

FRONT ELEVATION

NOTE: ALL CEILING HEIGHTS SHALL BE 8'-0" UNLESS OTHERWISE NOTED.

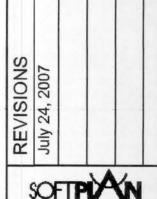
Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type XX gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inhches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly intito a room used for sleeping purposes shall not be permitted.

- Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
- 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
- 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

AREA SUMMARY

LIVING AREA	1480	S.F.
GARAGE AREA	493	S.F.
ENTRY PORCH AREA	32	S.F.
TOTAL AREA	2,005	S.F



SOFTPIXN

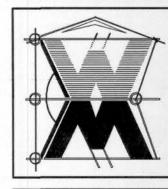
FLOOR PLAN SCALE: 1/4" = 1'-0"

CONSTRUCTION 3 ATH, FRSON

NATHAN PETE
LAKE CITY, FLORIDA 32025 DANIEL HOME

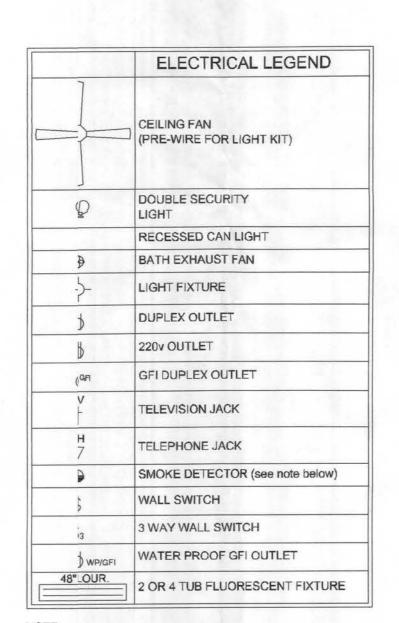
PROJECT ADDRESS: HWY 4

OVILLIAM MYERS DE.54(N P.O. BOX 1513 LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net



JOB NUMBER

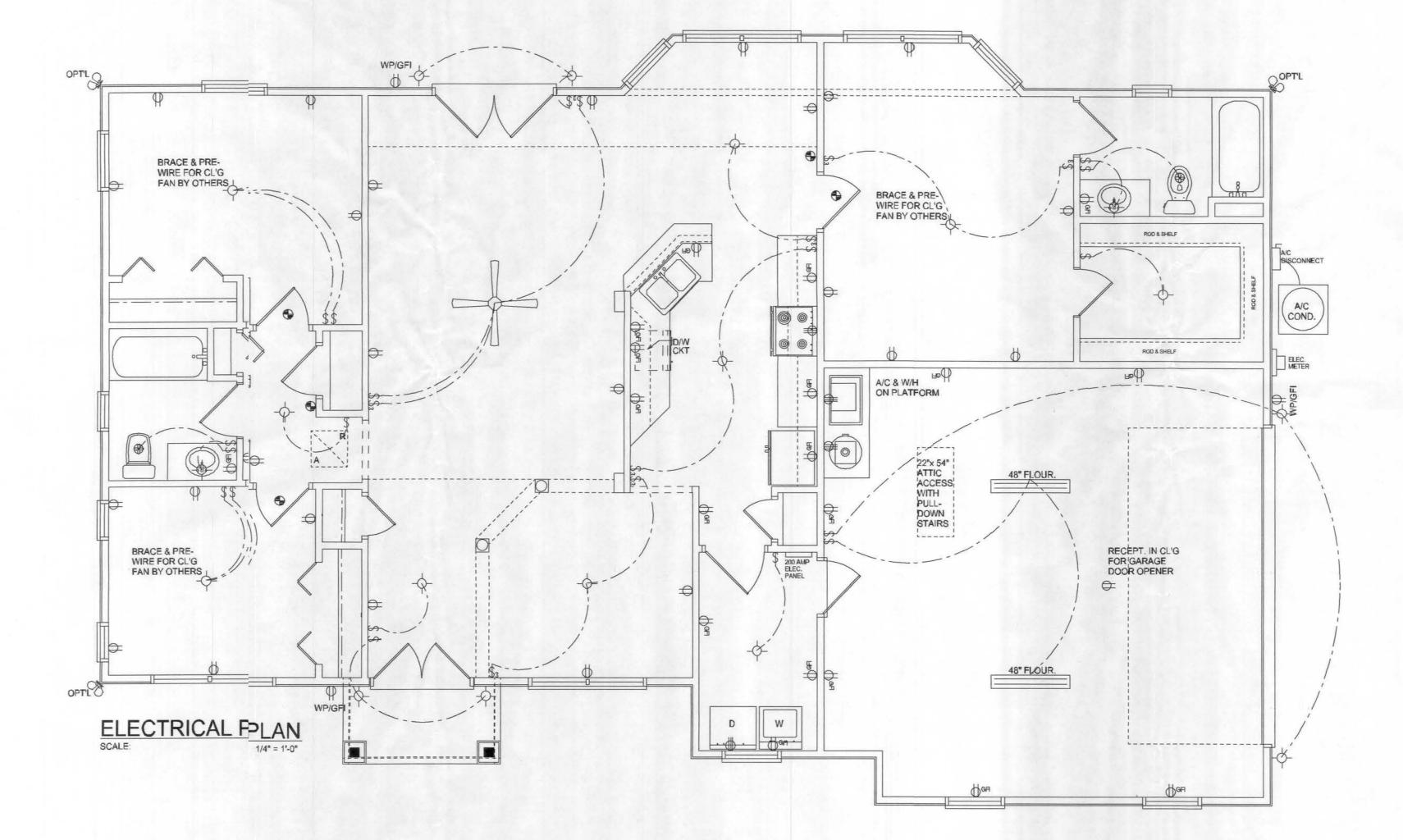
SHEET NUMBER



NOTE: ALL BEDDOM RECEPTACLES SHALL BE AFCI (ARC FALT CIRCUIT INTERRUPT)

ALL SMOE DETECTORS SHALL HAVE BATTERY BACKUP POWER AND ALL 'IRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVITE.

THE ELE(RICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALL® ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BUSED AS AN EQUIPMENT GROUND.



SOFTPIÓN ARCHIFECTURAL DESIGN SOFTWARE

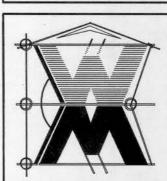
PLAN 1/4" = 1'-0" ELECTRICAL SCALE:

WINSBER NATHAN PETERSON CONSTRUCTION
LAKE CITY, FLORIDA 32025 A NEW CUSTOM HOME F

DANIEL

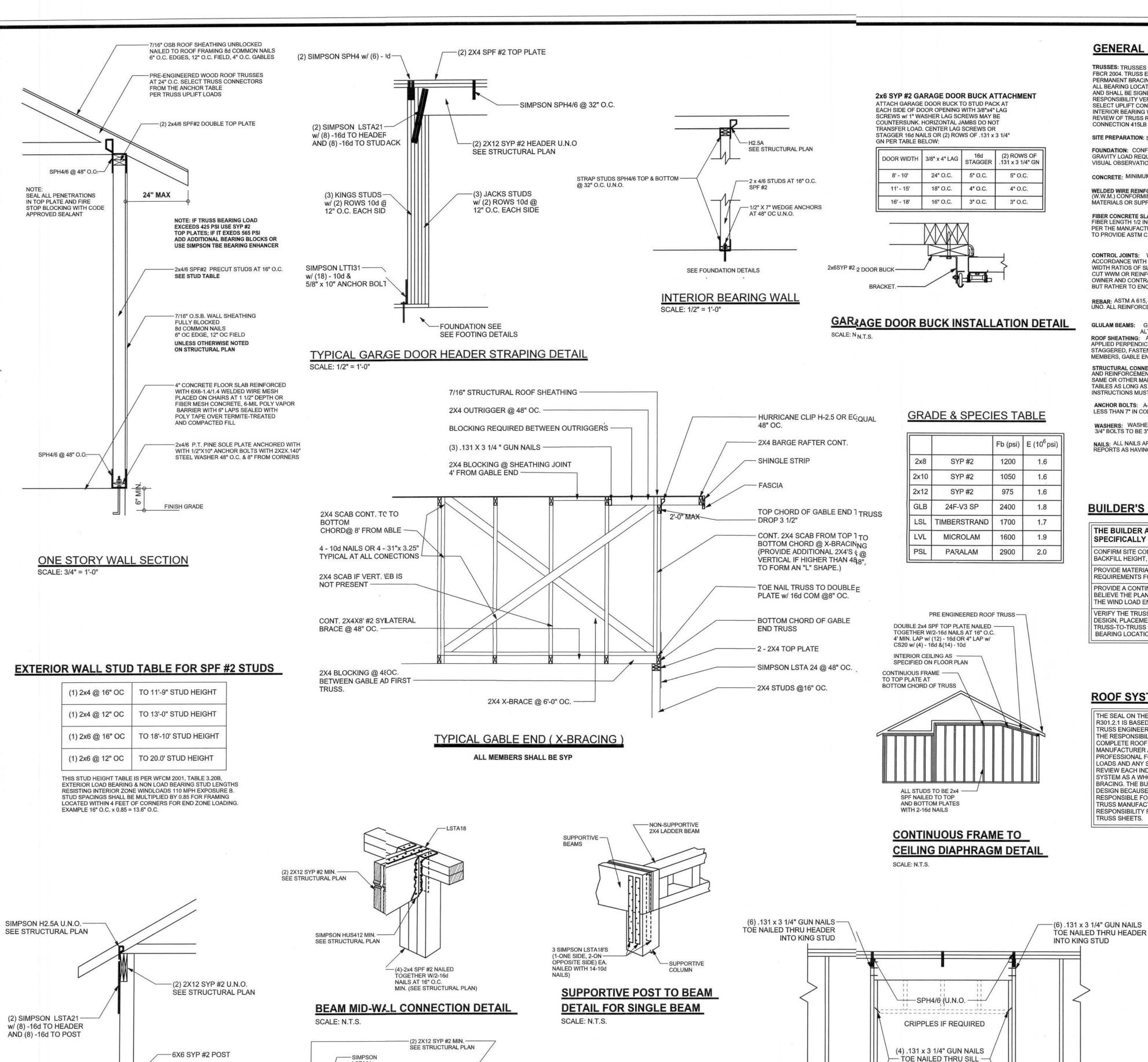
PROJECT ADDRESS: HWY 47

OWILLIAM MYERS DE.54(N P.O. BOX 1513 LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net



JOB NUMBER

SHEET NUMBER



SUPPORTIVE BEAM -

SUPPORTIVE CENTER POST TO BEAM DETAIL

POST CONNECTION,

LSTA18 ON ONE SIDE

SIMPSON ABU POST BASE

BEAM W/4-16d

BEAM MAY BE ATTACED IN EITHER METHOD SHVN ABOVE

BEAM CORNER CONECTION. DETAIL

IMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

ANCHOR BOLT

INSTALL ONE SIMPSON

4-SIMPSON LSTA18 -

(2-ONE SIDE,2-ON

OTHER SIDE)

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO WNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST EPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALI	Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	ONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND T, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PI	TINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
DESIGN, PLACE	SS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS MENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, S CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

Grout

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

Coating for corrosion protection

IN WRITING.

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

-SPH4 ALL OPENINGS (U.N.O.)

(SEE STRUCTURAL PLAN)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

TY/PICAL HEADER STRAPING DETAIL
SCAALE: 1/2" = 1'-0"

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

Specific Requirements

5.5"x2.75"x11.5"

Coating for corrosion protection | Joint reinforcement in walls exposed to

Pipes, conduits, and accessories | Any not shown on the project drawings

ASTM C 270, Type N, UNO

3" block bearing walls F'm = 1500 psi

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

ASTM C 476, admixtures require approval

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	1 - 1 - 1 - 1 - 1	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RC 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		The state of the s
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		344.034
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		
< 2320	< 2320	ABU88	18 - 16d		1/2" AB
-0-0	2020	Aboot	10 - 100		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B

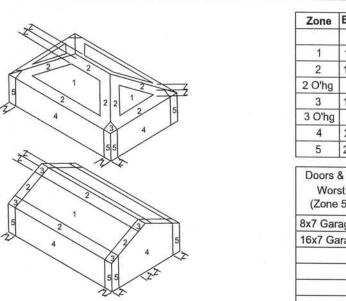
3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

MEAN ROOF HEIGHT = <30 FT

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

PRINTED DATE: July 12, 2007 DRAWN BY:

David Disosway

REVISIONS

SOFTPIXN

VINDLOAD ENGINEER: Mark Disosway.

PE No.53915, POB 868, Lake City, FL

stated dimensions supercede scaled

dimensions. Refer all questions to

Do not proceed without clarification.

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orm or manner without first the express writter

rmission and consent of Mark Disosway.

mined this plan, and that the applicable

portions of the plan, relating to wind engineeri comply with section R301.2.1, florida building

ode residential 2004, to the best of my

LIMITATION: This design is valid for one

P.E. 53915

Peterson Construction

Daniel & Kathy

Winsberg Residence

ADDRESS:

HWY 47

Columbia County, Florida

Mark Disosway P.E.

P.O. Box 868

Lake City, Florida 32056

Phone: (386) 754 - 5419

Fax: (386) 269 - 4871

ouilding, at specified location.

Mark Disosway, P.E. for resolution

32056, 386-754-5419

FINALS DATE: 12 / Jul / 07 JOB NUMBER:

> S-1 OF 3 SHEETS

707021

DRAWING NUMBER

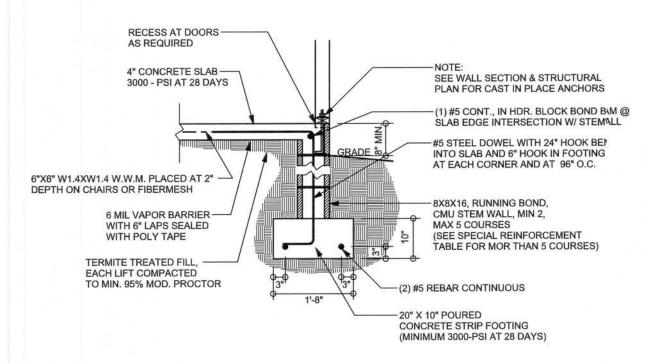
INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

Zone Effective Wind Area (ft2) 19.9 -21.8 18.1 -18.1

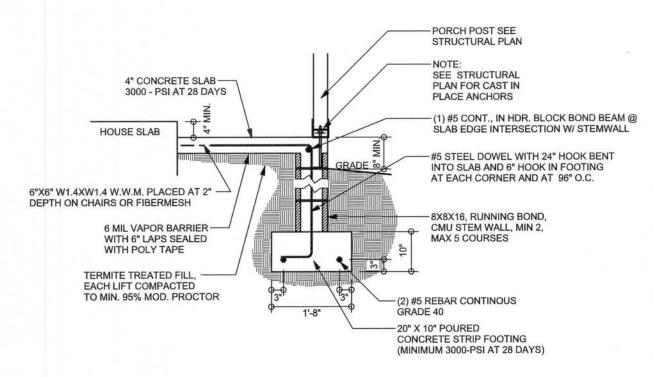
	2	19.9	-25.5	18.1
	2 O'hg		-40.6	
2 2 2 1	3	19.9	-25.5	18.1
4	3 O'hg		-68.3	
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	4	21.8	-23.6	18.5
55	5	21.8	-29.1	18.5
		& Winst Cas	e	21.8
5 2 3	8x7 Gar	age D	oor	19.5
2 4 5	16x7 Ga	arage l	Door	18.5
55 22				
DESIGN LOADS				
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)				

16 PSF (4:12 TO <12:12)

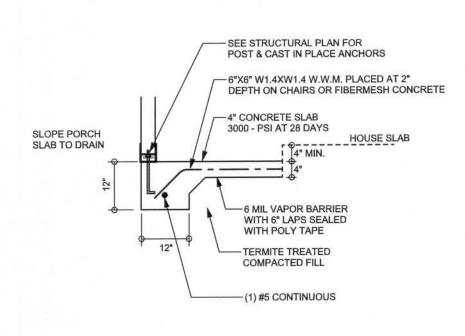
NOT IN FLOOD ZONE (BUILDER TO VERIFY



F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

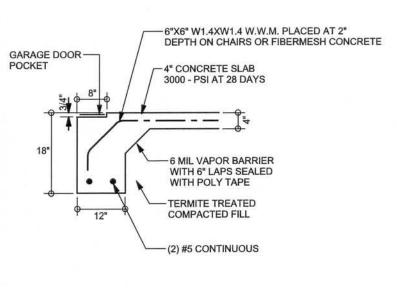


F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

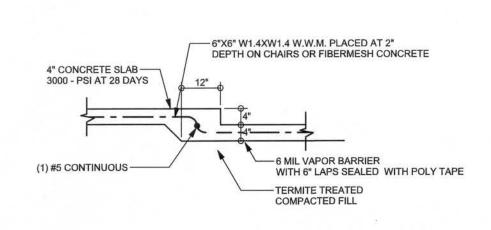
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

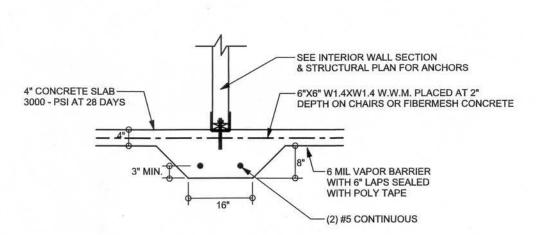
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	AL REINFOR 5" CMU STEN INCHES O.C	IWALL	FOR 12	AL REINFOR: 2" CMU STEN INCHES O.C	WALL
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



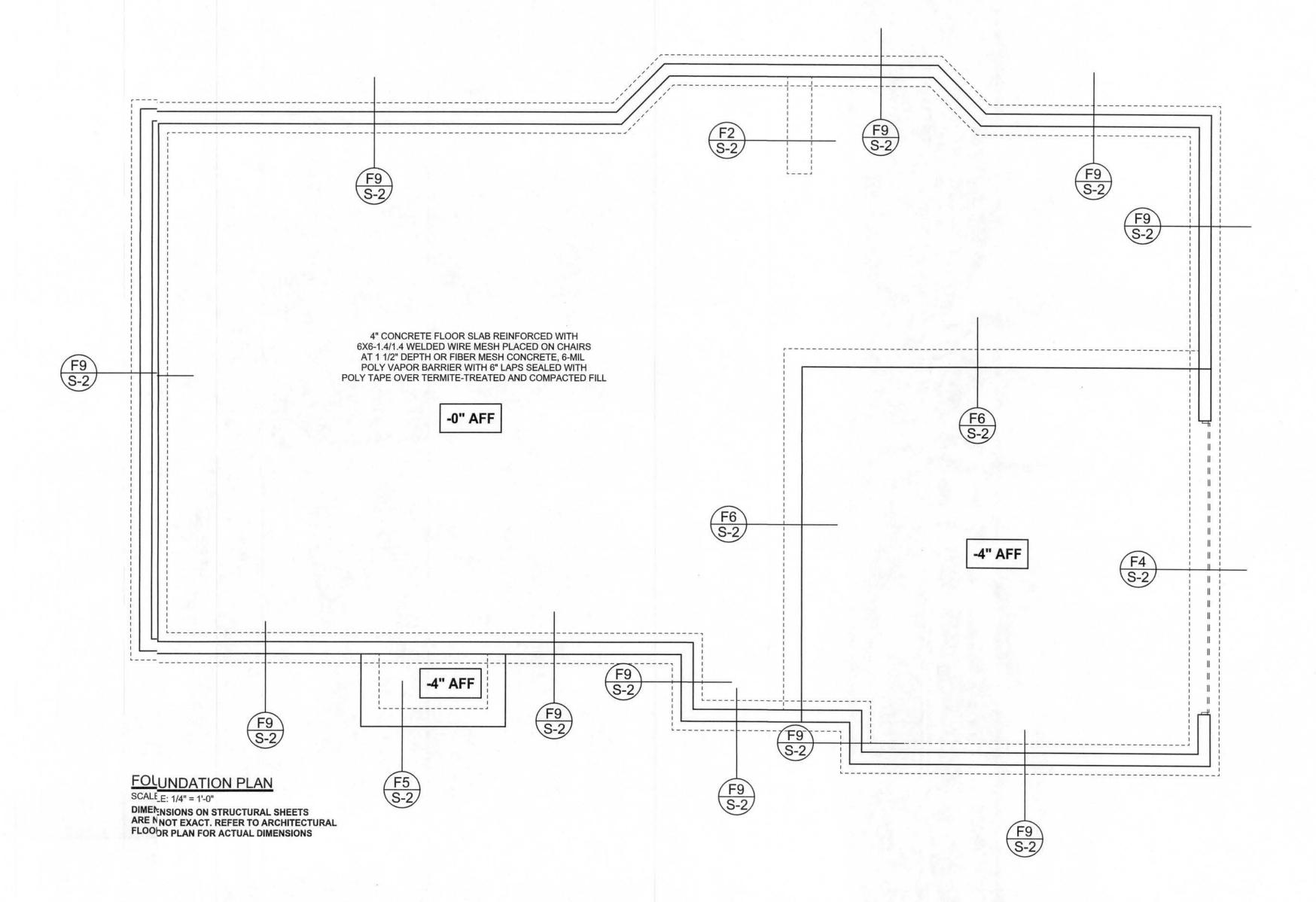
F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN

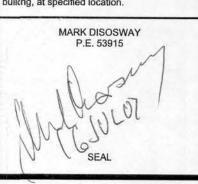
WINDLOAD ENGINEER: Mark Disosway, PE Io.53915, POB 868, Lake City, FL 3206, 386-754-5419 DIMENSIONS: Statd dimensions supercede scaled

dimisions. Refer all questions to Mar Disosway, P.E. for resolution. Do pt proceed without clarification.

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CEFTIFICATION: I hereby certify that I have examined this plan, and that the applicable portons of the plan, relating to wind engineering comply with section R301.2.1, florida building coduresidential 2004, to the best of my knowledge.

LIMFATION: This design is valid for one building, at specified location.



<u>Peterson Construction</u>

Daniel & Kathy Winsberg Residence

> ADDRESS: HWY 47 Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
July 12, 2007

CRAWN BY: CHECKED BY:

Lavid Disosway

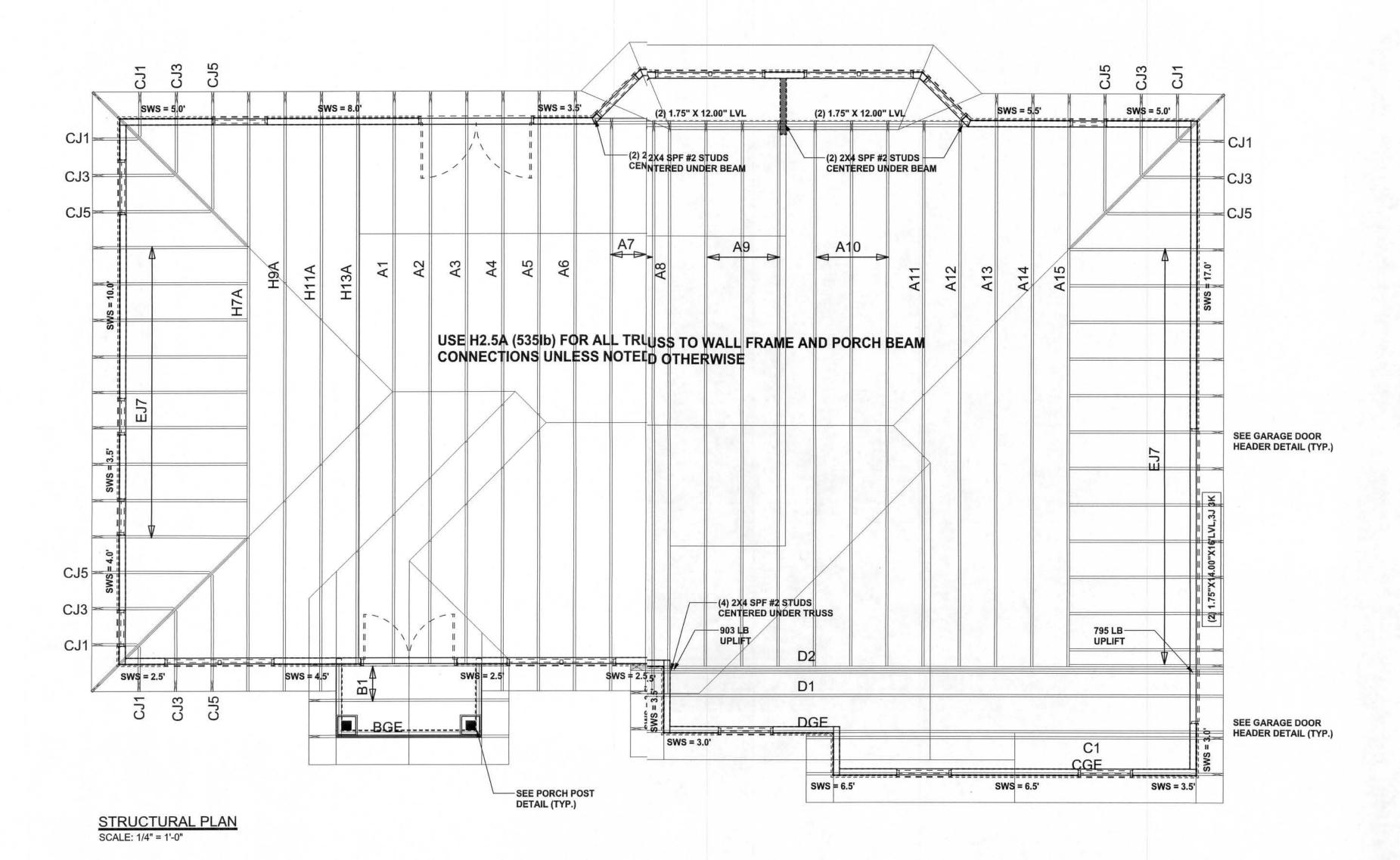
FNALS DATE: 12 / Jul / 07

JOB NUMBER: 707021 DRAWING NUMBER

> S-2 OF 3 SHEETS

REVISIONS

SOFTPIXN ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

sws = 0.0'	1ST FLOOR EXTERIOR WALL	
SWS = 0.0'	2ND FLOOR EXTERIOR WALL	
IBW	1ST FLOOR INTERIOR BEARING WALL	
IBW	2ND FLOOR INTERIOR BEARING WALL	

HEADER LEGEND

HEADER/BEAM CALL-OUT (U.N.O.)	
-NUMBER OF KING STUDS (FULL LENGTH)	
-NUMBER OF JACK STUDS (UNDER HEADER)	
—SPAN OF HEADER	
SIZE OF HEADER MATERIAL	
-NUMBER OF PLIES IN HEADER	
	NUMBER OF KING STUDS (FULL LENGTH)NUMBER OF JACK STUDS (UNDER HEADER)SPAN OF HEADERSIZE OF HEADER MATERIAL

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	41.0'
LONGITUDINAL	19.3'	58.5'

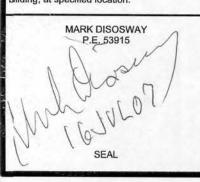
VINDLOAD ENGINEER: Mark Disosway, FE No.53915, POB 868, Lake City, FL 3056, 386-754-5419

EMENSIONS:
Sated dimensions supercede scaled dmensions. Refer all questions to lark Disosway, P.E. for resolution.
D not proceed without clarification.

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CERTIFICATION: I hereby certify that I have earnined this plan, and that the applicable prtions of the plan, relating to wind engineering omply with section R301.2.1, florida building ode residential 2004, to the best of my kowledge.

LMITATION: This design is valid for one bilding, at specified location.



Peterson Construction

Daniel & Kathy Winsberg Residence

ADDRESS: HWY 47 Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
July 12, 2007

DRAWN BY: CHECKED BY:
David Disosway

5000 0 DATE |

JOB NUMBER: 707021

> S-3 OF 3 SHEETS

DRAWING NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #7-195