

Office copy



NEW CUSTOM HOME FOR:
THE GRIEST RESIDENCE

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8867

DATE:

CHECKED BY:

SHEET NUMBER

A-1

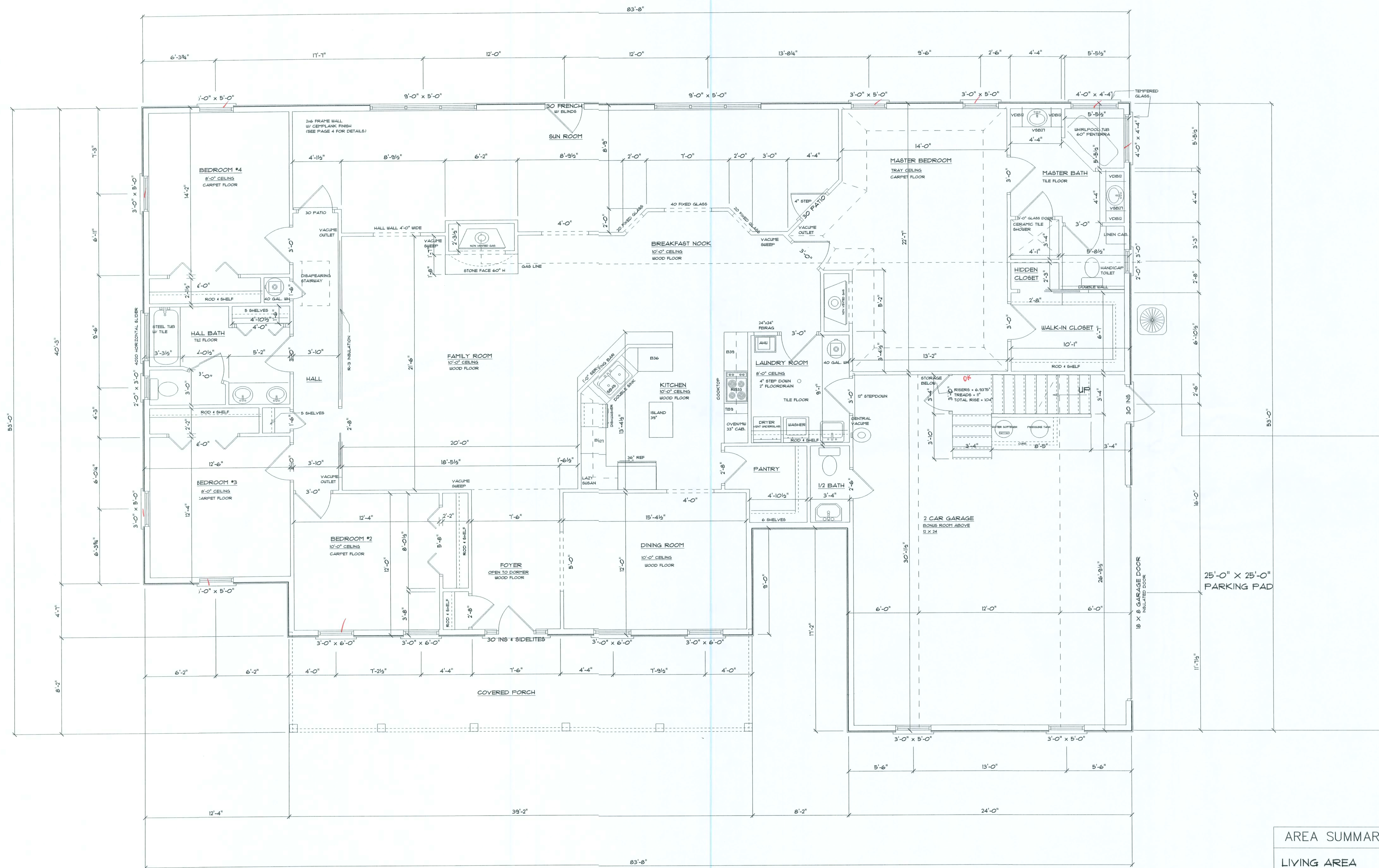
OF 4 SHEETS

AREA SUMMARY

LIVING AREA - 2668.30 SF
GARAGE - 723.00 SF
PORCHES - 165.80 SF
TOTAL AREA - 4157.10 SF

ELEVATION PLAN

SCALE: 1/4"=1'-0"



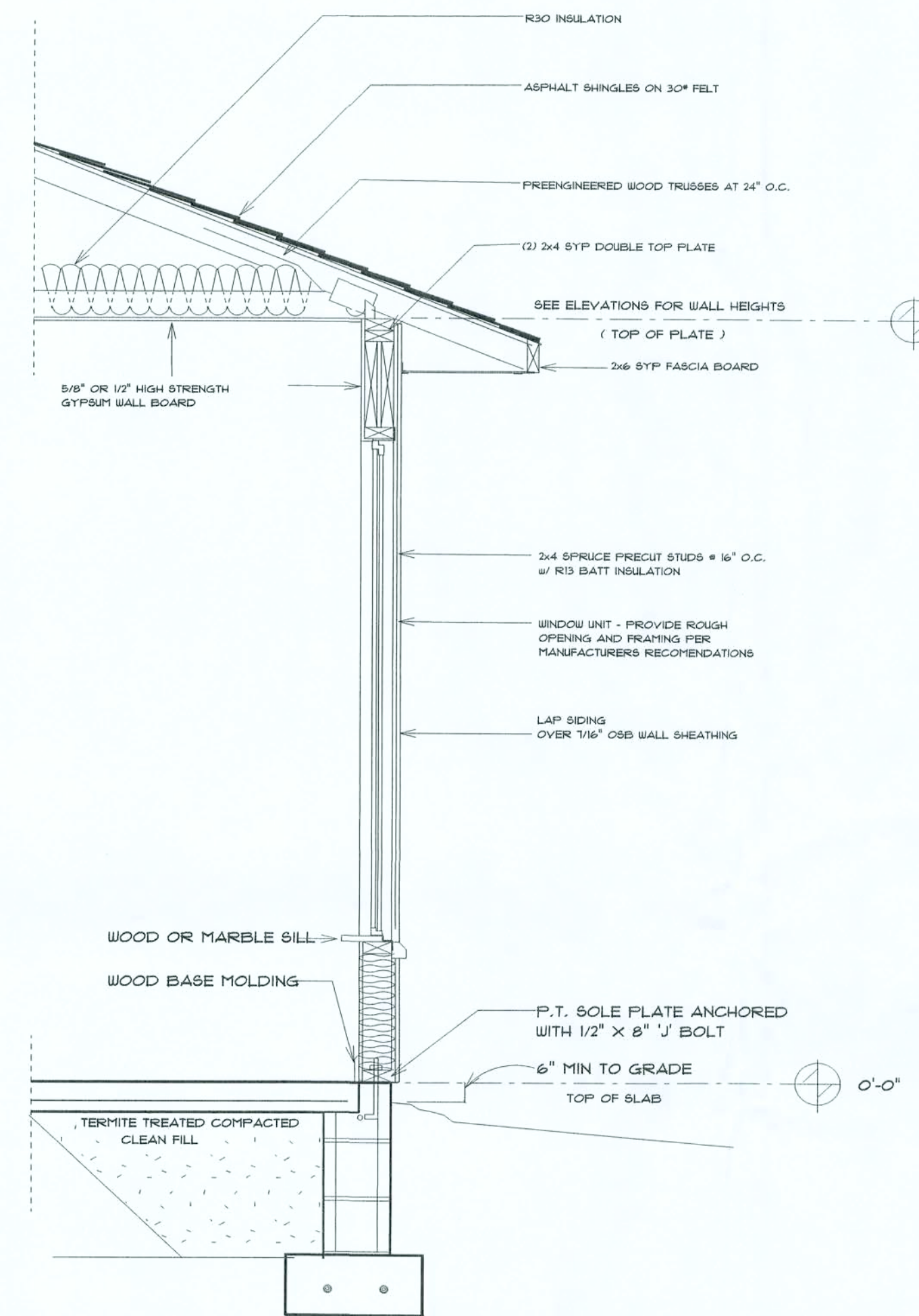
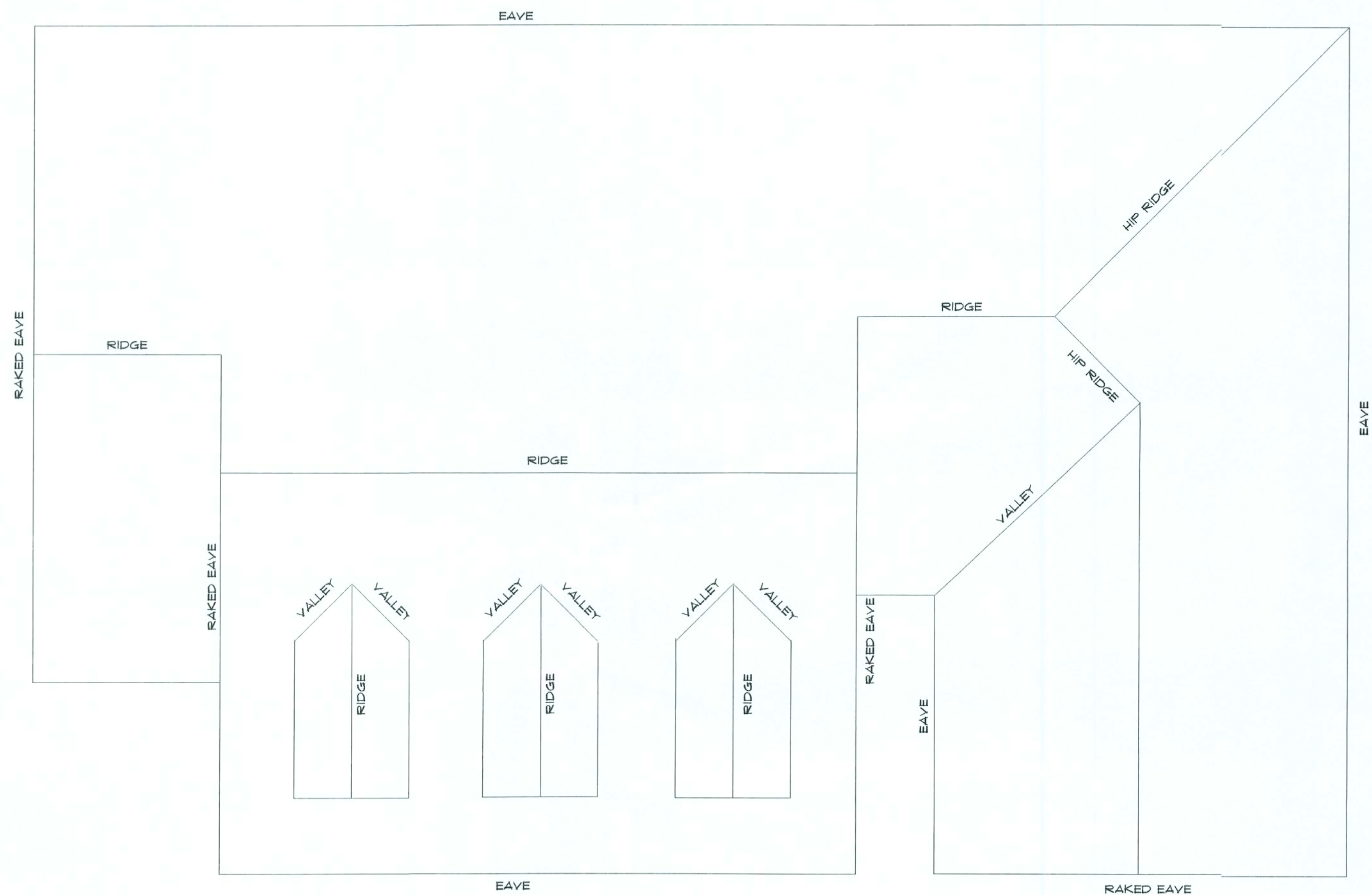
AREA SUMMARY	
LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

MAIN FLOORPLAN
SCALE: 1/4"=1'-0"

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A-2
OF 4 SHEETS



TYPICAL WALL SECTION

2 X 4 STUD WALL

AREA SUMMARY

LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

ROOF PLAN

SCALE: 1/4"=1'-0"

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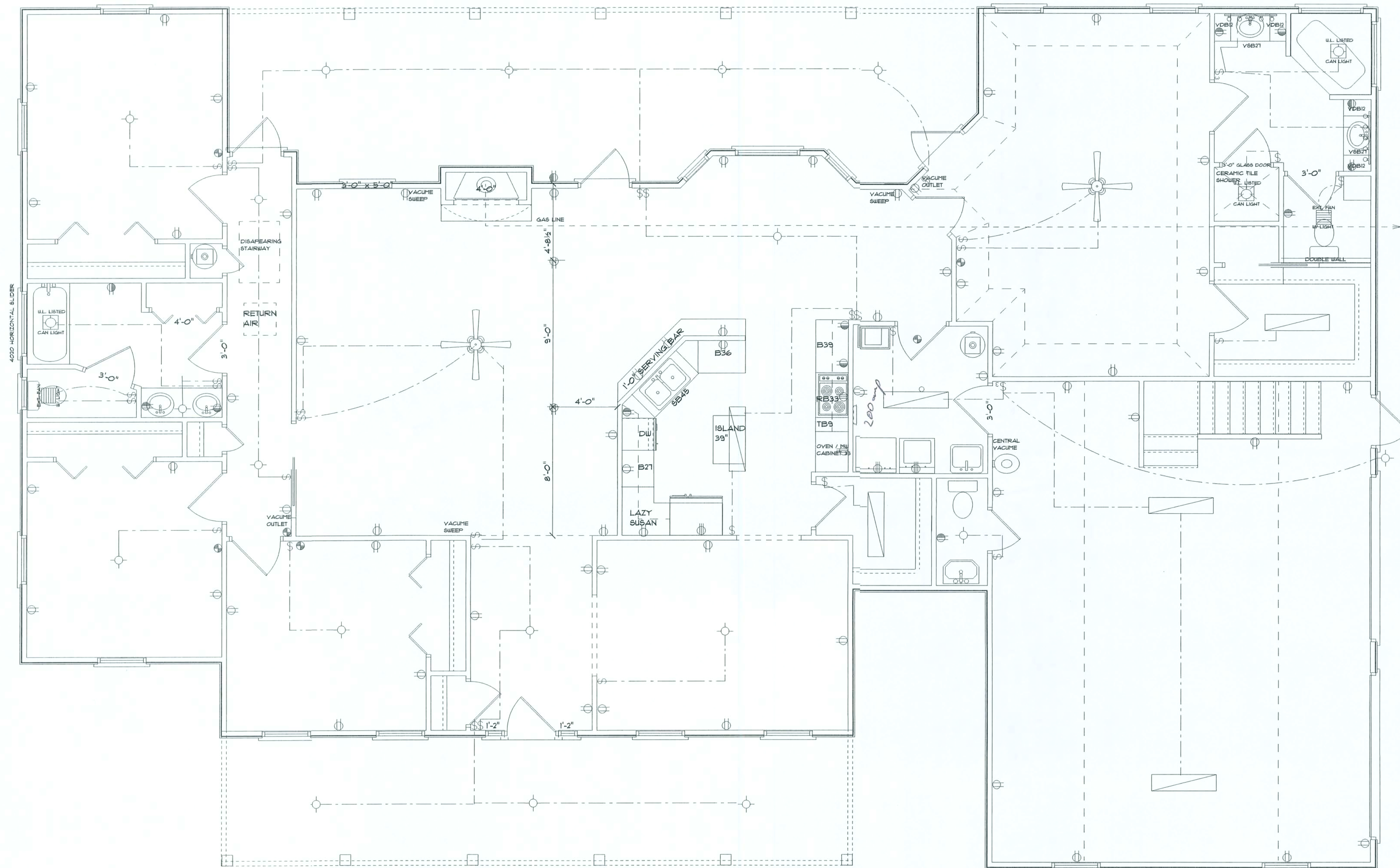
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SHEET NUMBER

A-3

OF 4 SHEETS



Electrical	
	220 Volt Outlet
	3 Way Switch
	CAN LIGHT
	EXT FAN
	Light
	Outlet
	Switch
	clg fan spots 2
	elec panel
	Fluor fixture
	smoke detector
	split recept
	vanity bar lt

ELECTRICAL PLAN NOTES

ALL RECEPTALS IN ALL BEDROOMS SHALL BE AFC CIRCUITS
WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONTR SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

AREA SUMMARY

LIVING AREA -	2668.30 SF
GARAGE -	123.00 SF
PORCHES -	165.80 SF
TOTAL AREA -	4157.10 SF

ELECTRICAL PLAN

SCALE: 1/4"=1'-0"

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A-4

OF 4 SHEETS

7/16" OSB ROOF SHEATHING UNBLOCKED
NAILED TO ROOF FRAMING @ COMMON NAILS
6" O.C. EDGES, 12" O.C. FIELD, 4" O.C. GABLES

PRE-ENGINEERED WOOD ROOF TRUSSES
AT 24" O.C. SELECT TRUSS CONNECTORS
FROM THE ANCHOR TABLE
PER TRUSS UPLIFT LOADS

(2) 2x4/6 SPF#2 DOUBLE TOP PLATE

SP#46 @ 48" O.C.

24" MAX

NOTE: SEAL ALL PENETRATIONS
IN TOP PLATE AND FIRE
STOP BLOCKING WITH CODE
APPROVED SEALANT

NOTE: IF TRUSS BEARING LOAD
EXCEEDS 425 PSF USE SYP #2
TOP PLATES; IF IT EXCEEDS 565 PSF
ADD ADDITIONAL BEARING BLOCKS OR
USE SIMPSON TBE BEARING ENHANCER

2x4/6 SPF#2 PRECUT STUDS AT 16" O.C.
SEE STUD TABLE

7/16" O.S.B. WALL SHEATHING
FULLY BLOCKED
8d COMMON NAILS
6" O.C. EDGE, 12" O.C. FIELD
UNLESS OTHERWISE NOTED
ON STRUCTURAL PLAN

4" CONCRETE FLOOR SLAB REINFORCED
WITH #6@4'x4' WELDED WIRE MESH
PLACED ON CHAIRS AT 1" DEPTH OR
FIBER MESH CONCRETE, 6-MIL POLY VAPOR
BARRIER WITH 6" EPS SEALED WITH
POLY TAPE OVER TERMITTE-TREATED
AND COMPACTED FILL

2x4/6 P.T. PINE SOLE PLATE ANCHORED WITH
1/2"x10" ANCHOR BOLTS WITH 2X2X140P
STEEL WASHER 48" O.C. & 8" FROM CORNERS

SP#46 @ 48" O.C.

6" MIN

FINISH GRADE

ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WIND LOADS: 110 MPH EXPOSURE B
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADINGS.
EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O.
SEE STRUCTURAL PLAN

(2) 2X10 SYP #2 U.N.O.
SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21
W/ (8) - 16d TO HEADER
AND (8) - 16d TO POST

6X6 SYP #2 POST

SIMPSON ABU POST BASE
W/ (12) - 16d & 5/8" x 10"
ANCHOR BOLT

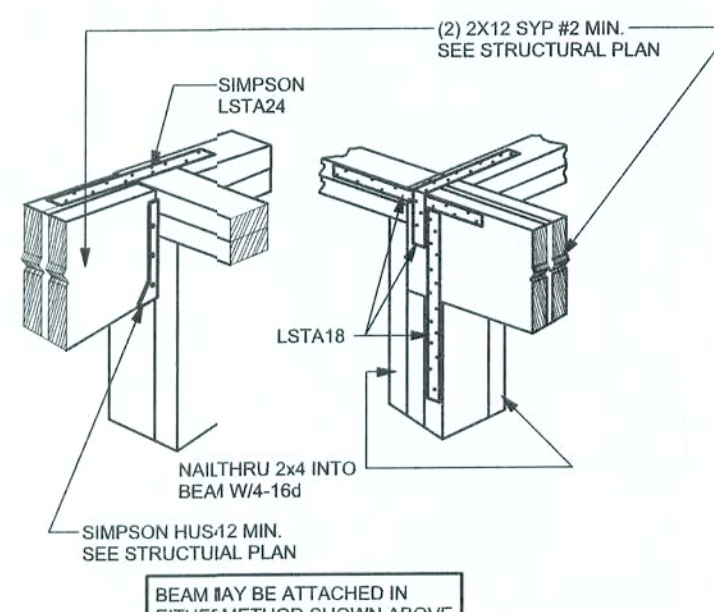
SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.

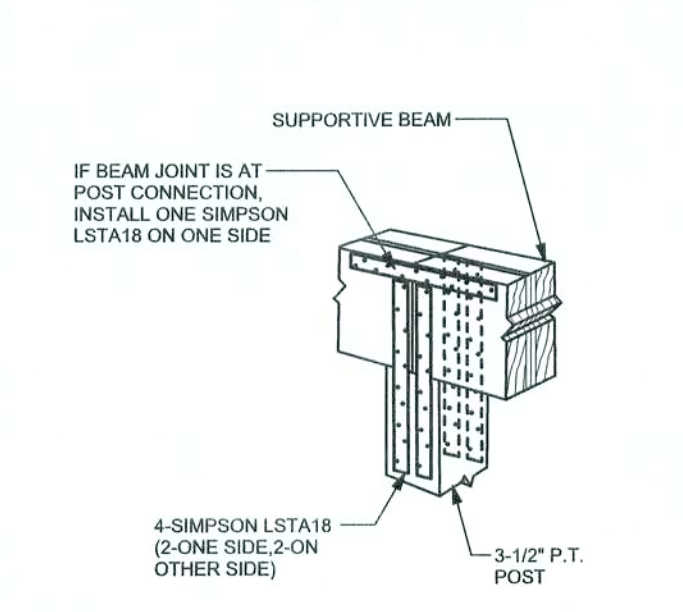


BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

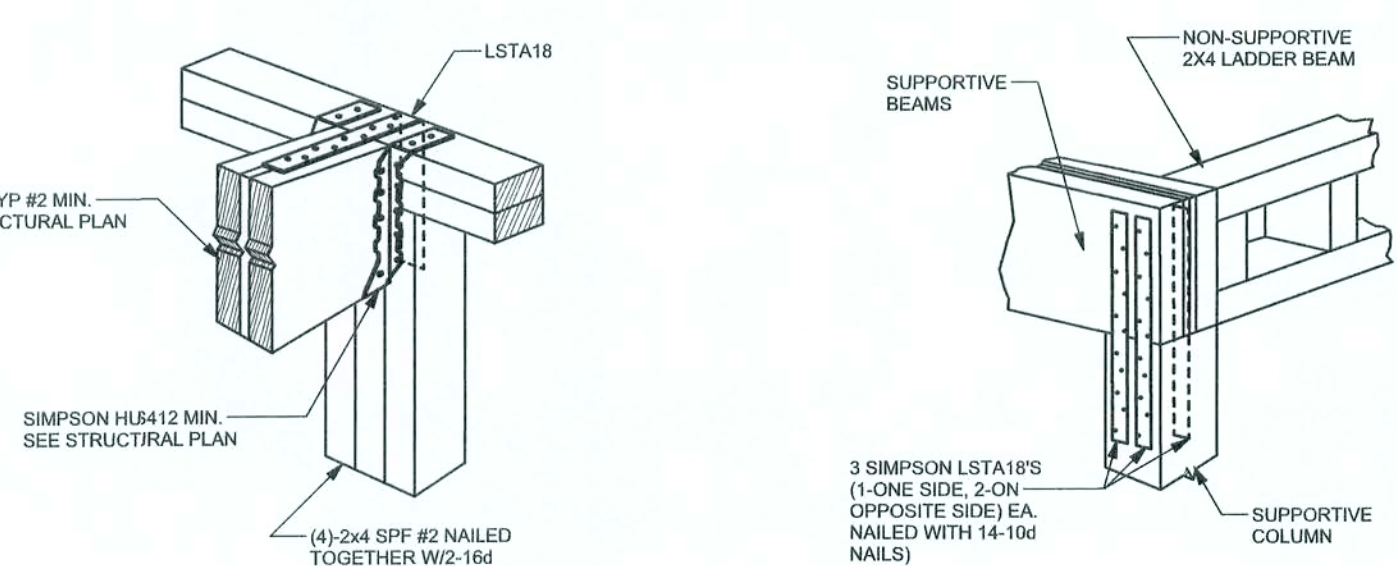


SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

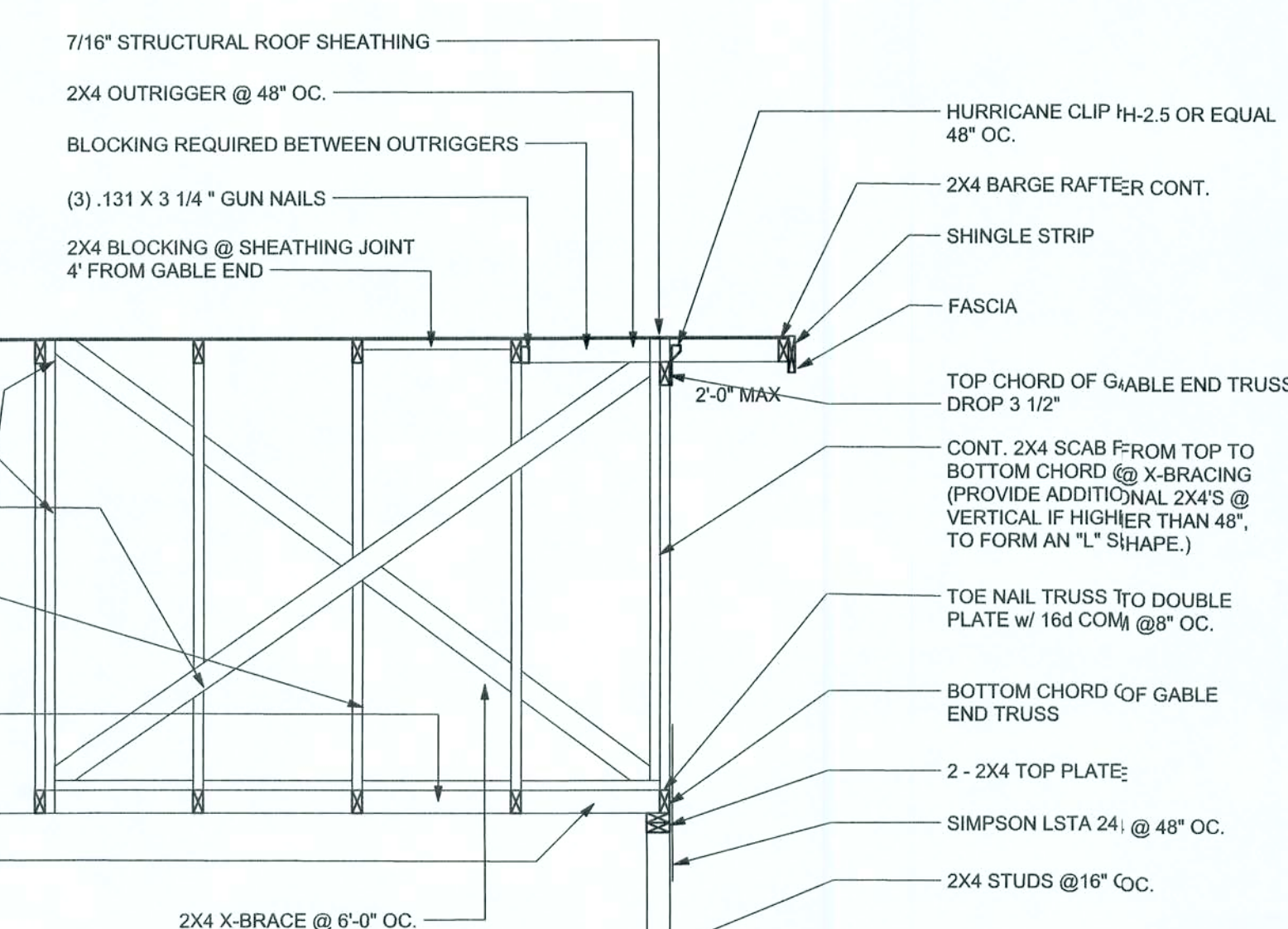
TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



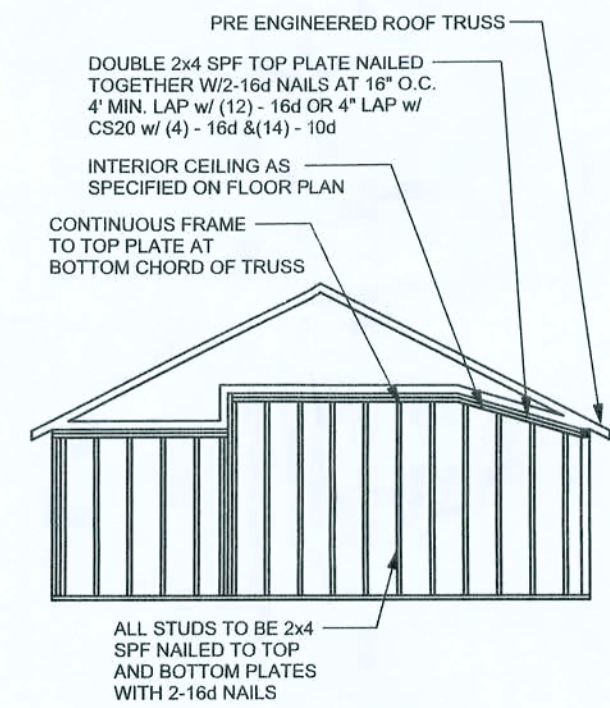
INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



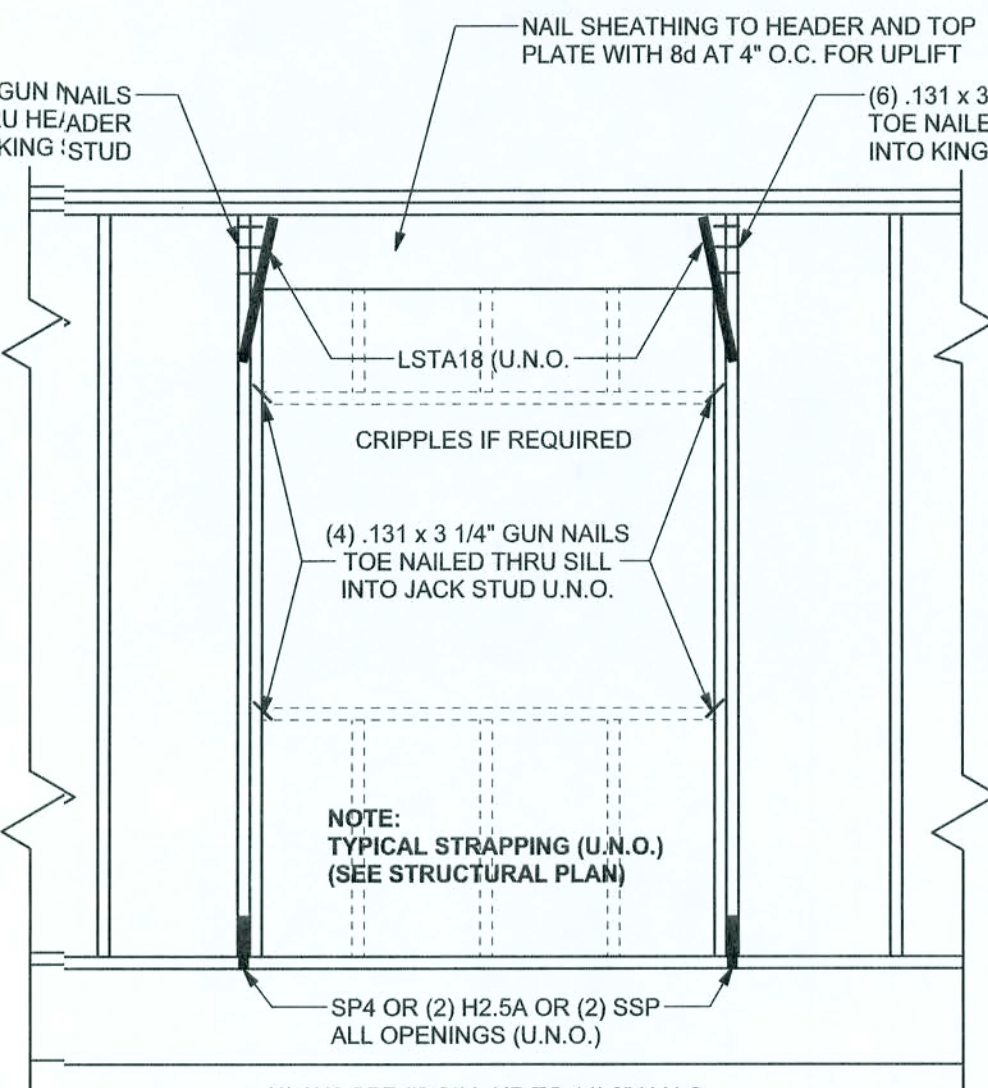
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDERS
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT
CONNECTION 151LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 6" W14 x W1.4 FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC
(W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10FT. DO NOT
CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS);
U.N.O. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,400, E = 1,800KSI, U.N.O. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED BEAM OR 12" IN GROUDED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH
3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", U.N.O.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU
BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL
CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY
STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON
MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF
ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER
IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, U.N.O
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d, 1 1/2"	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d, 1 1/2"	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14 - 16d	14 - 16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SINGLE SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-6/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND 10%
SLOPE AND UNOBSERVED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1) BASIC WIND SPEED = 110 MPH
- 2) WIND EXPOSURE = B
- 3) WIND IMPORTANCE FACTOR = 1.0
- 4) BUILDING CATEGORY = II
- 5) ROOF ANGLE = 10-45 DEGREES
- 6) MEAN ROOF HEIGHT = <30 FT
- 7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)		
	10	100	1000
1	19.9	-21.8	-18.1
2	19.9	-25.5	-21.8
2 Onq		-40.6	-40.6
3	19.9	-25.5	-21.8
3 Onq		-68.3	-42.4
4	21.8	-23.6	-20.4
5	21.8	-29.1	-22.6
Doors & Windows		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
	STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway,
PE No.53915, P.08 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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comply with section R301.2.1, Florida building
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LIMITATION: This design is valid for one
building, at specified location.

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P.E. 53915
11/11/06
SEAL

Stanley Crawford
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Richard & Patricia
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PRINTED DATE:
May 11, 2006

DRAWN BY: David Disosway

CHECKED BY:

FINALS DATE:
11 / May / 06

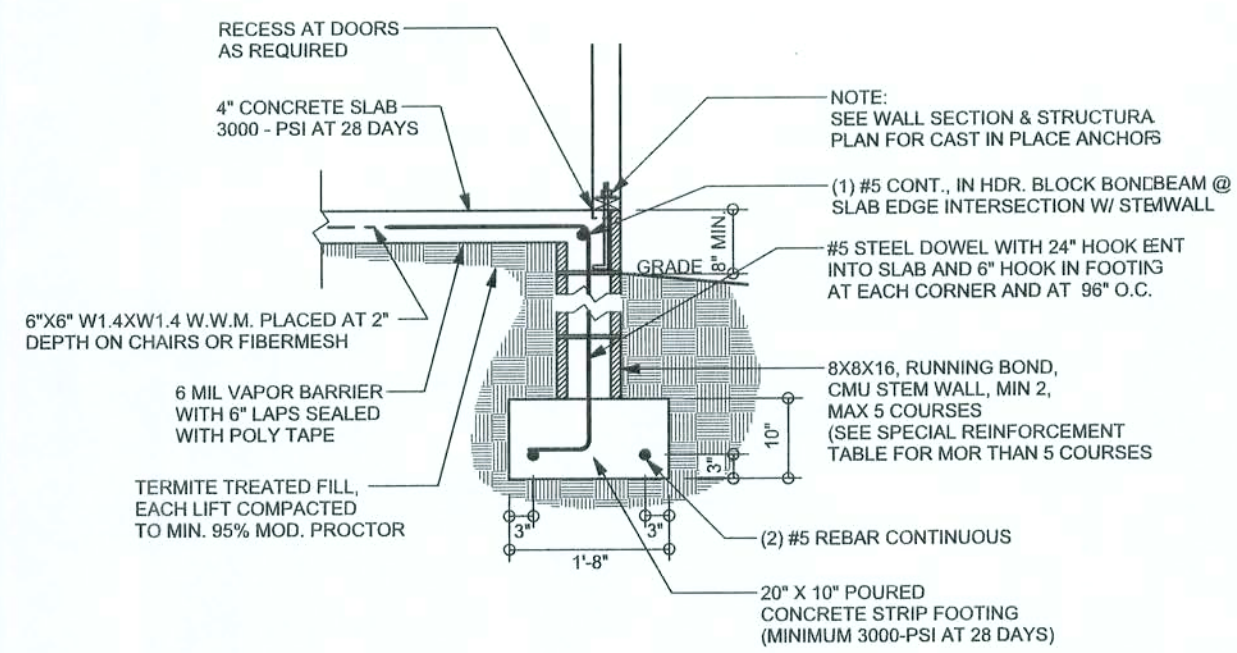
JOB NUMBER:
605084

DRAWING NUMBER

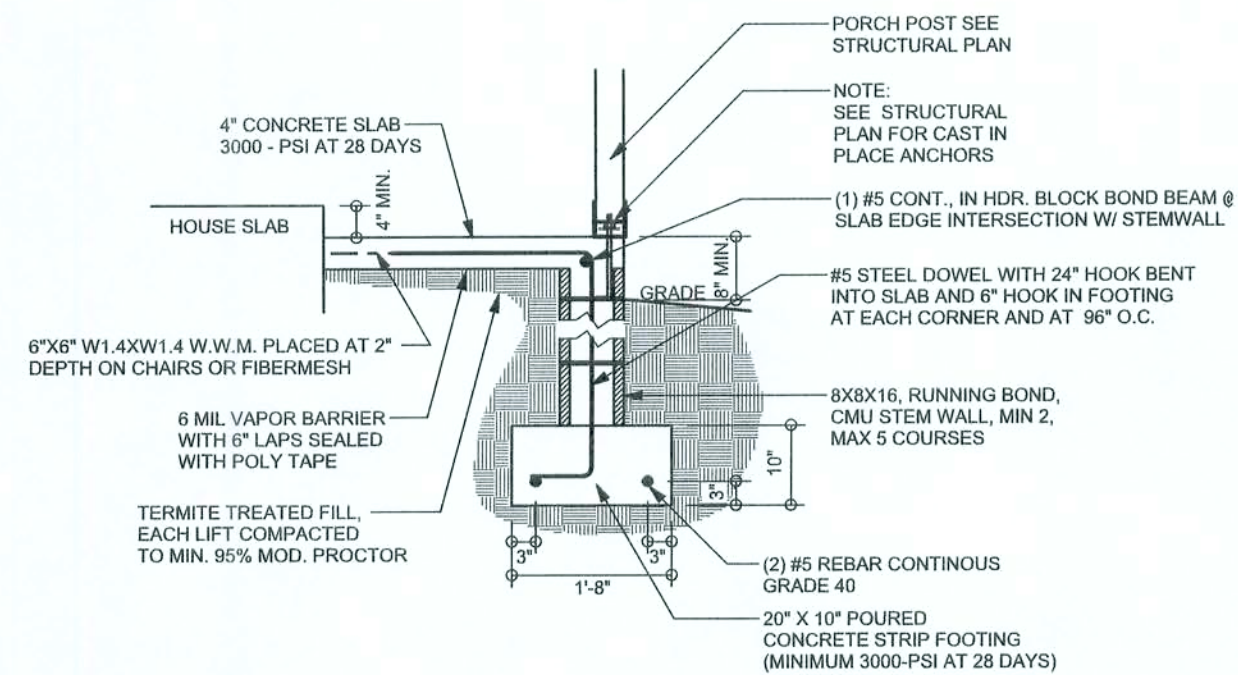
S-1
OF 3 SHEETS

REVISIONS

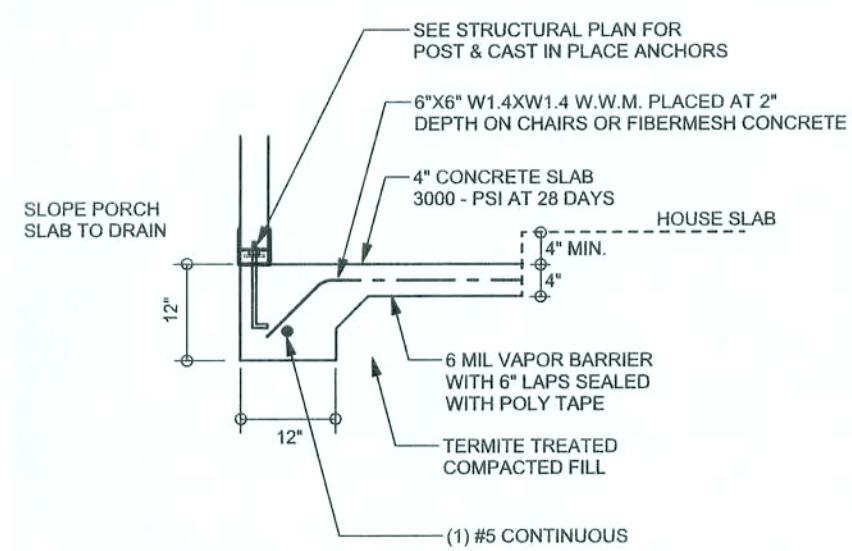
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



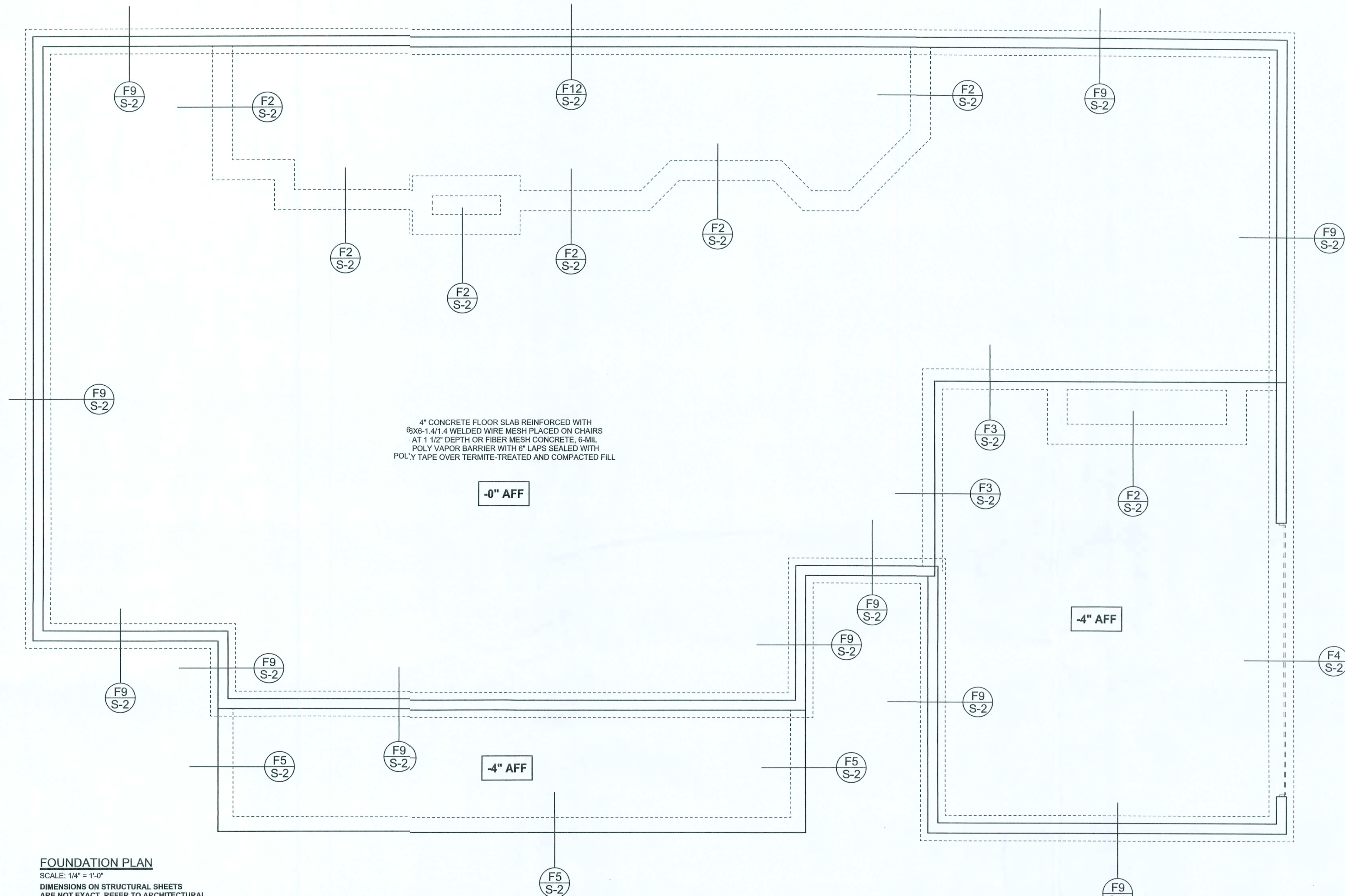
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

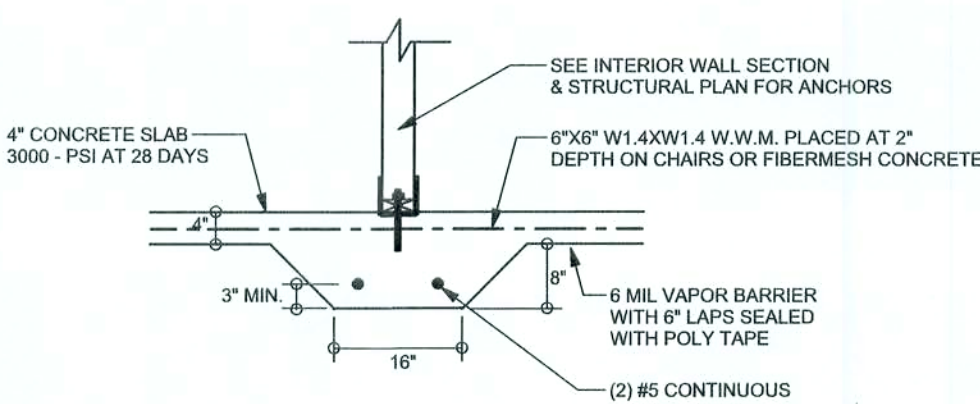
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

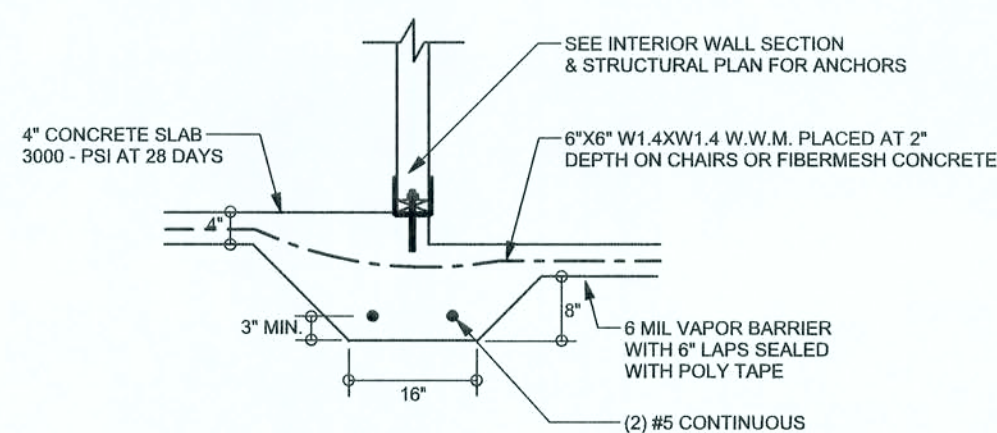
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duralwall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with HES continuous at mid-height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

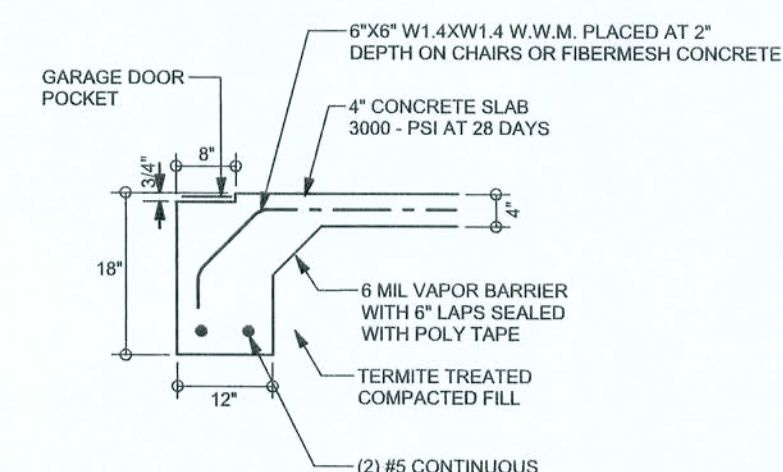
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 6" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53815, P.O. Box 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
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MARK DISOWAY
P.E. 53815

Mark Disoway
11/14/06
SEAL

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Construction**

**Richard & Patricia
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DRAWING NUMBER
S-2

OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

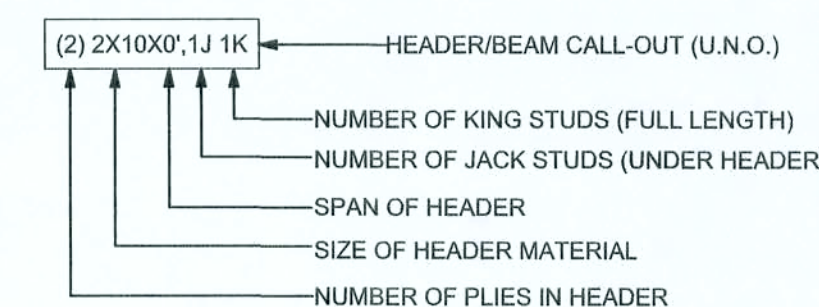
STRUCTURAL PLAN NOTES

- | | |
|------|--|
| SN-1 | ALL LOADING BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.) |
| SN-2 | ALL LOADING BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) |
| SN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS |
| SN-4 | PERMANENT STRUT BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED STRUT DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCISI-03, BCISI-81, BCISI-82, & BCISI-83. BCISI-81, BCISI-82, & BCISI-83 ARE FURNISHED BY THE STRUT SUPPLIER, WITH THE SEALED STRUT PACKAGE |

WALL LEGEND

 SWS = 0.0"	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
 SWS = 0.0"	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
 IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
 IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	38.5'	78.0'
LONGITUDINAL	36.5'	93.5'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSSENGINEERING
FURNISHED BY BUILDER. W.B. HOWLAND TRUSS
JOB #3476

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway,
PE No. 53915, POB 868, Lake City, FL
32056, 386-754-5419

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MARK DISOSWAY

P.E. 53915

Michael J. Brown
TMA406

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S-3
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