

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. RUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

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ROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRES LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >109 SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

Zone Effective Wind Area (ft2)

10 100

1 27.8 -30.5 25.3 -25.3

2 27.8 -35.7 25.3 -30.5

3 27.8 -35.7 25.3 -30.5 3 O'hg -95.6 -59.3

4 30.5 -33.0 25.9 -28.5

5 30.5 -40.7 25.9 -31.6

Doors & Windows 30.5 -40.7

6x7 Garage Door | 25.9 | -29.4

O'hg -56.8

Worst Case

(Zone 5, 10 ft2)

7 Garage Door

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

WIND IMPORTANCE FACTOR = 1.0

REVISIONS evised egineering for 3/8" rod wall achors 2011-01-05



Revision

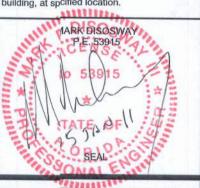


NDI OAD NGINEER: Mark Disosway, E No.53915POB 868, Lake City, FL 32056, 386-74-5419 tated dimenons supercede scaled dimensions. Pfer all questions to Mark Disoswa, P.E. for resolution.

Do not proced without clarification. COPYRIGHT: AND PROPERTY RIGHTS: Mark Disoswa, P.E. hereby expressly resen its common la copyrights and property right i hese instrumnts of service. This document not to be reprduced, altered or copied in any orm or manns without first the express written permission an consent of Mark Disosway. ERTIFICATION: I hereby certify that I have

examined this lan, and that the applicable portions of the lan, relating to wind engines comply with sction R301.2.1, florida building code residentil 2007, to the best of my

LIMITATION: his design is valid for one building, at spcified location.



Mike Ball Residence

ADDRESS: 790SW Barwick Terr. LakeCity, FLorida 32024

Mark Disosway P.E. PO. Box 868 Lake Cty, Florida 32056 Phone (386) 754 - 5419 Fax: \(\beta 86 \) 269 - 4871

RINTED DATE: Jauary 05, 2011 STRUCTURAL BY DRAWN B* David Disosway

FINALS DA'E: 17Mar10

> JOS NUMBER: 1003020 DRWING NUMBER

> > **S-1 IF 3 SHEETS**

8"CMU LINTEL BLOCK
BOND BEAM
W/IEA. *5 REBAR
POURED SOLID

DRAWN BY: MD

DATE: 4/022010 PROJECT NO MB042010

SHEET NUMBER:

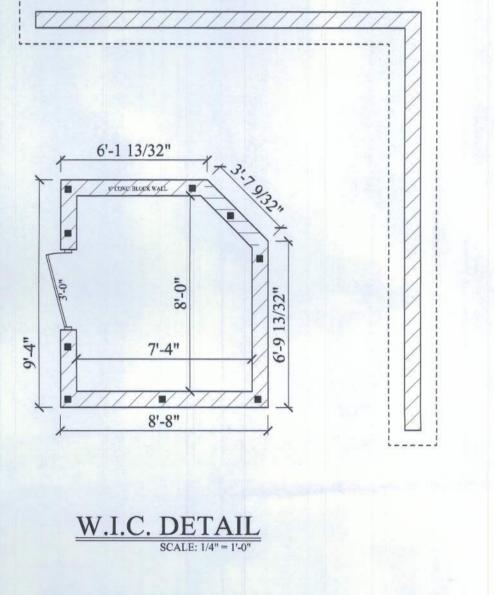
EXISTING 24X38 BUILDING W.I.C. DETAIL
SCALE: 1/4" = 1'-0"

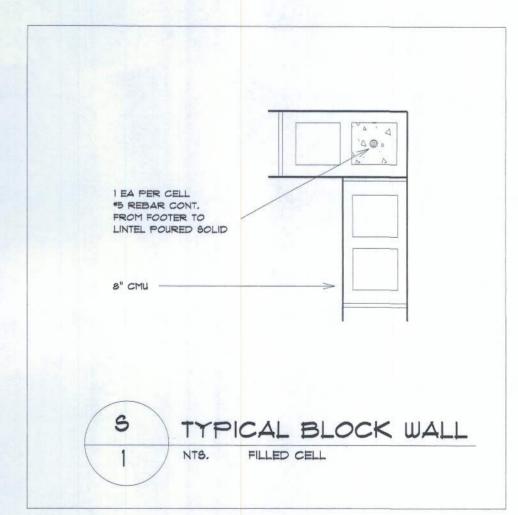
153.40'

SITE AN

MIKE BALL 790 S.W. BARWICK TERR. LAKE CITY, FL. 32024

S.W. BARWICK TERR. PAVED COUNTY ROAD



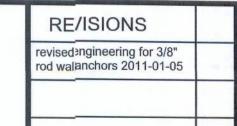


8" CMU WALL W/ *5 REBAR DOWL POURED SOLID AT EACH REBAR LOCATION

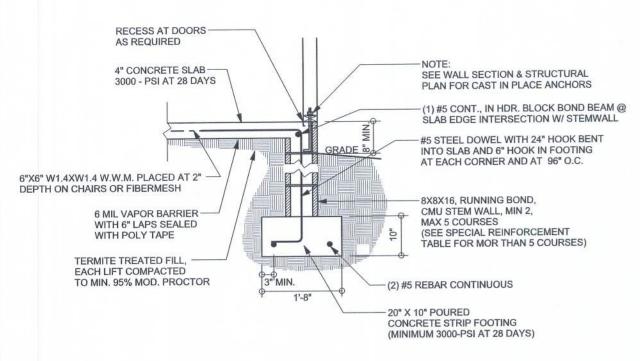
S

8" X 16" CONT. CONCRETE MONO FOOTER REINF, W/2 *5 REBAR LAPPED AND TIED ON CHAIRS

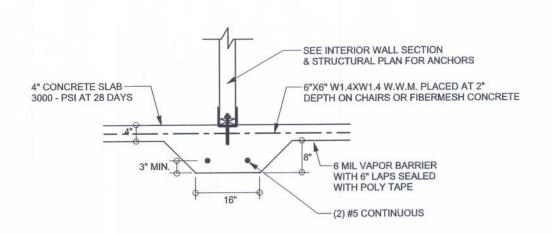
W.I.C. FOOTER.



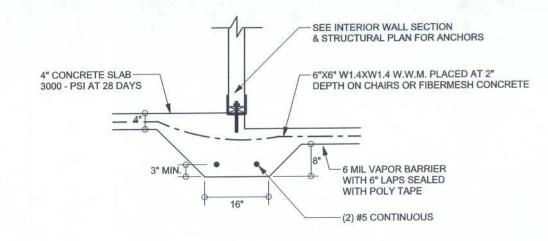




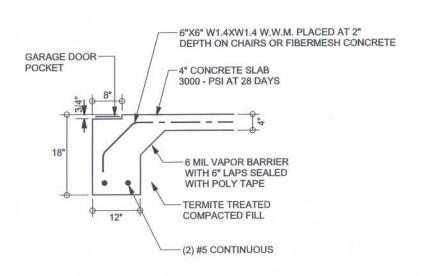
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



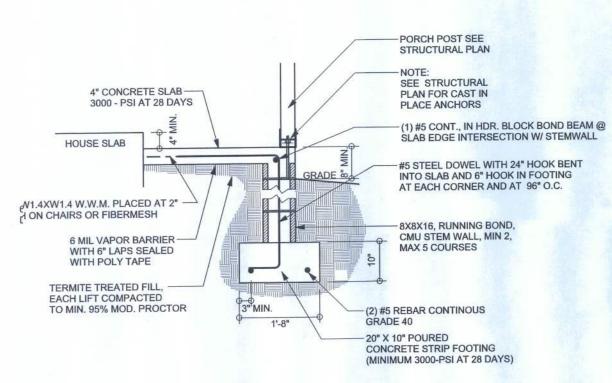
F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

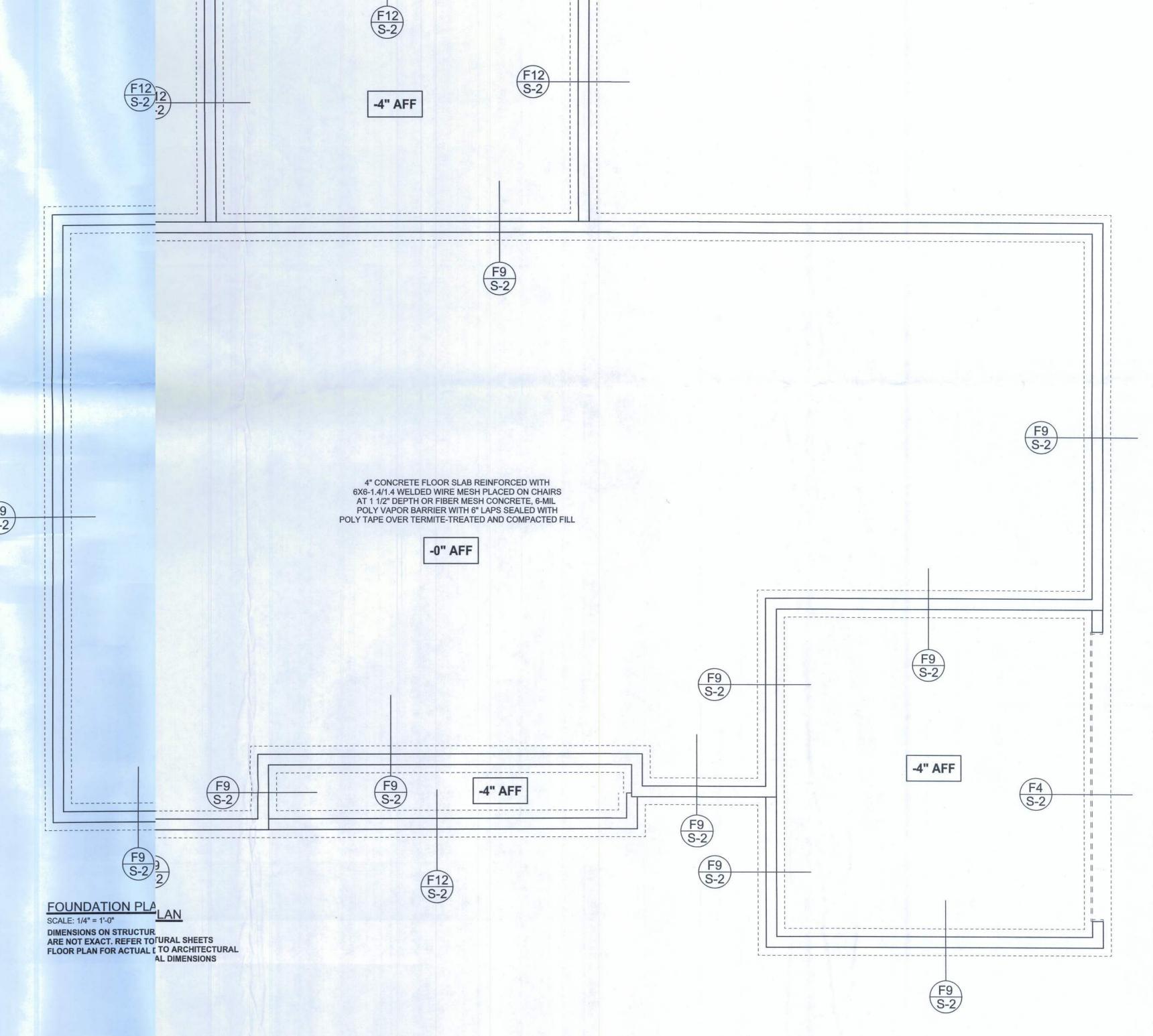


F12 STEM WALL PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLOA) ENGINEER: Mark Disosway, PE No.5345, POB 868, Lake City, FL 32056, 38-754-5419

DIMENSI(NS: Stated dimensions supercede scaled dimension. Refer all questions to Mark Disoway, P.E. for resolution. Do not preed without clarification.

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CERTIFICTION: I hereby certify that I have examined is plan, and that the applicable portions othe plan, relating to wind engineering comply with section R301.2.1, florida building code residntial 2007, to the best of my

corde residntial 2007, to the best of my knowledge

LIMITATI(N: This design is valid for one building, aspecified location.

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MARK DISOSWAY
P.E. 53915

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90 SW Barwick Terr.
Lke City, FLorida 32024

Mark Disosway P.E. P.O. Box 868 Lakı City, Florida 32056 Phoie: (386) 754 - 5419 Fa: (386) 269 - 4871

PRINTED DATE:
January 05, 2011

DRAW BY: STRUCTURAL BY:
David Disosway

FINAL: DATE: 17Ma10

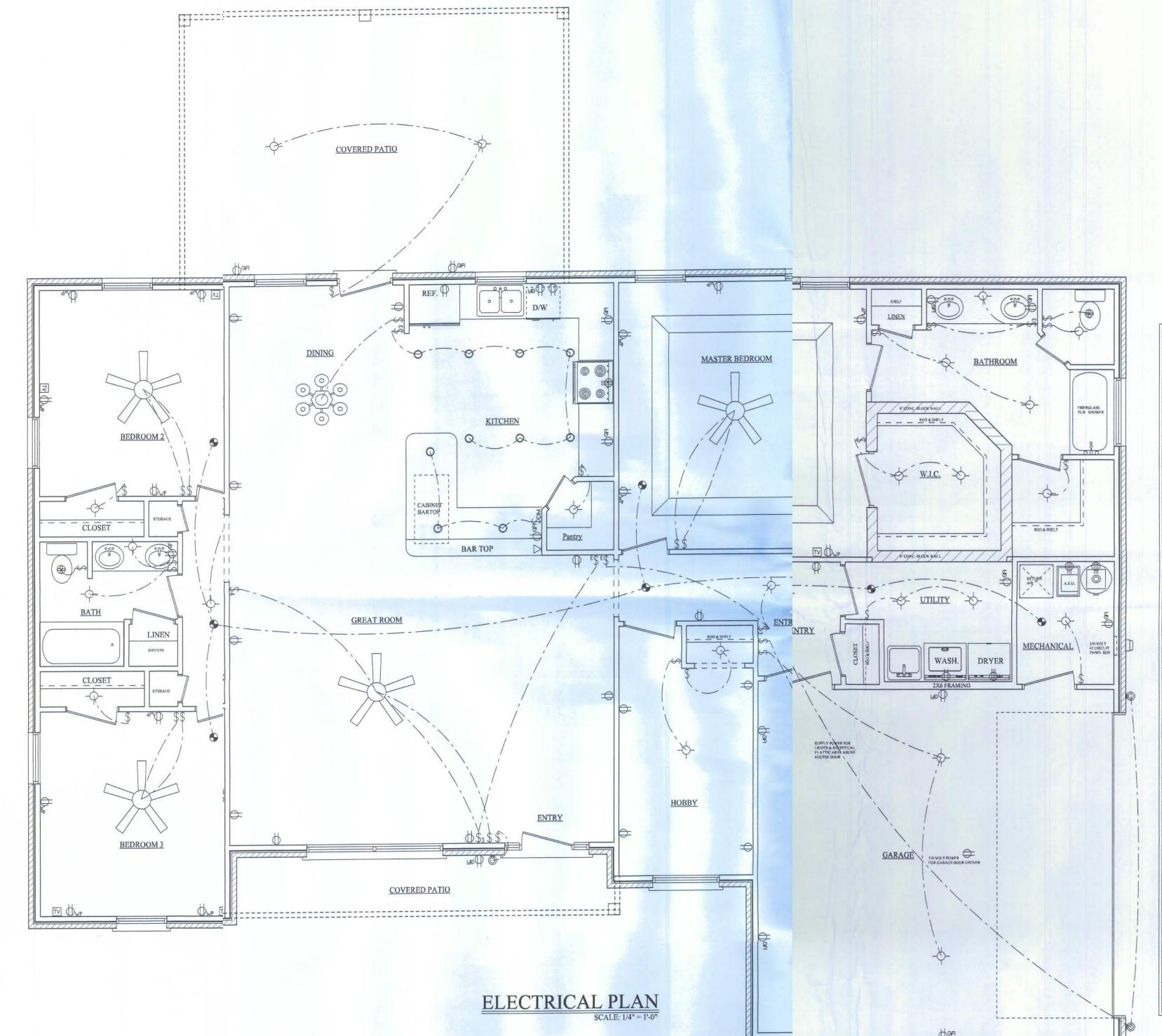
OB NUMBER:
1003020
DRAWING NUMBER

S-2 OF 3 SHEETS

DATE: 4/022010

PROJECT NO:MB042010

SHEET NUMBER:



ELECTRICAL	COUNT	SYMBOL
ceiling fan w/light	4	
chandelier	1	0 <u>0</u> 0 000
recessed light	10	0
exterior light	3	Q
electrical meter	1	8
electrical panel	1	taar
cable tv outlet	5	īν
dimmer switch	1	\$ _{DM}
exhaust fan	2	⊕
light	23	-
outlet	19	Ф
outlet 220v	4	Ф
outlet gfi	16	∯ gFl
outlet afi	15	Øw₽
smoke detector	7	•
switch	27	\$
switch 3 way	9	\$3
switch 4 way	1	\$4
telephone	- 1	∇

ELECTRICAL NOTES:

ELECTRICAL CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELECTRICAL PLAN, ADDITIONS TO THE ELECTRICAL PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CIRCUITS IDENTIFIED W/ CIRCUIT NUMBER, DESCRIPTION & BREAKER. SERVICE ENTRANCE & ALL UNDERGROUND WIRE LOCATIONS RISER DIAGRAM SHALL INCLUDE WIRE SIZES/TYPE AND EQUIPMENT TYPE W/RATINGS & LOADS. ALL WET LOCATIONS SHALL BE PROTECTED BY GFCI PROTECTION AND ALL BEDROOM RECEPTS SHALL BE PROTECTED BY AFCI PROTECTION.

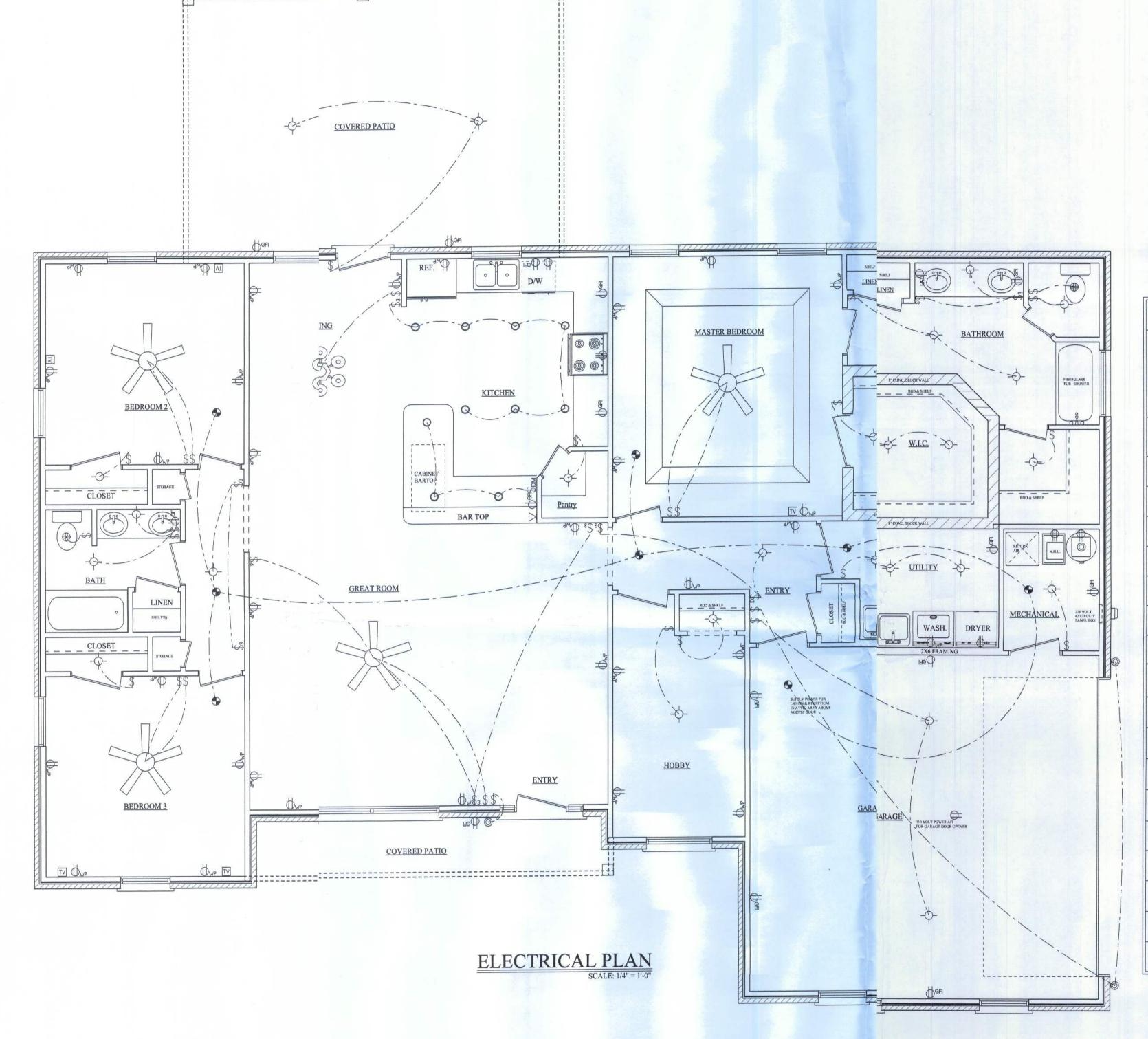
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ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER.

ALL BEDROOMS RECEPTS SHALL BE AFCI.

MIK

SHEET NUMBER:



ELECTRICAL	COUNT	SYMBOL		
ceiling fan w/light	4			
chandelier	1	080 080		
recessed light	10	0		
exterior light	3	Q		
electrical meter	1	a		
electrical panel	1			
cable tv outlet	5	TV		
dimmer switch	1	\$ _{DM}		
exhaust fan	2	₩		
light	23			
outlet 110v	2	Ф		
outlet 220v	4	Φ		
outlet gfi	16	⊕ GFI		
outlet afi	34	Øwp.		
smoke & carbon monoxide detector	8	•		
switch	27	\$		
switch 3 way	9	\$3		
switch 4 way	1	\$4		
telephone	1	∇		

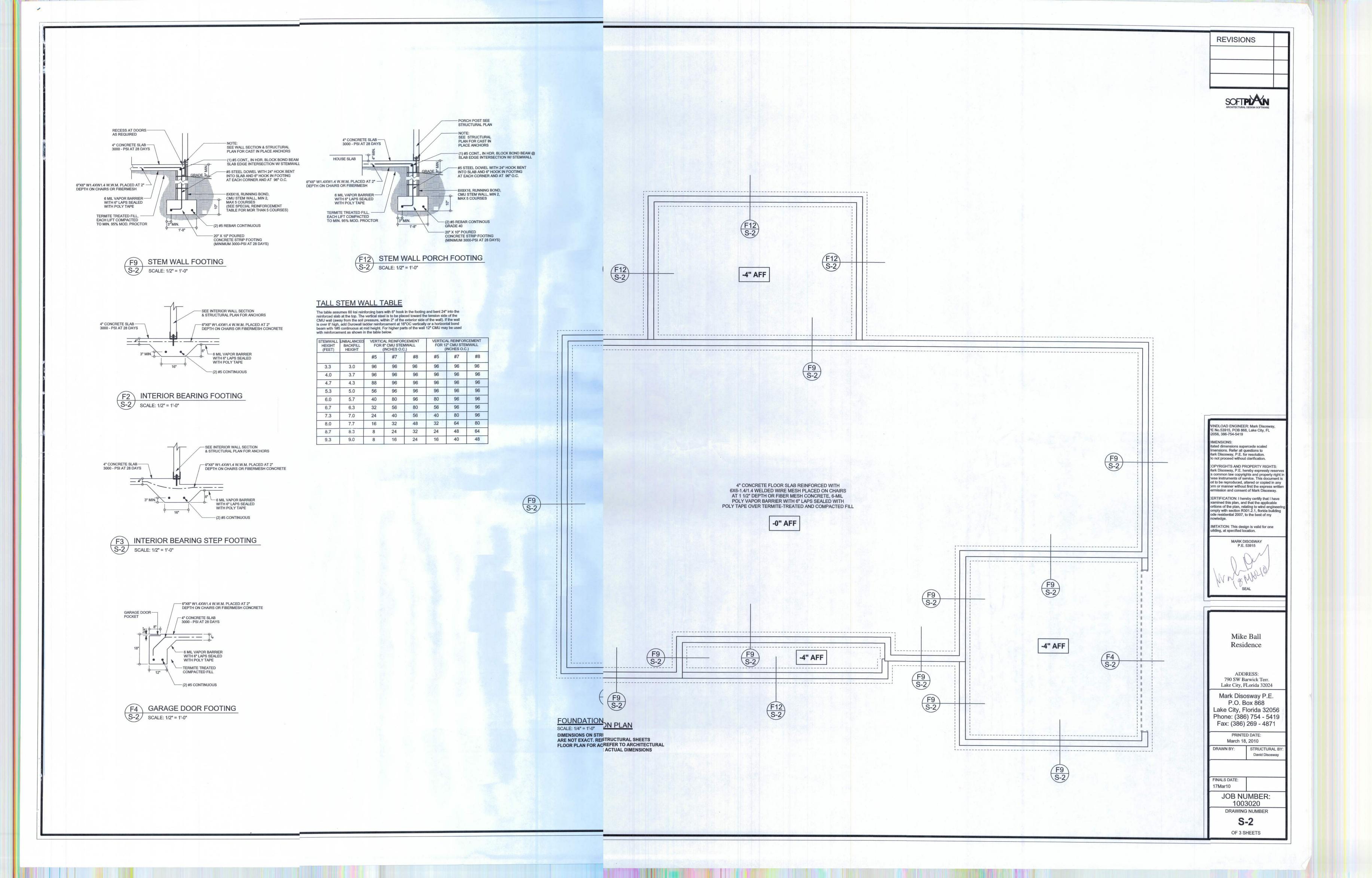
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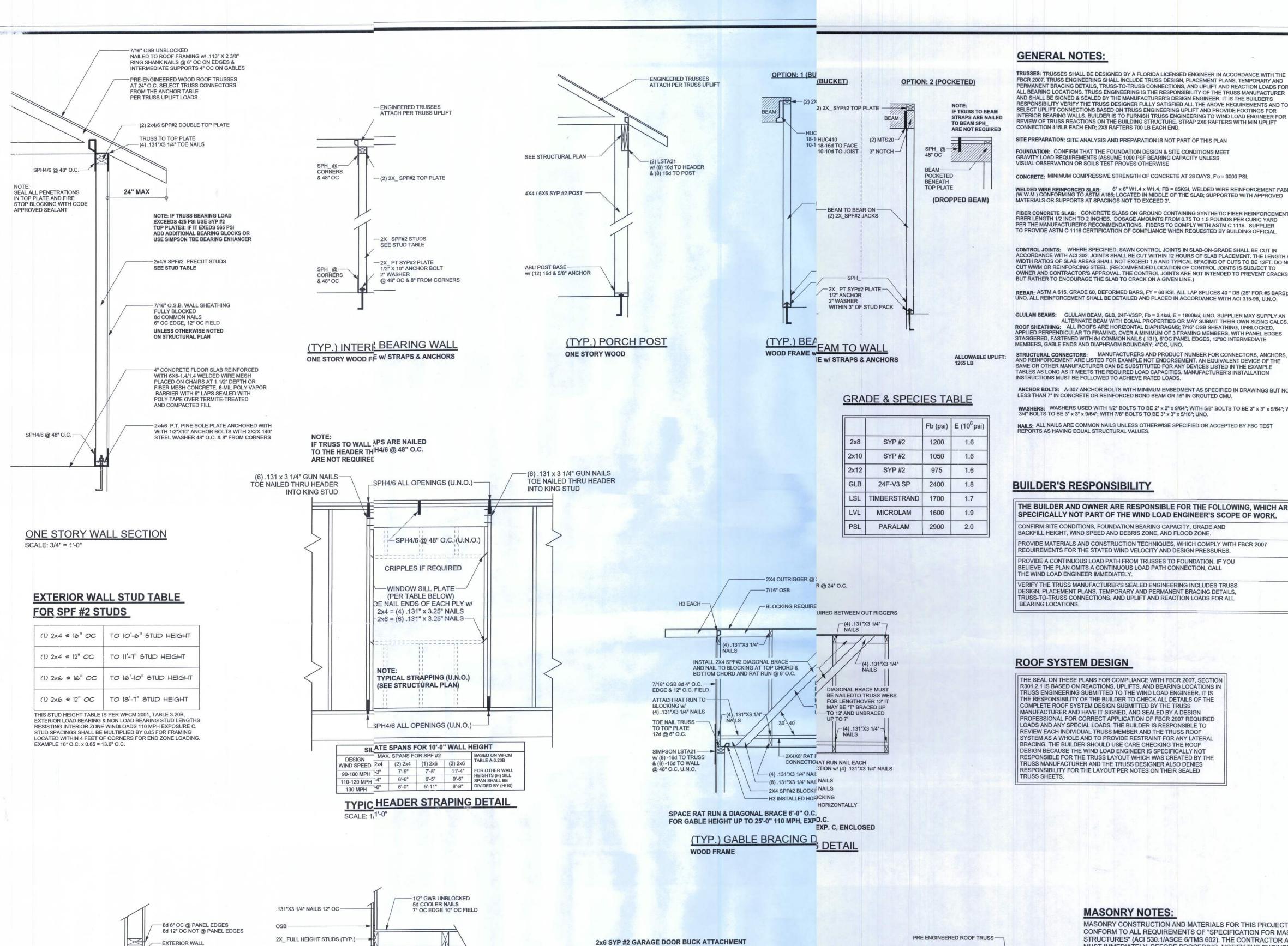
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8d 6" OC @ PANEL EDGES -

8d 6" OC @ PANEL EDGES -

.131"X3 1/4" NAILS 6" OC -

INTERIOR SHEARWALL-

1/2" GWB UNBLOCKED -

7" OC EDGE 10" OC FIELD

d COOLER NAILS

WOOD FRAME

.131"X3 1/4" NAILS 12" OC -

-8d 12" OC NOT @ PANEL EDGES

OSB-

(TYP.) INTERSECTING WALL FRAMING

-8d 6" OC THIS STUD

FOR SHEAR TRANSFER

- 8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

-2X_FULL HEIGHT STUDS (TYP.)

-8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES ---

2X_FULL HEIGHT STUDS (TYP.)

.131"X3 1/4" NAILS 12" OC -

CIDE CORNER

ILE CORNER

1/2" GWB UNBLOCKED

-8d 6" OC @ PANEL EDGES

8d 12" OC NOT @ PANEL EDGES -

5d COOLER NAILS 7" OC EDGE 10" OC FIELD

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

GN PER TABLE BELOW:

8' - 10'

11' - 15'

2x6SYP #2 DOOR BUCK -

SCALE: N.T.S.

BRACKET. -

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

16' - 18' 16" O.C. 3" O.C.

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH 3/8" x 4" LAG STAGGER .131 x 3 1/4" GN

24" O.C. 5" O.C.

18" O.C. 4" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

5" O.C.

4" O.C.

3" O.C.

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	CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 STATED WIND VELOCITY AND DESIGN PRESSURES.	
	LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU A CONTINUOUS LOAD PATH CONNECTION, CALL R IMMEDIATELY.	
DESIGN, PLACEMENT PLAN	FACTURER'S SEALED ENGINEERING INCLUDES TRUSS NS, TEMPORARY AND PERMANENT BRACING DETAILS, CTIONS, AND UPLIFT AND REACTION LOADS FOR ALL	

ROOF SYSTEM DESIGN

DOUBLE 2x4 SPF TOP PLATE NAILED -

FOGETHER W/2-16d NAILS AT 16" O.

4' MIN. LAP w/ (12) - 16d OR 4" LAP w/

CS20 w/ (4) - 16d &(14) - 10d

SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4

CONTINUOUS FRAME TO

CEILING DIAPHRAGM DETAIL

SPF NAILED TO TOP AND BOTTOM PLATES

WITH 2-16d NAILS

SCALE: N.T.S.

INTERIOR CEILING AS -

BOTTOM CHORD OF TRUSS

CONTINUOUS FRAME

TO TOP PLATE AT

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approval			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F Coating for corrosion protection		Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7 Movement joints		Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.			

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS

< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	Н3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	1.71
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	13000
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d	16	
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d	19.	
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

PE No 3915, POB 868, Lake City, FL 32056386-754-5419

DIMENIONS:

FEVISIONS

dimenons. Refer all questions to Mark Dosway, P.E. for resolution o not roceed without clarification COPYIGHTS AND PROPERTY RIGHTS: Mark Dosway, P.E. hereby expressly reserv these istruments of service. This document is not to b reproduced, altered or copied in any form omanner without first the express written

permision and consent of Mark Disosway. CERTIICATION: I hereby certify that I have examind this plan, and that the applicable portion of the plan, relating to wind engineer complyvith section R301.2.1, florida building code reidential 2007, to the best of my



Mike Ball Residence

ADDRESS: 790 SW Barwick Terr. Lake City, FLorida 32024

Mark Disosway P.E. P.O. Box 868 Lae City, Florida 32056 Phne: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March 18, 2010 STRUCTURAL B DRWN BY: David Disosway

FINLS DATE: 17Nar10

JOB NUMBER: 1003020 DRAWING NUMBER

OF 3 SHEETS

DESIGN DATA

100							
2.) WI	ND EXPOSURE = C			75-1			
3.) WI	ND IMPORTANCE FACTOR = 1.0				-		
4.) BL	ILDING CATEGORY = II						
5.) RC	OF ANGLE = 10-45 DEGREES						
6.) ME	AN ROOF HEIGHT = <30 FT						
	FERNAL PRESSURE COEFFICIENT = N/A (E	ENCLOSED	BUILD	ING)		1	
	MPONENTS AND CLADDING DESIGN WIN				2301.2/	211	
		- , , , , , ,	1120 (1	, were	1001.2(-//	
	*	Zone Effective Wil			nd Area (ft2)		
			1	0		100	
		1	27.8	-30.5	25.3	-25.3	
1	2 2	2	27.8	-35.7	25.3	-30.5	
5	7	2 O'hg		-56.8		-56.8	
2	2 2 2 5	3	27.8	-35.7	25.3	-30.5	
	4 3	3 O'hg		-95.6		-59.3	
		4	30.5	-33.0	25.9	-28.5	
,		5	30.5	-40.7	25.9	-31.6	
	/3	Doors	& Wind	dows	30.5	-40.7	
/	3	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	st Cas	50 July 200			
n		(Zone	5, 10	ft2)			
5	2 3	8x7 Gar	age D	oor	27.3	-32.0	
2	4 /2/ 5	16x7 Ga	arage [Door	25.9	-29.4	
	4 1						
	55 \$2						

DESIGN	LOADS						
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS	5)			-		
	30 PSF (SLEEPING ROOMS)						
	30 PSF (ATTICS WITH STORAGE)						
	10 PSF (ATTICS WITHOUT STORAGE, <	3:12)					
ROOF	20 PSF (FLAT OR <4:12)						
	16 PSF (4:12 TO <12:12)						
	12 PSF (12:12 AND GREATER)		-				
OTAIDO	40 DOE (01)E 4 THE ELLIN (11)						

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

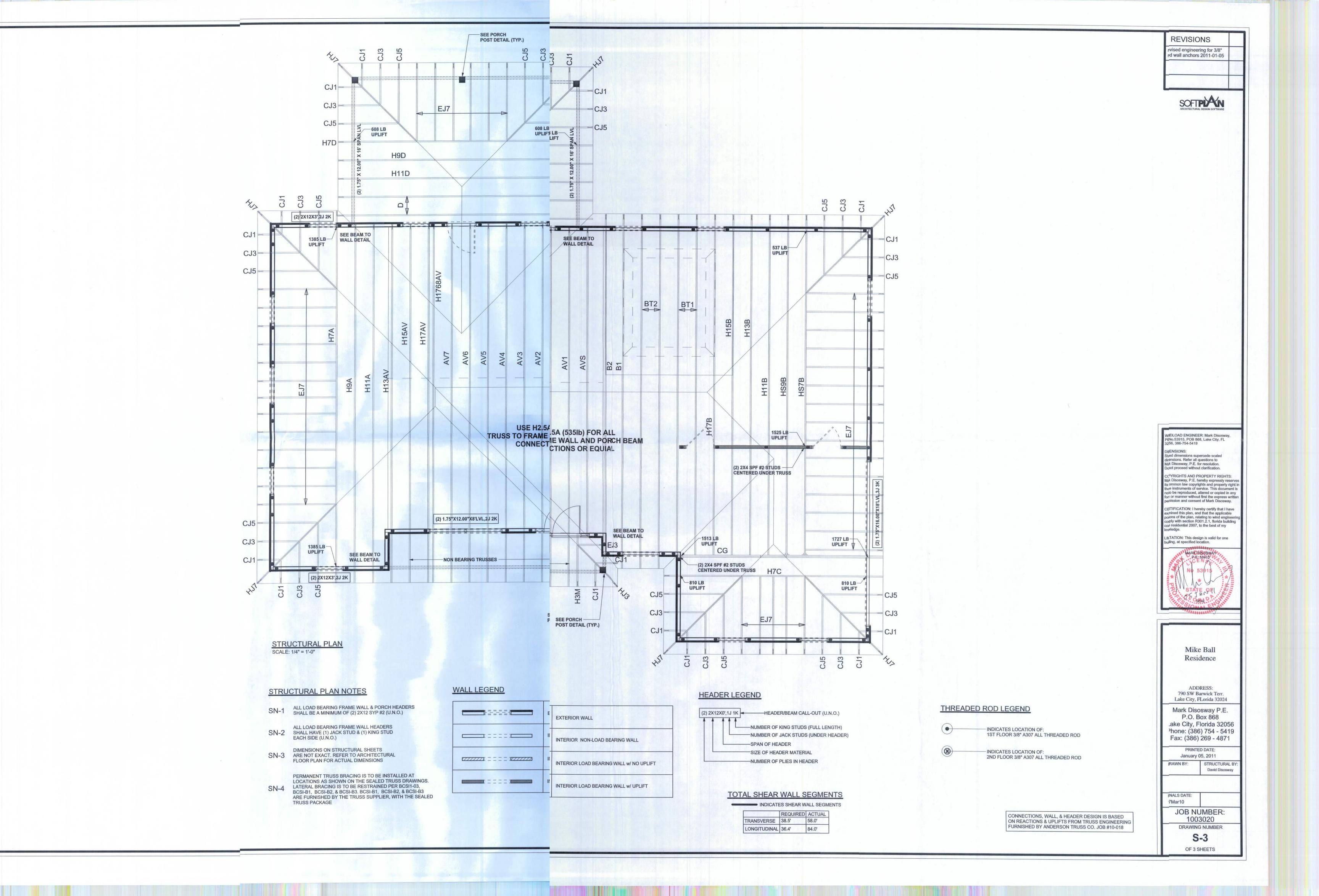
SOIL BEARING CAPACITY 1000PSF

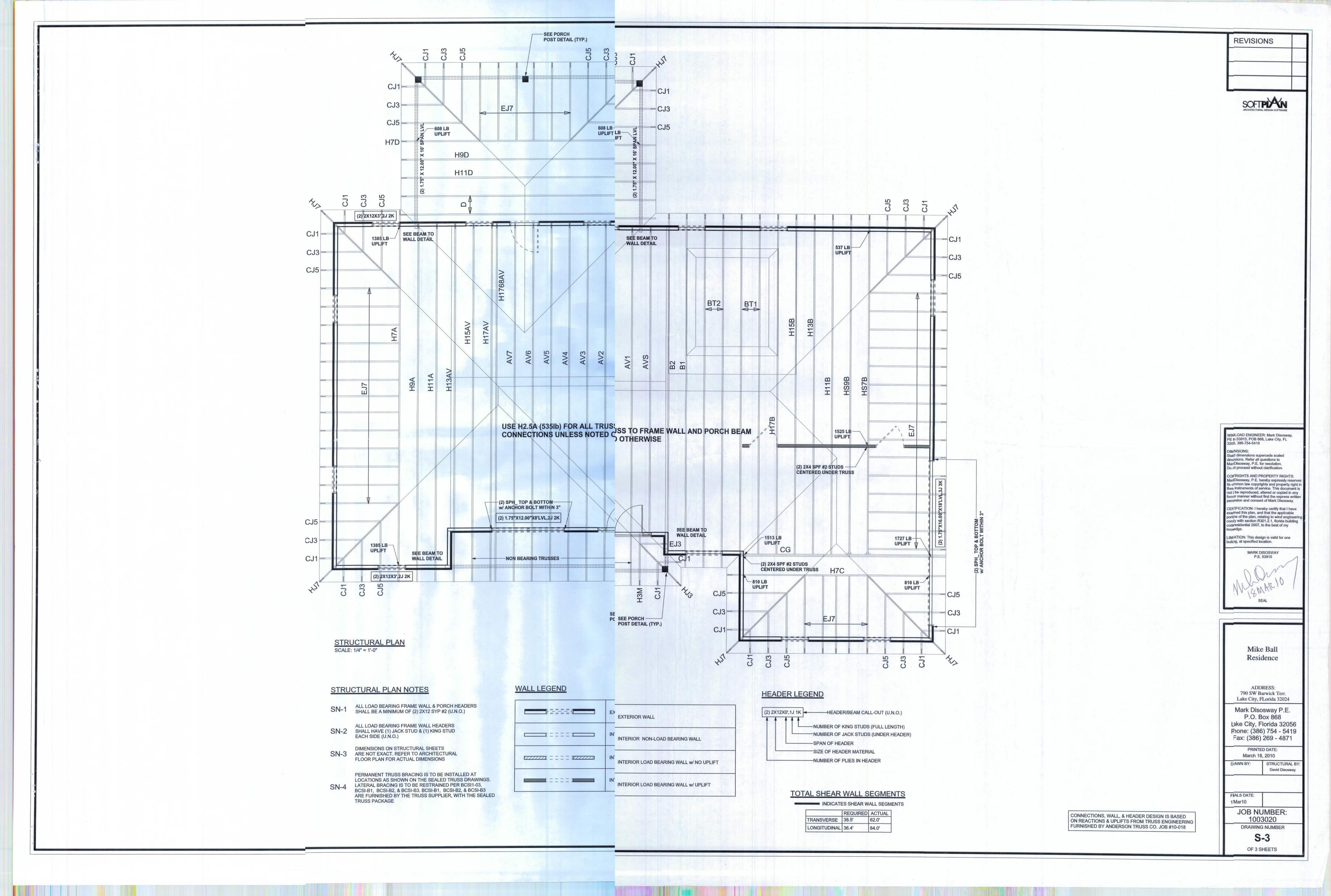
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

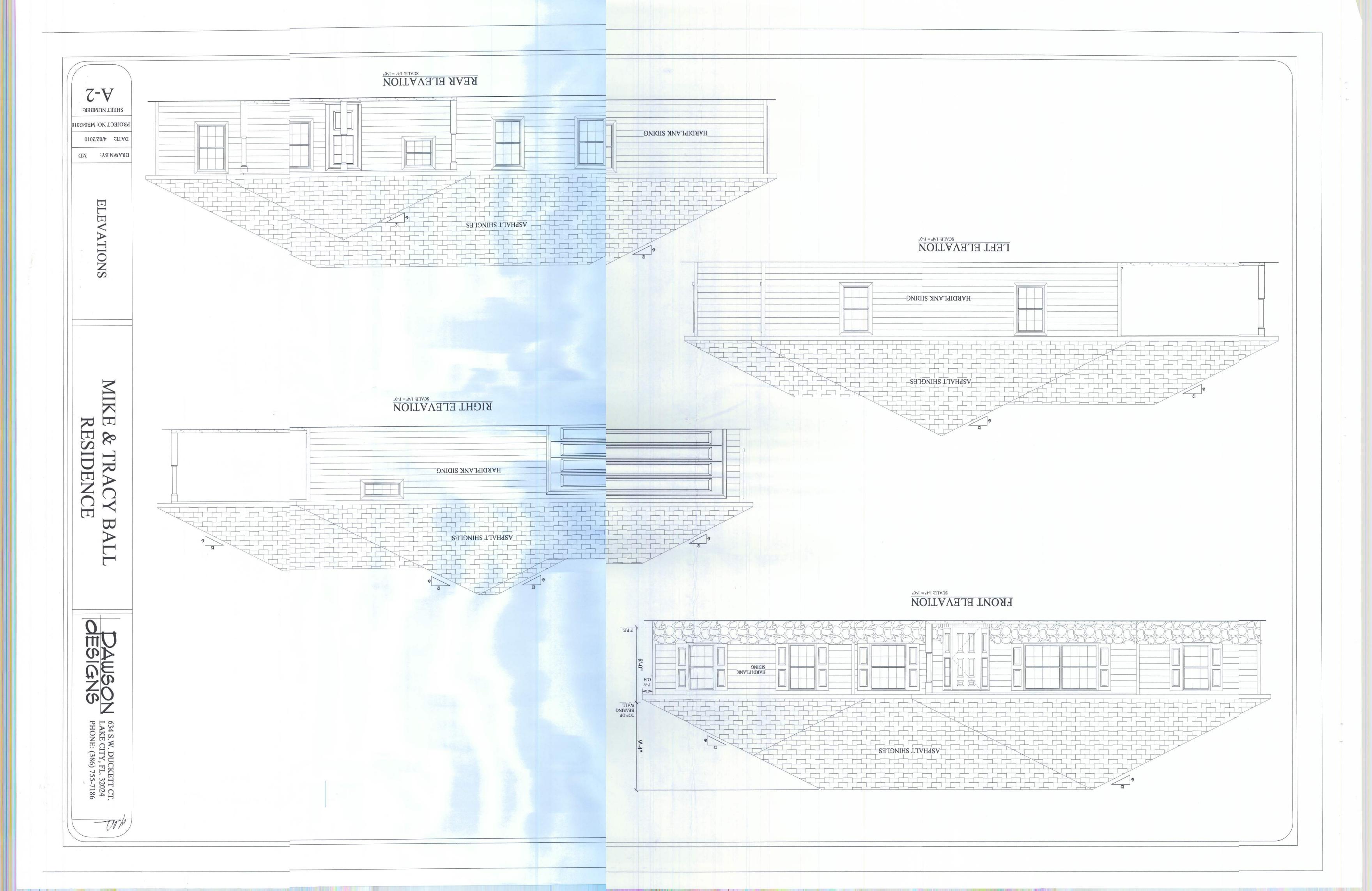
1.) BASIC WIND SPEED = 110 MPH

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)







PROJECT NO MB042010

SHEET NUMBER:

