

WIND ANALYSIS -- 120MPH Wind Velocity or as interpolated

2023 8th edition Florida Building Code

Calculations as per Section 1609ASCE 7-22

Prepared By
James Zaleski PE 51544

Prepared by (print legibly): James Zaleski
Design Professional FL Lic. #: 51544

Importance factor: 1.0 Building Category: ENCLOSED
Wind Exposure (s): B Risk Category II
Internal Pressure Coefficient +/- .18

Mean Roof Height 16.47 End Zone Length 6.0 feet

MAX OVERHANG 1.5 FT MAX

MANUFACTURED TRUSSES TO BE USED

Roof Slope = -4/12 - 7/12



TRUSS SPAN/LOCATION HURRICANE CLIPS
HC MODEL-1 Simpson H-10A IN ALL AREAS

ROOF SHEATHING MATERIAL - 7/16" OSB
NAILING - 8D RING SHANK

NAILING PATTERN
EDGES-

6" O.C FIELD - 6" O.C

4" O.C FIRST ROW AT ALL EAVES

Plan May Be Mirrored at Contractors Option

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Wall Exterior Panel – Sheath with 7/16” OSB
– 2 X 4 STUDS AT 16” O.C. UP TO 10 FEET

– 2 X 4 STUDS AT 12” O.C. UP TO 12 FEET

– 2 X 6 STUDS AT 16” O.C. UP TO 16 FEET

ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

SEE ATTACHED DETAILS

POSTS USE SIMPSON ABU BASE WITH 2-LSTA24 STRAPS AT TOP AND 2 SIMPSON SDWC 15600 SCREWS FROM POST TO BEAM

MIN NAIL PENETRATION – 1-1/2”

Nail Type 8D

Edge Nail Spacing 4” o.c

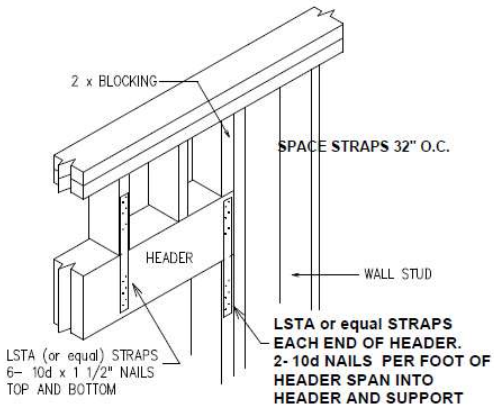
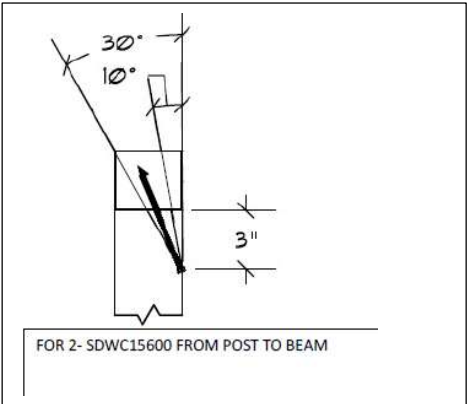
Intermediate Nail Spacing 8” o.c

SIMPSON SDWC15600 SCREWS AT THE TOP OF STUDS AND SIMPSON SDWC15450 SCREWS AT THE BOTTOM OF STUDS AT ALL CORNERS AND 48” O.C

SIMPSON SPH STRAPS MAY BE USED IN LIEU OF SCREWS

BEAM TO WALL/CORNER CONNECTION – POCKET AND NAIL INTO WALL W/ (10) 16 PENNY NAILS, STRAP W/ SIMPSON H7Z.

10" 'J' BOLT MINIMUM 48" O.C. w/ 3 GA. x 3" x 3" BEARING PLATE



HEADER CONN.

James Zaleski PE #51544 2305 Haverhill Rd Tall Fl 32312 ph 850-766-7778

USE (3) 2x HEADERS WITH 2x6 WALLS AND (2) 2x HEADERS WITH 2x4 WALLS.
VERIFY HEADER SIZE W/ OPENING FRAMING & HEADER SCHEDULE ON THIS SHEET.
ASSEMBLE ALL HEADERS WITH 16D NAILS STAGGERED 6” O.C. W/ 2” EDGE DISTANCE
ALL HEADERS SHALL BE 2x12 U.O.N.
ATTACH ALL LVL BEAMS W/ SIMPSON SDWS 0.220 SCREWS (OR EQUAL) STAGGERED W/ 1-1/2” MIN. EDGE DISTANCE.

- (2) ROWS @ 24” O.C. < 10” LVL
- (3) ROWS @ 24” O.C. > 10” LVL
- 2 & 3 PLY – 3-1/2” LONG STAGGER INSTALLED FROM BOTH OUTER PLIES
- 4 PLY – 6” LONG STAGGER INSTALLED FROM BOTH OUTER PLIES

FOR WINDOWS PLACED WITHIN 3’-0” OF EXTERIOR CORNERS, WALL STUDS SHALL BE SPACED @ 8” O.C.

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COMPONENTS AND CLADDING PRESSURES: (WORST CASE LOADS MAY BE USED)

COMPONENTS AND
CLADDING

ZONE per

SEE ATTACHED

MAIN WIND FORCE RESISTING SYSTEMS (MWFRS) (WORST CASE LOADS MAY BE USED)

SEE ATTACHED

All Load Bearing and Shear Walls To be Framed as per FBC
Alternative Hurricane Clips are acceptable as long as they meet the requirements shown

See Attached header schedule

PROVIDE GABLE END BRACING DETAIL, all vaulted or high ceilings shall be balloon framed to the ceiling diaphragm.

NOTES: PLEASE READ & complete all blanks!!!!

- 1. See floor plan for wall bracing locations or circle 100% if structural sheathing is required on all exterior walls, with the nailing pattern indicated above.
- 2. There are , there are not X interior shear walls, locate interior shear walls on plan.
- 3. Gable ends required to be sheathed with same material as shear wall? Yes or No (circle one)
- 4. Wall sheathing used in lieu of vertical straps: Nailing @ N/A o.c. along top & bottom plates
- 5. Provide detail for 2 story bldgs showing continuous load path between 2nd floor stud & 1st floor studs.
- 6. Provide additional information for column base & column/beam connection if required for porches.
- 7. Provide calculations or documentation to substantiate method used as an attachment to this form(SEE PLANS)

Instructions:

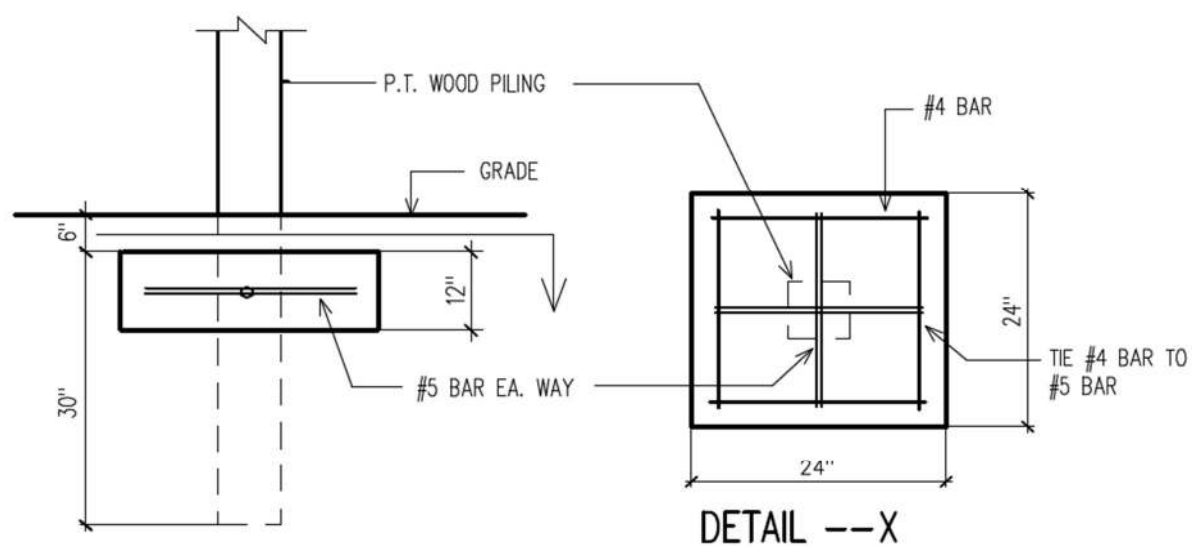
- 1. The form should be completed & signed, sealed & dated by a Fla. licensed engineer or architect.
- 2. Since more than one methodology for determination of wind forces is permitted under Section 1609ASCE7-22, to comply with State Building Codes a space has been provided to indicate method used.
- 3. Wind Analysis Forms submitted & permitted to be used as Master Plans will be for identical plans only, minor deviations such as door swings. Any deviation from the exterior form, opening sizes or locations will not be permitted unless noted by the design professional.

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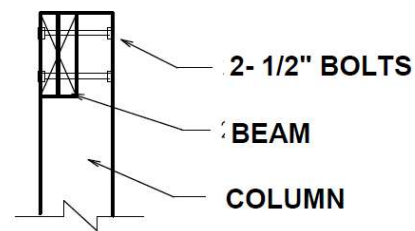
ALL JOIST HANGERS SHALL BE
SIMPSON LU26 HANGER OR EQUAL.

DECKING TO BE 5/4 P.T. WOOD OR BETTER
USE 2- 1/4"x 2" DECK SCREWS PER JOIST
OR (2- 10d x 3" RING SHANK NAILS)

ABOVE IS MINIMUM REQUIREMENTS FOR THE
RESISTANCE OF WIND PRESSURES



COLUMN BASE DETAIL



BEAM TO COLUMN CONNECTION

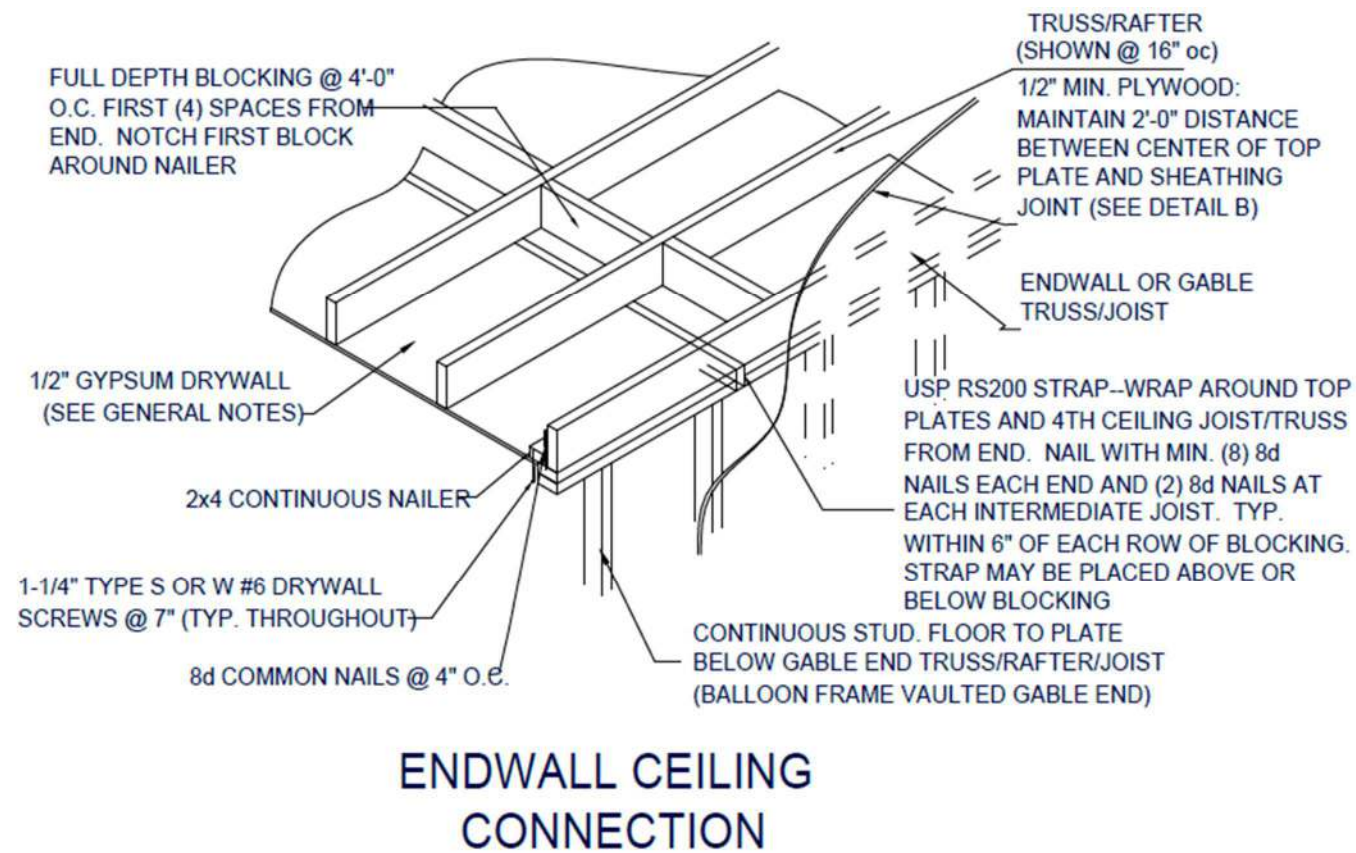
TYPICAL DECK CONNECTION DETAILS

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Header Table		
Span (FT.)	Header Size	(2 x) cripples per end
0' - 4'-0"	2-2x10 w/7/16" OSB Flitch Plate	1
4'-0" - 9'-4"	2-2x10 w/7/16" OSB Flitch Plate	2
9'-4" - 12'-0"	2-2x10 w/7/16" OSB Flitch Plate	3
12'-0" - 15'-4"	3 1/2" X 11 7/8" LVL (or equal)	3

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MecaWind v2502

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Calculations Prepared by:

JAMES ZALESKI P.E. 51544
2305 HAVERHILL RD
TALLAHASSEE, FL, 32312
Date: Apr 28, 2025

File Location: Current Project Not Saved

General:

Reference Abbreviations: T: Table, F: Figure, E: Equation, S: Section

Wind Load Standard	=	ASCE 7-22	Basic Wind Speed	=	120.0 mph
Exposure Classification	=	B	Risk Category	=	II
Structure Type	=	Building	Design Basis for Wind Pressures	=	ASD
MWFRS Analysis Method	=	Ch 27	C&C Analysis Method	=	Ch 30 Pt 1
Dynamic Type of Structure	=	Rigid	Show Advanced Options	=	False

Building:

Roof = Roof Type	=	Gabled	Encl = Enclosure Classification	=	Enclosed
Help = Help on Building Roof Type	=	Help	R _{ht} = Ridge Height	=	23.933 ft
E _{ht} = Eave Height	=	9.000 ft	W = Building Width	=	51.200 ft
L = Building Length	=	50.250 ft	Pitch = Pitch of Roof	=	6.61 :12
θ = Slope of Roof	=	28.857 °	OH = Overhang Configuration	=	All Soffit
Par = Parapet	=	None	Z _i = Highest Opening Elevation	=	0.0000 ft
HT _{over} = Override Mean Roof Height	=	False	Ht _{man} = Mean Roof Height	=	16.467 ft
RA _{over} = Override Roof Area	=	False	GC _{pi_o} = Override GC _{pi} value	=	False
IsElev = Building is Elevated	=	False			

Exposure Constants [T:26.11-1]:

α = 3-s Gust-speed exponent	=	7.500	Z _g = Nominal Ht of Boundary Layer	=	3280.000 ft
â = Reciprocal of α	=	0.133	b = 3 sec gust speed factor	=	0.840
α _m = Mean hourly Wind-Speed Exponent	=	0.222	b _m = Mean hourly Windspeed Exponent	=	0.470
c = Turbulence Intensity Factor	=	0.300	ε = Integral Length Scale Exponent	=	0.3333

Overhang Inputs:

Std	=	Overhangs on all sides are the same	=	True
OHType	=	Type of Roof Wall Intersections	=	Soffit
OH	=	Overhang of Roof Beyond Wall	=	1.500 ft

Main Wind Force Resisting System (MWFRS) Wind Calculations per Ch 27

REVIEWED BY JAMES ZALESKI P.E.

2305 HAVEFRHILL RD

TALLAHASSEE, FL 323132

PH 850-766-7778



James A
Zaleski

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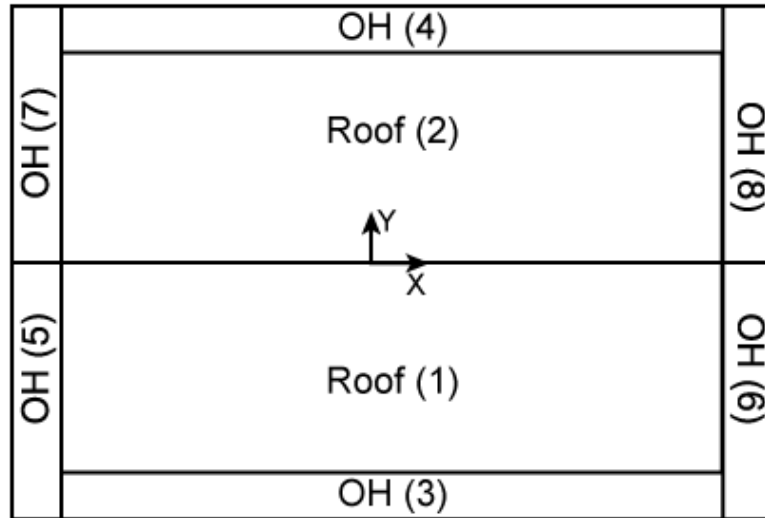
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Wind Parallel
to Ridge
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REVIEWED BY JAMES ZALESKI P.E.
2305 HAVEFRHILL RD
TALLAHASSEE, FL 323132
PH 850-766-7778



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Wind Normal
to Ridge
↑

h = Mean structure height	= 16.467 ft	$K_h = 2.41 \cdot (Z/Z_g)^{2/a}$ $T:26.10-1$	= 0.587
K_{zt} = No Topographic Feature	= 1.000	K_d = Directionality Factor $T:26.6-1$	= 0.85
GC_{pi} = $\pm$ Internal Press Coef $T:26.13-1$	= ± 0.18	LF = ASD Load Factor	= 0.60
K_e = Ground Elev Factor $T:26.10-1$	= 1.000	$q_h = .00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ $E:26.10-1$	= 12.99 psf
q_{in} = Negative Internal Pressure: q_h	= 12.99 psf	q_{ip} = For $+GC_{pi}$ use q_h	= 12.99 psf
A_{roof} = Roof Area	= 3,295.34 ft ²		

MWFRS Wind Loads [Normal to Ridge]

h = Mean Roof Height of Building	= 16.4667 ft	R_{ht} = Ridge Height Of Roof	= 23.9333 ft
B = Building Width Normal To Wind	= 50.2500 ft	L = Building Width Parallel To Wind	= 51.2000 ft
L/B = Ratio: L/B	= 1.019	h/L = Ratio: h/L	= 0.322
θ = Slope of Roof	= 28.86 °	G = Gust Factor: $\min(G_1, G_2)$	= 0.850
$C_{p_{WW}}$ = Windward Wall Coefficient	= 0.800	$C_{p_{LW}}$ = Leeward Wall Coefficient	= -0.496
$C_{p_{SW}}$ = Side Wall Coefficient	= -0.700		

Wall Wind Pressures [Normal to Ridge] All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
9.000	+0.18	12.99	0.573	1.000	12.67	5.34	-6.64	-8.56	11.98	9.60
9.000	-0.18	12.99	0.573	1.000	12.67	9.31	-2.67	-4.58	11.98	9.60

K_z	= $2.41 \cdot (Z/Z_g)^{2/a}$	K_{zt}	= No Topographic Feature
GC_{pi}	= $\pm$ Internal Coef $T:26.13-1$	q_z	= $.00256 \cdot K_z \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ $E:26.10-1$
q_{ip}	= For $+GC_{pi}$ use q_h	q_{in}	= Negative Internal Pressure: q_h
Side	= $q_h \cdot K_d \cdot G \cdot C_{p_{SW}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$	Leeward	= $q_h \cdot K_d \cdot G \cdot C_{p_{LW}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$
Windward	= $q_z \cdot K_d \cdot G \cdot C_{p_{WW}} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$	Total	= Windward - Leeward
+Press	= Pressure Acting Toward Surface	-Press	= Pressure Acting Away from Surface
$\$27.1.5$	= MWFRS Min Wall Pressure = 9.60 psf		

Roof Wind Pressures [Normal to Ridge] All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC_{pi}	C_{pMin}	C_{pMax}	P_{min} psf	P_{max} psf	P_{min} psf
Overhang	Leeward	7,8	All	All	28.86	N	0	-0.6	-0.6	-5.63	-5.63	4.80
Overhang	Windward	5,6	All	All	28.86	N	0	0.271	-0.207	2.55	-1.94	4.80
Overhang	Leeward	4	All	All	28.86	N	+0.18	-0.6	-0.6	-7.62	-7.62	4.80
Overhang	Windward	3	All	All	28.86	N	+0.18	0.271	-0.207	0.56	-3.93	4.80
Roof	Leeward	2	All	All	28.86	N	+0.18	-0.6	-0.6	-7.62	-7.62	4.80
Roof	Windward	1	All	All	28.86	N	+0.18	0.271	-0.207	0.56	-3.93	4.80
Soffit	Bottom	3	All	All	0.0	N/A	+0.18	0.8	0.8	5.52	5.52	4.80
Overhang	Leeward	4	All	All	28.86	N	-0.18	-0.6	-0.6	-3.64	-3.64	4.80

Overhang	Windward	3	All	All	28.86	N	-0.18	0.271	-0.207	4.53	0.05	4.80
Roof	Leeward	2	All	All	28.86	N	-0.18	-0.6	-0.6	-3.64	-3.64	4.80
Roof	Windward	1	All	All	28.86	N	-0.18	0.271	-0.207	4.53	0.05	4.80
Soffit	Bottom	3	All	All	0.0	N/A	-0.18	0.8	0.8	9.50	9.50	4.80

Roof Pressures based upon Ch 27:

Component = The building component for pressures

Start = Start Dist from Windward Edge

C_{pMin} = Smallest Coefficient Magnitude

P_{min} = $q_h \cdot K_d \cdot G \cdot C_{pMin} - q_{ip} \cdot K_d \cdot GC_{piE:27.3-1}$

GC_{pi} = +Internal Coef $E:26.13-1$

P_{min} = Min Press projected on vertical plane $E:27.1.5$

$E:27.1.5$ = MWFRS Min Wall Pressure = 9.60 psf

-Press = Pressure Acting Away from Surface

• The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Location = Reference Graphic in Output for Values

End = End Dist from Windward Edge

C_{pMax} = Largest Coefficient Magnitude

P_{max} = $q_h \cdot K_d \cdot G \cdot C_{pMax} - q_{in} \cdot K_d \cdot GC_{piE:27.3-1}$

Basis = P=Parallel to Ridge: N=Normal to Ridge

θ = Roof Slope Relative to Wind

+Press = Pressure Acting Toward Surface

MWFRS Wind Loads [Parallel to Ridge]

h = Mean Roof Height of Building = 16.4667 ft

B = Building Width Normal To Wind = 51.2000 ft

L/B = Ratio: L/B = 0.981

θ = Slope of Roof = 28.86 °

C_{pW} = Windward Wall Coefficient = 0.800

C_{pS} = Side Wall Coefficient = -0.700

R_{ht} = Ridge Height Of Roof = 23.9333 ft

L = Building Width Parallel To Wind = 50.2500 ft

h/L = Ratio: h/L = 0.328

G = Gust Factor: Min(G_1 , G_2) = 0.850

C_{pL} = Leeward Wall Coefficient = -0.500

Wall Wind Pressures [Parallel to Ridge] All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
23.933	+0.18	12.99	0.649	1.000	14.35	6.31	-6.68	-8.56	12.99	9.60
16.467	+0.18	12.99	0.587	1.000	12.99	5.52	-6.68	-8.56	12.20	9.60
9.000	+0.18	12.99	0.573	1.000	12.67	5.34	-6.68	-8.56	12.02	9.60
23.933	-0.18	12.99	0.649	1.000	14.35	10.28	-2.71	-4.58	12.99	9.60
16.467	-0.18	12.99	0.587	1.000	12.99	9.50	-2.71	-4.58	12.20	9.60
9.000	-0.18	12.99	0.573	1.000	12.67	9.31	-2.71	-4.58	12.02	9.60

K_z = $2.41 \cdot (Z/Z_g)^{2/5}$

GC_{pi} = +Internal Coef $E:26.13-1$

q_{ip} = For + GC_{pi} use q_h

Side = $q_h \cdot K_d \cdot G \cdot C_{pS} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$

Windward = $q_z \cdot K_d \cdot G \cdot C_{pW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$

+Press = Pressure Acting Toward Surface

$E:27.1.5$ = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature

q_z = $.00256 \cdot K_z \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF_{E:26.10-1}$

q_{in} = Negative Internal Pressure: q_h

Leeward = $q_h \cdot K_d \cdot G \cdot C_{pL} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ $E:27.3-1$

Total = Windward - Leeward

-Press = Pressure Acting Away from Surface

Roof Wind Pressures [Parallel to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC_{pi}	C_{pMin}	C_{pMax}	P_{min} psf	P_{max} psf	P_{min} psf
Overhang	Overhang 0 to h/2	5,7	0.000	1.500	0.0	P	0	-0.9	-0.18	-8.45	-1.69	4.80
Overhang	Overhang $\geq 2 \cdot h$	6,8	51.750	53.250	0.0	P	0	-0.3	-0.18	-2.82	-1.69	4.80
Overhang Bottom	Bottom	5,7	All	All	0.0	N/A	0	0.8	0.8	7.51	7.51	4.80
Overhang	Overhang 0 to h	3,4	1.500	16.467	0.0	P	+0.18	-0.9	-0.18	-10.43	-3.68	4.80
Overhang	Overhang h to $2 \cdot h$	3,4	16.467	32.933	0.0	P	+0.18	-0.5	-0.18	-6.68	-3.68	4.80
Overhang	Overhang $\geq 2 \cdot h$	3,4	32.933	51.750	0.0	P	+0.18	-0.3	-0.18	-4.80	-3.68	4.80
Roof	Roof 0 to h	1,2	1.500	16.467	0.0	P	+0.18	-0.9	-0.18	-10.43	-3.68	4.80
Roof	Roof h to $2 \cdot h$	1,2	16.467	32.933	0.0	P	+0.18	-0.5	-0.18	-6.68	-3.68	4.80
Roof	Roof $\geq 2 \cdot h$	1,2	32.933	51.750	0.0	P	+0.18	-0.3	-0.18	-4.80	-3.68	4.80
Overhang	Overhang 0 to h	3,4	1.500	16.467	0.0	P	-0.18	-0.9	-0.18	-6.46	0.30	4.80
Overhang	Overhang h to $2 \cdot h$	3,4	16.467	32.933	0.0	P	-0.18	-0.5	-0.18	-2.71	0.30	4.80
Overhang	Overhang $\geq 2 \cdot h$	3,4	32.933	51.750	0.0	P	-0.18	-0.3	-0.18	-0.83	0.30	4.80
Roof	Roof 0 to h	1,2	1.500	16.467	0.0	P	-0.18	-0.9	-0.18	-6.46	0.30	4.80
Roof	Roof h to $2 \cdot h$	1,2	16.467	32.933	0.0	P	-0.18	-0.5	-0.18	-2.71	0.30	4.80
Roof	Roof $\geq 2 \cdot h$	1,2	32.933	51.750	0.0	P	-0.18	-0.3	-0.18	-0.83	0.30	4.80

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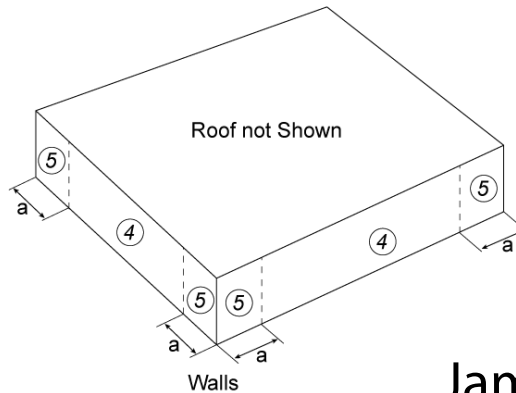
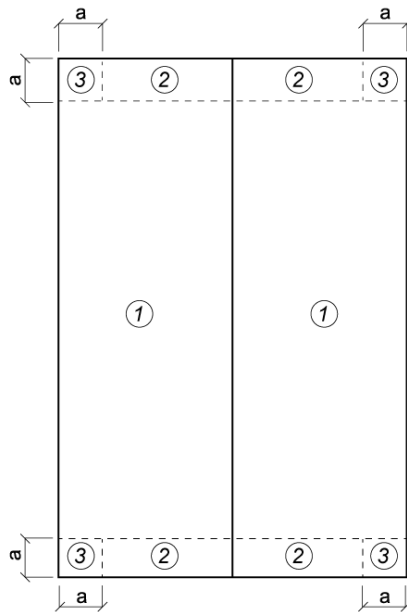
Roof Pressures based upon Ch 27:

Component = The building component for pressures
 Start = Start Dist from Windward Edge
 C_{pMin} = Smallest Coefficient Magnitude
 $P_{min} = q_h \cdot K_d \cdot G \cdot C_{pMin} - q_{ip} \cdot K_d \cdot GC_{piE:27.3-1}$
 GC_{pi} = +Internal Coef $T:26.13-1$
 P_{min} = Min Press projected on vertical plane $S27.1.5$
 $S27.1.5$ = MWFRS Min Wall Pressure = 9.60 psf
 -Press = Pressure Acting Away from Surface

Location = Reference Graphic in Output for Values
 End = End Dist from Windward Edge
 C_{pMax} = Largest Coefficient Magnitude
 $P_{max} = q_h \cdot K_d \cdot G \cdot C_{pMax} - q_{in} \cdot K_d \cdot GC_{piE:27.3-1}$
 Basis = P=Parallel to Ridge: N=Normal to Ridge
 θ = Roof Slope Relative to Wind
 +Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Components and Cladding (C&C) Wind Loads per Ch 30 Pt 1 Roof & Wall



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h = Mean structure height = 16.467 ft
 K_{zt} = No Topographic Feature = 1.000
 $GC_{pi} = \pm$ Internal Press Coef $T:26.13-1$ = ± 0.18
 K_e = Ground Elev Factor $T:26.10-1$ = 1.000
 θ = Slope of Roof = 28.86 °
 $a = \text{Max}(a_1, 0.04 \cdot B, 3 \text{ ft } [0.9 \text{ m}])$ = 5.025 ft

$K_h = 2.41 \cdot (Z/Z_g)^{2/\alpha}$ = 0.587
 K_d = Directionality Factor $T:26.6-1$ = 0.85
 LF = ASD Load Factor = 0.60
 $q_h = .00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF_{E:26.10-1}$ = 12.99 psf
 $a_1 = \text{Min}(0.1 \cdot B, 0.4 \cdot h)$ = 5.025 ft

C&C Wind Roof & Wall Detailed per Ch 30 Pt 1 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	GC_{pi}	GC_{pd}	GC_{pu}	P_{down} psf	P_{uplift} psf
Zone 1	1	1.0000	1.0000	1.00	No	F:30.3-2D	± 0.18	0.90	-1.80	11.93	-21.86
Zone 2	2	1.0000	1.0000	1.00	No	F:30.3-2D	± 0.18	0.90	-2.00	11.93	-24.07
Zone 3	3	1.0000	1.0000	1.00	No	F:30.3-2D	± 0.18	0.90	-2.50	11.93	-29.59
Zone 4	4	1.0000	1.0000	1.00	No	F:30.3-1	± 0.18	1.00	-1.10	13.03	-14.13
Zone 5	5	1.0000	1.0000	1.00	No	F:30.3-1	± 0.18	1.00	-1.40	13.03	-17.45

GC_{pd} = Down (+) External Coefficient
 $P_{down} = q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ $E:30.3-1$
 +Press = Pressure Acting Toward Surface
 $S30.2.2$ = C&C Min Pressure = 9.60 psf
 Width = Width of Component
 Area = Span $\cdot$ Width
 GC_{pi} = Internal Coef $T:26.13-1$

GC_{pu} = Uplift (-) External Coefficient
 $P_{uplift} = q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $E:30.3-1$
 -Press = Pressure Acting Away from Surface
 Zone = Applicable Zone per Figure
 Span = Span of Component
 1/3 Rule = Width limited to Span/3
 Reference = Applicable Reference from Standard

C&C Wind Roof & Wall Overhangs Detailed per Ch 30 Pt 4 All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	GC_{pi}	GC_{pd}	GC_{pu}	P_{down} psf	P_{uplift} psf
Zone 1_OHS	1_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	± 0.18	0.00	-2.80	9.60	-32.91
Zone 2_OHS	2_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	± 0.18	0.00	-3.00	9.60	-35.11

Zone	3_OHS	3_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	±0.18	0.00	-3.50	9.60	-40.63
------	-------	-------	--------	--------	------	----	--------------------	-------	------	-------	------	--------

GC_{pd} = Down (+) External Coefficient
 P_{down} = $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ E:30.7-1
+Press = Pressure Acting Toward Surface
\$30.2.2 = C&C Min Pressure = 9.60 psf
Width = Width of Component
Area = Span • Width
 GC_{pi} = Internal Coef T:26.13-1
#_OHS = Roof Zone # on Overhang Soffit

GC_{pu} = Uplift (-) External Coefficient
 P_{uplift} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ E:30.7-1
-Press = Pressure Acting Away from Surface
Zone = Applicable Zone per Figure
Span = Span of Component
1/3 Rule = Width limited to Span/3
Reference = Applicable Reference from Standard
Soffit = Soffit present so use building GC_{pi}

Warnings & Notes:

Overhang GC_p determined from adding applicable roof GC_p on top to applicable Wall GC_p on bottom

C&C Wind Roof & Wall Summary per Ch 30 Pt 1

Zone	Reference	P_{max} A ≤ 10 ft ² psf	P_{min} A ≤ 10 ft ² psf	P_{max} A = 20 ft ² psf	P_{min} A = 20 ft ² psf	P_{max} A = 50 ft ² psf	P_{min} A = 50 ft ² psf
1	F:30.3-2D	11.93	-21.86	10.90	-18.54	9.60	-14.15
2	F:30.3-2D	11.93	-24.07	10.90	-21.52	9.60	-18.14
3	F:30.3-2D	11.93	-29.59	10.90	-25.76	9.60	-20.69
4	F:30.3-1	13.03	-14.13	12.44	-13.55	11.67	-12.77
5	F:30.3-1	13.03	-17.45	12.44	-16.27	11.67	-14.72
1_OHS	F:30.3-2D/F:30.3-1	9.60	-32.91	9.60	-28.99	9.60	-23.82
2_OHS	F:30.3-2D/F:30.3-1	9.60	-35.11	9.60	-31.97	9.60	-27.82
3_OHS	F:30.3-2D/F:30.3-1	9.60	-40.63	9.60	-36.22	9.60	-30.37

Zone	Reference	P_{max} A = 100 ft ² psf	P_{min} A = 100 ft ² psf	P_{max} A = 200 ft ² psf	P_{min} A = 200 ft ² psf	P_{max} A > 500 ft ² psf	P_{min} A > 500 ft ² psf
1	F:30.3-2D	9.60	-10.82	9.60	-10.82	9.60	-10.82
2	F:30.3-2D	9.60	-15.58	9.60	-13.03	9.60	-13.03
3	F:30.3-2D	9.60	-16.86	9.60	-13.03	9.60	-13.03
4	F:30.3-1	11.08	-12.18	10.49	-11.60	9.72	-10.82
5	F:30.3-1	11.08	-13.55	10.49	-12.37	9.72	-10.82
1_OHS	F:30.3-2D/F:30.3-1	9.60	-19.91	9.60	-19.33	9.60	-18.55
2_OHS	F:30.3-2D/F:30.3-1	9.60	-24.68	9.60	-21.54	9.60	-20.76
3_OHS	F:30.3-2D/F:30.3-1	9.60	-25.95	9.60	-21.54	9.60	-20.76

P_{max} = Maximum Pressure
Area = Span • Width
Span = Span of Component
\$30.2.2 = C&C Min Pressure = 9.60 psf

P_{min} = Minimum Pressure
Width = Width of Component
Reference = Applicable Reference from Standard
Interpolate = Interpolate for Areas between columns

REVIEWED BY JAMES ZALESKI P.E.

2305 HAVEFRHILL RD

TALLAHASSEE, FL 323132

PH 850-766-7778



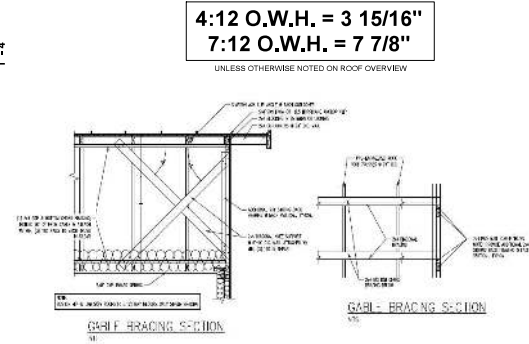
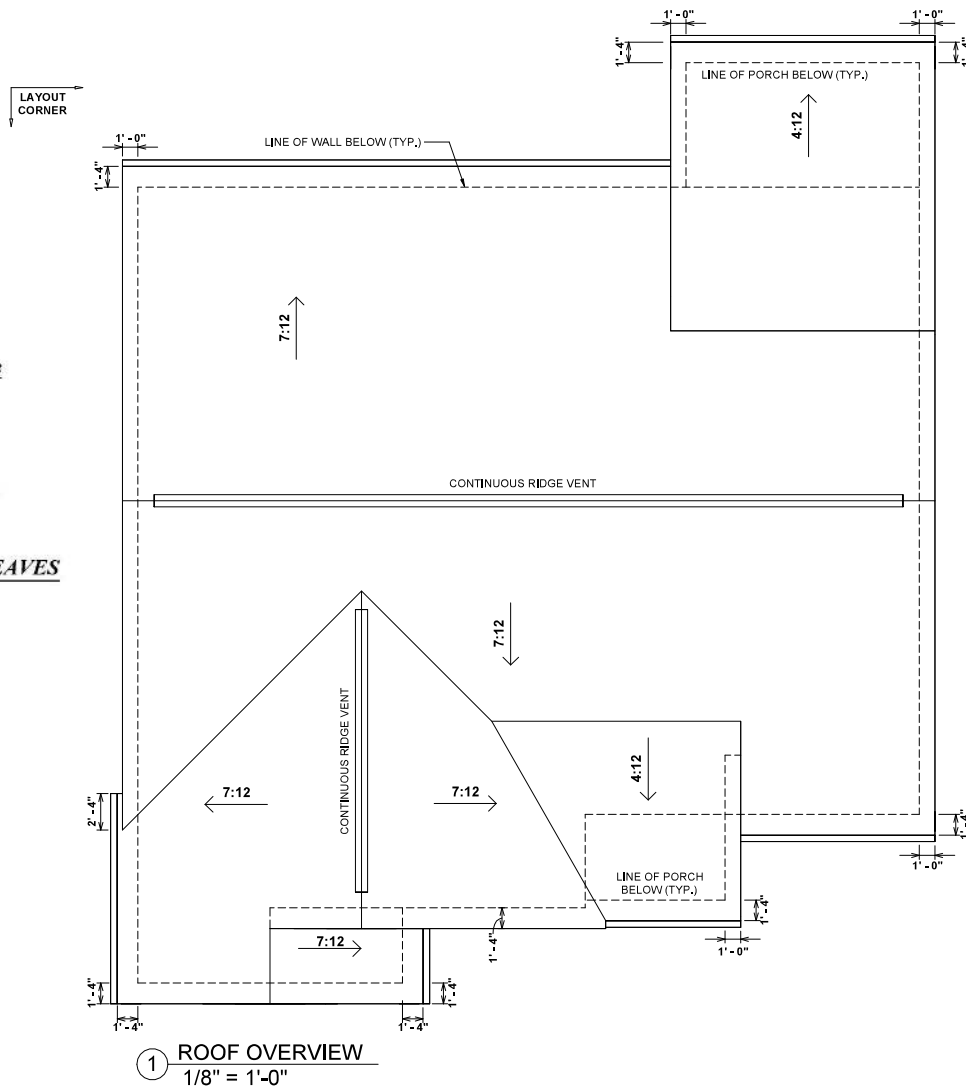
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by James A
Zaleski
Date: 2025.04.28
09:05:11 -04'00'

Digitally signed by
James A Zaleski
Date: 2025.04.28
09:04:53 -04'00'

4" O.C FIRST ROW AT ALL EAVES

WARRANT FOR ARREST: This form has been digitally signed and sealed by JAMES D. SLOPE, J/SA on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified in electronic copies.



- FLOOR & ROOF TRUSS NOTES

- [illegible]

SHEET #		PROPERTY OF:	DRAWN BY:	JOB #	THE:	AREAS:			REVISION SCHEDULE		
E-3			FABIO D.	59-25-008	MURRAY	FIRST FLOOR HEATED	2,281 SF	REV #	DESCRIPTION	DATE	
			CHECKED BY:	2x4 EXTERIOR WALLS	FOR:	FRONT PORCH	50 SF				
			PRINTED:	FOUNDATION TYPE:		REAR PORCH	120 SF				
			3/28/2025	MONOSLAB		TOTAL UNDER ROOF	170 SF				
			11:33 PM				2,451 SF				
		© CORVIEIGHT - 2025	OFFICE:	SOLD BY:							
			GAINESVILLE	L. HALLER							
ROOF OVERVIEW											

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

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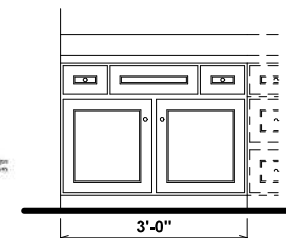
SCALE: 1/8" = 1'-0"

ROOF OVERVIEW

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Zaleski
Date:
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TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION.



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SHEET		PROPERTY OF:	DRAWN BY:	JOB #	THE:	AREAS:		REVISION SCHEDULE	
F-2			FABIO D.	59-25-008	MURRAY	FRONT PORCH	2,281 SF	REV #	DATE
			CHECKED BY:	2x4 EXTERIOR WALLS	TJ PIZZAGALLI	REAR PORCH	240 SF		
			X-CHECKER				120 SF		
			PRINTED:	FOUNDATION TYPE	245 SW COLONY GLEN LAKE CITY, FL 32024		2,451 SF		
			3/26/2025	MONOSLAB		TOTAL UNDER ROOF			
			11:33 PM						
SCALE: As indicated		© COPRIGHT-2025			OFFICE:				
FIRST FLOOR PLAN					SOLD BY:				
					L. HALLER				
					GAINESVILLE				

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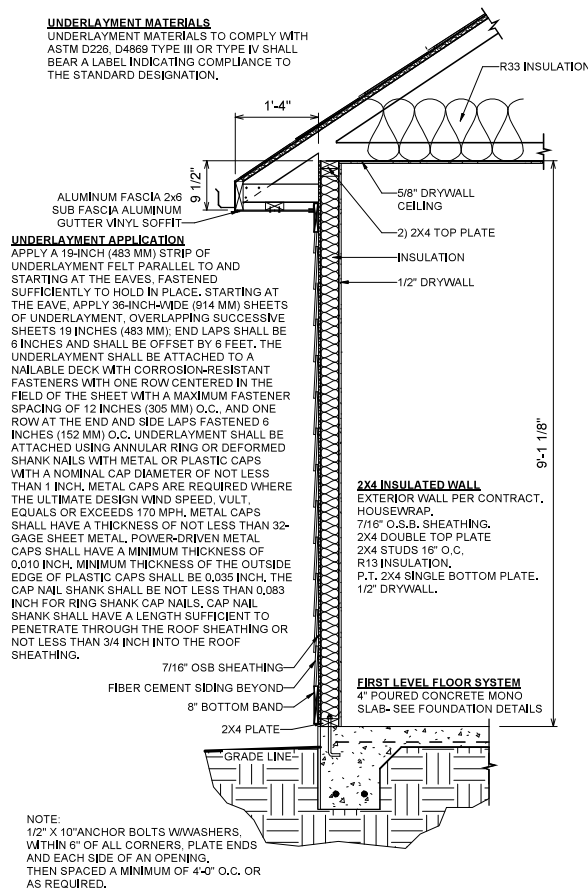
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James A Zaleski
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ROOF SYSTEM

ROOF COVERING AS SPECIFIED DOUBLE LAYER OF SYNTHETIC UNDERLAYMENT WITH 7/16" OSB SHEATHING NAILED AND CLIPPED PRE ENGINEERED ROOF TRUSSES PER PRINT
R-33 INSULATION IN FLAT AREAS
R-30 INSULATION IN SLOPED AREAS

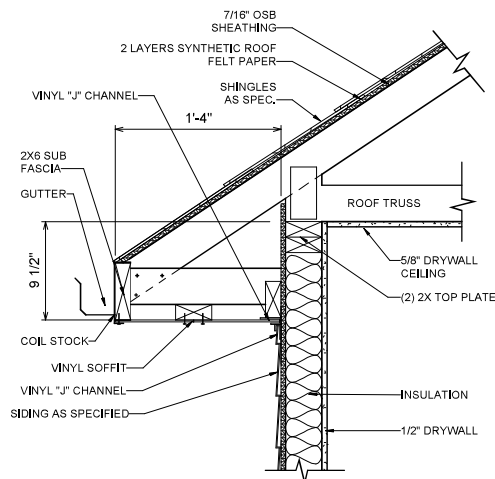
UNDERLAYMENT MATERIALS

UNDERLAYMENT MATERIALS TO COMPLY WITH ASTM D226, D4869 TYPE III OR TYPE IV SHALL BEAR A LABEL INDICATING COMPLIANCE TO THE STANDARD DESIGNATION.

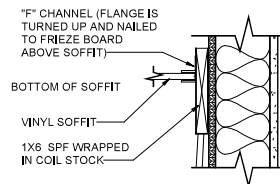


NOTE:
1/2" X 10" ANCHOR BOLTS W/WASHERS,
WITHIN 6" OF ALL CORNERS, PLATE ENDS
AND EACH SIDE OF AN OPENING,
THEN SPACED A MINIMUM OF 4'-0" O.C. OR
AS REQUIRED.

① 1 STORY WALL CONC. SIDING ON SLAB - FL
1/2" = 1'-0"



② 400.1 RAFTER TAIL AT CONC. SIDING WALL - FL
1" = 1'-0"



③ FRIEZE DETAIL AT SIDING
1 1/2" = 1'-0"

Wall Exterior Panel - Sheetrock with 7/16" OSB
- 2 X 8 STUDS @ 12" O.C. UP TO 10' Hgt.

= 28.0 STOPS @ 1st U.C. UP TO 1211.1

- 2 X 6 STUDS AT 12" O.C. UP TO 16' O.C.

REYNOLDS, DOROTHY L. & LUCY MARY. 1970.

SEE ATTACHED DETAILS

POSTS USE SIMPSON 500L BASE WITH 2 1/2" X 24 SHAPES AT TOP AND 2 SIMPSON 500L 15000 SHLWS FROM POST TO BEAM

MINI-MILL PENETRATION: 1-12"

Mail type BD

Intermediate Nail Spacing 8" o.c.

CANNERS AND 48" D.C.

CITIZENSHIP AND POLITICAL PARTICIPATION IN THE U.S.

REVIEWED BY JAMES ZALESKI P.E.

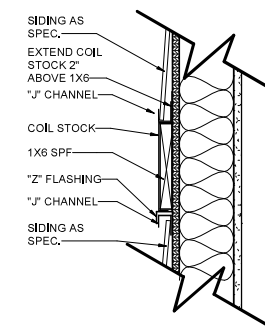
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TALLAHASSEE, FL 32313

PH 850-766-7778



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④ LINEAL BAND DETAIL CONC. SIDING
1 1/2" = 1'-0"

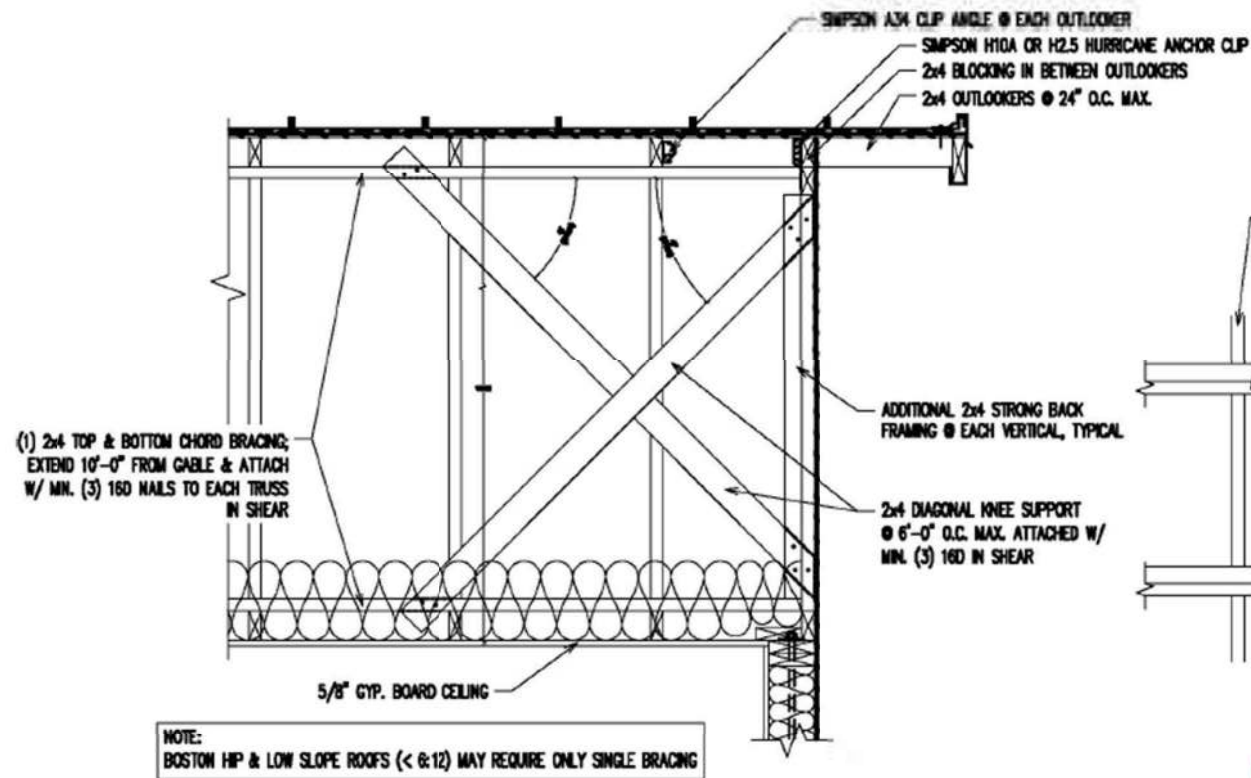
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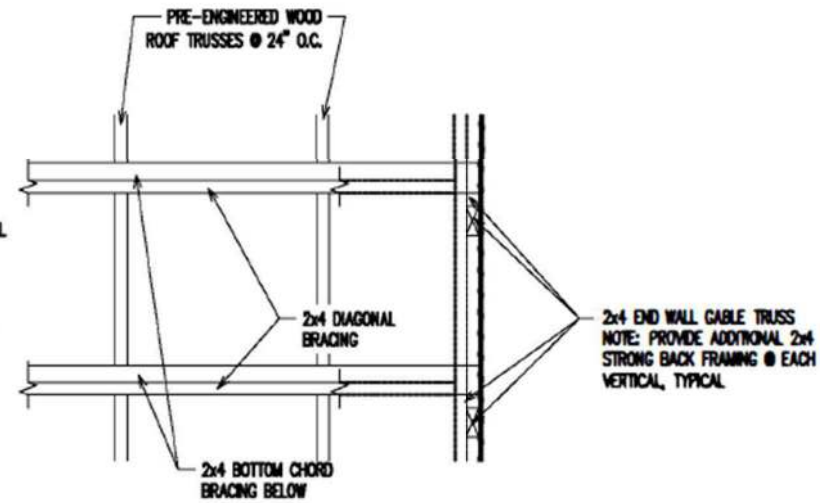
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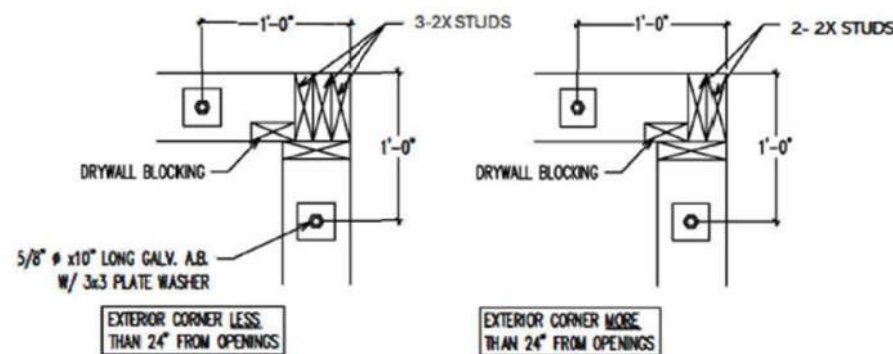
**MONOSLAB WALL SECTIONS -
CONCRETE SIDING**



GABLE BRACING SECTION
NTS



GABLE BRACING SECTION
NTS



CORNER FRAMING PLAN SECTIONS
NTS

INSTALL 5/8" ϕ x10" J-BOLT ANCHORS W/ MIN. 7" EMBEDMENT @ 32" O.C. FOR ALL LOAD BEARING WALLS.

JAMES ZALESKI P.E. 51544
2305 HAVERHILL RD
TALLAHASSEE, FL 32312
PH 850-766-7778



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James
A
Zaleski

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Date: 2025.04.28 09:02:38 -04'00'

ALL FLOOR TRUSS AND /OR ROOFFRAMING S TO BE DETERMINED BY THE TRUSS MANUFACTURER AND TRUSS DRAWINGS SHALL BE SIGNED AND SEALED BY THE SAME FRAMING LAYOUTS CONTAINED IN THESE DRAWINGS ARE TO BE CONSIDERED A PROPOSED SCHEMATIC REPRESENTATION ONLY AND FINAL MANUFACTURER DESIGN MAY VARY FROM THAT SHOWN. THE TRUSS MANUFACTURER'S CALCULATED SIZE AND SPACING OF PRE- ENGINEERED TRUSSES SHALL TAKE PRECEDENCE OVER WHAT HAS BEEN PROPOSED. THE OWNERAND / OR GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL TRAY, CATHEDRAL, AND OTHER DIMENSIONAL CEILING ASPECTS OF THIS PROJECT THAT MAY OR MAY NOT BE SHOWN ON PLANS PRIOR TO FABRICATION. IT IS RECOMMENDED THAT THE OWNER AND /OR GENERAL CONTRACTOR COORDINATE AND UNDERSTAND ALL ASPECTS OF THE SPECIFIED TRUSS PACKAGE LAYOUT BEFORE COMMENCING WITH INSTALLATION. DESIGN OF WOOD FLOOR TRUSSES (IF APPLICABLE) & ROOF TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE AND SUBMITTED TO BUILDING DEPARTMENT FOR PERMITTING. ANY REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION ONLY. ERECTION AND BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND THE TRUSS PLATE INSTITUTE'S "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS". SECURE EACH COMMON ROOF TRUSS/RAFTER TO TOP PLATE WITH ONE OF THE FOLLOWING SIMPSON ANCHOR CLIPS AT ALL BEARING POINTS: H- 10A (PREFERRED), (2) H2.5A, H7, OR MTS16. USE SIMPSON H-7 AT GIRDER TRUSSES. PROVIDE A MINIMUM OF TWO STUDS UNDER GIRDER TRUSS END BEARING. TRUSS TO TRUSS CONNECTIONS SHALL BE VERIFIED & PROVIDED BY THE TRUSS DESIGNER. CONTRACTOR TO PROVIDE ALL BLOCKING BETWEEN TRUSSES AND / OR RAFTER FRAMING. CONTRACTOR TO REFER TO F.B.C. 2023 & WFCM 2018 FOR FRAMING REQUIREMENTS OF WOOD FRAMED WALL SYSTEMS. TRUSS MANUFACTURER SHALL REFER TO ANY MEP DRAWINGS FOR OTHER ITEMS OR APPENDAGES THAT MAY EFFECT THE TRUSS LOADING. ANY SUCH ITEMS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER / ENGINEER OF RECORD.

BEAM SPAN TABLE		
Span (FT.)	Header Size	(2 x) cripples per end
0' - 4'-0"	2-2x10 w/7/16" OSB Flitch Plate	1
4'-0" - 9'-4"	2-2x10 w/7/16" OSB Flitch Plate	2
9'-4" - 12'-0"	2-2x10 w/7/16" OSB Flitch Plate	3
12'-0" - 15'-4"	3 1/2" X 11 7/8" LVL (or equal)	3

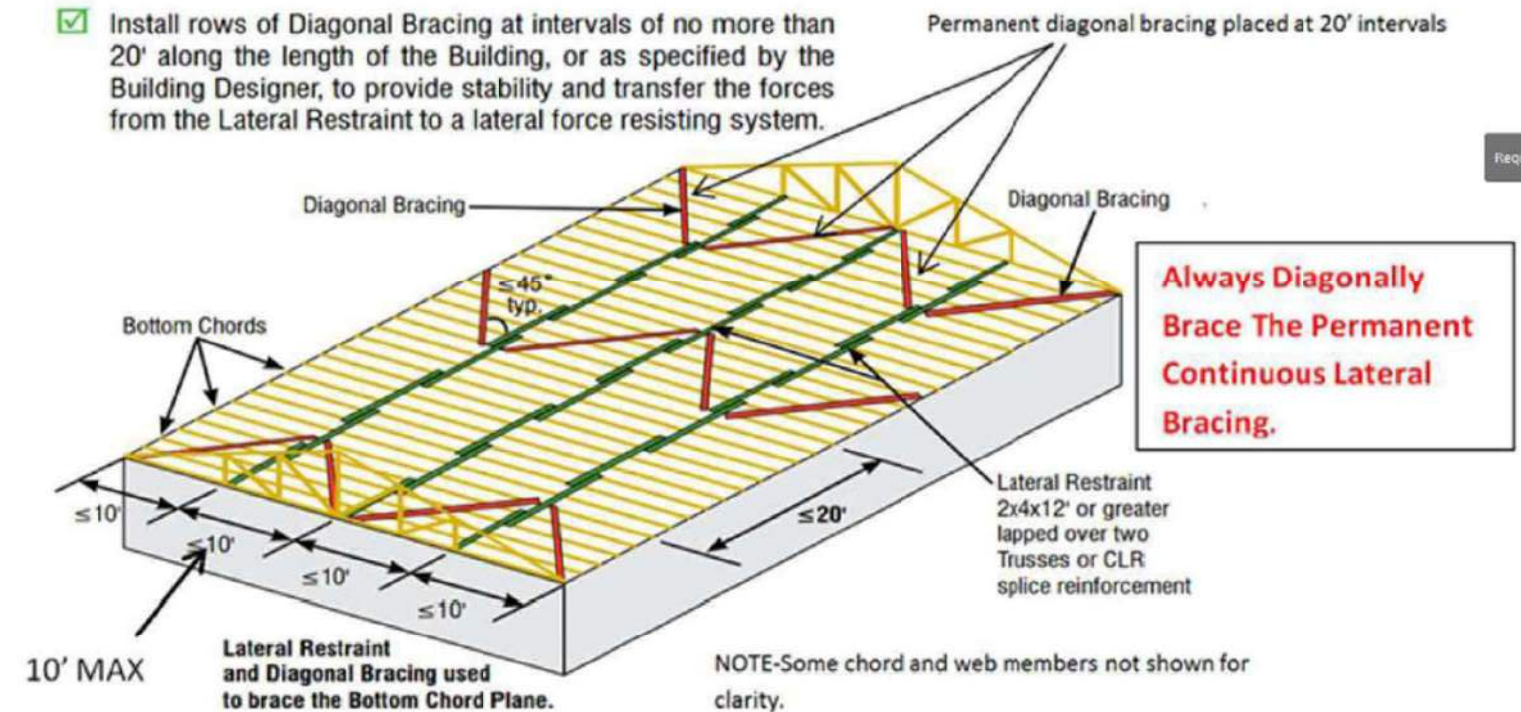
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2305 HAVERHILL RD
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James A Zaleski
Digitally signed
by James A Zaleski
Date: 2025.04.28
09:02:20 -04'00'

- ✓ Install rows of Diagonal Bracing at intervals of no more than 20' along the length of the Building, or as specified by the Building Designer, to provide stability and transfer the forces from the Lateral Restraint to a lateral force resisting system.



JAMES ZALESKI P.E. 51544

2305 HAVERHILL RD

TALLAHASSEE, FL 32312

PH 850-766-7778



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ES ZALESKI P.E. 51544 2305 HAVERHILL RD TALLAHASSEE, FL 32312 PH 850-766-7778

Install Simpson sheathing clip PSCL @ 24" c.c. for roof sheathing.
Gable ends per attached details. For vaulted ceilings, balloon framing is required.
Provide continuous structural sheathing on gable ends and block all edges on sheathing.

James A
Zaleski

Digitally signed by
James A Zaleski
Date: 2025.04.28
09:02:00 -04'00'

HEADER SIZE AND STRAPPING CHART

SPAN	HEADER SIZE	QUANTITY OF JACK STUDS AT EACH END	QUANTITY OF KING STUDS AT EACH END	STRAPPING TO JACK STUDS AT EACH END TOP AND BOTTOM	STRAPPING TO KING STUDS AT EACH END TOP AND BOTTOM
0'-0" TO 7'-6"	2 - 2X10" WITH ½" PLATE	1	1	1 SIMPSON MSTA24	1 SIMPSON SPH4
7'-6" – 11'-3"	2 - 2X12" WITH ½" PLATE OR 4-2 X 10" WITH ½" PLATE	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
11'-3" – 14'-0"	2- 1 ¼" X 9 ¼" LVL	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
IN LIEU OF STRAPPING <u>USE</u> A SDWC15600 AT THE TOP OF EACH JACK AND KING STUD AND ONE SDWC15450 AT THE BASE OF EACH JACK AND KING STUD					

JAMES ZALESKI P.E. 51544

2305 HAVERHILL RD

TALLAHASSEE, FL 32312

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James A Zaleski

Digitally signed
by James A Zaleski
Date: 2025.04.28
09:01:31 -04'00'

Shear Wall Panel Type "B" Specifications		
1 Face	Stud Spacing	16" O.C.
	Panel Grade	OSB Sheathing
	Minimum Panel Thickness	7/16"
	Minimum Nail Penetration in Framing	1 1/2"
	Nail Type	8d Common
	Edge Nail Spacing	4"
	Intermediate Nail Spacing (field)	12"
	Total Panel Shear Capacity	350 plf