

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

October 16, 2006

Studios

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PICAL WALL SECTION

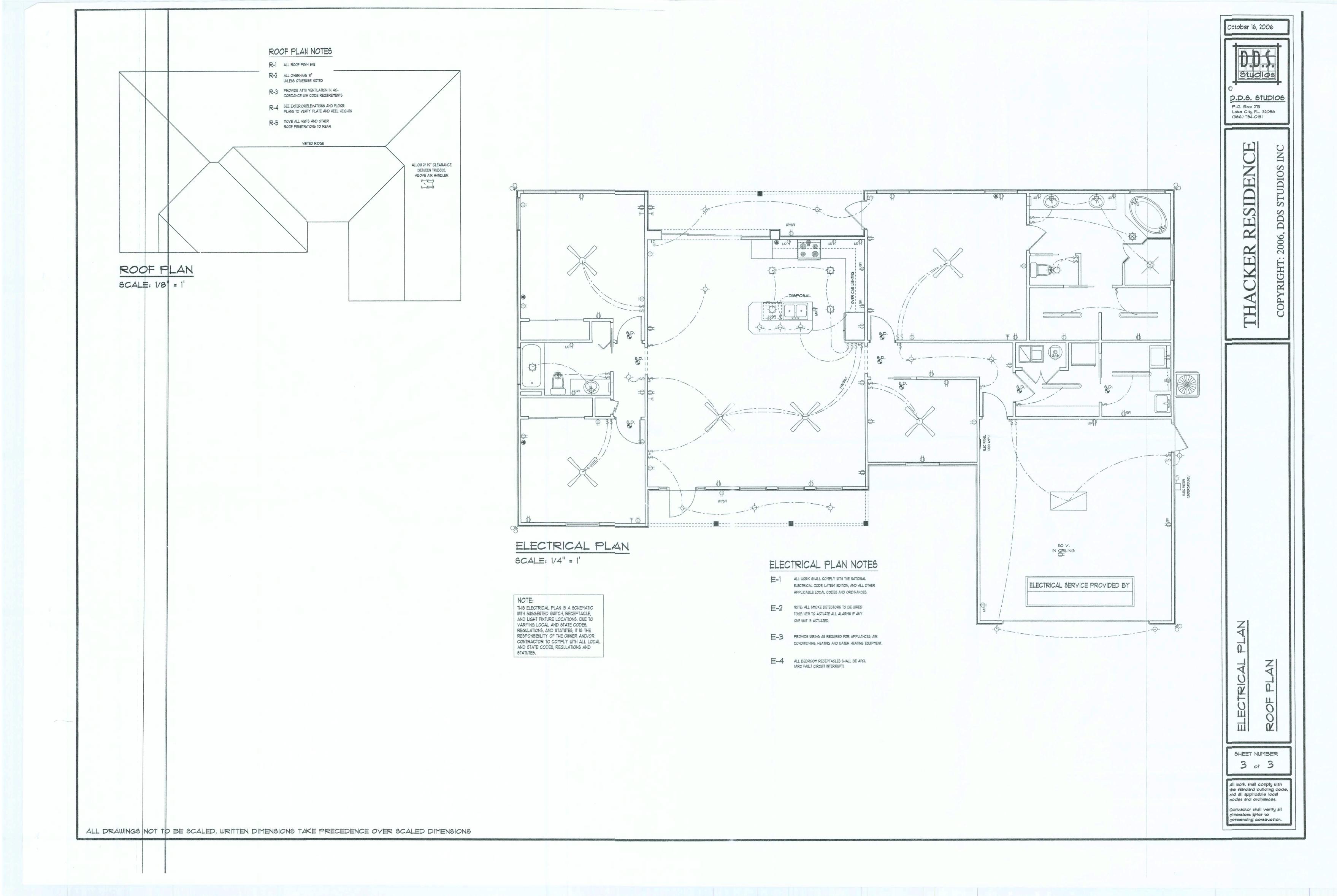
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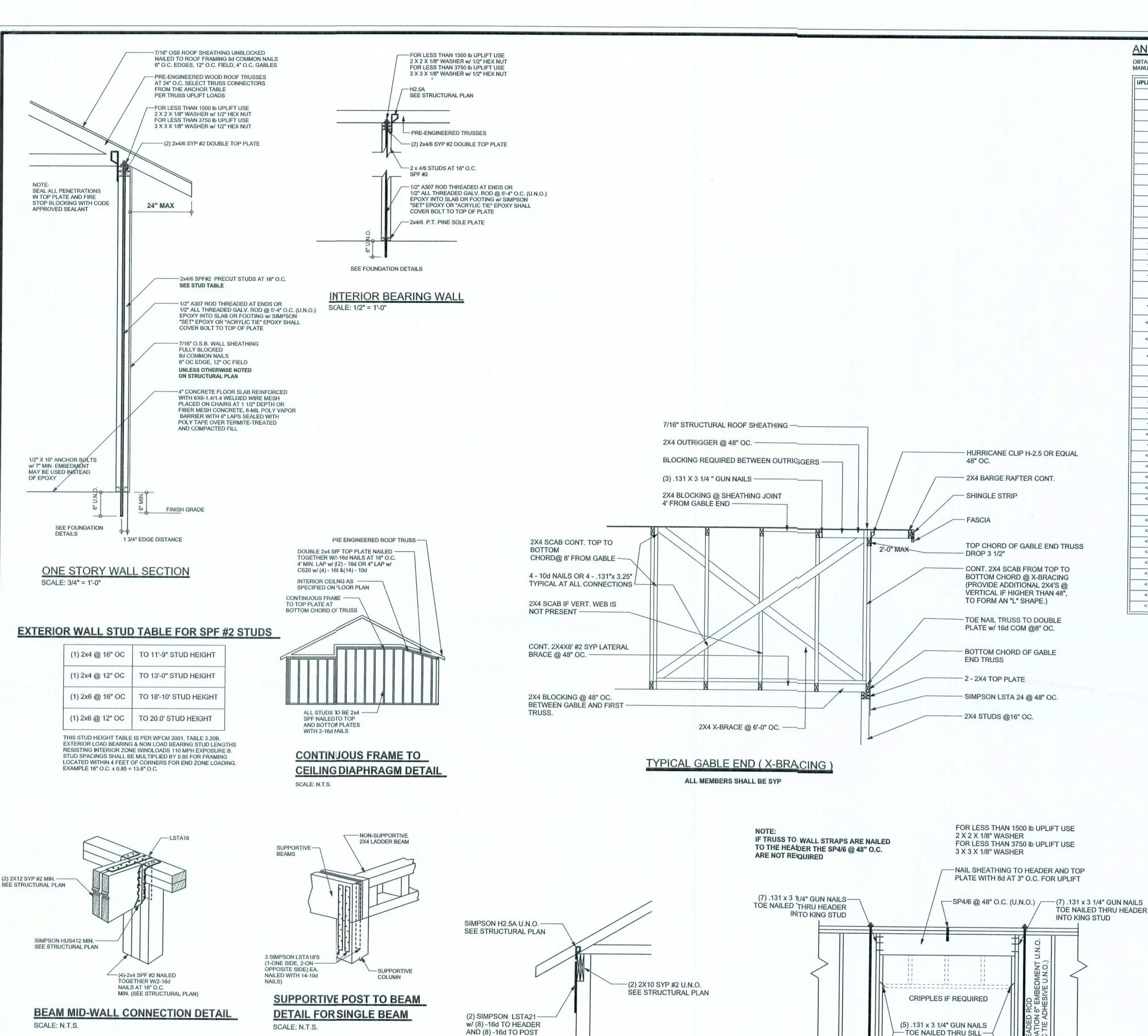
2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to

commencing construction.





-6X6 SYP #2 POST

-SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

(2) 2X12 SYP #2 MIN. ----

BEAM W/4-16d

BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

- SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

SEE STRUCTURAL PLAN

SUPPORTIVEBEAM ---

SUPPORTIVE CENTER POST TO BEAM DETAIL

POST CONNECTION.

INSTALL ONE SIMPSON LSTA18 ON ONE SIDE

4-SIMPSON LSTA18 ---

(2-ONE SIDE, 2-ON

OTHER SIDE)

SCALE: N.T.S.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER FEINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER NAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

CONFIRM SITE CONI BACKFILL HEIGHT, V	DITIONS, FOUNDATION BEARING CAPACITY, GRADE AND IND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIAL	S AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FECR 2004 R THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINU BELIEVE THE PLAN (OUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU MITS A CONTINUOUS LOAD PATH CONNECTION, CALL INEER IMMEDIATELY.
DESIGN, PLACEMEN	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, DINNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL S.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALER PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

WIND LOADS PER FLORIDA BUILDING CODE 2004	RESIDENTIA	AL, SE	СТІО	N R30	01.2.1		
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH MEAN ROOF HEIGHT NOT EXCEEDING LEAST HOF ON UPPER HALF OF HILL OR ESCARPMENT 60FT I SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HI	H FLAT, HIP RIZONTAL D	PED, O	OR G	ABLE OR 6	ROOFS 0 FT; NO		
BUILDING IS NOT IN THE HIGH VELOCITY HURRICA							
BUILDING IS NOT IN THE WIND-BORNE DEBRIS RE							
1.) BASIC WIND SPEED = 110 MPH							
2.) WIND EXPOSURE = B							
3.) WIND IMPORTANCE FACTOR = 1.0							
4.) BUILDING CATEGORY = II							
5.) ROOF ANGLE = 10-45 DEGREES							
6.) MEAN ROOF HEIGHT = <30 FT							
7.) INTERNAL PRESSURE COEFFICIENT = N/A (EN							
8.) COMPONENTS AND CLADDING DESIGN WIND	PRESSURI	ES (TA	BLE	R301	.2(2))		
	Zone	Zone Effective Wind Area (ft2)					
***************************************	20116	10 100					
	1	19.9	-21.8		-18.1		
2 2	2	19.9 -	-25.5	18.1	-21.8		
5	2 O'hg		40.6		-40.6		
2 2 2 5	3	19.9 -		18.1	-21.8		
3 4	3 O'hg	21.8 -	-68.3	40.5	-42.4		
55	5	21.8 -		18.5	-20.4		
ALL ALL							
2	Doors a	& Windo st Case		21.8	-29.1		
2		5, 10 ft					
A			1	19.5	-22.9		
	8x7 Gara		arage Door		-21.0		
5 2 2 5	8x7 Gara 16x7 Ga		oor	18.5			
5 2 4 5 5 T			oor	18.5	21.0		
5 2 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			oor	18.5	21.0		
5 2 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			oor	18.5	21.0		
5 2 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			oor	18.5	21.0		
2			oor	18.5	21.0		
DESIGN LOADS			oor	18.5			
DESIGN LOADS			oor	18.5			

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reser its common law copyrights and property right i these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express writte permission and consent of Mark Disosway CERTIFICATION: I hereby certify that I hav examined this plan, and that the applicable portions of the plan, relating to wind engineeri comply with section R301.2.1, florida building code residential 2004, to the best of my

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

32056, 386-754-5419

DIMENSIONS:

REVISIONS

SOFTPIAN

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Ewpl, Inc.

Thacker Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: November 18, 2006 DRAWN BY: STRUCTURAL E David Disosway

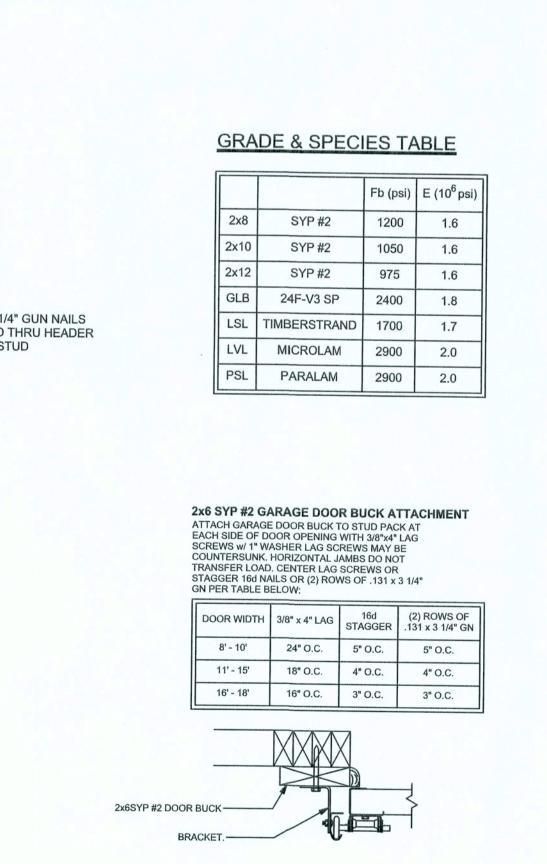
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OF 3 SHEETS

611154

DRAWING NUMBER

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GARAGE DOOR BUCK INSTALLATION DETAIL

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

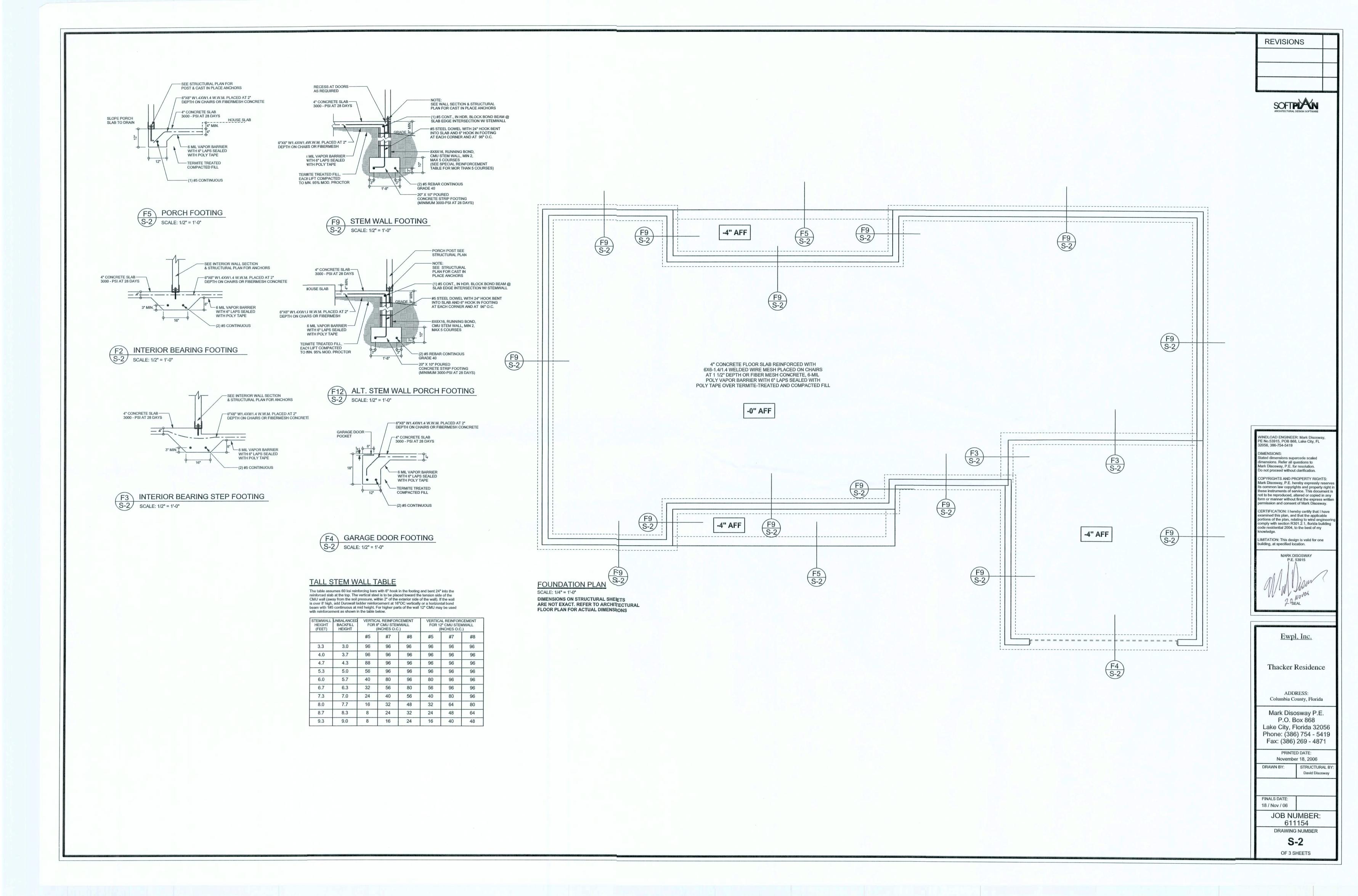
(SEE STRUCTURAL PLAN)

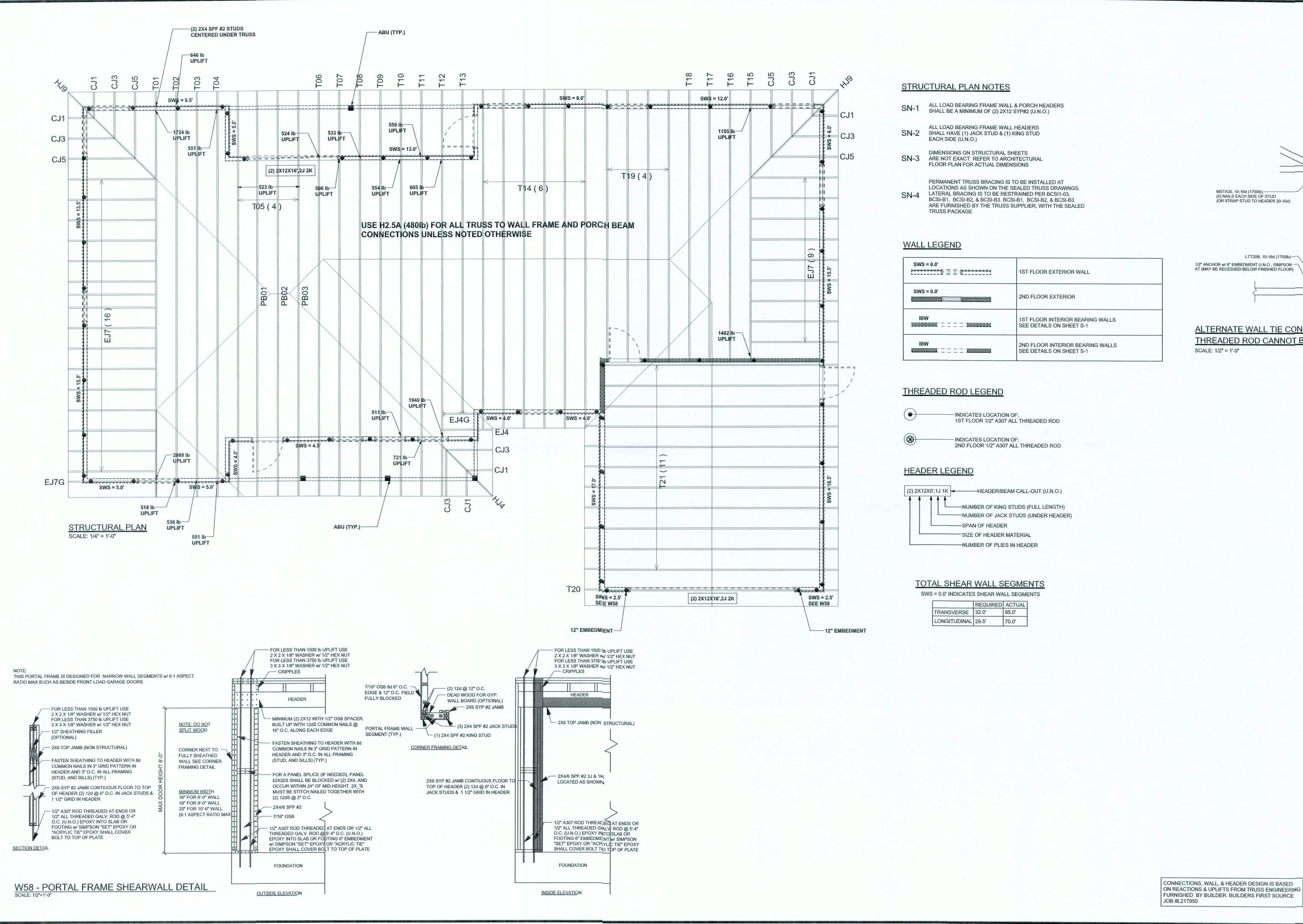
(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O. (2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

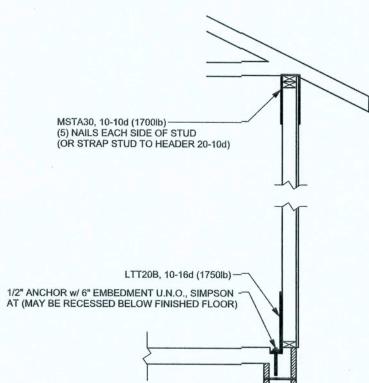
TYPICAL 1 STORY HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"





REVISIONS

SOFTPLAN



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL

> /INDLOAD ENGINEER: Mark Disosway. PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in

not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building

code residential 2004, to the best of my LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY

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PRINTED DATE: November 18, 2006 DRAWN BY: STRUCTURAL BY

FINALS DATE:

David Disosway

18 / Nov / 06 JOB NUMBER: 611154

> S-3 OF 3 SHEETS

DRAWING NUMBER