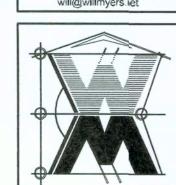


A NEW SPEC HO
A



JOB NUMBER 061204

S.F.

S.F.

S.F.

S.F.

1610

492

2,134

32

LIVING AREA

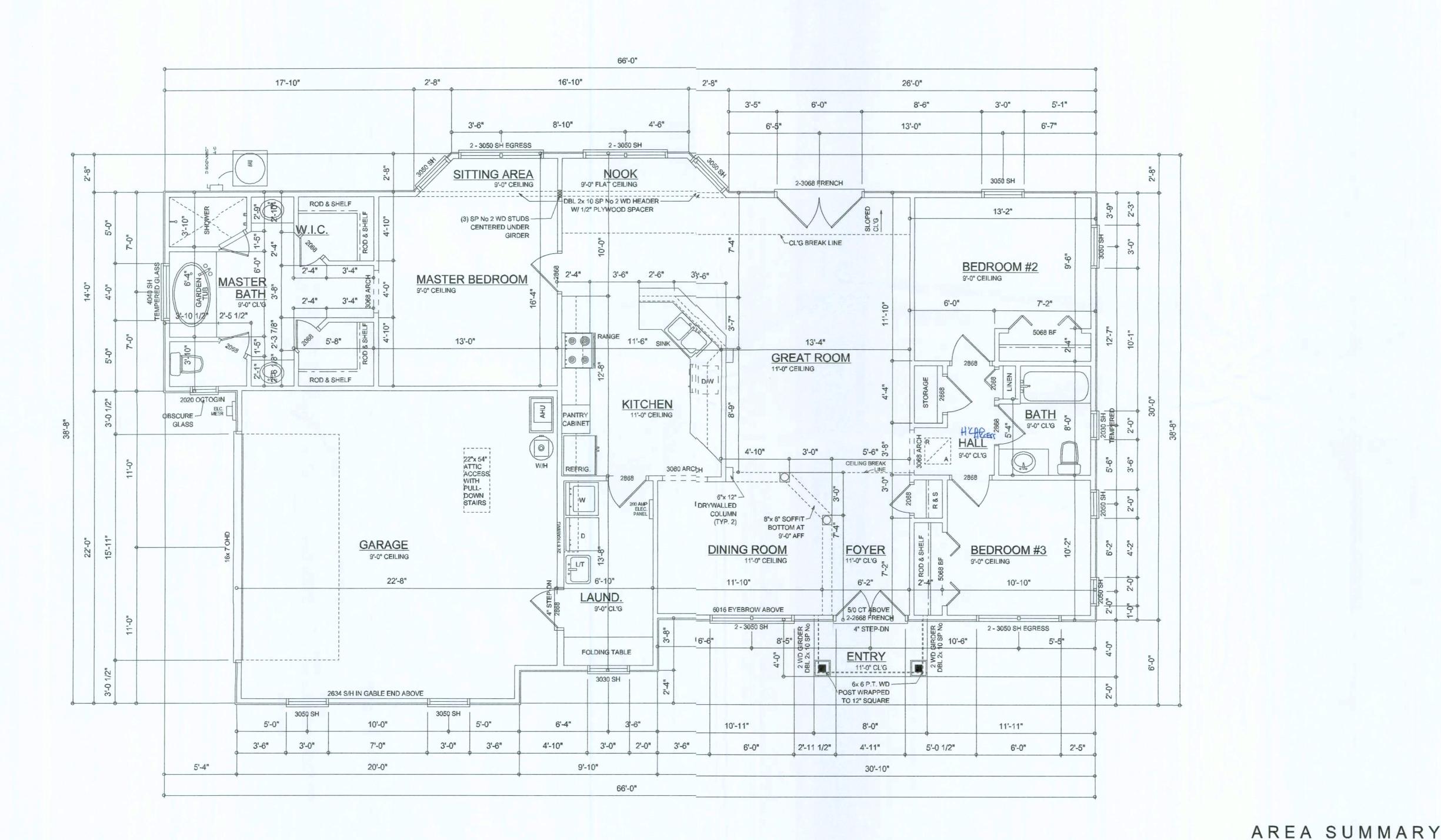
GARAGE AREA

TOTAL AREA

**ENTRY PORCH AREA** 

SHEET NUMBER

A.2
OF 3 SHEETS



Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped witheither solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or loors in compliance with Section 715.3.3. Openings from a private garage directly into a room used forsleeping purposes shall not be permitted.

- 2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel aind shall have no openings into the garage.
- 3. A separatition is not required between a Group R-3 and U carport provided the carport is entirely open, on two or more sides and there are not enclosed areas above.
- 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

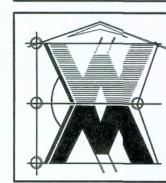
ELECTRICAL PLAN



NATHAN PETERSON CONSTRUCTIO A NEW SPEC HOUSE FOR:

A DEN PROJECT ADDRESS: HWY 47, COLU

ONLLIAM MYERS **DE.54CN**P.O. BOX 1513
LAKE CITY, FL 3:056 (386) 758-8406 will@willmyers.net



JOB NUMBER 061204

SHEET NUMBER **A.**3

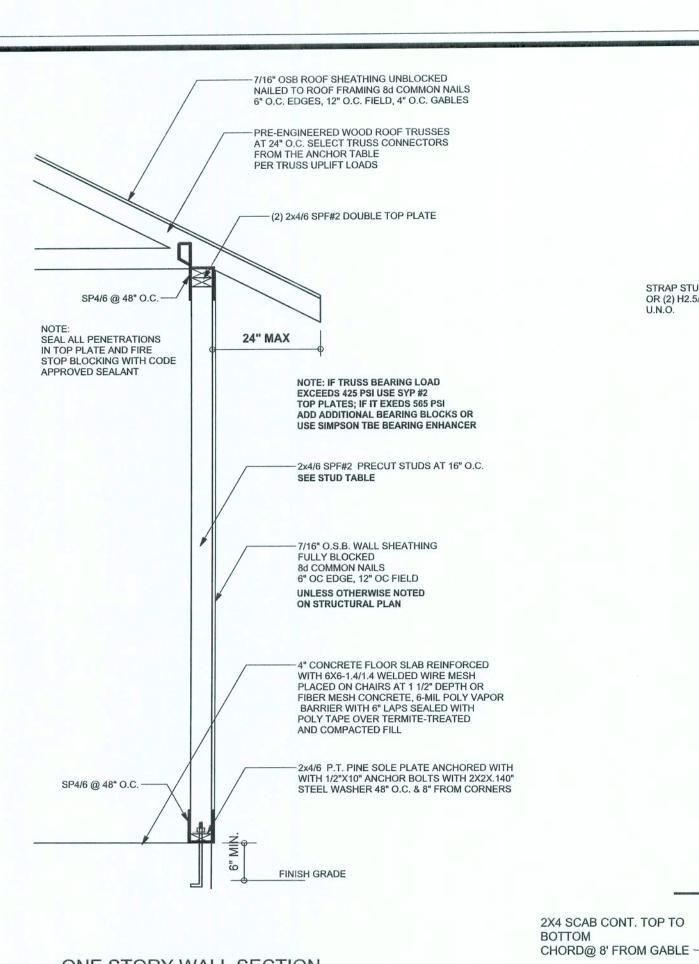
ATTIC
ACCESS
WITH
PULLDOWN
STAIRS BRACE AND PREWIRE FOR CL'GFAN BY OTHERS RECEPT IN CL'G FOR GARAGE DOOR OPENER

	ELECTRICAL LEGEND		
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)		
QD	DOUBLE SECURITY LIGHT		
0	RECESSED CAN LIGHT		
₩	BATH EXHAUST FAN		
	LIGHT FIXTURE		
Ф	DUPLEX OUTLET		
<b>\bar{\bar{\bar{\bar{\bar{\bar{\bar{</b>	220v OUTLET		
Вагі	GFI DUPLEX OUTLET		
† <b>V</b>	TELEVISION JACK		
PH	TELEPHONE JACK		
•	SMOKE DETECTOR (see note below)		
\$	WALL SWITCH		
\$3	3 WAY WALL SWITCH		
₩P/GFI	WATER PROOF GFI OUTLET		
48" FLOUR.			

NOTE: ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

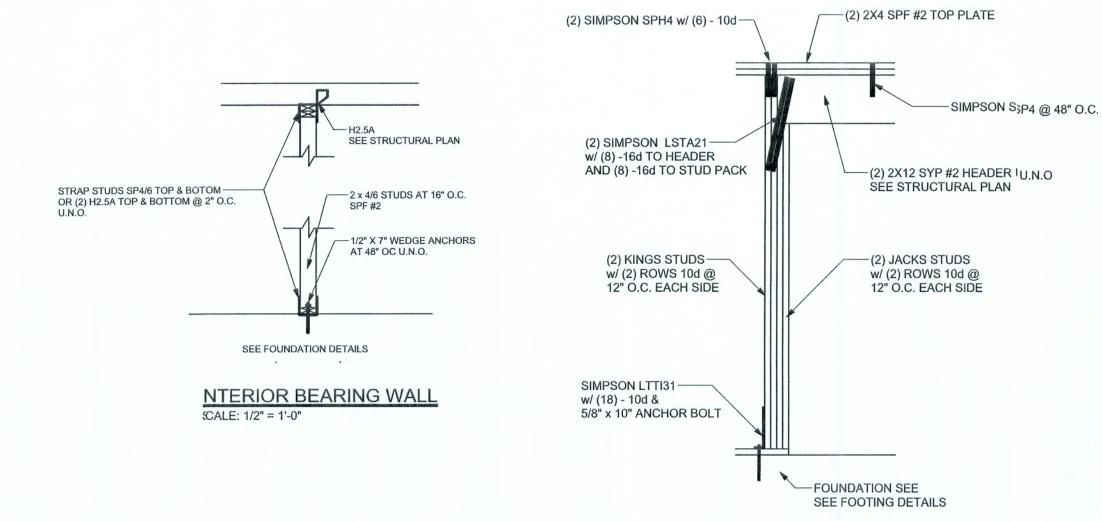
THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.



### **EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



7/16 STRUCTURAL ROOF SHEATHING -

2X4 X-BRACE @ 6'-0" OC. -

TYPICAL GABLE END ( X-BRACING )

SUPPORTIVE -

ALL MEMBERS SHALL BE SYP

TYPICAL GARAGE DOOR HEADER STRAPING DIETAIL SCALE: 1/2" = 1'-0"

PLATE w/ 16d COM @8" OC.

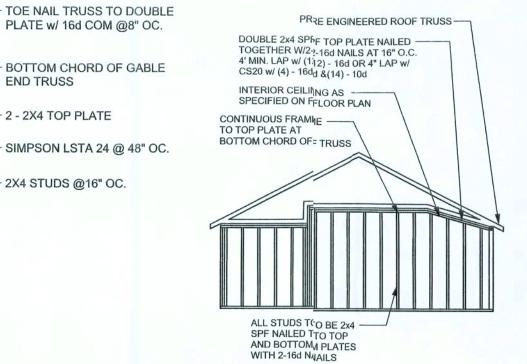
BOTTOM CHORD OF GABLE

**END TRUSS** 

- 2 - 2X4 TOP PLATE

2X4 STUDS @16" OC.

### 2X4DUTRIGGER @ 48" OC. -HURRICANE CLIP H-2.5 OR EQUAL Fb (psi) E (10<sup>6</sup> psi) BLCKING REQUIRED BETWEEN OUTRIGGERS -SYP #2 1200 2X4 BARGE RAFTER CONT. (3) .31 X 3 1/4 " GUN NAILS -SYP #2 1050 - SHINGLE STRIP 2X43LOCKING @ SHEATHING JOINT SYP #2 975 4' FIOM GABLE END -- FASCIA 24F-V3 SP 2400 TIMBERSTRAND | 1700 TOP CHORD OF GABLE END TRUSS MICROLAM 1600 - DROP 3 1/2" PARALAM 2900 CONT. 2X4 SCAB FROM TOP TO BOTTOM CHORD @ X-BRACING (PROVIDE ADDITIONAL 2X4'S @ VERTICAL IF HIGHER THAN 48". TO FORM AN "L" SHAPE.)



**GRADE & SPECIES TABLE** 

1.6

1.6

1.6

1.8

1.7

1.9

2.0

### CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

CONFIRM SITE COI BACKFILL HEIGHT,	IDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIA REQUIREMENTS F	LS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 OR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLAN	IUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL IGINEER IMMEDIATELY.
DESIGN, PLACEME	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS NT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL NS.

### **ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

**MASONRY NOTES:** 

Mortar

Grout

CMU standard

Clay brick standard

ACI530.1-02 Section

IN WRITING.

1.4A

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

Specific Requirements

5.5"x2.75"x11.5"

or 304SS

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

medium surface finish, 8"x8"x16" running

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

## **DESIGN DATA**

**ANCHOR TABLE** 

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

MANUFACTURER'S ENGINEERING

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR\*

H5

H2.5

H2.5A

H8

H14-1

H14-2

H10-1

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

**HEAVY GIRDER TIEDOWN** 

HGT-2

HGT-3

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

SP4

SP6

LSTA18

LSTA21

CS20

STUD ANCHORS

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU66

DSP SINGLE SILL PLATE

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

1-10d

6-10d

2-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

3-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

TO STUDS

TO FOUNDATION

1-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4-10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

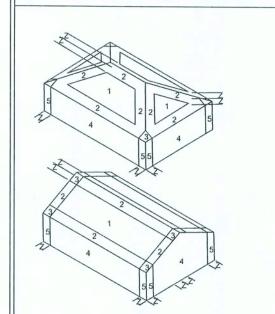
5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS MEAN ROOF HEIGHT NOT EXCEEDING LEAS ON UPPER HALF OF HILL OR ESCARPMENT SLOPE AND UNOBSTRUCTED UPWIND FOR	T HORIZONTAL DIMENSION OR 60 FT: NOT
BUILDING IS NOT IN THE HIGH VELOCITY HU	RRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBF	RIS REGION
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = B	
3.) WIND IMPORTANCE FACTOR = 1.0	7% I
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N	I/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN	WIND PRESSURES (TABLE R301.2(2))
Z. A	Zone Effective Wind Area (ft2)
	1 19.9 -21.8 18.1 -18.1



NOT IN FLOOD ZONE (BUILDER TO VERIFY)

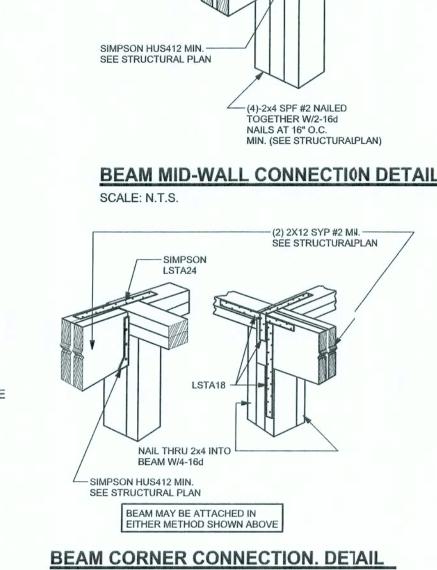
REVISIONS

SOFTPIAN

SIMPSON H2.5A U.N.O. SEE STRUCTURAL PLAN (2) 2X10 SYP #2 U.N.O. SEE STRUCTURAL PLAN (2) SIMPSON LSTA21 w/ (8) -16d TO HEADER AND (8) -16d TO POST -6X6 SYP #2 POST -SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL



4 - 10d NAILS OR 4 - .131"x 3.25"

2X4 SCAB IF VERT. WEB IS

CONT. 2X4X8' #2 SYP LATERAL

2X4 BLOCKING @ 48" OC.

BETWEEN GABLE AND FIRST

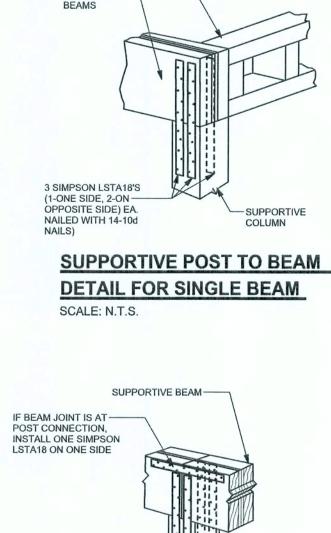
NOT PRESENT -

BRACE @ 48" OC. -

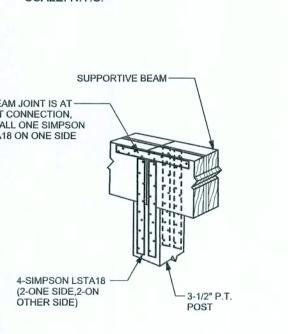
(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

SCALE: N.T.S.

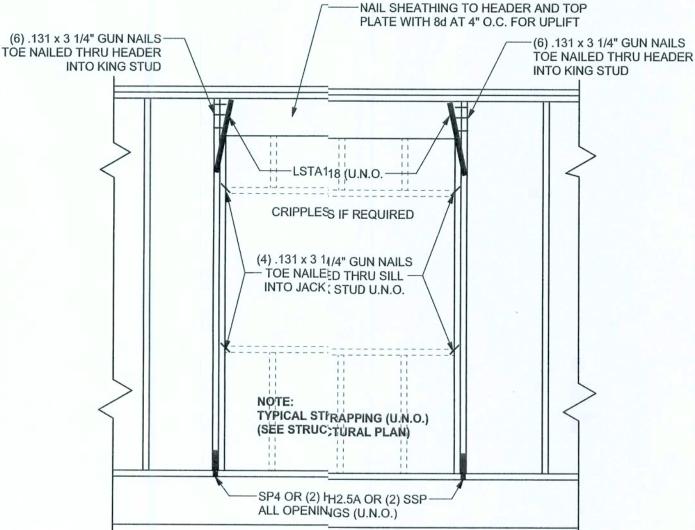
TYPICAL AT ALL CONNECTIONS L



2X4 LADDER BEAM







Reinforcing bars, #3 - #11 Coating for corrosion protection Coating for corrosion protection | Joint reinforcement in walls exposed to (1) 2X6 SPF #2 SIL\_L UP TO 11'-0" U.N.O. 3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings (1) 2X4 SPF #2 SILLL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-L0" WALL HIGHT U.N.O.) 3.3.E.7 Movement joints TYPICAL HEADER STRAPING DETAIL

WIND LOADS PER FLORIDA BUILDING CODE 2004 RES	CIDENTIA	ı er	CTIO	N Dag	14.0.4	
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FI MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZO ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN E SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIG	LAT, HIPI ONTAL D	PED, O	OR G	ABLE OR 6	ROOF 0 FT; N	OT %
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE						
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGIO	ON					
1.) BASIC WIND SPEED = 110 MPH						
2.) WIND EXPOSURE = B						
3.) WIND IMPORTANCE FACTOR = 1.0						
4.) BUILDING CATEGORY = II						
5.) ROOF ANGLE = 10-45 DEGREES						
6.) MEAN ROOF HEIGHT = <30 FT						
	0055	1 III 75 -	110;			
, , , , , , , , , , , , , , , , , , , ,			,		0.4=1-	
8.) COMPONENTS AND CLADDING DESIGN WIND PR	KESSURE	-S (T/	ABLE	R301	.2(2))	
7	Zone	Effect	ive W	ind Ar	ea (ft2)	
Ž.			0		100	
	1	_	-21.8	-	-18.1	
	2	19.9		18.1	-21.8	
5 2 2 2	2 O'hg	19.9	-40.6 -25.5	18 1	-40.6 -21.8	
2 5	3 O'hg	13.5	-68.3	10.1	-42.4	
3 4	4	21.8	-23.6	18.5	-20.4	
55	5	21.8	-29.1	18.5	-22.6	
124	Doors 8	& Wind	lows	21.8	-29.1	
[2]	1774 177	st Case				
	(Zone					
19 2 3	8x7 Gara			19.5	-22.9	
4 2 4 5	16x7 Ga	rage D	oor	18.5	-21.0	
222						
					,	
DESIGN LOADS						
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)						
30 PSF (SLEEPING ROOMS)						
30 PSF (ATTICS WITH STORAGE)						
10 PSF (ATTICS WITHOUT STORAGE, <3:12)						
ROOF 20 PSF (FLAT OR <4:12)						
16 PSF (4:12 TO <12:12)						
12 PSF (12:12 AND GREATER)						
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)						
SOIL BEARING CAPACITY 1000PSF						
NOT IN ELOOD ZONE (BLIII DER TO VERIEV)						

imensions. Refer all questions to Mark Disosway, P.E. for resoution to not proceed without clarification COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly rese s common law copyrights aid property right i ese instruments of service. This document i not to be reproduced, alteredor copied in any form or manner without first tie express writt rmission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have amined this plan, and that he applicable ortions of the plan, relating b wind engine comply with section R301.2.1 florida building code residential 2004, to the jest of my LIMITATION: This design is valid for one uilding, at specified location MARK DISOSVAY P.E. 53915 Nathan Peterson Construction Spec House ADDRESS:

INDLOAD ENGINEER: Mak Disosway

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercedescaled

MENSIONS:

DRAWN BY: CHECKED BY: David Disosway FINALS DATE: 01 / Dec / 06 JOB NUMBER: 611213 DRAWING NUMBER

HWY 47

Columbia County, Florida

Mark Disosway P.E.

P.O. Box 868

Lake City, Florida 32056

Phone: (386) 754 - 5419

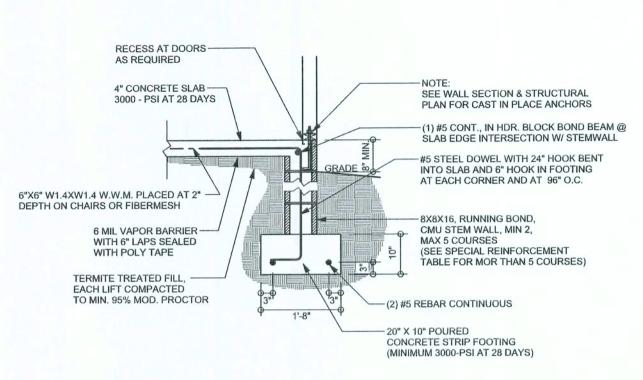
Fax: (386) 269 - 4871

PRINTED DATE:

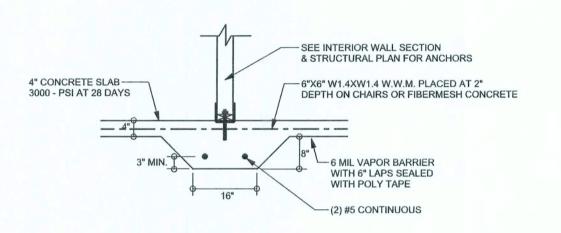
December 01,2006

OF 3 SHEETS

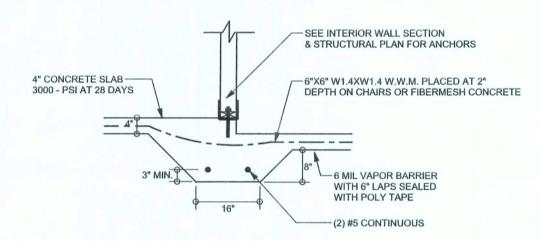
S-1



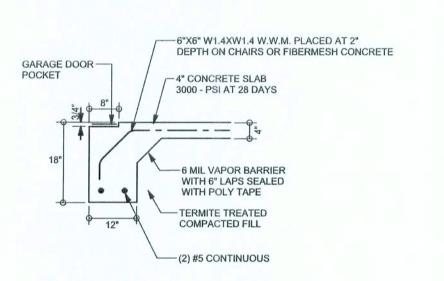
## F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



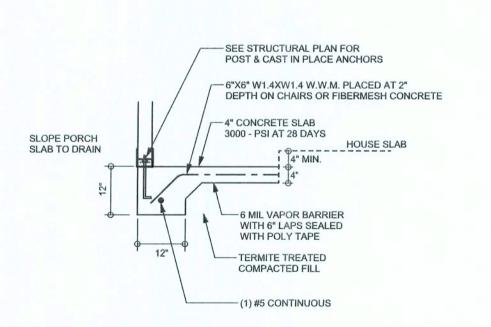
## F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



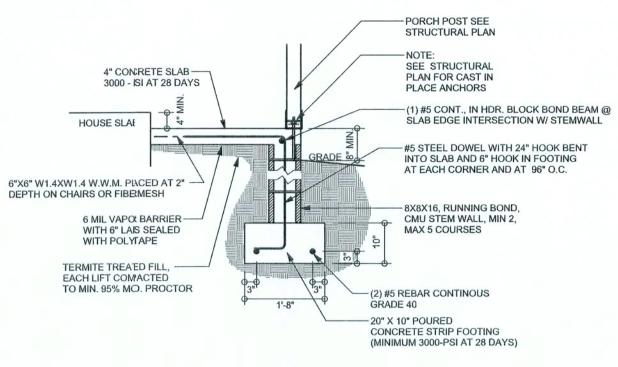
# F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



### F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

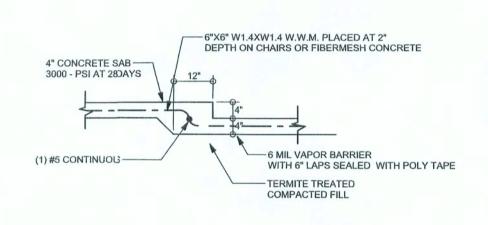


# F12 0PTIONAL STEM WALL PORCH FOOTING SCALE: 1/2" = 1'-0"

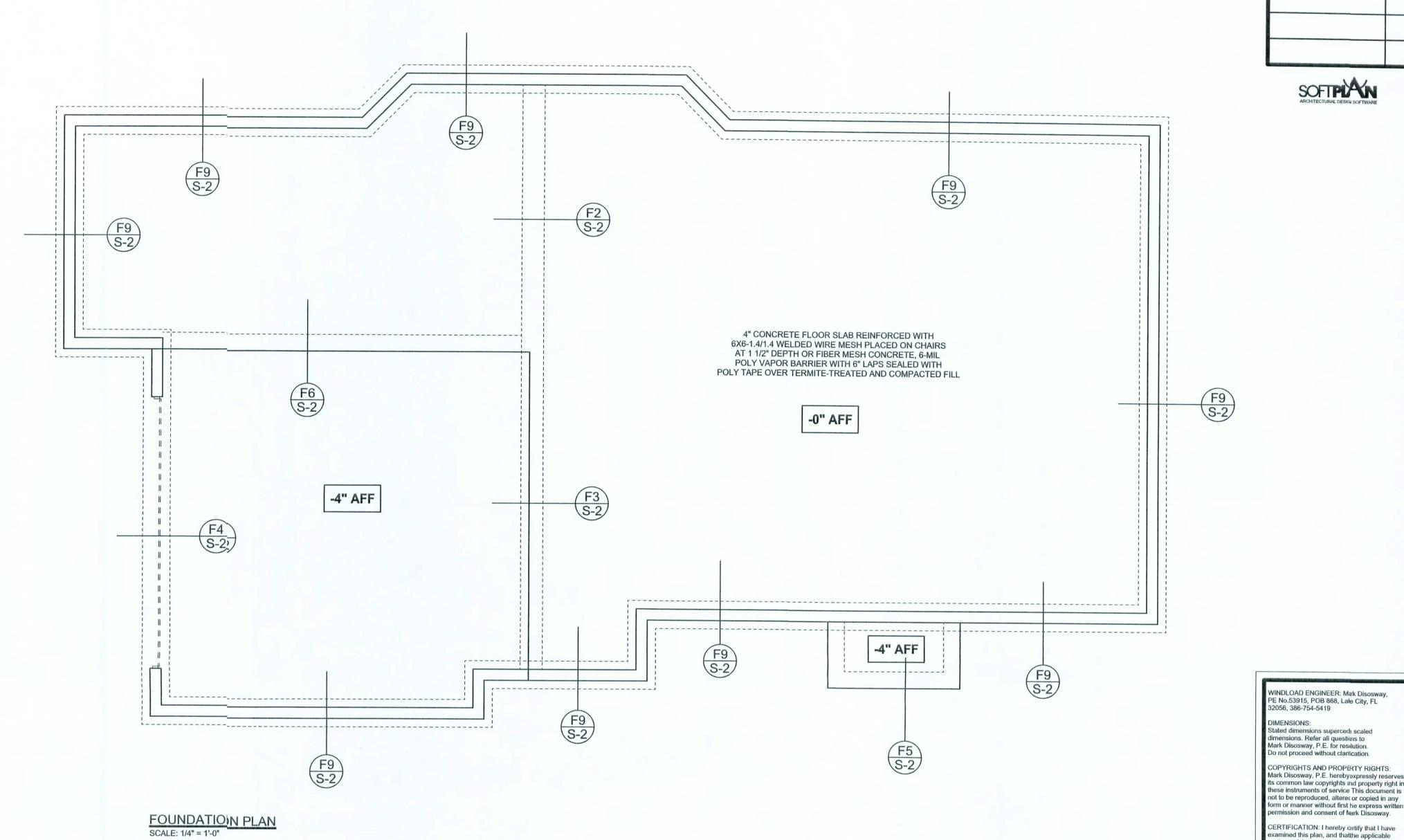
### TALL STEW WALL TABLE

The table assumes 60 ki reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the to. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durwall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as sbwn in the table below.

STEMWALL HEIGHT (FEET)	UNBALAICED BACKILL HEIGIT	ACKILL FOR 8" CMU STEMWALL FOR 12" CMU STEMWALL			MWALL		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: /2" = 1'-0"



comply with section R301.2.I, florida building code residential 2004, to thebest of my knowledge.

LIMITATION: This design is ralid for one building, at specified locatior.

MARK DISOSNAY
P.E. 5391i

portions of the plan, relating o wind engineeri

REVISIONS

Nathan Pelerson
Construction

Spec House

ADDRESS: HWY 47 Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

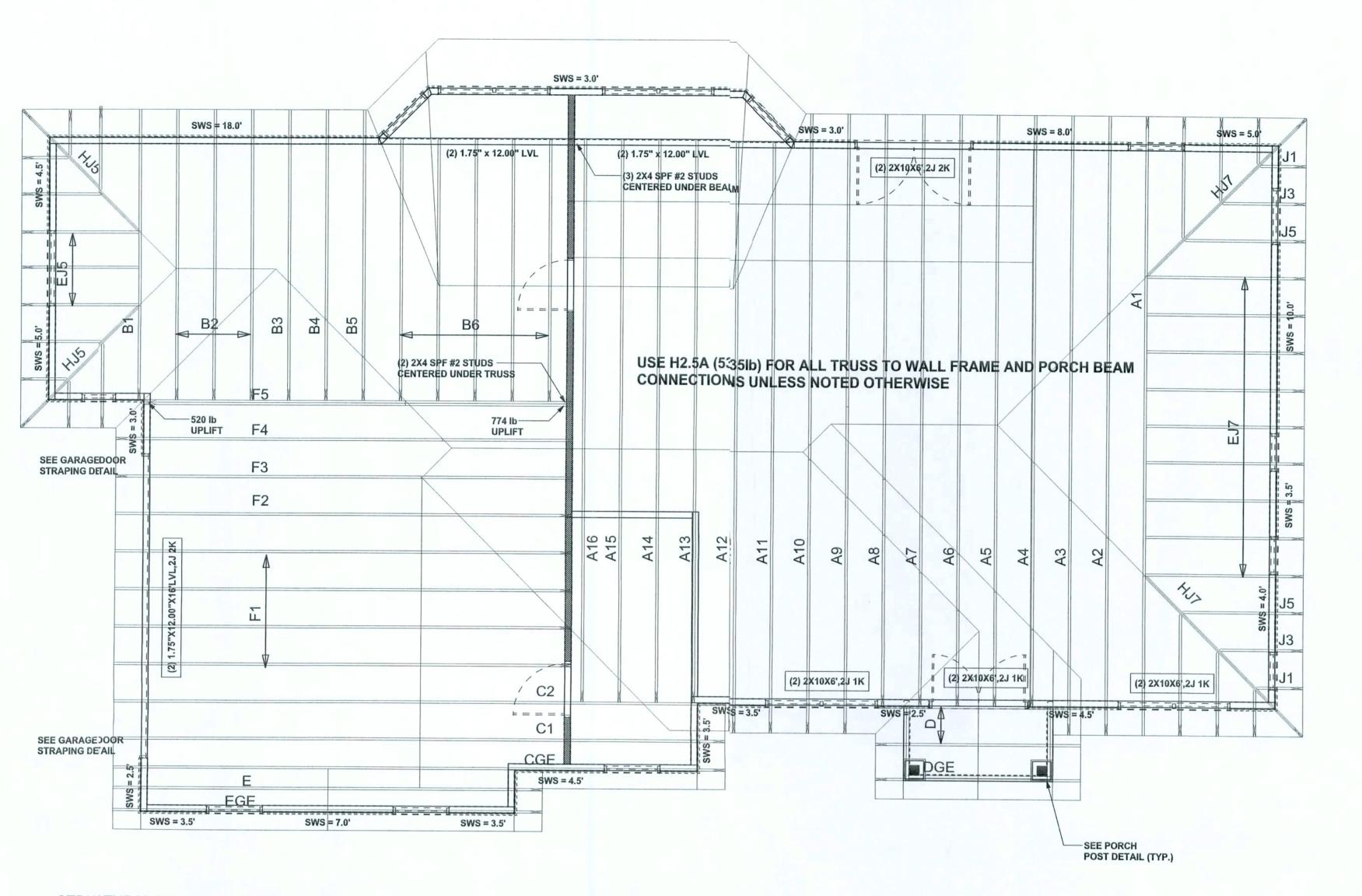
PRINTED D#TE:
December 01 2006

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 01 / Dec / 06

> JOB NUMBER: 611213 DRAWING NUNBER

> > S-2 OF 3 SHEE'S



STRUCTURAL PLAN
SCALE: 14" = 1'-0"

## STRUCTURAL PLAN NOTES

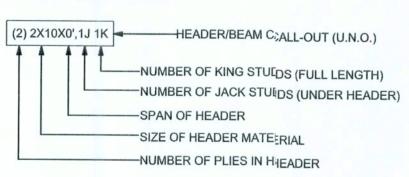
SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

### **HEADER LEGEND**



## TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS LONGITUDINAL 28.4' 66.0'

### WALL LEGEND

SMS =).0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.
SWS =).0*	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

O.O IIIDIO/IIL	O OTILITIES TO	ILL OLOW
	REQUIRED	ACTUAL
TRANSVERSE	32.5'	36.0'
LONGITUDINAL	20.41	00.01

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #6-403

SOFTPIAN ARCHITECTURAL DESIGNATION

**REVISIONS** 

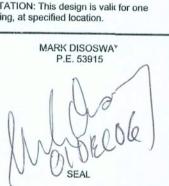
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake Cty, FL

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions o Mark Disosway, P.E. for resolution. Do not proceed without clarificaton.

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not to be reproduced, altered or copied in any
form or manner without first the express written
permission and consent of MarkDisosway. CERTIFICATION: I hereby certify that I have examined this plan, and that theapplicable

portions of the plan, relating to wnd engineering comply with section R301.2.1, flirida building code residential 2004, to the bes of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Nathan Peterson Construction

Spec House

ADDRESS: **HWY 47** Columbia County, Horida

Mark DisoswayP.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754- 5419 Fax: (386) 269 -4871

PRINTED DATE: December 01, 2006 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 01 / Dec / 06

> JOB NUMBER: 611213 DRAWING NUMBER

**S-3** OF 3 SHEETS