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FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

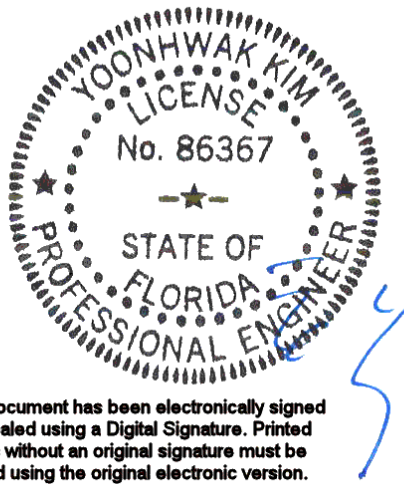
12/02/2022

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8612
Job Description: 1845F	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 through 22.01.01A JRef #: 1XL52150007
Wind Standard: ASCE 7-16 Wind Speed (mph): 130 Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 63 truss drawing(s) and 9 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	336.22.1426.48627	A01	2	336.22.1426.51730	A02
3	336.22.1426.53730	A03	4	336.22.1427.53740	A04
5	336.22.1428.37953	B01	6	336.22.1428.43047	B02
7	336.22.1428.45290	B03	8	336.22.1428.47317	B04
9	336.22.1428.54923	B05	10	336.22.1428.57347	B06
11	336.22.1429.05613	B07	12	336.22.1429.13740	B08
13	336.22.1429.29030	B09	14	336.22.1429.58380	B10
15	336.22.1430.25110	C01	16	336.22.1430.32207	C02
17	336.22.1430.37383	C03	18	336.22.1430.56123	C04
19	336.22.1431.09760	D01	20	336.22.1431.12480	D02
21	336.22.1431.15487	D03	22	336.22.1431.18730	D04
23	336.22.1431.21293	D05	24	336.22.1431.30180	D07
25	336.22.1432.10220	D09	26	336.22.1432.18780	D10
27	336.22.1432.23360	D11	28	336.22.1432.34473	D12
29	336.22.1432.37737	D13	30	336.22.1432.39973	D14
31	336.22.1432.41933	D15	32	336.22.1432.44757	E01
33	336.22.1432.47250	J01	34	336.22.1432.51367	J01HJ
35	336.22.1432.55540	J02	36	336.22.1432.57933	J02HJ
37	336.22.1433.07047	J03	38	336.22.1433.09820	J03HJ
39	336.22.1433.12083	J04	40	336.22.1433.13840	J05
41	336.22.1433.15617	J06	42	336.22.1433.17367	J07
43	336.22.1433.19913	J08	44	336.22.1433.22500	J09
45	336.22.1433.24670	J10	46	336.22.1433.30363	J11
47	336.22.1433.35283	PB01	48	336.22.1433.36903	PB02
49	336.22.1433.38780	PB03	50	336.22.1433.40570	PB04



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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8612
Job Description: 1845F	
Address:	

Item	Drawing Number	Truss
51	336.22.1433.44377	V01
53	336.22.1433.47000	V03
55	336.22.1433.51290	V05
57	336.22.1433.54877	V07
59	336.22.1434.00317	V09
61	336.22.1434.06663	V11
63	336.22.1442.21840	D08
65	GBLLETIN0118	
67	BRCLBSUB0119	
69	PB160160118	
71	VALTN160118	

Item	Drawing Number	Truss
52	336.22.1433.45657	V02
54	336.22.1433.49457	V04
56	336.22.1433.53047	V06
58	336.22.1433.56570	V08
60	336.22.1434.02150	V10
62	336.22.1441.37773	D06
64	A14015ENC160118	
66	CNNAILSP1014	
68	A14030ENC160118	
70	VAL180160118	
72	160TL	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

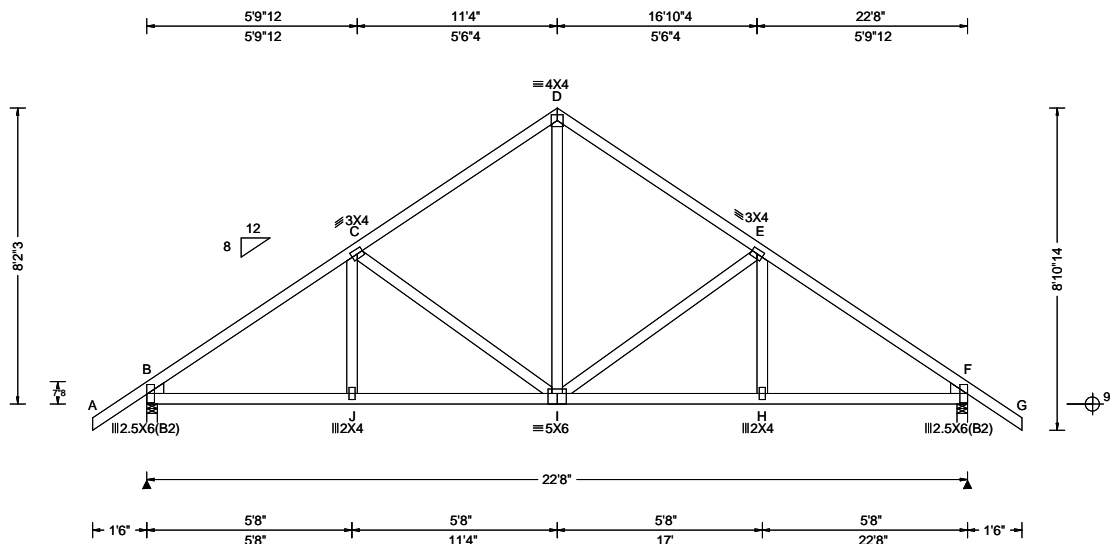
1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * =PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 E 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Closure: Closed	Lu: NA Cs: NA	VERT(CL): 0.005 E 999 180	B 286 /- /- /135 /38 /257
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 M - -	P* 82 /- /- /45 /14 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.006 M - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.256	P Brg Wid = 268 Min Req = -
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.060	Bearings B & B are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.140	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:(20(0)/10(0))		
	Loc. from endwall: Any	Plate Type(s):		
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1216.15	

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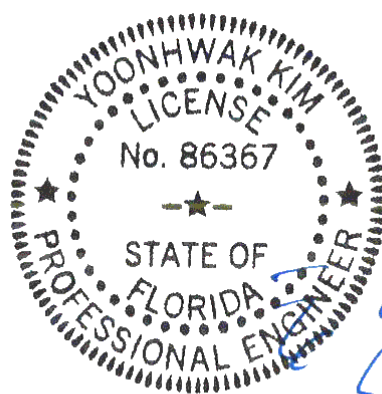
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14136 FROM:	COMN Ply: 1 Qty: 7	Job Number: 22-8612 1845F Truss Label: A02	Cust: R 215 JRef: 1XL52150007 T1 DrwNo: 336.22.1426.51730 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.048 I 999 240 VERT(CL): 0.098 I 999 180 HORZ(LL): 0.027 F - - HORZ(TL): 0.055 F - - Creep Factor: 2.0 Max TC CSI: 0.477 Max BC CSI: 0.486 Max Web CSI: 0.347 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1056 /- /- /652 /178 /267 F 1056 /- /- /652 /178 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 245 -1323 D - E 260 -946 C - D 261 -946 E - F 244 -1323

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;	Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - J 1008 -97 I - H 1007 -83 J - I 1007 -98 H - F 1008 -82
		Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - I 164 -375 D - I 577 -137

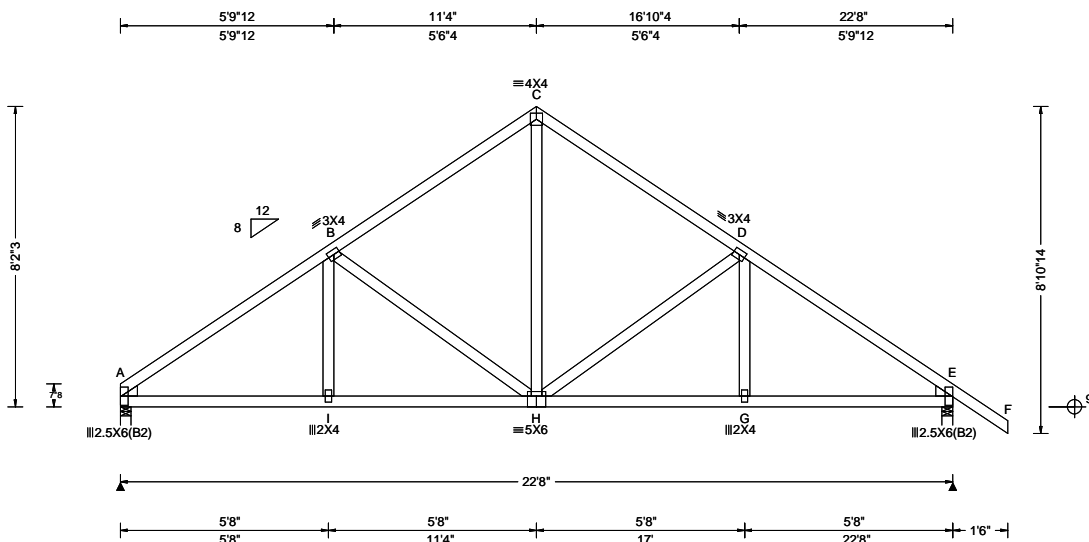


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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 14766 FROM:	COMN Ply: 1 Qty: 3	Job Number: 22-8612 1845F Truss Label: A03	Cust: R 215 JRef: 1XL52150007 T53 DrwNo: 336.22.1426.53730 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.046 H 999 240 VERT(CL): 0.095 H 999 180 HORZ(LL): 0.027 E - - HORZ(TL): 0.055 E - - Creep Factor: 2.0 Max TC CSI: 0.478 Max BC CSI: 0.488 Max Web CSI: 0.364 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL A 949 -/- /- /557 /148 /246 E 1059 -/- /- /652 /179 -/ Non-Gravity Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 253 -1340 C - D 265 -952 B - C 265 -953 D - E 249 -1329

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Wind

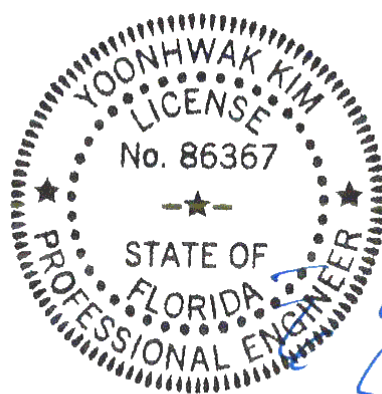
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - I	1027 -99	H - G	1012 -87
I - H	1026 -100	G - E	1013 -86

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	172 -392	H - D	164 -375
C - H	586 -144		



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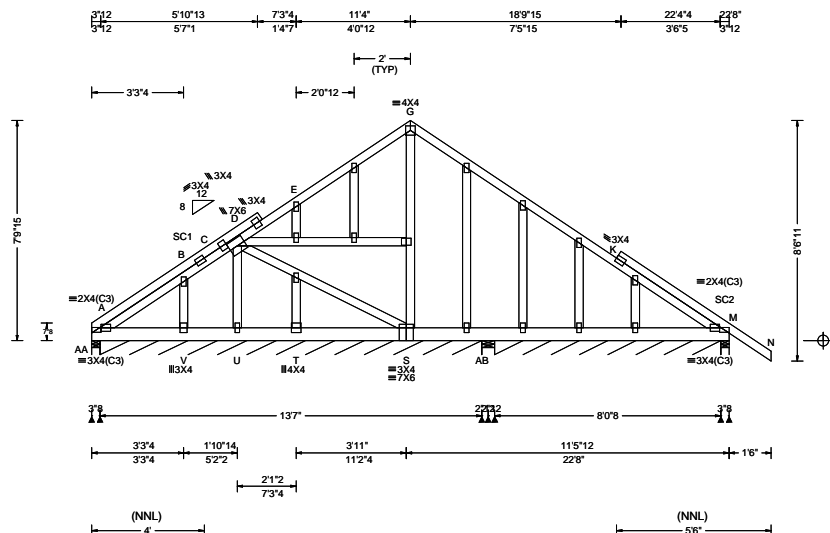
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14862 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: A04	Cust: R 215 JRef: 1XL52150007 T8 DrwNo: 336.22.1427.53740 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.045 F 999 240 VERT(CL): 0.090 F 999 180 HORZ(LL): 0.029 F - - HORZ(TL): 0.057 F - - Creep Factor: 2.0 Max TC CSI: 0.329 Max BC CSI: 0.481 Max Web CSI: 0.608 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity AA 1146 -/- /- /214 -/ AA*692 -/- /- /127 -/ AB 53 -/- /3 -/- /- AB*62 -/- /- /8 -/ M 302 -/- /- /83 -/ Wind reactions based on MWFRS AA Brg Wid = 3.5 Min Req = 1.5 (Truss) AA Brg Wid = 162 Min Req = - AB Brg Wid = 5.5 Min Req = 1.5 (Truss) AB Brg Wid = 96.5 Min Req = - M Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AA, AA, AB, AB, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 453 -67 B - C 419 -61 A - C 108 -495

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at 0.00 to 64 plf at 5.27
TC: From 32 plf at 5.27 to 32 plf at 7.27
TC: From 64 plf at 7.27 to 64 plf at 24.17
BC: From 10 plf at 0.00 to 10 plf at 11.19
BC: From 20 plf at 11.19 to 20 plf at 22.67
BC: From 5 plf at 22.67 to 5 plf at 24.17
BC: 1669 lb Conc. Load at 1.27, 3.27, 9.27
BC: 1501 lb Conc. Load at 5.27, 7.27
BC: 1553 lb Conc. Load at 11.27

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

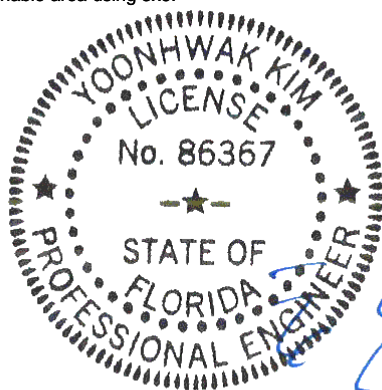
Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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Florida Certificate of Product Approval #FL 1999

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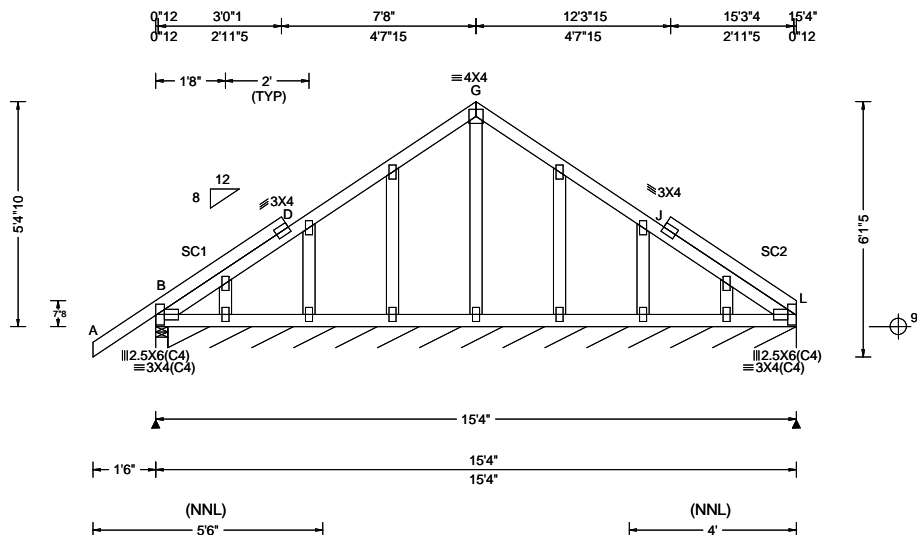
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14713 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B01	Cust: R 215 JRef: 1XL52150007 T13 DrwNo: 336.22.1428.37953 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 J 999 240 VERT(CL): 0.002 J 999 180 HORZ(LL): 0.002 J - - HORZ(TL): 0.002 J - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.048 Max Web CSI: 0.053 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 267 /- /- /151 /36 /169 L* 75 /- /- /42 /12 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) L Brg Wid = 180 Min Req = - Bearings B & B are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

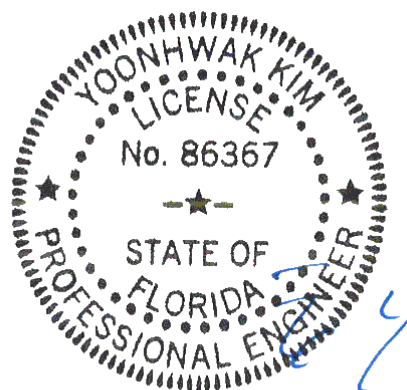
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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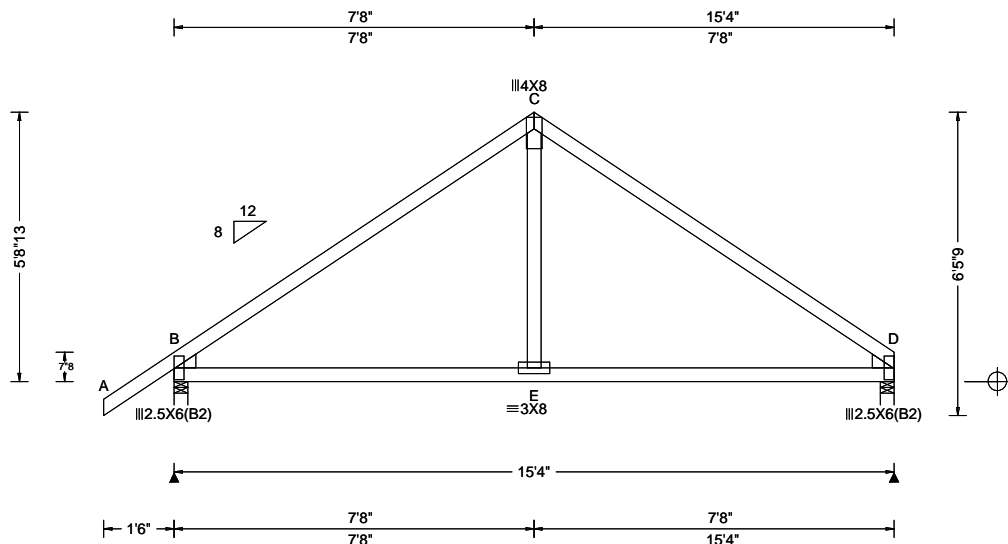
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14150 FROM:	COMN Ply: 1 Qty: 3	Job Number: 22-8612 1845F Truss Label: B02	Cust: R 215 JRef: 1XL52150007 T7 DrwNo: 336.22.1428.43047 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 E 999 240 VERT(CL): 0.019 E 999 180 HORZ(LL): 0.009 D - - HORZ(TL): 0.015 D - - Creep Factor: 2.0 Max TC CSI: 0.682 Max BC CSI: 0.571 Max Web CSI: 0.133 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 842 -/- /- /473 /130 /178 D 729 -/- /- /376 /98 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 345 -914 C - D 347 -911

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

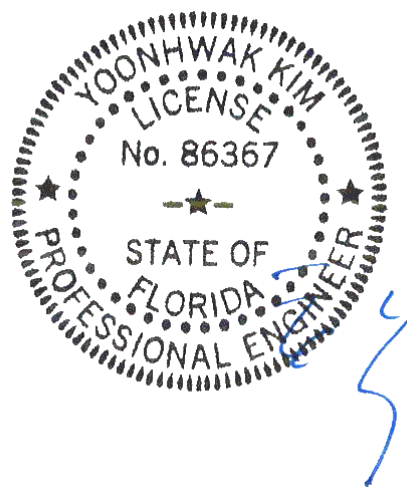
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

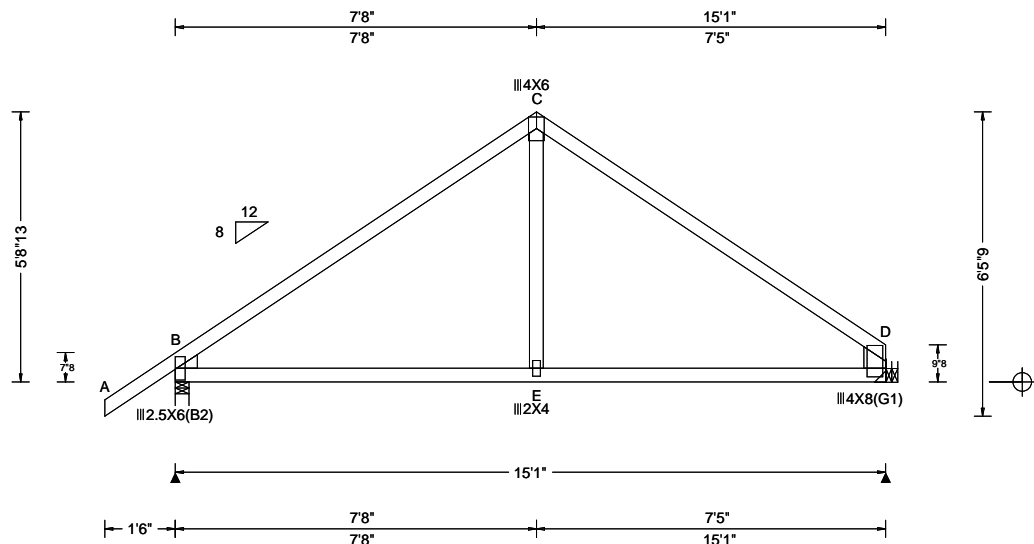


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14408 FROM:	SPEC Ply: 1 Qty: 13	Job Number: 22-8612 1845F Truss Label: B03	Cust: R 215 JRef: 1XL52150007 T64 DrwNo: 336.22.1428.45290 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.010 E 999 240 VERT(CL): 0.019 E 999 180 HORZ(LL): 0.012 D - - HORZ(TL): 0.019 D - - Creep Factor: 2.0 Max TC CSI: 0.634 Max BC CSI: 0.539 Max Web CSI: 0.128 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 830 - / - / - / 467 / 128 / 176 D 720 - / - / - / 367 / 97 / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 190 -887 C - D 192 -882

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;
Lt Wedge: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

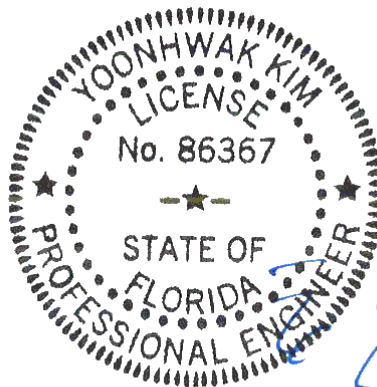
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

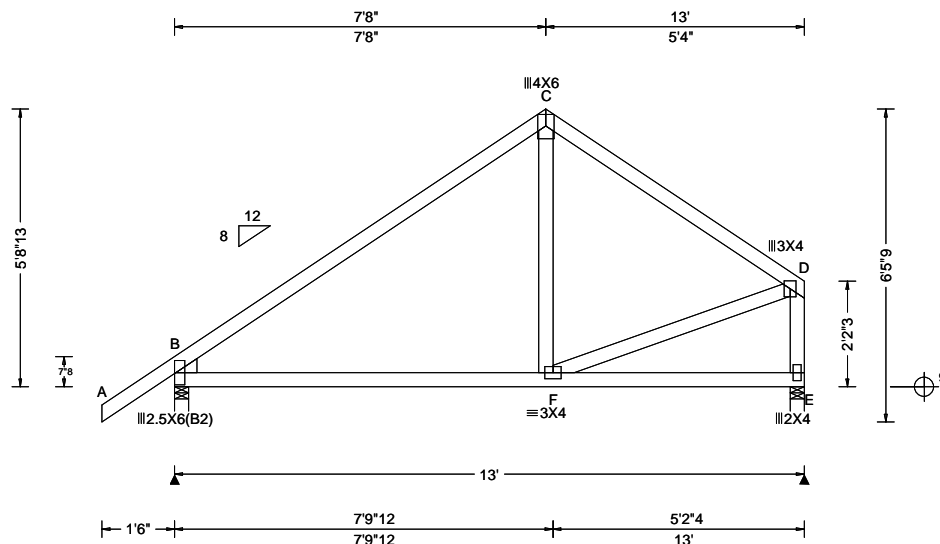


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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14152 FROM:	COMN Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: B04	Cust: R 215 JRef: 1XL52150007 T21 DrwNo: 336.22.1428.47317 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 F 999 240 VERT(CL): 0.009 F 999 180 HORZ(LL): 0.008 B - - HORZ(TL): 0.016 B - - Creep Factor: 2.0 Max TC CSI: 0.634 Max BC CSI: 0.512 Max Web CSI: 0.172 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 701 /- /- /423 /11 /187 E 584 /- /- /294 /2 /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 175 -648 C - D 190 -590

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

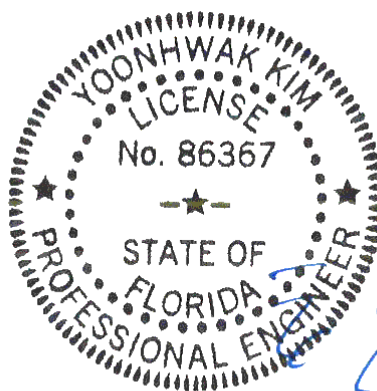
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.
B - F	431 -123

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
F - D	451 -99	D - E	178 -564



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Florida Certificate of Product Approval #FL 1999

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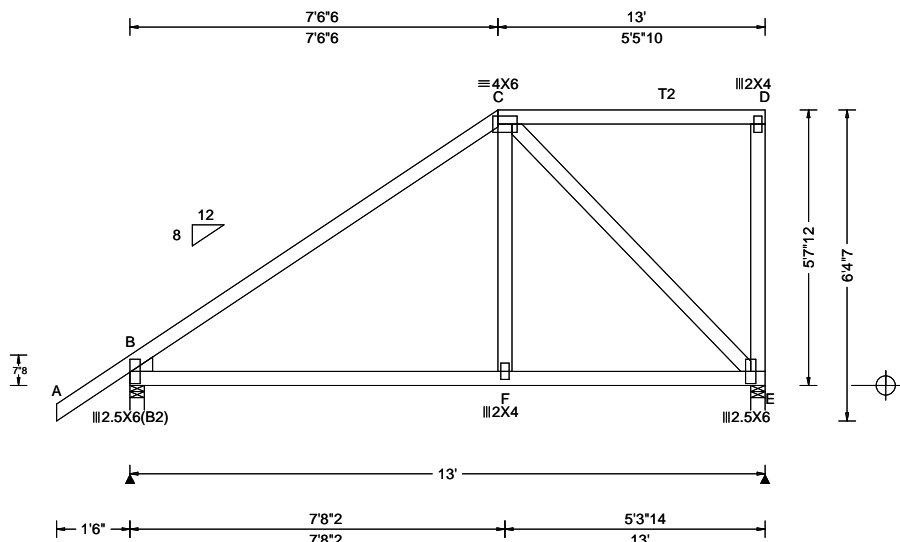
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14867 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B05	Cust: R 215 JRef: 1XL52150007 T11 DrwNo: 336.22.1428.54923 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 F 999 240 VERT(CL): 0.009 F 999 180 HORZ(LL): -0.010 B - - HORZ(TL): 0.020 B - - Creep Factor: 2.0 Max TC CSI: 0.621 Max BC CSI: 0.571 Max Web CSI: 0.688 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 655 -/- /- /436 /109 /245 E 540 -/- /- /327 /120 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 250 -558

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Purlins

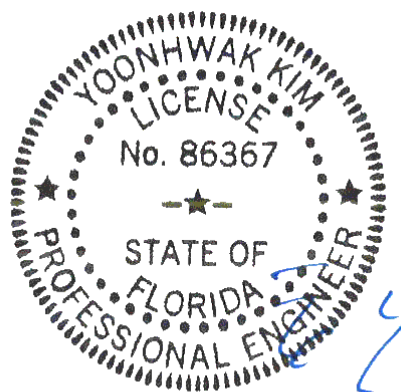
In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C
member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.

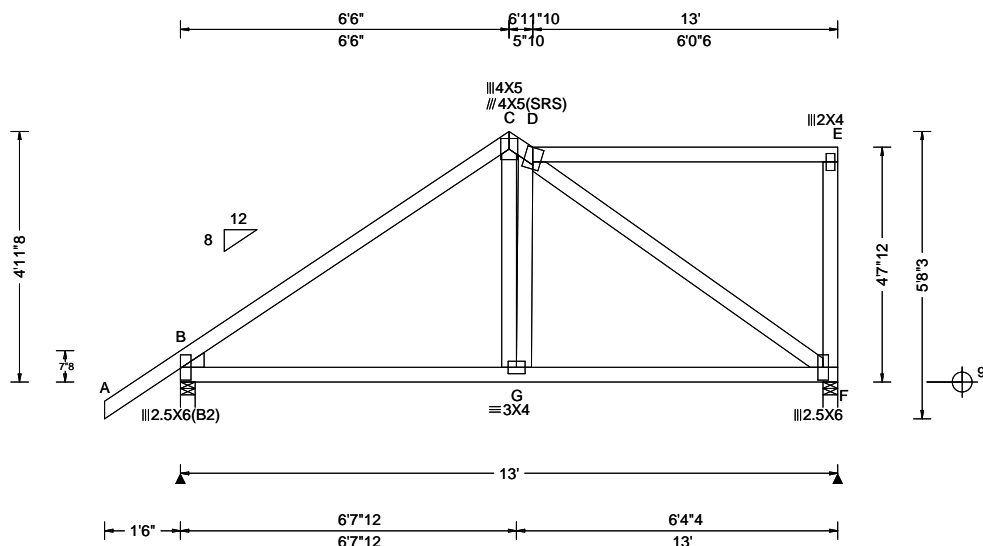


FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida State Seal of Product Approval #FL 1999

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14082 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B06	Cust: R 215 JRRef: 1XL52150007 T45 DrwNo: 336.22.1428.57347 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 D 999 240 VERT(CL): 0.019 D 999 180 HORZ(LL): -0.005 B - - HORZ(TL): 0.009 F - - Creep Factor: 2.0 Max TC CSI: 0.553 Max BC CSI: 0.543 Max Web CSI: 0.583 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 655 -/- /- /432 /110 /209 F 540 -/- /- /309 /105 -/ Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 303 -635 C - D 476 -519

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Purlins

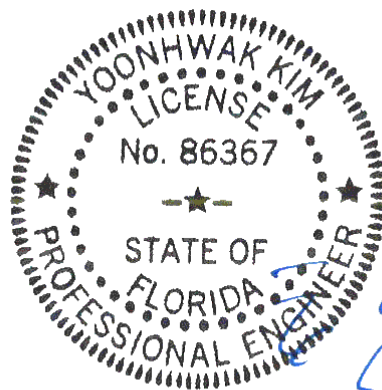
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - G	433 -375	G - F	455 -411

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - G	334 -434	D - F	429 -561
G - D	512 -167		

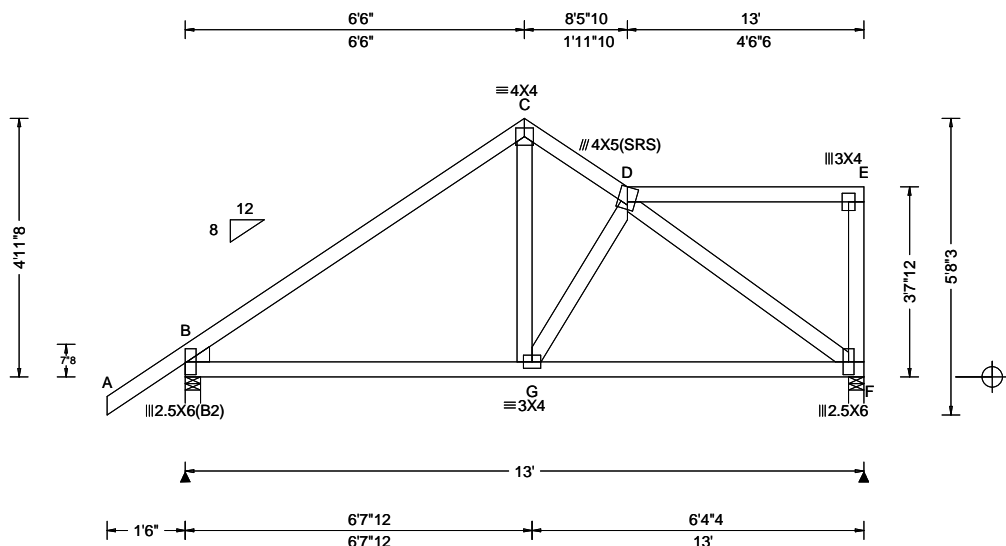
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Glenview, IL 60025

SEQN: 14083 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B07	Cust: R 215 JRRef: 1XL52150007 T42 DrwNo: 336.22.1429.05613 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 D 999 240 VERT(CL): 0.017 D 999 180 HORZ(LL): -0.005 B - - HORZ(TL): 0.008 F - - Creep Factor: 2.0 Max TC CSI: 0.433 Max BC CSI: 0.431 Max Web CSI: 0.357 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 655 - / - / 425 / 111 / 192 F 540 - / - / 301 / 95 / - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 245 -629 C - D 304 -527

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Purlins

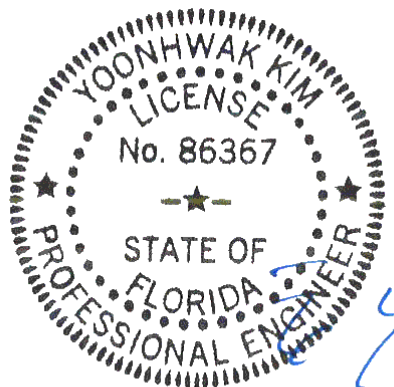
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.

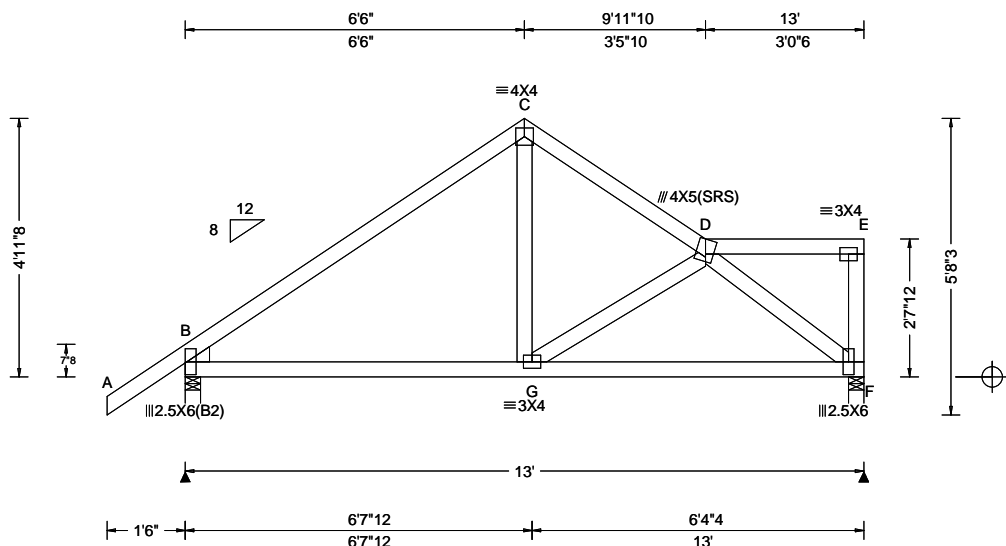


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SEQN: 14084 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B08	Cust: R 215 JRef: 1XL52150007 T47 DrwNo: 336.22.1429.13740 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 G 999 240 VERT(CL): 0.016 G 999 180 HORZ(LL): -0.005 B - - HORZ(TL): 0.009 F - - Creep Factor: 2.0 Max TC CSI: 0.429 Max BC CSI: 0.433 Max Web CSI: 0.199 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 655 - / - / 420 / 113 / 175 F 540 - / - / 292 / 90 / - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 208 -630 C - D 237 -559

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Purlins

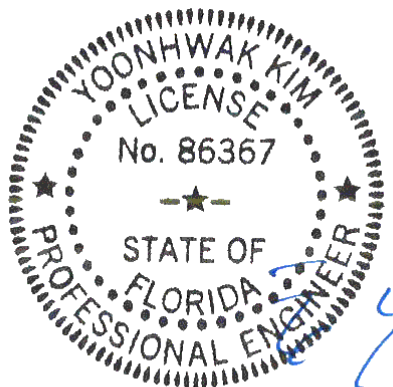
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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Wind loading based on both gable and hip roof types.



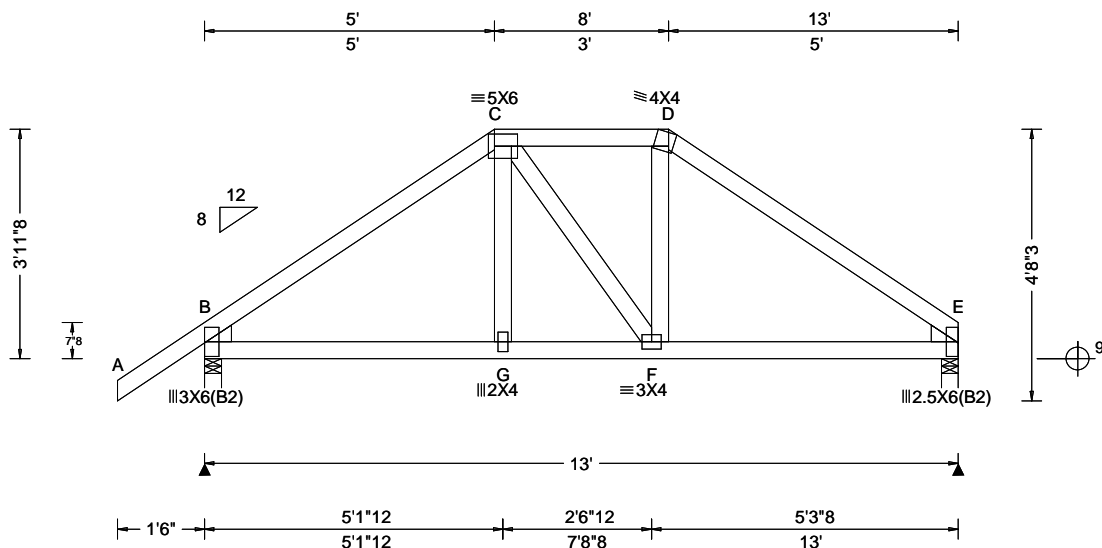
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SEQN: 14848 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: B10	Cust: R 215 JRef: 1XL52150007 T33 DrwNo: 336.22.1429.58380 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.041 G 999 240 VERT(CL): 0.083 G 999 180 HORZ(LL): 0.025 E - - HORZ(TL): 0.050 E - - Creep Factor: 2.0 Max TC CSI: 0.631 Max BC CSI: 0.817 Max Web CSI: 0.091 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1253 -/- /- /- /269 -/ E 1138 -/- /- /- /230 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 361 -1641 D - E 364 -1646 C - D 265 -1298

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Special Loads

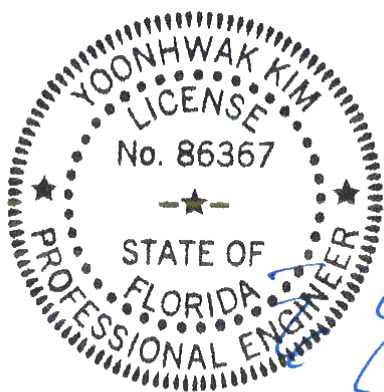
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at -1.50 to 64 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 8.00
TC: From 64 plf at 8.00 to 64 plf at 13.00
BC: From 5 plf at -1.50 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 7.97
BC: From 20 plf at 7.97 to 20 plf at 13.00
TC: 366 lb Conc. Load at 5.03, 7.97
TC: 141 lb Conc. Load at 6.50
BC: 177 lb Conc. Load at 5.03, 7.97
BC: 96 lb Conc. Load at 6.50

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



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Glenview, IL 60025

[illegible]

Lumber	Purlins	B - C	231	- 1098	F - G	166	- 434
Top chord: 2x4 SP M-31; T1 2x4 SP #2;	In lieu of structural panels use purlins to brace all flat	C - D	527	- 42	G - H	166	- 435
Bot chord: 2x4 SP #2; B3 2x4 SP M-31;	TC @ 24" oc.	D - E	1976	- 341	H - I	481	- 2075
Webs: 2x4 SP #3; W2,W4,W7,W11 2x4 SP M-31;		E - F	166	- 434	I - J	308	- 1277
W14 2x4 SP #2;	Wind						

BC: From 20 plf at 23.64 to 20 plf at 30.67	Refer to drawing CUNAILSBR1014 for more information.			
BC: From 4 plf at 30.67 to 4 plf at 32.17				
TC: 188 lb Conc. Load at 7.06, 9.06, 11.06, 13.06				
15.06, 15.60				
TC: 208 lb Conc. Load at 17.60				
TC: 213 lb Conc. Load at 19.60, 21.60, 23.60				
BC: 750 lb Conc. Load at 7.03				
BC: 129 lb Conc. Load at 9.06, 11.06, 13.06, 15.06				
15.60				
BC: 101 lb Conc. Load at 17.60				
BC: 83 lb Conc. Load at 19.60, 21.60				
BC: 710 lb Conc. Load at 23.64				

Plating Notes

All plates are 3X4 except as noted.

(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.

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Florida Department of Transportation
2020-0025

PROFESSIONAL ENGINEER

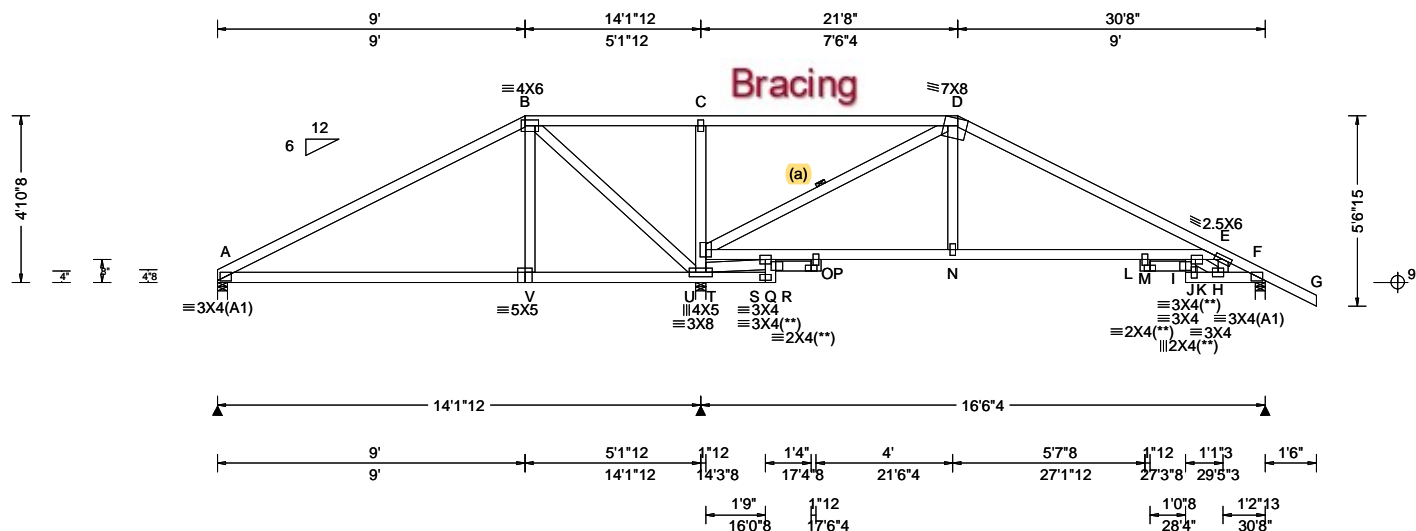
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SEQN: 14736 FROM:	HIPS Qty: 1	Job Number: 22-8612 1845F Truss Label: C02	Cust: R 215 JRef: 1XL52150007 T29 DrwNo: 336.22.1430.32207 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.07 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.263 M 746 240 VERT(CL): 0.534 M 368 180 HORZ(LL): 0.055 F - - HORZ(TL): 0.113 F - - Creep Factor: 2.0 Max TC CSI: 0.806 Max BC CSI: 0.771 Max Web CSI: 0.704 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL A 504 -/- /- /284 /115 /139 U 1530 -/- /- /818 /199 -/ F 661 -/- /- /431 /159 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) U Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A, U, & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

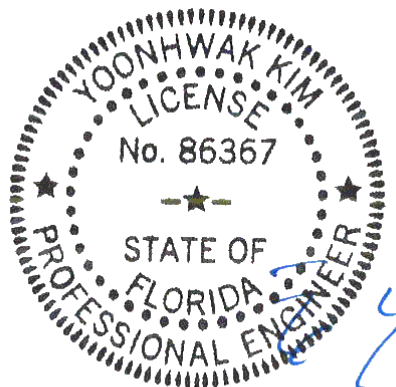
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.



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Florida Certificate of Product Approval #FL 1999

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025


FL REG# 278, Yoonhwak Kim, FL PE #86367
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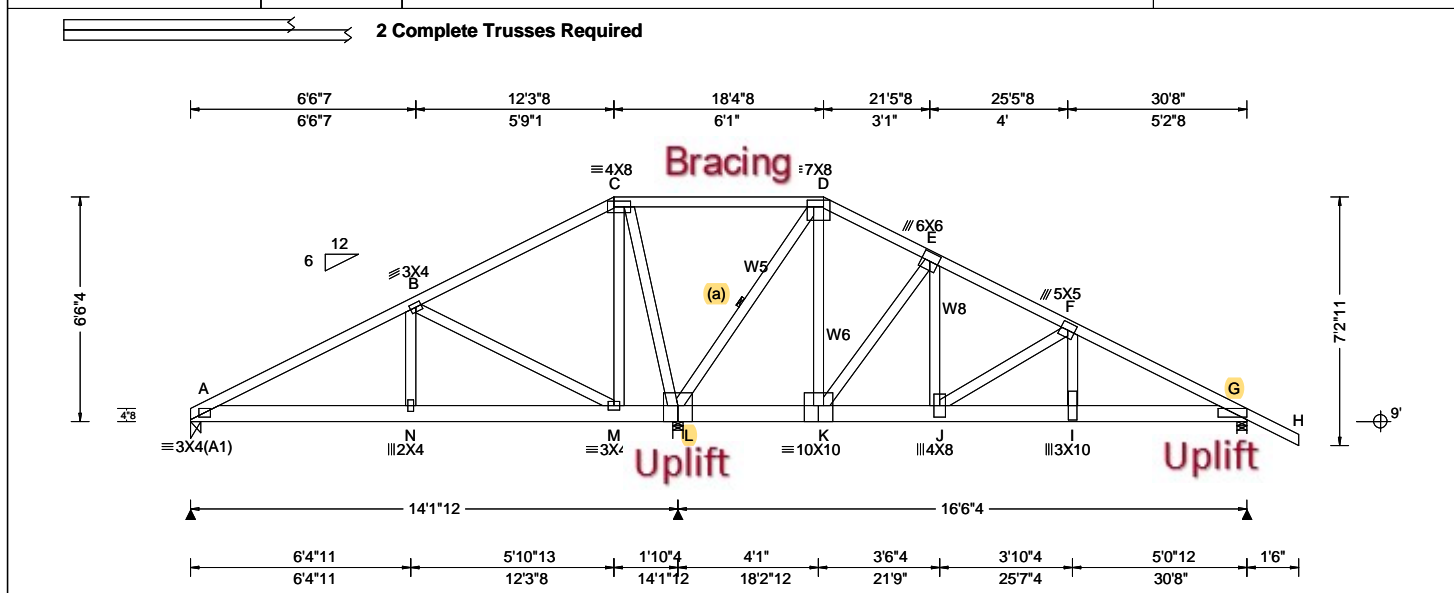
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 14812 FROM:	HIPS Ply: 2 Qty: 1	Job Number: 22-8612 1845F Truss Label: C04	Cust: R 215 JRRef: 1XL52150007 T14 DrwNo: 336.22.1430.56123 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.07 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.088 I 999 240 VERT(CL): 0.175 I 999 180 HORZ(LL): 0.021 D - - HORZ(TL): 0.041 D - - Creep Factor: 2.0 Max TC CSI: 0.717 Max BC CSI: 0.503 Max Web CSI: 0.859 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - -956 - / - /121 - / - L 9804 - / - / - /1471 - / - G 4657 - / - / - /880 - / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) L Brg Wid = 3.5 Min Req = 4.1 G Brg Wid = 3.5 Min Req = 1.9 (Truss) Bearings A, L, & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber	Wind	Maximum Bot Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; W5,W6 2x4 SP #2; W8 2x4 SP M-31;	Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.	Chords Tens.Comp. Chords Tens. Comp. A - B 1166 -187 D - E 133 -918 B - C 1391 -247 E - F 424 -2472 C - D 1384 -250 F - G 756 -4204

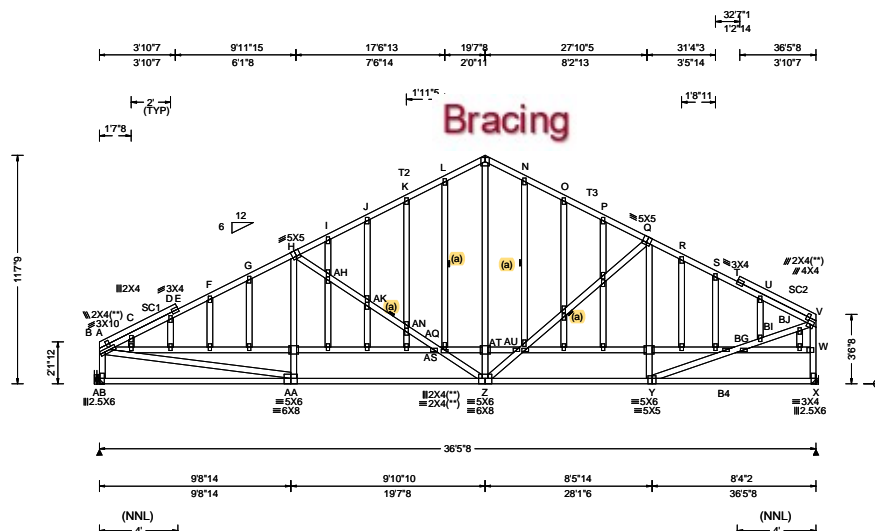
Bracing	Additional Notes	Maximum Web Forces Per Ply (lbs)
(a) Continuous lateral restraint equally spaced on member.	Negative reaction(s) of -956# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.	Chords Tens.Comp. Chords Tens. Comp. A - N 151 -1027 K - J 2066 -348 N - M 151 -1029 J - I 3684 -658 M - L 210 -1216 I - G 3736 -667 L - K 798 -110

Nailnote	Special Loads	Maximum Web Forces Per Ply (lbs)
Nail Schedule: 0.131"x3", min. nails Top Chord: 1 Row @ 12.00" o.c. Bot Chord: 1 Row @ 4.25" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.	----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC: From 62 plf at 0.00 to 62 plf at 32.17 BC: From 20 plf at 0.00 to 20 plf at 14.15 BC: From 10 plf at 14.15 to 10 plf at 27.60 BC: From 20 plf at 27.60 to 20 plf at 30.67 BC: From 4 plf at 30.67 to 4 plf at 32.17 BC: 1751 lb Conc. Load at 15.60 BC: 1658 lb Conc. Load at 17.60,19.60,25.60 BC: 1501 lb Conc. Load at 21.60,23.60 BC: 1493 lb Conc. Load at 27.60	Chords Tens.Comp. Chords Tens. Comp. C - L 139 -579 E - J 2415 -428 L - D 634 -3781 J - F 349 -1811 D - K 3362 -525 F - I 1651 -290 K - E 418 -2230

Purlins	Professional Engineer Seal	FL REG# 278, Yoonhwak Kim, FL PE #86367
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.		FL REG# 278, Yoonhwak Kim, FL PE #86367 FL 02/2022 Certificate of Product Approval #FL 1999

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SEQN: 14609 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D01	Cust: R 215 JRRef: 1XL52150007 T66 DrwNo: 336.22.1431.09760 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.73 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.65 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.160 F 999 240 VERT(CL): 0.324 F 999 180 HORZ(LL): 0.069 F - - HORZ(TL): 0.140 F - - Creep Factor: 2.0 Max TC CSI: 0.764 Max BC CSI: 0.842 Max Web CSI: 0.720 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AB 1493 -/- /- /- /349 -/ X 1553 -/- /- /- /367 -/ Wind reactions based on MWFRS AB Brg Wid = - Min Req = - X Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - C 511 -2152 L - M 357 -1480 C - D 509 -2145 M - N 357 -1482 D - E 514 -2159 N - O 357 -1491 E - F 503 -2131 O - P 359 -1495 F - G 501 -2126 P - Q 369 -1518 G - H 488 -2091 Q - R 399 -1692 H - I 382 -1554 R - S 411 -1720 I - J 362 -1503 S - T 402 -1696 J - K 360 -1497 T - U 413 -1724 K - L 358 -1495 U - V 403 -1697

Lumber

Top chord: 2x4 SP M-31; T2,T3 2x4 SP #2;
Bot chord: 2x4 SP M-31; B4 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 31 plf at 0.00 to 31 plf at 36.46
BC: From 10 plf at 0.00 to 10 plf at 36.46
BC: 86 lb Conc. Load at 1.94, 3.94, 5.94, 7.94
9.94, 11.94, 13.94, 15.94, 17.94, 19.94, 21.94, 23.94
25.94, 27.94, 29.94, 31.94, 33.94, 35.94

Plating Notes

All plates are 2X4 except as noted.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

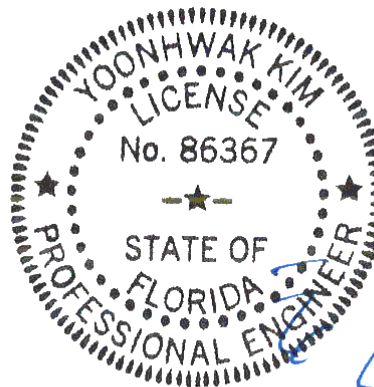
Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
AA - Z	1562 -353	Z - Y	1195 -258

Maximum Web Forces Per Ply (lbs)

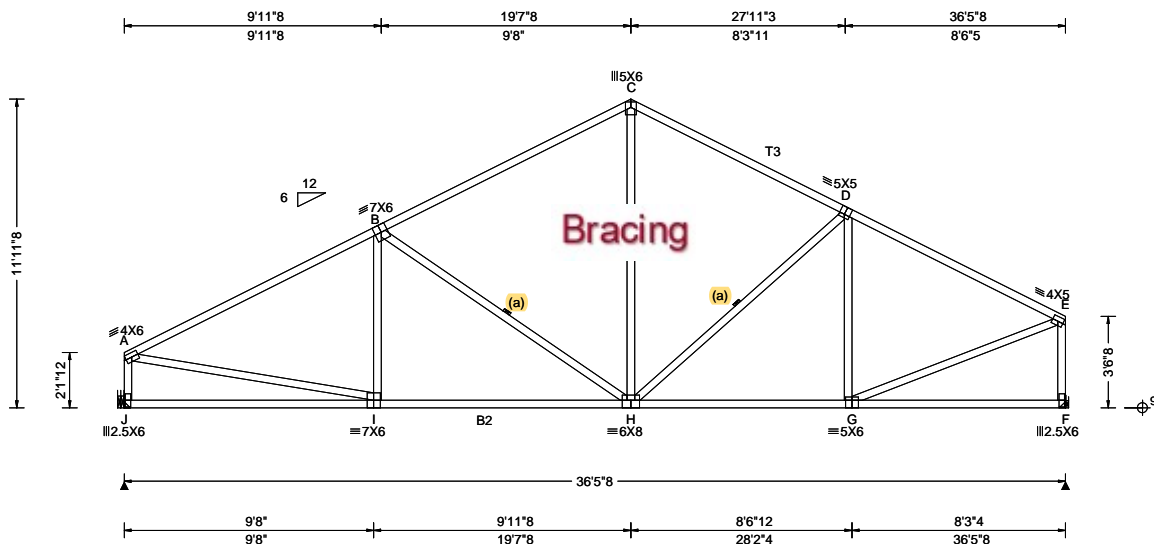
Webs	Tens.Comp.	Webs	Tens. Comp.
B - AB	330 -1317	Z - AU	128 -415
B - AA	1507 -340	AT - Z	1184 -273
H - AH	159 -681	AT - AU	457 -160
AH - AK	151 -661	Y - BG	1244 -265
AK - AN	156 -674	BG - BI	1577 -379
AN - AQ	164 -695	BI - BJ	1597 -386
AQ - AS	460 -160	BJ - V	1589 -383
AQ - Z	234 -833	W - V	327 -1328
AS - AT	460 -160	W - X	334 -1347
M - AT	1187 -276		

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Glenview, IL 60025

SEQN: 14596 FROM:	SPEC	Ply: 1 Qty: 3	Job Number: 22-8612 1845F Truss Label: D02	Cust: R 215 JRRef: 1XL52150007 T27 DrwNo: 336.22.1431.12480 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.05 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.65 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.086 H 999 240 VERT(CL): 0.160 H 999 180 HORZ(LL): 0.025 F - - HORZ(TL): 0.047 F - - Creep Factor: 2.0 Max TC CSI: 0.830 Max BC CSI: 0.858 Max Web CSI: 0.724 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL J 1658 - / - / 877 / 263 / 256 F 1669 - / - / 840 / 270 / - Wind reactions based on MWFRS J Brg Wid = - Min Req = - F Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 467 - 2289 C - D 490 - 1658 B - C 485 - 1686 D - E 401 - 1849

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

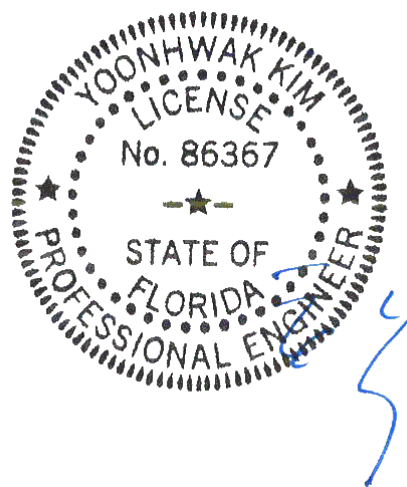
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

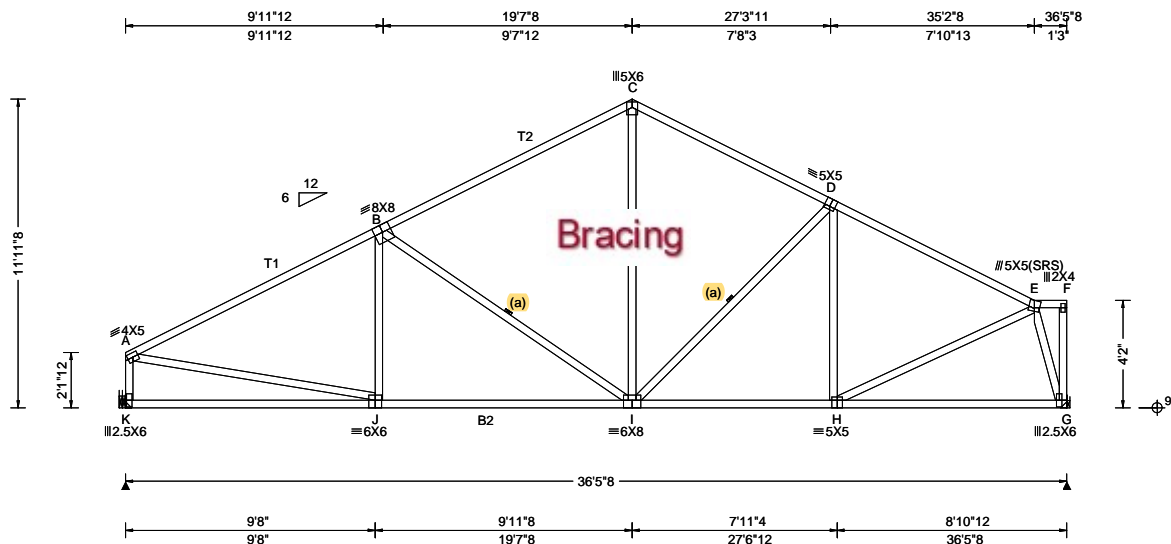


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14594 FROM:	SPEC	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D03	Cust: R 215 JRRef: 1XL52150007 T72 DrwNo: 336.22.1431.15487 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.05 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.65 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.068 B 999 240 VERT(CL): 0.139 B 999 180 HORZ(LL): 0.023 G - - HORZ(TL): 0.047 G - - Creep Factor: 2.0 Max TC CSI: 0.671 Max BC CSI: 0.814 Max Web CSI: 0.631 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL K 1501 - / - / 878 / 260 / 256 G 1501 - / - / 822 / 273 / - Wind reactions based on MWFRS K Brg Wid = - Min Req = - G Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 464 - 2024 C - D 489 - 1455 B - C 482 - 1497 D - E 410 - 1631

Lumber

Top chord: 2x4 SP #2; T1, T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

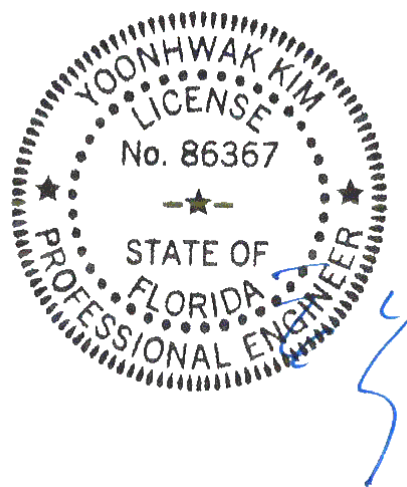
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

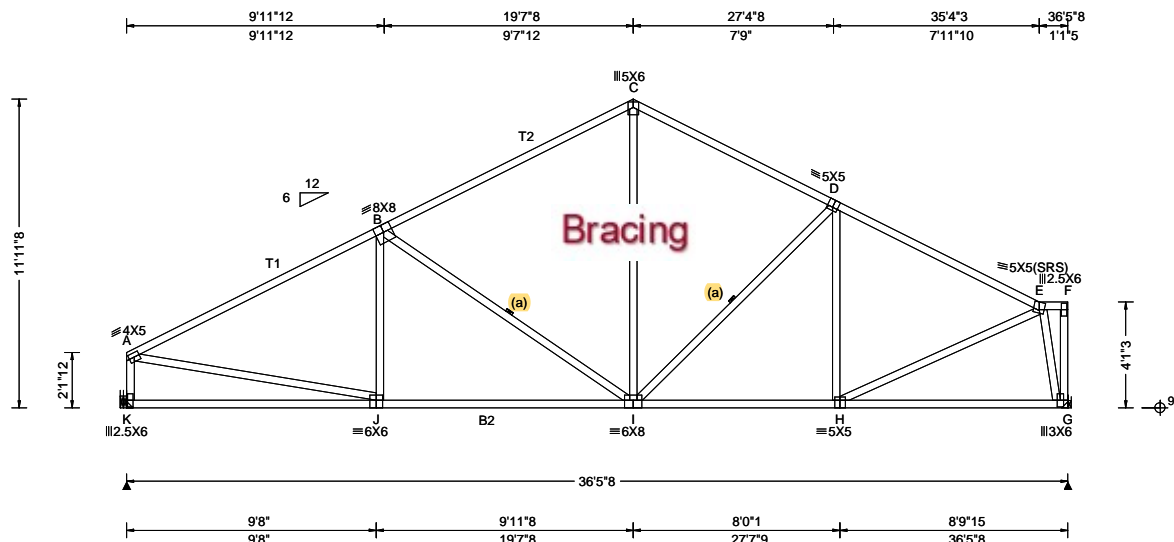


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SEQN: 14591 FROM:	SPEC	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D04	Cust: R 215 JRRef: 1XL52150007 T71 DrwNo: 336.22.1431.18730 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.05 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.65 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.068 B 999 240 VERT(CL): 0.139 B 999 180 HORZ(LL): 0.023 G - - HORZ(TL): 0.047 G - - Creep Factor: 2.0 Max TC CSI: 0.684 Max BC CSI: 0.813 Max Web CSI: 0.631 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K 1501 -/- /- /878 /261 /256 G 1501 -/- /- /824 /273 -/ Wind reactions based on MWFRS K Brg Wid = - Min Req = - G Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 464 -2024 C - D 489 -1457 B - C 483 -1498 D - E 410 -1631

Lumber

Top chord: 2x4 SP #2; T1, T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

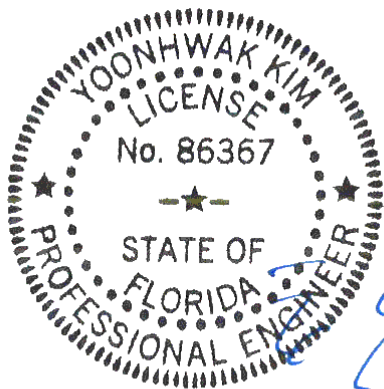
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	1710 -408	H - G	474 -146
I - H	1374 -283		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - K	365 -1420	C - I	727 -153
A - J	1656 -296	H - E	994 -155
B - I	250 -614	E - G	534 -1654

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

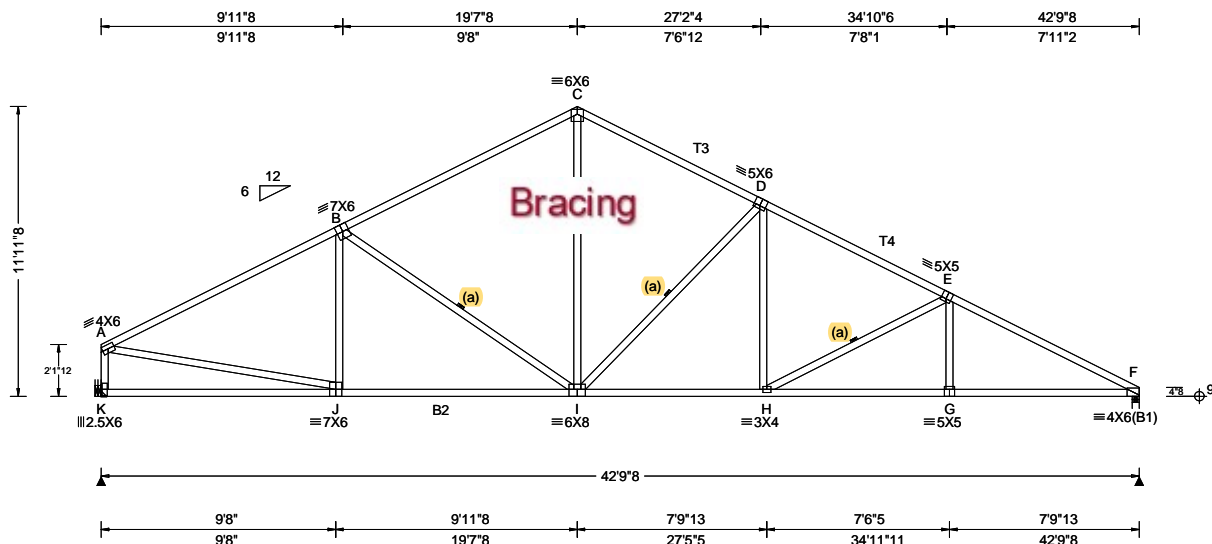
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14809 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D05	Cust: R 215 JRRef: 1XL52150007 T17 DrwNo: 336.22.1431.21293 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.17 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.28 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.164 H 999 240 VERT(CL): 0.338 H 999 180 HORZ(LL): 0.060 F - - HORZ(TL): 0.124 F - - Creep Factor: 2.0 Max TC CSI: 0.800 Max BC CSI: 0.846 Max Web CSI: 0.770 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL K 1751 - / - / - /1009 /32 /314 F 1761 - / - / - /1064 /31 - / - Wind reactions based on MWFRS K Brg Wid = - Min Req = - F Brg Wid = 3.5 Min Req = 2.1 (Truss) Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 556 -2427 D - E 658 -2624 B - C 593 -1972 E - F 713 -3288 C - D 602 -1926

Lumber

Top chord: 2x4 SP M-31; T3, T4 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

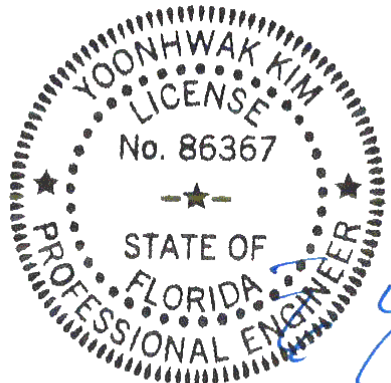
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL 1999

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
J - I	2073 -361	H - G	2848 -551
I - H	2238 -354	G - F	2850 -550

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - K	420 -1669	I - D	293 -873
A - J	2021 -380	D - H	541 -37
B - I	233 -539	H - E	240 -677
C - I	1113 -247		

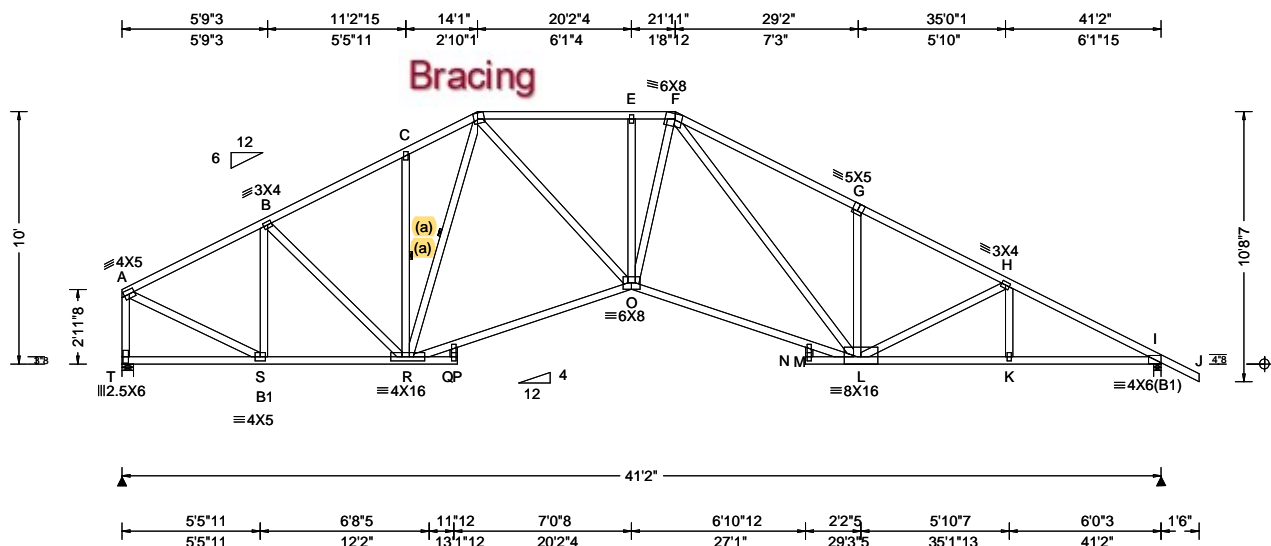
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14259 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D07	Cust: R 215 JRef: 1XL52150007 T12 DrwNo: 336.22.1431.30180 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.12 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.172 N 999 240 VERT(CL): 0.350 N 999 180 HORZ(LL): 0.074 I - - HORZ(TL): 0.151 I - - Creep Factor: 2.0 Max TC CSI: 0.756 Max BC CSI: 0.648 Max Web CSI: 0.847 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL T 1695 - / - / - / 955 / 30 / 316 I 1812 - / - / - / 1122 / 87 / - Non-Gravity Wind reactions based on MWFRS T Brg Wid = 5.5 Min Req = 2.0 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings T & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 553 - 1777 E - F 880 - 2623 B - C 694 - 1988 F - G 959 - 2810 C - D 780 - 1933 G - H 799 - 2768 D - E 880 - 2623 H - I 819 - 3198

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

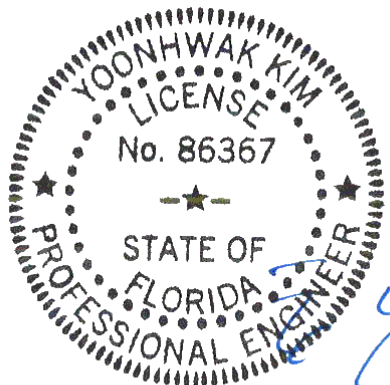
Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C. Max. including a lateral brace at chord ends.



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Florida Certificate of Product Approval #FL 1999

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
S - R	1556 - 333	M - L	2503 - 465
R - Q	1872 - 362	L - K	2780 - 633
Q - O	1926 - 365	K - I	2781 - 632
O - M	2557 - 468		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - T	487 - 1645	O - F	893 - 207
A - S	1682 - 472	G - L	269 - 447
S - B	265 - 629	L - H	137 - 417
D - O	1227 - 270		

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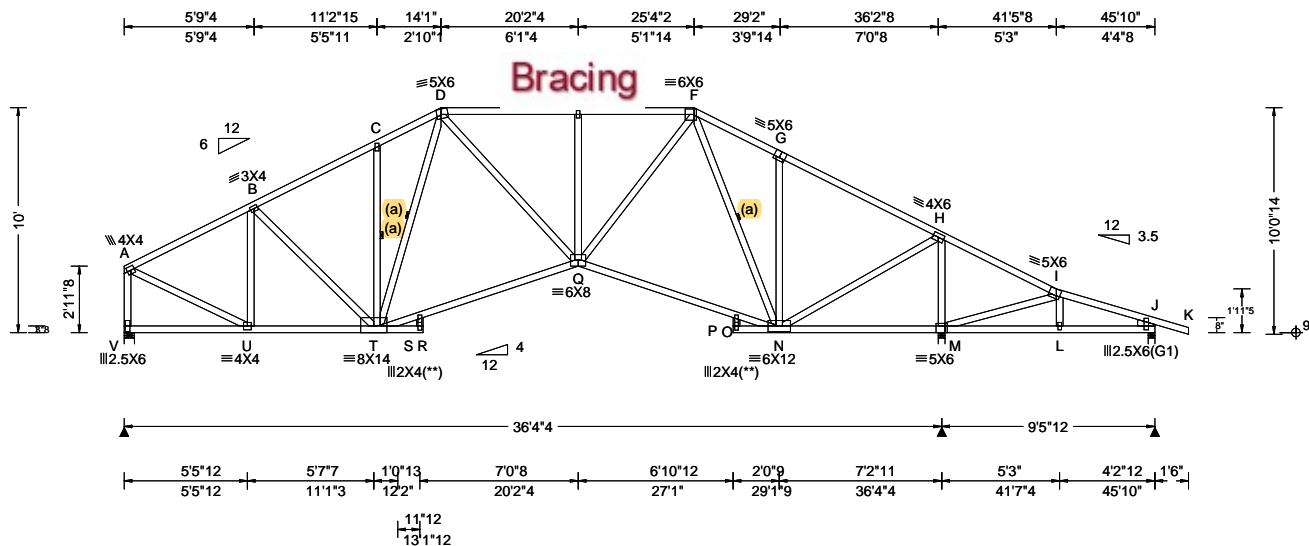
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 10486 FROM:	COMN Qty: 4	Ply: 1 Qty: 4	Job Number: 22-8612 1845F Truss Label: D09	Cust: R 215 JRRef: 1XL52150007 T61 DrwNo: 336.22.1432.10220 / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.58 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.107 R 999 240 VERT(CL): 0.281 R 999 180 HORZ(LL): 0.046 N - - HORZ(TL): 0.096 N - - Creep Factor: 2.0 Max TC CSI: 0.616 Max BC CSI: 0.894 Max Web CSI: 0.735 VIEW Ver: 22.01.01A.0520.12	Gravity Loc R+ / R- / Rh / Rw / U / RL V 1453 - / - / - /857 /38 /257 M 2229 - / - / - /1295 /112 - J 291 - / - / - /156 /51 - Non-Gravity V Brg Wid = 5.5 Min Req = 1.7 (Truss) M Brg Wid = 3.5 Min Req = 2.6 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings V, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x4 SP #3;

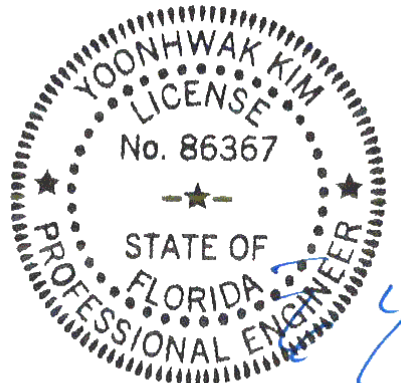
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
U - T	1306 -310	Q - O	1310 -234
T - S	1464 -321	O - N	1260 -230
S - Q	1516 -324	N - M	243 -449

Maximum Web Forces Per Ply (lbs)

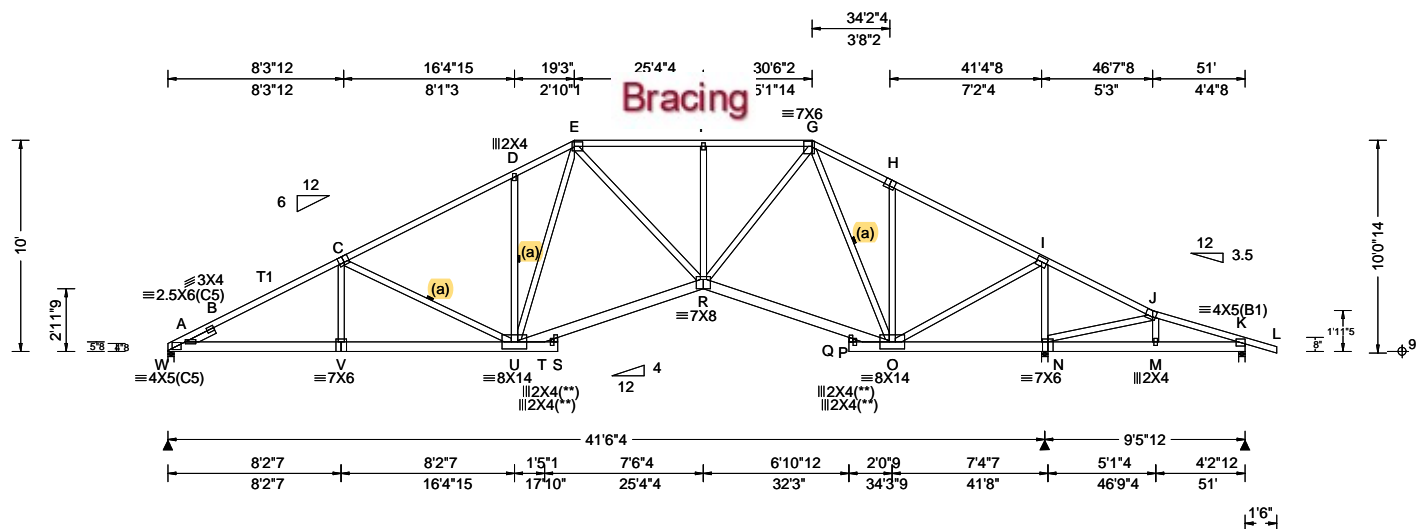
Webs	Tens.Comp.	Webs	Tens. Comp.
A - V	445 -1404	F - N	175 -711
U - B	225 -505	N - G	257 -377
D - Q	734 -177	N - H	1591 -410
E - Q	304 -384	H - M	638 -1970
Q - F	1133 -321	M - I	194 -500

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Florida Certificate of Product Approval #FL 1999

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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14748 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D10	Cust: R 215 JRRef: 1XL52150007 T41 DrwNo: 336.22.1432.18780 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.10 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.130 S 999 240 VERT(CL): 0.268 S 999 180 HORZ(LL): 0.046 O - - HORZ(TL): 0.096 O - - Creep Factor: 2.0 Max TC CSI: 0.780 Max BC CSI: 0.258 Max Web CSI: 0.801 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL W 1652 -/- /- /1027 /52 /280 N 2546 -/- /- /1457 /55 -/- K 242 -/104 -/- /140 /68 -/- Wind reactions based on MWFRS W Brg Wid = 3.5 Min Req = 1.5 (Truss) N Brg Wid = 3.5 Min Req = 2.1 (Truss) K Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings W, N, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

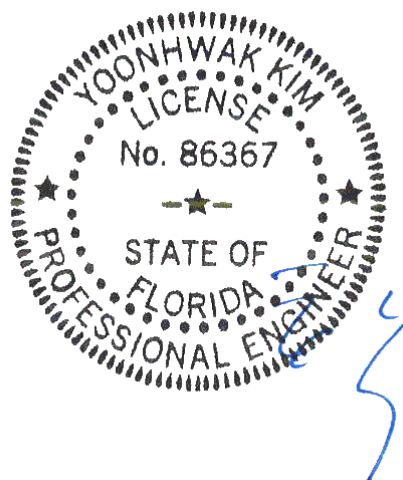
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 5X6 except as noted.
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.

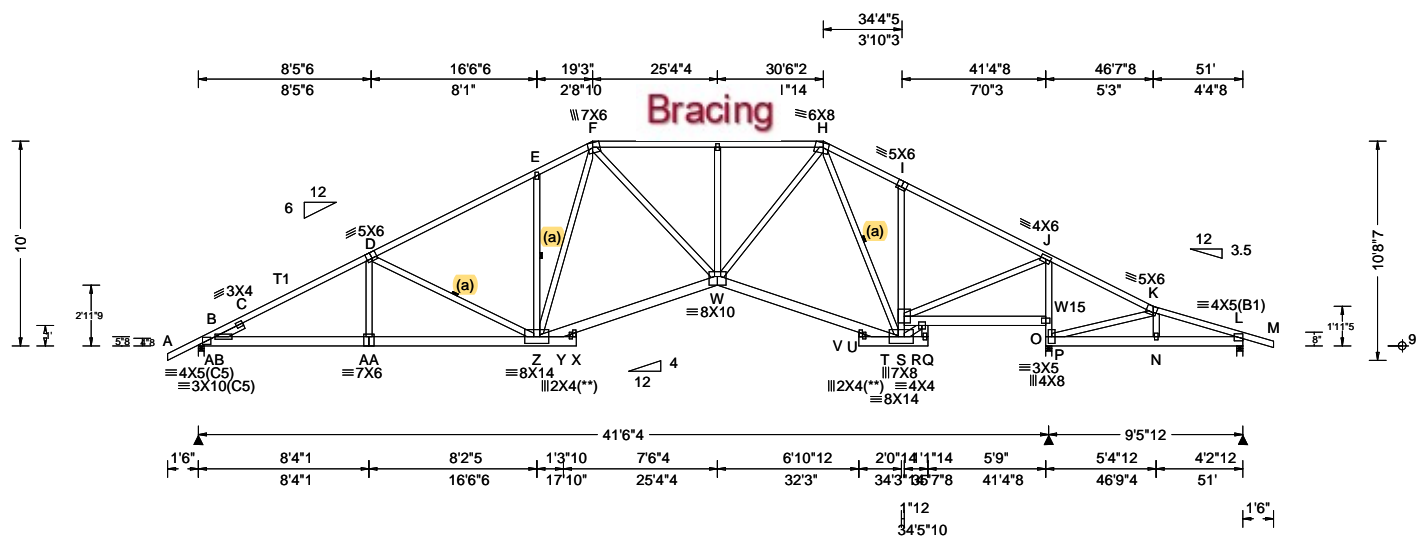


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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14243 FROM:	COMN Qty: 5	Ply: 1 Job Number: 22-8612 1845F Truss Label: D11	Cust: R 215 JRef: 1XL52150007 T60 DrwNo: 336.22.1432.23360 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.10 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.158 X 999 240 VERT(CL): 0.323 X 999 180 HORZ(LL): 0.082 S - - HORZ(TL): 0.167 S - - Creep Factor: 2.0 Max TC CSI: 0.791 Max BC CSI: 0.282 Max Web CSI: 0.754 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AB 1784 -/- /1134 /69 /296 O 2333 -/- /1329 /28 -/- L 365 -/- /242 /63 -/- Non-Gravity AB Brg Wid = 3.5 Min Req = 1.5 (Truss) O Brg Wid = 3.5 Min Req = 1.9 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AB, O, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W15 2x4 SP M-31;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

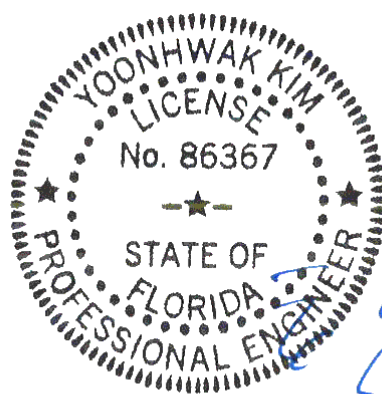
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida State Seal of Product Approval #FL 1999

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	869 -3092	G - H	931 -2377
C - D	865 -3025	H - I	695 -1446
D - E	801 -2344	I - J	565 -1501
E - F	939 -2289	J - K	588 -58
F - G	931 -2377		

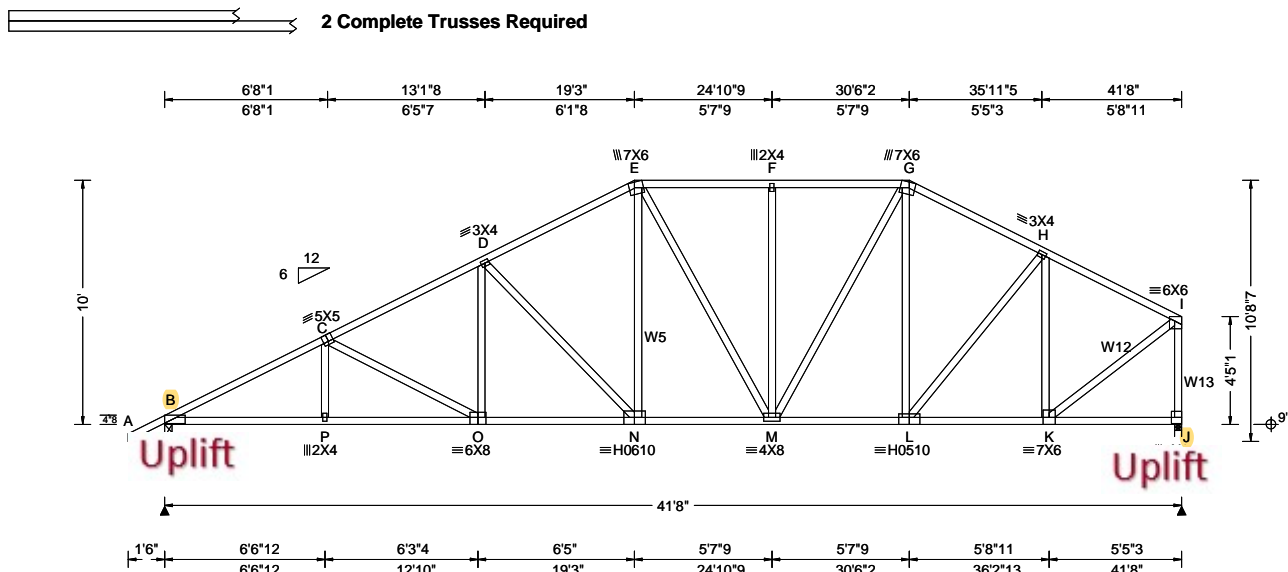
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - Z	235 -730	S - J	1738 -409
E - Z	282 -385	R - Q	32 -462
F - W	691 -191	J - P	593 -1936
W - H	1404 -411	P - O	609 -2177
H - T	197 -786	O - K	183 -511
T - R	1400 -318		

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14410 FROM:	COMN Ply: 2 Qty: 1	Job Number: 22-8612 1845F Truss Label: D12	Cust: R 215 JRef: 1XL52150007 T65 DrwNo: 336.22.1432.34473 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.210 N 999 240 VERT(CL): 0.414 N 999 180 HORZ(LL): 0.082 K - - HORZ(TL): 0.161 K - - Creep Factor: 2.0 Max TC CSI: 0.358 Max BC CSI: 0.830 Max Web CSI: 0.786 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 4692 -/- /- /- /770 -/ J 7946 -/- /- /- /1245 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.9 (Truss) J Brg Wid = 3.5 Min Req = 3.3 (Truss) Bearings B & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 743 -4637 F - G 598 -3813 C - D 721 -4514 G - H 586 -3678 D - E 662 -4154 H - I 502 -3165 E - F 598 -3813

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W5, W13 2x4 SP #2;
W12 2x4 SP M-31;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 6.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Special Loads

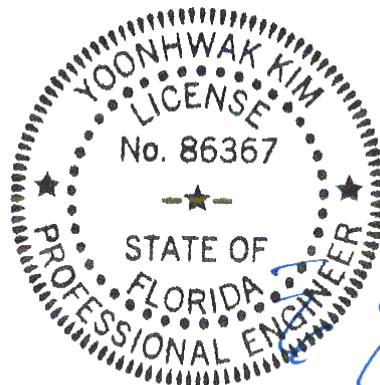
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at -1.50 to 62 plf at 41.67
BC: From 4 plf at -1.50 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 16.60
BC: From 10 plf at 16.60 to 10 plf at 41.67
BC: 720 lb Conc. Load at 16.60, 18.60, 20.60, 22.60
24.60, 26.60, 28.60, 30.60, 32.60, 34.60, 36.60, 38.60
40.60

Purlins

In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

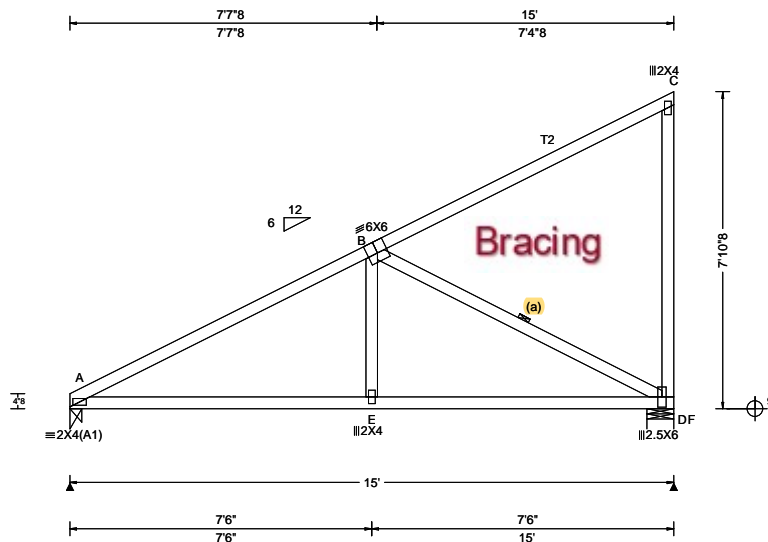


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Florida Certificate of Product Approval #FL 1999

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14864 FROM:	MONO Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D13	Cust: R 215 JRef: 1XL52150007 T23 DrwNo: 336.22.1432.37737 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.016 E 999 240 VERT(CL): 0.034 E 999 180 HORZ(LL): 0.008 A - - HORZ(TL): 0.016 A - - Creep Factor: 2.0 Max TC CSI: 0.711 Max BC CSI: 0.746 Max Web CSI: 0.302 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 622 - / - / - /394 /38 /264 F 610 - / - / - /437 /173 - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 8.0 Min Req = 1.5 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 108 -887

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

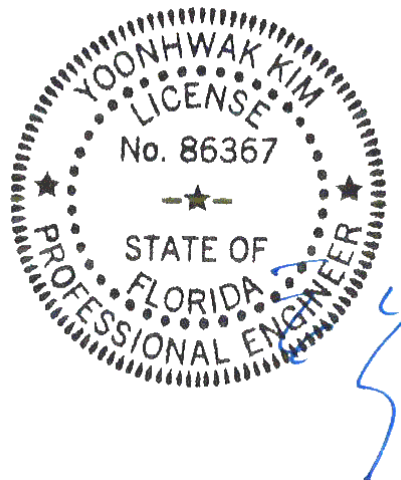
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - E	719 -369	E - D	715 -370

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
B - D	418 -806



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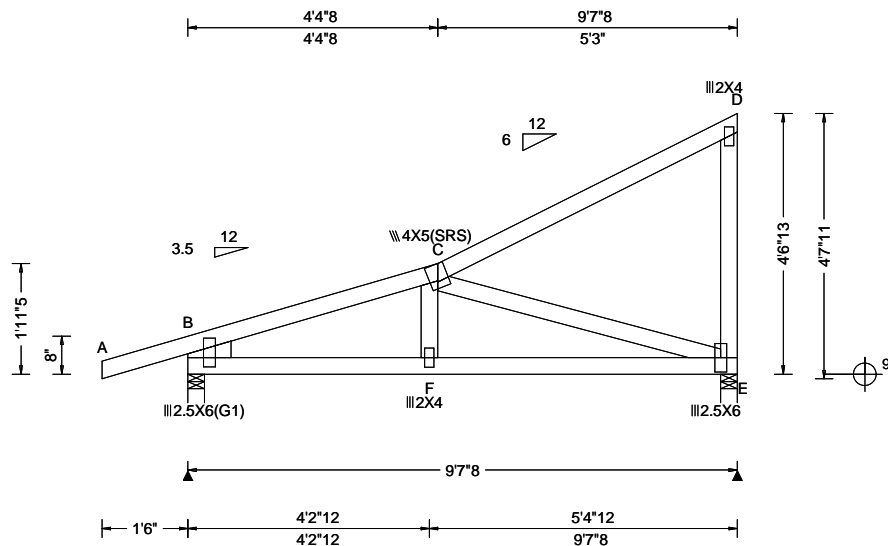
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14473 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D14	Cust: R 215 JRef: 1XL52150007 T16 DrwNo: 336.22.1432.39973 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.019 F 999 240 VERT(CL): 0.037 F 999 180 HORZ(LL): -0.007 D - - HORZ(TL): 0.014 D - - Creep Factor: 2.0 Max TC CSI: 0.395 Max BC CSI: 0.362 Max Web CSI: 0.408 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 496 /- /- /302 /71 /179 E 387 /- /- /274 /58 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 350 -702

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

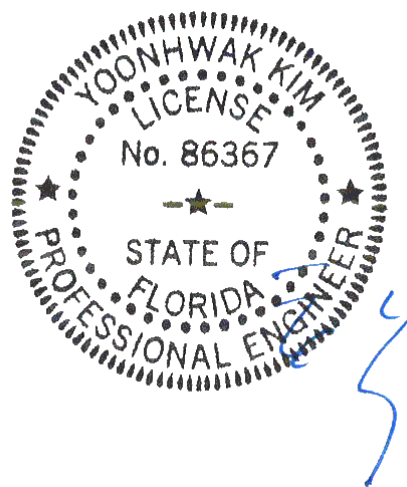
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - F	634 -511	F - E	626 -516

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
C - E	496 -656



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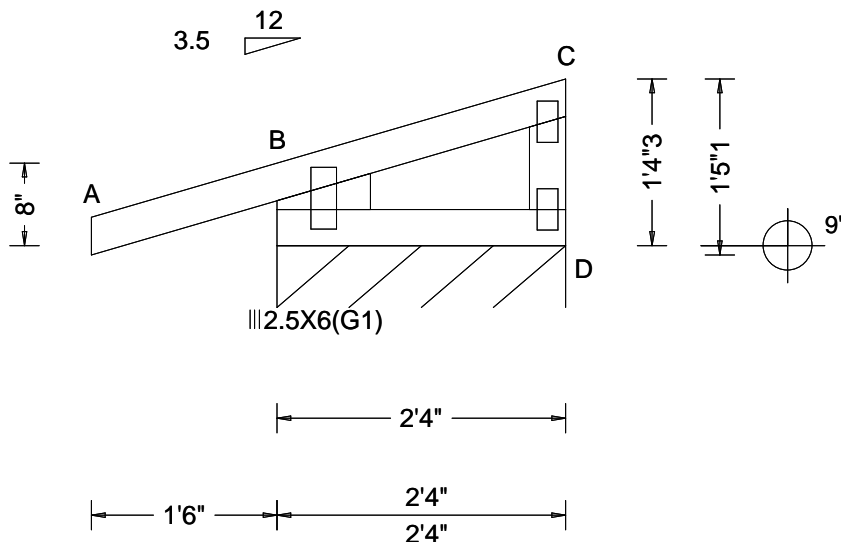
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14098 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: D15	Cust: R 215 JRef: 1XL52150007 T9 DrwNo: 336.22.1432.41933 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	D* 123	/-	/-	/80	/28	/16	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 B - -	D Brg Wid = 28.0 Min Req = -						
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 C - -	Bearing B is a rigid surface.						
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.148							
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.041							
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.045							
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)								
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 21.02.01.1216.15							
	GCpi: 0.18	WAVE								
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;

Plating Notes

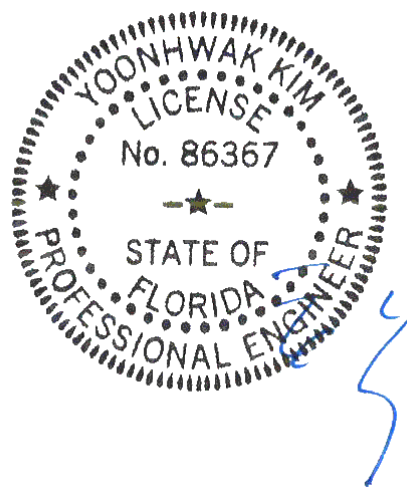
All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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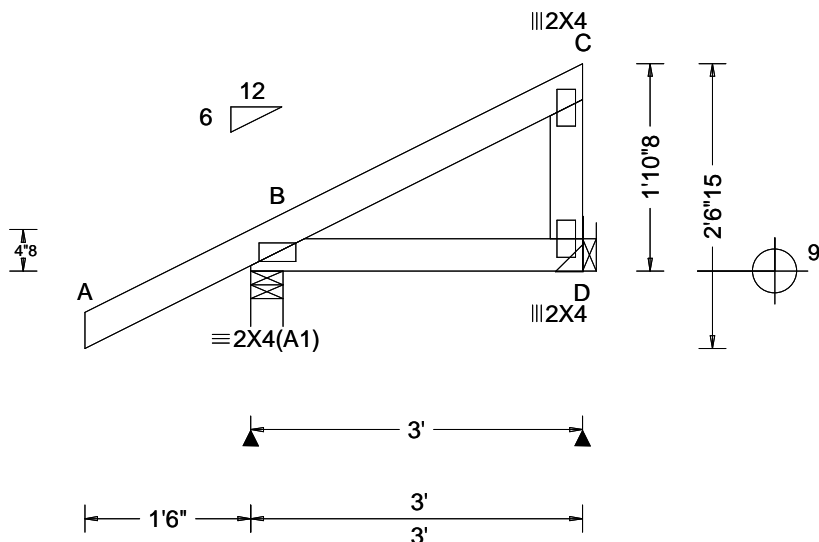
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14416 FROM:	MONO Ply: 1 Qty: 18	Job Number: 22-8612 1845F Truss Label: E01	Cust: R 215 JRef: 1XL52150007 T24 DrwNo: 336.22.1432.44757 KD / YK 12/02/2022
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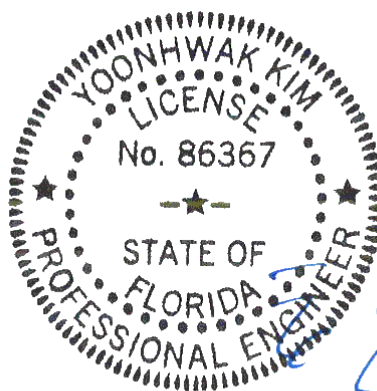
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.250 Max BC CSI: 0.062 Max Web CSI: 0.063 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 262 /- /- /190 /42 /73 D 86 /- /- /57 /25 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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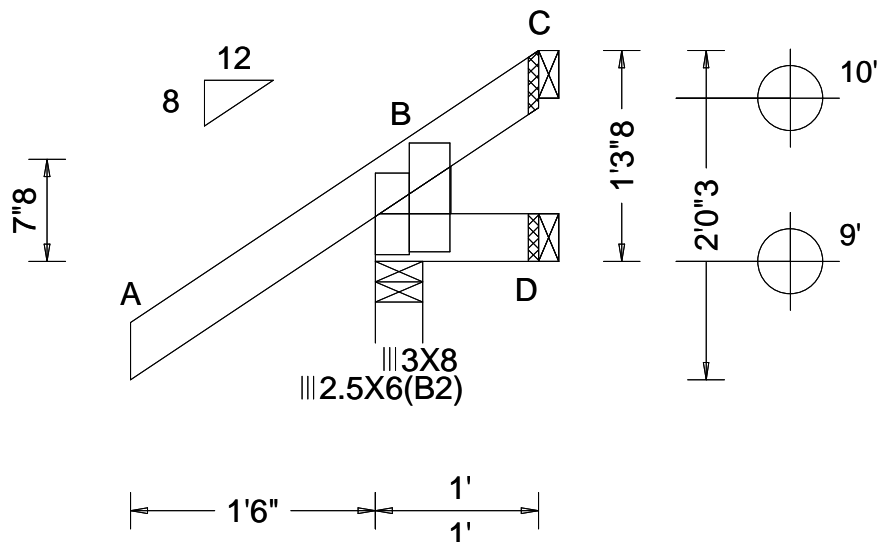
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14839 FROM:	JACK Ply: 1 Qty: 4	Job Number: 22-8612 1845F Truss Label: J01	Cust: R 215 JRef: 1XL52150007 T36 DrwNo: 336.22.1432.47250 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.261 Max BC CSI: 0.037 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 223 /- /- /185 /50 /53 D 5 /-18 /- /16 /18 /- C - /-18 /- /21 /28 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

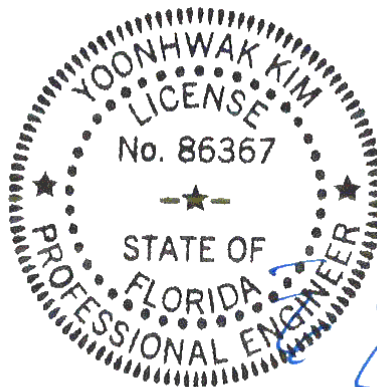
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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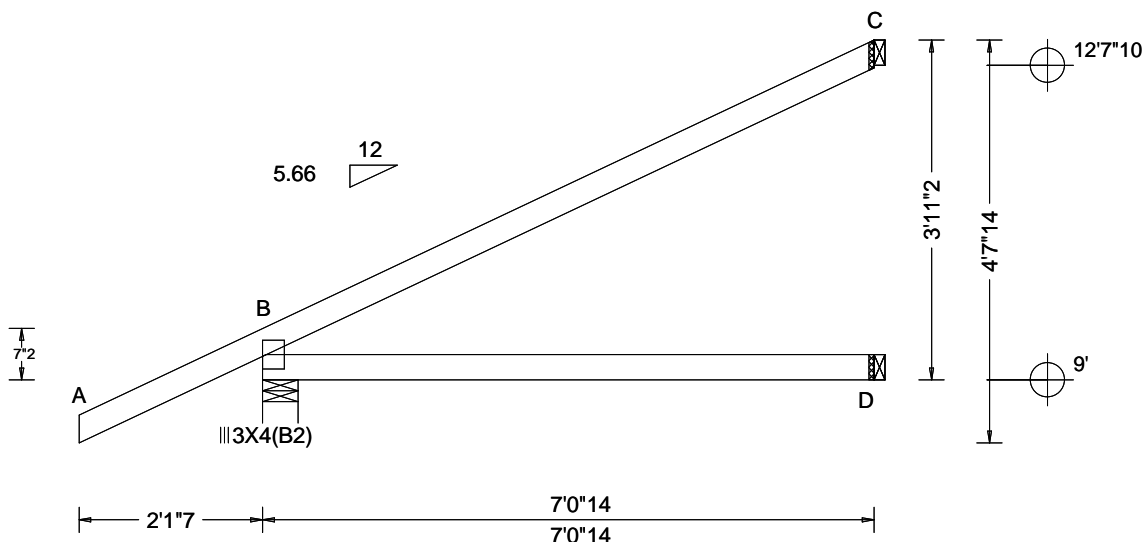
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14843 FROM:	HIP_	Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: J01HJ	Cust: R 215 JRef: 1XL52150007 T40 DrwNo: 336.22.1432.51367 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 B - - HORZ(TL): 0.012 B - - Creep Factor: 2.0 Max TC CSI: 0.793 Max BC CSI: 0.322 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 308 - / - / - /65 - D 81 - / - /34 - / - C 225 - / - / - /80 - Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

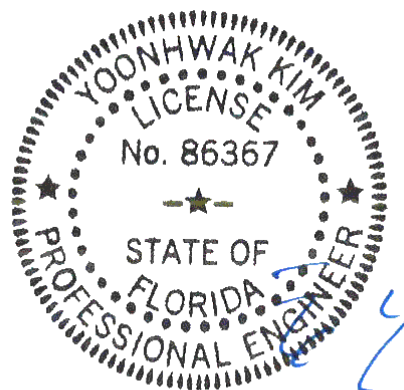
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Loading

Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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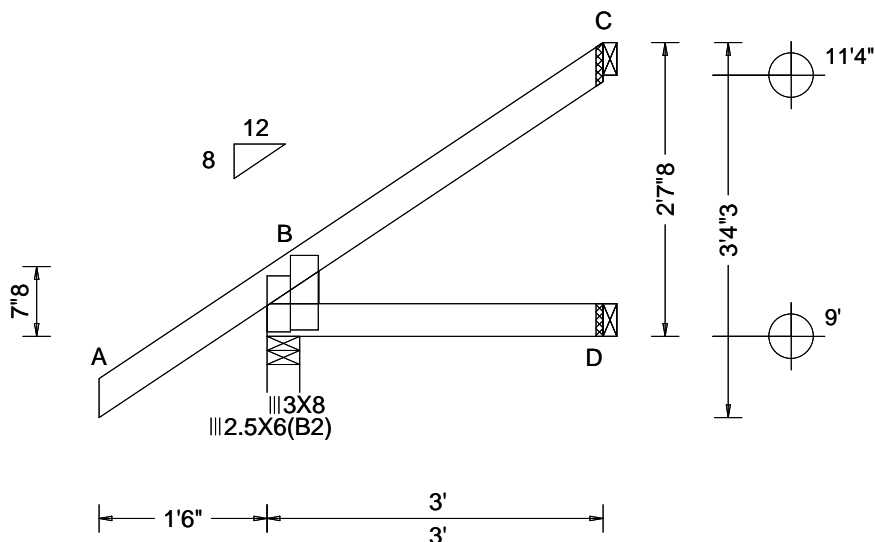
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14841 FROM:	JACK Ply: 1 Qty: 4	Job Number: 22-8612 1845F Truss Label: J02	Cust: R 215 JRef: 1XL52150007 T35 DrwNo: 336.22.1432.55540 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.261 Max BC CSI: 0.073 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 255 /- /- /191 /26 /100 D 53 /- /- /34 /- /- C 73 /- /- /52 /46 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

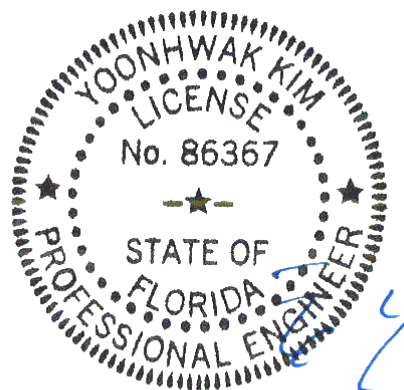
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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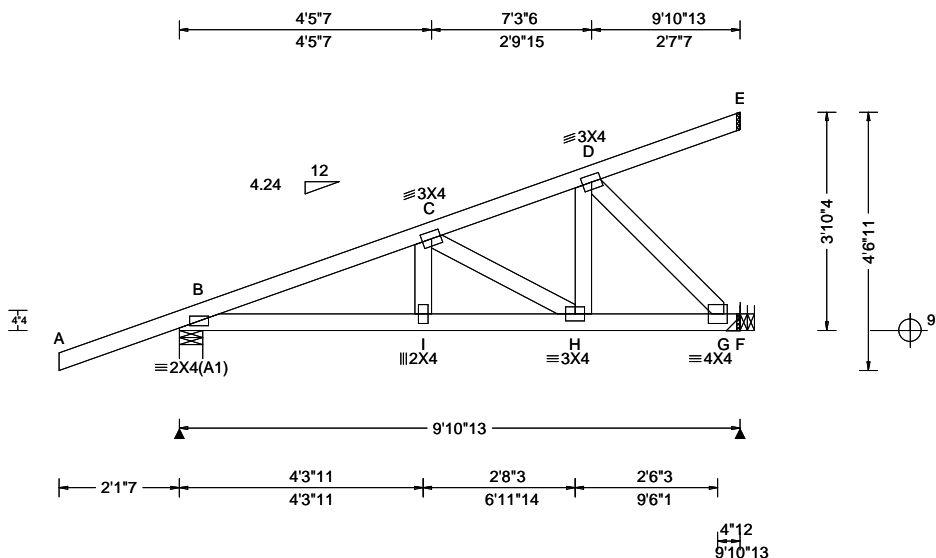
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14653 FROM:	HIP_	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: J02HJ	Cust: R 215 JRef: 1XL52150007 T3 DrwNo: 336.22.1432.57933 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.176 E 664 240 VERT(CL): 0.354 E 330 180 HORZ(LL): 0.059 E - - HORZ(TL): 0.118 E - - Creep Factor: 2.0 Max TC CSI: 0.461 Max BC CSI: 0.197 Max Web CSI: 0.164 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 461 /- /- /- /96 /- F 621 /- /- /- /105 /- Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 97 -645 C - D 125 -605

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Loading

Hipjack supports 7-0-0 setback jacks with no webs.

Wind

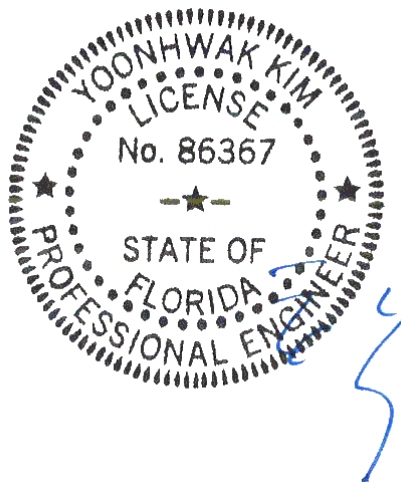
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	577 -80	H - G	574 -131
I - H	576 -82		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
D - G	198 -857



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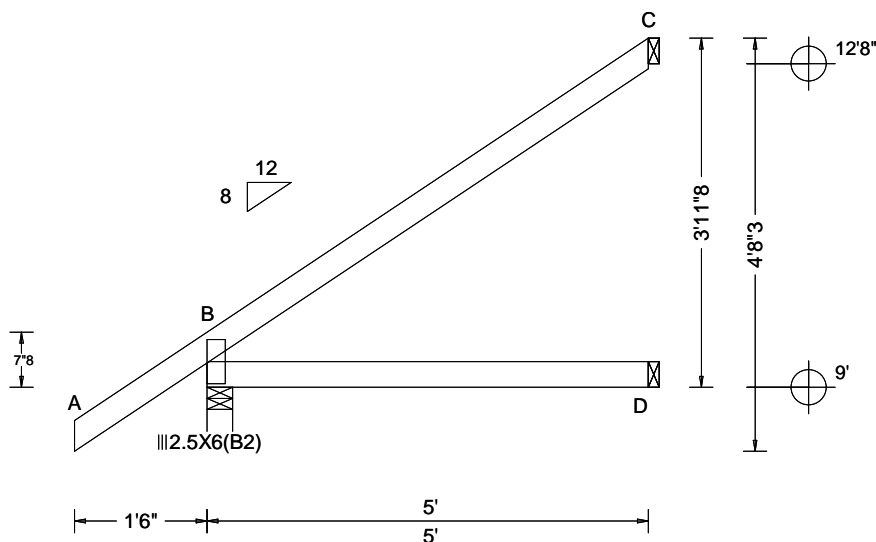
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14161 FROM:	EJAC Ply: 1 Qty: 3	Job Number: 22-8612 1845F Truss Label: J03	Cust: R 215 JRef: 1XL52150007 T38 DrwNo: 336.22.1433.07047 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.005 C - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.457 Max BC CSI: 0.274 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 329 - / - / - /234 /20 /147 D 96 - / - / - /52 - / - C 141 - / - / - /102 /88 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

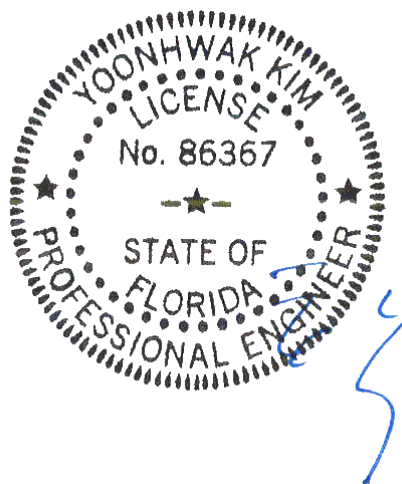
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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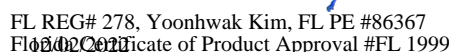
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Wind loading based on both gable and hip roof types.



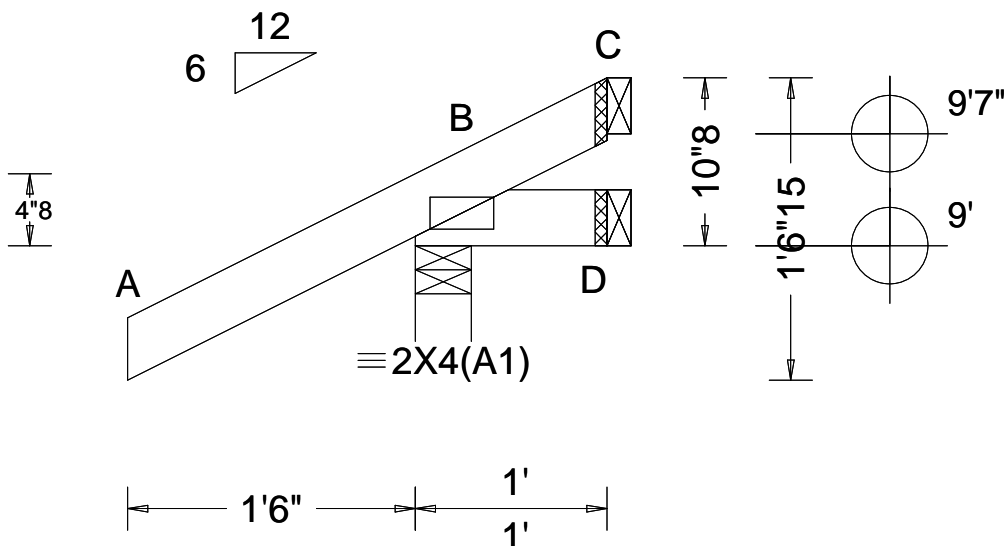
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SEQN: 14301 FROM:	JACK Ply: 1 Qty: 4	Job Number: 22-8612 1845F Truss Label: J04	Cust: R 215 JRef: 1XL52150007 T28 DrwNo: 336.22.1433.12083 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.250 Max BC CSI: 0.037 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 254 /- /- /201 /69 /38 D 5 /-16 /- /15 /15 /- C - /-55 /- /34 /52 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

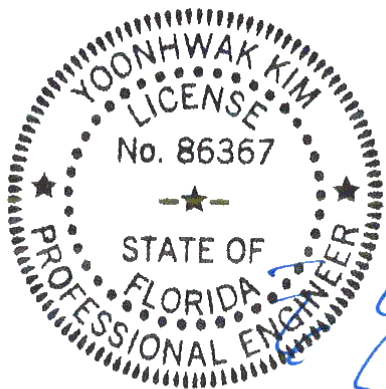
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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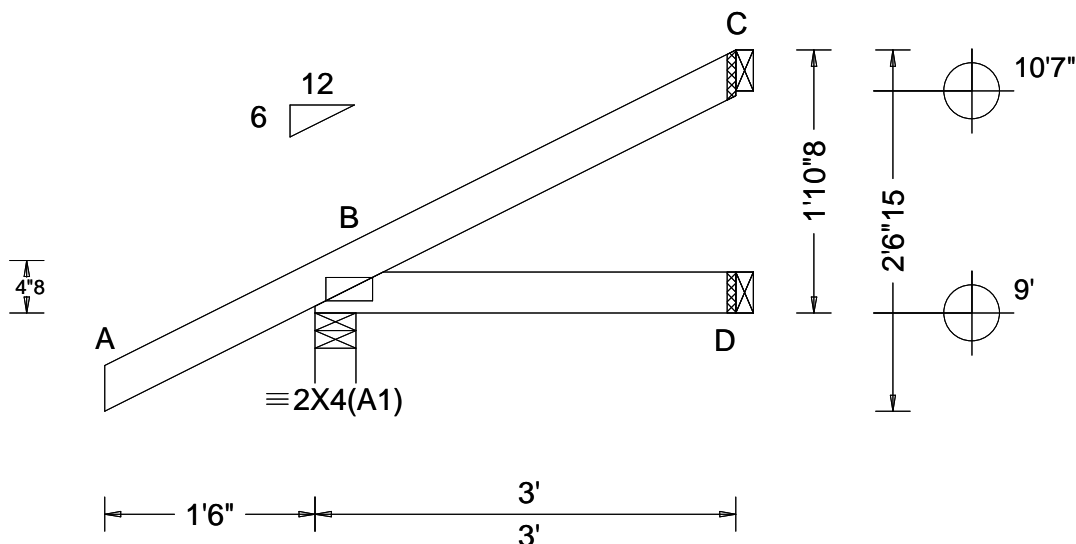
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14294 FROM:	JACK Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: J05	Cust: R 215 JRef: 1XL52150007 T26 DrwNo: 336.22.1433.13840 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.163 Max BC CSI: 0.065 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 262 /- /- /190 /42 /73 D 50 /- /- /26 /- /- C 62 /- /- /36 /35 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

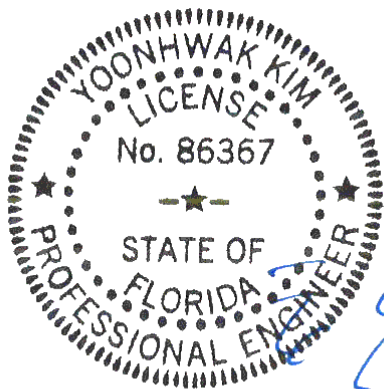
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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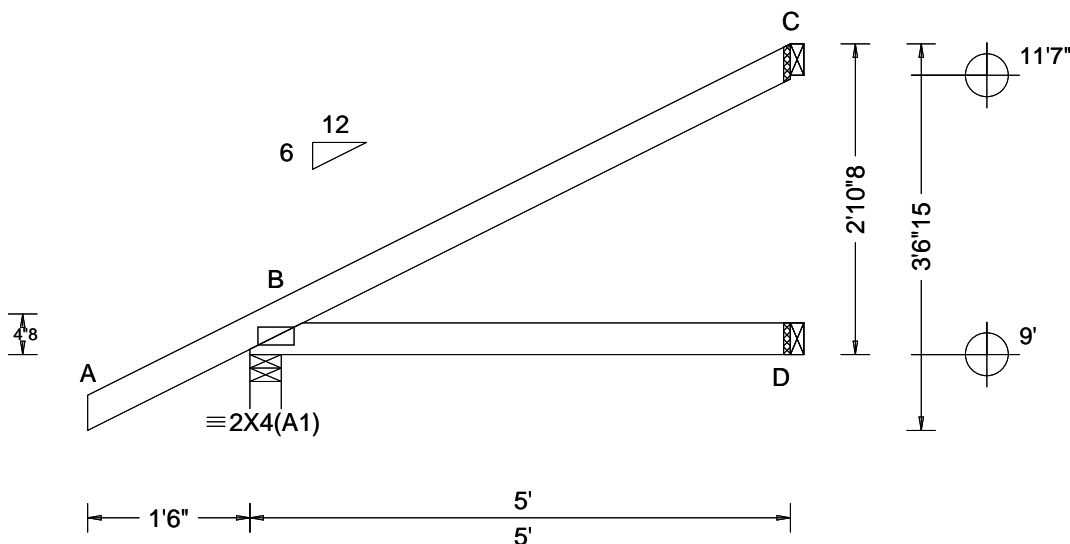
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14296 FROM:	JACK Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: J06	Cust: R 215 JRef: 1XL52150007 T25 DrwNo: 336.22.1433.15617 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.322 Max BC CSI: 0.235 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 331 - / - / - / 231 / 43 / 109 D 90 - / - / - / 48 - / - C 128 - / - / - / 79 / 65 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

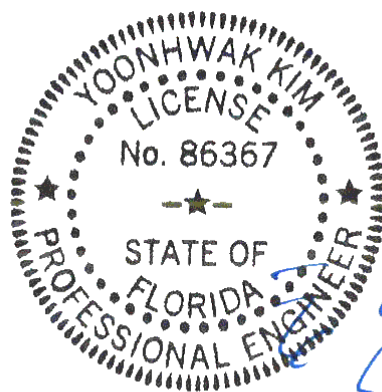
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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Florida State Seal of Product Approval #FL 1999

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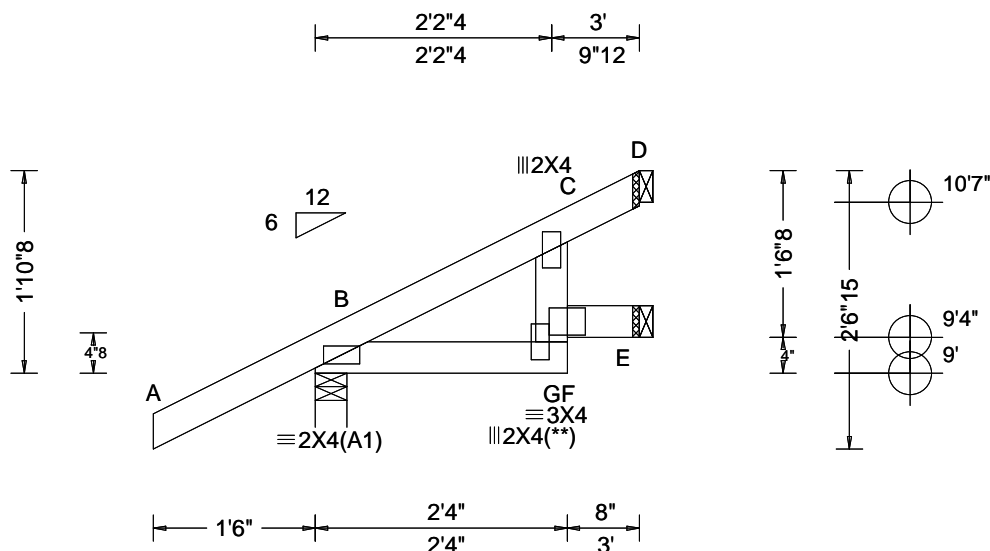
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14303 FROM:	JACK Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: J07	Cust: R 215 JRef: 1XL52150007 T32 DrwNo: 336.22.1433.17367 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 F 999 240 VERT(CL): 0.005 F 999 180 HORZ(LL): 0.001 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.163 Max BC CSI: 0.037 Max Web CSI: 0.030 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 262 /- /- /190 /42 /73 E 20 /- /- /12 /- /- D 72 /- /- /46 /25 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

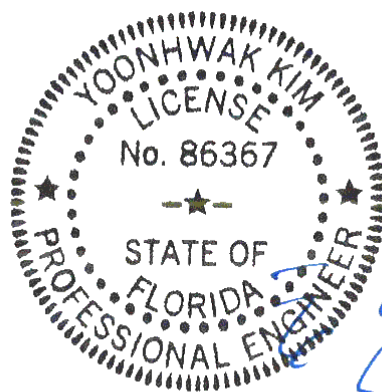
Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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Florida State Seal of Product Approval #FL 1999

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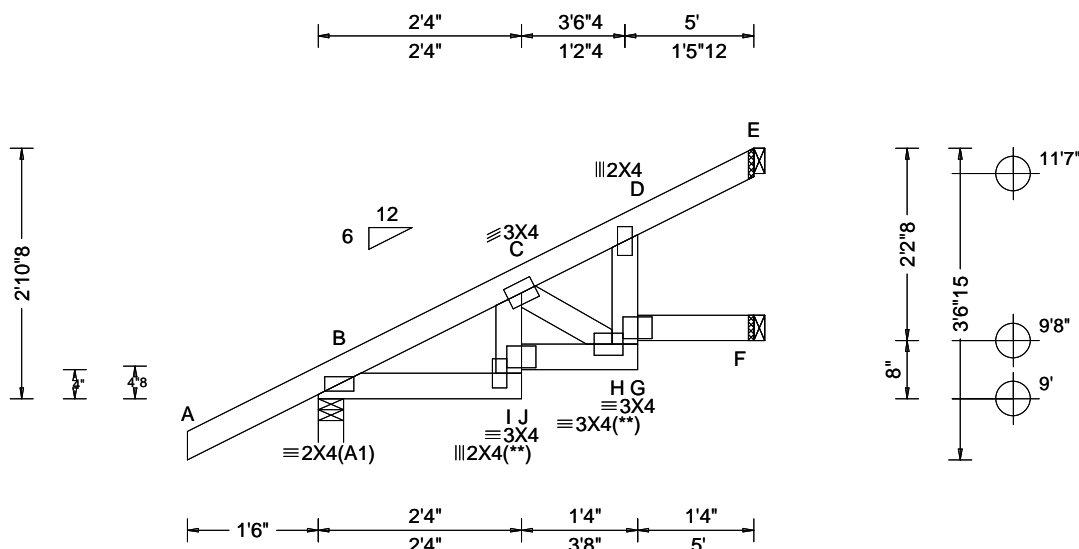
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14305 FROM:	JACK Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: J08	Cust: R 215 JRef: 1XL52150007 T34 DrwNo: 336.22.1433.19913 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.023 G 999 240 VERT(CL): 0.046 G 999 180 HORZ(LL): 0.011 D - - HORZ(TL): 0.023 D - - Creep Factor: 2.0 Max TC CSI: 0.289 Max BC CSI: 0.077 Max Web CSI: 0.200 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 331 - / - / - /231 /43 /109 F 38 - / - / - /24 - / - E 153 - / - / - /103 /54 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 1.5 Min Req = - E Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

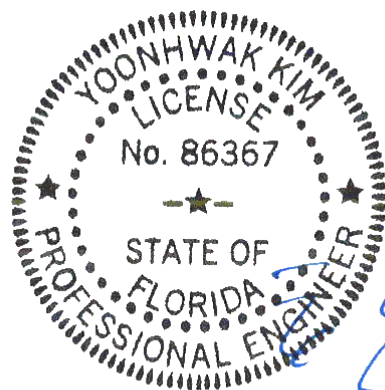
Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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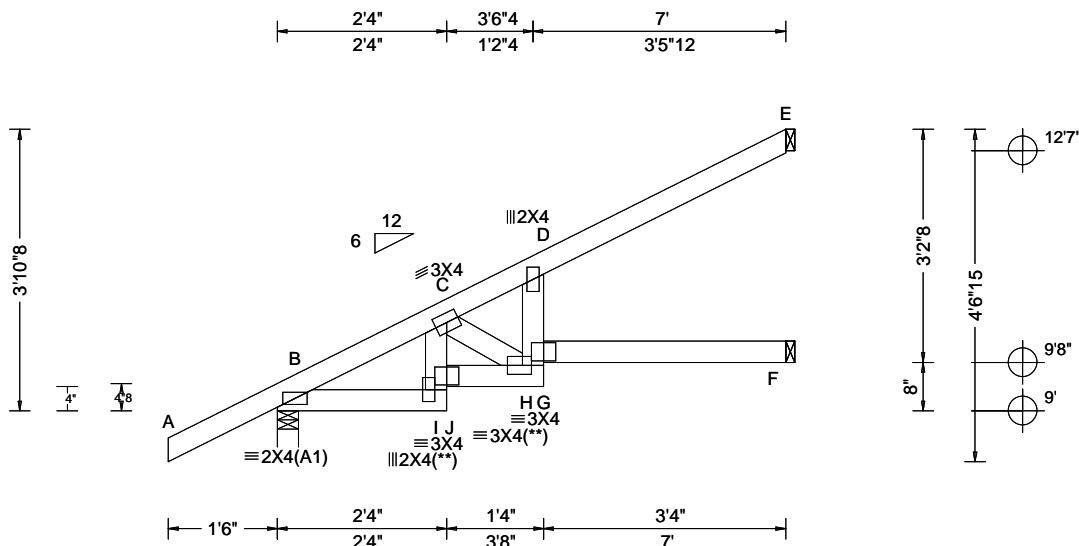
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14826 FROM:	EJAC Ply: 1 Qty: 3	Job Number: 22-8612 1845F Truss Label: J09	Cust: R 215 JRef: 1XL52150007 T4 DrwNo: 336.22.1433.22500 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.162 G 507 240 VERT(CL): 0.323 G 254 180 HORZ(LL): 0.084 D - - HORZ(TL): 0.167 D - - Creep Factor: 2.0 Max TC CSI: 0.690 Max BC CSI: 0.212 Max Web CSI: 0.373 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 408 - / - / - /278 /47 /144 F 83 - / - / - /49 /- /- E 213 - / - / - /141 /85 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 1.5 Min Req = - E Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. I - G 406 -297

Lumber

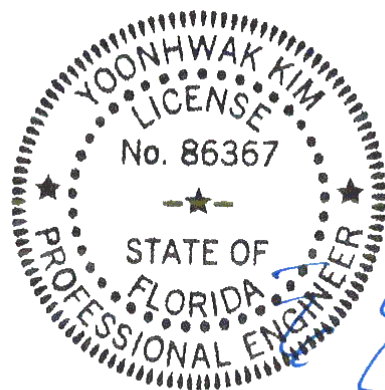
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

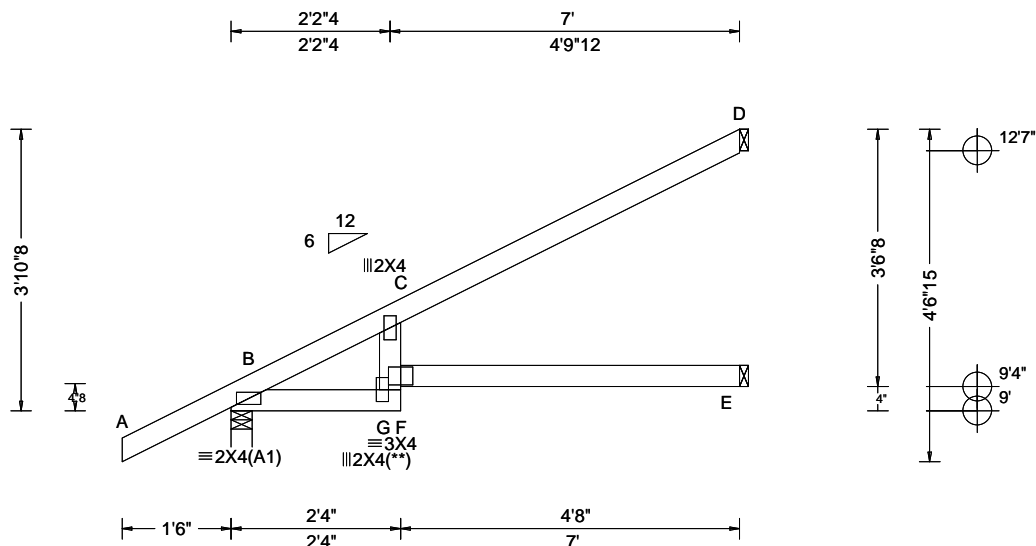


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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14829 FROM:	EJAC Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: J10	Cust: R 215 JRef: 1XL52150007 T22 DrwNo: 336.22.1433.24670 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.132 F 624 240 VERT(CL): 0.262 F 313 180 HORZ(LL): 0.071 C - - HORZ(TL): 0.142 C - - Creep Factor: 2.0 Max TC CSI: 0.370 Max BC CSI: 0.303 Max Web CSI: 0.188 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 408 /- /- /278 /47 /144 E 101 /- /- /56 /- /- D 208 /- /- /135 /94 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

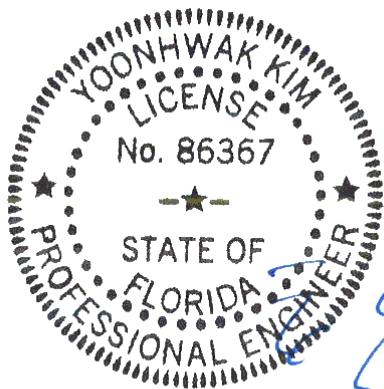
Plating Notes

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Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL 1999

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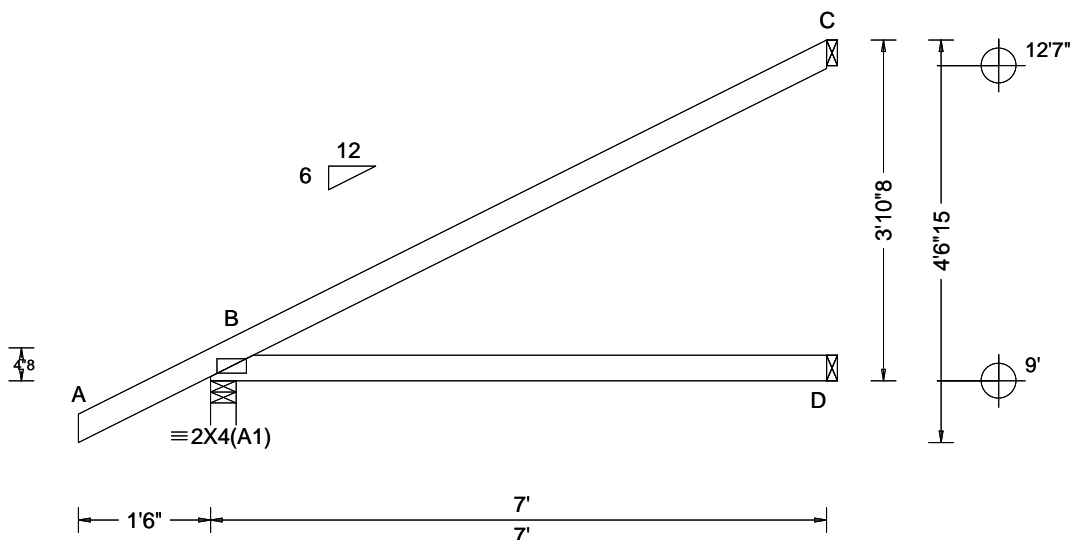
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SEQN: 14299 FROM:	EJAC Ply: 1 Qty: 6	Job Number: 22-8612 1845F Truss Label: J11	Cust: R 215 JRef: 1XL52150007 T31 DrwNo: 336.22.1433.30363 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.013 B - - HORZ(TL): 0.026 B - - Creep Factor: 2.0 Max TC CSI: 0.718 Max BC CSI: 0.514 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 408 - / - / - /278 /47 /144 D 129 - / - / - /72 /- /- C 188 - / - / - /118 /94 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

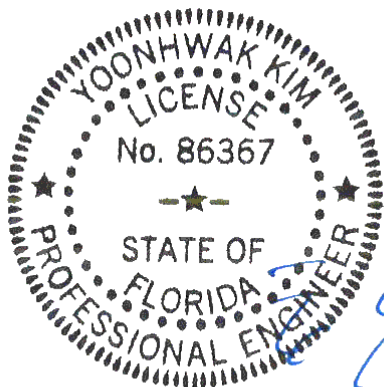
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

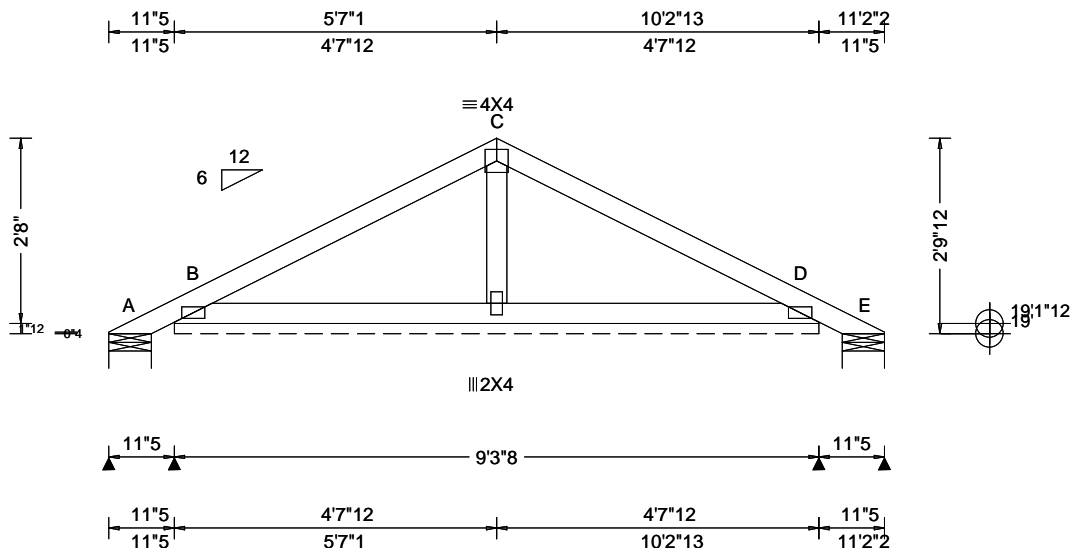
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 14518 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: PB01	Cust: R 215 JRef: 1XL52150007 T46 DrwNo: 336.22.1433.35283 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.42 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.005 B 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.229 Max BC CSI: 0.098 Max Web CSI: 0.029 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-91 /- /63 /103 /68 B* 94 /- /- /58 /33 /- E - /-91 /- /43 /73 /- B /-103 Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 111 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

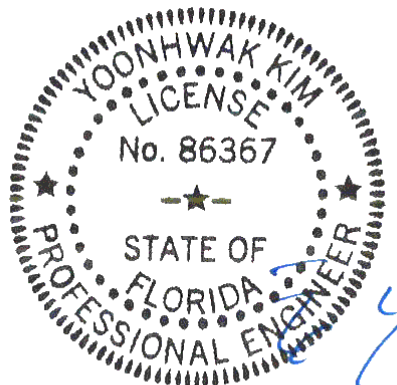
Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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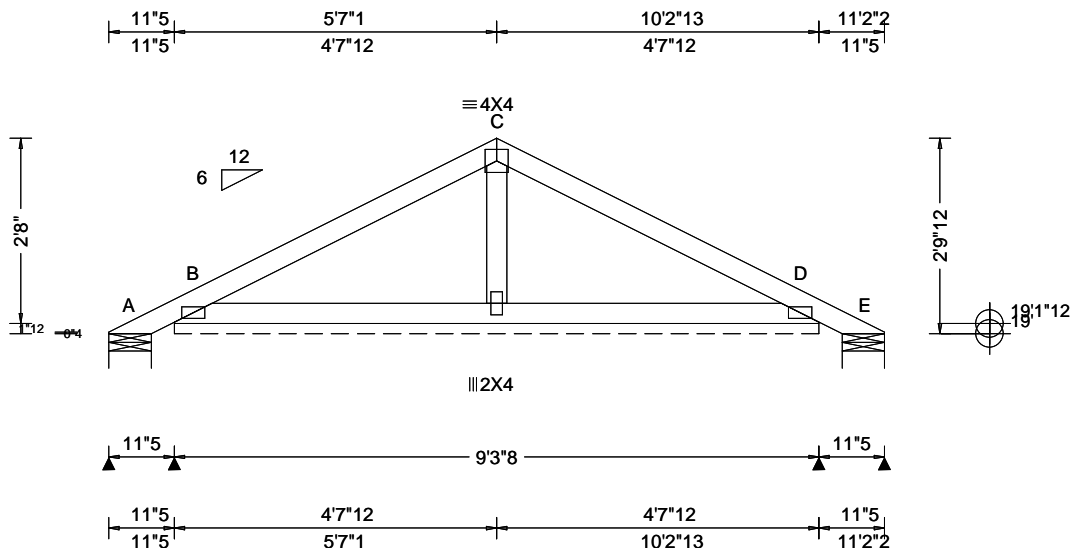
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14339 FROM:	COMN Ply: 1 Qty: 10	Job Number: 22-8612 1845F Truss Label: PB02	Cust: R 215 JRRef: 1XL52150007 T58 DrwNo: 336.22.1433.36903 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.42 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.005 B 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.228 Max BC CSI: 0.105 Max Web CSI: 0.029 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-86 /- /65 /105 /75 B* 94 /- /- /64 /13 /- E - /-86 /- /32 /72 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 111 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

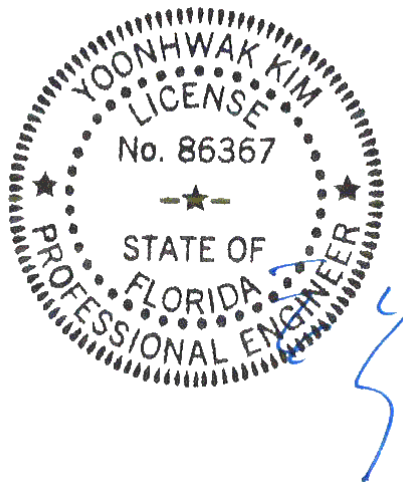
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida State Seal of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

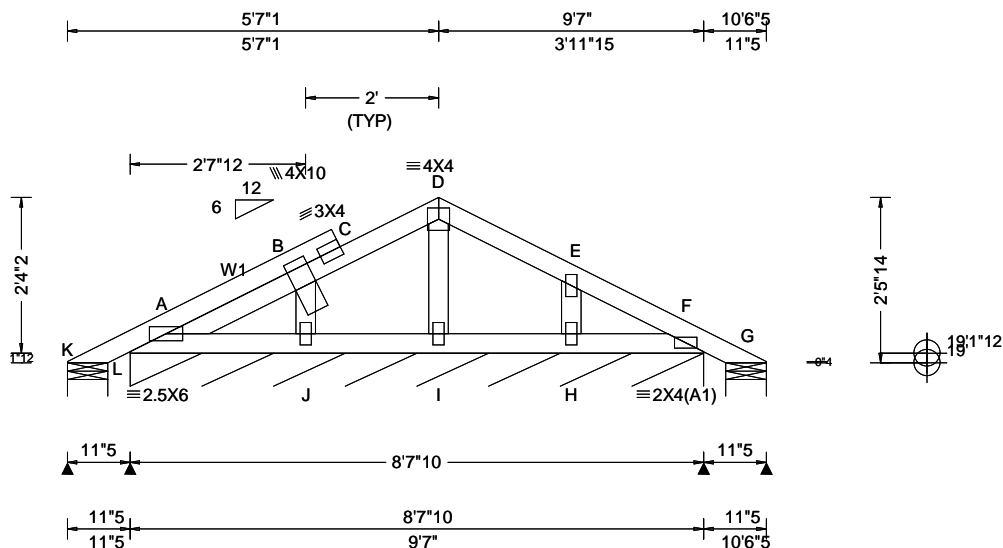
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14711 FROM:	GABL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: PB03	Cust: R 215 JRef: 1XL52150007 T55 DrwNo: 336.22.1433.38780 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.42 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 F 999 240 VERT(CL): 0.000 F 999 180 HORZ(LL): 0.000 F - - HORZ(TL): 0.000 F - - Creep Factor: 2.0 Max TC CSI: 0.054 Max BC CSI: 0.019 Max Web CSI: 0.045 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity K - /-4 /- /24 /29 /64 L* 64 /- /- /46 /6 /- G 17 /- /- /16 /3 /- Wind reactions based on MWFRS K Brg Wid = 7.3 Min Req = 1.5 (Truss) L Brg Wid = 103 Min Req = - G Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings K, L, & G are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

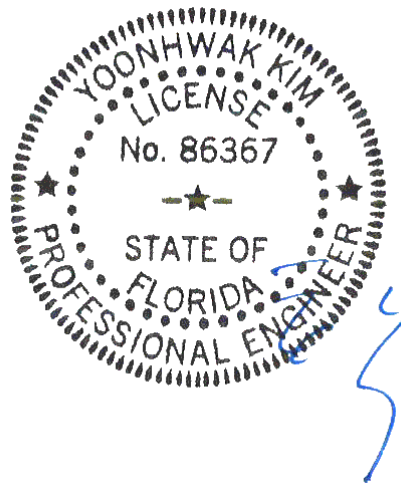
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Blocking

Blocking reinforcement required to prevent buckling of members over the bearings:
Bearing 1 located at 14.1' (blocking >= 11841.61" if used)

Additional Notes

Refer to DWG PB160160118 for piggyback details.

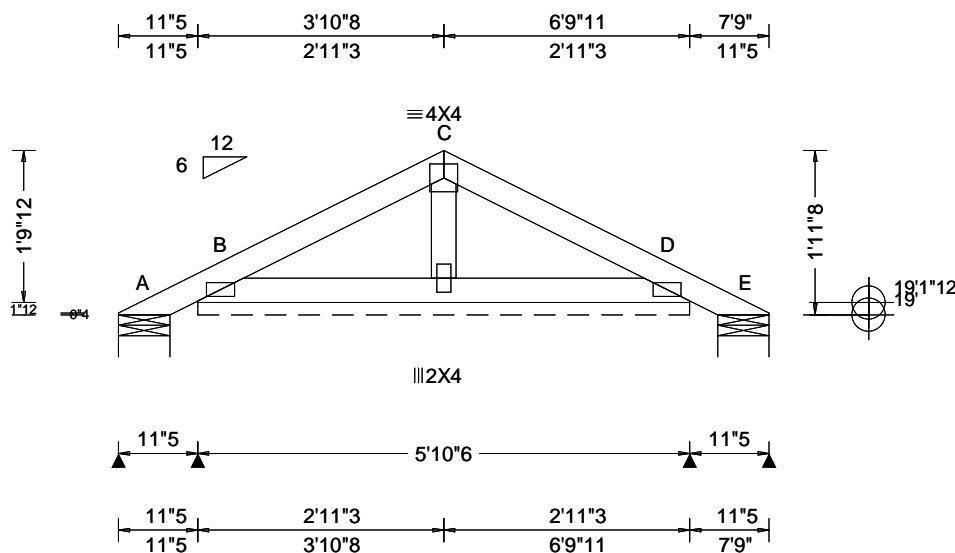


FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14823 FROM:	COMN Ply: 1 Qty: 2	Job Number: 22-8612 1845F Truss Label: PB04	Cust: R 215 JRef: 1XL52150007 T15 DrwNo: 336.22.1433.40570 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.99 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.081 Max BC CSI: 0.044 Max Web CSI: 0.018 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-8 /- /28 /28 /49 B* 84 /- /- /58 /10 /- E - /-8 /- /6 /6 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 70.3 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

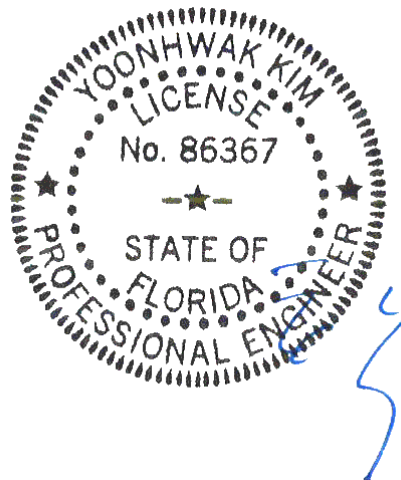
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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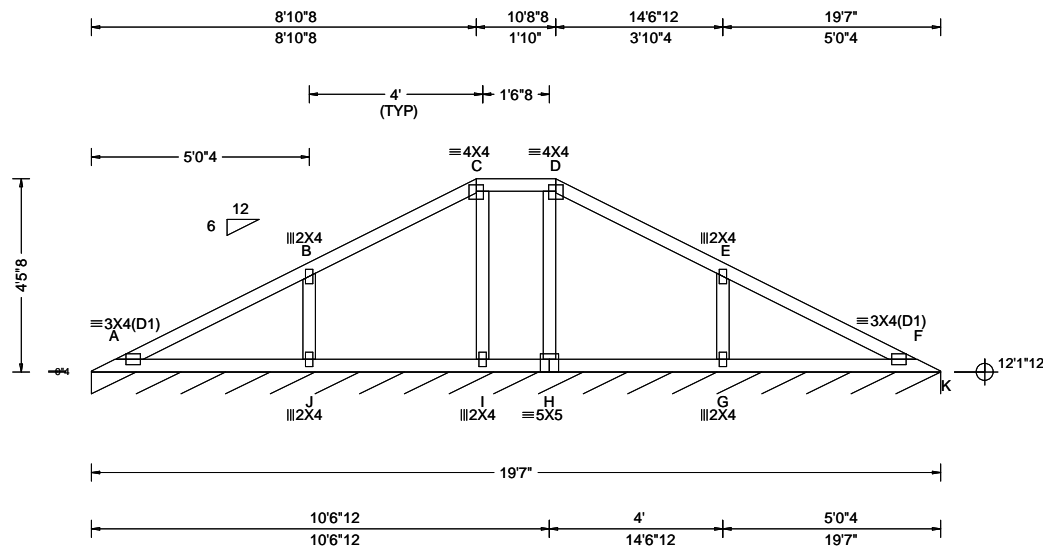
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14753 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V01	Cust: R 215 JRref: 1XL52150007 T43 DrwNo: 336.22.1433.44377 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 F 999 240 VERT(CL): 0.026 F 999 180 HORZ(LL): 0.004 A - - HORZ(TL): 0.009 A - - Creep Factor: 2.0 Max TC CSI: 0.349 Max BC CSI: 0.212 Max Web CSI: 0.087 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL K* 82 /- /- /42 /1 /5 Wind reactions based on MWFRS K Brg Wid = 234 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

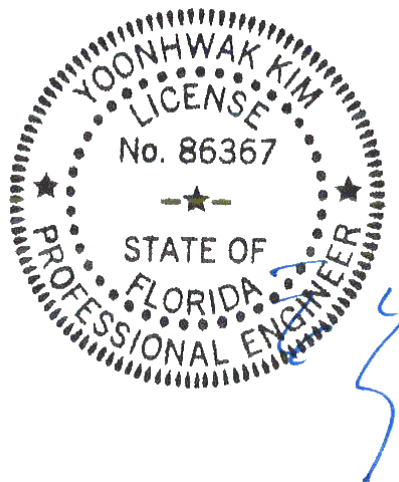
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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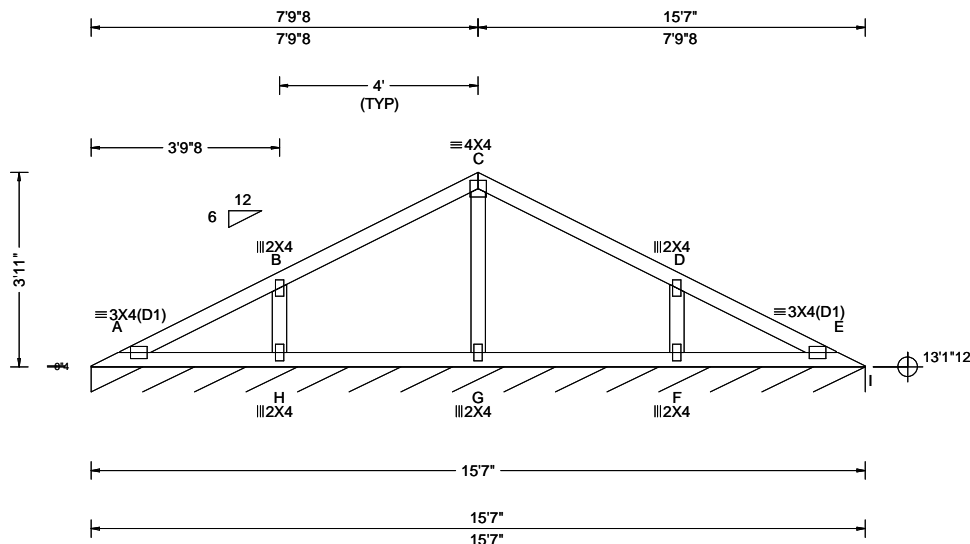
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14755 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V02	Cust: R 215 JRef: 1XL52150007 T52 DrwNo: 336.22.1433.45657 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.26 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 E 999 240 VERT(CL): 0.009 E 999 180 HORZ(LL): -0.002 E - - HORZ(TL): 0.003 E - - Creep Factor: 2.0 Max TC CSI: 0.281 Max BC CSI: 0.132 Max Web CSI: 0.072 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 82 /- /- /42 /1 /6 Wind reactions based on MWFRS I Brg Wid = 186 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

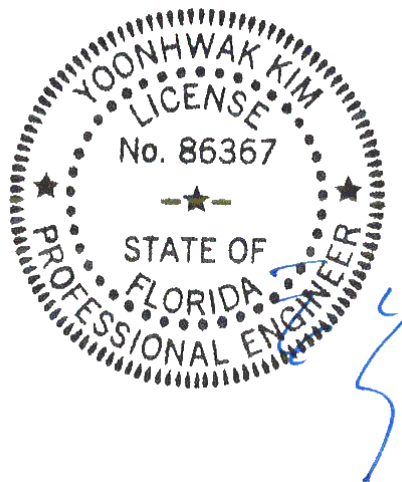
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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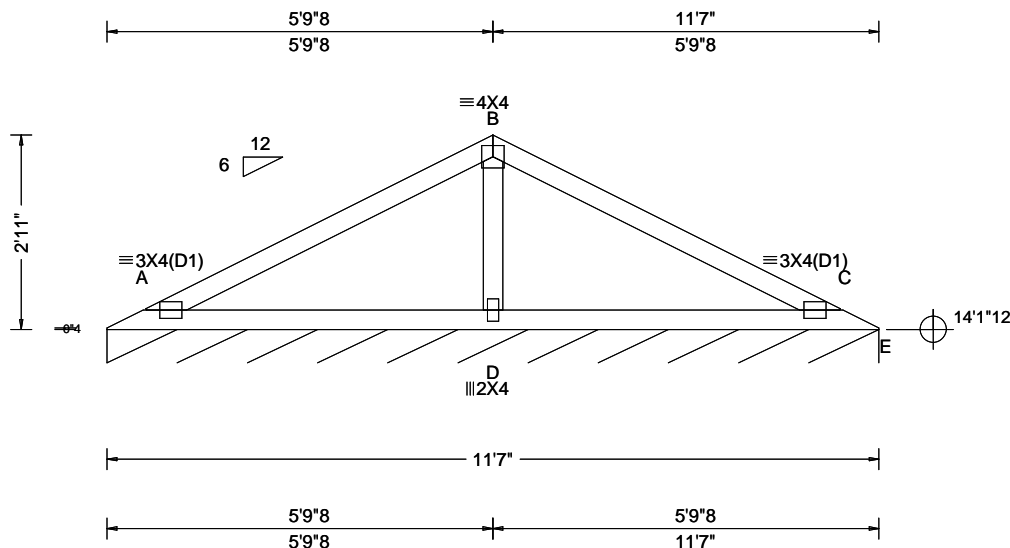
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14758 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V03	Cust: R 215 JRef: 1XL52150007 T49 DrwNo: 336.22.1433.47000 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.76 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.023 C 999 240 VERT(CL): 0.046 C 999 180 HORZ(LL): -0.009 C - - HORZ(TL): 0.019 C - - Creep Factor: 2.0 Max TC CSI: 0.457 Max BC CSI: 0.387 Max Web CSI: 0.139 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 82 /- /- /41 /0 /6 Wind reactions based on MWFRS E Brg Wid = 138 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 475 -249 B - C 475 -261 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 437 -648

Lumber

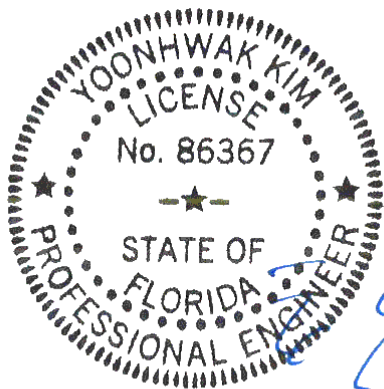
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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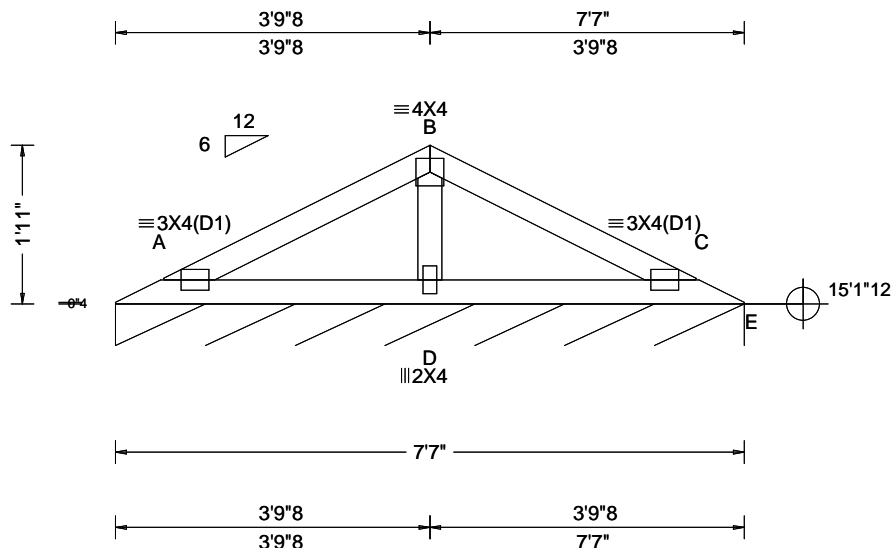
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Glenview, IL 60025

SEQN: 14760 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V04	Cust: R 215 JRef: 1XL52150007 T56 DrwNo: 336.22.1433.49457 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.26 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 C 999 240 VERT(CL): 0.012 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.165 Max BC CSI: 0.150 Max Web CSI: 0.081 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 82 /- /- /40 /- /5 Wind reactions based on MWFRS E Brg Wid = 91.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

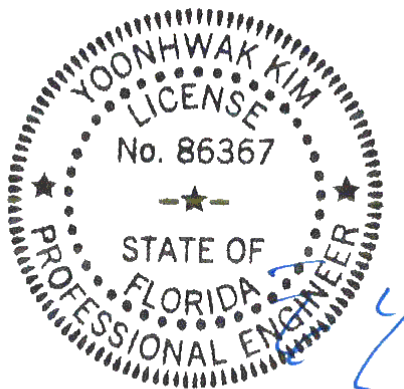
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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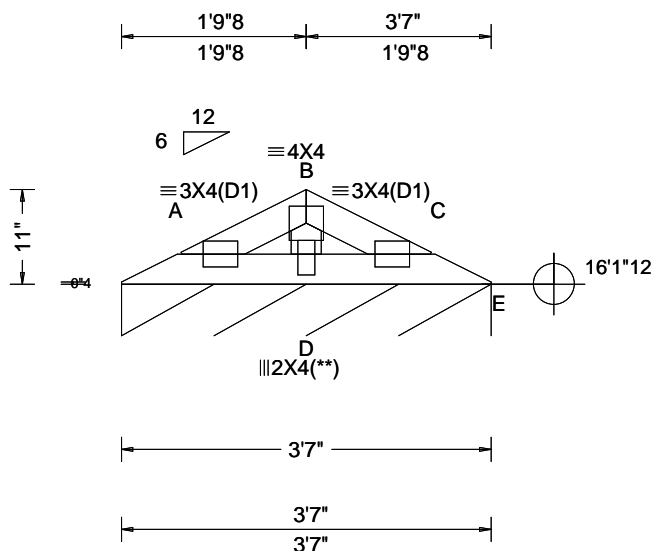
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Glenview, IL 60025

SEQN: 14762 FROM:	VAL Qty: 1	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V05	Cust: R 215 JRef: 1XL52150007 T51 DrwNo: 336.22.1433.51290 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.76 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.024 Max BC CSI: 0.026 Max Web CSI: 0.020 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 82 /- /- /36 /- /4 Wind reactions based on MWFRS E Brg Wid = 43.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

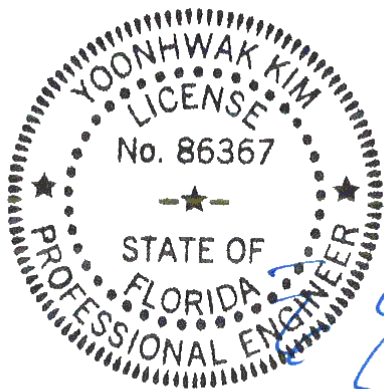
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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Florida Certificate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

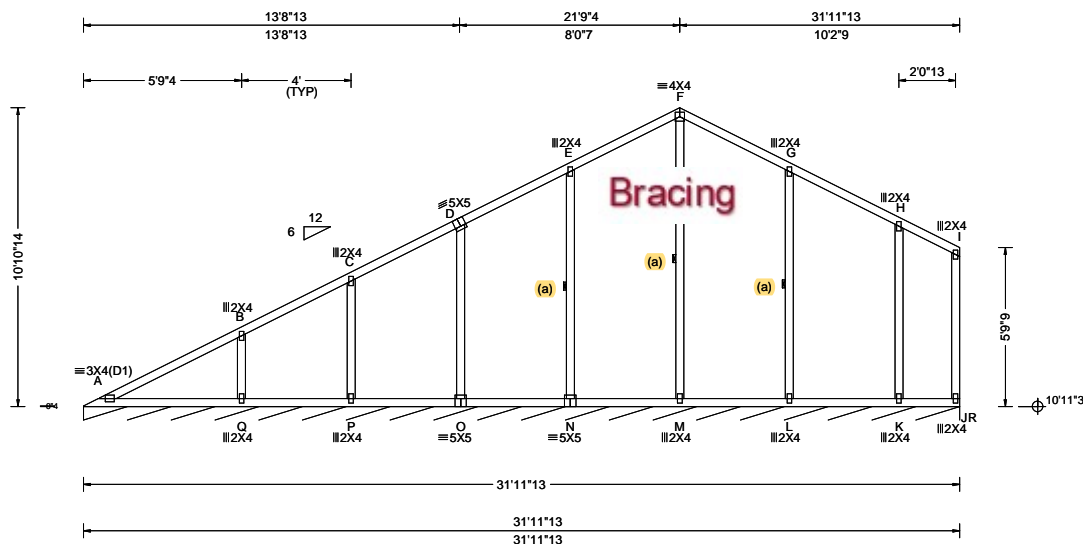
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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SEQN: 14821 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V06	Cust: R 215 JRef: 1XL52150007 T18 DrwNo: 336.22.1433.53047 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.54 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.20 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.023 A 999 240 VERT(CL): 0.047 A 999 180 HORZ(LL): -0.007 G - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.358 Max BC CSI: 0.273 Max Web CSI: 0.209 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL R* 82 /- /- /47 /14 /9 Wind reactions based on MWFRS R Brg Wid = 383 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

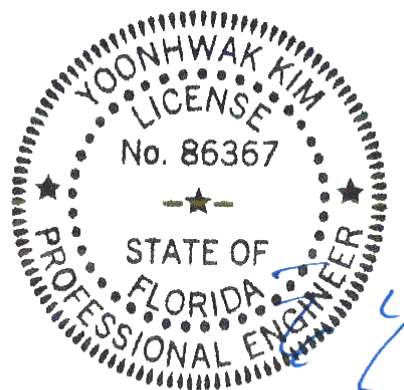
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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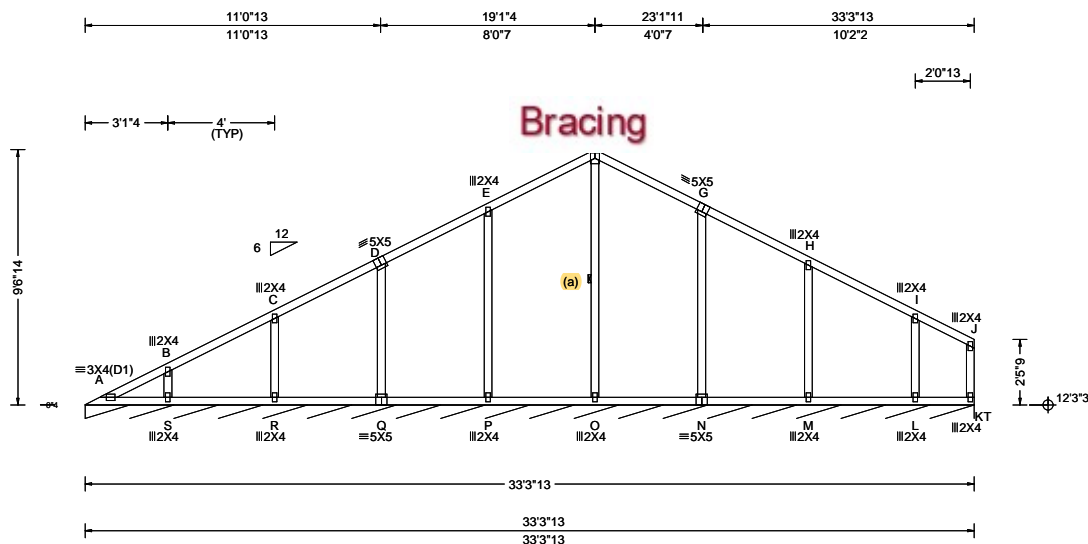
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14684 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V07	Cust: R 215 JRef: 1XL52150007 T54 DrwNo: 336.22.1433.54877 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.21 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.33 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.005 A 999 180 HORZ(LL): 0.006 I - - HORZ(TL): 0.011 I - - Creep Factor: 2.0 Max TC CSI: 0.203 Max BC CSI: 0.109 Max Web CSI: 0.296 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL T* 82 /- /- /44 /14 /7 Wind reactions based on MWFRS T Brg Wid = 399 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

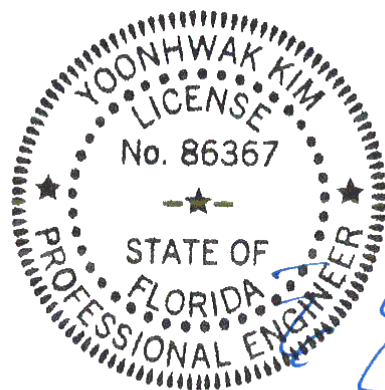
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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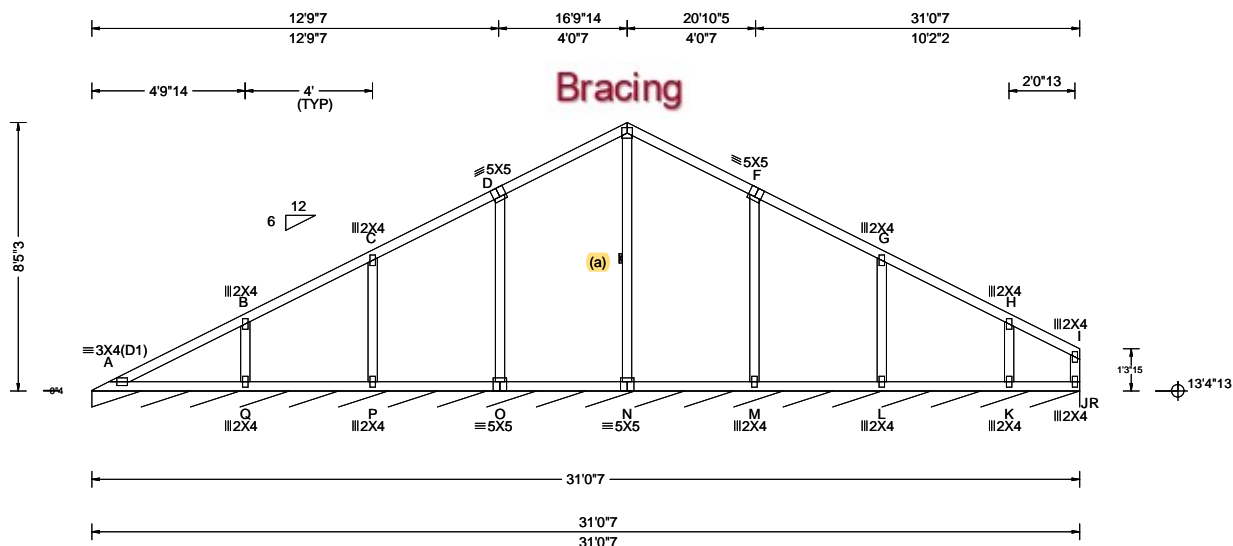
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14348 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V08	Cust: R 215 JRef: 1XL52150007 T44 DrwNo: 336.22.1433.56570 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.77 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.10 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 A 999 240 VERT(CL): 0.026 A 999 180 HORZ(LL): 0.006 H - - HORZ(TL): 0.010 I - - Creep Factor: 2.0 Max TC CSI: 0.274 Max BC CSI: 0.197 Max Web CSI: 0.210 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL R* 82 /- /- /43 /14 /7 Wind reactions based on MWFRS R Brg Wid = 372 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

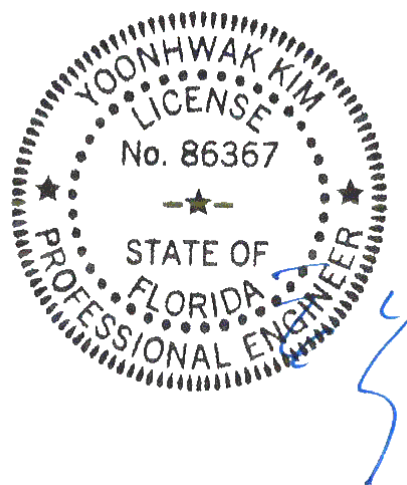
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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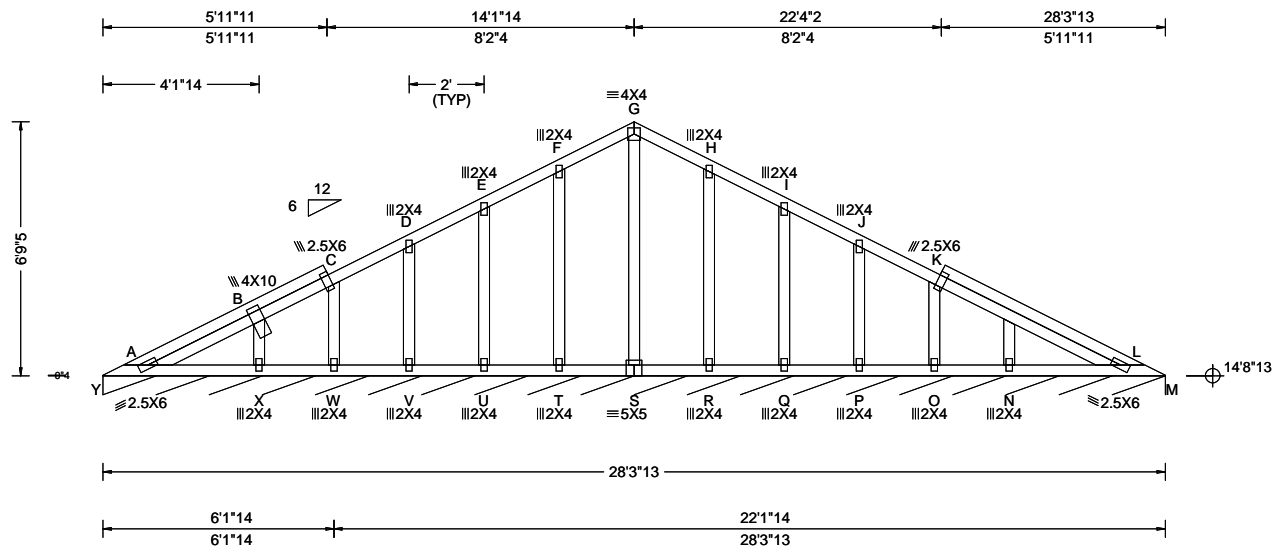
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14681 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V09	Cust: R 215 JRRef: 1XL52150007 T10 DrwNo: 336.22.1434.00317 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.27 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 F 999 240 VERT(CL): 0.001 F 999 180 HORZ(LL): 0.001 K - - HORZ(TL): 0.002 I - - Creep Factor: 2.0 Max TC CSI: 0.220 Max BC CSI: 0.049 Max Web CSI: 0.451 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL Y* 81 /- /- /41 /12 /6 Wind reactions based on MWFRS Y Brg Wid = 339 Min Req = - Bearing Y is a rigid surface. Members not listed have forces less than 375#

Lumber

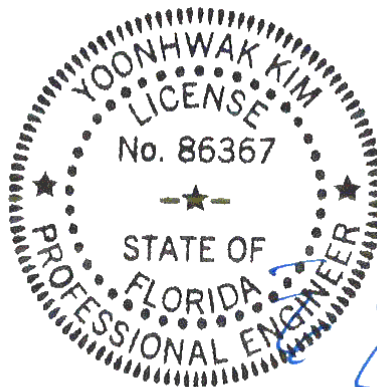
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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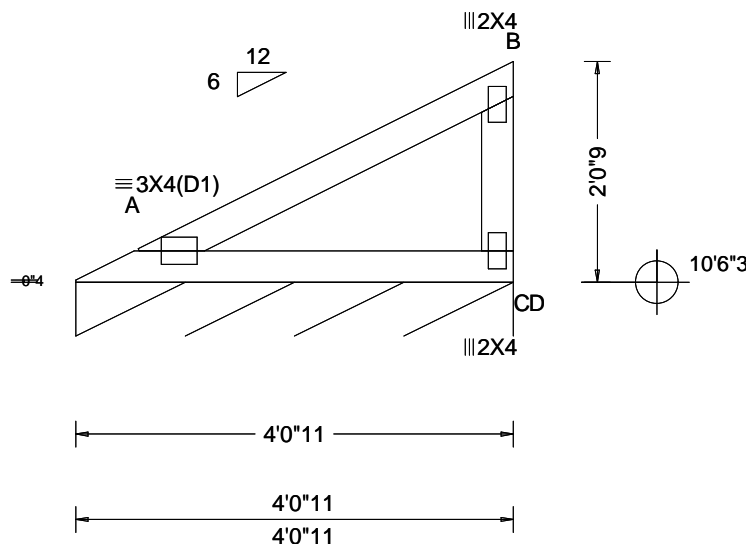
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 14788 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V10	Cust: R 215 JRef: 1XL52150007 T59 DrwNo: 336.22.1434.02150 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 A - - HORZ(TL): 0.006 A - - Creep Factor: 2.0 Max TC CSI: 0.201 Max BC CSI: 0.155 Max Web CSI: 0.088 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 82 /- /- /50 /2 /11 Wind reactions based on MWFRS D Brg Wid = 48.7 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

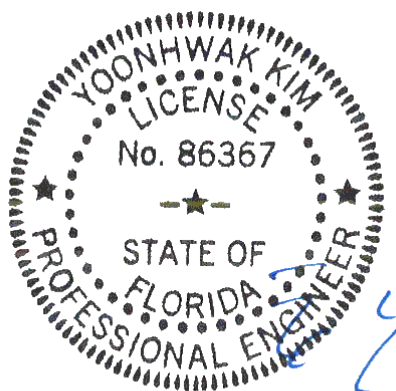
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
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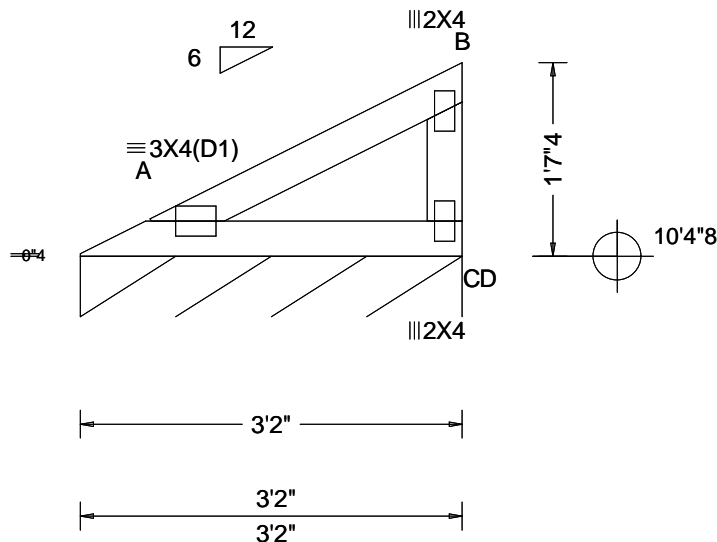
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SEQN: 14805 FROM:	VAL Ply: 1 Qty: 1	Job Number: 22-8612 1845F Truss Label: V11	Cust: R 215 JRef: 1XL52150007 T62 DrwNo: 336.22.1434.06663 KD / YK 12/02/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.111 Max BC CSI: 0.096 Max Web CSI: 0.055 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 82 /- /- /49 /1 /10 Wind reactions based on MWFRS D Brg Wid = 38.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

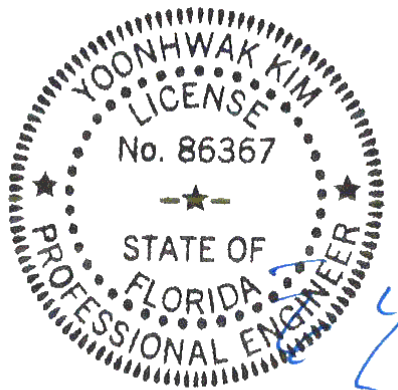
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999-02-0001

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Leading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * =PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.114 AE 999 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.251 AE 999 180	AI 1440 /- /- /852 /37 /264
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.046 AA - -	Z 2121 /- /- /1209 /86 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.095 AA - -	U* 50 /-4 /- /26 /11 /-
NCBCLL: 10.00	Mean Height: 15.00 ft		Creep Factor: 2.0	X /-390
Soffit: 2.00	TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.547	Wind reactions based on MWFRS
Load Duration: 1.25	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.890	AI Brg Wid = 5.5 Min Req = 1.7 (Truss)
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max Web CSI: 0.554	Z Brg Wid = 3.5 Min Req = 2.5
	C&C Dist a: 4.58 ft	Rep Fac: Yes		U Brg Wid = 111 Min Req = -
	Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)		Bearings AI, Z, & Z are a rigid surface.
	GCpi: 0.18	Plate Type(s):		Members not listed have forces less than 375#
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.01.01A.0520.12	Maximum Top Chord Forces Per Ply (lbs)

Plating Notes			Maximum Bot Chord Forces Per Ply (lbs)		
All plates are 2X4 except as noted.			Chords	Tens.	Comp.
(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.			Chords	Tens.	Comp.
			AH-AG	1297	- 303
			AG-AF	1441	- 316
			AF-AD	1493	- 318
			AD-AB	1398	- 262
			AB-AA	1346	- 257

Wind		webs	tens.Comp.	webs	tens. Comp.
Wind loads based on MWFRS with additional C&C member design.		A -AI	447 - 1389	AT-AU	232 - 474
Left end vertical not exposed to wind pressure.		A -AH	1397 - 398	N -AU	519 - 1712
Wind loading based on both gable and hip roof types.		AH - B	231 - 497	AU - Z	583 - 1853
		D -AD	711 - 170	AU-AV	182 - 380
		AD-AK	1020 - 277	Z -AX	267 - 540
		AK - H	1020 - 209	AV-AW	401 - 305


AR-H	1030	-296	AV-AVV	191	-390
H-AL	127	-633	AW-AX	204	-398
AL-AA	145	-662	AX-AZ	214	-502
AA-AP	254	-377	AX-Y	271	-466
AA-AS	1534	-380	AZ-R	200	-497
AS-A	1540	-403	R-X	384	-108
AS-A	1782	-509			

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	#1	4' 6"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
	SP DFL	Stud	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			#2	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	10' 2"	10' 7"	11' 10"	14' 0"	14' 0"	14' 0"
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	10' 2"	10' 7"	11' 10"	14' 0"	14' 0"	14' 0"
		#1 / #2	#1	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	#1	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 3"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	Stud	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
	SPF	#1 / #2	#1	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	Standard	#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	Stud	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

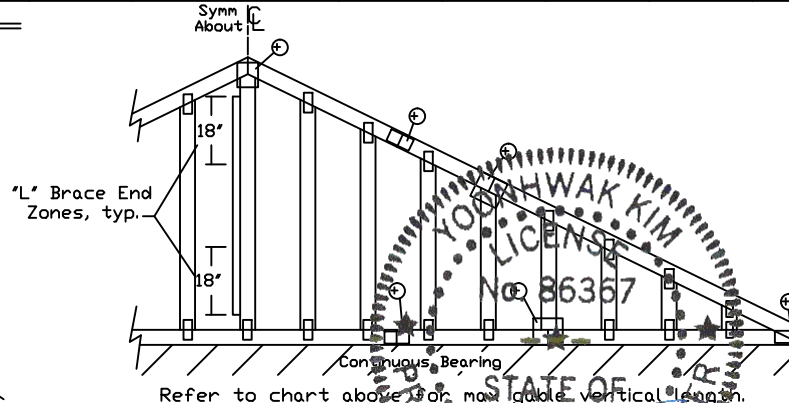
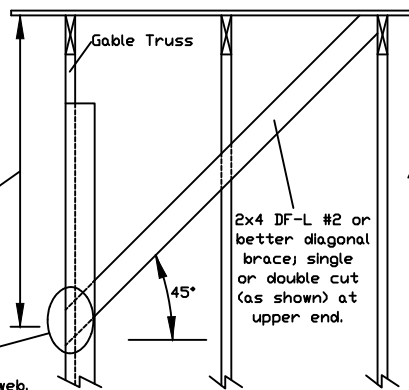
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



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MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

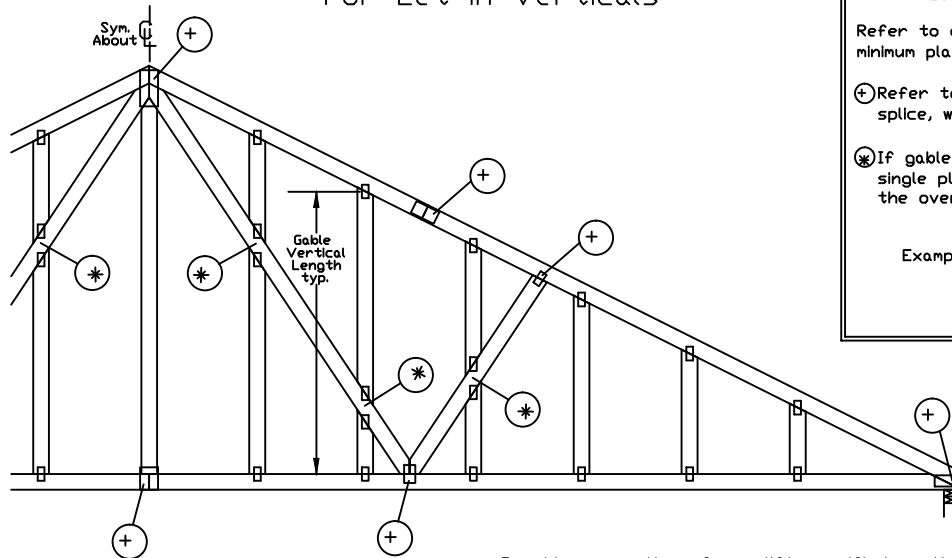
REF ASCE7-16-GAB14015

DATE 01/26/2018

DRWG A14015ENC160118

FL REG. 278, Yoonhwak Kim, FL PE #86367

Gable Detail For Let-in Verticals

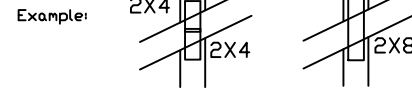


Gable Truss Plate Sizes

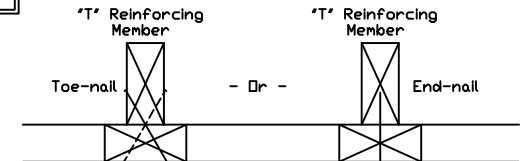
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(X) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

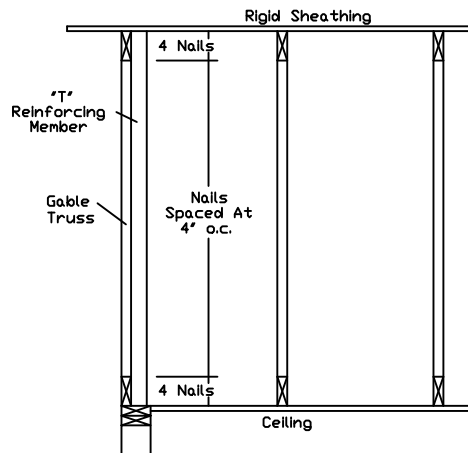
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A10015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015P100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A10030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030P100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015P100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030P100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



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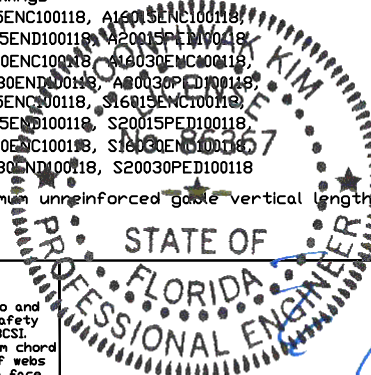
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Glenview, IL 60025



Yoonhwak Kim, FL PE #86367

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

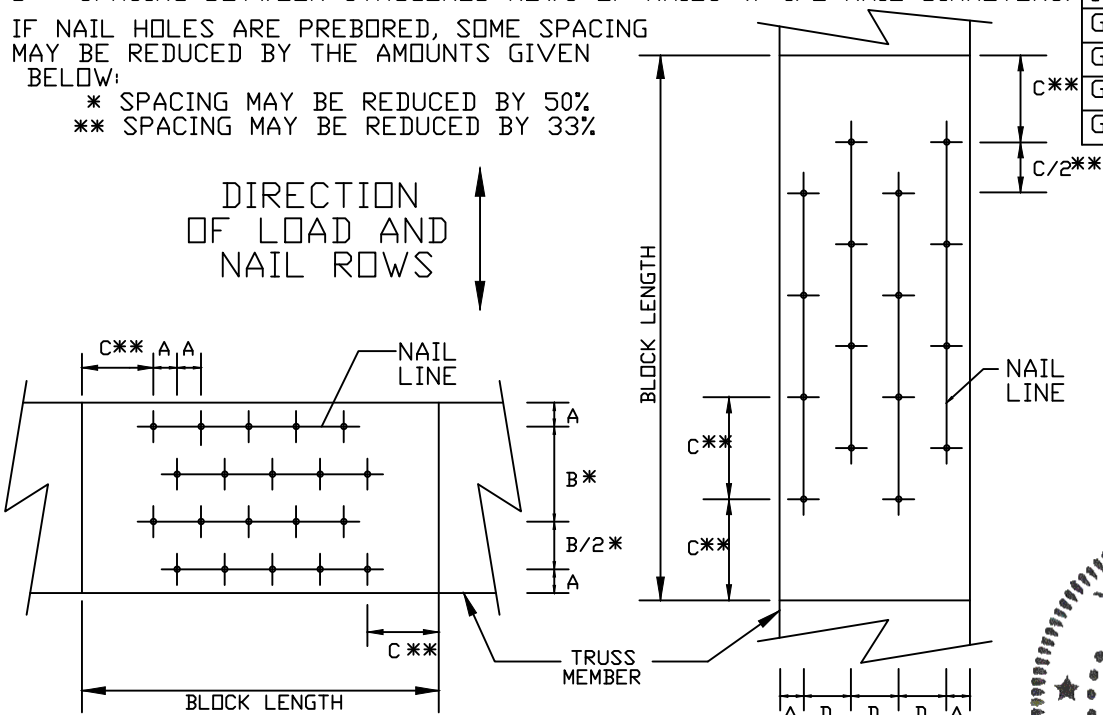
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

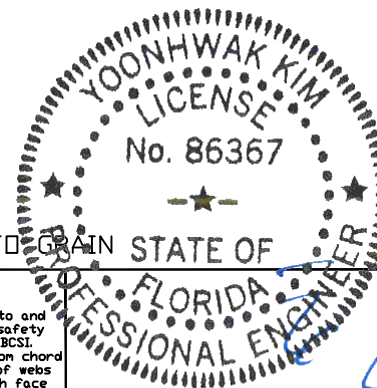
* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

REF NAIL SPACE
 DATE 10/01/14
 DRWG CNNAILSP1014

Yoonhwak Kim, FL PE #86367

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

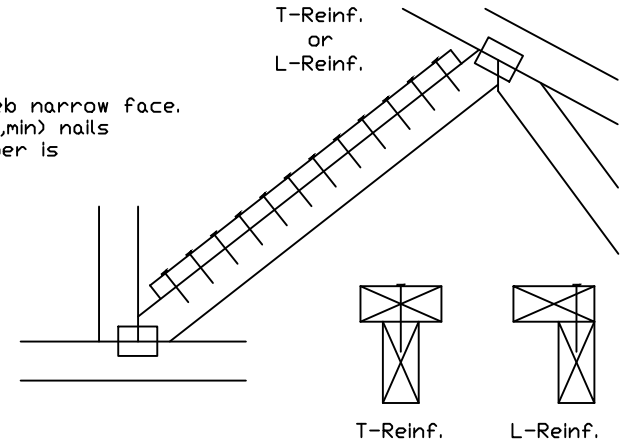
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(✕)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(✕)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(✕) Center scab on wide face of web. Apply (1) scab to each face of web.

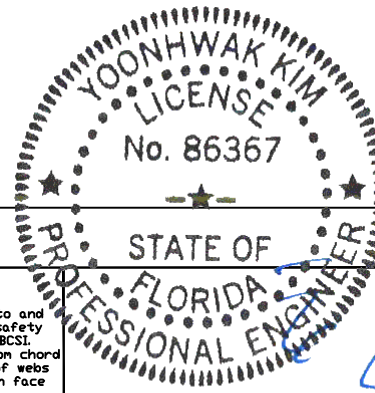
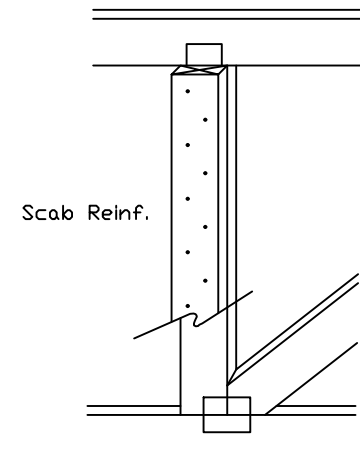
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

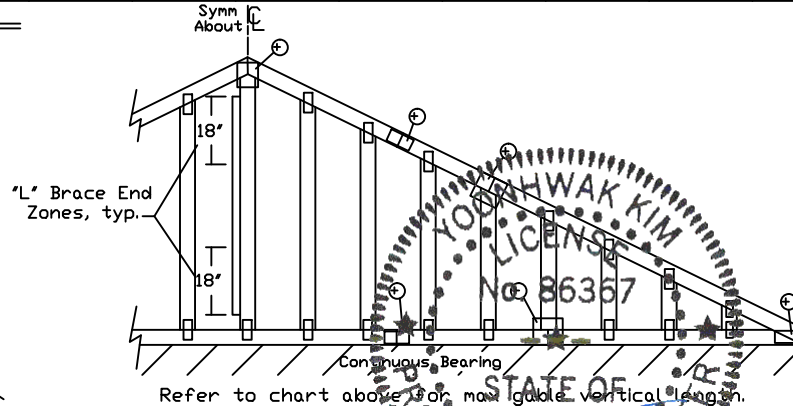
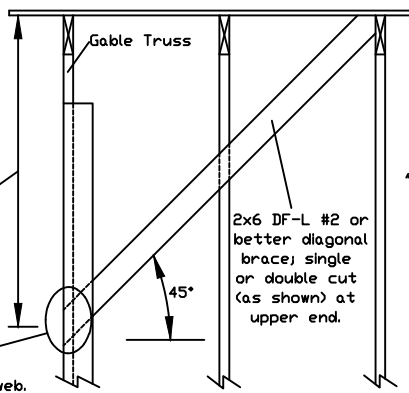
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"	
		SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"		
		#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"		
	#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"		
	#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15'	14' 0"	14' 0"		
	16" O.C.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
Stud			4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
Standard			4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
SP DFL			#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
Standard		4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
	#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"		
	#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"		
Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"		

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Bracing Group Species and Grades:									
Group A:									
Spruce-Pine-Fir					Hem-Fir				
#1 / #2	Standard	#2	Stud		#2	Stud			
#3	Stud	#3	Stud		#3	Stud			
Douglas Fir-Larch					Southern Pine***				
#3		#3			#3				
Stud		Stud			Stud				
Standard		Standard			Standard				
Group B:									
Hem-Fir									
#1 & Btr		#1							
#1		#1							
Douglas Fir-Larch					Southern Pine***				
#1		#1			#1				
#2		#2			#2				

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c.

in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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ALPINE: www.alpineitw.com TPI: www.tpinet.org SBCA: www.sbcacomponents.com ICC: www.icc-es.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB14030

DATE 01/26/2018

DRWG A14030ENC160118

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

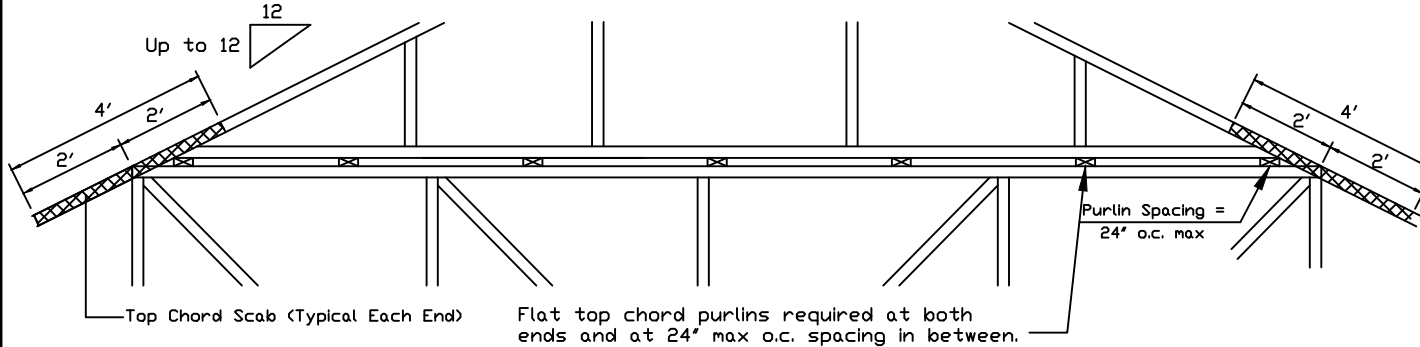
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

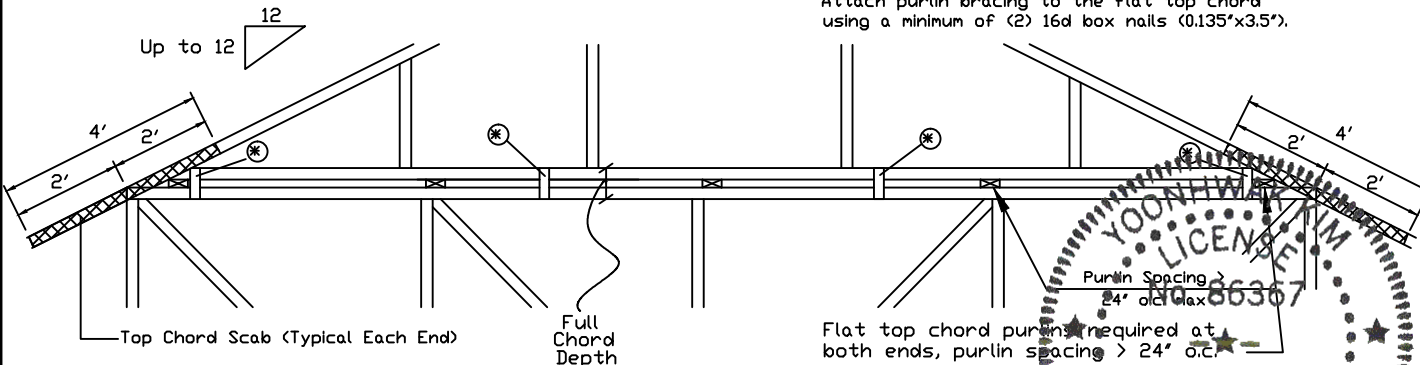


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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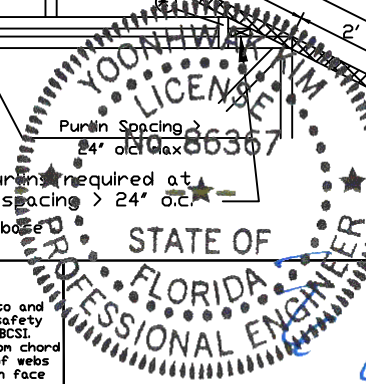
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF PIGGYBACK

DATE 01/02/2018

DRWG PB160160118

SPACING 24.0"

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

**** Attach each valley to every supporting truss with:**
 535# connection or with (1) Simpson H2.5A or
 equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut
 as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

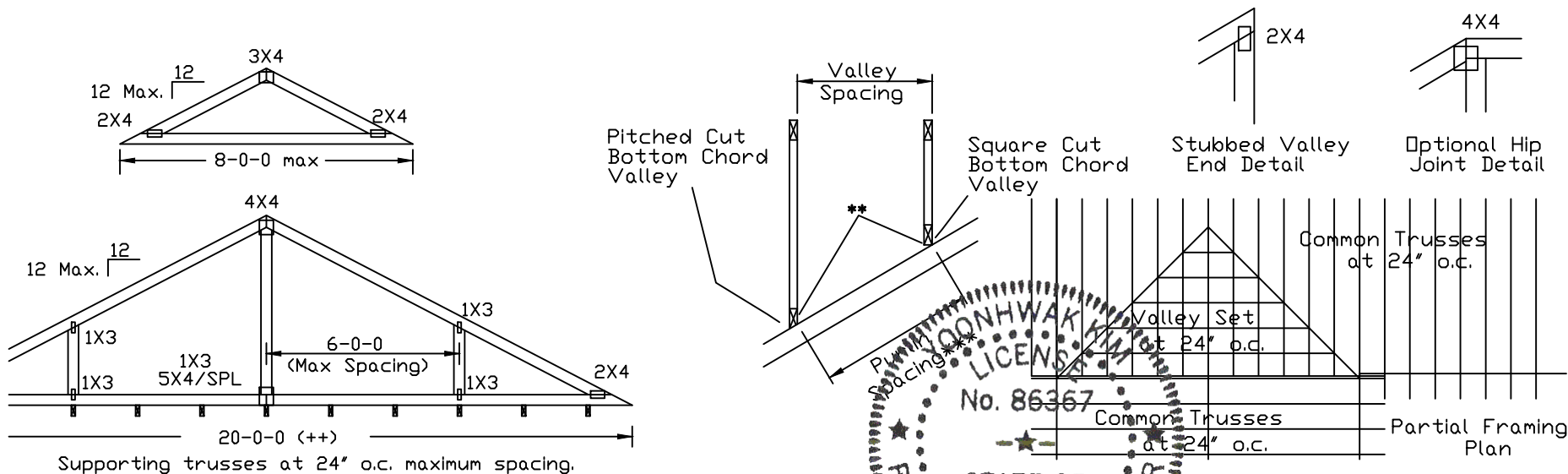
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

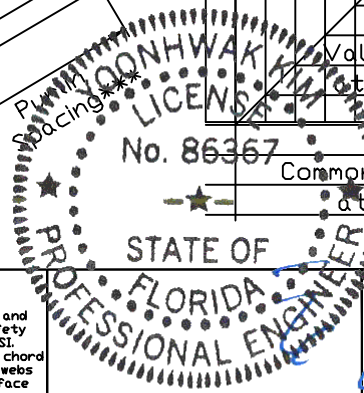
By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

***** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.**

**++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".**



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 shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
 shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face
 of truss and position as shown above and on the Joint Details, unless noted otherwise.
 Refer to drawings 160A-Z for standard plate positions.
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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING			24.0"		

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

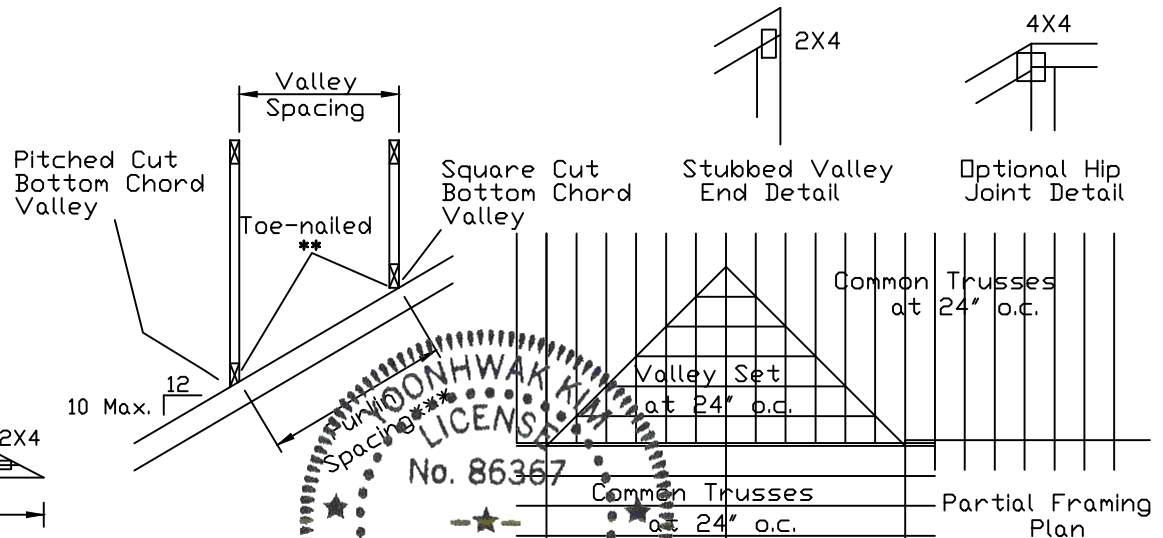
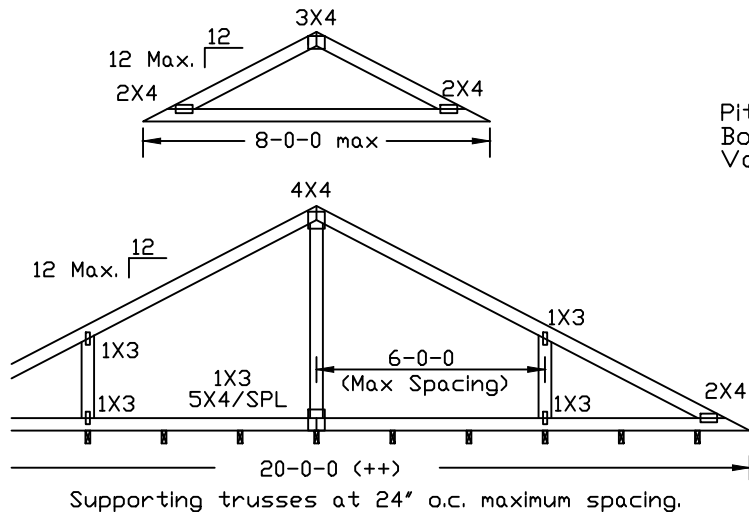
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

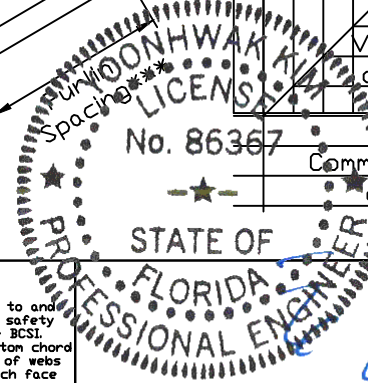
*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

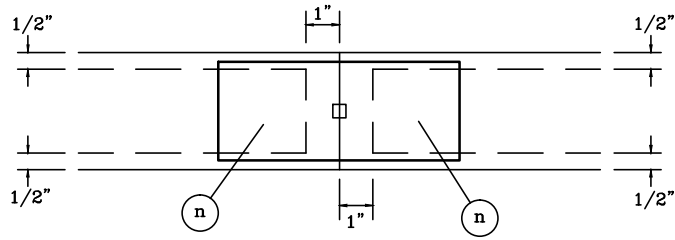
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 shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
 shall have bracing installed per BCSI sections 83, 87 or 810, as applicable. Apply plates to each face
 of truss and position as shown above and on the Joint Details, unless noted otherwise.
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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VALTN160118
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				

TRULOX INFORMATION DETAIL

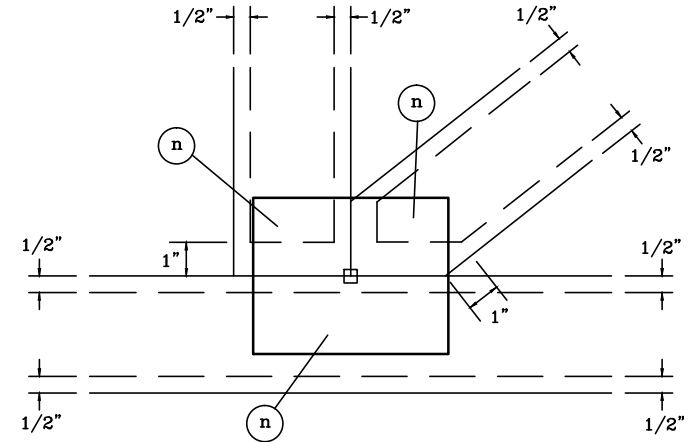
TYPICAL OFF PANEL SPLICE



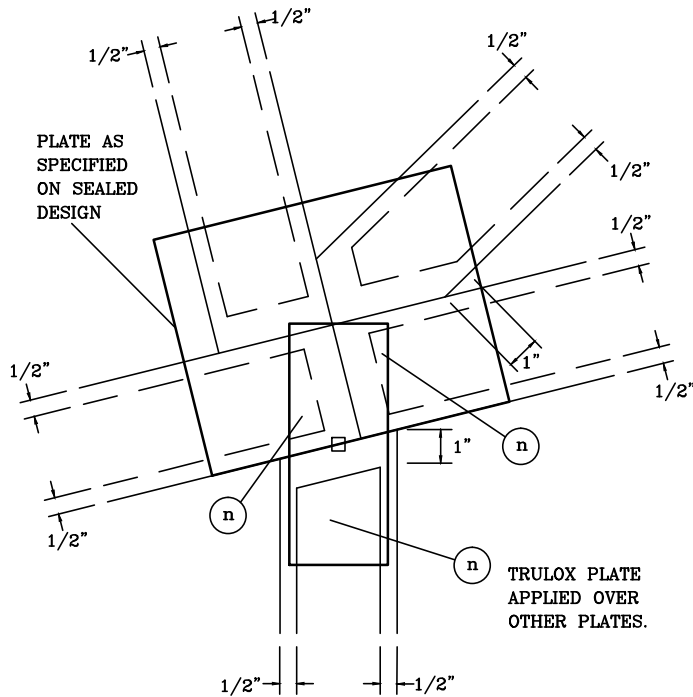
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

TYPICAL PANEL POINT WITHOUT SPLICE

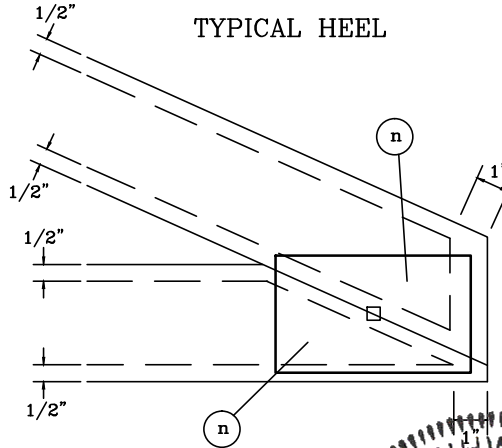


TYPICAL FILLER

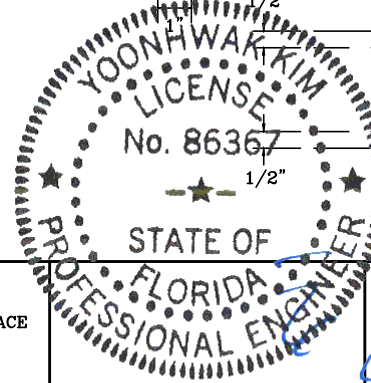
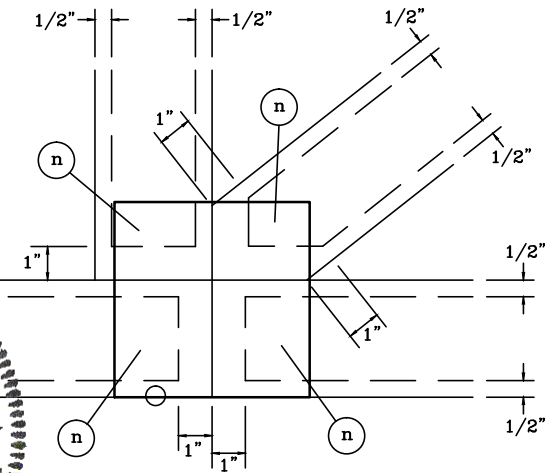


TRULOX PLATE APPLIED OVER OTHER PLATES.

TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



NOTES:

(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.

○ LOCATES PLATE CORNER OR FLUSH EDGE.

□ LOCATES PLATE CENTER.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

12/03/2023
FL REG# 278, Yoonhwak Kim, FL PE #86367

TRULOX PLATING

160
TL

PAGE 1 OF 1

DATE 10/01/14