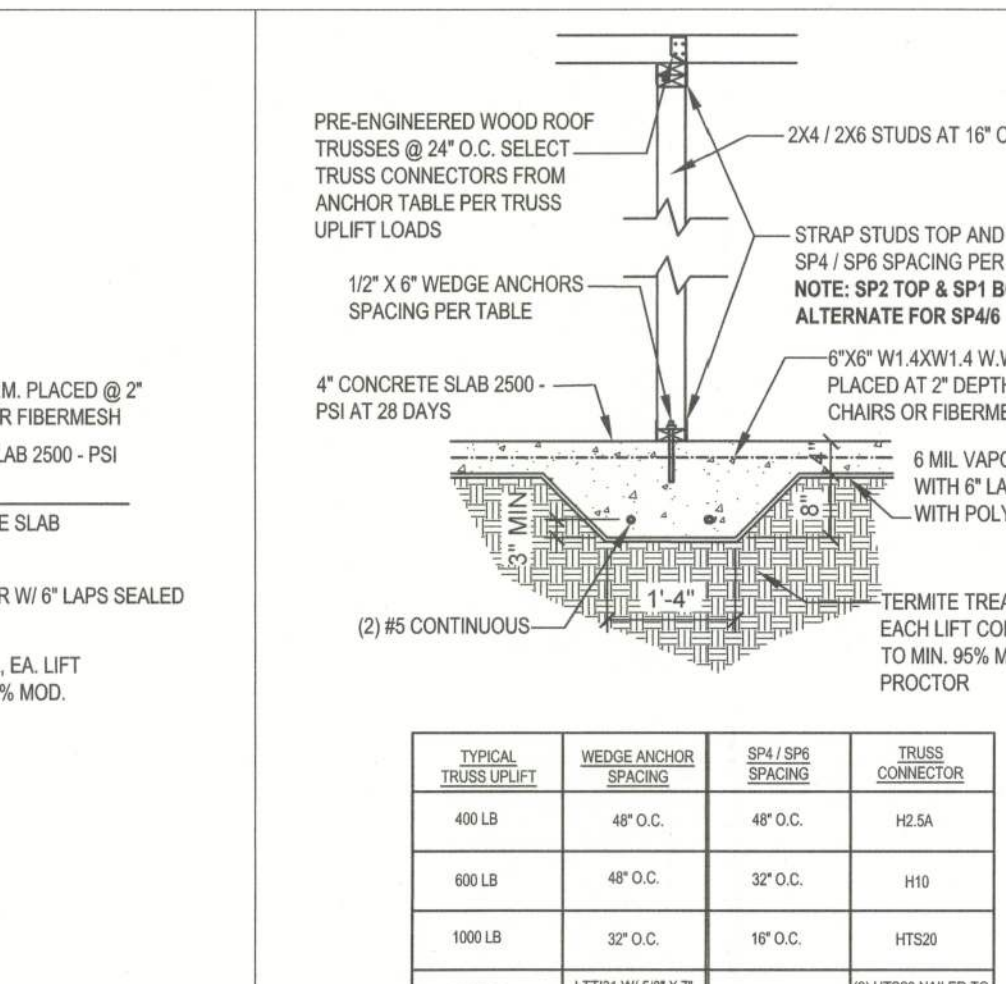
W68 - SHEARWALL PIER STRAPPING
SCALE: 1/2\"/>



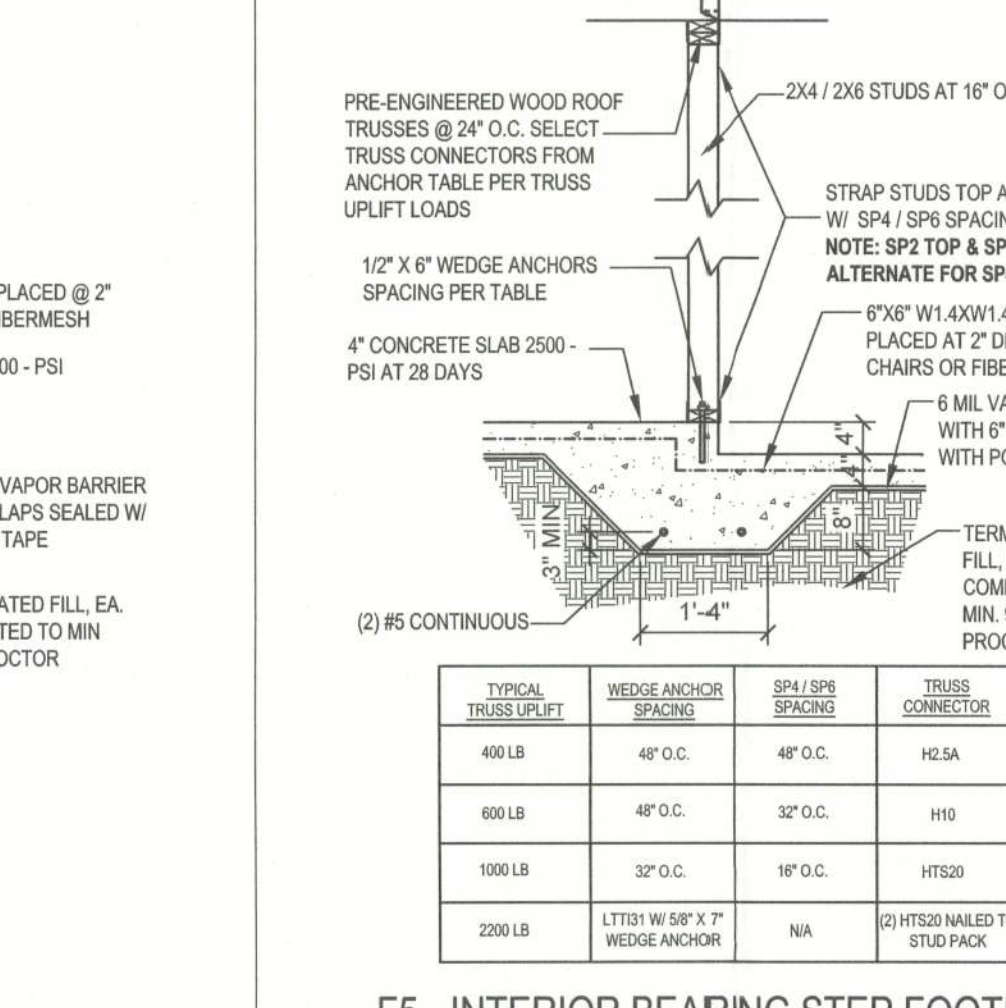
W10 - TYPICAL GABLE END (X-BRACING)
SCALE: 1/2\"/>



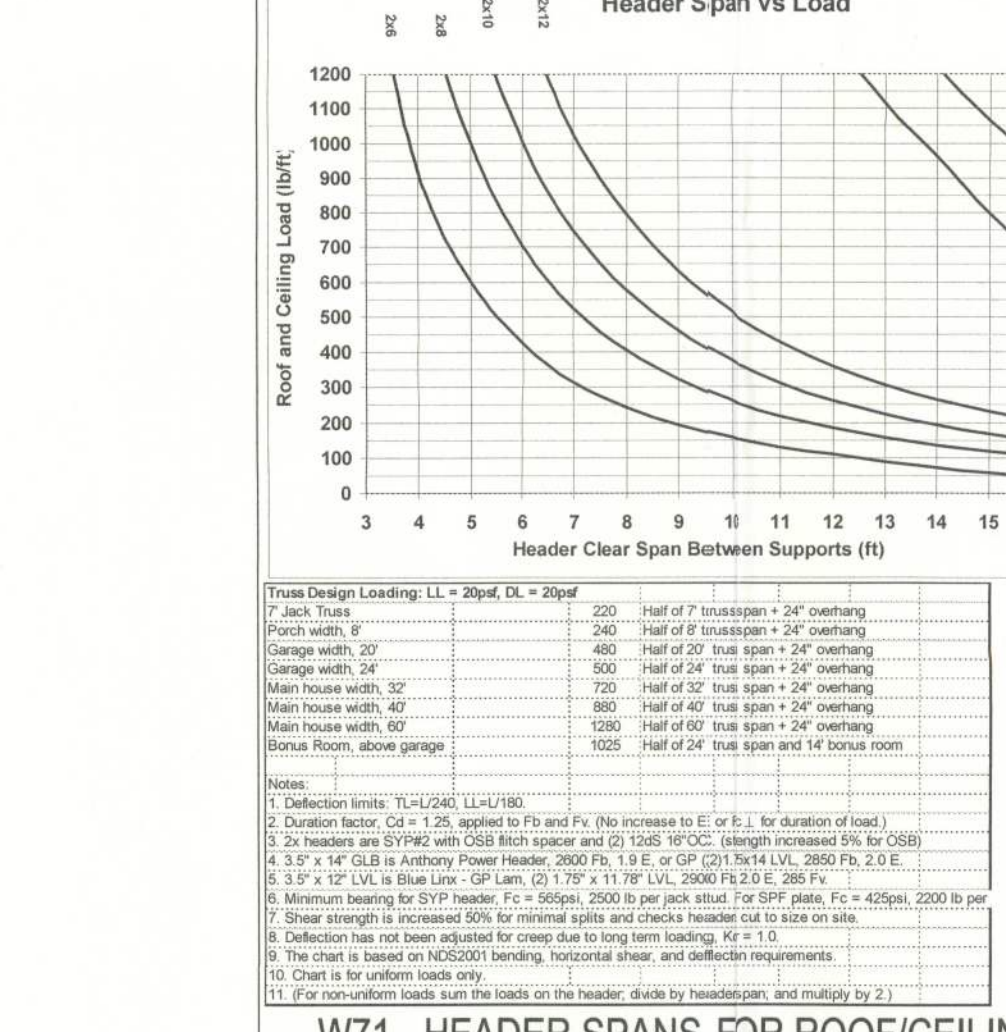
W23 - GYPSUM CEILING DIAPHRAGM OPTION - GABLE END WALL
SCALE: N.T.S.



F4 - INTERIOR BEARING FOOTING
SCALE: 1/2\"/>



F5 - INTERIOR BEARING STEP FOOTING
SCALE: 1/2\"/>



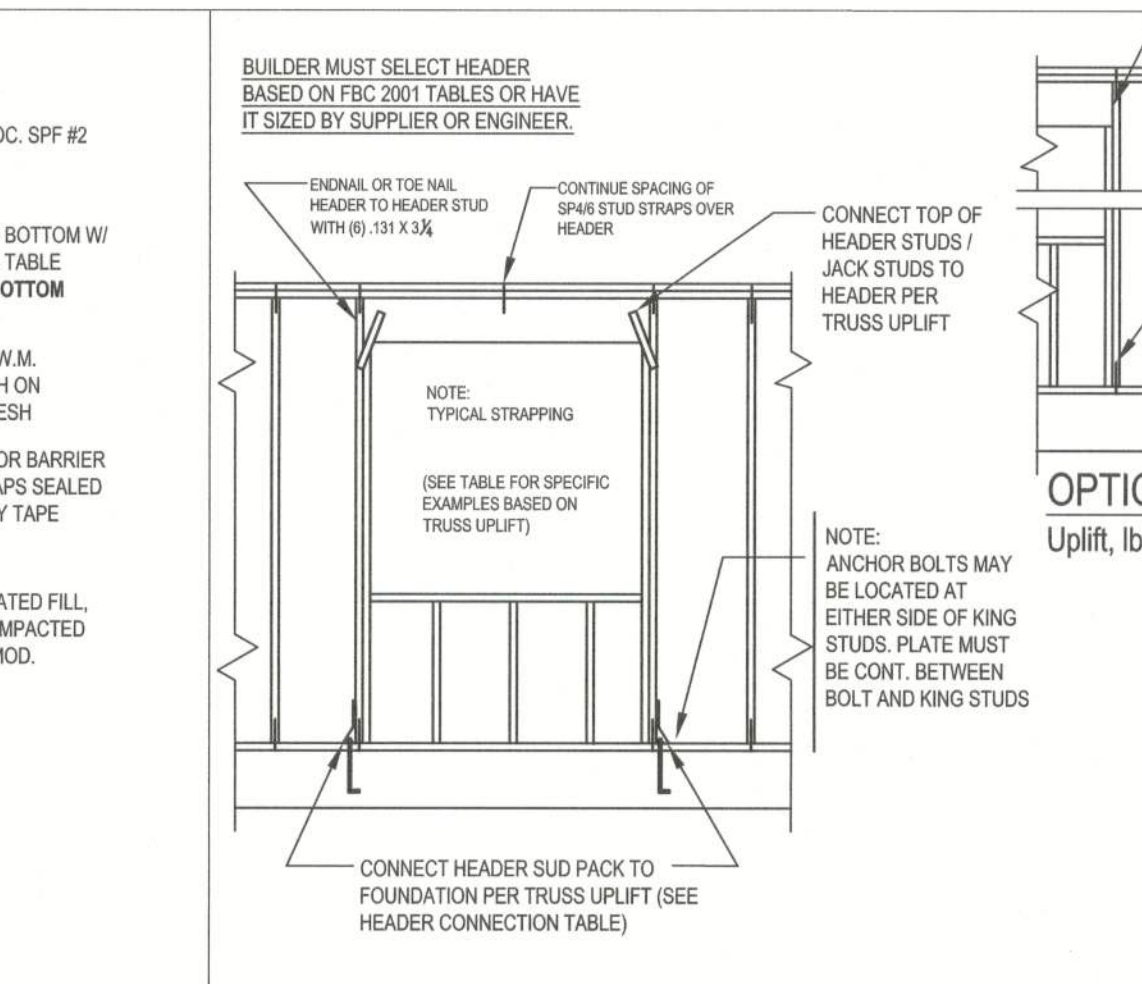
W71 - HEADER SPANS FOR ROOF/CEILING LOAD

N5 - TRUSS UPLIFT CONNECTOR TABLE REV-19-NOV-04

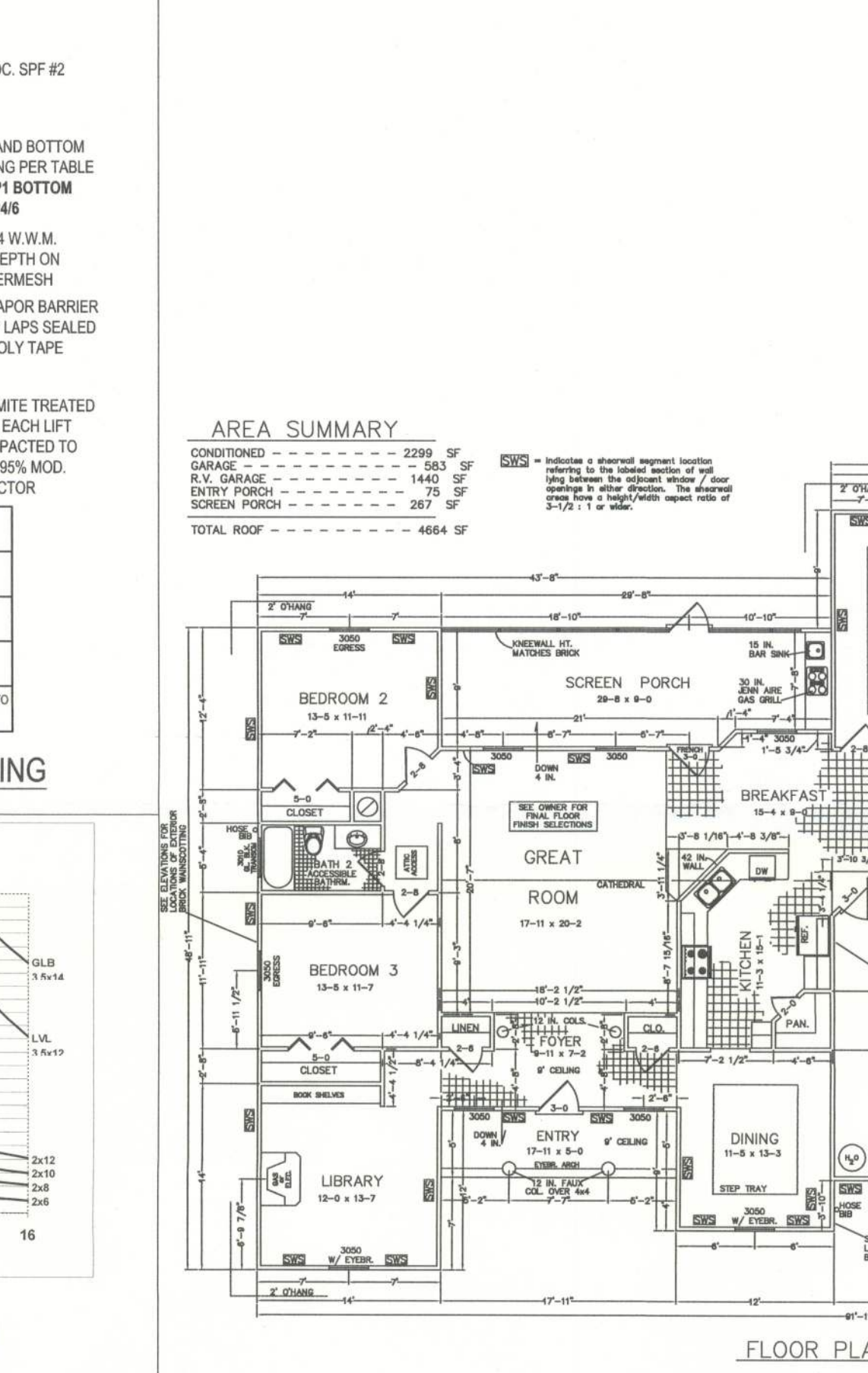
All connectors are Simpson Strongtie, uno. Sealed top and bottom connections from this table or SST catalog to meet truss uplift. Use fasteners as specified.

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	600	H25A	5-8d	5-8d
620	720	H10	6-10d(1 1/2\"/>	6-10d(1 1/2\"/>
850	990	LTS12	8-8d(1 1/2\"/>	8-8d(1 1/2\"/>
1245	1450	HTS20	10-10d (2-10d(1 1/2\"/>	10-10d (2-10d(1 1/2\"/>
1265	1470	H16, H16-2	10-10d(1 1/2\"/>	2-10d(1 1/2\"/>
1785	2050	LG2	14-10d linker	18-18d Sinker
3655	4200	MGT	5\"/>	22-10d
SPF	SYP	Strap Connector	To One Member	To Other Member
760	885	SP4	6-10d(1 1/2\"/>	N/A
865	1005	CS20	9-8d or 10d	9-8d or 7-10d
1085	1285	LSTA18-24	7-10d	7-10d
1170	1380	SP4	12-10d(1 1/2\"/>	N/A
420	455	SSP	4-10d	3-10d to double plate or 1-10d to single
600	825	DSP	8-10d	6-10d to double plate or 2-10d to single
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d
SPF	SYP	Column Anchor	To foundation	To Column / Truss
1160	1350	LTT19	5\"/>	8-18d Sinkers
1985	2310	LTT31	5\"/>	18-10d(1 1/2\"/>
2385	2775	H22A	5\"/>	25\"/>
3590	4175	HTT18	5\"/>	18-16d
1975	2200	ABUR6	5\"/>	12-16d

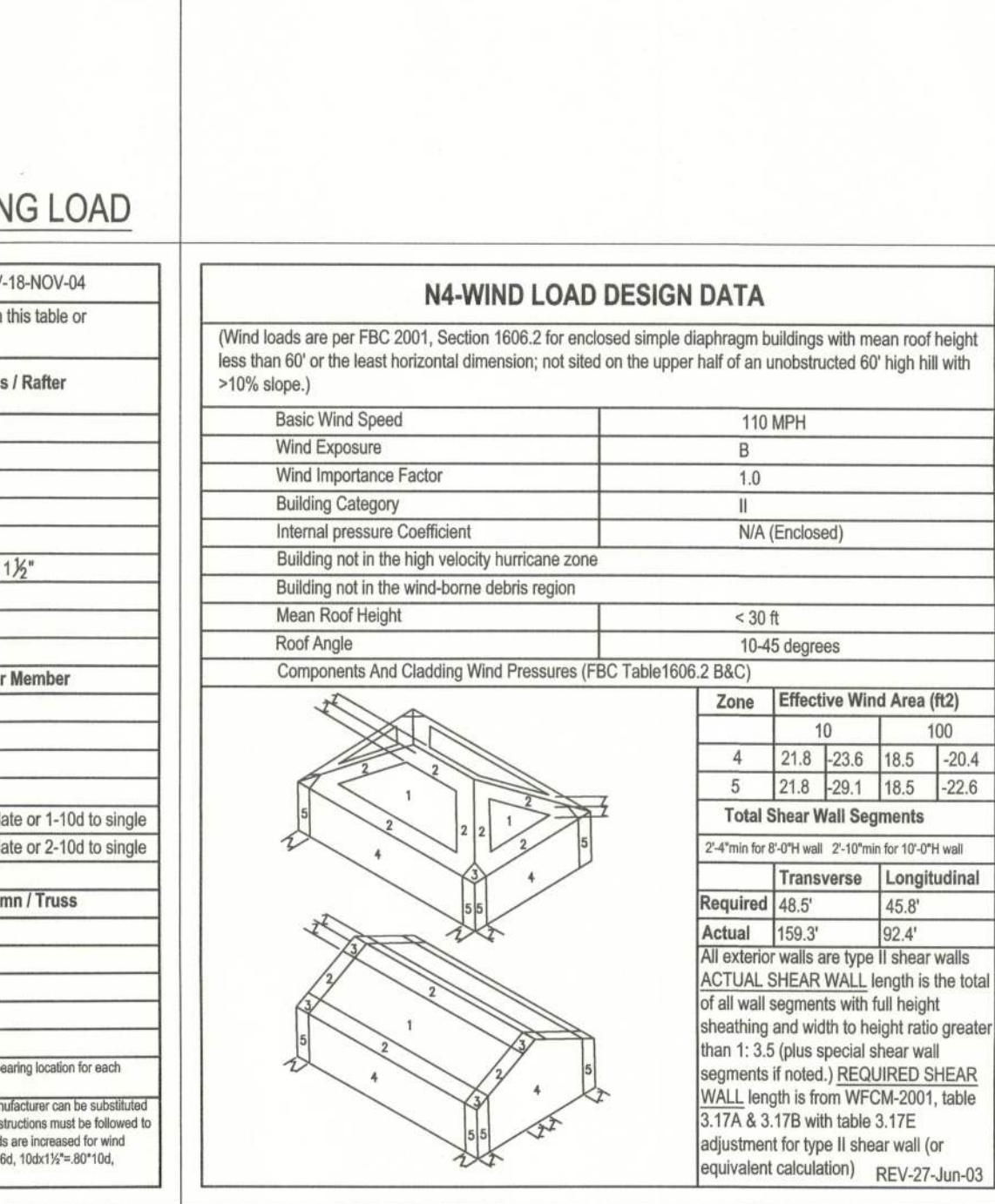
Notes:
1. Decking loads: LL = 20psf, DL = 20psf.
2. Jack Truss:
Plate width: 8\"/>



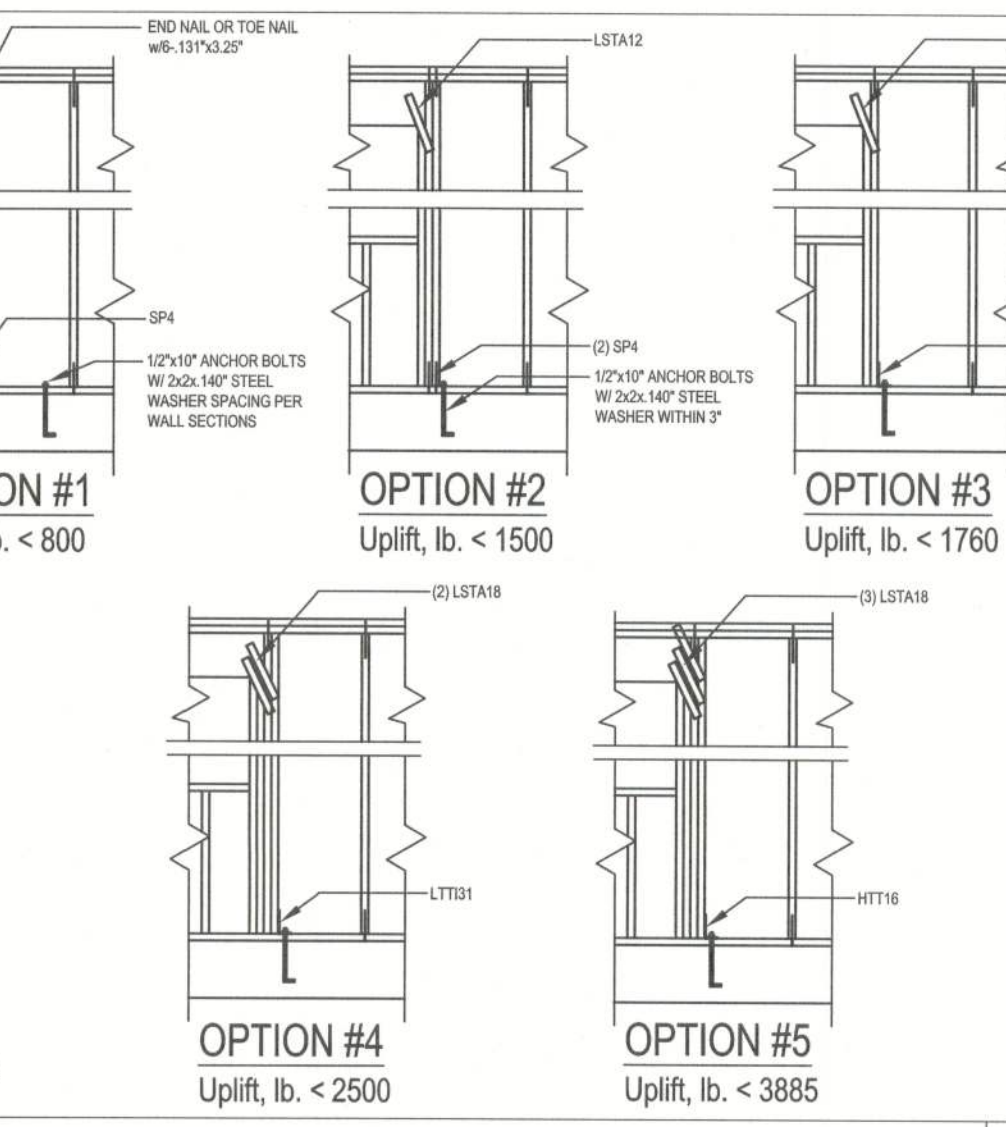
W13-TYPICAL HEADER SIZING & STRAPPING DETAIL
SCALE: N.T.S. REV-22-AUG-03



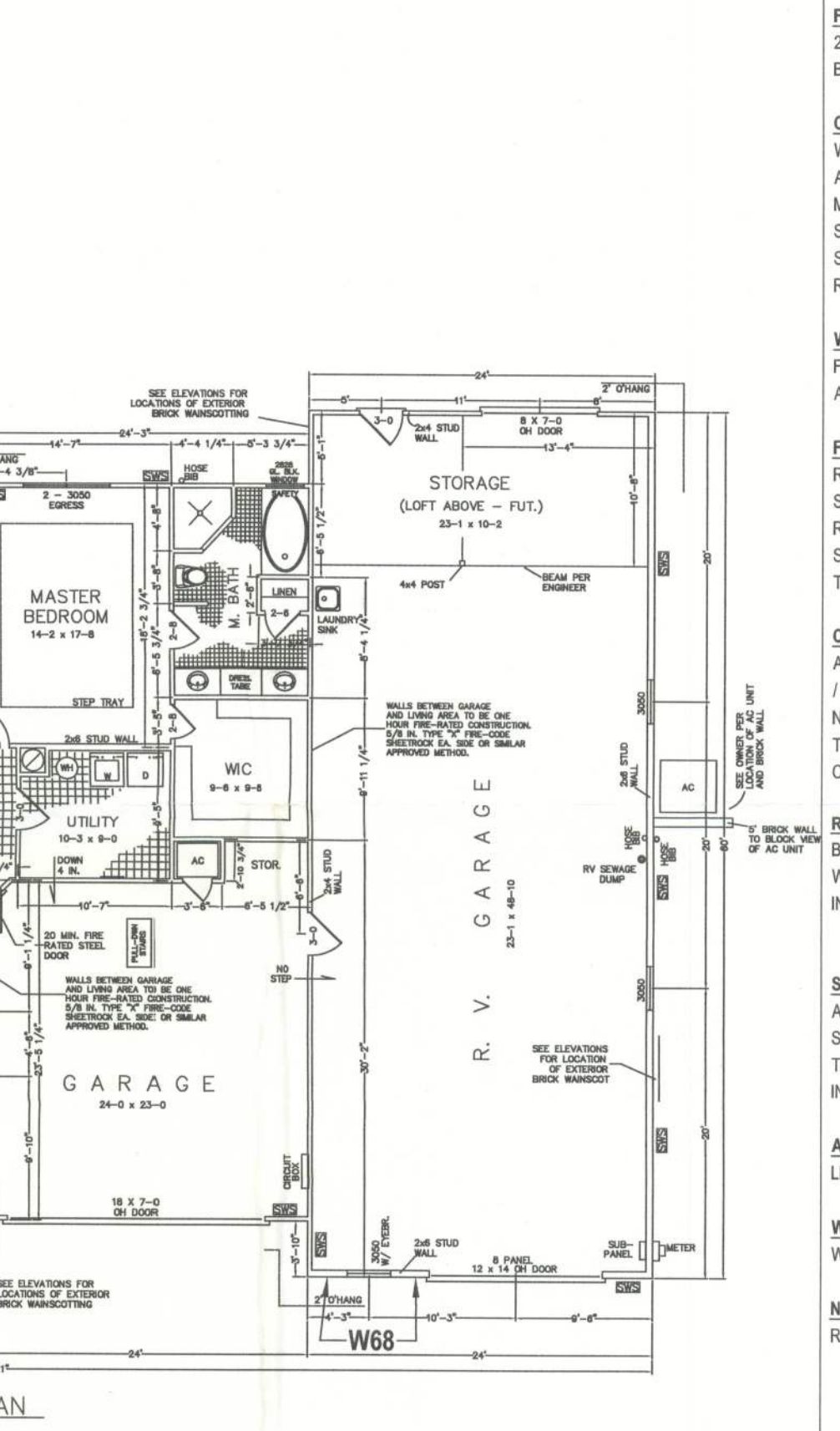
FLOOR PLAN



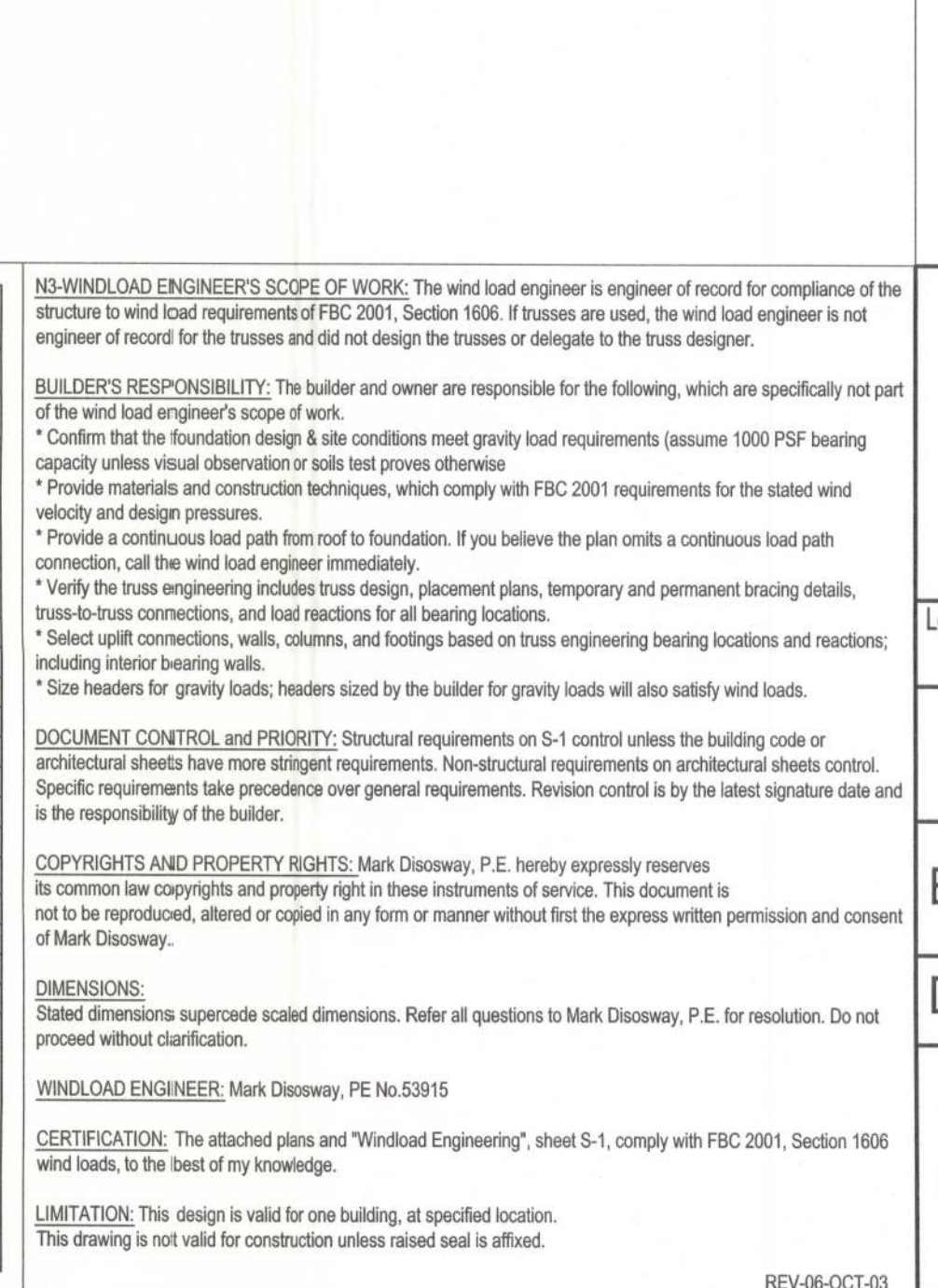
N4-WIND LOAD DESIGN DATA



OPTION #1, OPTION #2, OPTION #3, OPTION #4, OPTION #5



FLOOR PLAN



N3-WINDLOAD ENGINEER'S SCOPE OF WORK

Load Bearing Header Sizing Methods (BY BUILDER)

- Determine header size from FBC 2001, Tables 2308.3 A, B, C, or 2308.5.
- Use supplier published data or Southern pine span tables.
- For engineered lumber beams have suppliers engineer size beam.
- Lookup jack studs from FBC 2001, Tables 2308.3 A, B, C, or 2308.5.
- Use one jack stud for every 3000 lb vertical load.
- Total king plus jack studs = studs needed to be there if no opening was there.

Header Uplift Connectors (BY BUILDER)

- Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.
- Select header connections from table below or mfg. catalog to connect header to stud (top connection) and stud to foundation (bottom connection).

Option #	Uplift, lb.	Top Connector	Bottom Connector
#1	< 800	End nail or bar nail w/ 131x3.25"	SP4, 6-10d(1 1/2")
#2	< 1500	LSTA12, 10-10d	(2) SP4, 6-10d(1 1/2")
#3	< 1760	LSTA18, 14-10d	HTT18, 18-16d (1 1/2")
#4	< 2500	(2) LSTA18, 14-10d	HTT18, 18-16d (1 1/2")
#5	< 3885	(2) LSTA18, 14-10d	HTT18, 18-16d (1 1/2")

Uplift greater than 3885 lb requires engineering design.

FBC2001, TABLE 2308.3A
Header Spans For Exterior Bearing Walls Supporting Roof+Ceiling (20psf+20psf)

Header Spans (ft-in)	Building Width / Truss Span (ft)			
	Span	NJ	Span	NJ
2-2x4	3-6	1	3-2	1
2-2x6	5-5	1	4-8	1
2-2x8	6-10	1	5-11	2
2-2x10	8-5	2	7-3	2
2-2x12	9-9	2	8-5	2
3-2x8	8-4	1	7-5	1
3-2x10	10-6	1	8-1	2
3-2x12	12-2	2	10-7	2
4-2x8	9-2	1	8-4	1
4-2x10	11-8	1	10-6	1
4-2x12	14-1	1	12-2	2

Notes: NJ = Number of jack studs required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on header.

N2-GENERAL NOTES:

FOUNDATION: FOR POINT LOADS GRATER THAN 5000 LB OR REPETITIVE TRUSS LOADS GRATER THAN 2000 LB TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0" X 1 sq ft. FOR EVERY 1000 LB OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE Fc = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 X W1.4, Fy = 80 KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185 LOCATED IN MIDDLE OF SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 12 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 80 KSI. ALL LAPS SPICES 48" @ 30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" X 2" X 9/64"; WITH 5/8" BOLTS TO BE 3" X 3" X 9/64"; WITH 3/4" BOLTS TO BE 3" X 3" X 9/64"; WITH 7/8" BOLTS TO BE 3" X 3" X 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

REV-27-JUL-04

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

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Ted & Catherine Bryan
Residence

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Designer: Tim Delbene Job #05038

Approved: FLPE#53915 Revisions:

Sheet S-1 of 1 Sheet
Windload Engineering
Job # 506282