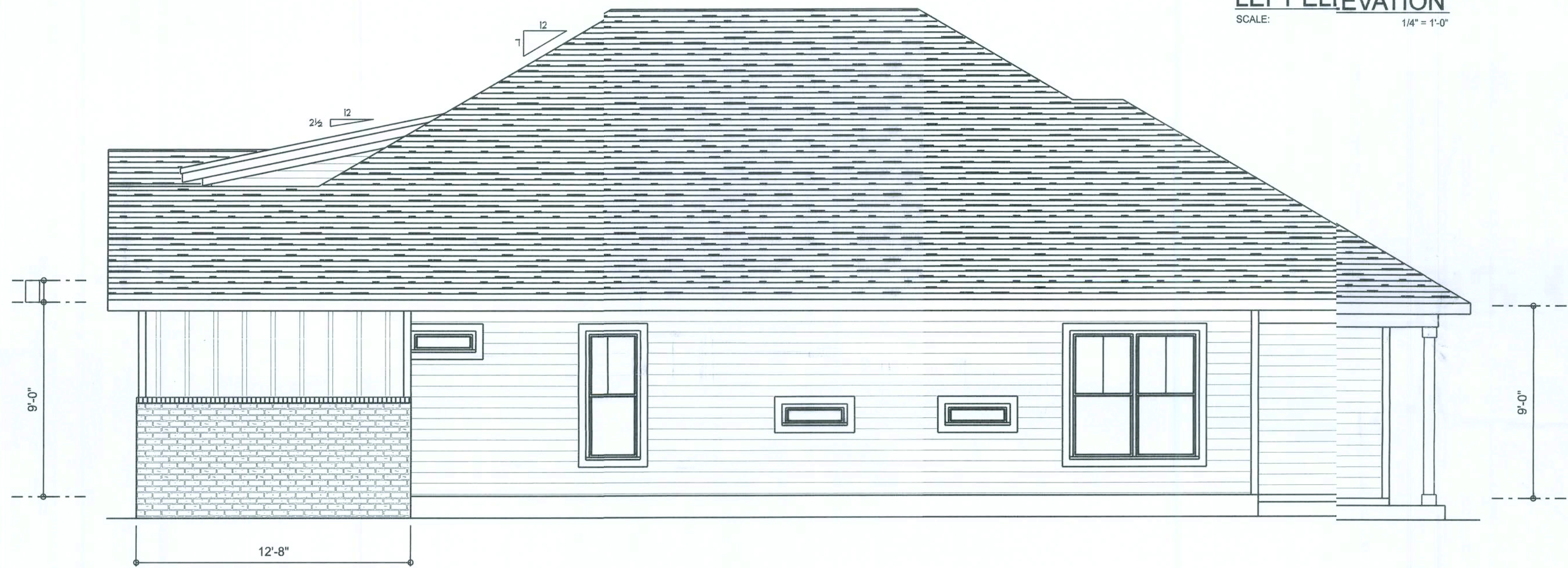


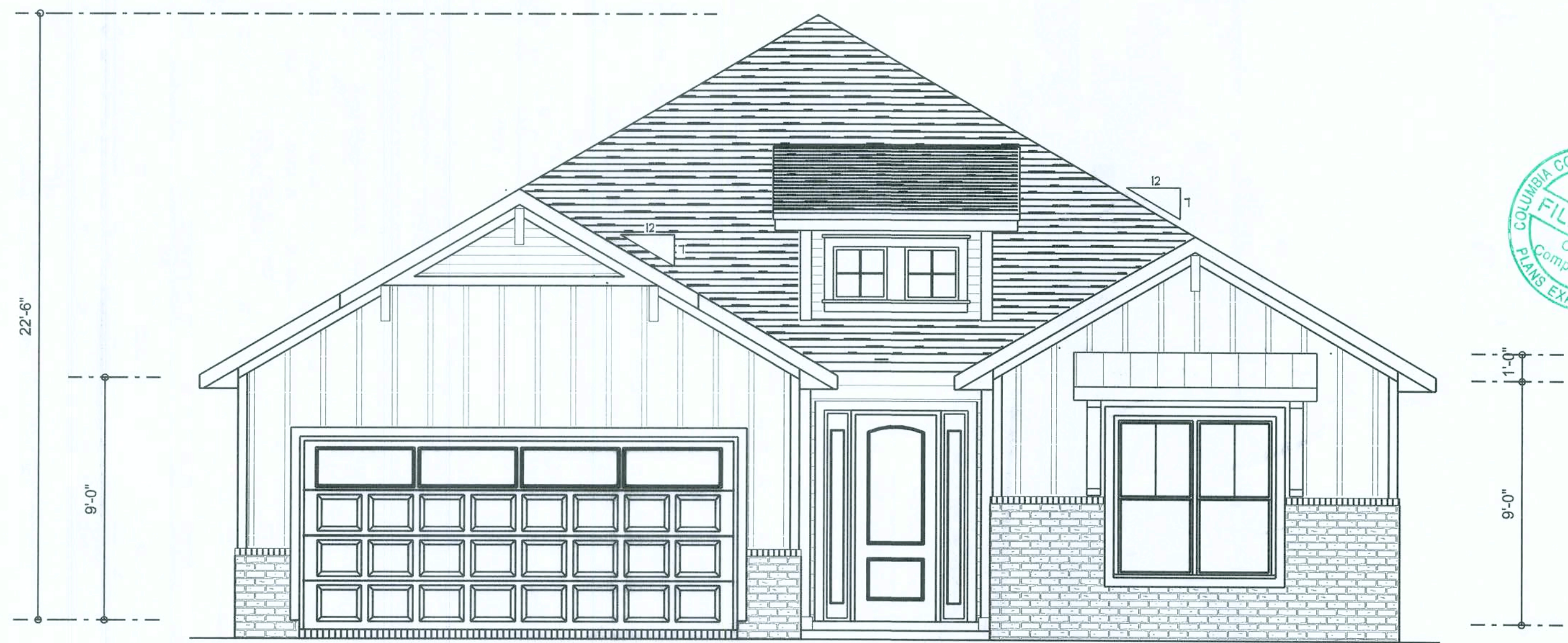
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

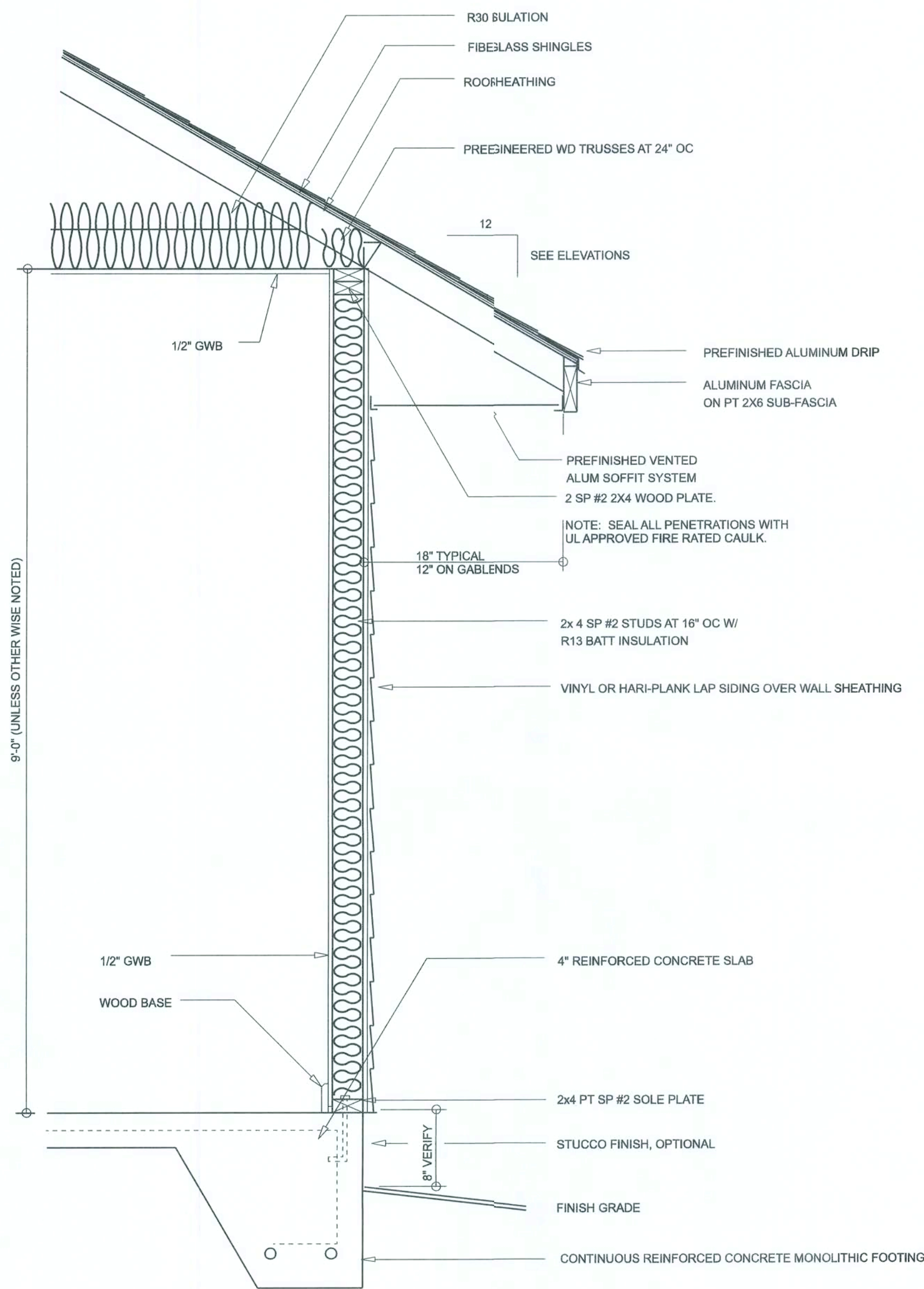


REVISIONS SCHEDULE	
Oct. 23rd, 2020	CONST. DRWGS

THE IVY MODEL FOR:
AARON SIMQUE HOMES, INC.
LOT 52, THE PRESERVES, LAKE CITY, FL 32024

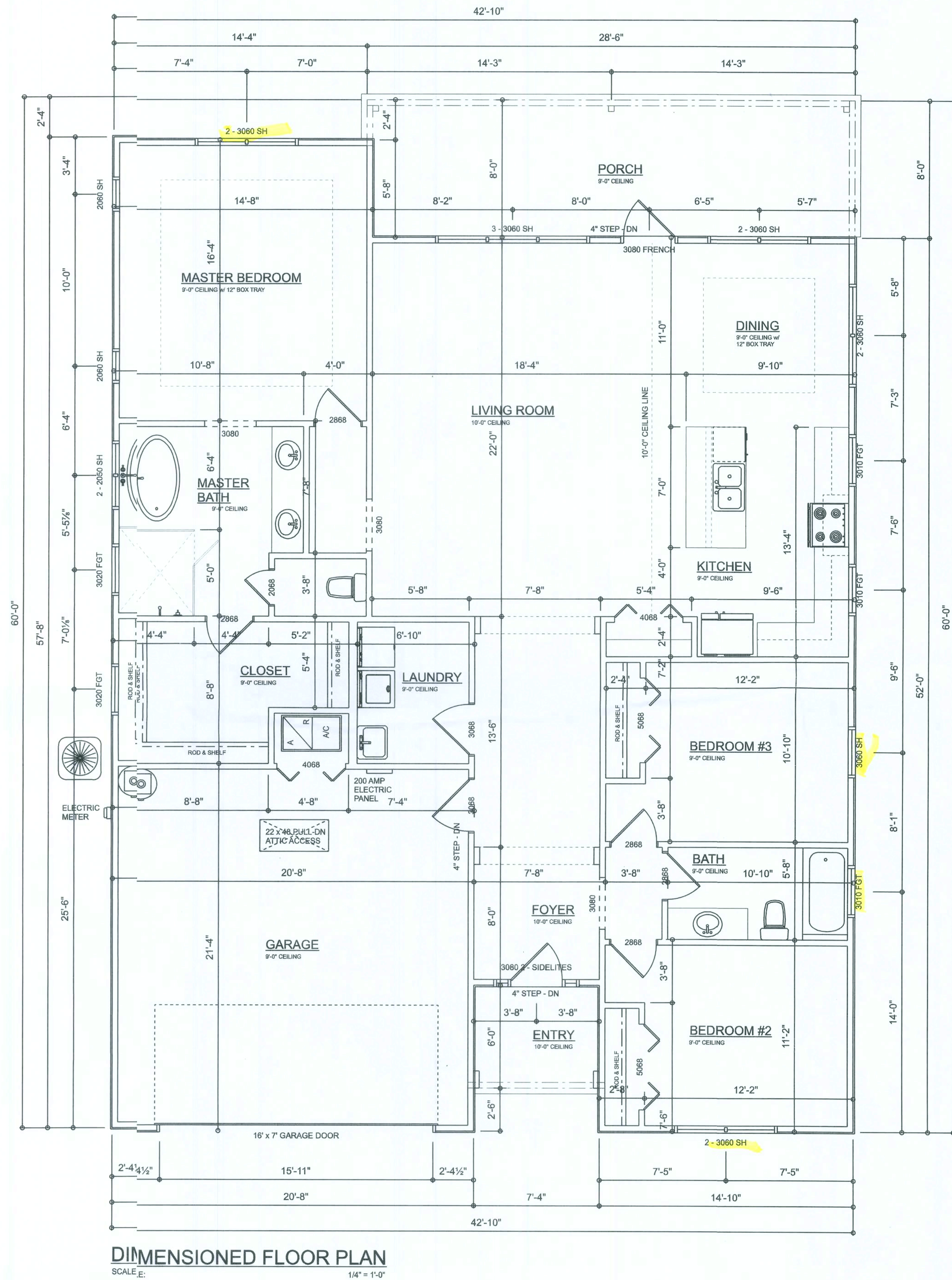
RIDGEPOINT DESIGN
566 SW ARLINGTON BLVD., STE 101, LAKE CITY, FL 32025
P: 386-288-1188
E: RIDGEPOINTDESIGN@GMAIL.COM

SHEET NUMBER
A.1
OF 3 SHEETS



TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

AREA SUMMARY		
HEATED & COOLED	1,807	S.F.
ENTRY PORCH	38	S.F.
REAR COVERED PORCH	226	S.F.
GARAGE	441	S.F.
TOTAL LIVING	2,512	S.F.

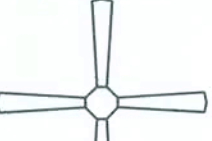


















REVISIONS SCHEDULE	
Oct. 23rd, 2020	CONST. DRWGS

THE IVY MODEL FOR:
AARON SIMQUE HOMES, INC
LOT 24, THE PRESERVES, LAKE CITY, FL 32114

RIDGEPOINT DESIGN
566 SW ARLINGTON BLVD, STE 101 LAKE CITY, FL 32025
P: 386-288-1188
E: RIDGEPOINTDESIGN@GMAIL.COM

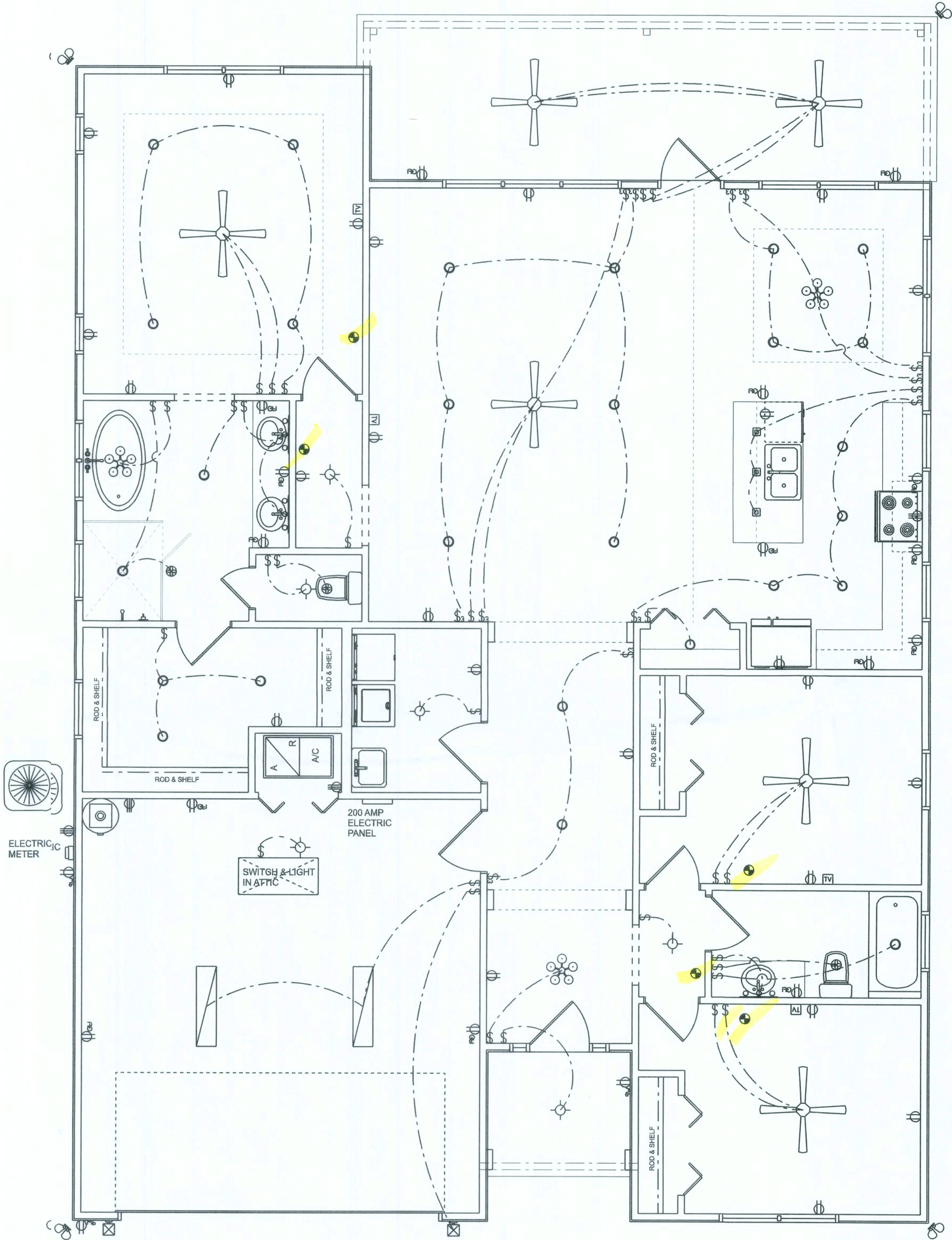
SHEET NUMBER
A.2
OF 3 SHEETS

ELECTRICALLEGEND		
ELECTRICAL	COUNT	SYMBOL
ceiling fan 4 bladed 01	6	
can light 6inch	27	
ceiling light 14	3	
fluorescent light 1 x 4	2	
pendant cube	3	
exterior light 02	2	
spotlight double	4	
electrical meter	1	
cable tv outlet	4	
fan	3	
light	6	
outlet	24	
outlet 220v	4	
outlet gfi	15	
outlet wp	3	
smoke detector	5	
switch	30	
switch 3 way	12	
vanity bar light 02	3	

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY B&KUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IACTUATED THEY
ALL ACTIVATE.

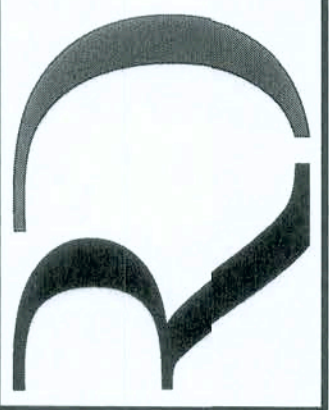
NOTE:
UPPER grounding required per N.E.C.
Arc fault breakers required per N.E.C.
GFCI breakers required per N.E.C.
Tamper resistant neceptacles required per N.E.C.



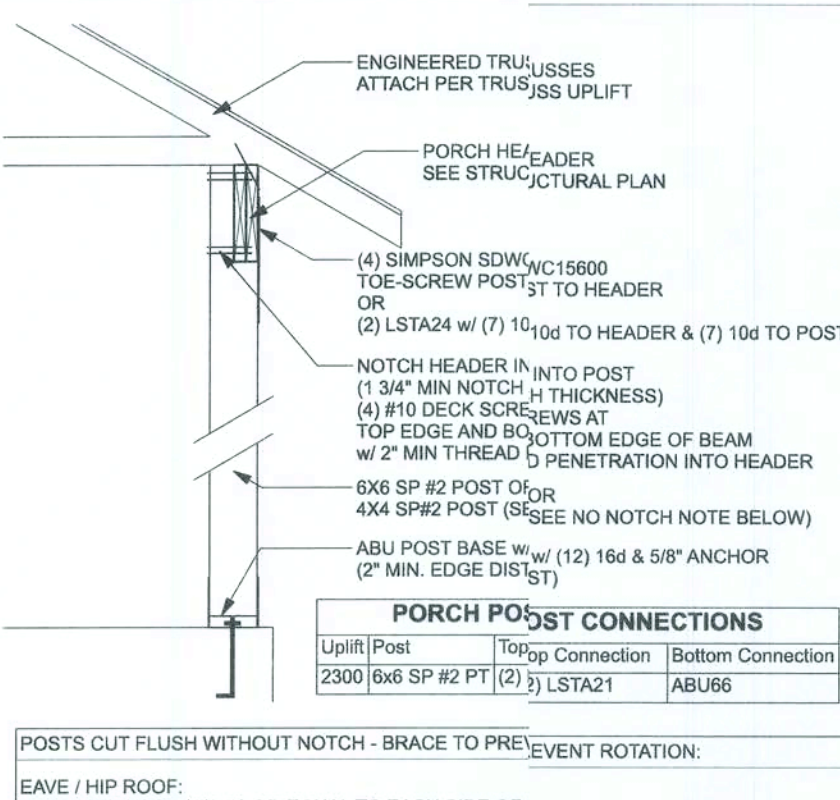
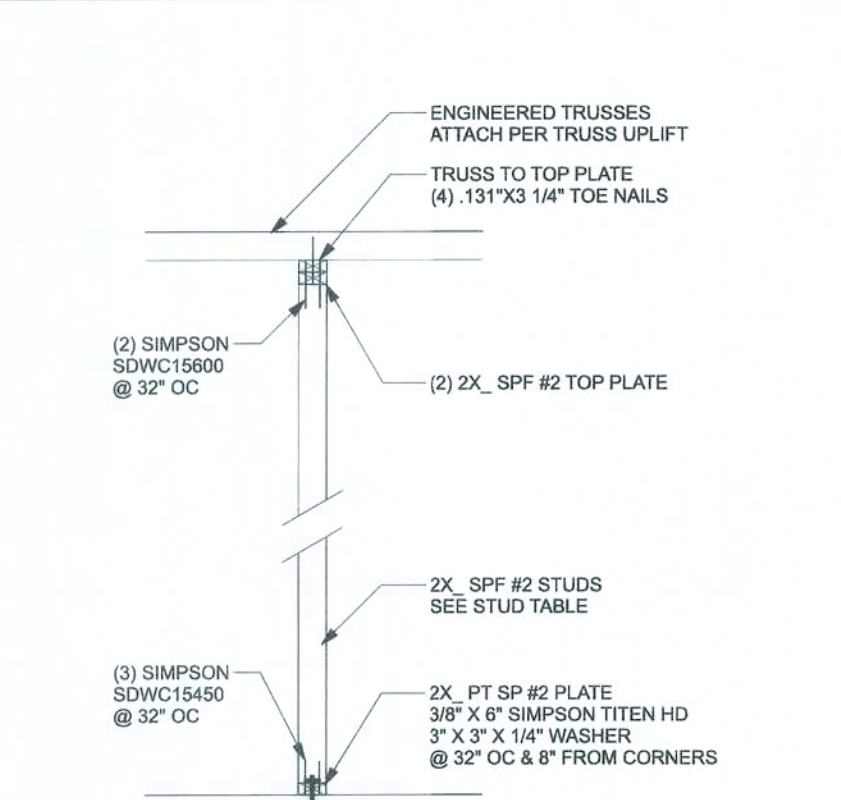
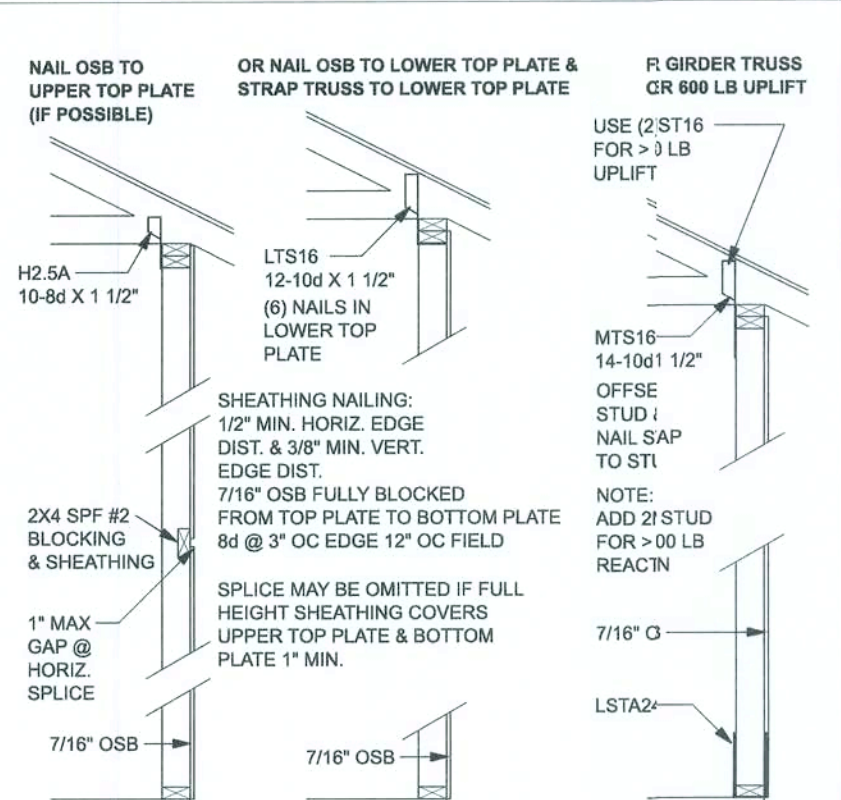
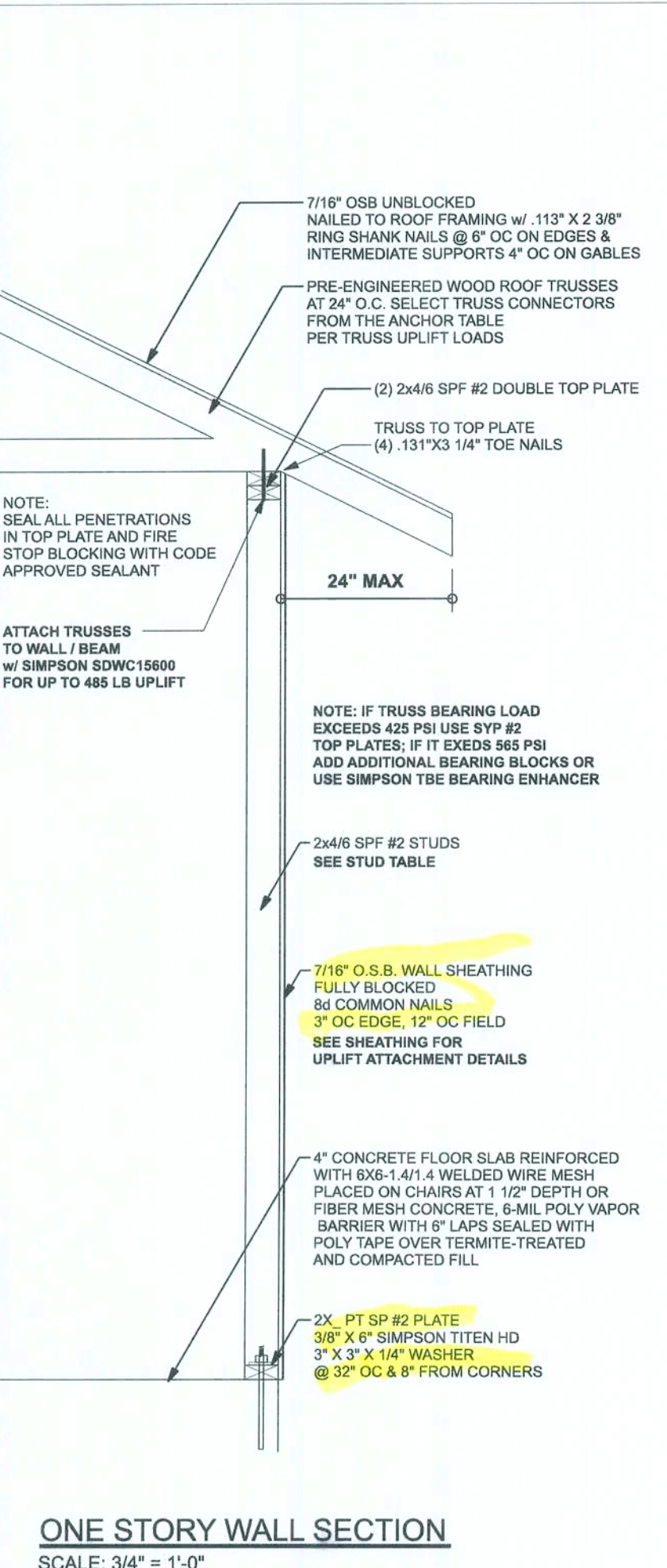
ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

REVISIONS SCHEDULE	
Oct. 23rd, 2020	CONST. DRWGS

THE IVY MODEL FOR:
AARON SIMQUE HOMES, INC
LOT 52, THE PRESERVES, LAKE CITY, FL 32024



**RIDGEPOINT
DESIGN**
544 NW ANDERSON BLVD, STE 101 LAKE CITY, FL 32032
P: 386-288-1188
E: RIDGEPOINTDESIGN@GMAIL.COM



CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8dX1 1/2"	4-8dX1 1/2"
415	290	H3	H2.5A	5-8dX1 1/2"
575	465	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1240	1245	H1325-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
665	635	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1655	1650	DTT22	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	12"x6" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
2200	2200	ABU44	5/8"x12" Drill & Epoxy	5/8"x12" Drill & Epoxy
2300	2300	ABU66	5/8"x12" Drill & Epoxy	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	2200	ABU44	5/8"x7" Drill & Epoxy	5/8"x7" Drill & Epoxy
2300	2300	ABU66	5/8"x7" Drill & Epoxy	5/8"x7" Drill & Epoxy

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WELD LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $P_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W1 x 4" W1 x 4" FB = 8XSL WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH & WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" DB (2' FOR 6 BAR); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 11" x 2 3/8" RING SHANK NAILS @ 8" OC ON EDGES & INTERMEDIATE SUPPORTS 4" OC ON GABLES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

(TYP.) GABLE BRACING DETAIL
WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR WALL BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

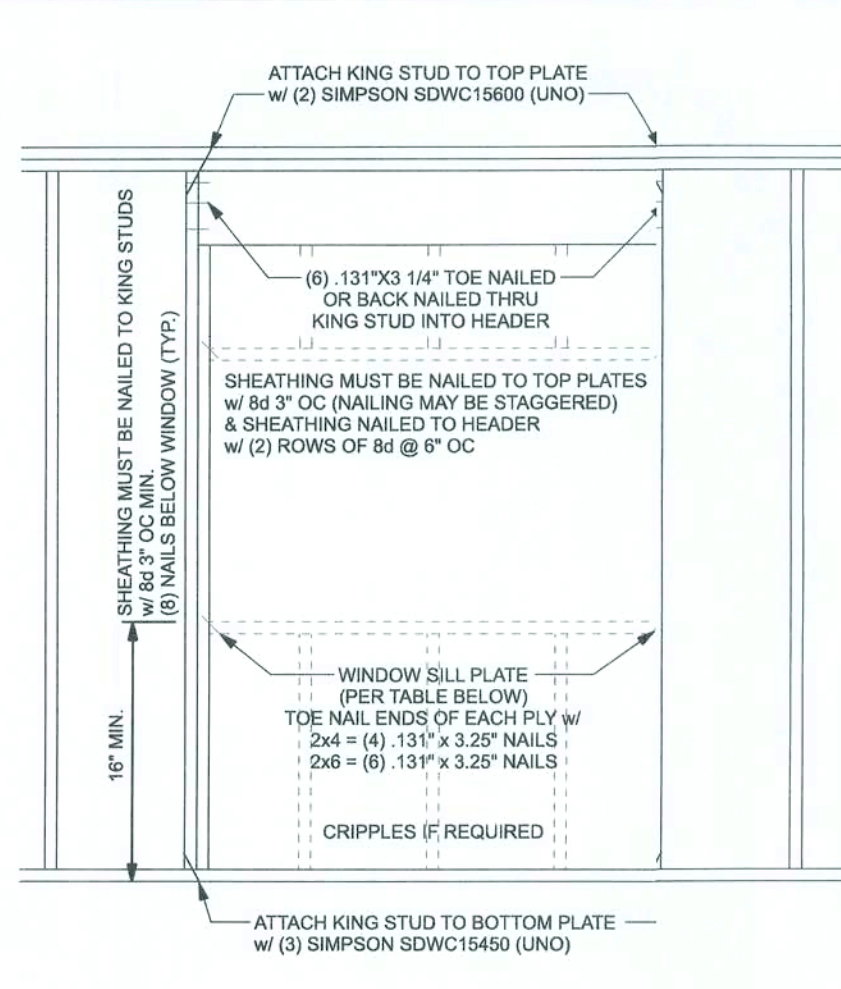
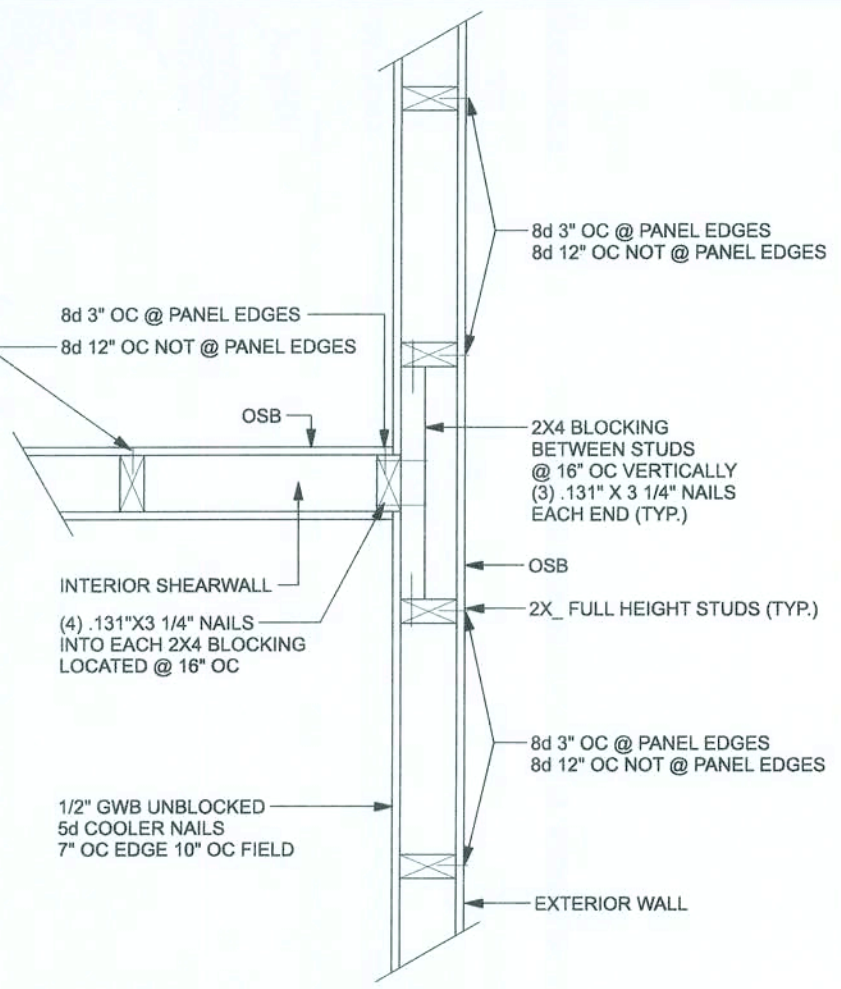
STUD HEIGHT	STUD WIDTH
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

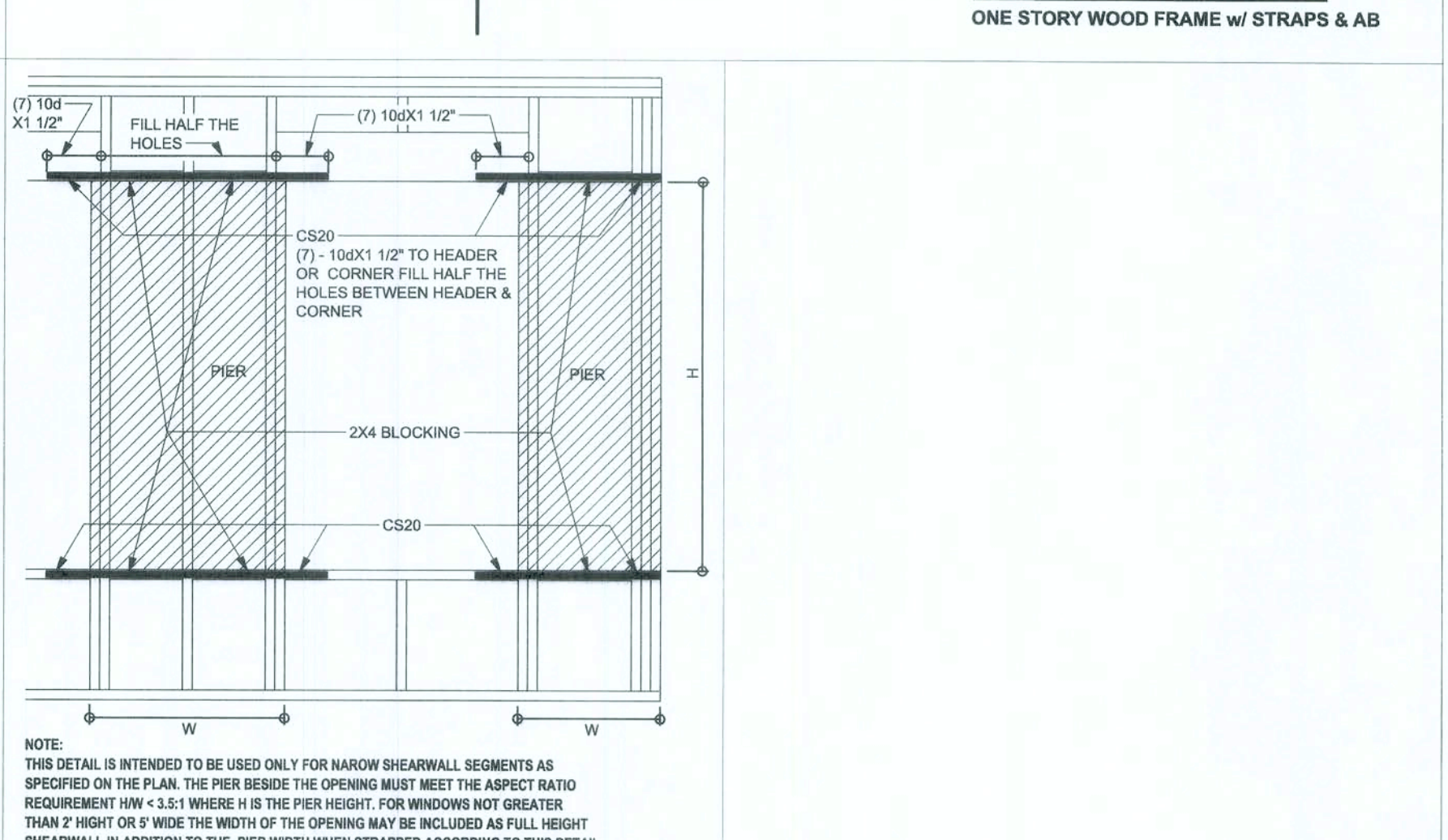
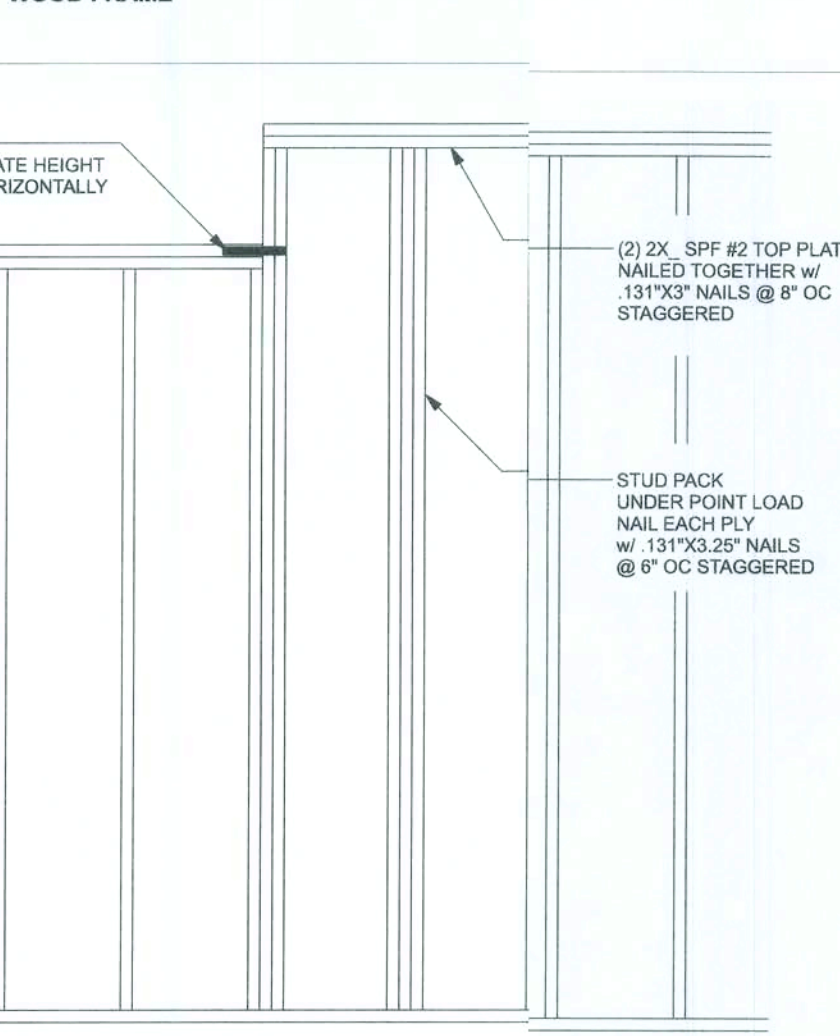
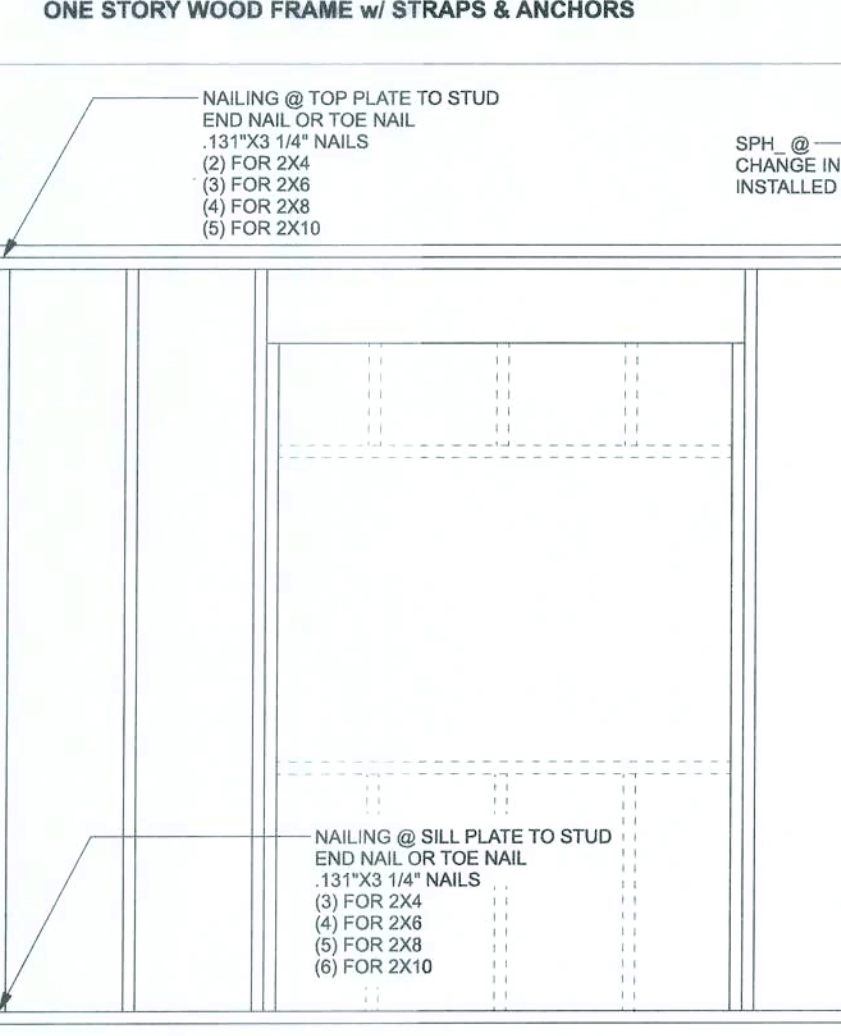
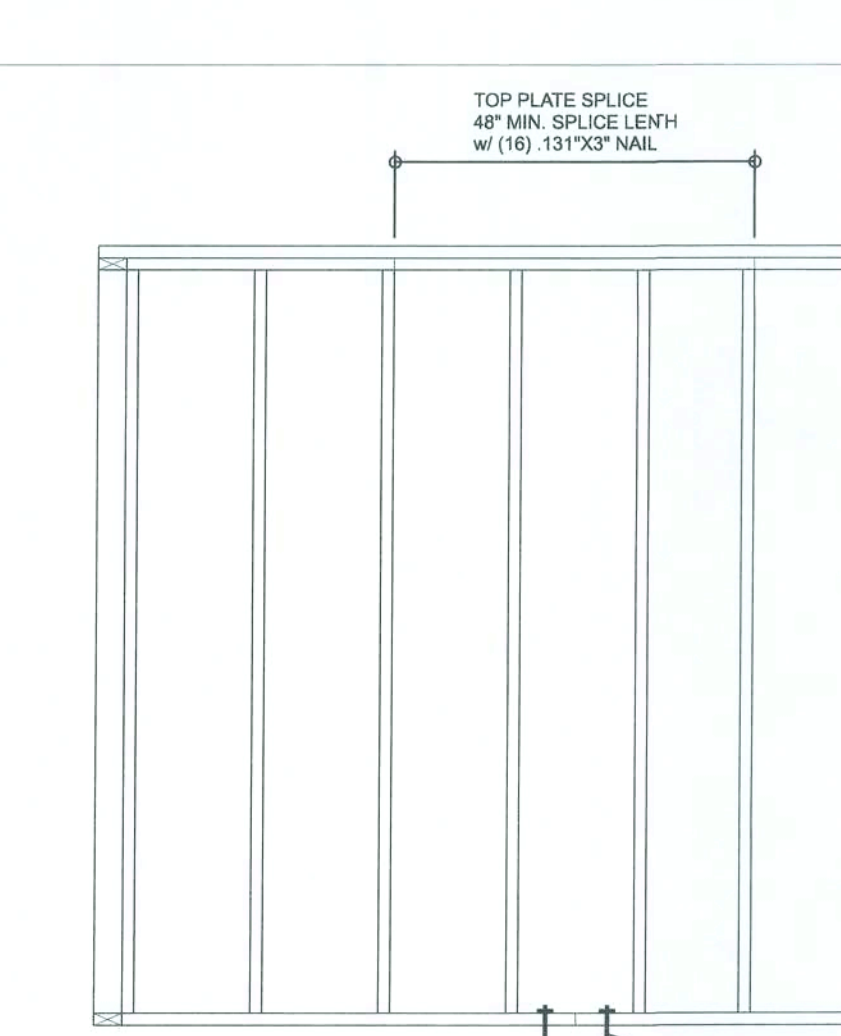
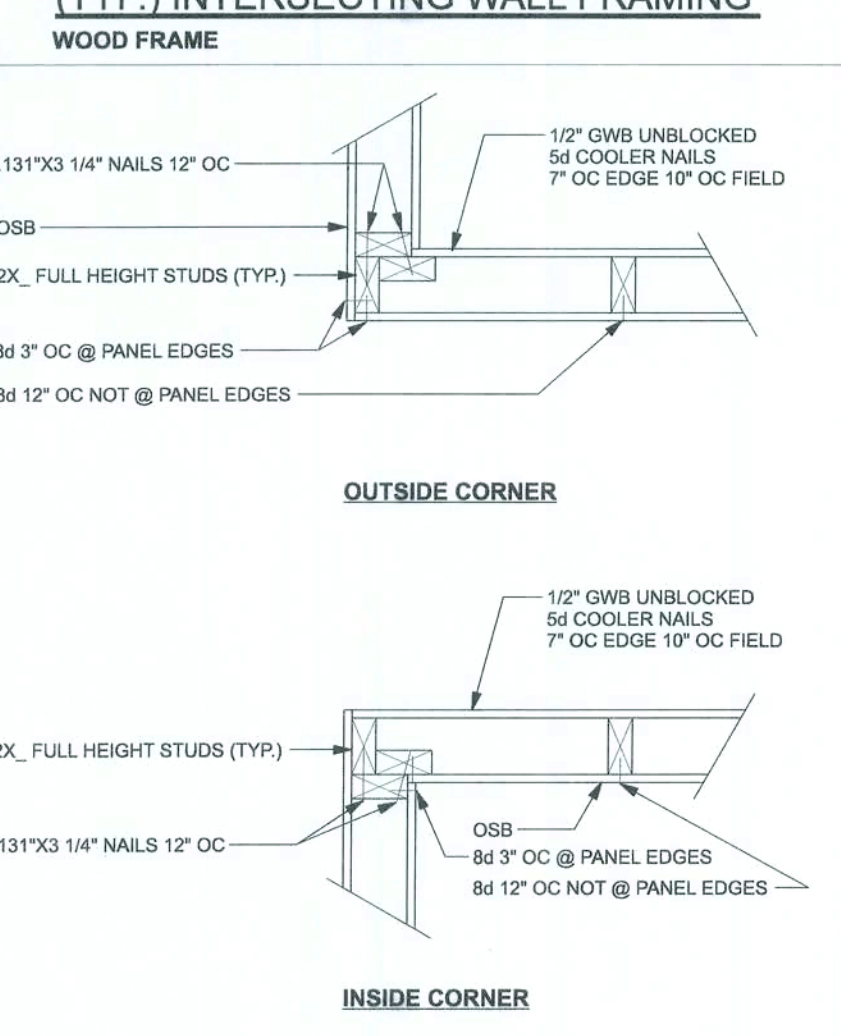
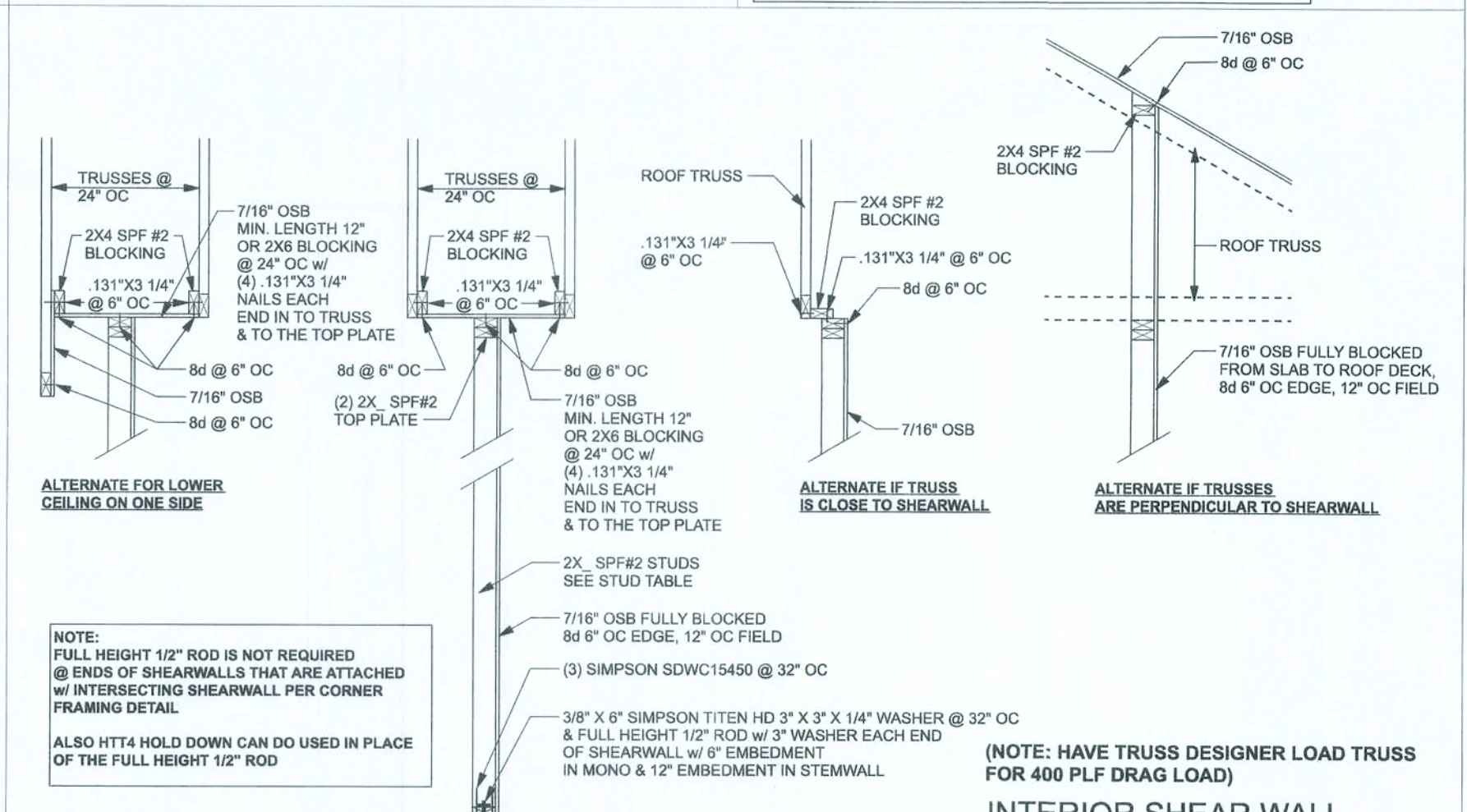
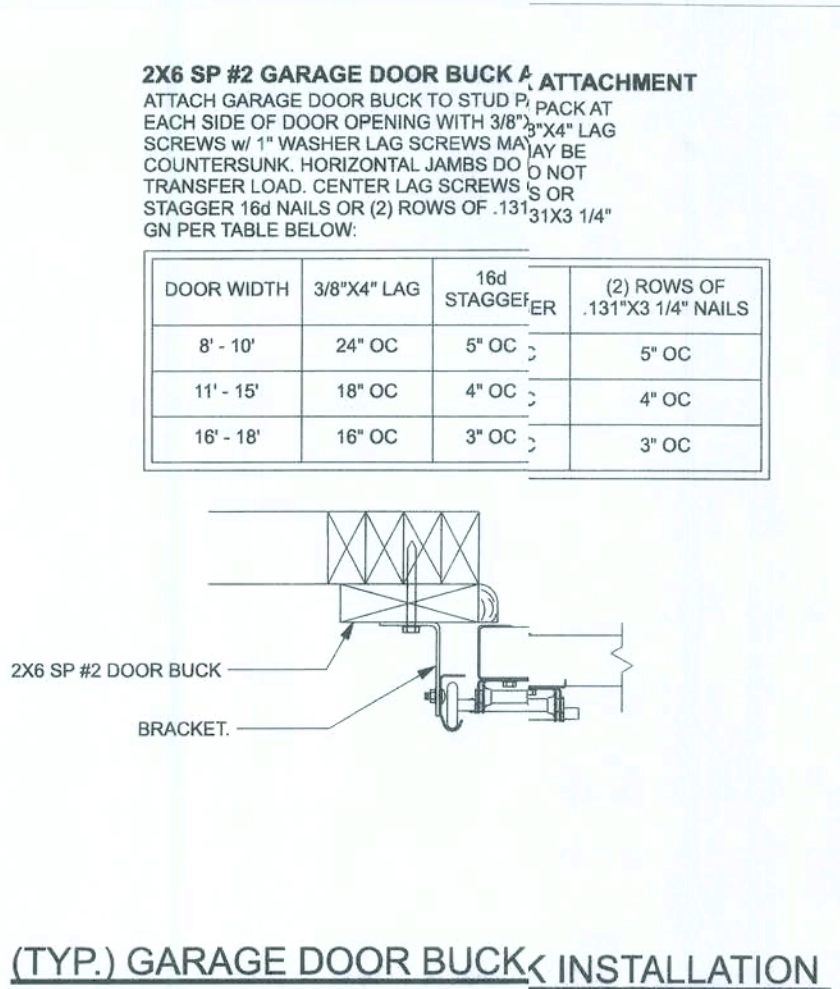
GRADE & SPECIES TABLE

GRADE	SP #2	F _b	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	760	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR. BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.





DESIGN CRITERIA & LOADS:

BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	18 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (F _{T2})	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasnd) -27.8(Vasnd)	+25.6(Vasnd) -34.2(Vasnd)
0 - 20	+42.6(Vasnd) -46.2(Vasnd)	+42.6(Vasnd) -57(Vasnd)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.8(Vasnd) -25.5(Vasnd)
16x7 GARAGE DOOR	+21.7(Vasnd) -24.1(Vasnd)

Aaron Siqueira Homes

PROJECT ADDRESS: Lot 52 The Preserves Lake City, FL

Key - Lot 52 The Preserves

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all creations to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 89815

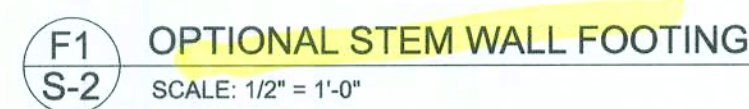
FLORIDA PROFESSIONAL ENGINEER No. 5996

Wednesday, October 28, 2020

Mark Disosway P.E.
163 SW Milltown Place
Suite 103
Lake City, Florida 32025
386.74.5419
disoswaydesign@gmail.com

JOB NUMBER:
201152

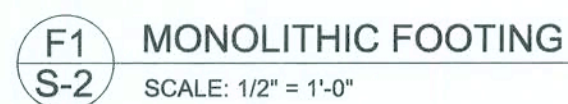
S-1
OF 3 SHEETS



TALL STEM WA TABLE:

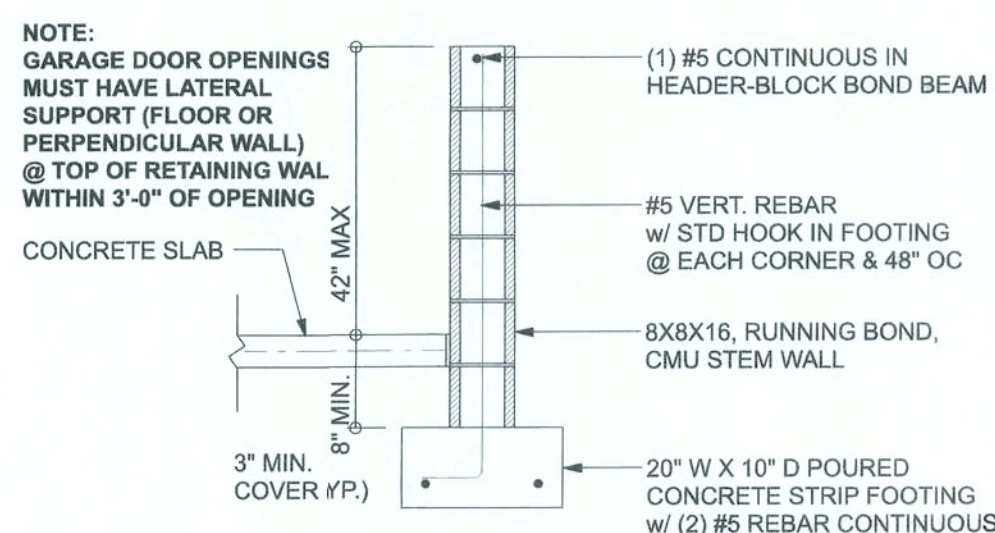
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, a Duowall ladder reinforcement at 16"OC vertically or a horizontal bond with #4s continuous at height. For taller parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNLANCED HEIGHT (FEET)	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)	VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)
		#5 #7 #8	#5 #7 #8
3.3	3.0	96 96 96	96 96 96
4.0	3.7	96 96 96	96 96 96
4.7	4.3	88 96 96	96 96 96
5.3	5.0	56 96 96	96 96 96
6.0	5.7	40 80 96	80 96 96
6.7	3.3	32 56 80	56 96 96
7.3	7.0	24 40 56	40 80 96
8.0	7.7	16 32 48	32 64 80
8.7	3.3	8 24 32	24 48 64
9.3	3.0	8 16 24	16 40 48



<p>MASONRY NCE:</p> <p>MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY CONSTRUCTION" (ACI 308.1-14) AND SFS 102.02. THE CONTRAOR AND MASON MUST IMMEDIATELY BEFORE PROCEEDING/NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN AG300-1422 AND THESE DESIGN DRAWINGS.</p> <p>ANY EXCEEDS TO AG300-1422 MUST BE APPROVED BY THE ENGINEER IN WRITING.</p>	
ACI/CS 2 Section	Specific Requirements
1.AA Compensate strength	R' block bearing walls $P = 1500$ psi
ASTM C 270, Type N, UNO	
2.D Grout	ASTM C 476, admixtures require approval
3.CMU stand	ASTM C 90-02, normal weight, Hollow, medium surface finish, P_{max} 10" turning bond and 12"x12" or 16"x16" max
2.B.3 Clay brickload	ASTM C 216.02, Grade SW, Type FBS, 5.9, 2.75"x11.5"
2.4F Reinforci bars, #3 -11	ASTM A615, Grade 40, $F_y = 40$ ksi, Lap splice, 12" bar dia. (25" max)
2.4F Coasting corrosion protection	ASTM E168, metal tiles less completely embedded in mortar or grout, ASTM A502, Class 680, 0.60 carbon, 0.045 S, 0.045 AS
2.4F Coasting corrosion protection	Joint reinforcement in walls exposed to moisture or water tiles, anchors, steel mesh not completely embedded in mortar or grout, ASTM A153, Class B2, 1/80 cold2 or 304SS
3.3.2 Pipes, culits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.2 Movements	Contrator assumes responsibility for type and location of movement joints not installed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF
12" BELOW UNSTURBED SOIL OR ENGINEERED FILL
PER FBC 2017-ES, SECTION R403.1.4

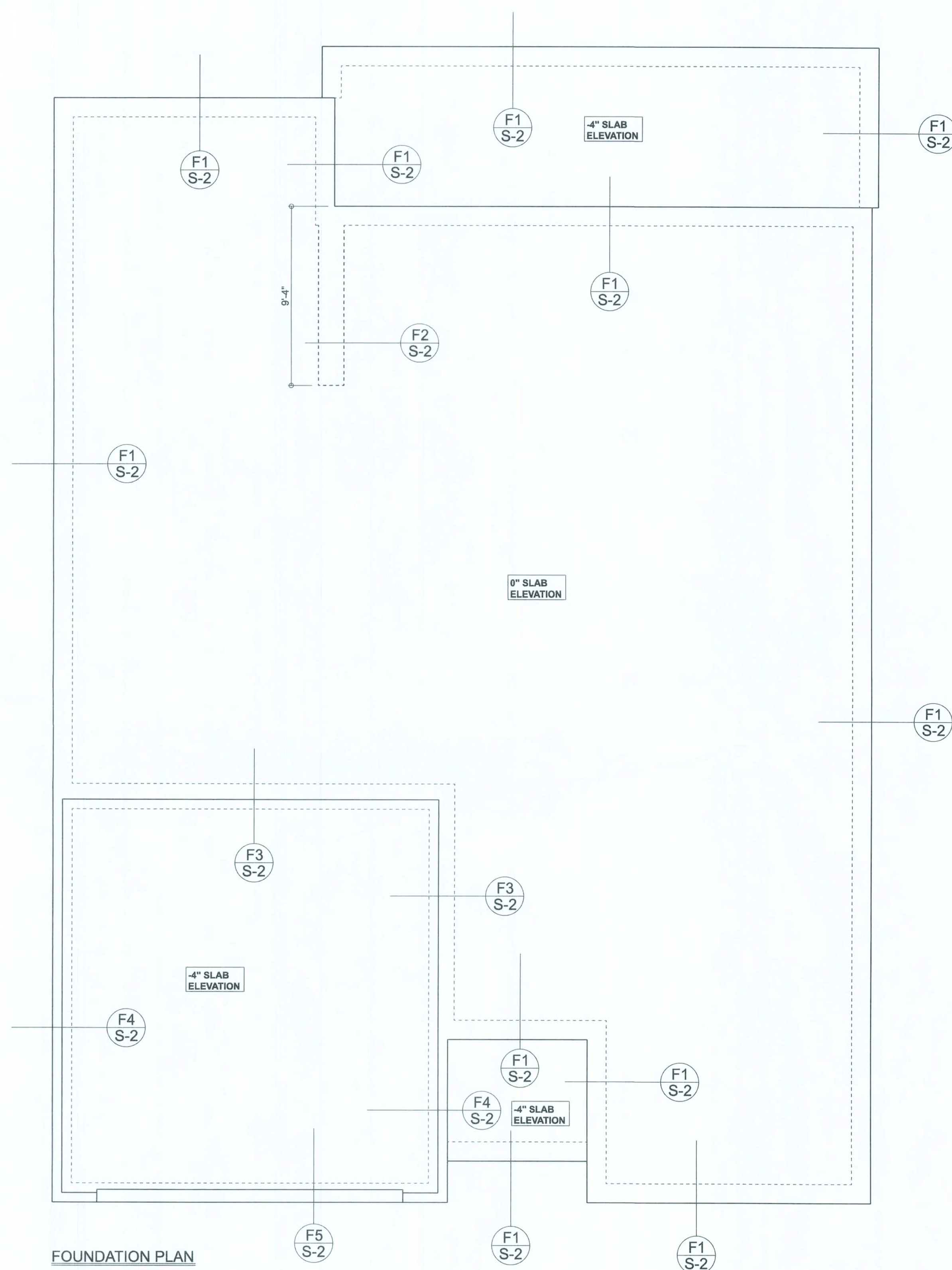


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING/THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X6-1/4 L414 WELDED WIRE MESH PLACED ON CHAIRS @ 1/4" DEPTH OR ORDER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER, 2" EPS SEA POLY THERM OVER THERM-TREATED & COMPACTED FILL.



Aaron Simque Homes

Survey - Lot 52 The Preserves

PROJECT ADDRESS:
Lot 52 The Preserves
Lake City, FL

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. or resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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OF 33 SHEETS

