

_--

REAR ELEVATION
SCALE: 1/4" = 1'-0"



SOFTPIAN ARCHITECTURAL SIGN SOFTMARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Like City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supdimensions. Refer all qestions to Mark Disosway, P.E. fc resolution. Do not proceed withou/clarification.

COPYRIGHTS AND PIOPERTY RIGHTS:
Mark Disosway, P.E. hreby expressly
reserves its common lav copyrights and
property right in these istruments of service.
This document is not tobe reproduced, altered
or copied in any form a manner without first
the express written pernission and consent
of Mark Disosway.

CERTIFICATION: I herby certify that I have examined this plan, an that the applicable portions of the plan, retting to wind engineering compy with section R301.2.1, florida buildig code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified loation.

Johnson Builders

Spec House

ADDRESS: Lo 23 Russwood Estaes S/D Phase 4 Columbia Canty, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Forida 32056 Phone: (386 754 - 5419 Fax: (386)269 - 4871

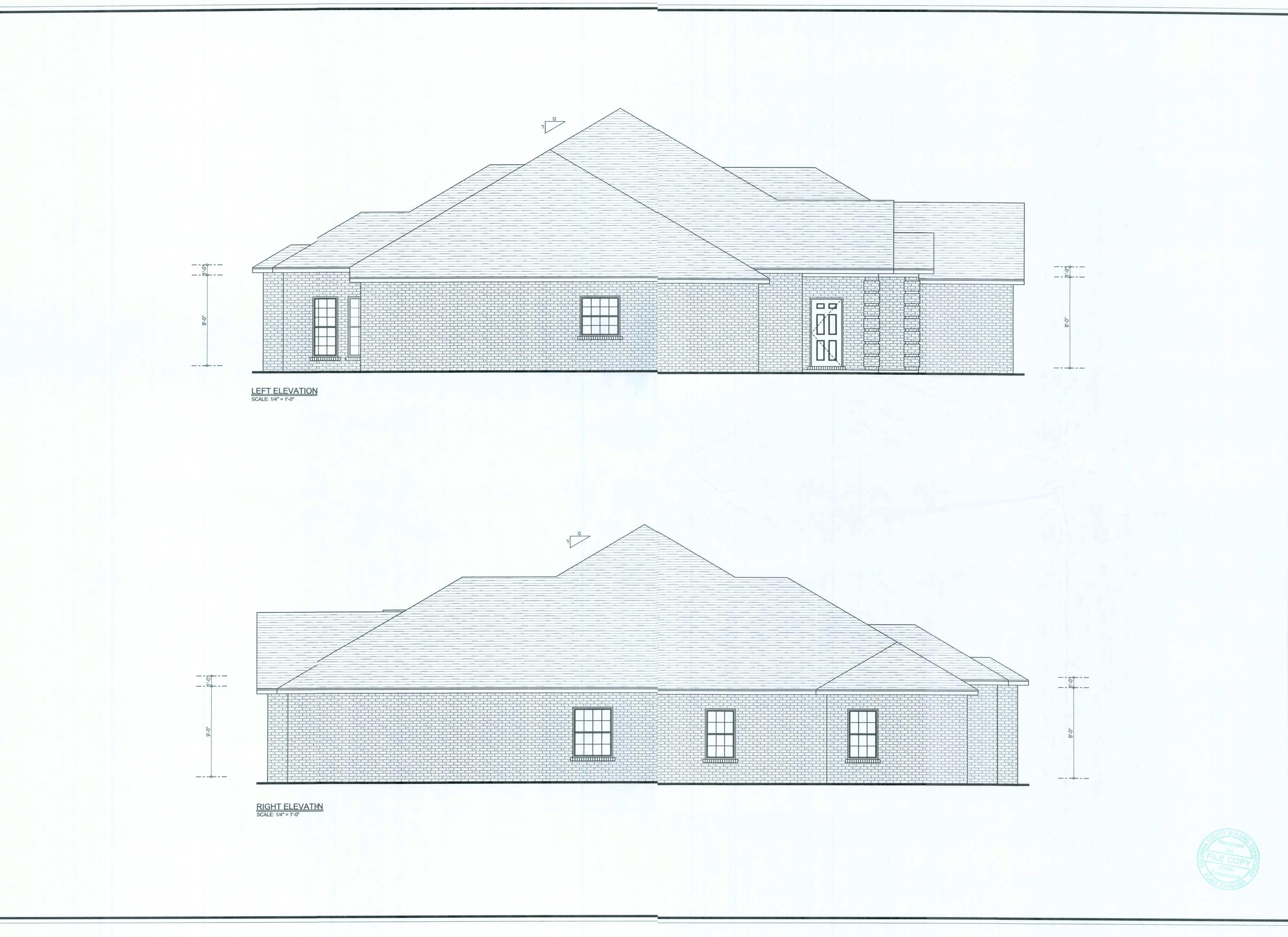
PRINTED DATE: July 31, 208

DRAWN BY: STRUCTURAL BY: David Disosway

FINALS DATE: 31Jul08

JOB NUMBER: 807114 DRAWING NUMBER

OF 7 SHEETS





REVISIONS

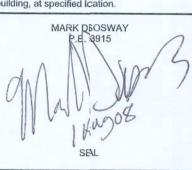
WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 868, lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions sup

dimensions. Refer all destions to Mark Disosway, P.E. fr resolution. Do not proceed withou clarification.

COPYRIGHTS AND POPERTY RIGHTS:
Mark Disosway, P.E. breby expressly
reserves its common lw copyrights and
property right in thesenstruments of service.
This document is not to be reproduced, altered
or copied in any form a manner without first
the express written pemission and consent
of Mark Disosway.

CERTIFICATION: I heeby certify that I have examined this plan, and that the applicable portions of the plan, reating to wind engineering com/y with section R301.2.1, florida buildig code residential 2004, to the best of my knowedge.

LIMITATION: This desgn is valid for one building, at specified lcation.



Johnson Builders

Spec House

ADDRESS: Lc 23 Russwood Estates S/D Phase 4 Columbia Cunty, Florida

Mark Discsway P.E. P.O. Box 868 Lake City, Forida 32056 Phone: (386) 754 - 5419 Fax: (386)269 - 4871

PRINTED DATE: July 31, 2008

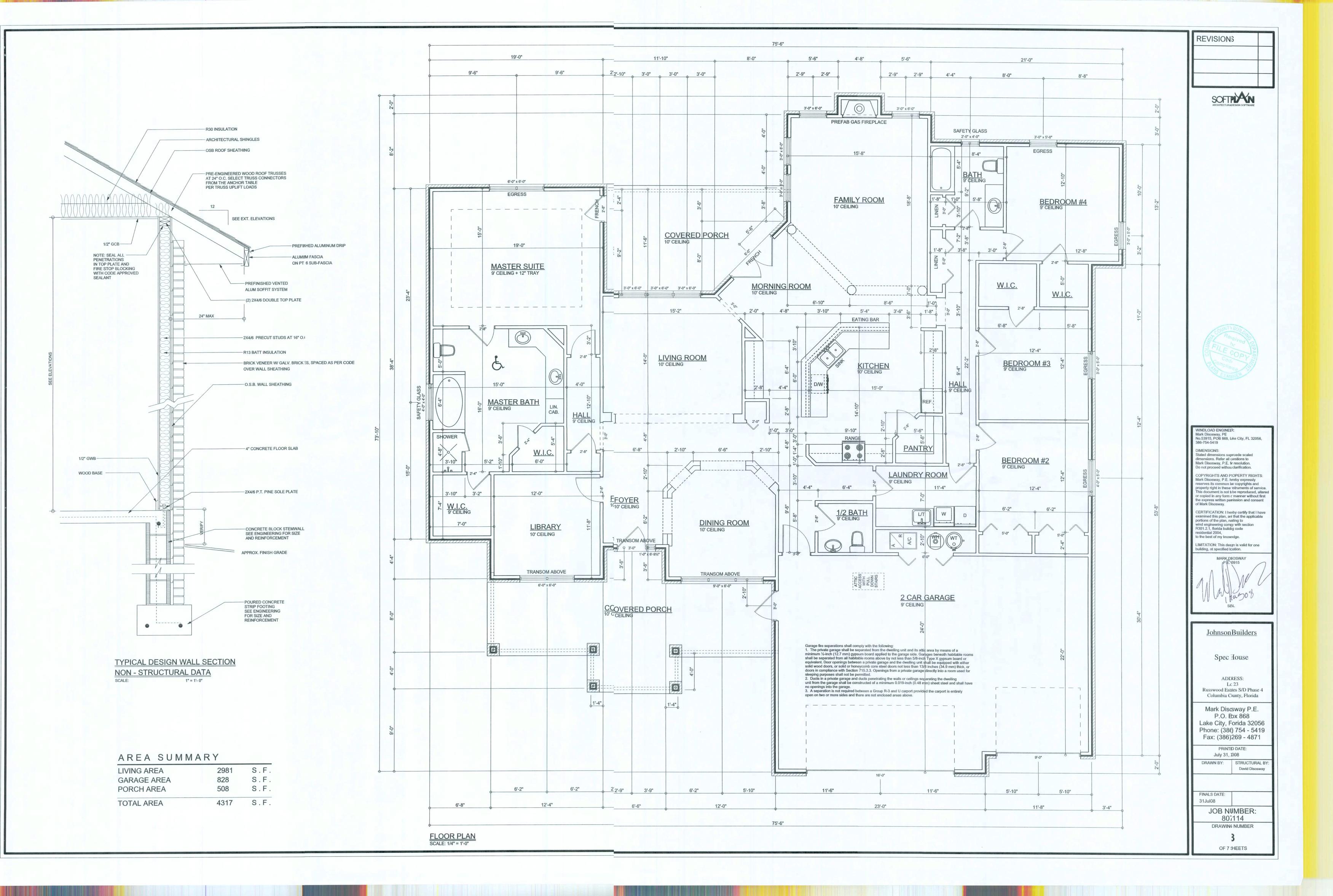
DRAWN BY: STRUCTURAL BY: David Disosway

FINALS DATE: 31Jul08

JOB NUMBER:

OF 7 SHEETS

DRAWING NUMBER



ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5

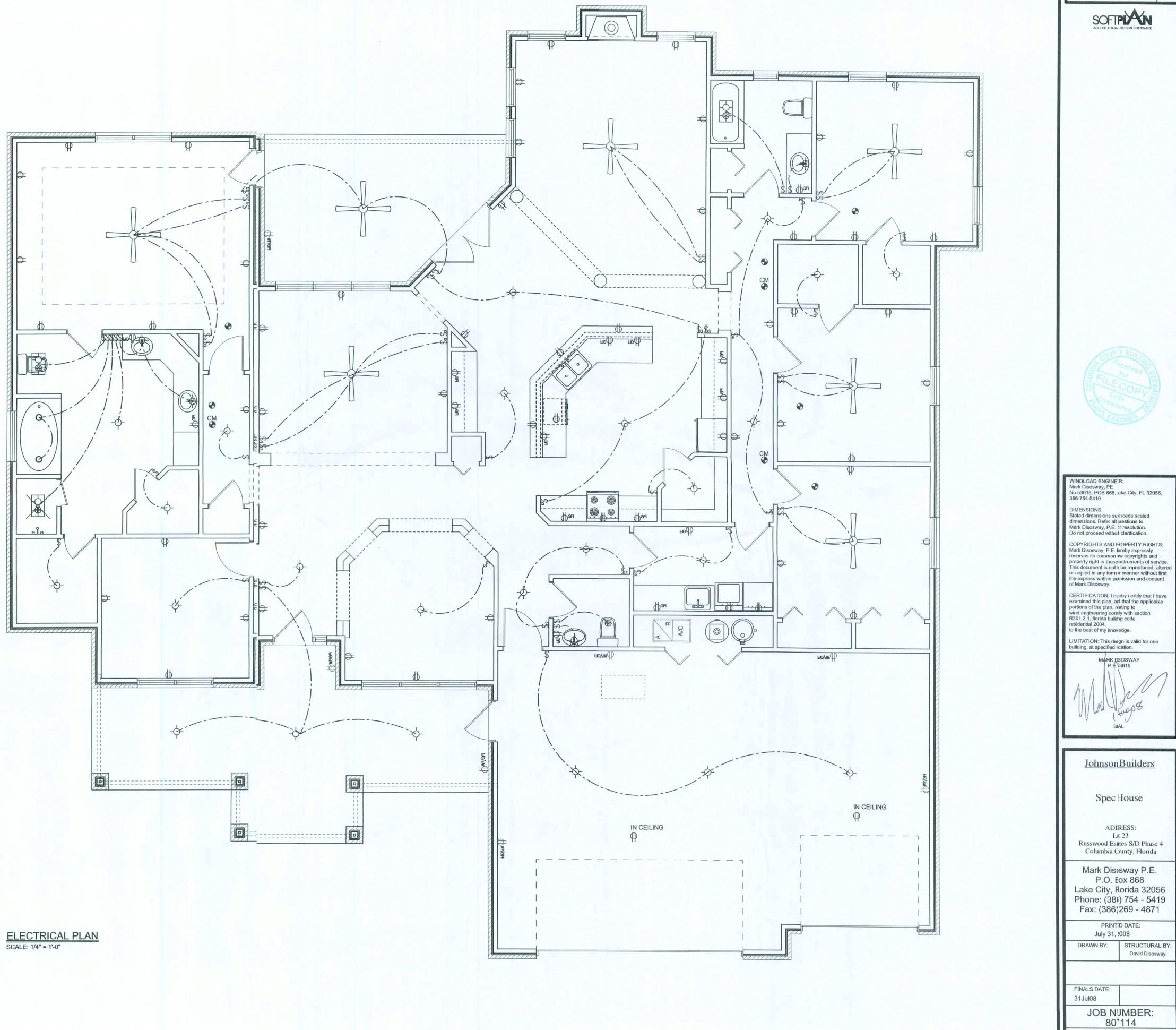
 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
 SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

APPROVAL OF THE BUILDING OFFICIAL

- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC E -10 CONDUCTORS ENTER THE BUILDING.
 SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL
- CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' E -11 OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

	EECTRICAL LEGEND
Π	
	CLING FAN (PE-WIRE FOR LIGHT KIT)
QD	DUBLE SECURITY LIHT
	2) FLUORESCENT LIHT FIXTURE
0	RCESSED CAN LIGHT
- ♦ - *	B/H EXAUST FAN WH LIGHT
₩	B/H EXAUST FAN
- 	LIHT FIXTURE
Ф	DPLEX OUTLET
Ф	22 OUTLET
 Вая	GIDUPLEX OUTLET
•	SIDKE DETECTOR
\$	W.L SWITCH
\$3	3 AY WALL SWITCH
\$ 4	4 AY WALL SWITCH
₩P/GFI	WER PROOF GFI OUTLET
∇	PI)NE JACK
0	TEVISION JACK
P	GAGE DOOR OPENER

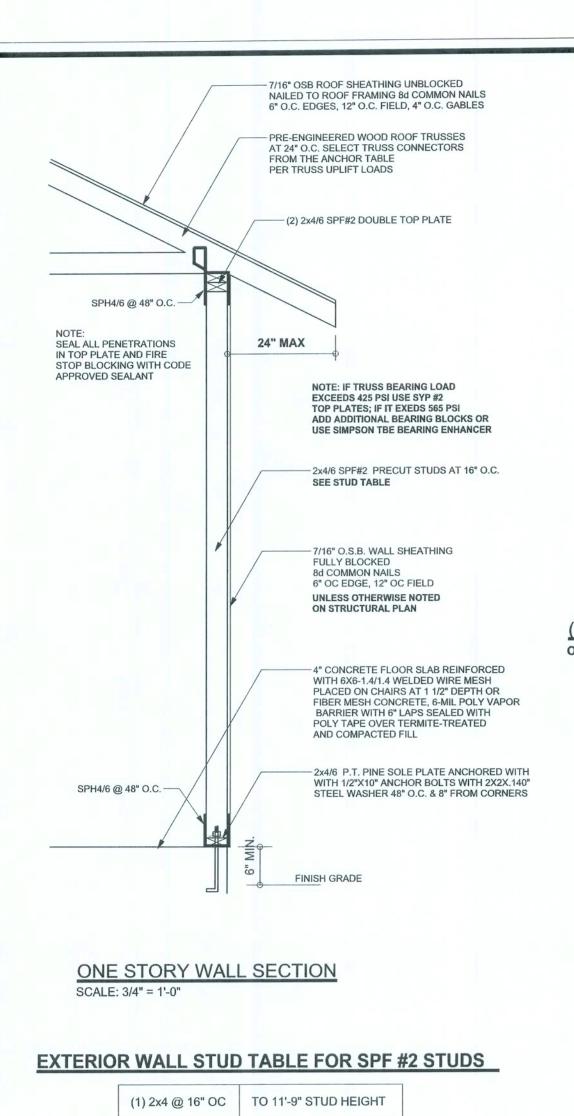
CABON MONOXIDE ALARM



REVISIONS

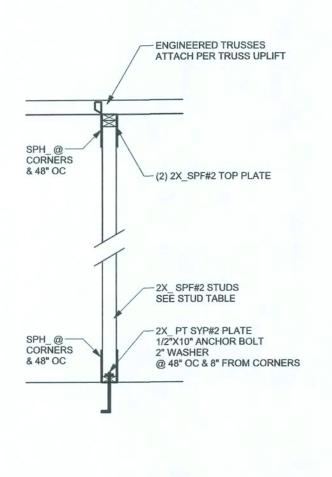
DRAWING NUMBER

OF 7 SHEETS



(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT (1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT (1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001. TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



(TYP.) INTERIOR BERING WALL ONE STORY WOOD FRAME w/ STAPS & ANCHORS

(4) .13:3 1/4" -

INSTALL 2X4 SPF#JIAGONAL BRACE -

7/16" OSB 8d 6" O.C. --

EDGE & 12" O.C. FIELD

ATTACH RAT RUN TO -

(4) .131"X3 1/4" NAILS

w/ (8) -16d TO TRUSS

& (8) -16d TO WALL

@ 48" O.C. U.N.O.

TOE NAIL TRUSS -

BLOCKING w/

12d @ 6" O.C.

AND NAIL TO BLOWNG AT TOP CHORD & BOTTOM CHORD & RAT RUN @ 6' O.C.

-(4) .1'X3 1/4" -

SPACE RAT RUN & DIAONAL BRACE 6'-0" O.C.

FOR GABLE HEIGHT UTO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GALE BRACING DETAIL

-2X4 OUTRIGGER @ 24" O.C.

8d @ 6" OC EDGES, 12" OC FIELD, 4" OC GABLES

BLOCKING REQUIRED BETWEEN OUT RIGGERS

-(4) .131"X3 1/4"

DIAGONAL BRACE MUST

MAY BE "T" BRACED UP

TO 12' AND UNBRACED

2X4X8' RAT RUN NAIL EACH

-(4) .131"X3 1/4" NAILS

-(8) .131"X3 1/4" NAILS

-2X4 SPF#2 BLOCKING

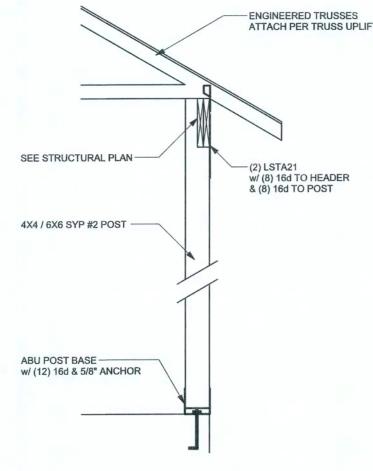
- H3 INSTALLED HORIZONTALLY

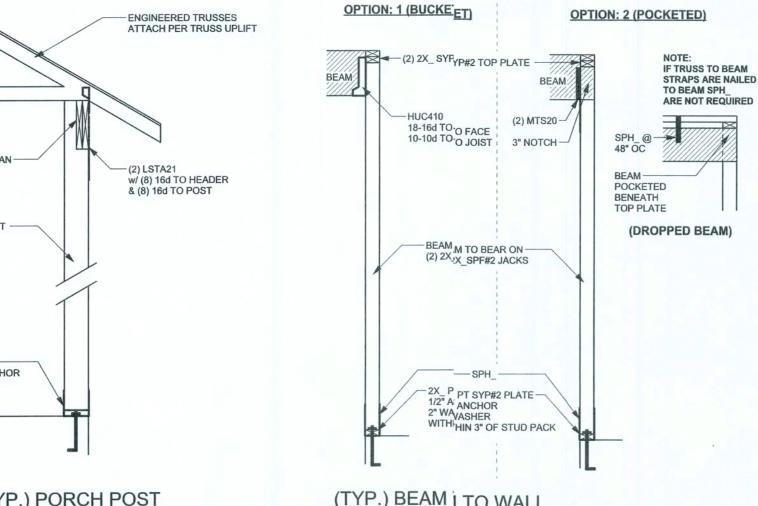
CONNECTION w/ (4) .131"X3 1/4" NAILS

BE NAILEDTO TRUSS WEBS

-(4) .131"X3 1/4" ¬

(4) .131"X3 1/4"



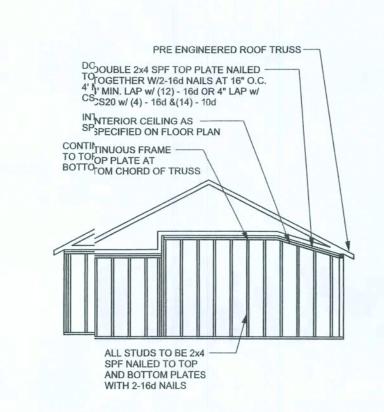


WOOD FRAME w/ STFTRAPS & ANCHORS

GRADE & SPECIES TABLE

ALLOWABLE UPLIFT:

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **<u>CCEILING DIAPHRAGM DETAIL</u>**

IF TRUSS TO WALL STRAP PS ARE NAILED TO THE HEADER THE SPH44/6 @ 48" O.C.

ARE NOT REQUIRED

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS. TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" × 0" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTMIC 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

BUILDER'S RESPONSIBILITY

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ANCHOR TABLE

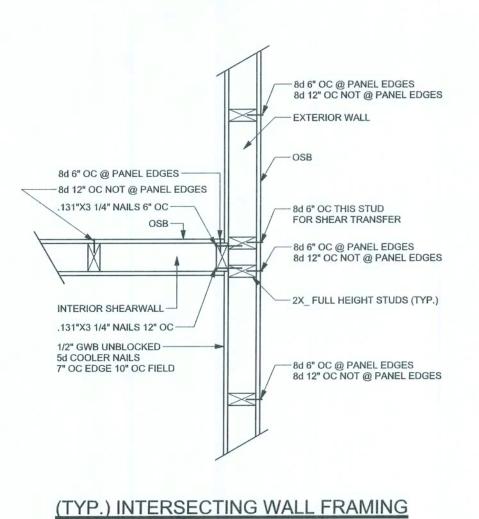
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

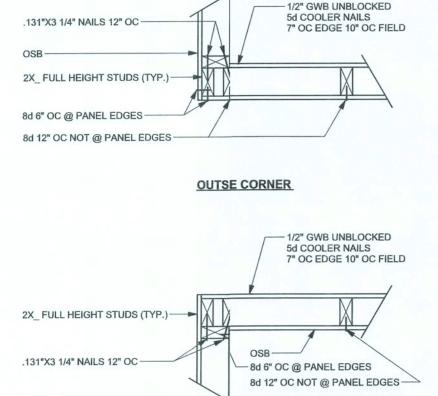
JPLIFT LBS. SYP	FT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*		TO PLATES TO RAFTER/TRUS		S TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A			5/8" AB	
< 4175	< 3695	HTT16 18 - 16d			5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22				
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320		2320 ABU88				

ROOF SYSTEM DESIGN

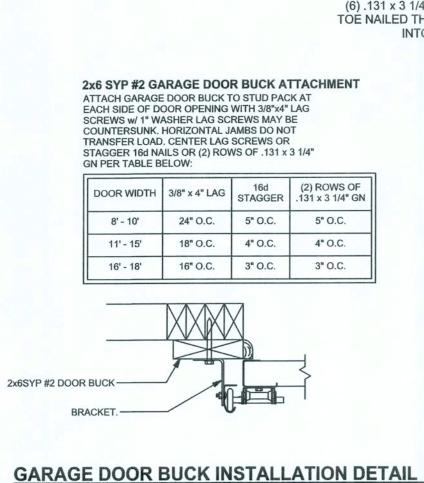
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

(6) .131 x 3 1/4" GUN NAILS-





INSIDCORNER (TYP.) CORNR FRAMING



-(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER - SF3PH4/6 ALL OPENINGS (U.N.O.)-TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD SPH4/6 @ 48" O.C. (U.N.O.) CRIPPLES IF REQUIRED (2 (4) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL -INTO JACK STUD U.N.O. T TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) - SP3PH4/6 ALL OPENINGS (U.N.O.)-(1) 2X6(6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X-X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 1110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL PHEADER STRAPING DETAIL

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	T		***		
	ACI530.1-02 Section	Specific Requirements	3		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi	bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approval	The state of the s		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block	2 4 5 7		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"	2/2		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)	DESIGN LOADS		
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely	FLOOR 40 PSF (ALL OTHER DWELLING RO			
	embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS	30 PSF (SLEEPING ROOMS)			
2.4F Coating for corrosion protection	Coating for corrosion protection	Joint reinforcement in walls exposed to	30 PSF (ATTICS WITH STORAGE)		
		moisture or wire ties, anchors, sheet metal	10 PSF (ATTICS WITHOUT STORAGE		
	ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2	ROOF 20 PSF (FLAT OR <4:12)			
		or 304SS	16 PSF (4:12 TO <12:12)		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.	12 PSF (12:12 AND GREATER)		
2257	Movement joints	Contractor assumes responsibility for type	STAIRS 40 PSF (ONE & TWO FAMILY DWELL		
3.3.E.7		and location of movement joints if not	SOIL BEARING CAPACITY 1000PSF		
		detailed on project drawings.	NOT IN FLOOD ZONE (BUILDER TO VERIFY)		

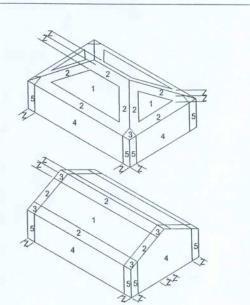
DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS: MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Win		ind Ar	ea (ft2)
				100
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	Doors & Windows Worst Case		21.8	-29.1
(Zone				
8x7 Garage Door 16x7 Garage Door		oor	19.5	-22.9
		18.5	-21.0	

(3)		(Zone 5, 10 ft2)		
5	2 3	8x7 Garage Door	19.5	-22.9
2	4 /3/ 5	16x7 Garage Door	18.5	-21.0
	55 22			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			

REVISONS

SOFTPLAIN



Mark Disoswa, PE No.53915, PC3 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS

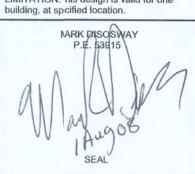
imensions. Fefer all guestions to Mark Disoswa, P.E. for resolution. Do not proceed without clarification. COPYRIGHT: AND PROPERTY RIGHTS: Mark Disoswa, P.E. hereby expressly reserves its common law copyrights and property right a these instruments of service This documen is not to be reproduced, altered or copied in any form or manner without first

Stated dimensons supercede scaled

of Mark Disosray. CERTIFICATION: I hereby certify that I have examined thisolan, and that the applicable portions of theplan, relating to wind engineering comply with section R301.2.1, floria building code to the best of ny knowledge.

the express witten permission and consent

IMITATION: his design is valid for one ouilding, at specified location.



Johison Builders

Spec House ADDRESS:

Lot 23 Russwood Estates S/D Phase 4 Columbia County, Florida

MarkDisosway P.E. PO. Box 868 Lake Cty, Florida 32056 Phone:(386) 754 - 5419 Fax: (386) 269 - 4871

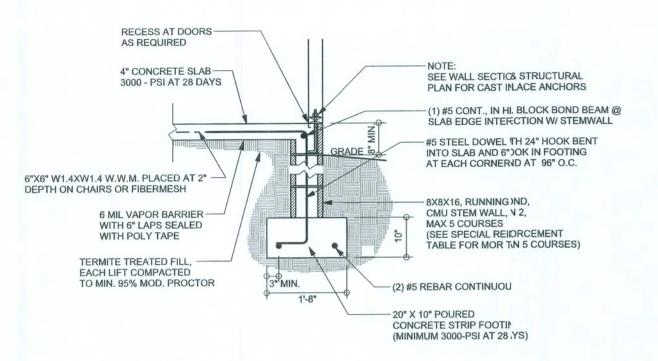
PRINTED DATE: Juy 31, 2008 STRUCTURAL BY David Disosway

FINALS DATE:

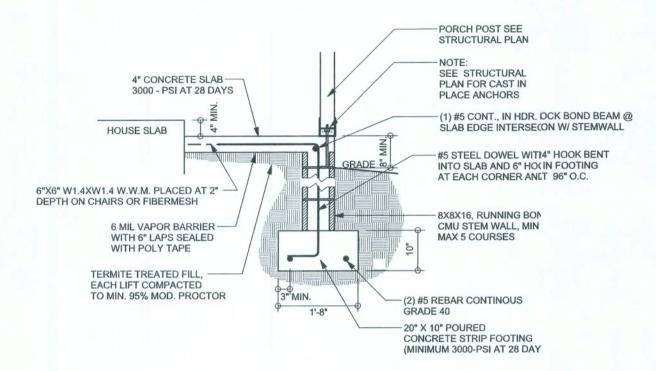
JOB NUMBER: 807114 DRAWING NUMBER

S-1

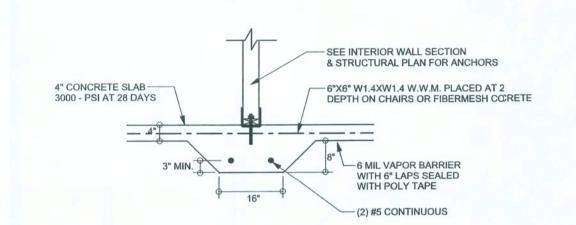
OF 7 SHEETS



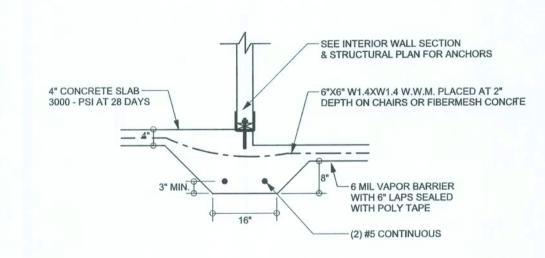
STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



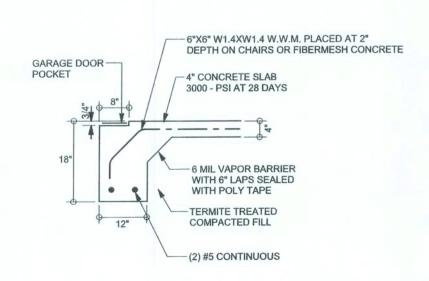
STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



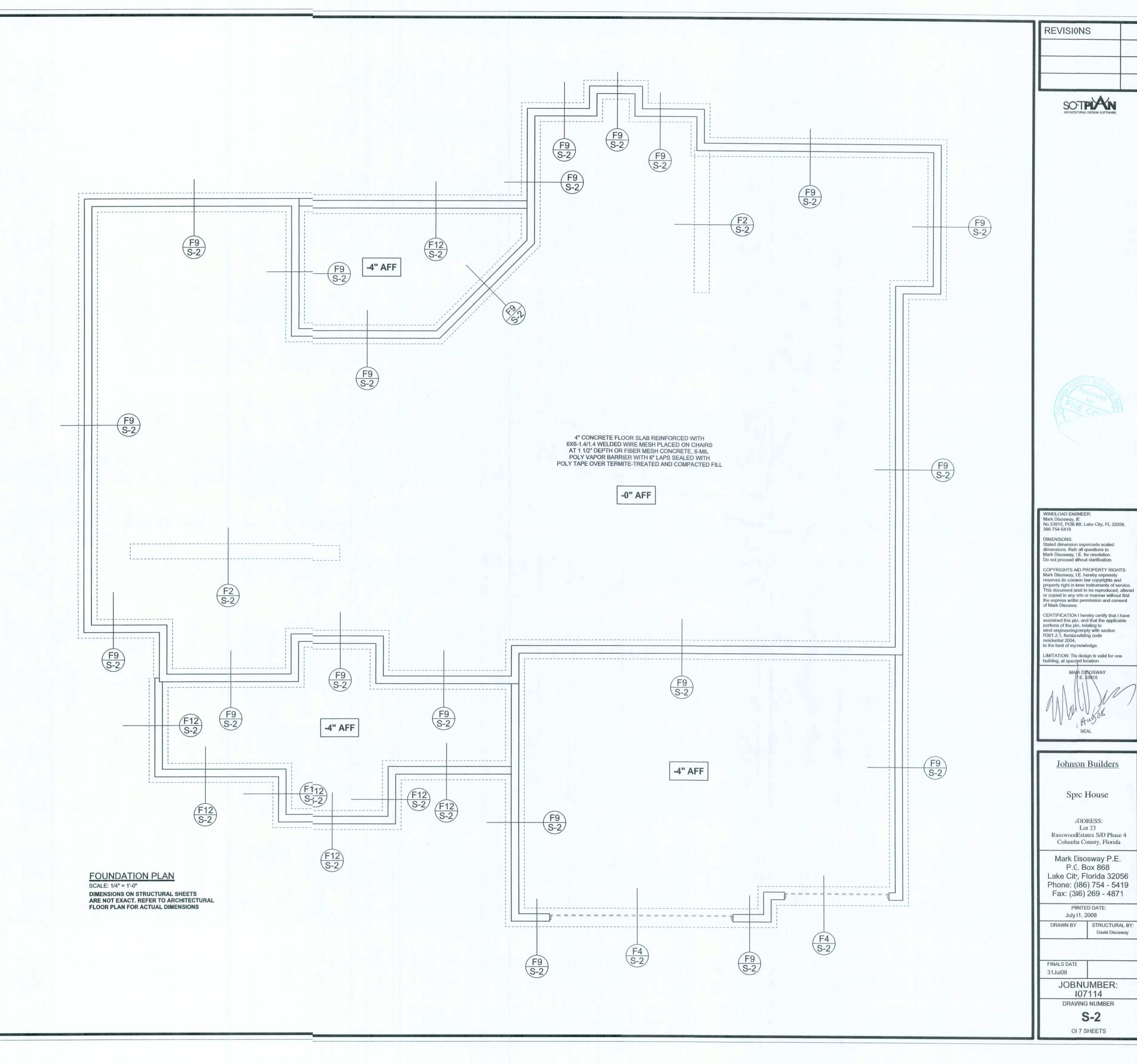
INTERIOR BEARING FOOTING SCALE: 1/2" = 1'-0"



INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



David Disosway

WALL LEGEND

EXTERIOR WALL
INTERIOR NON-LOAD BEARING WALL
INTERIOR LOAD BEARING WALL w/ NO UPLIFT
INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) NUMBER OF KING STUDS (FULL LENGTH) NUMBER OF JACK STUDS (UNDER HEADER) SPAN OF HEADER SIZE OF HEADER MATERIAL NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

REQUIRED ACTUAL

REQUIRED ACTUAL
TRANSVERSE 45.0' 125.8'
LONGITUDINAL 38.2' 56.1'

