

Lymber design values are in accordance with ANSI/TPI 1 section 6.3

RE: 1665-A - McLeod Addition MiTek, Inc.

Site Information:

16023 Swingley Ridge Rd. Chesterfield, MO 63017

Customer Info: Ponderosa Complete Project Name: McLeod Addition Model: .

314.434.1200

Lot/Block:

Subdivision: .

Address: 751 SW Jacobs Ct., .

City: Fort White State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):** 

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.8

Wind Code: ASCE 7-22 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 5 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T35223009	G1	10/9/24
2	T35223010	M1	10/9/24
3	T35223011	M2	10/9/24
4	T35223012	T1	10/9/24
5	T35223013	T2	10/9/24

Review for Code Compliance **Universal Engineering Science** 

PX2707

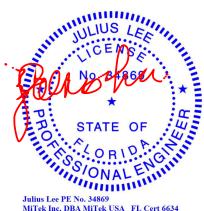
10/24/2024

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by 19 Lumber, Inc..

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2025.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

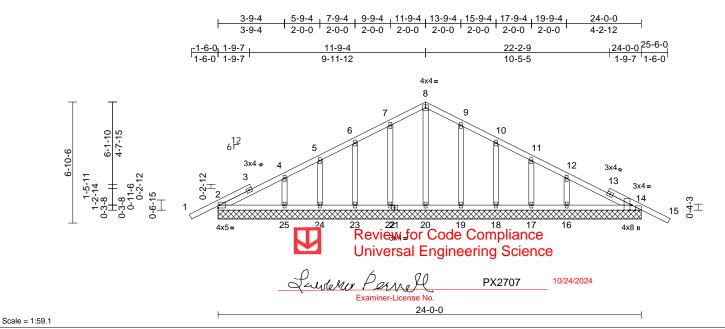


16023 Swingley Ridge Rd. Chesterfield, MO 63017

Ply Job Truss Truss Type Qtv McI end Addition T35223009 1665-A G1 1 Common Supported Gable 1 Job Reference (optional)

19 Lumber, Inc., Old Town, FL - 32680,

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.10	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	14	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 131 lb	FT = 20%

LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.2
DD 4 OINIO	

LIMBER

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (size) 2=24-0-0, 14=24-0-0, 16=24-0-0, 17=24-0-0, 18=24-0-0, 19=24-0-0, 20=24-0-0, 22=24-0-0, 23=24-0-0,

24=24-0-0, 25=24-0-0, 26=24-0-0, 29=24-0-0

Max Horiz 2=-115 (LC 13), 26=-115 (LC 13) Max Uplift 2=-29 (LC 12), 14=-55 (LC 13),

16=-90 (LC 13), 17=-43 (LC 13), 18=-61 (LC 13), 19=-57 (LC 13), 22=-58 (LC 12), 23=-59 (LC 12), 24=-46 (LC 12), 25=-86 (LC 12),

26=-29 (LC 12), 29=-55 (LC 13) Max Grav 2=242 (LC 25), 14=251 (LC 1),

16=302 (LC 1), 17=102 (LC 26), 18=173 (LC 1), 19=166 (LC 26), 20=177 (LC 22), 22=167 (LC 25), 23=167 (LC 1), 24=124 (LC 25), 25=260 (LC 25), 26=242 (LC 25),

29=251 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/42, 2-4=-111/89, 4-5=-49/77 5-6=-27/96, 6-7=-47/137, 7-8=-65/187, 8-9=-65/187, 9-10=-47/137, 10-11=-27/84, 11-12=-25/40, 12-14=-68/68, 14-15=0/42

BOT CHORD 2-25=-45/128, 24-25=-45/128, 23-24=-45/128, 22-23=-45/128, 20-22=-45/128, 19-20=-45/128, 18-19=-45/128, 17-18=-45/128, 16-17=-45/128, 14-16=-45/128 **WEBS** 

8-20=-137/4, 7-22=-128/94, 6-23=-123/99, 5-24=-98/85, 4-25=-185/132, 9-19=-128/93, 10-18=-126/99, 11-17=-88/83,

12-16=-205/135

## NOTES

- Unbalanced roof live loads have been considered for 1) this design
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=5.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 (||) MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 2-0-0 oc. 7)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 2, 55 lb uplift at joint 14, 58 lb uplift at joint 22, 59 lb uplift at joint 23, 46 lb uplift at joint 24, 86 lb uplift at joint 25, 57 lb uplift at joint 19, 61 lb uplift at joint 18, 43 lb uplift at joint 17, 90 lb uplift at joint 16, 29 lb uplift at joint 2 and 55 lb uplift at joint 14.

LOAD CASE(S) Standard



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

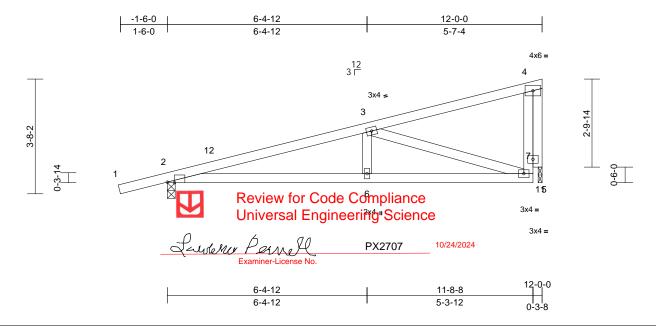




Job	Truss	Truss Type	Qty	Ply	McLeod Addition	
1665-A	M1	Monopitch	12	1	Job Reference (optional)	T35223010

19 Lumber, Inc., Old Town, FL - 32680.

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Scale = 1:36.9

Plate Offsets (X, Y): [2:0-2-12,Edge]

-												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.37	Vert(LL)	0.06	6-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.48	Vert(CT)	-0.12	6-10	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.49	Horz(CT)	0.02	11	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 55 lb	FT = 20%

## LUMBER

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.2 OTHERS

## BRACING

TOP CHORD Structural wood sheathing directly applied or

5-2-9 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=0-3-0, 11=0-1-8

Max Horiz 2=124 (LC 8)

Max Uplift 2=-165 (LC 8), 11=-115 (LC 12)

Max Grav 2=575 (LC 1), 11=442 (LC 1) (lb) - Maximum Compression/Maximum

**FORCES** 

Tension 1-2=0/22, 2-3=-1062/247, 3-4=-130/10,

TOP CHORD

5-7=-70/333, 4-7=-70/333

BOT CHORD 2-6=-329/1007, 5-6=-329/1007 **WEBS** 3-6=0/256, 3-5=-968/308, 4-11=-448/144

NOTES

- Unbalanced roof live loads have been considered for
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=5.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 -1-6-7 to 1-5-9, Zone1 1-5-9 to 11-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 11.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 2 and 115 lb uplift at joint 11.

LOAD CASE(S) Standard



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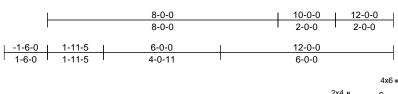


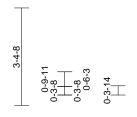
Job	Truss	Truss Type	Qty	Ply	McLeod Addition	
1665-A	M2	Monopitch Supported Gable	1	1	Job Reference (optional)	T35223011

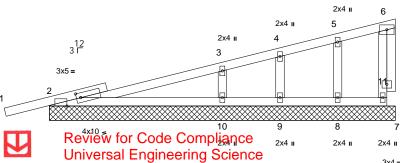
19 Lumber, Inc., Old Town, FL - 32680.

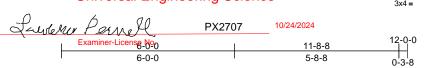
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Scale = 1:35.7

Plate Offsets (X, Y): [2:0-1-12,0-2-0], [2:0-5-12,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.36	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 53 lb	FT = 20%

## LUMBER

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.2 **OTHERS** 

## **BRACING**

Structural wood sheathing directly applied or TOP CHORD

6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=12-0-0, 7=12-0-0, 8=12-0-0, 9=12-0-0, 10=12-0-0, 12=12-0-0

Max Horiz 2=125 (LC 9), 12=125 (LC 9) Max Uplift 2=-157 (LC 8), 7=-14 (LC 9), 8=-55 (LC 12), 9=-54 (LC 1), 10=-150 (LC

12), 12=-157 (LC 8)

Max Grav 2=405 (LC 1), 7=44 (LC 1), 8=213 (LC 1), 9=17 (LC 12), 10=555 (LC

1), 12=405 (LC 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-3=-191/100, 3-4=-100/59, 4-5=-91/70, 5-6=-50/53, 6-7=-39/54

2-10=-58/59, 9-10=-44/59, 8-9=-44/59, BOT CHORD 7-8=-44/59

WEBS 5-8=-148/166 4-9=0/20 3-10=-350/365

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=5.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 2, 14 lb uplift at joint 7, 55 lb uplift at joint 8, 54 lb uplift at joint 9, 150 lb uplift at joint 10 and 157 lb uplift at joint 2.

LOAD CASE(S) Standard



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017



Job Truss Truss Type Qty McLeod Addition T35223012 1665-A T1 10 1 Common Job Reference (optional)

19 Lumber, Inc., Old Town, FL - 32680,

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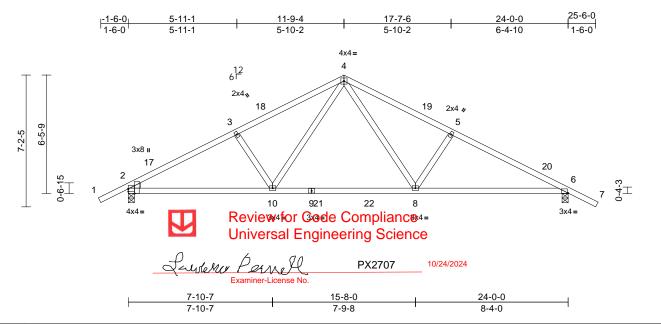


Plate Offsets (X, Y): [2:Edge,0-1-12], [2:0-3-0,Edge], [6:0-0-8,Edge]

-												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.15	8-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.25	8-10	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 113 lb	FT = 20%

## LUMBER

Scale = 1:62.1

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.2 WEBS WFDGF Left: 2x4 SP No.2

## **BRACING**

Structural wood sheathing directly applied or TOP CHORD

3-11-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=0-4-0, 6=0-4-0

Max Horiz 2=-120 (LC 13)

Max Uplift 2=-195 (LC 12), 6=-199 (LC 13) Max Grav 2=1119 (LC 2), 6=1119 (LC 2)

**FORCES** (lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=0/42, 2-3=-1696/267, 3-4=-1554/271, 4-5=-1630/285, 5-6=-1778/281, 6-7=0/42

BOT CHORD 2-10=-249/1468, 8-10=-73/1014,

6-8=-159/1560

**WEBS** 4-8=-135/711, 5-8=-374/223, 4-10=-119/603,

3-10=-325/210

## NOTES

- 1) Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=5.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 -1-6-13 to 1-5-3, Zone1 1-5-3 to 11-9-4, Zone2 11-9-4 to 16-0-2, Zone1 16-0-2 to 25-6-13 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 2 and 199 lb uplift at joint 6.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

October 9,2024



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not Design Valid for use only with will relew connectors. This design is based only upon parameters shown, and is for an individual unulang component, nor a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/I/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



Job	Truss	Truss Type	Qty	Ply	McLeod Addition	
1665-A	T2	Common	7	1	Job Reference (optional)	T35223013

19 Lumber, Inc., Old Town, FL - 32680.

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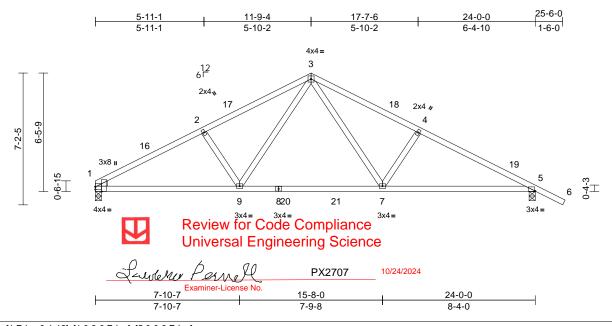


Plate Offsets (X, Y): [1:Edge,0-1-12], [1:0-3-0,Edge], [5:0-0-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.15	7-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.80	Vert(CT)	-0.25	7-15	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.05	5	n/a	n/a		
BCDL	10.0	Code	FBC2023/TPI2014	Matrix-MS							Weight: 111 lb	FT = 20%

## LUMBER

Scale = 1:62.1

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.2 WEBS WFDGF Left: 2x4 SP No.2

## **BRACING**

TOP CHORD Structural wood sheathing directly applied or

3-11-5 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 1=0-4-0, 5=0-4-0

Max Horiz 1=-133 (LC 13)

Max Uplift 1=-158 (LC 12), 5=-200 (LC 13) Max Grav 1=1039 (LC 2), 5=1121 (LC 2)

**FORCES** (lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=-1712/283, 2-3=-1569/295,

3-4=-1635/286, 4-5=-1783/282, 5-6=0/42 BOT CHORD 1-9=-258/1483, 7-9=-75/1020, 5-7=-171/1564 WEBS 3-7=-134/710, 4-7=-374/223, 3-9=-126/617,

2-9=-335/214

## NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=5.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior (2) zone and C-C Zone3 0-0-0 to 3-0-0, Zone1 3-0-0 to 11-9-4, Zone2 11-9-4 to 16-0-2, Zone1 16-0-2 to 25-6-13 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 1 and 200 lb uplift at joint 5.

LOAD CASE(S) Standard



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

October 9,2024



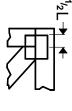
🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

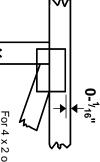


## Symbols

## PLATE LOCATION AND ORIENTATION



offsets are indicated. and fully embed teeth Apply plates to both sides of truss Center plate on joint unless x, y Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates required direction of slots in This symbol indicates the

\* Plate location details available in MiTek software or upon request

## **PLATE SIZE**



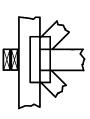
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

## LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. ndicated by symbol shown and/or

## **BEARING**



number/letter where bearings occur reaction section indicates joint Min size shown is for crushing only (supports) occur. Icons vary but Indicates location where bearings

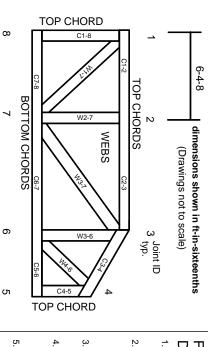
## Industry Standards:

ANSI/TPI1:

National Design Specification for Metal Plate Connected Wood Trusses Installing, Restraining & Bracing of Metal Guide to Good Practice for Handling Building Component Safety Information, Design Standard for Bracing. Plate Connected Wood Truss Construction.

DSB-22: BCSI:

## Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

# Design General Notes

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

established by others section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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## SiTe

MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

# **General Safety Notes**

## Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.

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- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1. Place plates on each face of truss at each

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Universal Engineering Science

- 11. Plate type, size, orientation and location dimensions
- The environment in accord with ANSI/TPI 1.

  8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

  9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lume.

  10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.

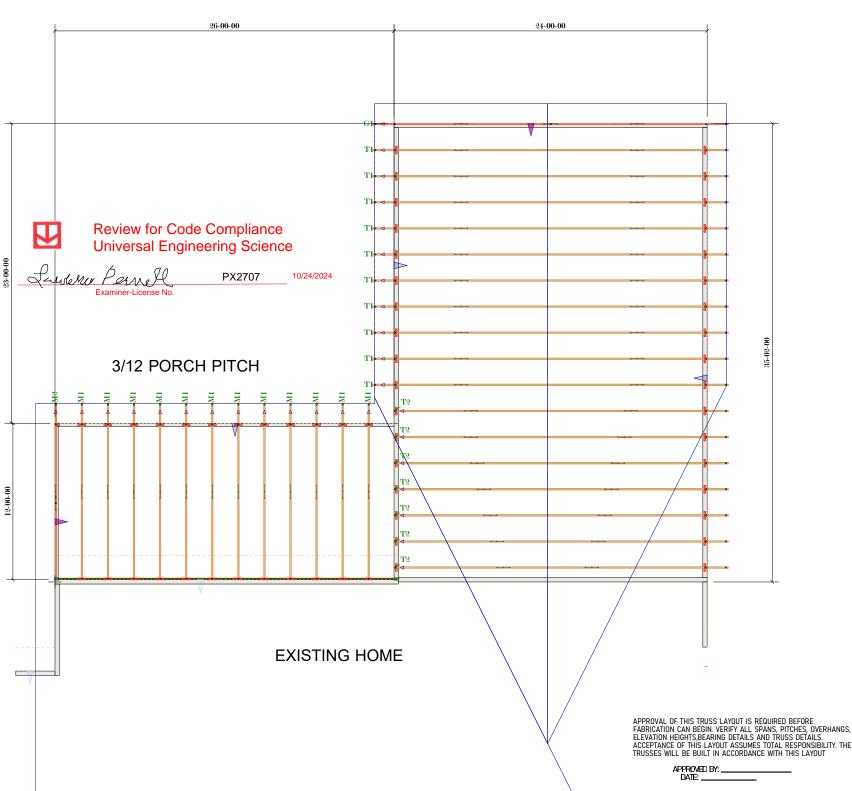
  11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.

  12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.

  Review of the species and size, and in all respects, equal to or better than that specified. Lumber used shall be of the species and size, and

Lawle Mar Parmell Examiner-License No.

- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



JOB NAME: Mcleod Addition McLeod Residence

CUSTOMER NAME: Parrish Builders Group

ADDRESS: 751 SW Jacobs CT. Fort Whte FL 32028 JOB# 1665

DATE: 10/2/2024

PITCH: 6/12 OVERHANG: 1-06-00 ROOF SPACING: 24"

WALL HEIGHT: 9'

### LOADING CRITERIA

BUILDING CODE: FBC 2023 WIND STANDARD: ASCE722 WIND VELOCITY: 130 mph EXPOSURE CATEGORY: B

Roof Loading
TCLL:20.0 lb/ft²
TCDL:10.0 lb/ft²
BCDL:10.0 lb/ft²
Floor Loading
TCLL:40.0 lb/ft²
TCDL:10.0 lb/ft²
BCDL:5.0 lb/ft²

## WARNING

IT IS THE RESPONSIBLITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE ALL CONNECTIONS OTHER THAN TRUSS TO TRUSS, UNLESS SPECIFIED OTHERWISE.

CONVENTIONAL
FRAMING,ERECTION OR
PERMANENT BRACING IS NOT
THE RESPONSIBLITY OF THE
TRUSS DESIGNER OR TRUSS
MANUFACTURER.

TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE

> DO NOT CUT OR ALTER TRUSSES IN ANY WAY

## NOTES:

\*ALL DEMENSIONS ARE FEET-INCHES- SIXTEENTHS

\*NO BACKCHARGES WILL BE ACCEPTED



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