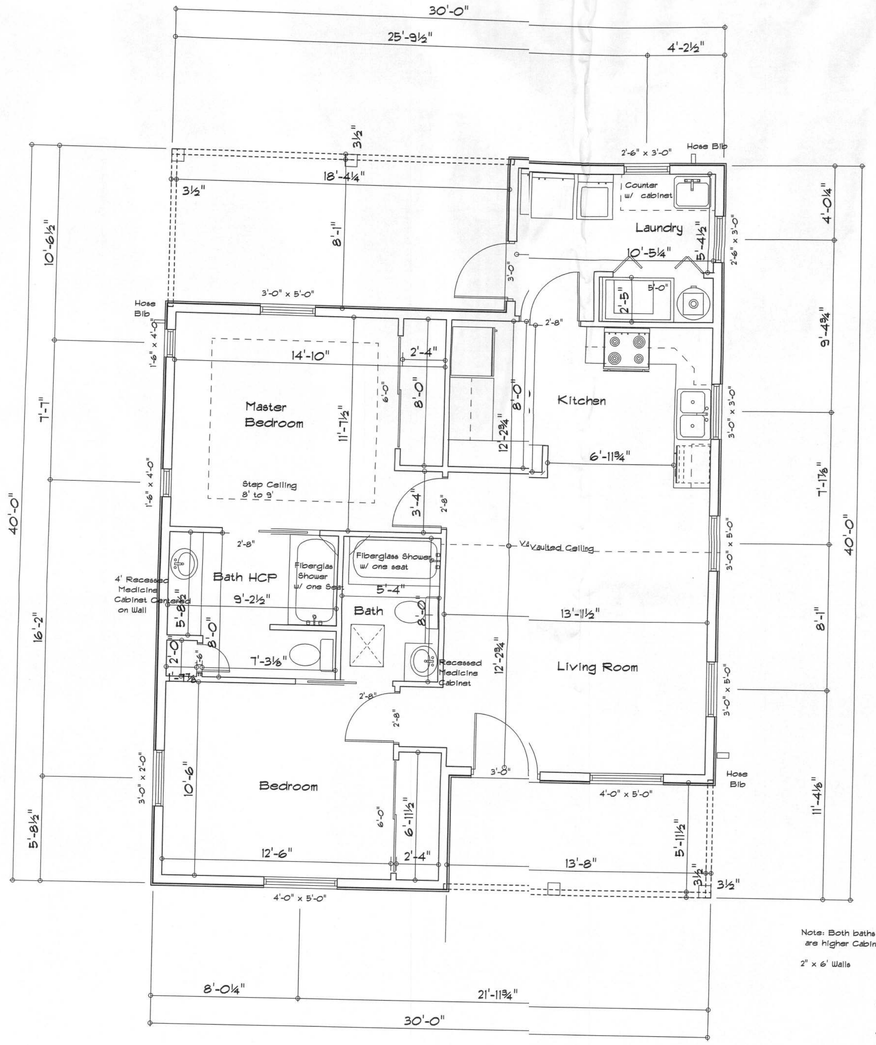
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



Floor Plan
Scale 1/4" = 1'

AREA SUMMERY

Living Area	949	S.F.
Back Porch	165	S.F.
Front Porch	86	S.F.
Total Area	1200	S.F.

RESIDENCE

Healy
385 NE Poggs Glen
Lake City FL 32055

ADDRISS:
Columbia County, Florida

Woodman ParkBuilders, Inc.
Lake City,Florida
Phone: (386)755 - 2411
Fax: (386) 755-8684
Emai:

PRINTED IATE:

DRAWN BY: | CHECKED BY:

DESIGNED BY:
Mark Haddox

FINALS DATE:

JOB NUNBER:

DRAWING NIMBER
A-1

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

Electrica Plan Notes:

E-1 Wire l appliances, HVAC units and other equiptment per inufactures specifications.

E-2 Consit the owner for the number or seperate telepons lines to be installed. Owner is responsible for all overages not noted on plan.

E-3 All itallations shall be per national code 2008.

E-4 All oke detectors shall be 120V with battery backup of the photoelectric type, and shall be intercked together. Install inside and near all bedioms.

E-5 Telehons, television and other low voltage devies or outlets shall be as per the owners direcions and in accordance with applicable sectins of the National Electric Codes latest editin. Owner is responsible for all overages not ted on plan.

E-6 Elecrical contractor shall be responsible for the esign and sizing of electrical service and circts.

E-7 Entrof service (underground or overhead) to to b determined by contractor agreement.

E-8 All otlets located in residential to be tamer-resistant per NEC.

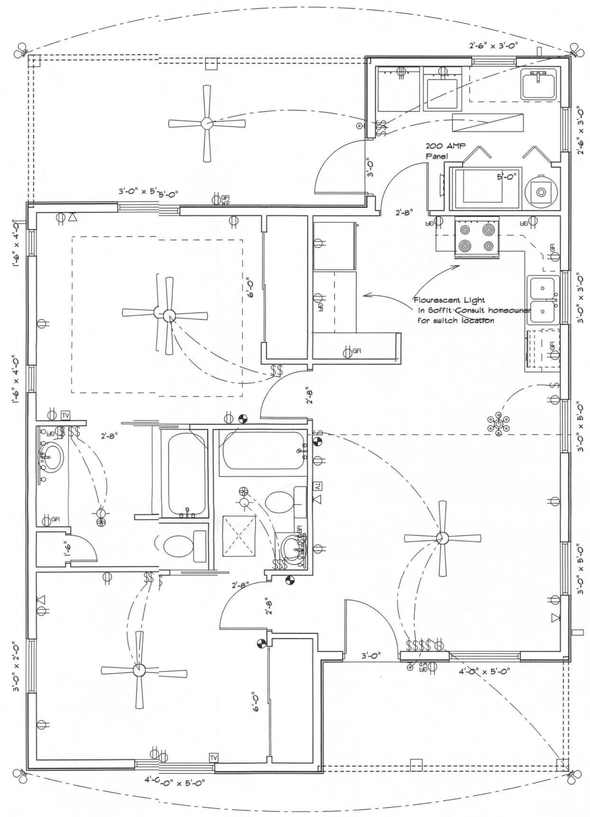
E-9 All otlets to be located above base flood elevtion.

E-10 All xterior GFI outlets shall be weatherproof.

E-11 Overcurrent Protection devico shall be instaed on the exterior of structures on the ad side of the meter to serve as aisconnecting means. Conductors used from the xterior disconnecting means to a panel or sub anel shall have four-wire conductors, of whic one conductor shall be used as an equitment ground.

E-12 All 12-VOLT, single phase, 15 and 20 ampere branch circuits supplying outlets installed in dwelling unit imily rooms,dining rooms, living rooms, parlors, libraes, dens, bedrooms, sun rooms, recreation rooms, closes, hallways, or similar rooms or areas shall be protcted by a listed arc-fault circuit interrupter, combation-type installed to provide protection of the branc circuit.

E-13 Carbn Monoxide alarms shall be required within 10' of arooms for sleeping purposes in buildings having a foil-fuel burning heater or appliance, a fireplace or ached garage.



Electrical Plan

ELECTRICAL	SYMBOL
ceiling fan	
ceiling fan globe l	
chandelier	
double spotlight	
fluorescent fixture	
vanity bar light	
wall sconce	
electrical panel	
Outlet WP GFI	
cable tv outlet	
fan	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	
telephone	

RESIDENCE

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Lake City, FL 32055

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Email:

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DRAWN BY: | CHECKED BY:

DESIGNED BY: Mark Haddox

FINALS DATE:

JOB NUMBER:

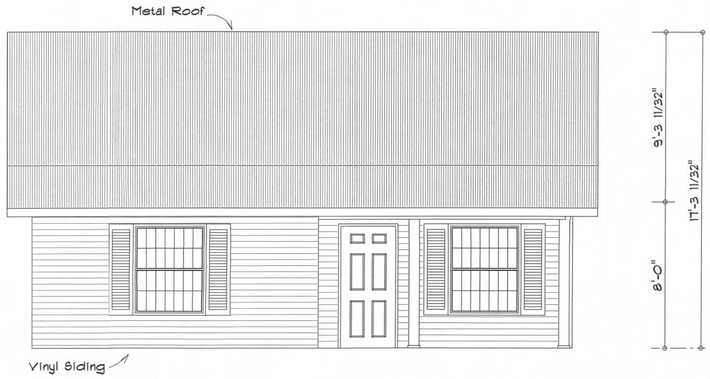
DRAWING NUMBER
A-2

Notes:

R-1 All roof pitches shall be 6/12 unless otherwise noted.

R-2 All overhangs shall be 24" except on gables 18".

R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).



Front Elevation

Rear Elevation

Left Elevation

REVISONS

SOFTPLAN ARCHITECTRAL DESIGN SOFTWARE

RESDENCE

Heal
385 IE Froggs Glen
LakeCity, FL 32055

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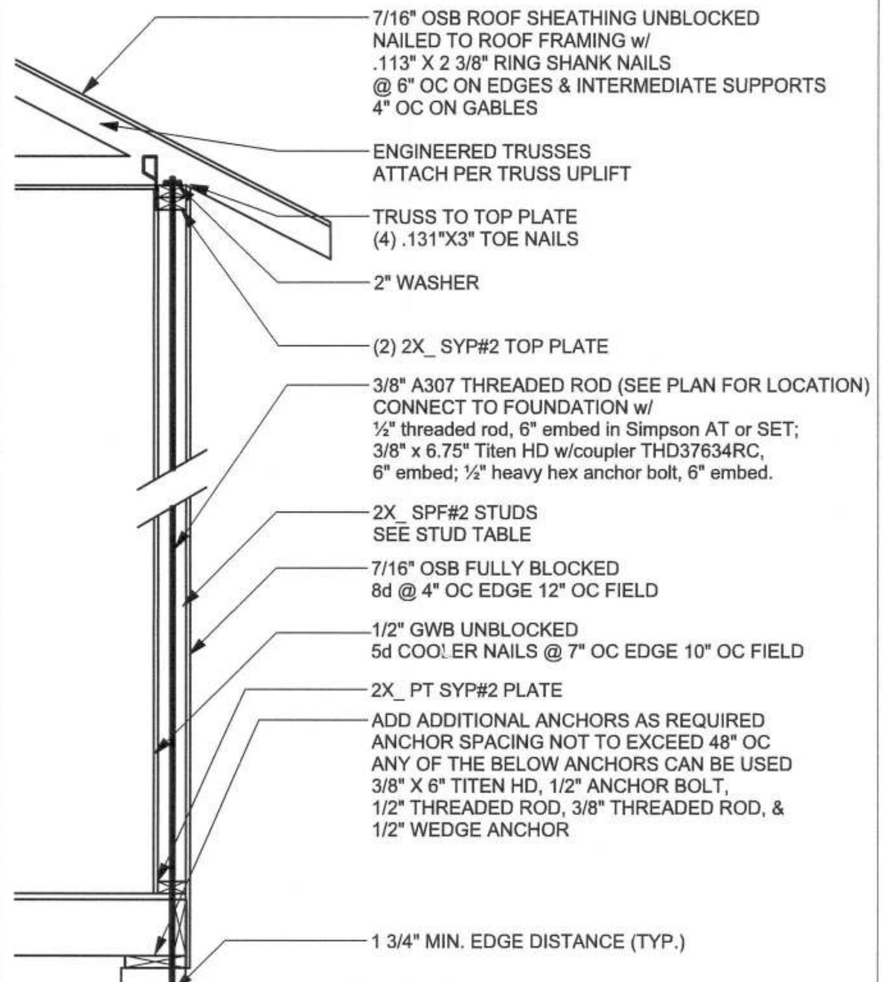
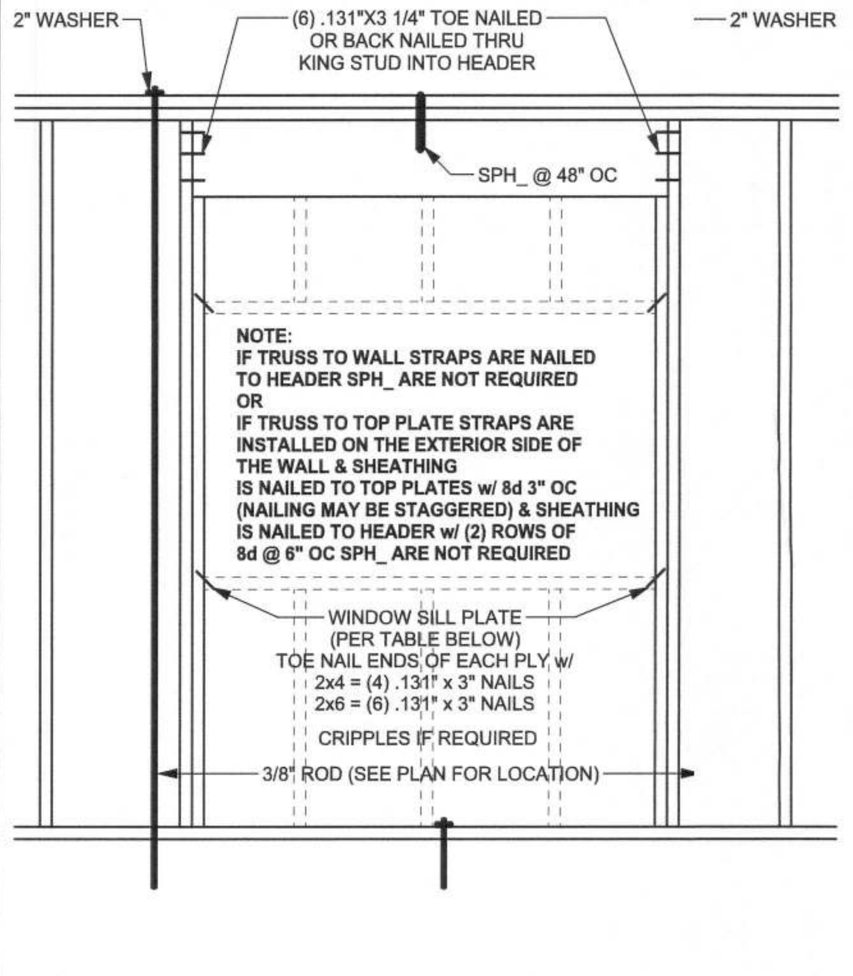
FINALS DATE:

JOB NUMBER:

DRAWNG NUMBER
A-3

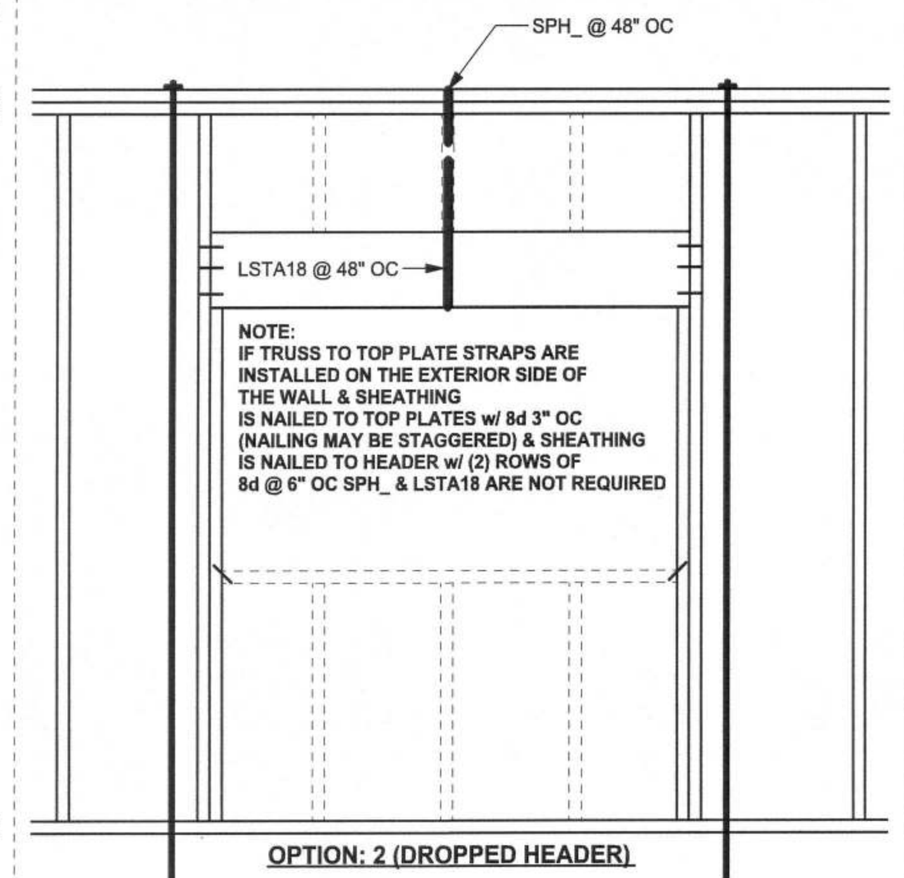


(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ RODS



(TYP.) HEADER
ONE STORY WOOD FRAME w/ RODS

OPTION: 1 (FLUSHEADER)



OPTION: 2 (DROPPED HEADER)

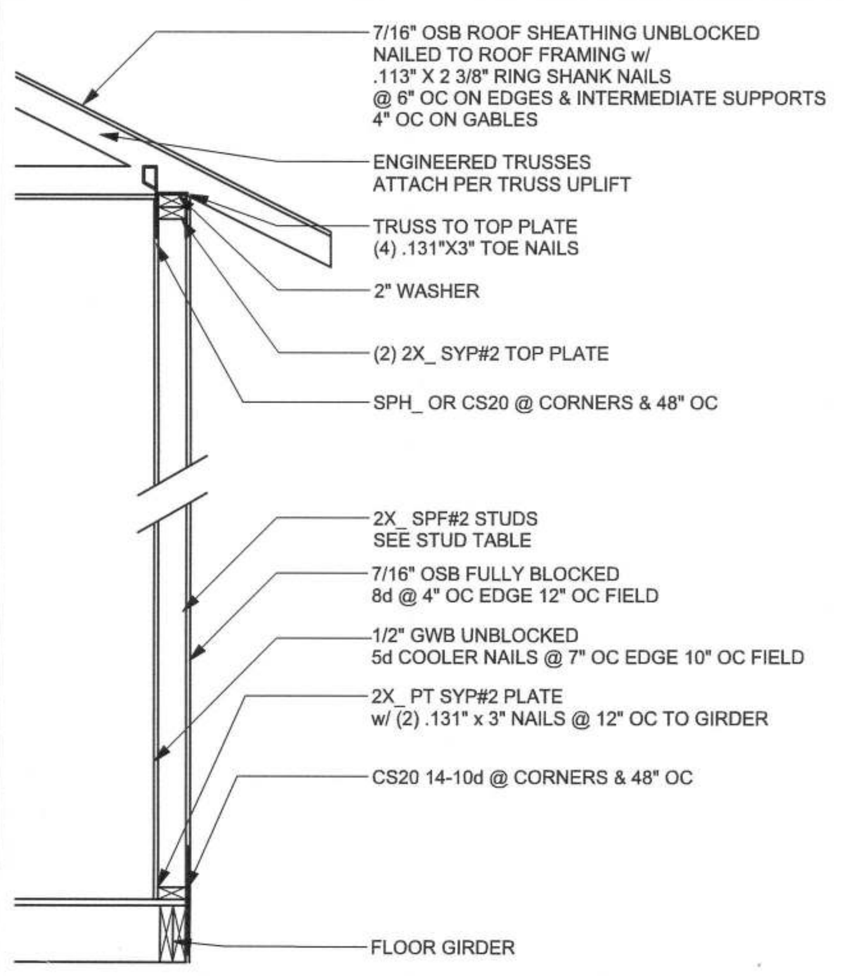
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.23B FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
90-100 MPH	5'-3"	7'-9"	7'-8"	11'-4"	
110-120 MPH	4'-4"	6'-6"	6'-5"	9'-6"	
130 MPH	4'-0"	6'-0"	5'-11"	8'-9"	

ANCHOR TABLE

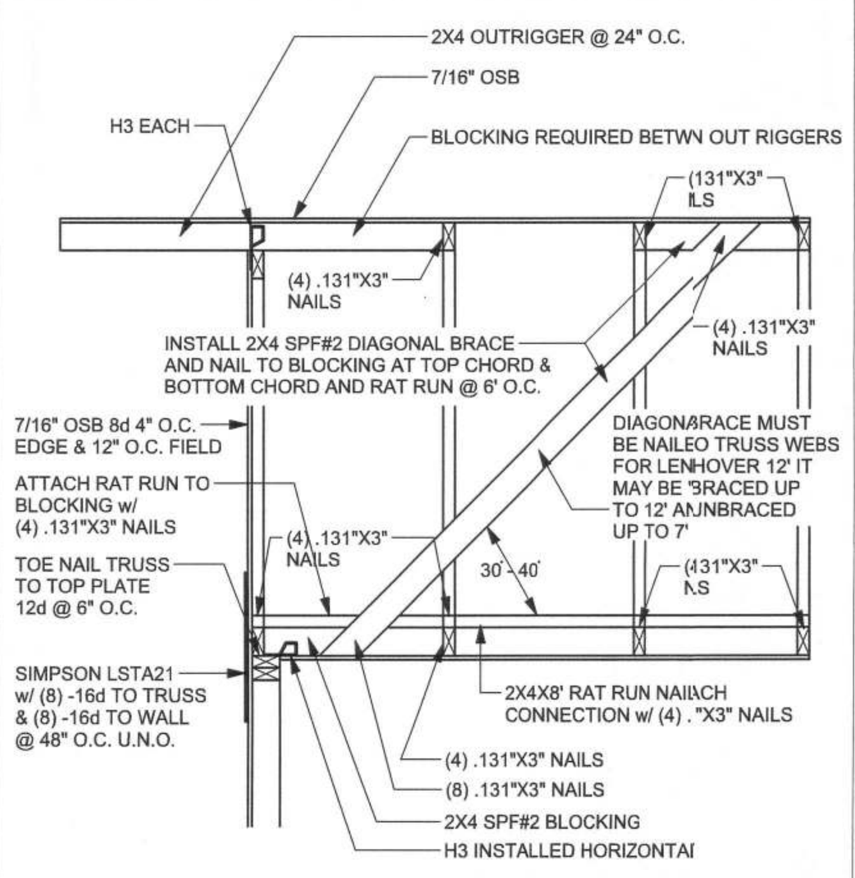
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES	
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"	
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"	
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"	
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"	
H6	950	820					8-8d	8-8d	
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"	
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d	
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d	
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"	
H10-2	760	655	455	395	390	340	6-10d	6-10d	
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"	
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"	
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"	
HEAVY GIRDER TIEDOWNS									**TO FOUNDATION**
LGT2	2050	1785	700	170	700	170	14-16d	14-16d	
LGT3-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS	
LGT4-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS	
MGT	3965	3330					22 -10d		5/8" ANCHOR
HGT-2	10980	6485					16 -10d		2-5/8" ANCHOR
HGT-3	10530	9035					16 -10d		2-5/8" ANCHOR
HGT-4	9250	9250					16 -10d		2-5/8" ANCHOR
STUD STRAP CONNECTOR									**TO STUDS**
SSP DOUBLE TOP PLATE	435	435						3-10d	4 -10d
SSP SINGLE SILL PLATE	455	420						1-10d	4 -10d
DSP DOUBLE TOP PLATE	825	825						6 -10d	8 -10d
DSP SINGLE SILL PLATE	825	600						2 -10d	8 -10d
SP1	585	535						4 -10d	6 -10d
SP2	1065	605						6 -10d	6 -10d
SP4	885	760							6-10d x 1 1/2"
SPH4	1240	1065							10-10d x 1 1/2"
SP6	885	760							6-10d x 1 1/2"
SPH6	1240	1065							10-10d x 1 1/2"
LSTA18	1235	1110							14-10d
LSTA21	1235	1235							16-10d
CS20	1030	1030							14-10d
CS16	1705	1705							22-10d
STUD ANCHORS							**TO STUDS**		**TO FOUNDATION**
LTT19	1350	1305					8-16d		1/2" ANCHOR
LTTI31	2310	2310					18-10d x 1 1/2"		5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS		5/8" ANCHOR
HTT16	4175	3695					18-16d		5/8" ANCHOR
HTT22	5260	5250					32-16d		5/8" ANCHOR
ABU44	2200	2200					12-16d		5/8" ANCHOR
ABU66	2300	2300					12-16d		5/8" ANCHOR
ABU88	2320	2320					18-16d		2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL IL HOLES
(2) FOR SYP GIRDER & SPF STUDS

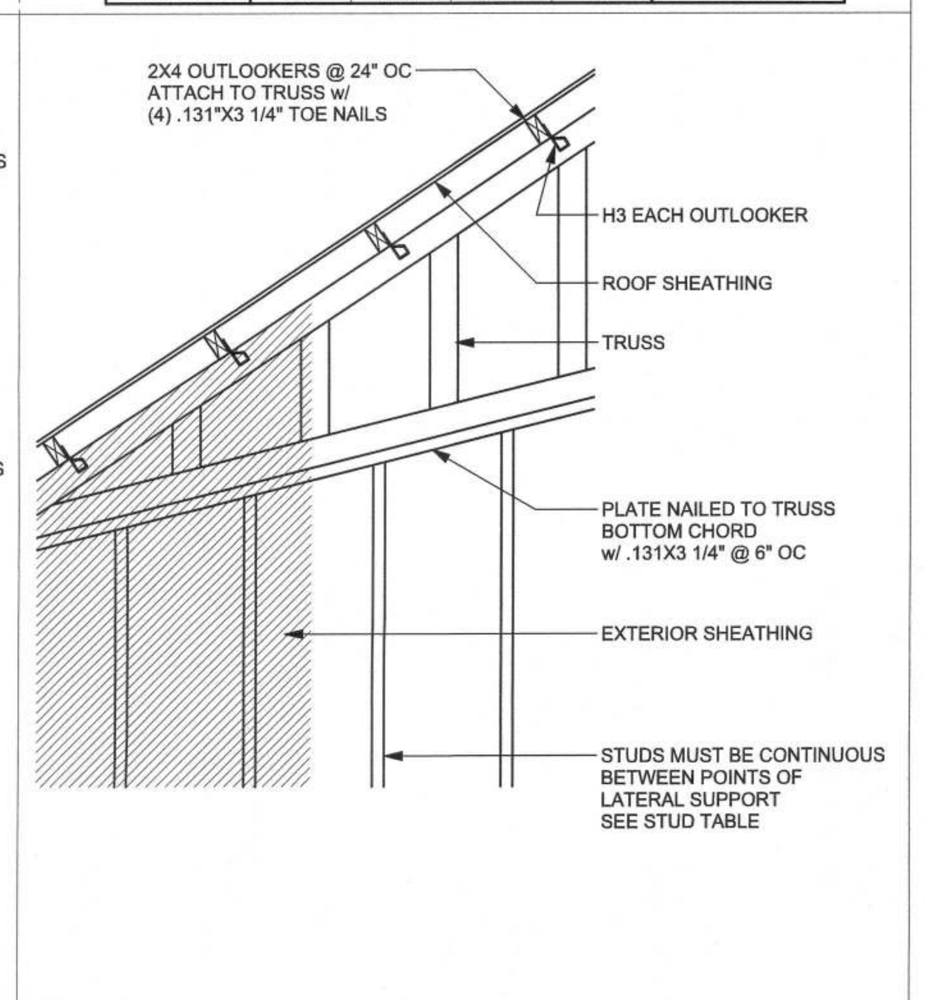


(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ STRAPS

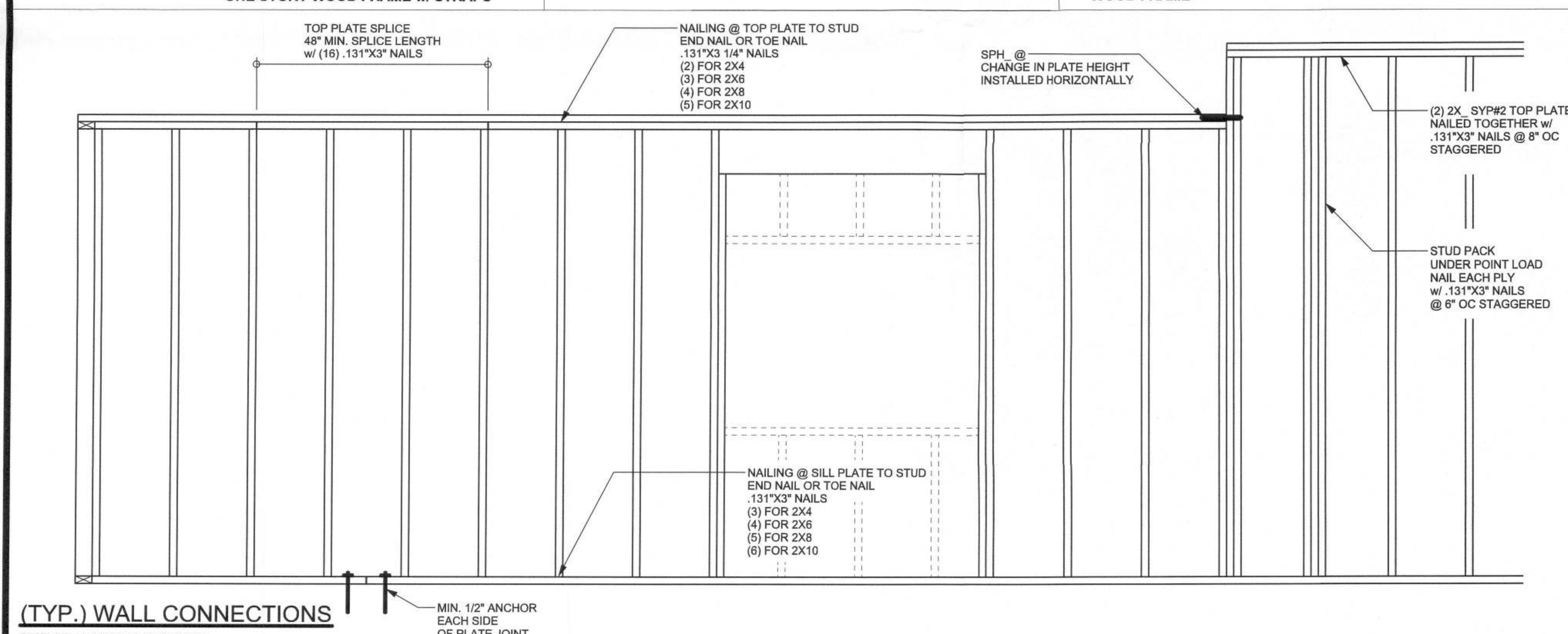


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL
WOOD FRAME



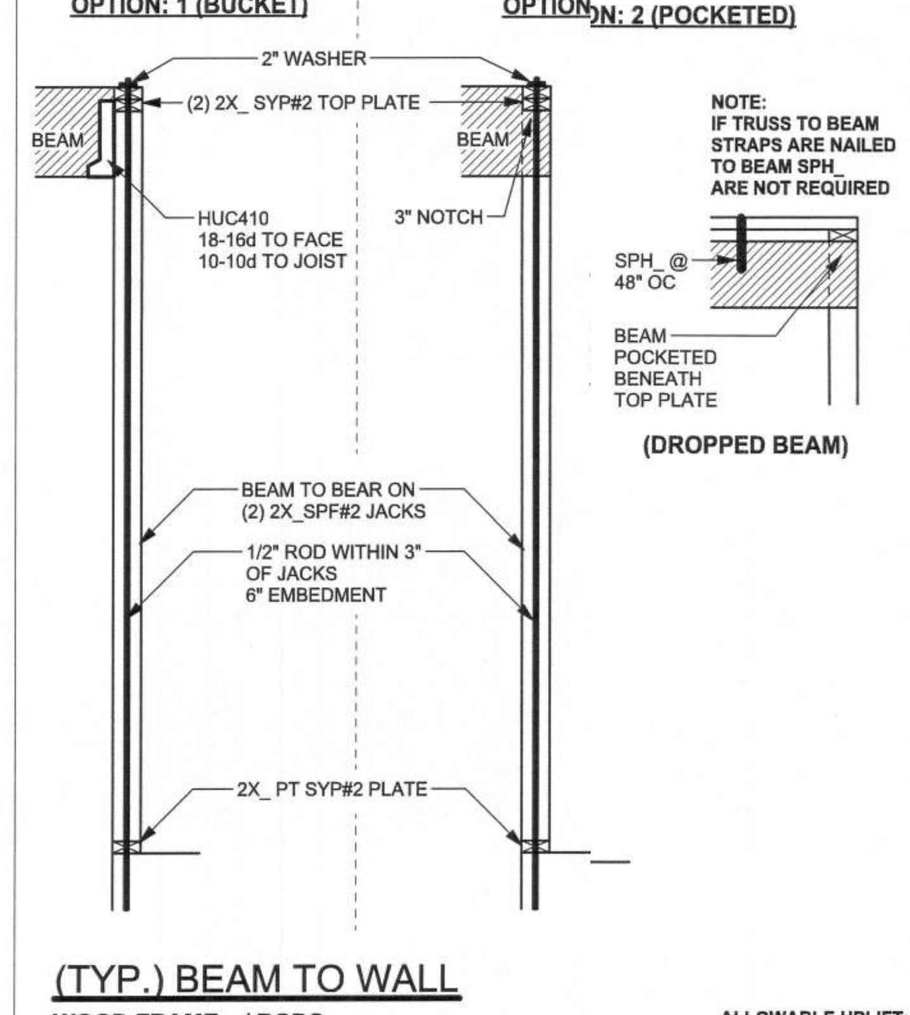
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



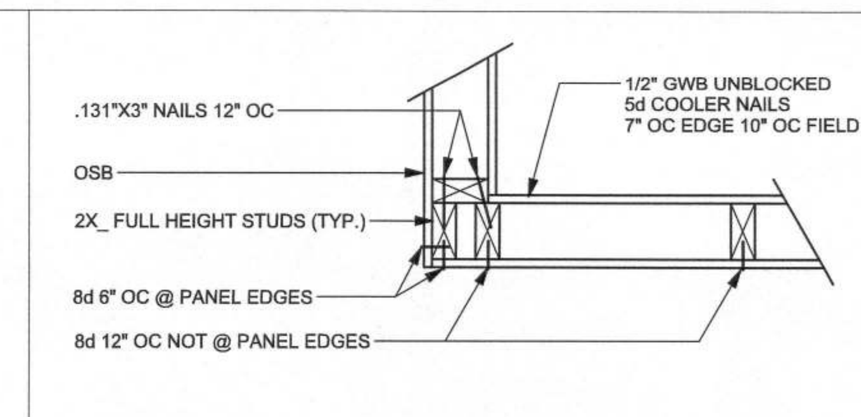
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

OPTION: 1 (BUCKET)

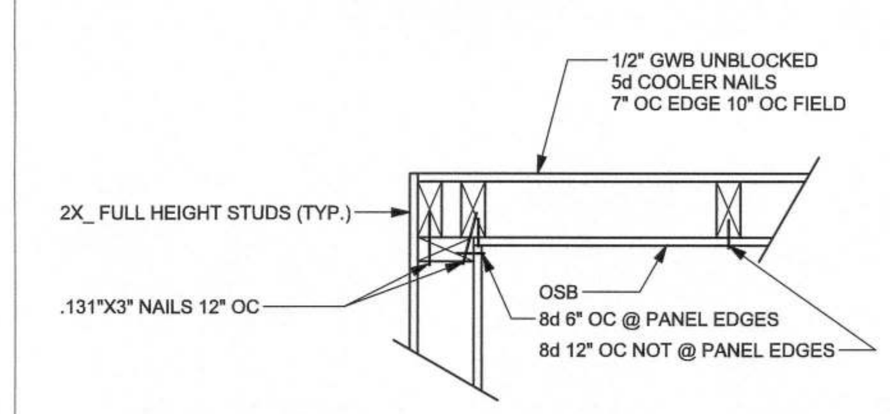
OPTION: 2 (POCKETED)



(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS

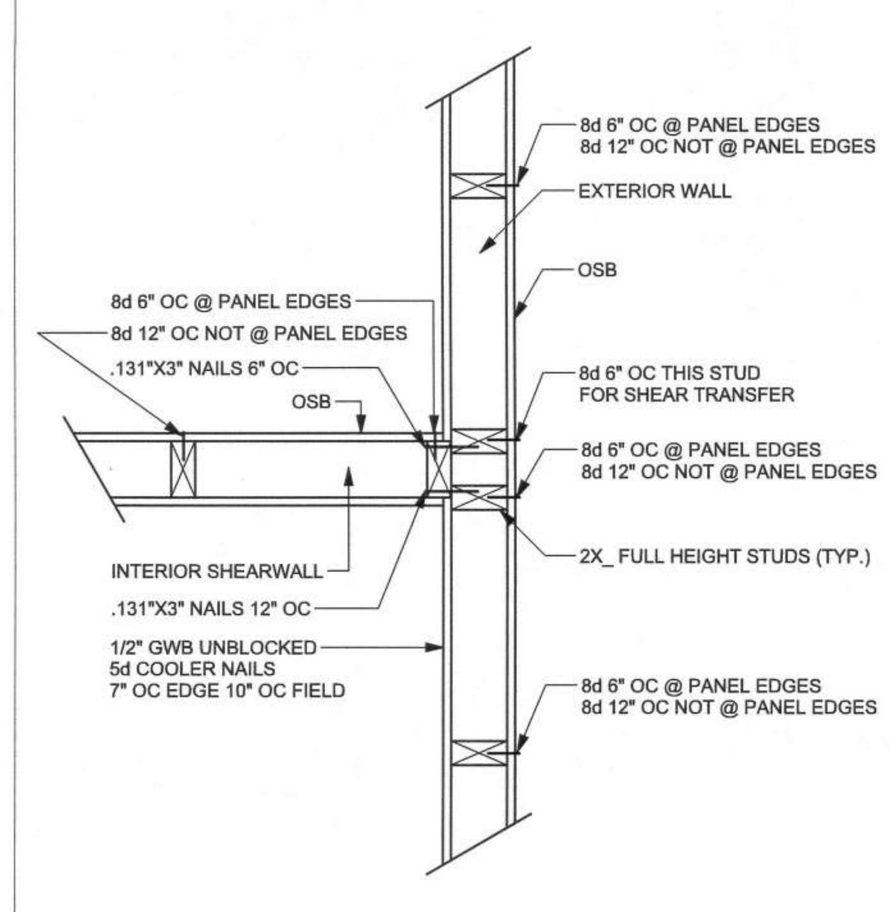


OUTSIDE CORNER



INSIDE CORNER

(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

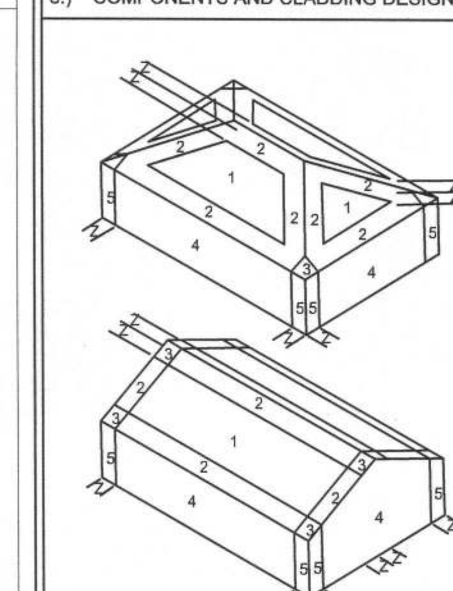
WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = C
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	27.8	-30.5	25.3	-25.3
2	27.8	-35.7	25.3	-30.5
2 O'hg		-56.8		-56.8
3	27.8	-35.7	25.3	-30.5
3 O'hg		-95.6		-59.3
4	30.5	-33.0	25.9	-28.5
5	30.5	-40.7	25.9	-31.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	30.5	-40.7
8x7 Garage Door	27.3	-32.0
16x7 Garage Door	25.9	-29.4

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER
Mark Disosway, PE
No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions superede scaled dimensions. Refer all qustions to Mark Disosway, P.E. for esolution. Do not proceed without carification.

CERTIFICATION: I herely certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering complywith section R301.2.1, florida buildingcode residential 2007, and 200 supplement to the best of my knowlege.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Woodman Park Builders

Healy Residence

ADDRESS:
385 NE Froggs Glen
Lake City, FL 32055

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386)754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: November 24, 2010

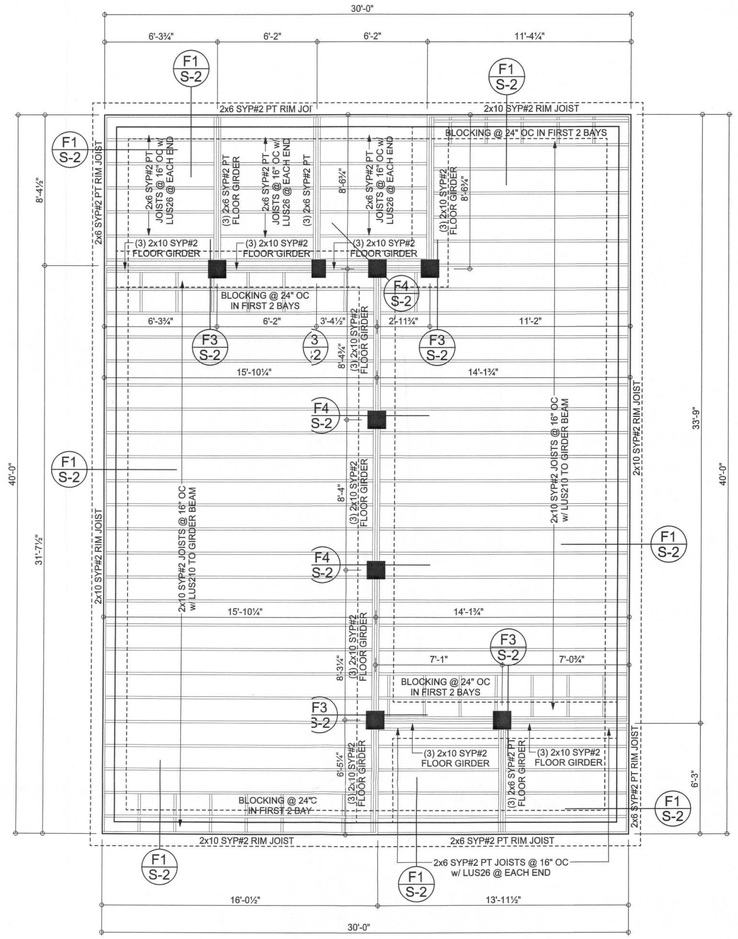
DRAWN BY: | STRUCTURAL BY: Evan Beamsley

FINALS DATE: 2010-11-24

JOB NUMBER: 1011078

DRAWING NUMBER **S-1**

OF 2 SHEETS

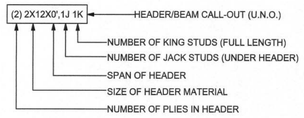


FOUNDATION & FLOOR FRAMIG PLAN

SCALE: 1/4" = 1'-0"

R408.3 Access.
Access shall be provided to all under-floor spaces. Access openings through the floor shall be a minimum of 18 inches by 24 inches (457 mm by 610 mm). Openings through a perimeter wall shall be 16 inches by 24 inches (407 mm by 610 mm). When any portion of the through wall access is below grade, an areaway of not less than 16 inches by 24 inches (407 mm by 610 mm) shall be provided. The bottom of the areaway shall be below the threshold of the access opening. Through wall access openings shall not be located under a door to the residence. See M1305.1.4 for access requirements where mechanical equipment is located under floors.

HEADER LEGEND



TOTAL SHEAR WALL TABLE

SWS = INDICATES SHEAR WALL SEGMENTS

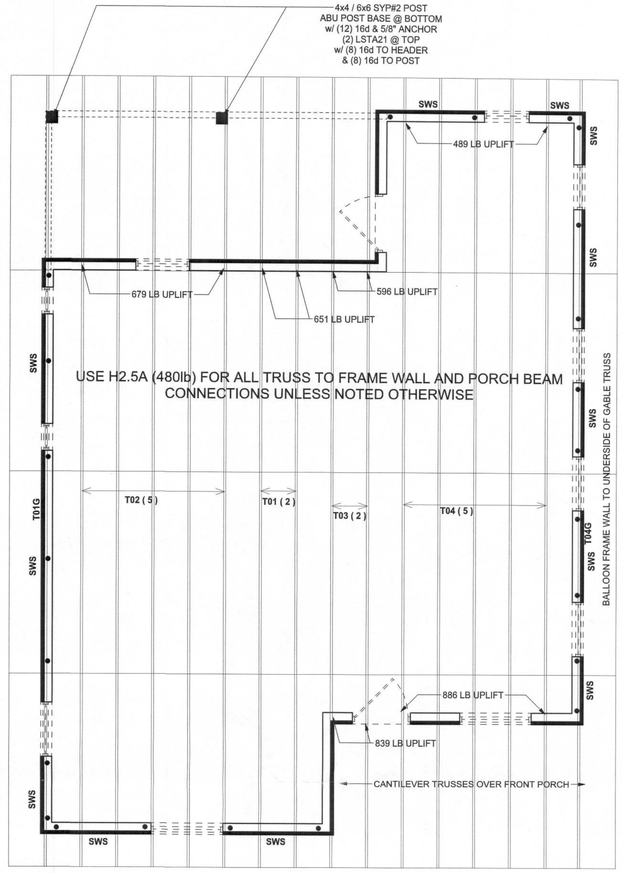
	TRANSVERSE	LONGITUDINAL
ACTUAL	47.5'	20.5'
REQUIRED	18.0'	16.2'

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

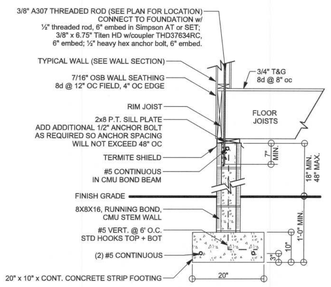
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



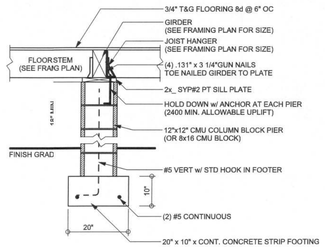
STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

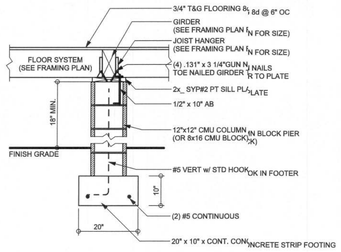
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB # 354330



F1 STEMWALL FOUNDATION (CRAWLSPACE)
SCALE: 1/2" = 1'-0"



F3 PIR FOUNDATION
CMPIER w/ UPLIFT SCALE: 1/2" = 1'-0"



F4 PIER FOUNDATION
CMU PIER w/o UPLIFT SCALE: 1/2" = 1'-0"

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAI ENGINEER:
Mark Disosway, PE
No.53915, IOB 868, Lake City, FL 32056,
386-754-549

DIMENSIONS:
Stated dimvsions supercede scaled dimensionsRefer all questions to Mark Disosay, P.E. for resolution.
Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined tis plan, and that the applicable portions of 'e plan, relating to wind enginering comply with section R301.2.1, frida building code residential 207, and 2009 supplement to the best f my knowledge.

LIMITATIOI: This design is valid for one building, at one address.

MARK DISOSWAY P.E. 53915
No 53915
STATE OF FLORIDA
SEAL
PROFESSIONAL ENGINEER

Woodnan Park Builders

Hraly Residence

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