STRUCTURAL GENERAL NOTES

A. GENERAL

1. The contractor shall verify all dimensions prior to starting construction. The architect shall be notified of any discrepancies or inconsistencies.
2. Dimensions shall take precedence over scale shown on drawings.
3. Notes and details on drawings shall take precedence over general notes and typical notes.
4. All work shall conform to the minimum standards of the following code. The Florida Building Code, 2017 Edition (2015 IBC), and any other regulating agencies which have authority over any portion of the work, and those codes and standards listed in these notes and specifications.
5. See architectural drawings for the following:
   - Size and location of all door and window openings, except as noted.
   - Size and location of all interior and exterior nonbearing partitions.
   - Size and location of all concrete curbs, floor drains, slopes, depressed areas, changes in level, chamfers, grooves, inserts, etc.
   - Size and location of floor and roof openings except as shown
   - Floor and roof finishes
   - Stair framing and details (except as shown)
6. See mechanical, plumbing, and electrical drawings for the following:
   - Pipe runs, sleeves, hangers, trenches, wall and slab openings, etc. Except as shown or noted.
   - Electrical conduit runs, boxes, outlets in walls and slabs.
   - Concrete inserts for electrical, mechanical or plumbing fixtures.
   - Size and location of machine or equipment bases, anchor bolts for mounts.
7. The contract structural drawings and specifications represent the finished structure. They do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure during construction. Such measure shall include, but not be limited to, bracing, shoring for loads due to construction equipment, etc. Observation visits to the site by the structural engineer shall not include inspection of the above structural members.
8. Openings, pockets, etc. larger than 6 inches shall not be placed in slabs, decks, beams, joists, columns, walls, etc. unless specifically detailed on the structural drawings. Notify the structural engineer when drawings by others show openings, pockets, etc. not shown on the structural drawings, but which are located on structural members.
9. ASTM specifications noted shall be the latest revision.
10. Contractor shall investigate site during clearing and earthwork operations for filled excavations or buried structures such as cesspools, cisterns, foundations, etc. If any such structures are found, the structural engineer shall be notified immediately.
11. Construction materials shall be spread out if placed on floors or roof. Load shall not exceed the design live load per square foot. Provide adequate shoring and/or bracing where structure has not attained design strength.
12. Design Loads:
    - Roof:
      - 18 psf DEAD
      - 20 psf LIVE
    - Floor:
      - 18 psf DEAD
      - 40 psf LIVE (Reducible)
    - Wind:
      - Velocity 121 mph (3 sec. Gust)
      - Exposure "C"
      - Risk Category = II
    - Seismic:
      1. Importance Factor: I = 1
      2. S_S = 0.078 S_1 = 0.047
      3. Site Class: "D"
      4. S_{DS} = 0.083 S_{D1} = 0.075
      5. Seismic Design Category "B"
      6. Seismic Force Resisting System: Timber roof & floor diaphragms with wood shear walls.
      7. Base Shear:
         - V = 0.8 kips
      8. C_S = 0.013
      9. R = 6.5
      10. Analysis Procedure: Equivalent lateral force procedure.
      11. Risk Category: "II"

B. FOUNDATION

1. Footings are designedased on an allowable soil pressure of 1500 PSF. Vector Structural Engieering strongly recommends independent soils testing be performed ta licensed geotechnical engineer to verify soil bearing capacity, slopstability, and any other related soil parameters, as required.
2. Contractor shall provic for proper de-watering of excavations from surface water, groundater, seepage, etc.
3. Footings shall be placi according to depths shown on the drawings.
4. Footing back fill and uty trench back fill within building area shall be mechanically compactl in layers. Flooding will not be permitted.
5. All abandoned footing utilities, etc. that interfere with new construction shall be removed.
6. The soil under perimer beams and slabs shall be above optimum moisture prior to concte placement.
7. Holdown anchor bolts all meet the requirements of detail 11/SD-1.
8. All Ø1/2" anchor boltsay be replaced with ICC approved Ø1/2" Titen HD screws or Ø1/2" all thad rod in Ø5/8" hole with 4" embed using Simpson SET-XP epoxy at the acing indicated below.

WALL TYF	RETROFIT Ø1/2" TITEN OR ALL-THREAD ROD SPACING
S1, S2, NON-HEAR	SAME AS Ø1/2" A.B.
S3 & S	12" O.C.

C. CONCRETE

1. All phases of work perining to the concrete construction shall conform to the "Building Code Reiirements for Reinforced Concrete" (ACI 318 latest approved edition) withodifications as noted in the drawings and specifications.
2. Reinforced concrete dign is by the "Ultimate Strength Design Method", ACI 318-(latest edition
3. Schedule of structural ncrete 28-day strengths and types:

Location in sucture	Strength PSI	Type
Slabs on Gde	3000	Hard rock
Footings	3000	Hard rock

   Design based 2500 PSI, 28-day strength, special inspection is not required uess noted otherwise in note M/S1.
4. Concrete mix design sall be submitted to the engineer for approval with the following requiremits:
   a. Compressivetrength at age 28 days as specified above.
   b. Large aggreate-hardrock, ¾" maximum size conforming to ASTM C-33
   c. Cement-AST C-150, Type I or II Portland cement
   d. Maximum slnp 5-inches, max water cement ratio: 0.5
   e. No admixtun, except for entrained air, and as approved by the engineer.
5. Concrete mixing operans, etc. shall conform to ASTM C-94
6. Placement of concretehall conform to ACI standard 514 and project specifications.
7. Clear coverage of conete over outer reinforcing bars shall be as follows: Concrete poured direc against earth - 3 inches clear, structural slabs - 3/4 inches clear (top al bottom), formed concrete with earth back fill - 2 inches clear.
8. All reinforcing bars, anor bolts and other concrete inserts shall be well secured in position prito placing concrete.
9. Provide sleeves for plubing and electrical openings in concrete before placing. Do not cut anyeinforcing that may conflict. Coring in concrete is not permitted except ashown. Notify the structural engineer in advance of conditions not showon the drawings.
10. Conduit or pipe size (O.) shall not exceed 30% of slab thickness and shall be placed betweethe top and bottom reinforcing, unless specifically detailed otherwise. Coentrations of conduits or pipes shall be avoided except where detailed enings are provided.
11. Modulus of elasticity ofoncrete, when tested in accordance with ASTM C-460, shall be at leashe value given by the equations in section 8.5.1 of ACI 318 for the specifie 28-day strength.
12. Shrinkage of concrete, hen tested in accordance with ASTM C-157, shall not exceed 0.0004 incs/inch.

D. REINFORCING STEEL

1. Reinforcing bars shall conform to the requirements of ASTM A-615 grade 60.
2. All reinforcing bar bends shall be made cold
3. Minimum lap of welded wire fabric shall be 6 inches or one full mesh and one half, which ever is greater.
4. All bars shall be marked so their identification can be made when the final in-place inspection is made.
5. Rebar splices are to be: Class "B"
6. Reinforcing splices shall be made only where indicated on the drawings.
7. Dowels between footings and walls or columns shall be the same grade, size and spacing or number as the vertical reinforcing, respectively.

E. WOOD

1. Framing Lumber
   a. Douglas fir larch No. 2 grade for 2x and 4x framing except for 2x4-4, 2x6 studs use Douglas fir stud grade, U.N.O.
   b. 6x framing DFL No. 1 grade
2. Bolt holes shall be 1/16" maximum larger than the bolt size. Re-tighten all nuts prior to closing in.
3. Standard cut washers shall be used under all sill plate anchor bolts, U.N.O. at shear walls. See the Shear Wall Schedule on sheet S1.1 for anchor bolt spacing and washer requirements at shear walls.
4. All sills or plates resting on concrete or masonry shall be pressure treated Douglas Fir. Bolts shall be placed 9 inches from the end of a plate, or from a notch greater than ½ the width of the plate, and spaced at intervals noted.
5. Do not notch joists, rafters or beams except where shown in details. Obtain engineer's approval for any holes or notches not detailed. Holes through sills, plates, studs and double plates in interior, bearing and shear walls shall conform with detail 5/S1.3.
6. Connection hardware shall be by USP or Simpson Strong-Tie, or ICC approved equal.

DUAL SPECIFICATION TABLE

SIMPSON CONNECTOR	USP CONNECTOR	SIMPSON CONNECTOR	USP CONNECTOR
CS16	RS150	HDU2	PHD2A
ST6224	KST224	HDU4	PHD4A
A35	MPA1	HDU5	PHD5A
LUS24-2	JUS24-2	HDU8	PHD8
H1	RT15	HDU11	UPHD11
H10	RT16A		
LTP4	MP4F	STHD10	STAD10
LSSU	LSSH	STHD14	STAD14

7. Fastening schedule per Florida Building Code, 2017 Edition (2015 IBC), table No. 2304.10.1. Unless noted otherwise.
8. All nails, bolts, holdowns, straps or other steel fasteners in contact with pressure treated timber shall be hot-dipped galvanized, stainless steel or otherwise treated or isolated to prevent chemical attack. Contractor shall verify treatment method and confirm appropriate corrosion resistance be provided in accordance with hardware supplier recommendations.
9. Non-bearing, non-shear interior walls to be anchored to floor and /or roof as indicated on detail 4/S1.3.

F. PREFABRICATED WOOD TRUSSES

1. Prefabricated wood roof trusses shall be as designed by the truss manufacturer. Bridging size and spacing by truss manufacturer unless noted otherwise. Contractor shall submit shop drawings, erection drawings and design calculations sealed by an engineer, registered in the state of Florida, for review prior to manufacture. Calculations and shop drawings shall show any special details required at bearing points. All connectors shall be Simpson or equivalent with current ICC approval.
2. Truss manufacturer to design trusses for lateral load (LAT. = xxxx) in pounds, as shown on plans. Lateral loads are ASD level loads.
3. Additional trusses shall be supplied as required to support mechanical equipment.
4. All truss-to-truss and truss-to-beam connectors per truss manufacturer.

SHEET INDEX

ABBREVIATIONS

A.B.	ANCHOR BOLT	LVL	LAMINATED VENEER LUMBER
ARCH'L	ARCHITECTURAL DRAWINGS	MFR	MANUFACTURED
BLDG	BUILDING	N.T.S.	NOT TO SCALE
BLK	BLOCK	o/	OVER
BLK'G	BLOCKING	O.C.	ON CENTER
BM	BEAM	OPT'L	OPTIONAL
CANT'L	CANTILEVERED	O.S.B.	ORIENTED STRAND BOARD
C.L.	CENTER LINE	PSL	PARALLEL STRAND LUMBER
CLG	CEILING	PL	PLATE
CMU	CONCRETE MASONRY UNIT	REQ'D	REQUIRED
COL	COLUMN	SHTH'G	SHEATHING
CONT	CONTINUOUS	SHT	SHEET
DBL	DOUBLE	SIM	SIMILAR
DTL	DETAIL	STL	STEEL
EL	ELEVATION	SW	STRONG-WALL
EOR	ENGINEER OF RECORD	T.O.F.	TOP OF FOOTING
FND	FOUNDATION	T.O.W.	TOP OF WALL
FTG	FOOTING	T&B	TOP AND BOTTOM
GL	GLUE LAMINATED (BEAM)	TYP.	TYPICAL
HDR	HEADER	U.N.O.	UNLESS NOTED OTHERWISE
HORIZ.	HORIZONTAL	VERT.	VERTICAL
H.D.	HOLD DOWN	w/	WITH
LSL	LAMINATED STRAND LUMBER	u/	UNDER

RELEASE DATE: SEPTEMBER 17, 2020

DATE: 09-17-2020 | ENG: JAB | DWN: MGP | CHK: JCS

REV. #	DATE	BY:	DESCRIPTION

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NATALIA SCOTT
SCOTT RESIDENCE
WHITE, FL
STRUCTURAL GENERAL NOTES

JACOB S. PROCTOR LICENSE No. 74277 STATE OF FLORIDA PROFESSIONAL ENGINEER
09/21/2020
JACOB S. PROCTOR, P.E. 74277

U3942-001-201

S1

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STRUCTURAL GENERAL NOTES

G. GLUE LAMINATED BEAMS (GLB)

1. Glue laminated beams shall be 24F-V4 (cantilevers and continuous beams shall be 24F-V8) and have the following minimum properties: fb=2400 psi, Fv=190 psi, Fc (perpendicular)=650 psi, E=1,800,000 psi. All beams shall be fabricated using waterproof glue. Fabrication and handling per latest AITC and WCCA standards. Beams to bear grade stamp and AITC stamp and certificate. Moisture content shall be limited to 12% or less.

H. LAMINATED VENEER LUMBER (LVL)

1. Laminated veneer lumber to have: Fb=2600 psi, Fv=285 psi, E=1.9x10^6psi
2. Double & triple LVL beams shall be nailed together as follows:
   Provide (2) rows of 16d sinkers at 12" O.C. for beams < 11 7/8" deep
   Provide (3) rows of 16d sinkers at 12" O.C. for beams > 11 7/8" deep
3. Beams w/ (4) or more plies shall be bolted together as indicated in the manufacturer's written specifications.

I. WOOD STRUCTURAL PANELS

1. All wood structural panels shall be plywood or APA rated oriented strand board. Panels shall bear the stamp of an approved agency. Panels shall be of the span/index rating shown on the plans. Fastening shall be indicated on the plans.
2. All plywood shall be C-D interior sheathing with exterior glue. Plywood shall be 4-ply, minimum.

J. SHOP DRAWINGS

1. Shop drawings shall be submitted for all structural items in addition to items required by architectural specifications.
2. The contractor shall review all shop drawings prior to submittal. Items not in accordance with contract drawings shall be flagged for review.
3. Verify all dimensions with architect.
4. Any changes, substitutions, or deviations from original contract drawings shall be redlined or flagged by submitting parties, shall be considered approved after engineers review, unless noted otherwise.
5. The engineer has the right to approve or disapprove any changes to the original drawings at anytime before or after shop drawings review.
6. The shop drawings do not replace the original contract drawings. Items omitted or shown incorrectly and are not flagged by the structural engineer or architect are not to be considered changes to the original contract drawings.
7. The adequacy of engineering designs and layout performed by the others rests with the designing or submitting authority.
8. Reviewing is intended only as an aid to the contractor in obtaining correct shop drawings. Responsibility for corrections shall rest with the contractor.

K. SHEATHING

1. Roof sheathing
   15/32" wood structural panel: plywood or oriented strand board (O.S.B.) panel index = 32/16, unblocked, nail with 8d common nails at 6" O.C. at all boundaries and supported edges, 12" O.C. field. Minimum penetration 1" in supporting member (NER 272).
2. Floor sheathing
   3/4 " (min.) wood structural panel: plywood or oriented strand board (O.S.B.) T & G, panel index = 48/24, unblocked, nail with 10d common nails at 6" O.C. at all boundaries and supported edges, 12" O.C. field.
3. Shear wall sheathing
   Sheathing for shear walls shall be as indicated on the shear wall plans and schedules. Sheathing at shear walls may be installed with panels horizontal or vertical. All shear wall panels shall have minimum wood structural panel span rating of 24/0 or "Wall-16."

L. STRUCTURAL STEEL

1. Hot-rolled structural steel shapes & plates shall be per ASTM A36 with the following exception. W-Flange shapes shall be per ASTM A992.
2. Structural steel pipe shall be per ASTM A53 grade B, Tube steel per ASTM A500 Grade B.
3. Nuts & bolts in structural steel connections shall be per ASTM 325N, with hardened washers. Design is based upon bearing type connections with thread not excluded, therefore, no special inspection required, U.N.O. in note M below.
4. Anchor bolts shall be per ASTM F1554, U.N.O.
5. Welds shall be by E70X, low hydrogen electrodes, all welding shall be performed in a shop approved by the building official.
6. Grout material for baseplates shall be non-metallic, non-shrink, pre-packaged grout conforming to ASTM C 1107.

E. SPECIAL INSPECTION / QUALITY ASSURANCE PLAN

1. The seismic lateral loadresisting system consists of timber roof & floor diaphragms with woodshear walls
2. Special inspections shall be required:
   - All post-installed anchorage to concrete (epoxy grout applications)
   - When required by the local building department: All timber elements of the lateral force resisting system components
   a. The owners shallemploy special inspectors who shall provide additional inspections during construction in accordance with IBC section 17.
   b. All special inspections shall be performed by an independent certified inspectorfrom an established testing agency, licensed and approved by the building department.
   c. The testing agency shall send copies of all structural testing and inspection reportdirectly to Vector Structural Engineering and all interested parties.
3. Structural testing is notrequired.
4. All reports shall be distributed on a monthly basis to the engineer of record, owner, contractor, and the building official.
5. No structural observation is required. However, the engineer of record reserves the right to make field observations during construction approximately once perweek.

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74277

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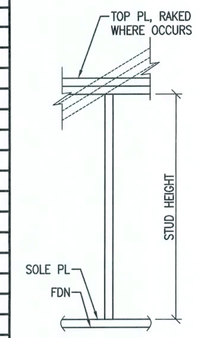
S1.1

STUD HEIGHT TABLE

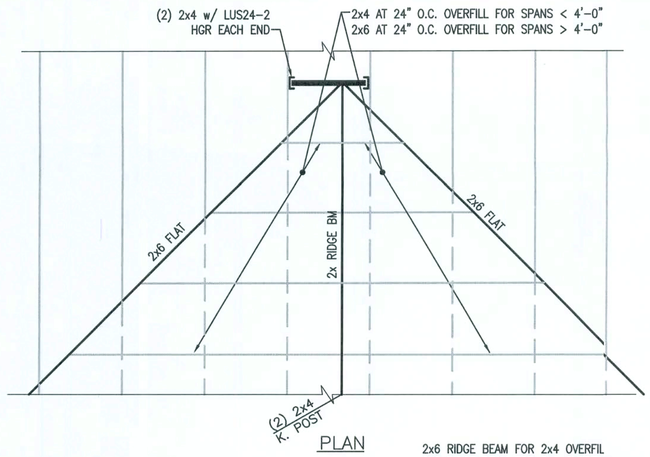
STUD WALL TYPE	BEARING AND/OR SHEAR WALLS (MAX. HEIGHT)		NON-BEARING AND NON-SHEAR WALL (MAX. HEIGHT)
	EXTERIOR	INTERIOR	INTERIOR ONLY
2x4 STUD @ 16" O.C.	8'-6"	10'-0"	13'-0"
2x4 STUD @ 12" O.C.	9'-6"	11'-6"	14'-0"
(2) 2x4 STUD @ 16" O.C.	12'-0"	13'-6"	14'-0"
2x4 DFL #2 @ 16" O.C.	9'-0"	11'-0"	13'-0"
2x4 DFL #2 @ 12" O.C.	10'-6"	13'-0"	14'-0"
(2) 2x4 DFL #2 @ 16" O.C.	13'-0"	13'-6"	14'-0"
2x6 STUD @ 16" O.C.	14'-6"	19'-0"	20'-0"
2x6 STUD @ 12" O.C.	17'-0"	21'-0"	22'-0"
(2) 2x6 STUD @ 16" O.C.	21'-0"	22'-0"	22'-6"
2x6 DFL #2 @ 16" O.C.	16'-6"	19'-6"	20'-0"
2x6 DFL #2 @ 12" O.C.	18'-6"	21'-6"	22'-0"
(2) 2x6 DFL #2 @ 16" O.C.	22'-6"	22'-6"	22'-6"
2x8 DFL #2 @ 16" O.C.	22'-0"	26'-6"	27'-0"
2x8 DFL #2 @ 12" O.C.	25'-6"	28'-0"	30'-0"
(2) 2x8 DFL #2 @ 16" O.C.	29'-6"	29'-6"	30'-0"
1-3/4 x 7-1/4 LVL STUDS @ 16" O.C.	27'-0"	30'-0"	30'-0"
1-3/4 x 5-1/2 LVL STUDS @ 16" O.C.	20'-6"	21'-6"	22'-0"



NOTES:
1. THIS TABLE ASSUMES IBC WIND LOADS w/ 115 mph, EXP. "C" AT EXTERIOR WALLS & 5#sf LATERAL LOAD AT INTERIOR WALLS.
2. THIS TABLE ASSUMES AXIAL DL = 710 lb/ft, LL = 760 lb/ft. AT EXTERIOR AND INTERIO WALLS.
3. THIS TABLE ASSUMES IBC 5psf LATERAL LOAD @ INTERIOR WALLS.

STANDARD STUD TABLE

N.T.S. 4



TYPICAL OVERBUILD

N.T.S. 5

SHEAR WALL SCHEDULE

MARK	MIN. BLOCKED MATERIAL	EDGE / BOUNDARY NAILING	FIELD NAILING	SOLE PL NAILING, WHERE OCCURS	SHEAR WALL CAPACITY	DEFAULT SILL ANCHORAGE, U.N.O.
P1	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 6" O.C.	8d COMMON @ 12" O.C.	16d SINKERS @ 6" O.C.	260 plf	S1
P2	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 4" O.C.	8d COMMON @ 12" O.C.	16d SINKERS @ 4" O.C.	350 plf	S2
P3	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 3" O.C.	8d COMMON @ 12" O.C.	16d SINKERS @ 3" O.C.	490 plf	S3
P4	3/8" PLYWOOD OR O.S.B.	8d COMMON NAILS @ 2" O.C.	8d COMMON @ 12" O.C.	16d SINKERS @ 2" O.C.	640 plf	S4

SILL ANCHORAGE SCHEDULE

MARK	NOMINAL SILL PL THICKNESS	ø1/2" A.B. SPACING	ø5/8" A.B. SPACING	CAPACITY
S1	2x	32" O.C.	48" O.C.	370 plf
S2	2x	24" O.C.	32" O.C.	520 plf
S3	2x	16" O.C.	24" O.C.	740 plf
S4	2x	12" O.C.	16" O.C.	1040 plf

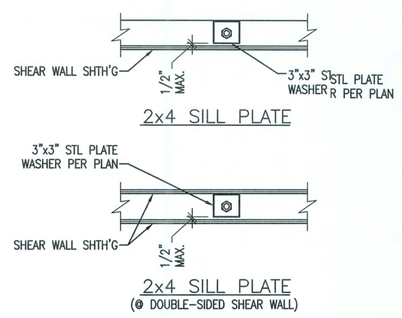
SHEAR WALL LENGTH TOLERANCES

SPECIFIED SHEAR WALL LENGTH	ACCEPTABLE SHEAR WALL TOLERANCE
UP TO 3'-0"	± 2"
OVER 3'-0" AND UP TO 5'-0"	± 3"
OVER 5'-0" AND UP TO 7'-0"	± 4"
OVER 7'-0" AND UP TO 10'-0"	± 8"
OVER 10'-0"	± 8"

1. ALL SHEAR WALLS SHALL BE FRAMED TO THE MINIMUM LENGTHS SHOWN ON THE PLANS WITH THE TOLERANCES INDICATED ON THE TABLE ABOVE, U.N.O. ON PLAN w/ MINIMUM WALL LENGTH.
2. ALL SHEAR WALLS SHALL TERMINATE ON AT LEAST (1) FULL HEIGHT STUD. ADDITIONAL STUDS OR SOLID POSTS SHALL BE INSTALLED AS REQUIRED FOR HOLDOWNS WHERE THEY OCCUR.
3. 8d COMMON NAIL SHANK DIAMETER = .131", 16d SINKER SHANK DIAMETER = .148"
4. FOR "P3" AND "P4" SHEAR WALLS, ALL FRAMING RECEIVING EDGE NAILING FROM ADJOINING PANEL EDGES SHALL BE 3-INCH NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED. AS AN ALTERNATE, (2) 2x STUDS MAY BE USED PROVIDED THEY ARE NAILED TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT.
5. FOR "P2", "P3" AND "P4" DOUBLE-SIDED SHEAR WALLS, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING SHALL BE 3-INCH NOMINAL OR WIDER AT ADJOINING PANEL EDGES AND NAILS ON EACH SIDE SHALL BE STAGGERED.
6. ALL ANCHOR BOLTS SHALL HAVE 7" MINIMUM EMBEDMENT.
7. ALL SHEAR WALL ANCHOR BOLTS SHALL INCLUDE A STEEL 3"x3"x0.229" PLATE WASHER BETWEEN THE SILL PL & NUT. THE HOLE IN THE PLATE WASHER IS PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1¾", PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. ANCHOR BOLTS & PLATE WASHERS ARE TO BE OFFSET TOWARD THE SHEATHED WALL EDGE TO LIMIT THE GAP BETWEEN THE EDGE OF WASHER TO SHEATHING TO A MAXIMUM OF 1/2". WHERE BOTH SIDES OF A 2x6 WALL IS SHEATHED A STEEL 4-1/2"x3"x0.229" PLATE WASHER SHALL BE CENTERED ON THE SILL PLATE, PER DTL 2/-.

STANDARD SHEAR WALL SCHEDULE

N.T.S. 1



STANDARD HOLDOWN ANCHORAGE SCHEDULE

N.T.S. 2

FOOTING SCHEDULE

MARK	SIZE	REINFORCING, BOTTOM
F2.0	2'-0" SQ. x 12" THICK	(3) #4 EACH WAY
F2.5	2'-6" SQ. x 12" THICK	(4) #4 EACH WAY
F3.0	3'-0" SQ. x 12" THICK	(4) #4 EACH WAY
F3.5	3'-6" SQ. x 12" THICK	(5) #4 EACH WAY
F4.0	4'-0" SQ. x 12" THICK	(6) #4 EACH WAY
F4.5	4'-6" SQ. x 12" THICK	(6) #4 EACH WAY
F5.0	5'-0" SQ. x 12" THICK	(7) #4 EACH WAY
F5.5	5'-6" SQ. x 12" THICK	(8) #4 EACH WAY

STANDARD SPOT FOOTING SCHEDULE

N.T.S. 3

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NATALIA SCOTT
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STANDARD DETAILS & SCHEDULES

09/21/2020
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S1.2

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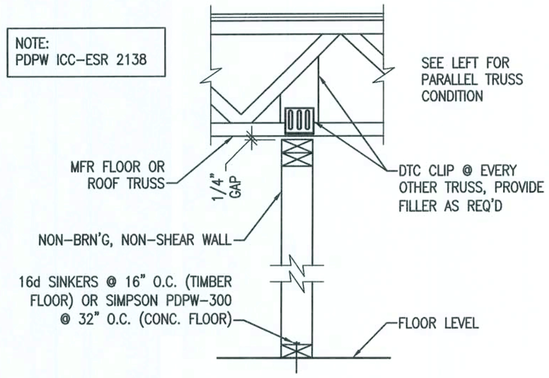
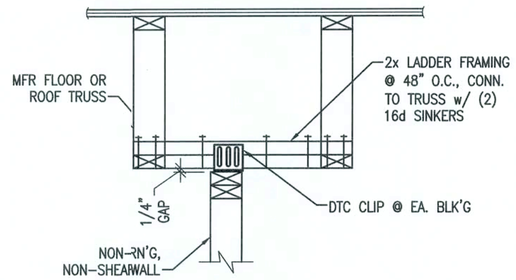
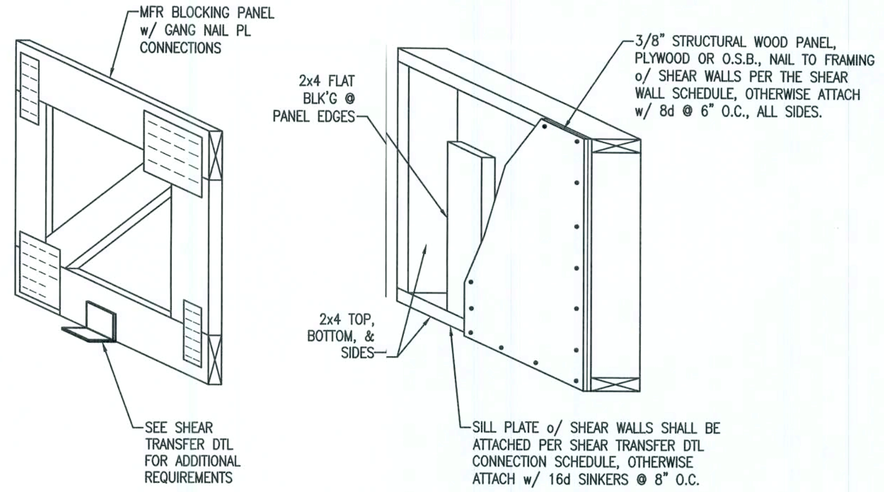




NON-BRN'G & NON-SHEAR WALL CONN. 4

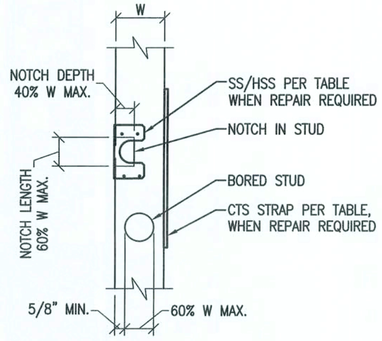
N.T.S.



TYPICAL BLOCKING PANEL 1

N.T.S.

STUD NOTCHES OR HOLES



ALLOWABLE HOLES OR NOTCHES FOR NON-BEARING, NON-SHEAR OR INTERIOR PARTITIONS (NO REPAIR REQ'D)

HOLE / NOTCH SCHEDULE

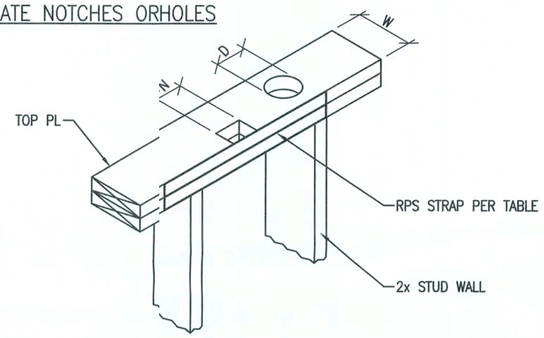
HOLE / NOTCH % OF 'W'	2x4 STUD	2x6 STUD
25%	3/4"	1-3/8"
40%	1-3/8"	2-1/8"
60%	2"	3-1/4"

NOTES:
1. HOLES & NOTCHES SHALL NOT OCCUR IN THE SAME STUD.
2. WHERE HOLES OR NOTCHES EXCEED THOSE SHOWN ABOVE, REPAIR PER TABLE BELOW.
3. ALL NOTCHES IN BEARING OR SHEAR OR EXTERIOR WALLS REQUIRE REPAIRS.

STUD HOLE REPAIR

	2x4 STUD HOLE DIA. 'D'	2x6 STUD HOLE DIA. 'D'	REPAIR
NON-BEARING & NON-SHEAR & INTERIOR	≤ 2 3/4"	≤ 4 1/2"	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ 3/4"	≤ 1 3/8"	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR	≤ 2 3/4"	≤ 4 1/2"	(2) CTS218 TWO-SIDED w/ 10d

PLATE NOTCHES ORHOLES



2x4 STUD HOLE DIA. 'D'	2x6 STUD HOLE DIA. 'D'	2x4 & 2x6 PLATE NOTCH WIDTH 'N' (MAX. NOTCH DEPTH = W/2	RPS STRAP
≤ 7/8"	≤1"	≤ 1"	NONE
≤ 1"	≤1 3/8"	≤ 2 1/2"	(1) RPS18
≤ 1 3/8"	≤2 1/8"	≤ 5 1/2"	(2) RPS18
≤ 2"	≤3 1/4"	≤ 12"	(2) RPS28

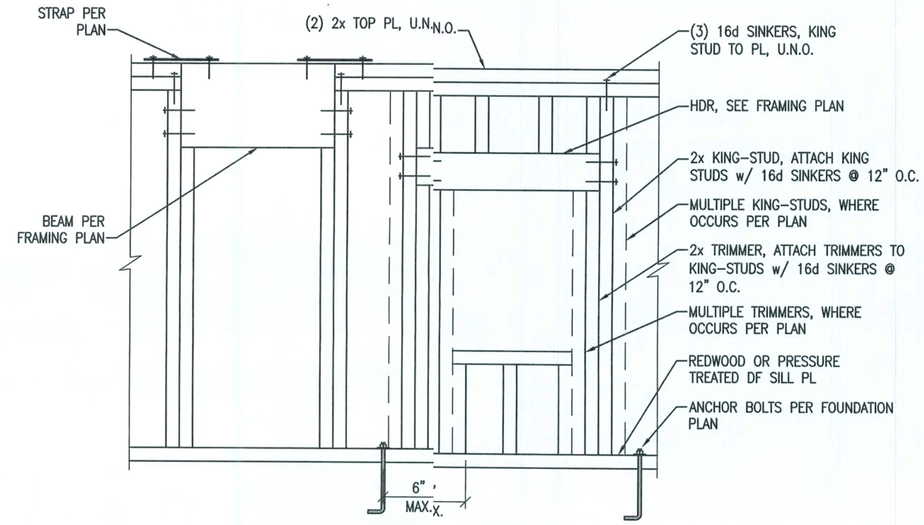
NOTES:
1. USE RPSZ FOR SILL LATE.
2. CENTER STRAPS @ NCH OR HOLE.
3. WHERE ROOF TRUSS R FLOOR JOIST IS BEARING WITHIN STUD BAY OF THE HOLE OR NOTCH, INSLL AN ADDITIONAL STUD DIRECTLY BELOW THE TRUSS OR JOIST UNLESS NRPS STRAP IS REQUIRED OR WHERE EXISTING STUD FACE IS WITHIN 3" C TRUSS OR JOIST FACE.
4. NOTCHES & HOLES IST BE SEPARATED BY "2xD" OR "2xN".
5. WHERE MULTIPLE HOS ARE LOCATED ADJACENT TO EACH OTHER, THE STRAP REPAIR MAY BE WITH CS16 STRAP ON EACH SIDE OF THE UPPER PLATE. THE STRAPS AND NANG SHALL EXTEND AT LEAST 9" BEYOND EACH END OF THE WHOLE GROUP. NAILING BETWEEN THE HOLES IS NOT REQUIRED. NAILS IN THE CS16 STRAPSMAY BE N8'S OR N10'S.

STUD NOTCH REPAIR

	2x STU NOTCH DEPT	2x4 STUD NOTCH LENGTH	2x6 STUD NOTCH DEPTH	2x6 STUD NOTCH LENGTH	REPAIR
NON-BEARING & NON-SHEAR & INTERIOR	≤ 2 '2"	≤ 4 1/2"	≤ 3 3/4"	≤ 4 1/2"	(1) CTS218 w/ 10d
BEARING OR SHEAR OR EXTERIOR WALL	≤ S '2"	≤ 2 1/2"	≤ 2 1/2"	≤ 2 1/2"	SS w/ 10d
BEARING OR SHEAR OR EXTERIOR	≤ 2 '4"	≤ 4 1/2"	≤ 4 1/2"	≤ 4 1/2"	(2) CTS218 TWO-SIDED w/ 10d

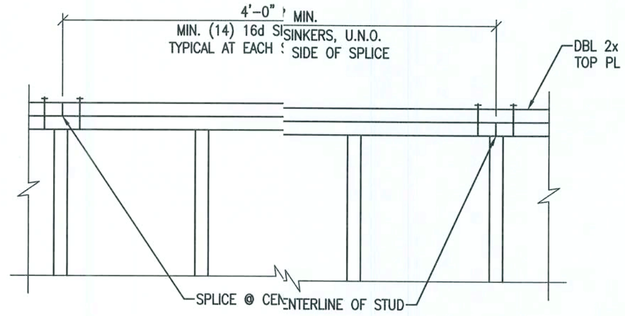
DRILLING &NOTCHING OF PLATES & STUDS 5

N.T.S.



TYPICAL WALL FRAMING 2

N.T.S.



NOTE:
WHERE SPLICE LENGTH IS LESS THAN 4'-0" INSTALL ST6224 STRAP AT PL SPLICES. STRAPS ARE NOT REQUIRED WHERE ONE OF THE PLATES IS CONTINUOUS FOR AT LEAST 4'-0" IN EACH DIRECTION.

TYPICAL TOP PLATE SPLICE 3

N.T.S.

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SCOTT RESIDENCE
WHITE, FL
STANDARD DETAILS & SCHEDULES

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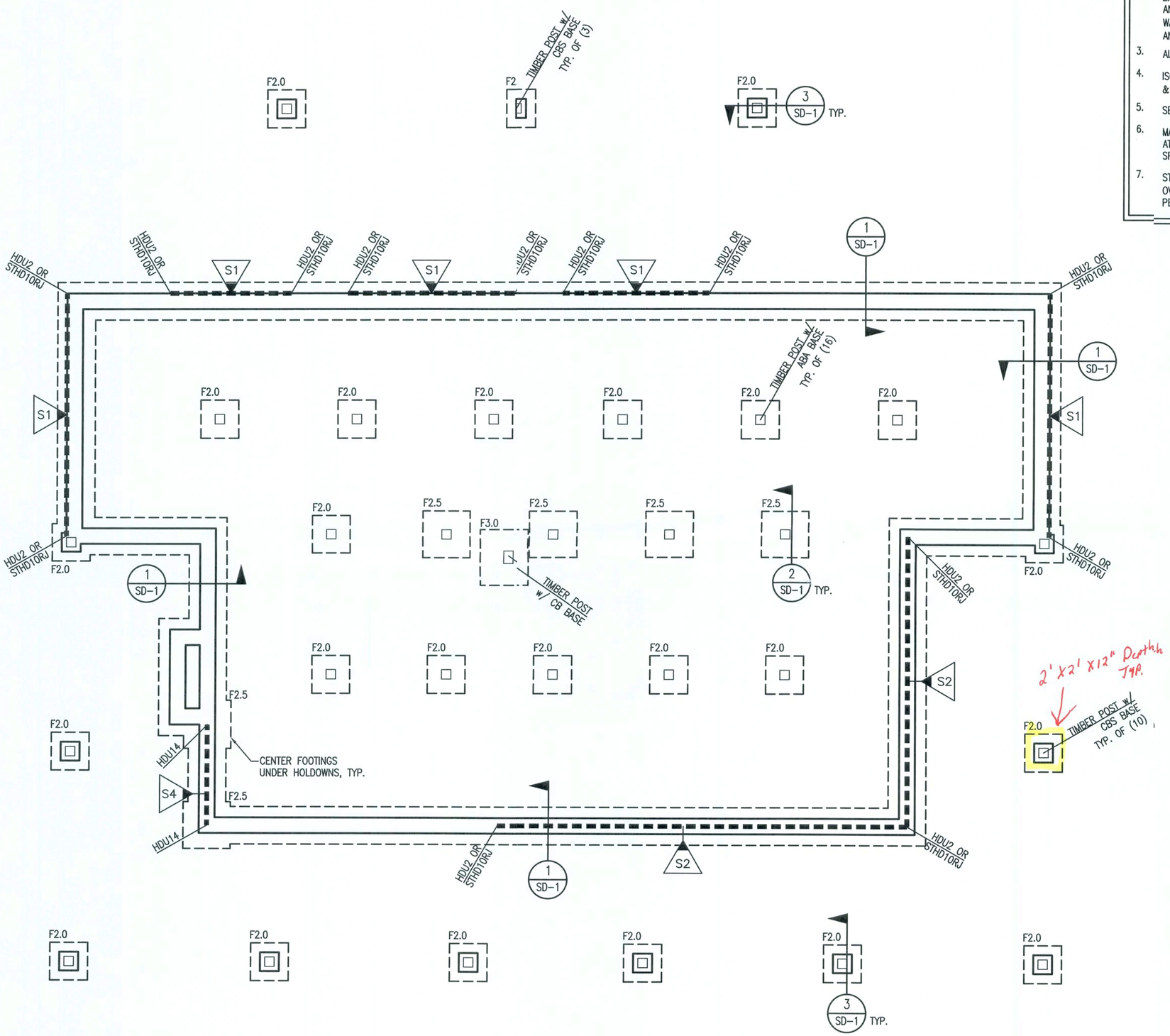
S1.3

9/18/2020 10:11 AM



FOUNDATION PLAN

1/4" = 1'-0"

FOUNDATION NOTES:

1. ALL DIMENSIONS ARE PER ARCHITECTURAL DRAWINGS.
2. ALL EXTERIOR WALLS, INTERIOR BEARING WALLS & SHEAR WALLS TO BE ATTACHED TO THE FOUNDATION w/ ø1/2" x 10" LONG ANCHOR BOLTS (7" EMBED.) AT 32" O.C., U.N.O. SEE THIS PLAN & SHEAR WALL SCHEDULE FOR ANCHOR BOLT REQUIREMENTS AT SHEAR WALLS. ANCHOR BOLTS AT SHEAR WALLS TO HAVE WASHERS PER SHEAR WALL SCHEDULE (S1.1). ALL OTHER ANCHOR BOLTS TO HAVE WASHERS PER NOTE "E" IN GENERAL NOTES (S1).
3. ALL HOLDOWNS SHALL BE INSTALLED AS SHOWN ON DETAIL 4/SD-1.
4. ISOLATED FOOTINGS & INTERIOR STRIP FOOTINGS TO BE CENTERED BELOW POSTS & BEARING/SHEAR WALLS, RESPECTIVELY.
5. SEE SHEET S1.1 FOR FOOTING SCHEDULE.
6. MASA MUDSILL ANCHORS MAY BE USED IN PLACE OF ANCHOR BOLTS, INSTALLED AT THE SAME SPACING INDICATED FOR ANCHOR BOLTS, INCLUDING REDUCED SPACING AT SHEAR WALLS.
7. STRIP & REMOVE EXISTING VEGETATION, REMOVE UNCONTROLLED FILL, OVEREXCAVATE AND REPLACE w/ PROPERLY COMPACTED FILL AS REQUIRED PER GEOTECHNICAL REPORT.

NOTE:
SEE SHEET S1.1 FOR SILL ANCHORAGE SCHEDULE

SILL ANCHORAGE KEY

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NATALIA SCOTT
SCOTT RESIDENCE
WHITE, FL
FOUNDATION PLAN

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S2

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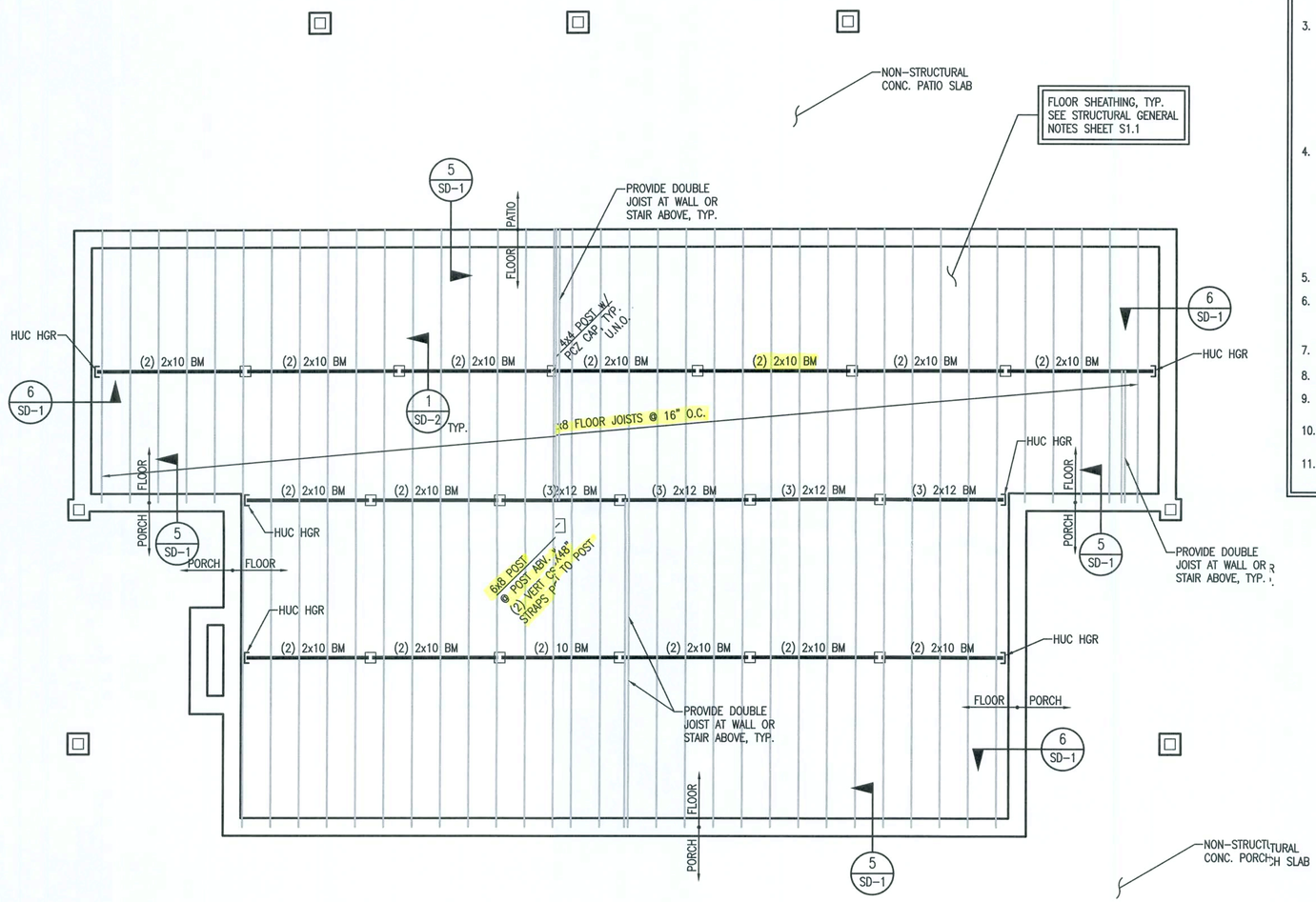

MAIN FLOOR FRAMING PLAN

1/4" = 1'-0"

9/18/2020 10:11 AM

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
2. FOR 2x4 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x4 TRIMMER & (1) 2x4 KING STUD AT OPENINGS < 6'-0" U.N.O.
   B. PROVIDE (2) 2x4 TRIMMERS & (2) 2x4 KING STUDS AT OPENINGS ≥ 6'-0" & ≤ 10'-0" U.N.O.
   C. PROVIDE (2) 2x4 TRIMMERS & (3) 2x4 KING STUDS AT OPENINGS ≥ 10'-0" & ≤ 18'-0" U.N.O. (1) KING STUD REQUIRED AT BAY WINDOW OPENINGS & AT GARAGE OPENINGS WHERE ADDITIONAL KING STUDS WOULD NOT FIT.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
3. FOR 2x6 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x6 TRIMMER & (1) 2x6 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x6 TRIMMERS & (2) 2x6 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x6 TRIMMERS & (3) 2x6 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
4. FOR 2x8 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x8 TRIMMER & (1) 2x8 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x8 TRIMMERS & (2) 2x8 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x8 TRIMMERS & (3) 2x8 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
5. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.
6. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6/S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER
7. INTERIOR BEARING WALLS
8. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.
9. PROVIDE (2) 2x POST, EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O.
10. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.
11. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

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COA 26626

NATALIA SCOTT
SCOTT RESIDENCE
WHITE, FL
MAIN FLOOR FRAMING PLAN

09/21/2020
JACOB S. PROCTOR, P.E.
74277

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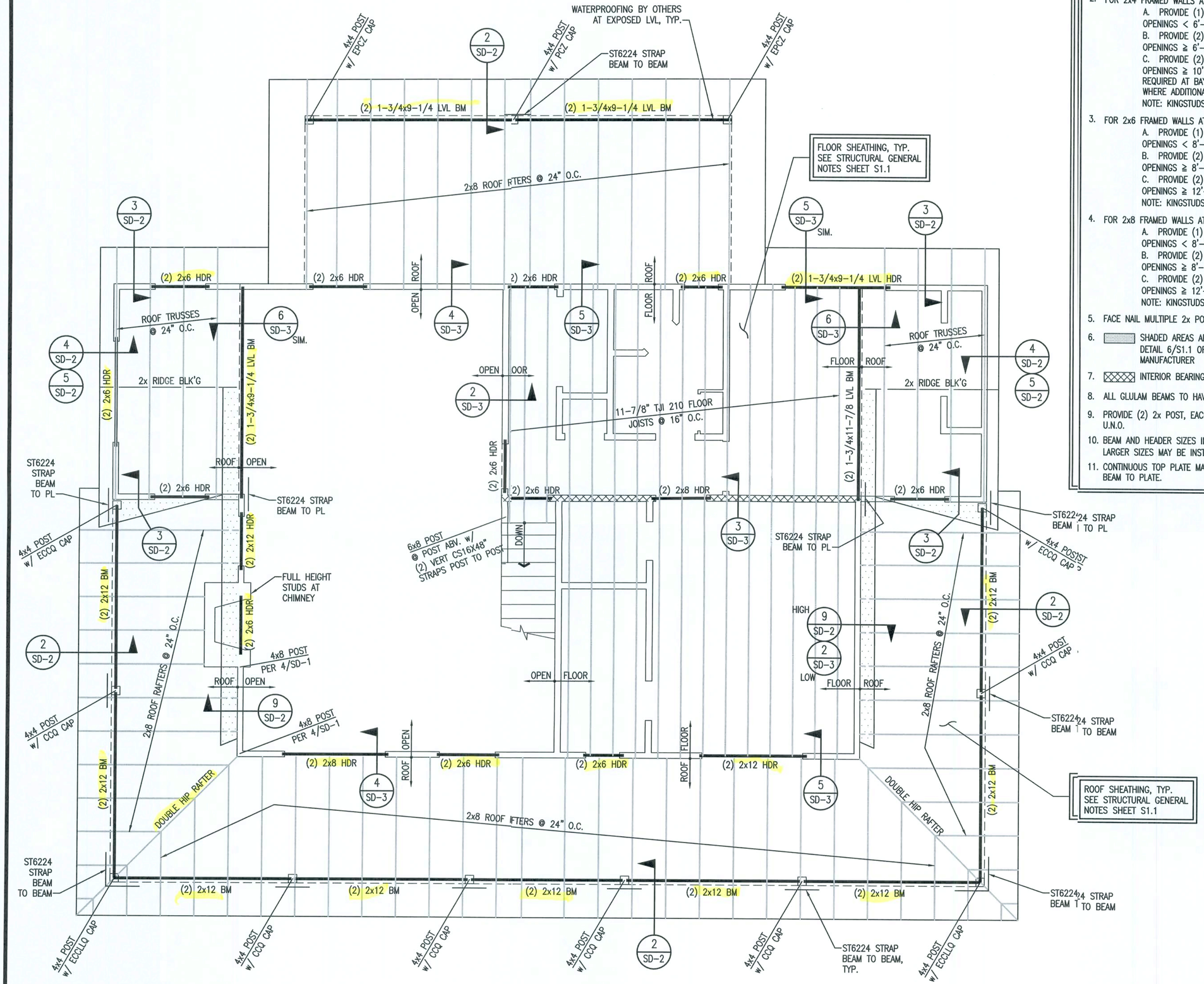
S3



UPPER FLOOR FRAMING PLAN

1/4" = 1'-0"

9/18/2020 10:11 AM

FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
2. FOR 2x4 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x4 TRIMMER & (1) 2x4 KING STUD AT OPENINGS < 6'-0" U.N.O.
   B. PROVIDE (2) 2x4 TRIMMERS & (2) 2x4 KING STUDS AT OPENINGS ≥ 6'-0" & ≤ 10'-0" U.N.O.
   C. PROVIDE (2) 2x4 TRIMMERS & (3) 2x4 KING STUDS AT OPENINGS ≥ 10'-0" & ≤ 18'-0" U.N.O. (1) KING STUD REQUIRED AT BAY WINDOW OPENINGS & AT GARAGE OPENINGS WHERE ADDITIONAL KING STUDS WOULD NOT FIT.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
3. FOR 2x6 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x6 TRIMMER & (1) 2x6 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x6 TRIMMERS & (2) 2x6 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x6 TRIMMERS & (3) 2x6 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
4. FOR 2x8 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x8 TRIMMER & (1) 2x8 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x8 TRIMMERS & (2) 2x8 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x8 TRIMMERS & (3) 2x8 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
5. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.
6. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6/S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER
7. INTERIOR BEARING WALLS
8. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.
9. PROVIDE (2) 2x POST, EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O.
10. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.
11. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

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NATALIA SCOTT
SCOTT RESIDENCE
WHITE, FL
UPPER FLOOR FRAMING PLAN

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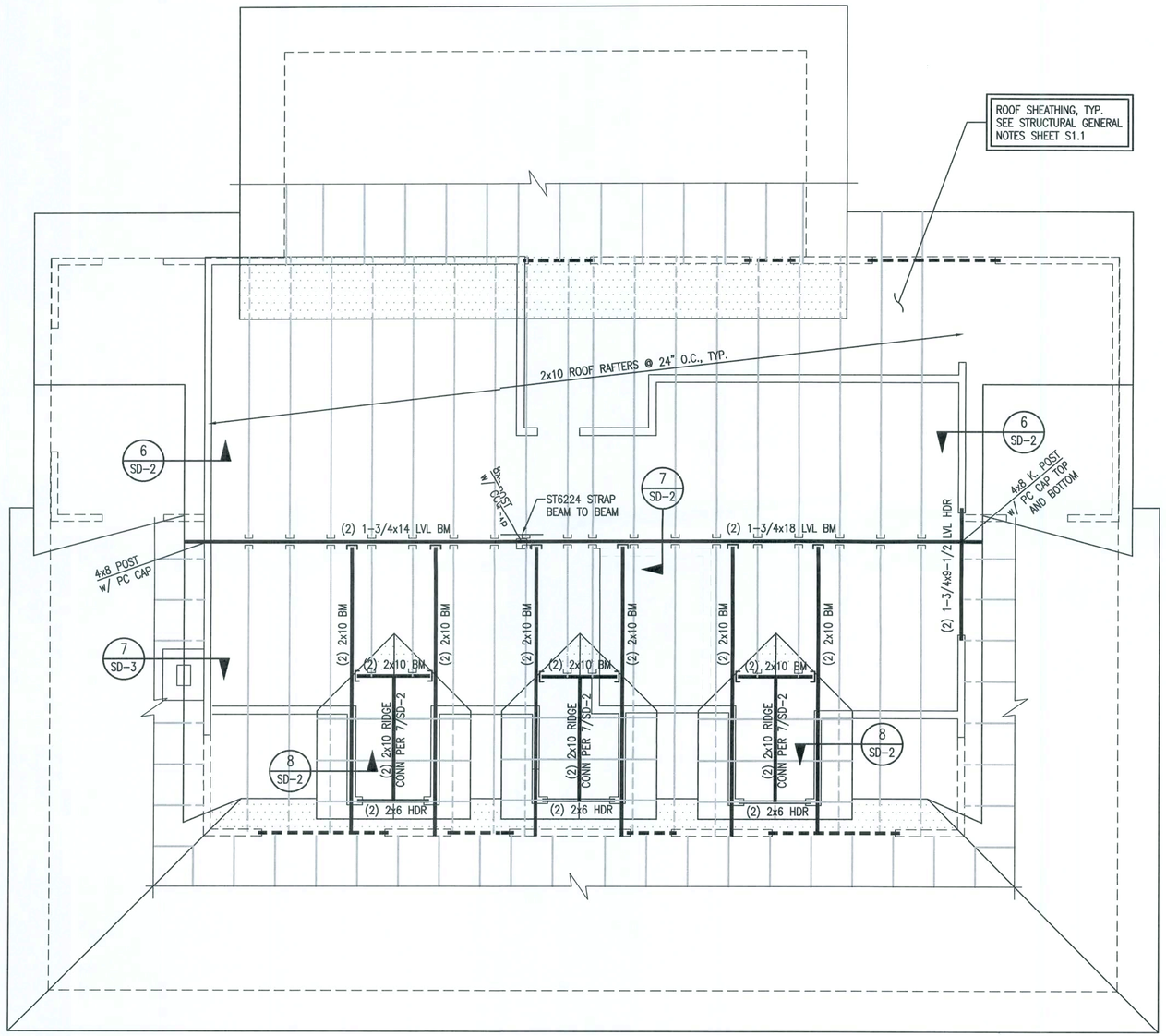
S4



ROOF FRAMING PLAN

1/4" = 1'-0"

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FRAMING NOTES:

1. ALL FRAMED WALLS TO BE 2x @ 16" O.C. (MAX) PER ARCHITECTURAL PLANS AND SHALL MEET REQUIREMENTS OF WALL TABLE ON SHEET S1.1.
2. FOR 2x4 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x4 TRIMMER & (1) 2x4 KING STUD AT OPENINGS < 6'-0" U.N.O.
   B. PROVIDE (2) 2x4 TRIMMERS & (2) 2x4 KING STUDS AT OPENINGS ≥ 6'-0" & ≤ 10'-0" U.N.O.
   C. PROVIDE (2) 2x4 TRIMMERS & (3) 2x4 KING STUDS AT OPENINGS ≥ 10'-0" & ≤ 18'-0" U.N.O. (1) KING STUD REQUIRED AT BAY WINDOW OPENINGS & AT GARAGE OPENINGS WHERE ADDITIONAL KING STUDS WOULD NOT FIT.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
3. FOR 2x6 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x6 TRIMMER & (1) 2x6 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x6 TRIMMERS & (2) 2x6 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x6 TRIMMERS & (3) 2x6 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
4. FOR 2x8 FRAMED WALLS AT HEADERS (HDR):
   A. PROVIDE (1) 2x8 TRIMMER & (1) 2x8 KING STUD AT OPENINGS < 8'-0" U.N.O.
   B. PROVIDE (2) 2x8 TRIMMERS & (2) 2x8 KING STUDS AT OPENINGS ≥ 8'-0" & ≤ 12'-0" U.N.O.
   C. PROVIDE (2) 2x8 TRIMMERS & (3) 2x8 KING STUDS AT OPENINGS ≥ 12'-0" & ≤ 20'-0" U.N.O.
   NOTE: KINGSTUDS NOT REQUIRED AT BEAMS (BM)
5. FACE NAIL MULTIPLE 2x POSTS WITH 16d SINKERS @ 6" O.C.
6. SHADED AREAS ARE TYPICAL OVERFILL, STICK FRAMED PER DETAIL 6/S1.1 OR OVERBUILD TRUSSES PER TRUSS MANUFACTURER
7. INTERIOR BEARING WALLS
8. ALL GLULAM BEAMS TO HAVE STANDARD CAMBER (R = 2000') U.N.O.
9. PROVIDE (2) 2x POST, EACH END OF ALL BEAMS & GIRDER TRUSSES, U.N.O.
10. BEAM AND HEADER SIZES INDICATED ON THE PLANS ARE MINIMUM SIZES. LARGER SIZES MAY BE INSTALLED AT THE CONTRACTOR'S OPTION.
11. CONTINUOUS TOP PLATE MAY BE USED IN LIEU OF ST6224 STRAP FROM BEAM TO PLATE.

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WHITE, FL
ROOF FRAMING PLAN

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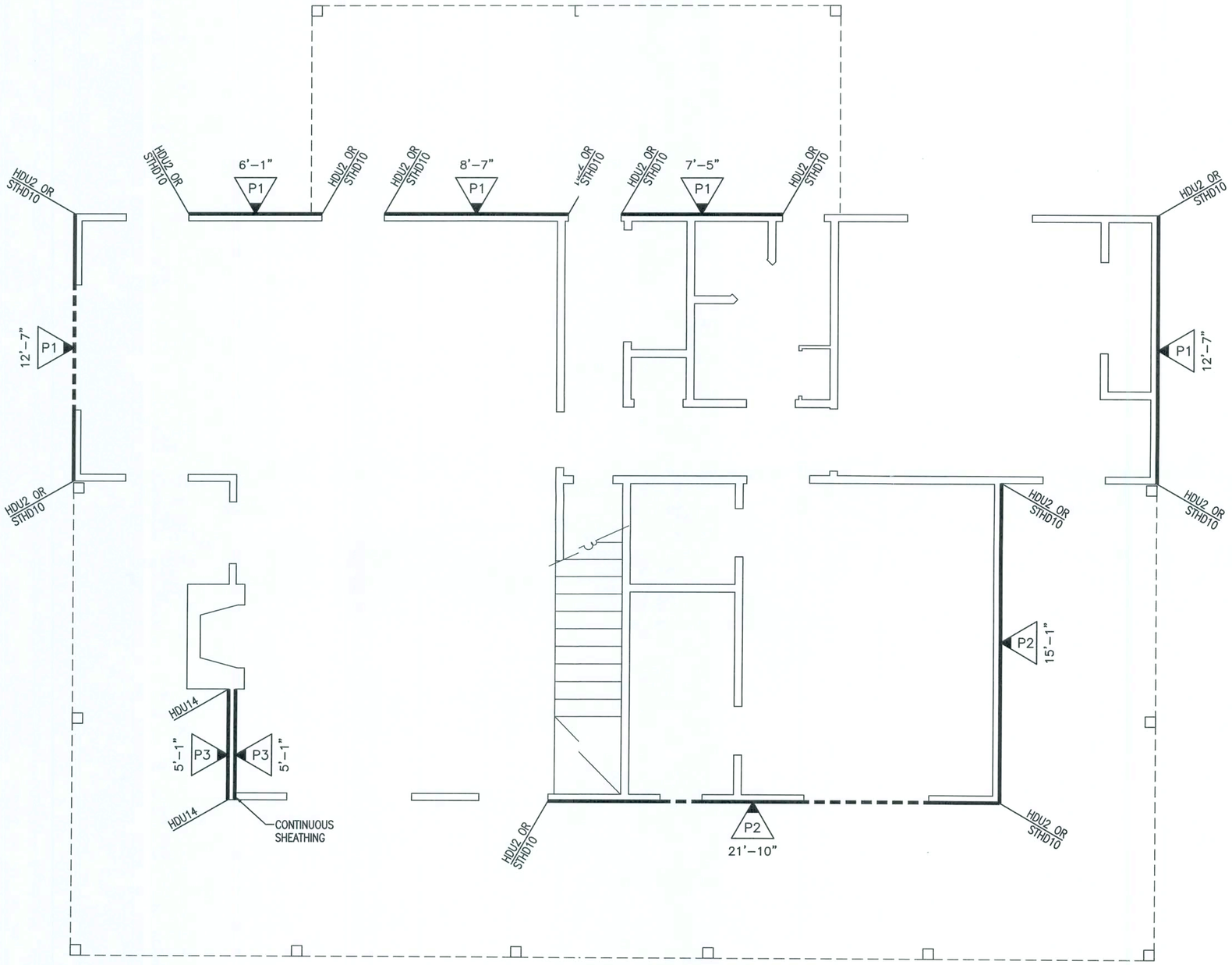
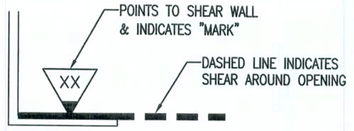
S5



MAIN LEVEL SHEAR WALL PLAN

1/4" = 1'-0"



NOTE:
SEE SHEET S1.1 FOR SHEAR WALL SCHEDULE
SEE SHEET SD-1 FOR HOLDOWN ANCHORAGE SCHEDULE

SHEAR WALL KEY

NOTE:
WHERE STRAP HOLDOWN IS ATTACHED TO A SINGLE KINGSTUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.

NOTE:
SHEAR WALL SHEATHING MAY BE ON EITHER SIDE OF INDICATED WALL.

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MAIN LEVEL SHEAR WALL PLAN

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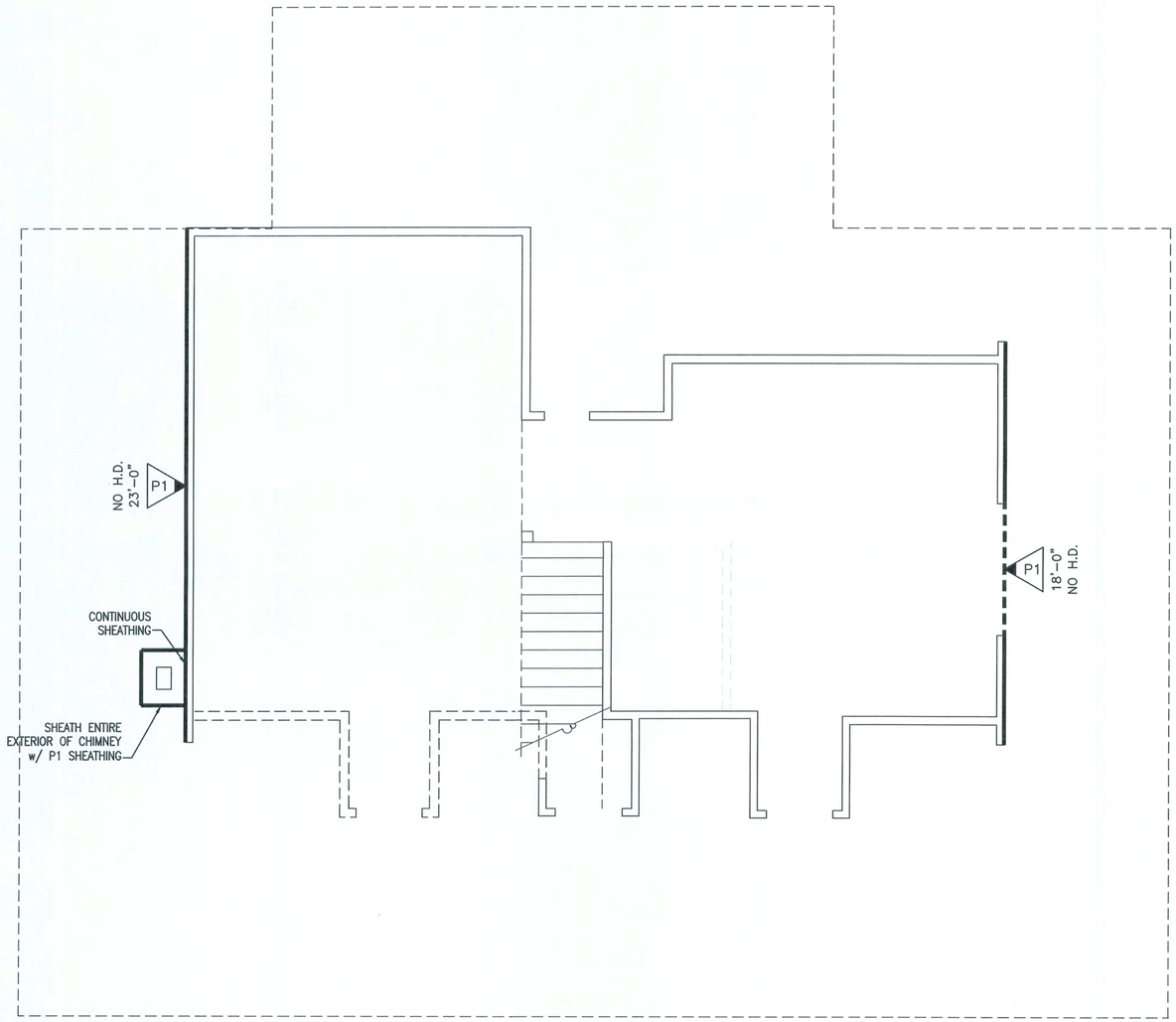

UPPER LEVEL SHEAR WALL PLAN

1/4" = 1'-0"

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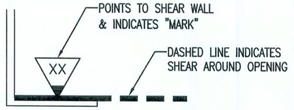

NOTE:
SEE SHEET S1.1 FOR SHEAR WALL SCHEDULE
SEE SHEET SD-1 FOR HOLDOWN ANCHORAGE SCHEDULE

SHEAR WALL KEY

NOTE:
WHERE STRAP HOLDOWN IS ATTACHED TO A SINGLE KINGSTUD & A SINGLE TRIMMER, ATTACH THE TWO TOGETHER w/ (2) 16d SINKERS @ 6" O.C. FULL HEIGHT OR w/ LTP4 @ 12" O.C. FULL HEIGHT.

NOTE:
SHEAR WALL SHEATHING MAY BE ON EITHER SIDE OF INDICATED WALL.

DATE: 09-17-2020	ENG: JAB	DWN: MGP	CHK: JCS
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UPPER LEVEL SHEAR WALL PLAN

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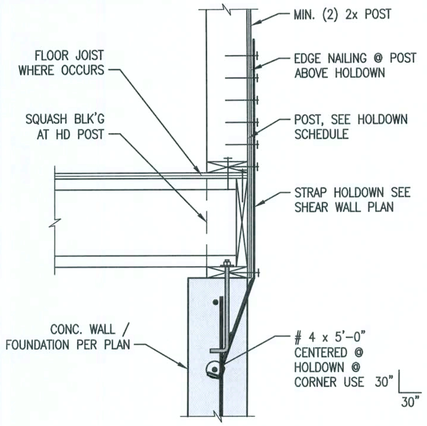
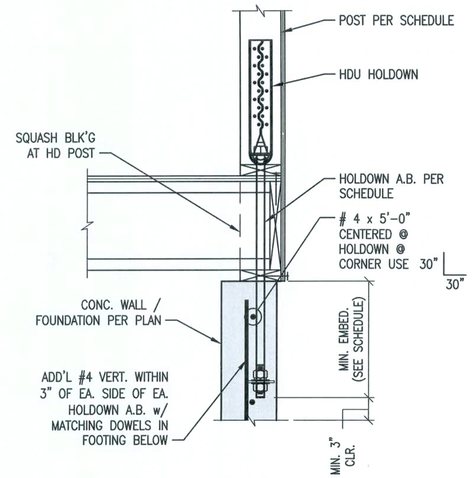
S7



STRAP HOLDOWN

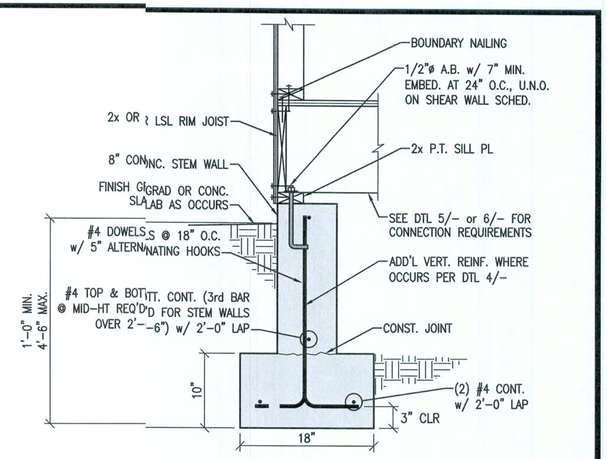


HDU HOLDOWN

ANCHORAGE						
	ANCHORAGE (CAST IN PLACE)		ANCHORAGE (RETROFIT)			
HOLDOWN	SSTB	ALL THREAD ROD (NOTE 1)	ALL THREAD ROD (NOTE 2 & 3)	HOLE DIAMETE	EMBEDMENT	POST
HDU2 – SDS2.5	SSTB24	5/8"ø A307	5/8"ø A307	3/4"	10"	(2) 2x
HDU4 – SDS2.5	SSTB24	5/8"ø A307	5/8"ø A307	3/4"	10"	(2) 2x
HDU5 – SDS2.5	NONE	5/8"ø A307	5/8"ø A307	3/4"	13"	(2) 2x
HDU14 – SDS2.5	NONE	1"ø A307	1"ø A307	1–1/4	8" INTO FOOTING	4x8

NOTES:
1. PROVIDE 2-1/8"x2-1/8"x3/8" STEEL PLATE w/ (2) NUTS @ CAST IN PLACE ANCHORS.
2. RETROFIT ALL-THREAD ROD IN HOLES w/ SIMPSON SET-XP EPOXY. PREPARE HOLES & INSTALL EPOXY PER MFR DIRECTIONS w/ EMBEDMENT AND EDGE DISTANCES AS SHOWN.
3. SIMPSON SET-XP EPOXY PER ICC-ES 2508.
4. INCREASE FOOTING DEPTH AS REQUIRED FOR 3" MIN. COVER BELOW BOLT & COORDINATE EXACT LOCATIONS WITH THE FRAMING CONTRACTOR.
5. HOLDOWNS MAY BE INSTALLED 4" MAX. FROM SHEAR WALL EDGE. BOUNDARY NAILING MUST BE PROVIDED @ STUDS ALIGNED WITH HOLDOWNS.
6. SEE DETAIL 9/S1.1 FOR INTERIOR HDU HOLDOWN ANCHORAGE AND CONCRETE COVER REQUIREMENTS.

N.T.S. 4



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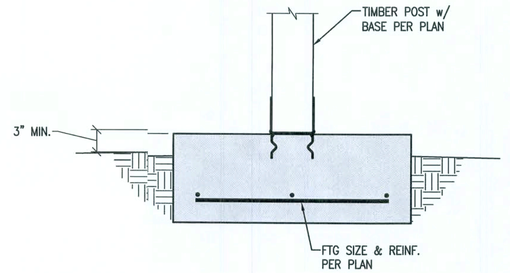

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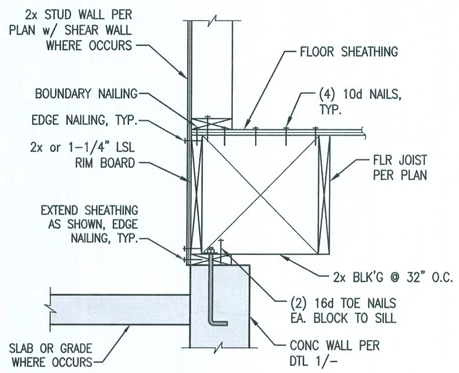

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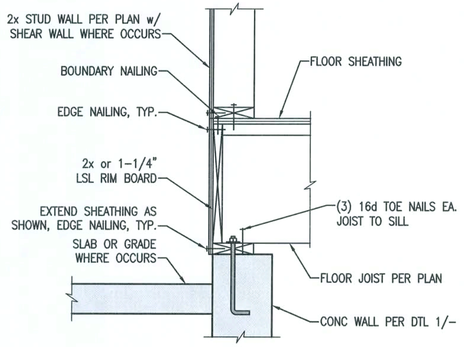

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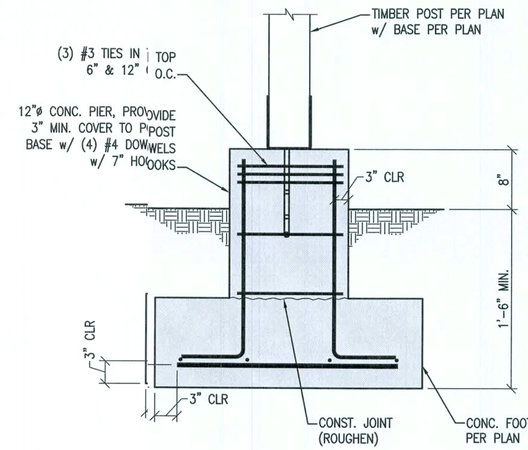

N.T.S. 3

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FL FIRM LICENSE NUMBER CDA 26626

NATALIA SCOTT
SCOTT RESIDENCE
WHITE, FL
STRUCTURAL DETAILS

JACOB S. PROCTOR LICENSE No. 74277 STATE OF FLORIDA PROFESSIONAL ENGINEER
09/21/2020
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74277

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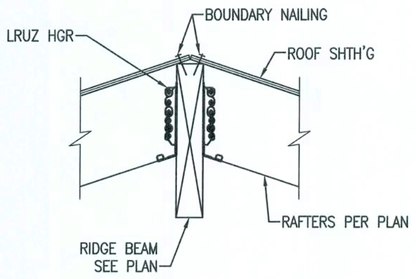

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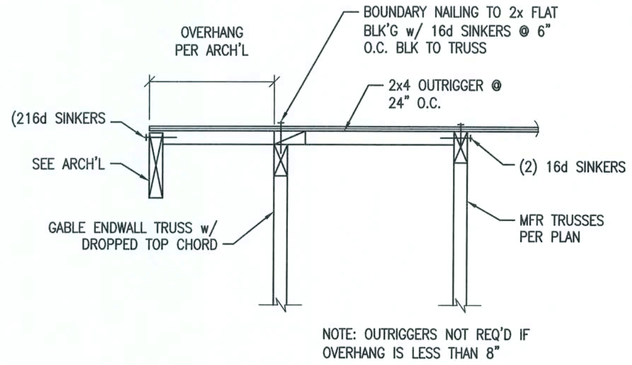

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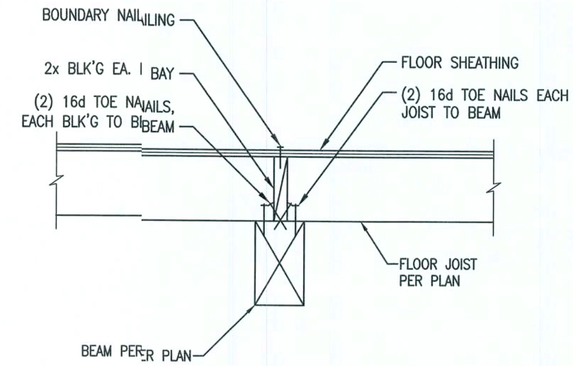

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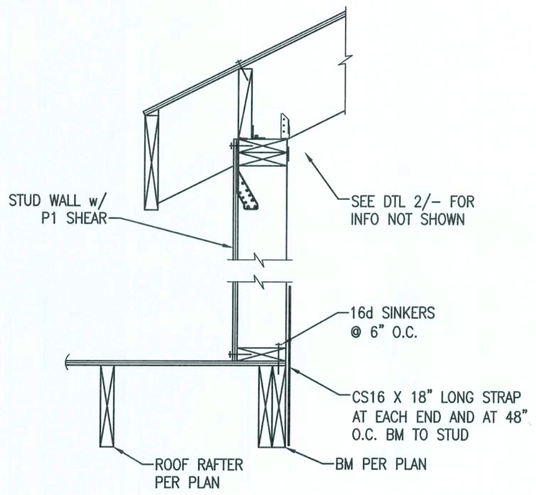

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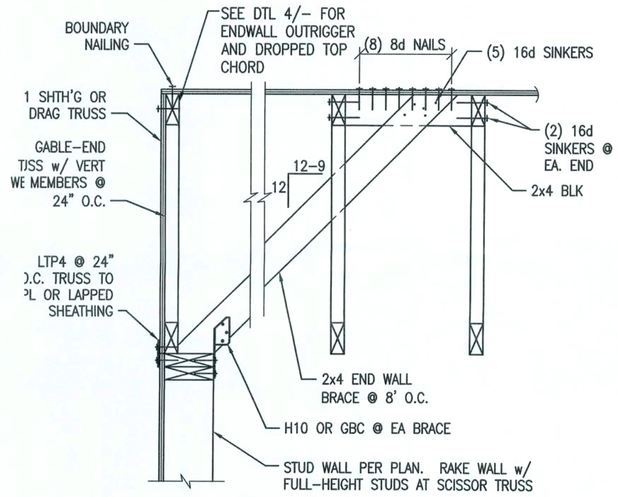

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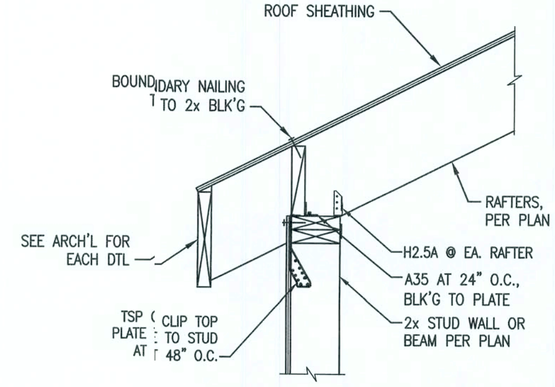

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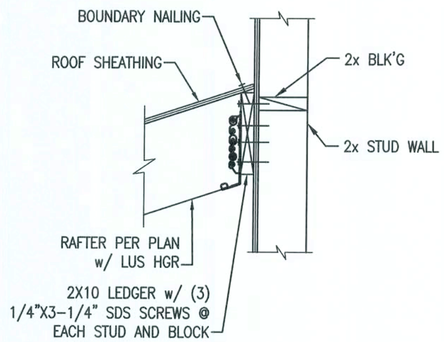

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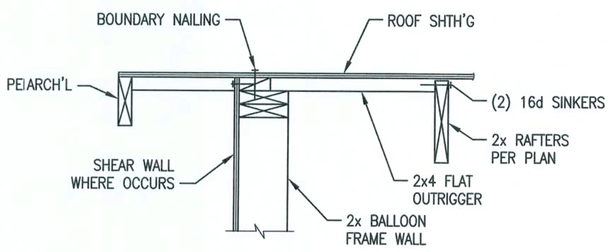

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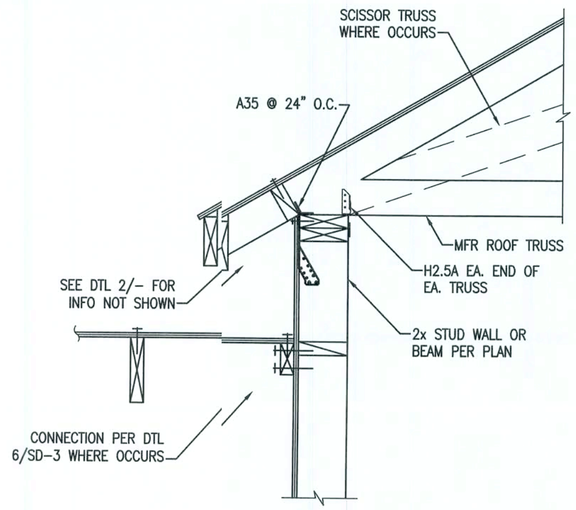

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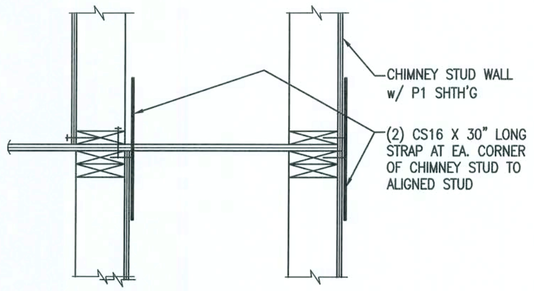

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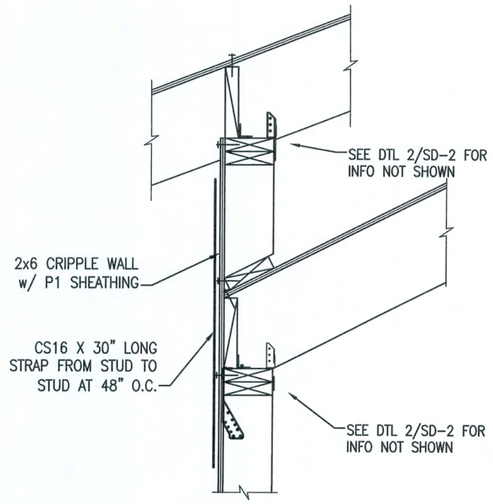

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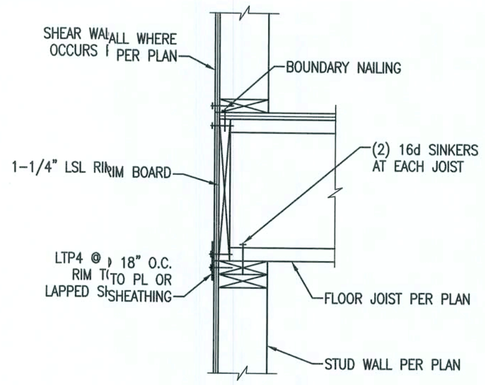

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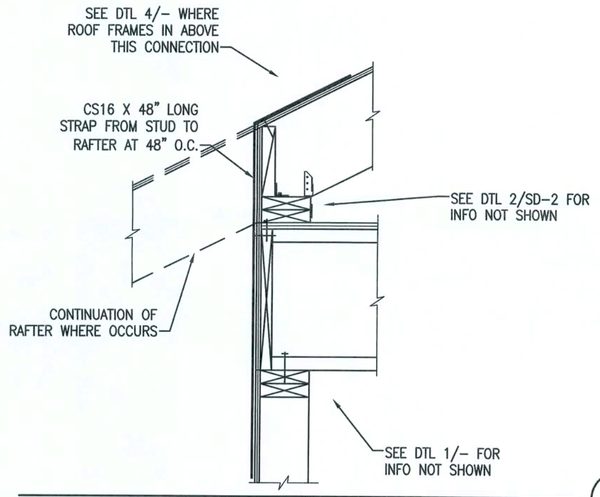

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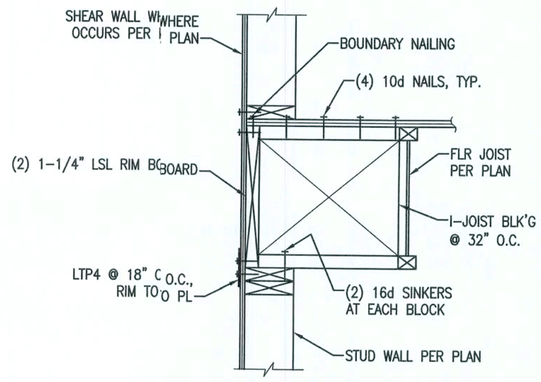

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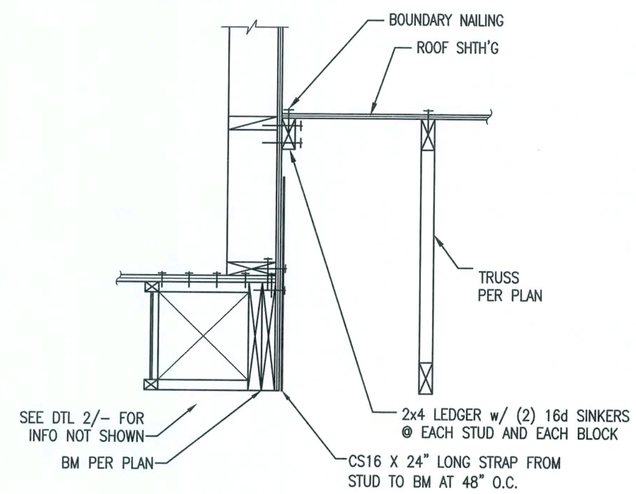

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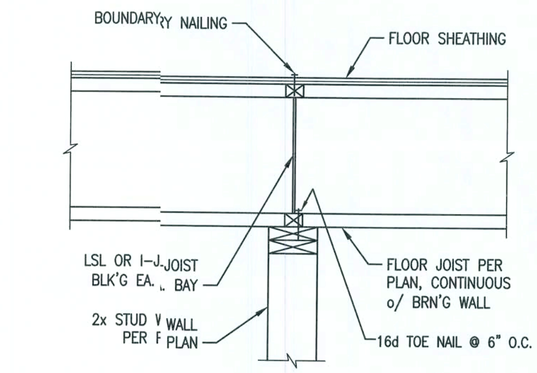

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