

DATE 09/16/2008

Columbia County Building Permit  
This Permit Must Be Prominently Posted on Premises During Construction

PERMIT  
000027346

APPLICANT SHIRLEY BENNETT PHONE 288-2428  
ADDRESS 3104 SW OLD WIRE RD FT. WHITE FL 32038  
OWNER SHALEDA GAINER MIRRA PHONE 623-3611  
ADDRESS 793 SW NATILUS RD LAKE CITY FL 32024  
CONTRACTOR TIM EVANS PHONE 352 474-0483  
LOCATION OF PROPERTY 41S, TR ON CR 131, TR ON NAUTILUS, 7/10 OF MILE  
ON RIGHT  
TYPE DEVELOPMENT ATTACHED GARAGE ESTIMATED COST OF CONSTRUCTION 12000.00  
HEATED FLOOR AREA TOTAL AREA 480.00 HEIGHT STORIES 1  
FOUNDATION CONC WALLS FRAMED ROOF PITCH 3/12 FLOOR SLAB  
LAND USE & ZONING A-3 MAX. HEIGHT 14  
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00  
NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 19-5S-17-09284-009 SUBDIVISION  
LOT BLOCK PHASE UNIT TOTAL ACRES 5.00

CGC1512656  
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor  
X08-287 BK HD N  
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: IMPACT FEE EXEMPT

Check # or Cash 4481

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by  
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by  
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by  
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by  
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by  
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by  
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by  
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 60.00 CERTIFICATION FEE \$ 2.40 SURCHARGE FEE \$ 2.40  
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ FIRE FEE \$ 0.00 WASTE FEE \$  
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ CULVERT FEE \$ TOTAL FEE 64.80  
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

## Columbia County Building Permit Application

4481

For Office Use Only Application # 0809-04 Date Received 9/2/08 By GT Permit # 27346  
Zoning Official BLK Date 16.09.08 Flood Zone X FEMA Map # N/A Zoning A-3  
Land Use A-3 Elevation N/A MFE N/A River N/A Plans Examiner HO Date 9-9-08  
Comments Imperv Fec Exempt

☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # \_\_\_\_\_  
☐ Dev Permit # \_\_\_\_\_ ☐ In Floodway ☒ Letter of Authorization from Contractor  
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 08-0479 Fax 386-466-1866

Name Authorized Person Signing Permit Wendy Grennell Phone 386-288-2428

Address 3104 SW Old Wire Road Ft White FL 32038

Owners Name Shaleda Gaur Mirra Phone 386-623-3611

911 Address 793 SW Nautilus Rd Lake City FL 32024

Contractors Name Tim Evans Phone 352-474-0483

Address 23808 SW 30th Ave Newberry FL 32669

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address CTG PO Box 187 West Howard St Live Oak FL 32064

Mortgage Lenders Name & Address NA

Circle the correct power company - FL Power & Light Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 19-55-17-09284-009 Estimated Cost of Construction \$12,000

Subdivision Name NA Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_

Driving Directions 41 South, TR on CR 131, TR on Nautilus  
7/10 of mile on R

Number of Existing Dwellings on Property 1

Construction of Garage Attached Total Acreage 5 Lot Size \_\_\_\_\_

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 14

Actual Distance of Structure from Property Lines - Front 65' Side 84' Side 116' Rear 415'

Number of Stories 1 Heated Floor Area \_\_\_\_\_ Total Floor Area 480 Roof Pitch 3/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

spoke to Wendy  
9/16/08

27346

## NOTICE OF COMMENCEMENT

Inst: 200812017104 Date: 9/17/2008 Time: 8:07 AM  
14 DC, P. DeWitt Cason, Columbia County Page 1 of 1 B: 1158 P: 1390Tax Parcel Identification Number 19-55-17-09284-009

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

## 1. Description of property (legal description):

a) Street (job) Address: 793 SW Nautilus Rd Lake City FL 320242. General description of improvements: new garage

## 3. Owner Information

a) Name and address: Shaleda Gainer Mirra 793 SW Nautilus Rdb) Name and address of fee simple titleholder (if other than owner) NA Lake City FLc) Interest in property owner

## 4. Contractor Information

a) Name and address: Tim Evans 23808 SW 30<sup>th</sup> Ave Newberry FLb) Telephone No.: 352-474-0483 Fax No. (Opt.) \_\_\_\_\_

## 5. Surety Information

a) Name and address: \_\_\_\_\_

b) Amount of Bond: \_\_\_\_\_

c) Telephone No.: \_\_\_\_\_ Fax No. (Opt.) \_\_\_\_\_

## 6. Lender

a) Name and address: NA

b) Phone No.: \_\_\_\_\_

## 7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:

a) Name and address: NA

b) Telephone No.: \_\_\_\_\_ Fax No. (Opt.) \_\_\_\_\_

8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b).  
Florida Statutes:

a) Name and address: NA

b) Telephone No.: \_\_\_\_\_ Fax No. (Opt.) \_\_\_\_\_

9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): \_\_\_\_\_

**WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.**

STATE OF FLORIDA  
COUNTY OF COLUMBIA

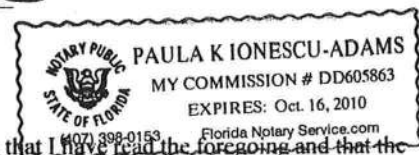
10. Shaleda A Gainer-Mirra  
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager  
Shaleda A Gainer-Mirra  
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 2 day of September, 2008, by:

Shaleda Gainer-Mirra owner (type of authority, e.g. officer, trustee, attorney  
fact) for \_\_\_\_\_ (name of party on behalf of whom instrument was executed).

Personally Known \_\_\_\_\_ OR Produced Identification ☒ Type Drivers License

Notary Signature Paula K. Ionescu-Adams Notary Stamp or Seal:



—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Shaleda A Gainer-Mirra  
Signature of Natural Person Signing (in line #10 above.)

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

**FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment**

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

**NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:**

**YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

**OWNERS CERTIFICATION:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

*Saleda A. Shiner Murra*

Owners Signature

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

*[Signature]*

Contractor's Signature (Permittee)

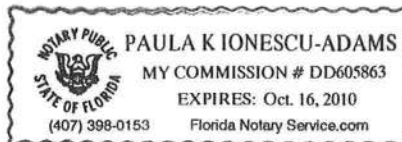
Contractor's License Number CGC1572656  
Columbia County  
Competency Card Number \_\_\_\_\_

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2<sup>nd</sup> day of September 2008.  
Personally known \_\_\_\_\_ or Produced Identification Drivers License

*Paula K. Ionescu-Adams*

State of Florida Notary Signature (For the Contractor)

SEAL:



# New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

**Public reporting burden** for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

27346

## Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.  
Company Address: P.O. Box 1795 City Lake City State FL Zip 32055  
Company Business License No. JB109476 Company Phone No. 386-755-3611 • 352-494-5751  
FHA/VA Case No. (if any) \_\_\_\_\_

## Section 2: Builder Information

Company Name: Ironwood Homes Company Phone No. \_\_\_\_\_

## Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 793 S.W. Rooting Rd. Lake City, FL 32055

Type of Construction (More than one box may be checked) ☐ Slab ☐ Basement ☒ Crawl ☐ Other \_\_\_\_\_  
Approximate Depth of Footing: Outside \_\_\_\_\_ Inside \_\_\_\_\_ Type of Fill \_\_\_\_\_

## Section 4: Treatment Information

Date(s) of Treatment(s) 9-9-08  
Brand Name of Product(s) Used B-400  
EPA Registration No. 93443-149  
Approximate Final Mix Solution % 1.6  
Approximate Size of Treatment Area: Sq. ft. 2484 Linear ft. \_\_\_\_\_ Linear ft. of Masonry Voids \_\_\_\_\_  
Approximate Total Gallons of Solution Applied 192  
Was treatment completed on exterior? ☒ Yes ☐ No  
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) \_\_\_\_\_

Comments Treated all stem walls

Name of Applicator(s) Steve Brannon Certification No. (if required by State law) JP104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [Signature] Date 9-9-08

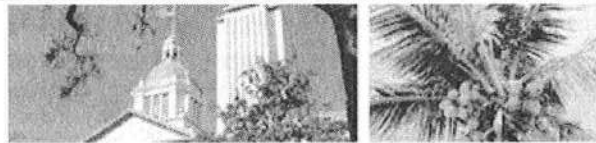
**Warning:** HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

Florida Department of  
**Business &  
Professional  
Regulation**

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10:57:32 AM 9/16/2008

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**Licensee Details**

**Licensee Information**

Name: **EVANS, TIMOTHY ALLEN SR (Primary Name)**  
**EVANS PLUMBING INC (DBA Name)**  
 Main Address: **23808 SW 30 AVE**  
**NEWBERRY Florida 32669-4806**  
 County: **ALACHUA**

License Mailing:

LicenseLocation: **23808 SW 30 AVE**  
**NEWBERRY FL 32669-4806**  
 County: **ALACHUA**

**License Information**

License Type: **Certified General Contractor**  
 Rank: **Cert General**  
 License Number: **CGC1512656**  
 Status: **Current,Active**  
 Licensure Date: **01/08/2007**  
 Expires: **08/31/2010**

**Special Qualifications**  
**Qualified Business License Required**  
**Qualification Effective**  
**01/08/2007**

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This Instrument Prepared by & return to:

Name: **KIM WATSON, an employee of  
TITLE OFFICES, LLC**  
Address: **343 NW COLE TERRACE, SUITE 101  
LAKE CITY, FLORIDA 32055  
File No. 08Y-05040KW**

COPY

Parcel I.D. #: 09284-009

SPACE ABOVE THIS LINE FOR PROCESSING DATA

Inst:200812011224 Date:6/12/2008 Time:3:37 PM  
Doc Stamp-Deed:0.70  
DC,P.DeWitt Carson, Columbia County Page 1 of 2 B:1152 P:907

**THIS WARRANTY DEED** Made the 10th day of June, A.D. 2008, by **SHALEDA GAINER MIRRA, JOINED BY HER HUSBAND CHRISTOPHER B. MIRRA**, hereinafter called the grantor to **CHRISTOPHER B. MIRRA and SHALEDA GAINER MIRRA, HIS WIFE**, whose post office address is 186 SW RUSSWOOD TERRACE, LAKE CITY, FLORIDA 32024, hereinafter called the grantees:

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in Columbia County, State of Florida, viz:

A PART OF THE SOUTHEAST ¼ OF THE NORTHWEST ¼ OF SECTION 19, TOWNSHIP 5 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: THE EAST 220 FEET OF THE WEST 640 FEET OF THE SOUTH ½ OF THE SOUTHEAST ¼ OF THE NORTHWEST ¼ OF SAID SECTION 19 AND THE EAST 220 FEET OF THE WEST 640 FEET OF THE SOUTH ½ OF THE NORTH ¼ OF THE SOUTHEAST ¼ OF THE NORTHWEST ¼ OF SAID SECTION 19.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold the same in fee simple forever.


And the grantor hereby covenants with said grantees that it is lawfully seized of said land in fee simple; that it has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2008.

In Witness Whereof, the said grantor has caused these presents to be executed in its name and its corporate seal to be hereunto affixed by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in the presence of:

  
Witness Signature  
Regina Simpkins

Printed Name


  
Witness Signature  
Kim Watson  
Printed Name

  
SHALEDA GAINER MIRRA

  
CHRISTOPHER B. MIRRA

STATE OF FLORIDA  
COUNTY OF COLUMBIA

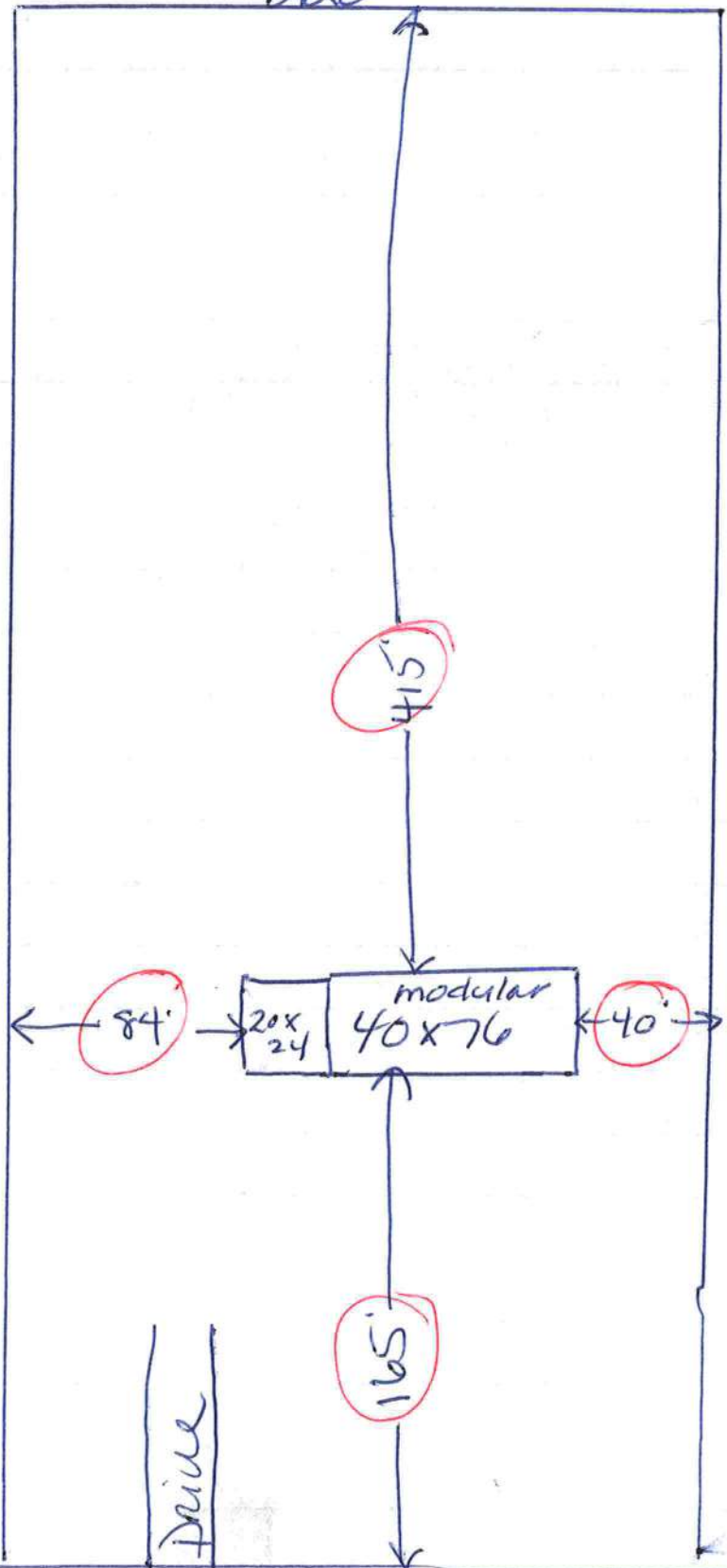
The foregoing instrument was acknowledged before me this 10th day of June, 2008, by **SHALEDA GAINER MIRRA AND CHRISTOPHER B. MIRRA, HER HUSBAND** who is known to me or who has produced driver's license as identification.

  
Notary Public

My commission expires

NOTARY PUBLIC-STATE OF FLORIDA  
KIM WATSON  
Commission # DD685151  
Expires: AUG. 01, 2011  
BONDED THRU ATLANTIC BONDING CO., INC.

220'



640'

drive

Nautilus Rd

**LIMITED POWER OF ATTORNEY**

I, Tim Evans, license number CGC151268 authorize Wendy Grennell, Danielle Andrews or Shirley Bennett to be my representative and act on my behalf in all aspects of applying for permits for following described property in Columbia County, State of Florida.

Home Owner Name: Shaleda Gainer Mirra

Property Owner Name: Same

911 Address: 793 SW Nautilus Rd City Lake City

Sec: 19 Twp: 55 Rge: 17 Tax Parcel # 09284-009

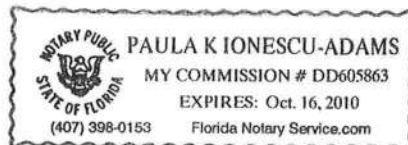
Signed: [Signature]  
General Contractor

Sworn to and described before me this 2<sup>nd</sup> day of September 2008

Paula K. Ionescu-Adams  
Notary public

Paula K. Ionescu-Adams Personally known \_\_\_\_\_  
Notary Name

DL ID ✓



# ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844  
Florida Engineering Certificate of Authorization Number: 0 278  
Florida Certificate of Product Approval # FL1999  
Page 1 of 1 Document ID:1TKO215-Z0205101336

Truss Fabricator: W.B. Howland  
Job Identification: 5671-/Ironwood Homes - 20' x 24' /Contractor -- LAKE CITY, FL  
Truss Count: 2  
Model Code: Florida Building Code 2004 and 2006 Supplement  
Truss Criteria: ANSI/TPI-2002(STD)/FBC  
Engineering Software: Alpine Software, Version 7.38.  
Structural Engineer of Record: The identity of the structural EOR did not exist as of  
Address: the seal date per section 61615-31.003(5a) of the FAC  
Minimum Design Loads: Roof - 40.0 PSF @ 1.25' Duration  
Floor - N/A  
Wind - 110 MPH ASCE 7-02 -Closed

## Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR215

Details: A11015EE-GBLLETIN-

#	Ref	Description	Drawing#	Date
1	49373--A1		08249005	09/05/08
2	49374--A2		08249006	09/05/08

Seal Date: 09/05/2008

-Truss Design Engineer-  
James F. Collins Jr.  
Florida License Number: 52212  
1950 Marley Drive  
Haines City, FL 33844



(5671-)/Ironwood Homes - 20' x 24' /Contractor -- LAKE CITY, FL - A1)

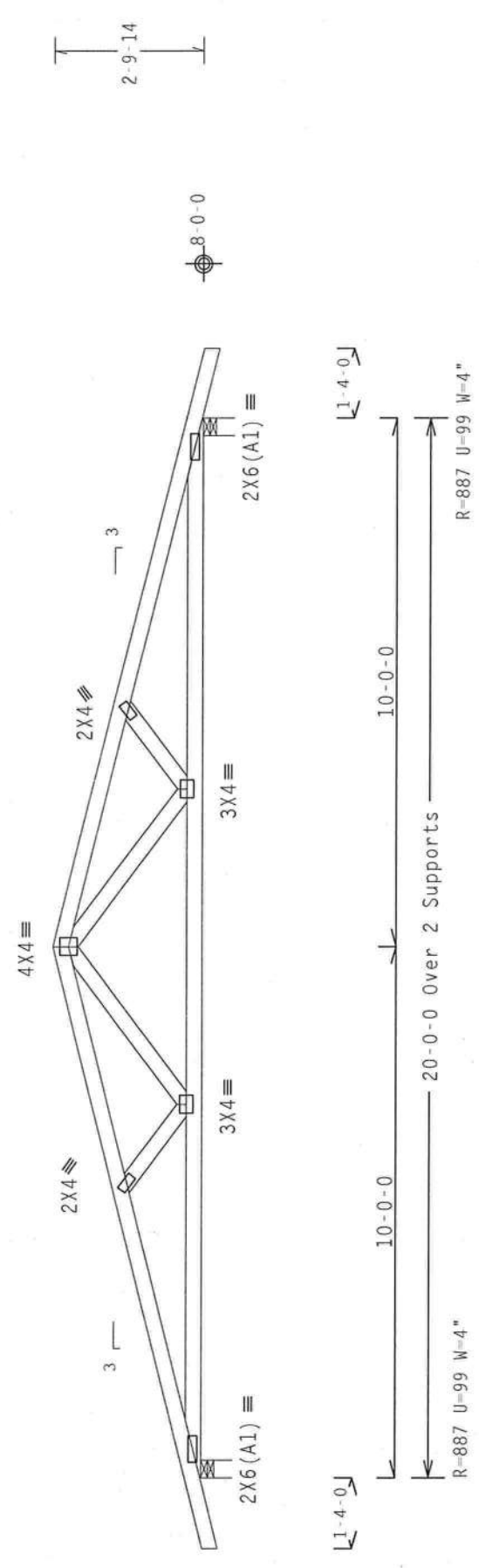
Top chord 2x4 SP #2 N  
Bot chord 2x4 SP #2 N  
Webs 2x4 SP #2 N

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf,  $I_w=1.00$  GCPI (+/-)=0.18

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MWFRS pressures.

The overall height of this truss excluding overhang is 2'-9"-14".



Design Crit: TPI-2002(STD)/FBC  
Cq/RT=1.00(1.25)/0(0)

Scale = .3125"/Ft.  
TY: 14 FL/-/5/-/1-/R/-

PLT TYP. Wave

<div><div><div>ALPINE</div></div><div>ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278</div></div>	<b>**WARNING**</b> TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		TC LL	20.0 PSF	REF	R215 -- 49373
	<b>**IMPORTANT**</b> FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		TC DL	10.0 PSF	DATE	09/05/08
	CONNECTIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AREA) AND TPI. 1TH BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS.		BC DL	10.0 PSF	DRW	HCUSR215 08249005
	ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX 43 OF TPI 2002 SEC. 3.2. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS. UNLESS OTHERWISE INDICATED, ALL CONNECTIONS SHALL BE DESIGNED TO RESIST THE FULL DESIGN LOADS.		BC LL	0.0 PSF	HC-ENG	WHK/WHK *
	DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ABS/TPI 1 SEC. 2.		TOT.LD.	40.0 PSF	SEQN-	241714
			DUR.FAC.	1.25	FROM	CDM
			SPACING	24.0"	JREF-	1TK0215_Z02



Top chord 2x4 SP #2 N  
Bot chord 2x4 SP #2 N  
Webs 2x4 SP #2 N

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 Gcpl(+/-)=0.18

Gable end supports 8" max rake overhang.

Wind reactions based on MWFRS pressures.

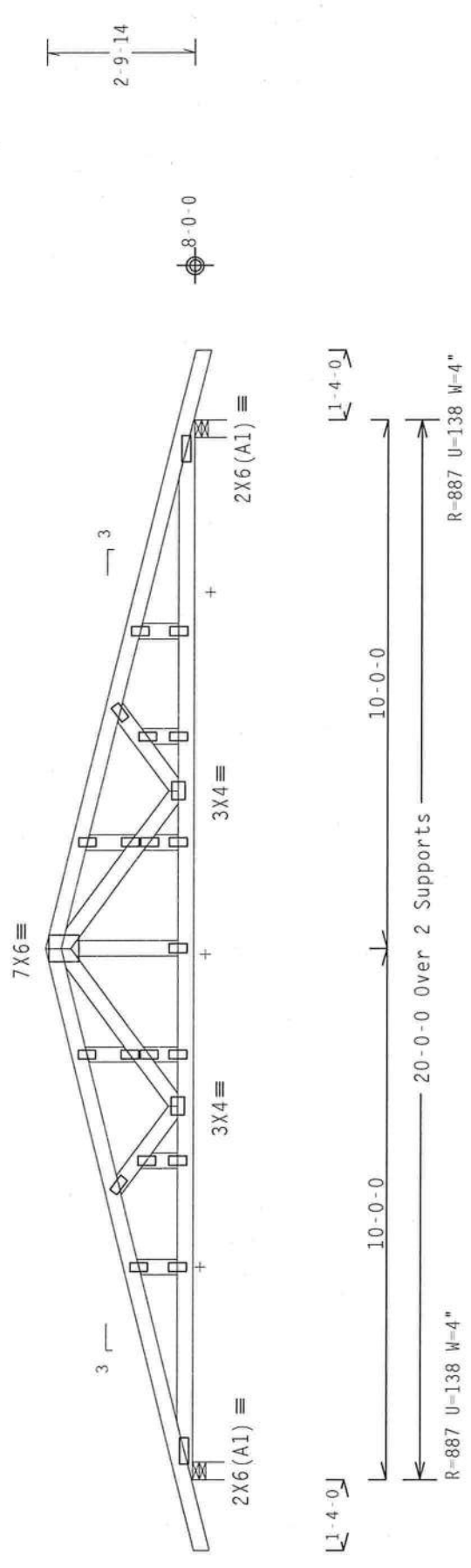
See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 2-9-14.

+ MEMBER TO BE Laterally Braced for HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

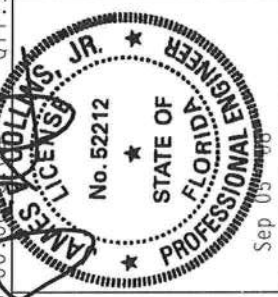
FL/-5/-1-/R/-

Scale = .3125"/Ft.

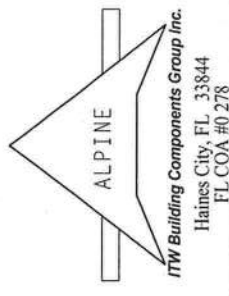
PLT TYP. Wave

**\*\*WARNING\*\*** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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TC LL	20.0 PSF	REF R215-- 49374
TC DL	10.0 PSF	DATE 09/05/08
BC DL	10.0 PSF	DRW HCUSR215 08249006
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	40.0 PSF	SEON- 241716
DUR.FAC.	1.25	FROM CDM
SPACING	SEE ABOVE	JREF- 1TK0215_Z02

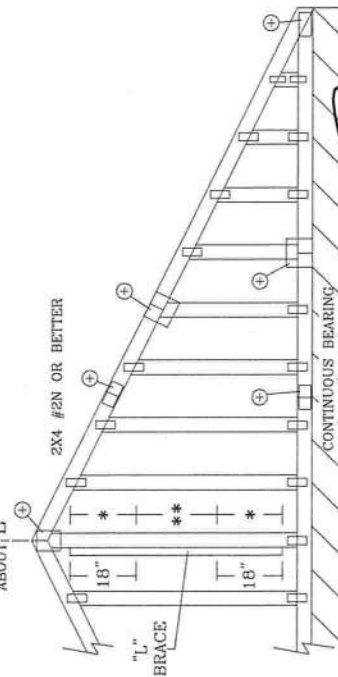


MAX GABLE VERTICAL LENGTH														
2X4 GABLE VERTICAL SPACING SPECIES		BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
12" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	
	HF	#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	
	SP	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	
	DFL	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	
			STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
		#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
	16" O.C.	SPF	#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	
SP		#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
			STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
		#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.		SPF	#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 0"	8' 5"	9' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
			STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

DIAGONAL BRACE OPTION:  
VERTICAL LENGTH MAY BE  
DOUBLED WHEN DIAGONAL  
BRACE IS USED. CONNECT  
DIAGONAL BRACE FOR 600'  
AT EACH END. MAX WEB  
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN  
IN TABLE ABOVE.

CONNECT DIAGONAL AT  
MIDPOINT OF VERTICAL.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES

+ REFER TO COMMON TRUSS DESIGN PEAK SPLICE AND HEEL PLATES

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER  
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

[illegible]

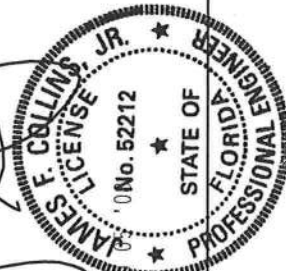
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ITW BUILDING COMPONENTS GROUP, INC.  
POMPANO BEACH, FLORIDA

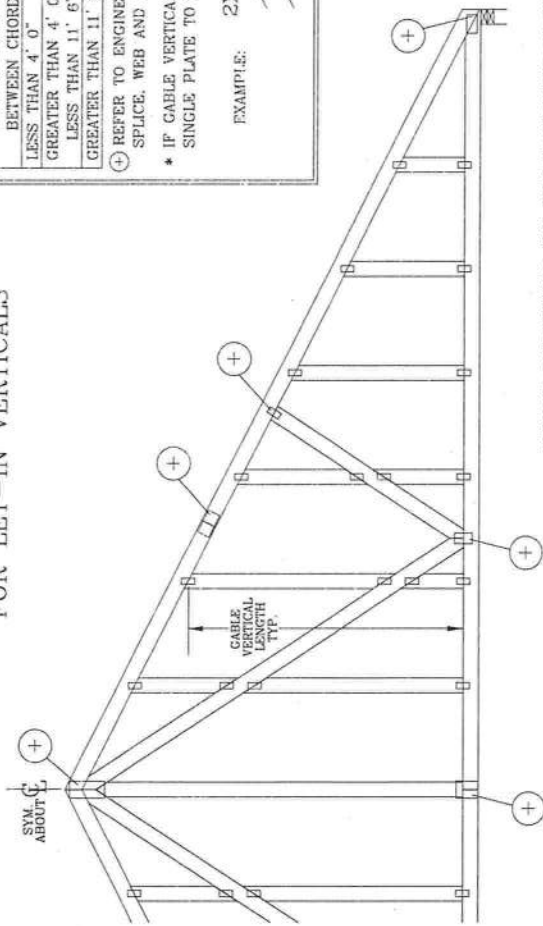
REF	ASCE7-02-GABI1015
DATE	2/23/07
DRWG	A11015EE0207
	-ENG

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



# GABLE DETAIL FOR LET-IN VERTICALS

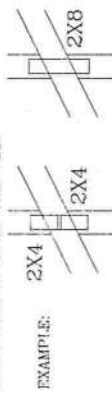


GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

\* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:

8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A1015EN0207, A10015EN0207, A09015EN0207, A07015EN0207,

A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08015EC0207,

A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08030EC0207

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08015EE0207,

A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08030EE0207

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08015E50207,

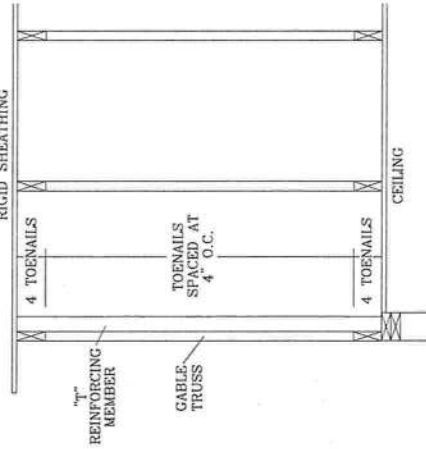
A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08030E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH.

RIGID SHEATHING



ITW BUILDING COMPONENTS GROUP, INC.  
POMPANO BEACH, FLORIDA

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2X6 "T" REINFORCING MEMBER

2X4 "T" REINFORCING MEMBER

TOENAIL

TOENAIL

TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2X4	10 %	10 %
15 FT	2X6	40 %	50 %
110 MPH	2X4	10 %	10 %
30 FT	2X6	50 %	50 %
100 MPH	2X4	10 %	10 %
15 FT	2X6	30 %	50 %
100 MPH	2X4	10 %	10 %
30 FT	2X6	40 %	40 %
90 MPH	2X4	20 %	10 %
15 FT	2X6	20 %	40 %
90 MPH	2X4	10 %	10 %
30 FT	2X6	30 %	50 %
80 MPH	2X4	10 %	20 %
15 FT	2X6	10 %	30 %
80 MPH	2X4	20 %	10 %
30 FT	2X6	20 %	40 %
70 MPH	2X4	0 %	20 %
15 FT	2X6	0 %	20 %
70 MPH	2X4	10 %	20 %
30 FT	2X6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH

1.10 x 6' 7" = 7' 3"

THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

REF LET-IN VERT

DATE 2/23/07

DRWG GBLLETO0207

-ENG DLJ/KAR

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0"





## **STRUCTURAL AND WIND LOAD CALCULATIONS**

**For**

**Iron Wood Homes  
Mira Garage Addition**

  
Gary Gill, P.E. 51942  
P.O. Box 187  
130 West Howard Street  
Live Oak, FL 32064  
Ph. (386) 362-3678  
Fax (386) 362-6133  
AUTH # 9461



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Beam Calcs .....	7



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02

<b>Analysis by:</b> Gary Gill	<b>Company Name:</b> GTC Design Group
<b>Description:</b> Ironwood Homes - Wind Direction 1	

Structure Type	Building	
Basic Wind Speed (V)	110	mph
Struc Category (I, II, III, or IV)	II	
Exposure (B, C, or D)	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof	4.0	:12
Slope of Roof (Theta)	18.4	Deg
Type of Roof	Gabled	
Kd (Directionality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	15.00	ft
Mean Roof Height (Ht)	12.00	ft
Width Perp. To Wind Dir (B)	24.00	ft
Width Paral. To Wind Dir (L)	20.00	ft

Type of Structure	
Height/Least Horizontal Dim	0.60
Flexible Structure	No

Importance Factor	1	
<i>Hurricane Prone Region (V&gt;100 mph)</i>		
Table 6-2 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Zm	Zmin	30.00 ft
Izm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$I*(zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.9276
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8823
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

**Fig 6-5 Internal Pressure Coefficients for Buildings, Gcpi**

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
<b>Enclosed Buildings</b>	<b>0.18</b>	<b>-0.18</b>



**WIND02 v2-21**

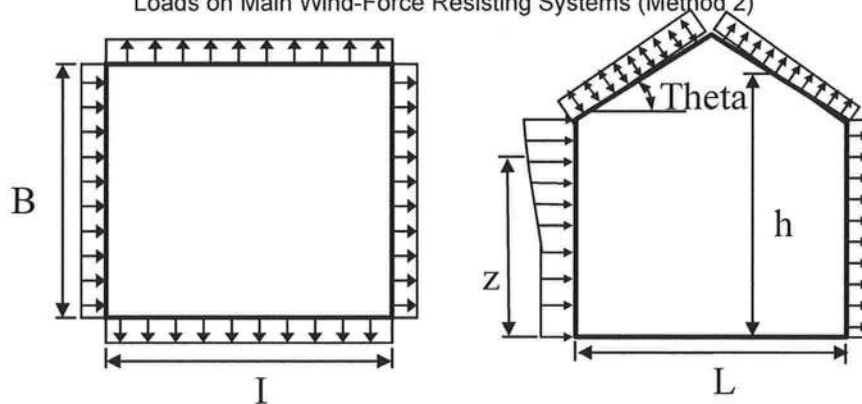
Detailed Wind Load Design (Method 2) per ASCE 7-02  
**6.5.12.2.1 Design Wind Pressure - Buildings of All Heights**

Elev  ft	Kz	Kzt	qz  lb/ft^2	Pressure (lb/ft^2)						
				Windward Wall*		Leeward Wall		Total	Shear	Moment
				+GCpi	-GCpi	+GCpi	-GCpi	+/-Gcpi	(Kip)	(Kip-ft)
15	0.57	1.00	15.13	7.57	13.01	-9.15	-3.71	16.72	6.02	45.15

Note: 1) Positive forces act toward the face and Negative forces act away from the face.

**Figure 6-6 - External Pressure Coefficients, Cp**

Loads on Main Wind-Force Resisting Systems (Method 2)

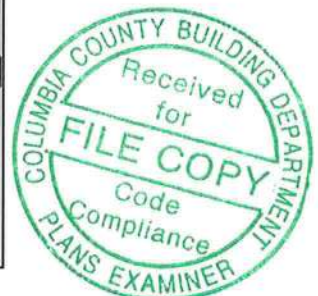


Kh	$2.01 \cdot (15/z_g)^{(2/\alpha)}$	0.57	
Kht	Topographic factor (Fig 6-4)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot I \cdot Kh \cdot Kht \cdot Kd$	15.13	psf
Khcc	Comp & Clad: Table 6-3 Case 1	0.70	
Qhcc	$.00256 \cdot V^2 \cdot I \cdot Khcc \cdot Kht \cdot Kd$	18.45	psf

Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Area (sq. ft.)	-
Reduction Factor	1.00

Leeward Walls (Wind Dir Normal to 24 ft wall)	-0.50	-9.15	-3.71
Leeward Walls (Wind Dir Normal to 20 ft wall)	-0.46	-8.64	-3.19
Side Walls	-0.70	-11.73	-6.28
Windward - Min Cp	-0.56	-9.88	-4.43
Windward - Max Cp	-0.08	-3.78	1.67
Leeward Normal to Ridge	-0.57	-10.11	-4.66
Overhang Top (Windward)	-0.56	-7.15	-7.15
Overhang Top (Leeward)	-0.57	-7.39	-7.39
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02

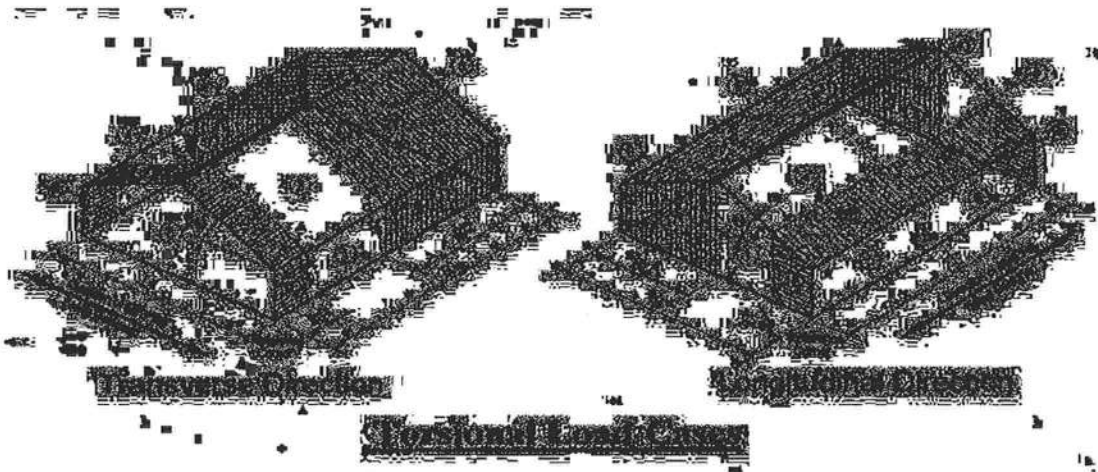
Dist from Windward Edge: 0 ft to 24 ft - Max Cp	-0.18	-5.04	0.41
Dist from Windward Edge: 0 ft to 6 ft - Min Cp	-0.90	-14.30	-8.85
Dist from Windward Edge: 6 ft to 12 ft - Min Cp	-0.90	-14.30	-8.85
Dist from Windward Edge: 12 ft to 24 ft - Min Cp	-0.50	-9.15	-3.71

\* Horizontal distance from windward edge

**Figure 6-10 - External Pressure Coefficients, GCpf**

Loads on Main Wind-Force Resisting Systems w/ Ht &lt;= 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{2/\alpha} &= & 0.70 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 18.45 \\
 \text{Theta} &= \text{Angle of Roof} &= & 18.4 \text{ Deg}
 \end{aligned}$$



Wind Pressures on Main Wind Force Resisting System						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.52	0.18	-0.18	18.45	6.20	12.84
2	-0.69	0.18	-0.18	18.45	-16.05	-9.41
3	-0.47	0.18	-0.18	18.45	-11.96	-5.32
4	-0.42	0.18	-0.18	18.45	-10.98	-4.34
5	-0.45	0.18	-0.18	18.45	-11.62	-4.98
6	-0.45	0.18	-0.18	18.45	-11.62	-4.98
1E	0.78	0.18	-0.18	18.45	11.06	17.70
2E	-1.07	0.18	-0.18	18.45	-23.06	-16.42
3E	-0.67	0.18	-0.18	18.45	-15.73	-9.09
4E	-0.62	0.18	-0.18	18.45	-14.71	-8.07

$$* p = qh \cdot (GCpf - GCpi)$$

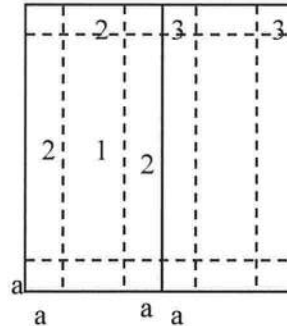
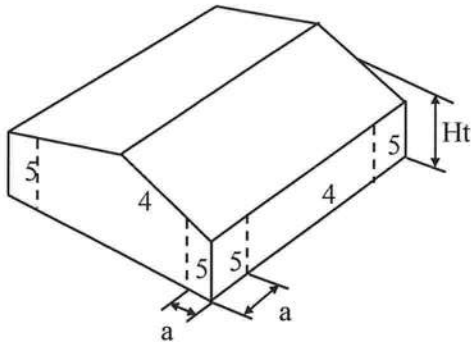
**Figure 6-11 - External Pressure Coefficients, GCp**

Loads on Components and Cladding for Buildings w/ Ht &lt;= 60 ft



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02



Gabled Roof  
 $7 < \text{Theta} \leq 45$

a = 2 ==> 3.00 ft

Double Click on any data entry line to receive a help Screen

Component	Width (ft)	Span (ft)	Area (ft <sup>2</sup> )	Zone	GCp		Wind Press (lb/ft <sup>2</sup> )	
					Max	Min	Max	Min
Wall	10	1	10.00	4	1.00	-1.10	21.77	-23.61
Wall Exterior	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof	10	1	10.00	1	0.50	-0.90	12.54	-19.92
Roof Exterior	10	1	10.00	2	0.50	-1.70	12.54	-34.68
Roof Corner	10	1	10.00	3	0.50	-2.60	12.54	-51.28
	25	1	25.00	4	0.93	-1.03	20.47	-22.31
	25	1	25.00	4	0.93	-1.03	20.47	-22.31
	25	1	25.00	5	0.93	-1.26	20.47	-26.55
	25	1	25.00	1	0.42	-0.86	11.08	-19.19
	25	1	25.00	2	0.42	-1.50	11.08	-31.01
	25	1	25.00	3	0.42	-2.36	11.08	-46.88
			0.00					
			0.00					
			0.00					
			0.00					

Note: \* Enter Zone 1 through 5, or 1H through 3H for overhangs.



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02

<b>Analysis by:</b> Gary Gill	<b>Company Name:</b> GTC Design Group
<b>Description:</b> Ironwood Homes - Wind Direction 2	

Structure Type	Building	
Basic Wind Speed (V)	110	mph
Struc Category (I, II, III, or IV)	II	
Exposure (B, C, or D)	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof	4.0	:12
Slope of Roof (Theta)	18.4	Deg
Type of Roof	Gabled	
Kd (Directionality Factor)	0.85	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	15.00	ft
Mean Roof Height (Ht)	12.00	ft
Width Perp. To Wind Dir (B)	20.00	ft
Width Paral. To Wind Dir (L)	24.00	ft

Type of Structure	
Height/Least Horizontal Dim	0.60
Flexible Structure	No

Importance Factor	1	
<i>Hurricane Prone Region (V&gt;100 mph)</i>		
<b>Table 6-2 Values</b>		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Zm	Zmin	30.00 ft
Izm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$I*(zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.9322
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8850
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85

**Fig 6-5 Internal Pressure Coefficients for Buildings, Gcpi**

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
<b>Enclosed Buildings</b>	<b>0.18</b>	<b>-0.18</b>



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02

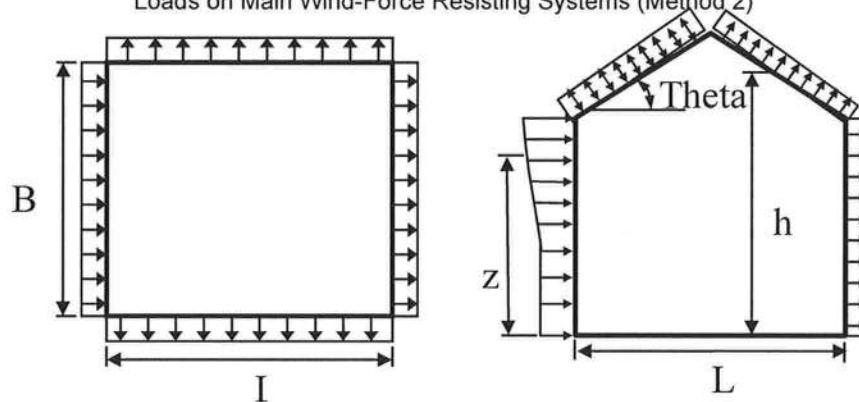
**6.5.12.2.1 Design Wind Pressure - Buildings of All Heights**

Elev ft	Kz	Kzt	qz lb/ft <sup>2</sup>	Pressure (lb/ft <sup>2</sup> )					Shear (Kip)	Moment (Kip-ft)
				Windward Wall*		Leeward Wall		Total		
				+GCpi	-GCpi	+GCpi	-GCpi	+/-Gcpi		
15	0.57	1.00	15.13	7.57	13.01	-8.64	-3.19	16.21	4.86	36.46

Note: 1) Positive forces act toward the face and Negative forces act away from the face.

**Figure 6-6 - External Pressure Coefficients, Cp**

Loads on Main Wind-Force Resisting Systems (Method 2)



Kh	$2.01 \cdot (15/z_g)^{2/\alpha}$	0.57	
Kht	Topographic factor (Fig 6-4)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot I \cdot Kh \cdot Kht \cdot Kd$	15.13	psf
Khcc	Comp & Clad: Table 6-3 Case 1	0.70	
Qhcc	$.00256 \cdot V^2 \cdot I \cdot Khcc \cdot Kht \cdot Kd$	18.45	psf

Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Area (sq. ft.)	-
Reduction Factor	1.00

Leeward Walls (Wind Dir Normal to 20 ft wall)	-0.46	-8.64	-3.19
Leeward Walls (Wind Dir Normal to 24 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Windward - Min Cp	-0.50	-9.10	-3.66
Windward - Max Cp	-0.06	-3.46	1.98
Leeward Normal to Ridge	-0.57	-10.03	-4.58
Overhang Top (Windward)	-0.50	-6.38	-6.38
Overhang Top (Leeward)	-0.57	-7.31	-7.31
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02

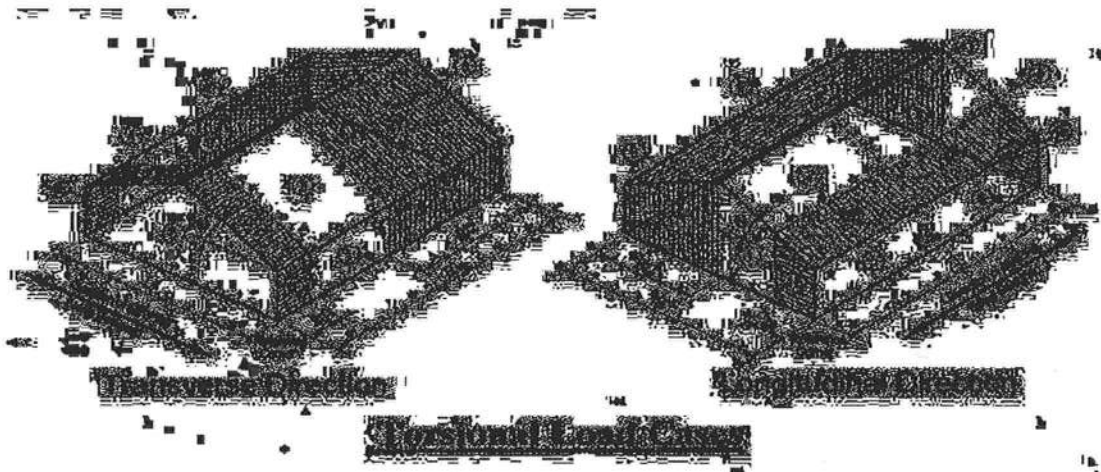
Dist from Windward Edge: 0 ft to 24 ft - Max Cp	-0.18	-5.04	0.41
Dist from Windward Edge: 0 ft to 6 ft - Min Cp	-0.90	-14.30	-8.85
Dist from Windward Edge: 6 ft to 12 ft - Min Cp	-0.86	-13.79	-8.34
Dist from Windward Edge: 12 ft to 20 ft - Min Cp	-0.54	-9.67	-4.22

\* Horizontal distance from windward edge

**Figure 6-10 - External Pressure Coefficients, GCpf**

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{(2/\alpha)} &= & 0.70 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 18.45 \\
 \text{Theta} &= \text{Angle of Roof} &= & 18.4 \text{ Deg}
 \end{aligned}$$

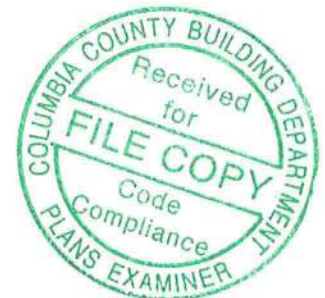


Wind Pressures on Main Wind Force Resisting System						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.52	0.18	-0.18	18.45	6.20	12.84
2	-0.69	0.18	-0.18	18.45	-16.05	-9.41
3	-0.47	0.18	-0.18	18.45	-11.96	-5.32
4	-0.42	0.18	-0.18	18.45	-10.98	-4.34
5	-0.45	0.18	-0.18	18.45	-11.62	-4.98
6	-0.45	0.18	-0.18	18.45	-11.62	-4.98
1E	0.78	0.18	-0.18	18.45	11.06	17.70
2E	-1.07	0.18	-0.18	18.45	-23.06	-16.42
3E	-0.67	0.18	-0.18	18.45	-15.73	-9.09
4E	-0.62	0.18	-0.18	18.45	-14.71	-8.07

\* p = qh \* (GCpf - GCpi)

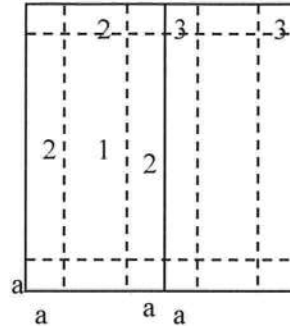
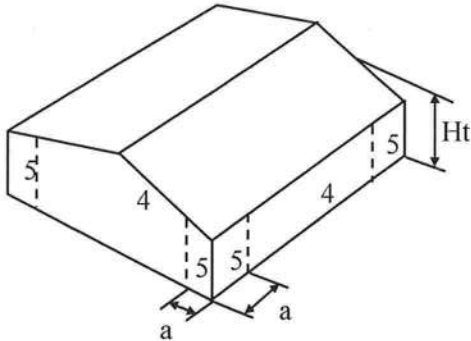
**Figure 6-11 - External Pressure Coefficients, GCp**

Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft



**WIND02 v2-21**

Detailed Wind Load Design (Method 2) per ASCE 7-02



Gabled Roof  
 $7 < \text{Theta} \leq 45$

a = 2 ==> 3.00 ft

Double Click on any data entry line to receive a help Screen

Component	Width (ft)	Span (ft)	Area (ft <sup>2</sup> )	Zone	GCp		Wind Press (lb/ft <sup>2</sup> )	
					Max	Min	Max	Min
Wall	10	1	10.00	4	1.00	-1.10	21.77	-23.61
Wall Exterior	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof	10	1	10.00	1	0.50	-0.90	12.54	-19.92
Roof Exterior	10	1	10.00	2	0.50	-1.70	12.54	-34.68
Roof Corner	10	1	10.00	3	0.50	-2.60	12.54	-51.28
	25	1	25.00	4	0.93	-1.03	20.47	-22.31
	25	1	25.00	4	0.93	-1.03	20.47	-22.31
	25	1	25.00	5	0.93	-1.26	20.47	-26.55
	25	1	25.00	1	0.42	-0.86	11.08	-19.19
	25	1	25.00	2	0.42	-1.50	11.08	-31.01
	25	1	25.00	3	0.42	-2.36	11.08	-46.88
			0.00					
			0.00					
			0.00					
			0.00					

Note: \* Enter Zone 1 through 5, or 1H through 3H for overhangs.



Shear Wall Design - Case A				
	Wall 1	Wall 2	Wall 3	Wall 4
Shear force @ wall (V)	1003.2	1003.2	1459	729.5
Full wall length	20	20	24	4
Shearwall Length	20	14	24	4
Percent Full-Height Sheathing Shearwall	100.00	70.00	100.00	100.00
Shear capacity adjustment, C <sub>o</sub> (table 2305.3.7)	1.00	0.77	1.00	1.00
SG	1	1	1	1
Shearwall rating (plf)	380	380	380	380
Shearwall with wind allowance	532	532	532	532
Allowable Shear Capacity	532	409.64	532	532
unit shear, v	50.16	71.66	60.79	182.38
pass?	yes	yes	yes	yes
shear and uplift between holddown, v and u (plf)	50.16	93.06	60.79	182.38
anchor bolts pass?	yes	yes	yes	no
T on hold-downs (lbf)	401.28	744.49	486.33	1459.00
hold-downs pass?	yes	yes	yes	no



Wall 4
729.5
4
4
100.00
1.00
1
380
532
532
182.38
yes
182.38
no
1459.00
no



Beam: **M1**

Shape: **3.5X11.25FS**

Material: **LVL**

Length: **16 ft**

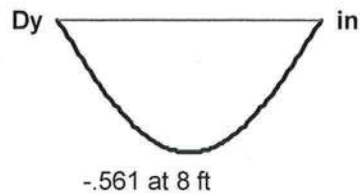
I Joint: **N1**

J Joint: **N2**

LC 3: (D+RLL)IBC 16-10 (a)

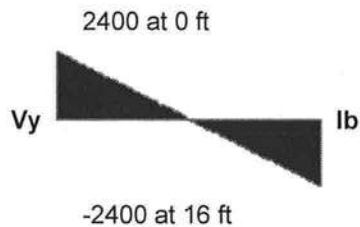
Code Check: **0.522 (bending)**

Report Based On 97 Sections



Dz \_\_\_\_\_ in

A \_\_\_\_\_ lb



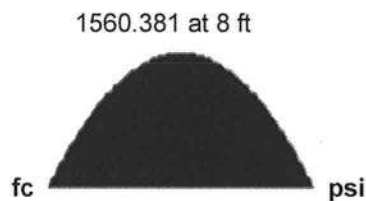
Vz \_\_\_\_\_ lb

T \_\_\_\_\_ lb-ft



My \_\_\_\_\_ lb-ft

fa \_\_\_\_\_ psi



### NDS 2005 Code Check

Max Bending Check **0.522**

Location **8 ft**

Equation **3.9-3**

CD **1.25** RB **13.279**

Cr **1** Cf<sub>u</sub> **1**

Max Shear Check **0.257 (y)**

Location **0 ft**

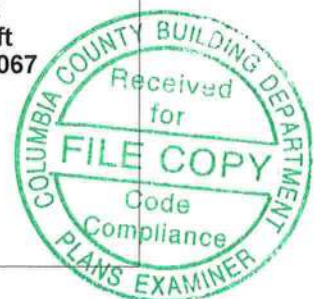
Max Defl Ratio **L/342**

CL **.921** CV **1**

CP **.197**

	(psi)	C <sub>m</sub>	C <sub>t</sub>	CF
F <sub>c</sub> '	185.041	1	1	1
F <sub>t</sub> '	1943.75	1	1	1
F <sub>b1</sub> '	2991.634	1	1	1
F <sub>b2</sub> '	3250	1	1	1
F <sub>v</sub> '	356.25	1	1	
E'	1.9e+6	1	1	

	Y-Y	Z-Z
Lb	16 ft	16 ft
le/d	54.857	17.067
Sway	No	No
Le-Bending Top	16 ft	
Le-Bending Bot	16 ft	



Company : GTC Design Group  
 Designer : Gary Gill  
 Job Number :

Iron Wood

Aug 12, 2008  
 12:06 PM  
 Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area (Me...	Surface (...)
1	Dead	DL						1	
2	Live Roof	RLL						1	

### Wood Design Parameters

	Label	Shape	Length[...]	le2[ft]	le1[ft]	le-bend top[ft]	le-bend b...	Kyy	Kzz	CV	Cr	y sway	z sway
1	M1	Garage Gi...	16										

### Load Combination Design

	Description	ASIF	CD	ABIF	Service	Hot Rolled	Cold Formed	Wood	Concrete	Footings
1	IBC 16-8		.9		Yes	Yes	Yes	Yes	Yes	Yes
2	IBC 16-9				Yes	Yes	Yes	Yes	Yes	Yes
3	(D+RLL)IBC 16-10 (a)		1.25		Yes	Yes	Yes	Yes	Yes	Yes
4	IBC 16-10 (b)		1.15		Yes	Yes	Yes	Yes	Yes	Yes
5	IBC 16-10 (c)		1.15		Yes	Yes	Yes	Yes	Yes	Yes
6	IBC 16-11 (a)		1.25		Yes	Yes	Yes	Yes	Yes	Yes
7	IBC 16-11 (b)		1.15		Yes	Yes	Yes	Yes	Yes	Yes
8	IBC 16-11 (c)		1.15		Yes	Yes	Yes	Yes	Yes	Yes
9	IBC 16-12 (a)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
10	IBC 16-12 (b)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
11	IBC 16-13 (a)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
12	IBC 16-13 (b)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
13	IBC 16-13 (c)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
14	IBC 16-13 (d)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
15	IBC 16-13 (e)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
16	IBC 16-13 (f)		1.6		Yes	Yes	Yes	Yes	Yes	Yes
17	IBC 16-14		1.6		Yes	Yes	Yes	Yes	Yes	Yes
18	IBC 16-15		1.6		Yes	Yes	Yes	Yes	Yes	Yes

### Member Area Loads

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
	No Data to Print ...						

### Wood Material Properties

	Label	Species	Grade	Cm	Emod	Nu	Therm (1/E...	Dens[k/ft^3]
1	DF Larch	Douglas Fir-Larch	No.1		1	.3	.3	.035
2	So Pine	Southern Pine	No.2		1	.3	.3	.035
3	S0 Pine SS	Southern Pine	Select Structural		1	.3	.3	.035
4	So Pine 2D	Southern Pine	No.2 Dense		1	.3	.3	.035
5	Glu-lam	24F-V3 SP/SP	na		1	.3	.3	.035
6	Cypress	BaldCypress	No.2		1	.3	.3	.035
7	LVL	MicroLam LVL	na		1	.3	.3	.035

### Member Wood Code Checks

	LC	Member	Shape	UC Max	Loc[ft]	Shear ...	Loc[ft]	Dir	Fc[psi]	Ft[psi]	Fb1[psi]	Fb2[psi]	Fv[psi]	RB	CL	CP	Egn
1	3	M1	3.5X11.2...	522	8	257	0	v	185.041	1943.75	2991....	3250	356.25	13.279	921dr	197	8.9.3

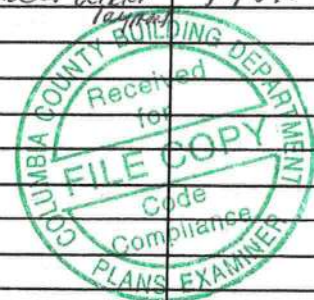


App # 0809-04

**PRODUCT APPROVAL SPECIFICATION SHEET**Location: 793 SW Nautilus Rd Project Name: Miera

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
<b>A. EXTERIOR DOORS</b>			
1. Swinging	Masonite	metal edge steel door	19.1
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
<b>B. WINDOWS</b>			
1. Single hung			
2. Horizontal Slider	1		
3. Casement			
4. Double Hung	Better Built	Fin Frame 52x72 model 78/3010	1463.13
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
<b>C. PANEL WALL</b>			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
<b>D. ROOFING PRODUCTS</b>			
1. Asphalt Shingles	Tamko	Glas-seal AR-3 Tabs	1456.1
2. Underlayments	Tamko	Master Smooth-asphalt under	1481.1
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			



Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives - Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
<b>E. SHUTTERS</b>			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
<b>F. SKYLIGHTS</b>			
1. Skylight			
2. Other			
<b>G. STRUCTURAL COMPONENTS</b>			
1. Wood connector/anchor	Simon Strong	see front of plans	
2. Truss plates	Alpine	CC46	
3. Engineered lumber			1218.13
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
<b>H. NEW EXTERIOR ENVELOPE PRODUCTS</b>			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

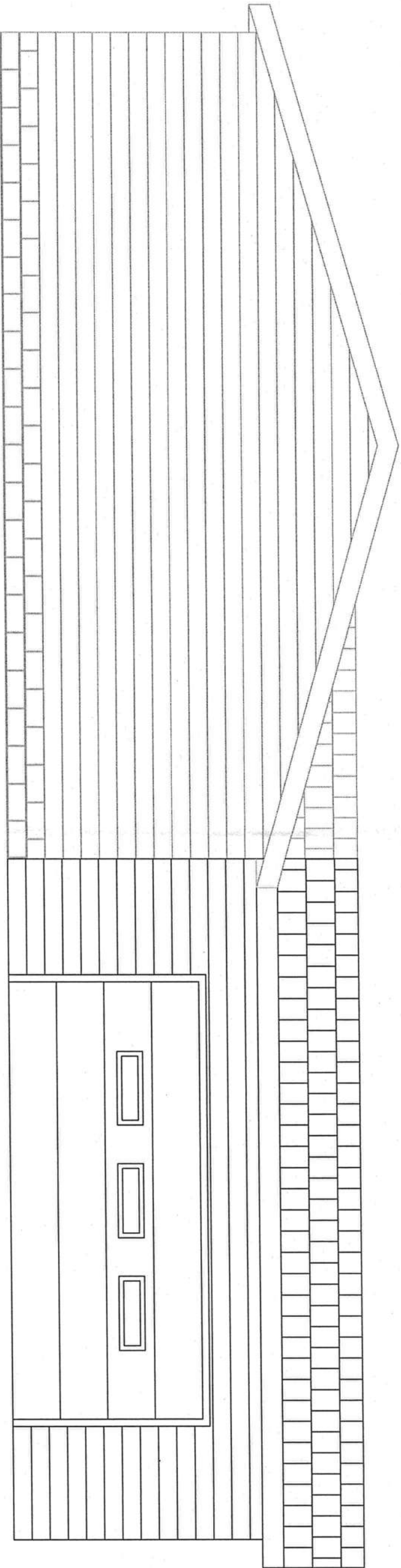
Wendy Brennell  
Contractor or Contractor's Authorized Agent Signature  
793 SW Nautilus  
Location

Wendy Brennell 9/9/08  
Print Name Date

Permit # (FOR STAFF USE ONLY)

# IRONWOOD HOMES

## MIRA GARAGE ADDITION



PRODUCT APPROVAL SPECIFICATION TABLE

PRODUCT APPROVAL SPECIFICATION TABLE				
CATEGORY / SUBCATEGORY	MANUFACTURER	PROJECT DESCRIPTION	APPROVAL NUMBERS	
EXTERIOR DOORS				
SWINGING	MASONITE INTERNATIONAL	METAL EDGE STEEL DOOR UNITS	19.1	
WINDOWS				
DOUBLE HUNG	BETTERBILT	FIN FRAME 52x72 MODEL 740/3740	663.13	
ROOFING PRODUCTS				
ASPHALT SHINGLES	TAKKO	GLAS-SEAL AR - 3 TAB	1956.1	
UNDERLAYMENTS	TAKKO	MASTER SMOOTH -ASPHALT UNDERLAYMENT	1481.1	
STRUCTURAL COMPONENTS				
WOOD CONNECTORS	SIMPSON STRONG-TIE			
	-	CS16	1901.4	
	-	SPH6	538.35	
	-	H10	474.96	
	-	HD2A	503.10	
	-	H6	474.119	
TRUSS PLATES	ALPINE	CC46	1218.13	
		IUT14	474.216	

### NOTE:

NEW GARAGE ADDITION IS SELF SUPPORTING  
& SHALL NOT BE STRUCTURALLY CONNECTED TO EXISTING MOBILE HOME.

CONNECTION LISTING

MANUFACTURER	MODEL #	FASTENER COUNT	ALLOWABLE LOAD
SIMPSON STRONG TIE CO.	HD2A	(2) 3/8 A.B.	2775
SIMPSON STRONG TIE CO.	H10	(12) 10d NAILS	905

SHEET LIST TABLE

Sheet Number	Sheet Title	Rev #
T-1.0	TITLE SHEET	
S-1.0	FOUNDATION PLAN	
S-2.0	ROOF FRAMING PLAN	
S-3.0	BUILDING SECTIONS	
S-4.0	WALL SECTIONS	
S-5.0	FRAMING DETAILS	
S-6.0	GENERAL NOTES	
A-1.0	FLOOR PLAN	
E-1.0	ELEVATIONS	

IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA



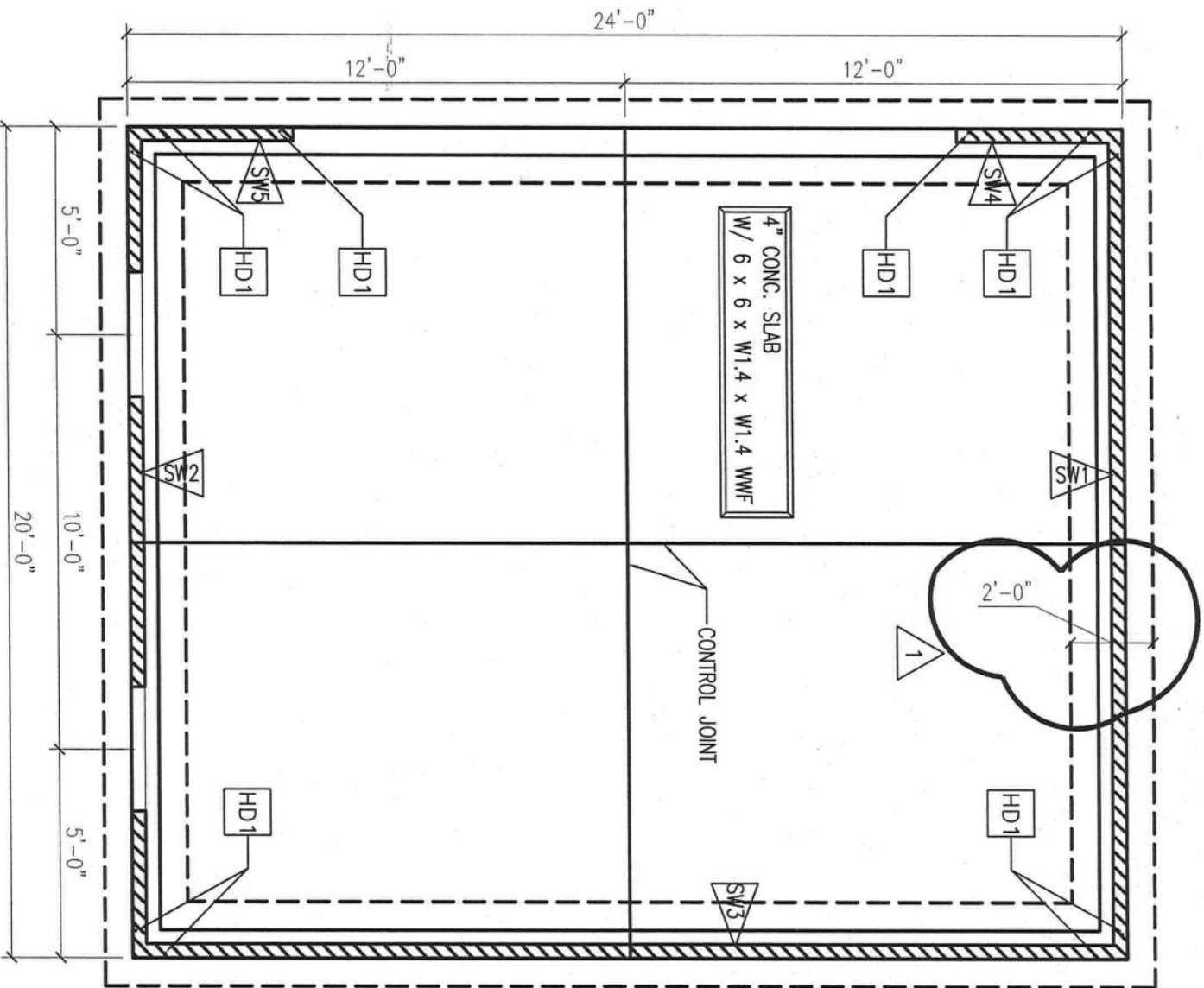
P.O. Box 187  
130 West Howard Street  
Live Oak FL 32064  
Phone: (386) 362-3678  
Fax: (386) 362-6133  
Gary J. Gill, PE 51942  
Auth. # 9461

REV #	DATE	REVISION NOTES

TITLE SHEET

T-1.0

9-Sep-08

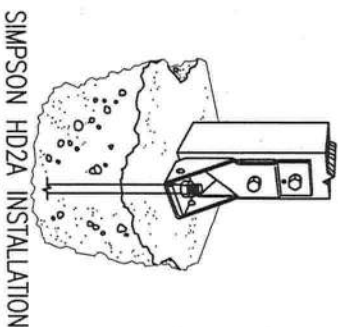


FOUNDATION PLAN  
1/4" = 1'

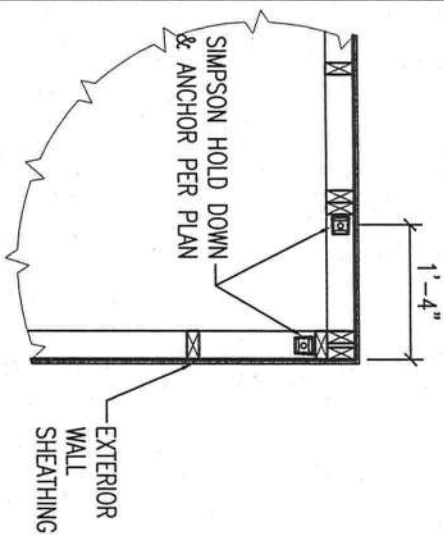
SHEARWALL SCHEDULE				
MARK	NAILING PATTERN	HOLD DOWN REQUIREMENT	ANCHOR BOLTS SPACING	REMARKS
SW1	8d NAILS @ 4" O.C. EDGES AND 8" O.C. FIELD	SIMPSON HD2A W/ 5/8" A.B. @ EA. END OF SHEAR	1/2"Ø @ 48" O.C.	
SW2	8d NAILS @ 4" O.C. EDGES AND 8" O.C. FIELD	SIMPSON HD2A W/ 5/8" A.B. @ EA. END OF SHEAR	1/2"Ø @ 48" O.C.	
SW3	8d NAILS @ 4" O.C. EDGES AND 8" O.C. FIELD	SIMPSON HD2A W/ 5/8" A.B. @ EA. END OF SHEAR	1/2"Ø @ 48" O.C.	
SW4	8d NAILS @ 4" O.C. EDGES AND 8" O.C. FIELD	SIMPSON HD2A W/ 5/8" A.B. @ EA. END OF SHEAR	1/2"Ø @ 48" O.C.	
SW5	8d NAILS @ 4" O.C. EDGES AND 8" O.C. FIELD	SIMPSON HD2A W/ 5/8" A.B. @ EA. END OF SHEAR	1/2"Ø @ 48" O.C.	

\*NOTES:

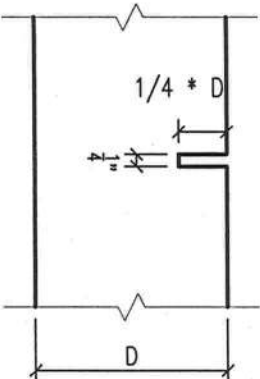
- (1) HD1 = SIMPSON HD2A HOLD DOWN
- (2) CONTRACTOR SHALL VERIFY ALL FOUNDATION DIMENSIONS PRIOR TO CONSTRUCTION. IF A DIMENSION CONFLICT OCCURS BETWEEN FLOOR PLAN AND THE FOUNDATION PLAN, THE FLOOR PLAN SHALL CONTROL.



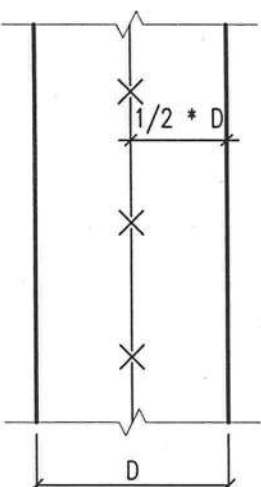
SIMPSON HD2A INSTALLATION



TYPICAL HOLD DOWN DETAIL  
1/2" = 1'



TYPICAL JOINT DETAIL  
3" = 1'



TYPICAL SLAB DETAIL  
3" = 1'

ISSUED FOR CONSTRUCTION

9/19/08

FOUNDATION &  
SHEAR WALL PLAN

IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA

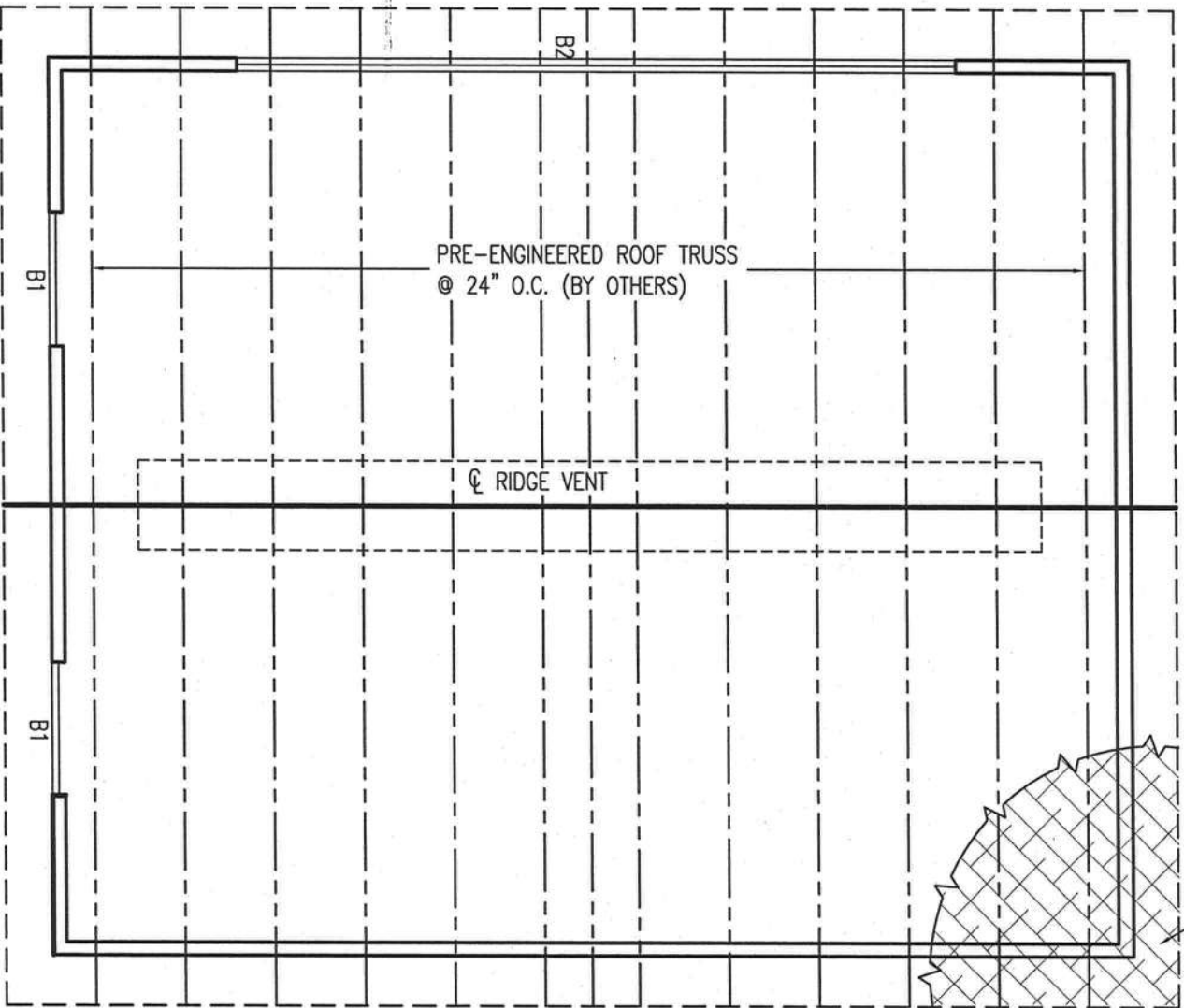


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Auth. # 9461

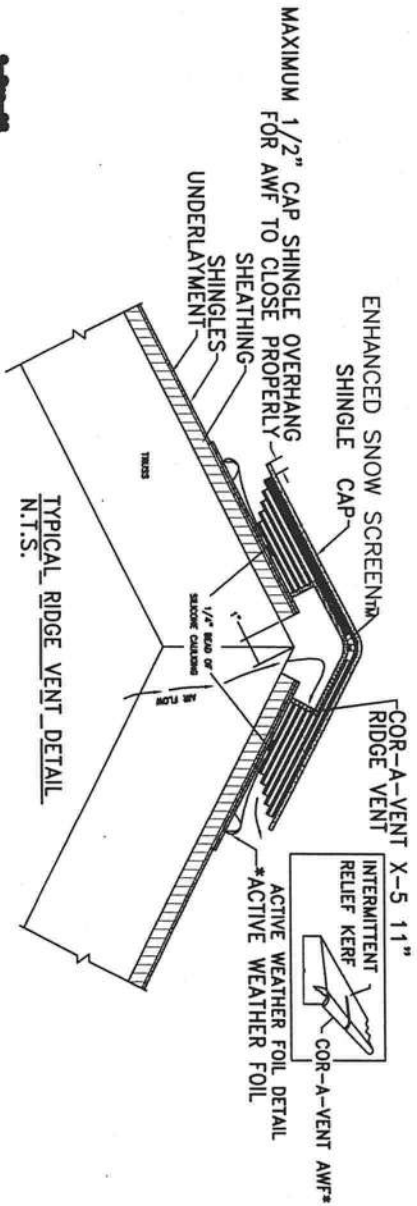
S-1.0

PROJECT NUMBER  
F08-146  
DRAWN BY  
F. VALENCH  
CHECKED BY  
08

SLOPED ROOF SHEATHING SHALL BE NAILED PER  
ROOF FASTENING ZONE SCHEDULE (THIS SHEET)

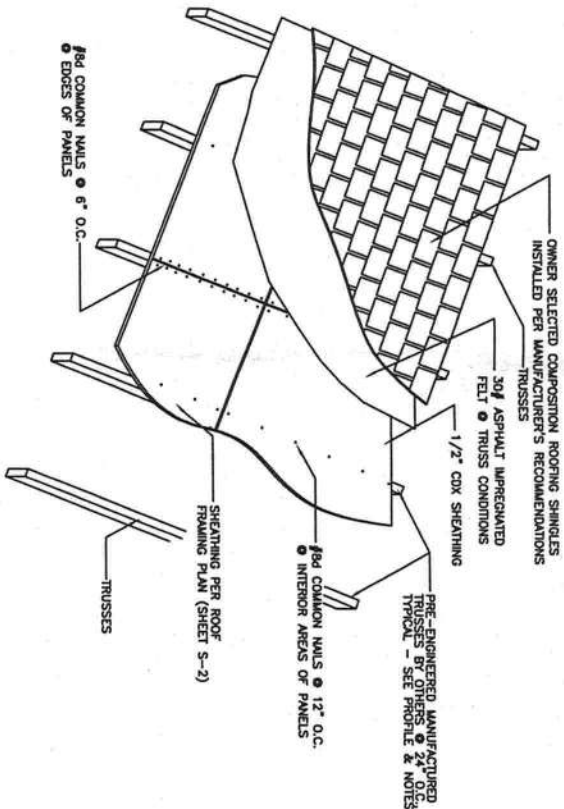


ROOF FRAMING PLAN  
1/4" = 1'

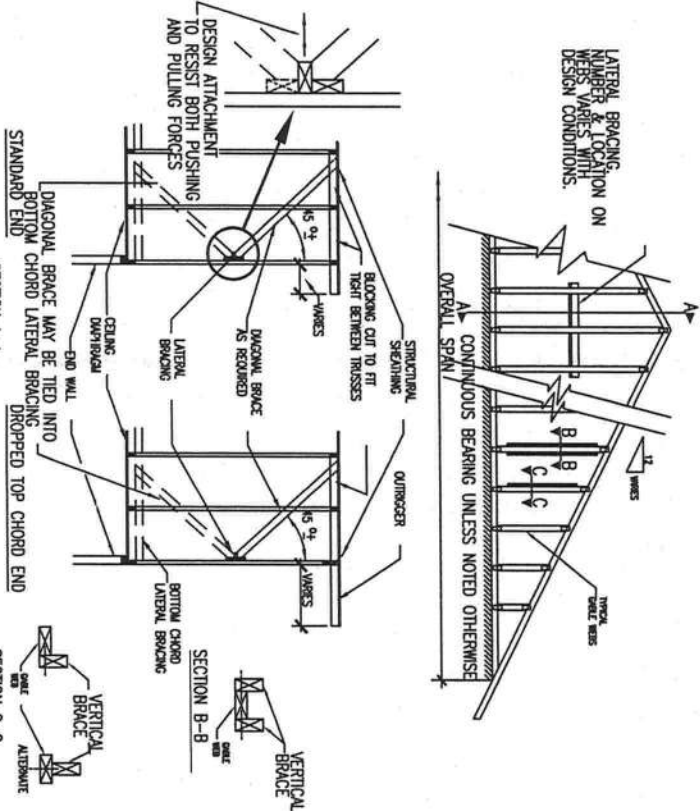


- PLAN NOTES:
1. TYPICAL ROOF TRUSS BEARING @ ELEVATION 9'-4" UNLESS OTHERWISE NOTED ON PLANS. USE 3/4" APA 708 C.D. EXP. 1 PLYWOOD DECK ON ALL ROOF COMPONENTS.
  2. COORDINATE LOCATION OF ALL DOORS AND WALL OPENINGS WITH THE ARCHITECTURAL DRAWINGS.
  3. ASPHALT SHINGLES SHALL BE CLASSIFIED IN ACCORDANCE WITH ASTM D3161 CLASS F OR ASTM D7158 CLASS G OR TAS 107

BEAM SCHEDULE	
I.D.	DESCRIPTION
B1	(2) 2 x 10 W/ 1/2" PLYWOOD
B2	3 1/2" x 11 1/4" LVL Fb MIN. = 2600



ROOFING & SHEATHING CONNECTIONS TO TRUSSES

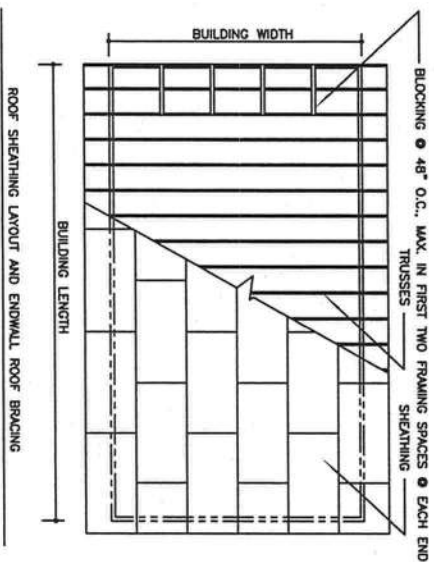


- NOTES:
- 1) ACTUAL BRACING REQUIREMENTS WILL VARY DUE TO WIND LOAD, CODE CRITERIA, BUILDING HEIGHT, TRUSS SPAN, WEB LUMBER GRADE/SPECIES/ON CENTER SPACING AND OTHER VARIABLES. BRACING (AND ATTACHMENT) REQUIREMENTS SHOULD BE DESIGNED FOR EACH SPECIFIC JOB.
  - 2) CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL, AS WELL AS THE DESIGN AND SPECIFICATION OF TEMPORARY AND PERMANENT BRACING OF THE ROOF SYSTEM IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

3	2	2	3
2	1	2	1
3			3

ROOF FASTENING PLAN  
1/4" = 1'

ROOF FASTENING ZONE SCHEDULE					
WIND VELOCITY (3-SECOND GUST)	PANEL LOCATION	MAIN ROOF	1	2	3
FASTENING SCHEDULE (INCHES ON CENTER)					
120 MPH	SUPPORTED PANEL	6	6	6	6
	END AND EDGES	12	6	4	6
	PANEL FIELD			3	6



ISSUED FOR  
CONSTRUCTION

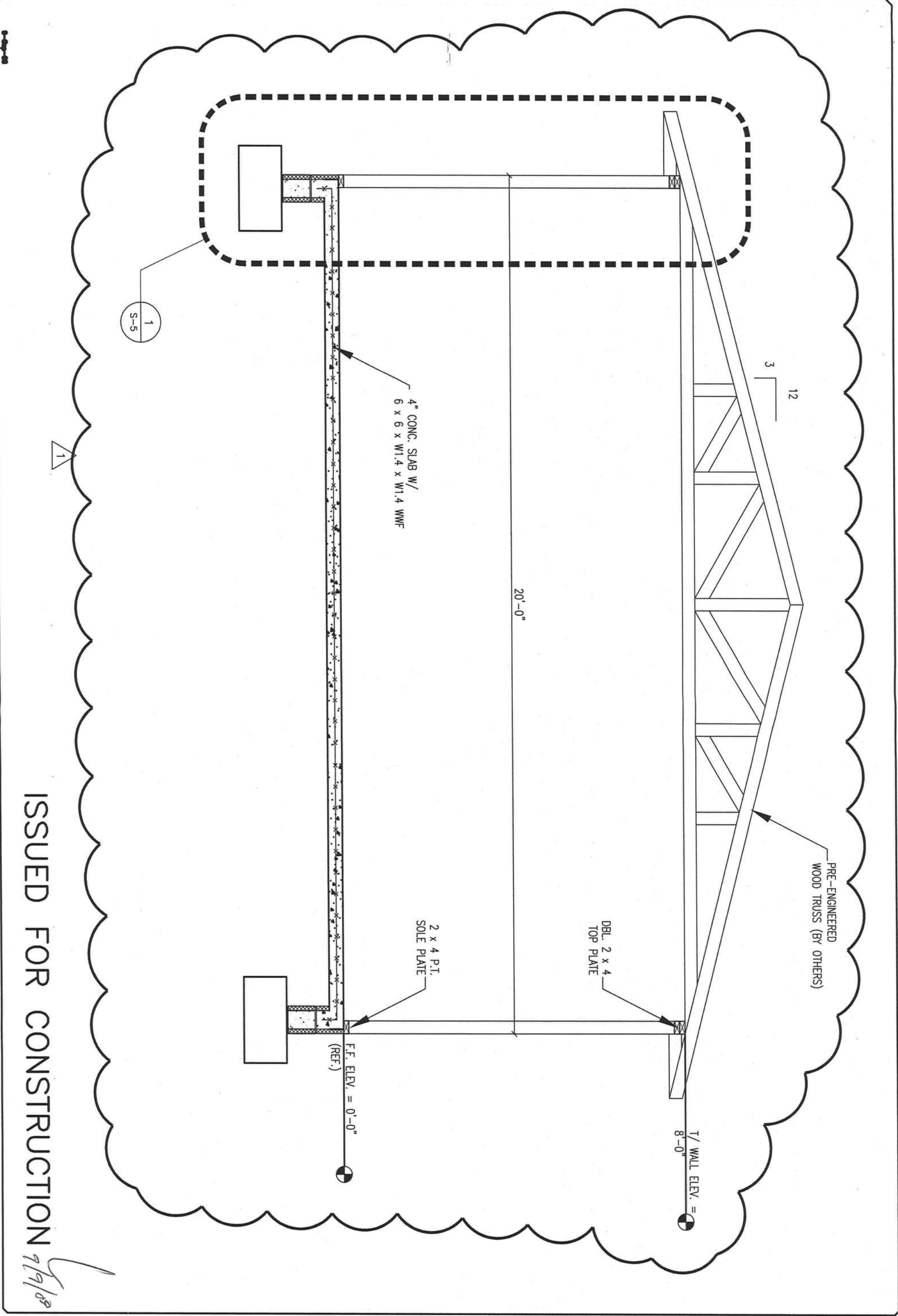


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IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA

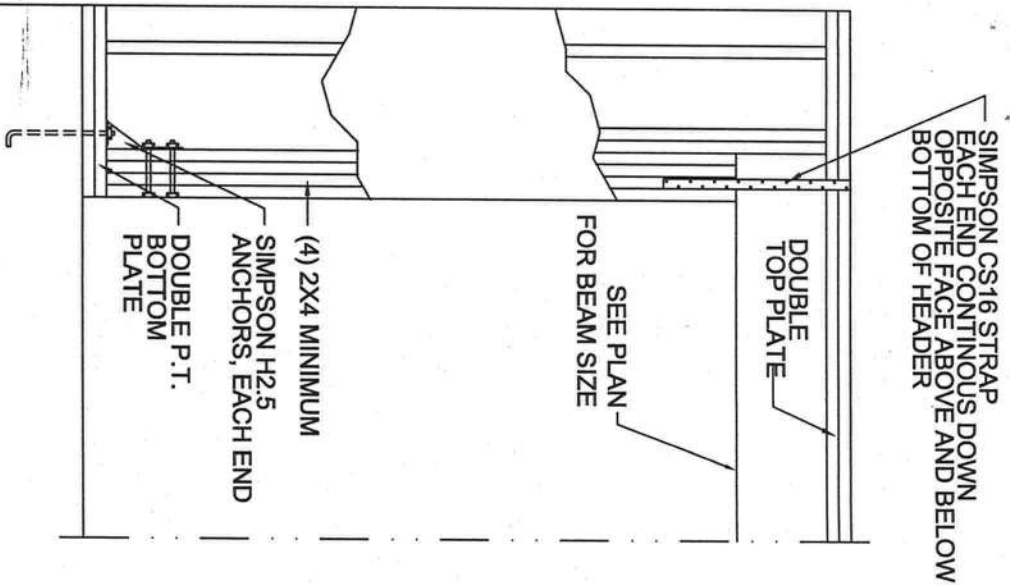
ROOF FRAMING PLAN

PROJECT NUMBER:  
PT08-146  
DRAWN BY:  
F. WALECH  
CHECKED BY:  
08  
S-2.0

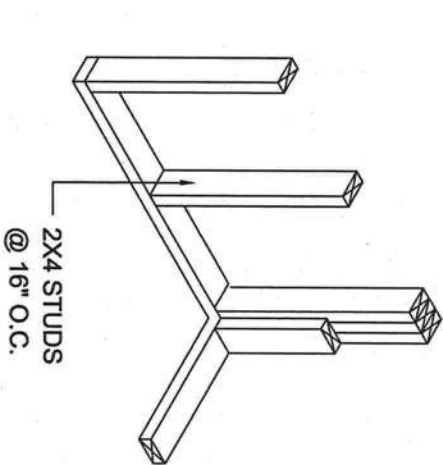


ISSUED FOR CONSTRUCTION

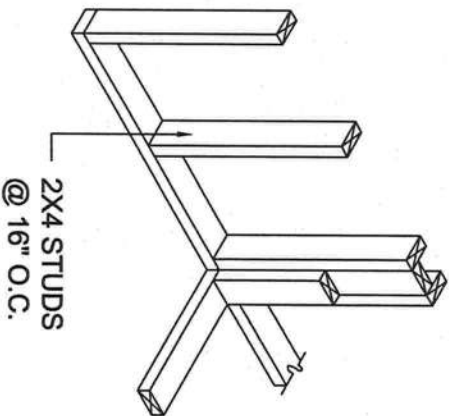
7/9/08



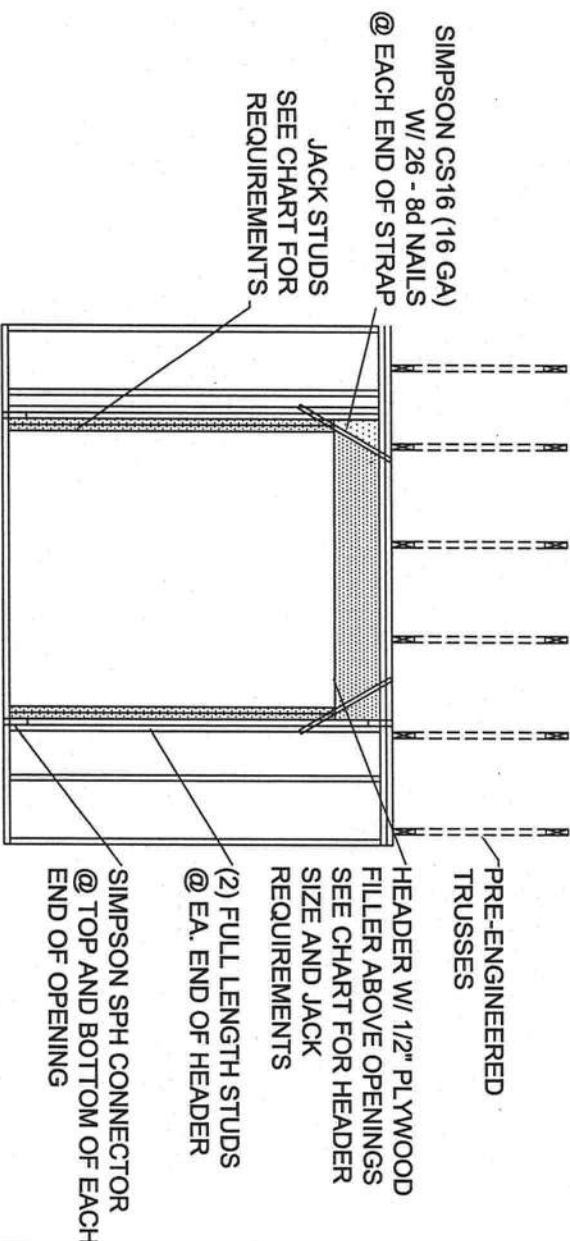
GARAGE HEADER DETAIL



WALL CORNER

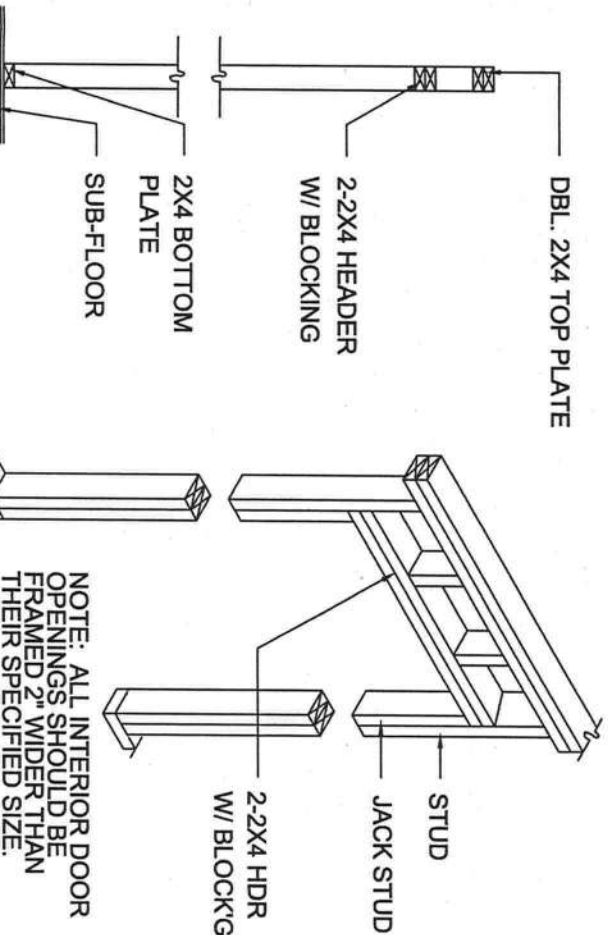


WALL INTERSECTION

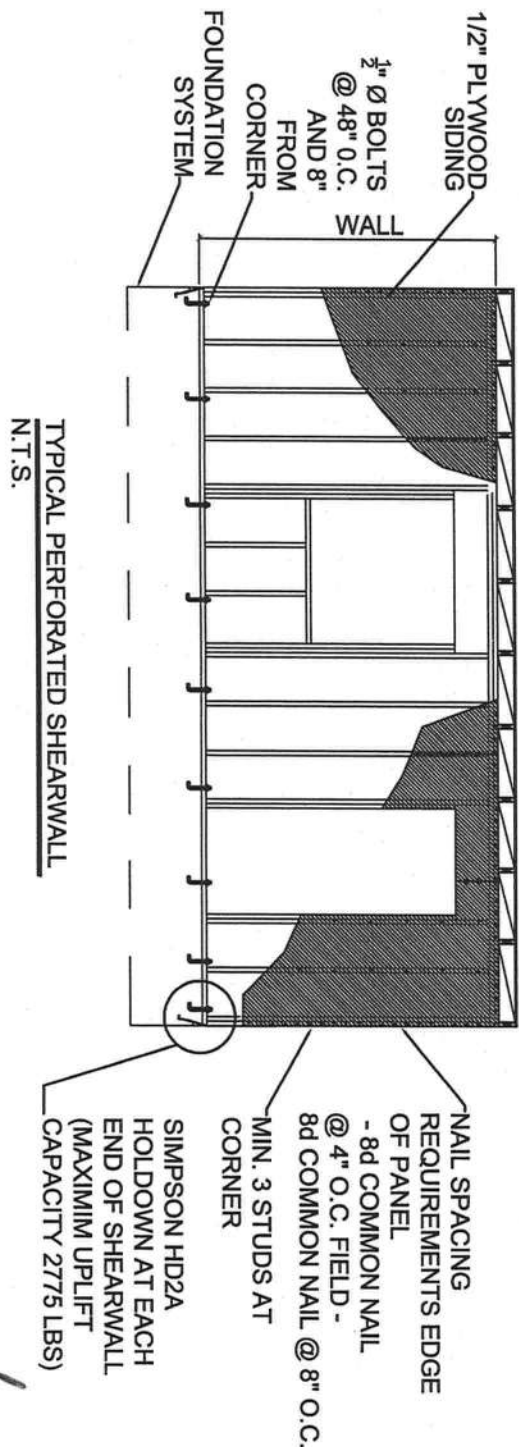


TYP. FRAMING & UPLIFT  
CONNECTIONS FOR OPENINGS

SUPPORTING:	HEADER SIZE	BUILDING WIDTH (FT)					
		20'		26'		36'	
SPAN	# JACKS	SPAN	# JACKS	SPAN	# JACKS	SPAN	# JACKS
2-2x4	3-6"	1	3-2"	1	2-10"	1	1
2-2x6	6-6"	1	4-6"	1	4-2"	1	1
2-2x6	6-10"	1	5-11"	2	5-4"	1	1
2-2x6	8-6"	2	7-9"	2	6-6"	2	2
2-2x12	9-6"	2	8-6"	2	7-6"	2	2
3-2x6	8-4"	1	7-6"	1	6-8"	1	1
3-2x10	10-6"	1	9-1"	2	8-2"	1	1
3-2x12	12-2"	2	10-7"	2	9-5"	2	2
4-2x6	8-2"	1	8-4"	1	8-2"	1	1
4-2x10	11-6"	1	10-6"	1	9-5"	1	1
4-2x12	14-1"	1	12-2"	2	10-11"	1	1



NON-BEARING WALL HEADER



TYPICAL PERFORATED SHEARWALL  
N.T.S.

ISSUED FOR CONSTRUCTION

DESIGN CRITERIA

DESIGN PER 2004 W/ 2006 & 2007 REVISIONS FLORIDA BUILDING CODE, UNLESS OTHERWISE NOTED.

Table with 2 columns: Category (Live Loads, Roofs and Canopies, etc.) and Value (0 to 200 SF, 160PSF, etc.).

WIND LOADS:

Table with 2 columns: Parameter (Basic Wind Speed, Wind Importance Factor, etc.) and Value (110 MPH, 1.0, etc.).

THIS BUILDING IS NOT LOCATED IN THE WIND BORNE DEBRIS REGION. IMPACT RESISTANT GLAZING IS NOT REQUIRED.

DESIGN WIND PRESSURES FOR COMPONENTS & CLADDING:

Table with 4 columns: Tributary Area, End Zone, Interior Zone, and Linearly Interpolate Between Stated Values.

CONCRETE (DESIGN PER CURRENT EDITION ACI 318)

Table with 2 columns: Item (Slab on Grade, Footings, etc.) and Value (FC= 3000 PSI, etc.).

ALL REINFORCING STEEL, ASTM A615 GRADE 60

ALL WELDED WIRE FABRIC, ASTM A195

CONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 301)

Table with 2 columns: Item (Compressive Strength, etc.) and Value (Fm= 1500 PSI, etc.).

STRUCTURAL STEEL (DESIGN PER CURRENT EDITION AISC), UNLESS OTHERWISE NOTED MATERIALS SHALL BE AS FOLLOWS:

Table with 2 columns: Item (W-shapes, Other Shapes & Plates, etc.) and Value (ASTM A992, etc.).

Table with 2 columns: Item (Soil Bearing (Design Maximum), Foundations) and Value (1000PSF, etc.).

IF FOOTING EVALUATIONS SHOWN OCCUR IN A DISTURBED, UNSTABLE, OR UNSUITABLE SOIL, THE ENGINEER SHALL BE NOTIFIED.

STEPS IN WALL FOOTINGS SHALL NOT EXCEED A SLOPE OF (1) VERTICAL TO TWO (2) HORIZONTAL

PROVIDE A MINIMUM OF TWO #4 BARS IN TOP OF CONTINUOUS WALL FOOTINGS AT DOOR AND OTHER OPENINGS.

4'-0" LONGER THAN THE OPENING.

GENERAL NOTES

CONCRETE

UNLESS OTHERWISE NOTED ON THE DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:

Table with 2 columns: Item (Footings, Pile Caps, etc.) and Value (SEE TYPICAL DETAIL, etc.).

ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 318 DURING THE PLACEMENT OF CONCRETE.

UNLESS OTHERWISE NOTED, SPICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS FOLLOWS:

Table with 2 columns: Item (Welded Wire Fabric, Reinforcing Bars) and Value (WIRE SPACING PLUS 6", 40 BAR DIAMETERS).

ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE NOTED.

TRUSS FASTENER SCHEDULE

Table with 5 columns: Location, Uplift, Fastener (L), Truss, Plate.

NOTES:

- 1) ALL CONNECTORS LISTED ARE SIMPSON STRONG-TIE, UNLESS OTHERWISE NOTED.
- 2) TRUSS ENGINEER MAY PROVIDE ALTERNATE CONNECTIONS.

SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

EMBEDMENT FOR EXPANSION BOLTS SHALL BE 3 1/2" MINIMUM FOR 3/4" BOLTS IN CONCRETE, 5 1/2" IN ROUTED MASONRY, HILTI HMK BOLT IT OR EQUAL.

EXPLOY GROUT SHALL BE POWER FIST CARTRIDGE SYSTEM BY BAYM. HYDRO CARTRIDGE SYSTEM BY HILTI. (HILTI BESSO). IF HOLE IS CORED INSTEAD OF DRILLED) OR APPROVED EQUAL. UNLESS OTHERWISE NOTED, 12 BAR DIAMETERS MINIMUM. HOLE SIZES SHALL BE 1/2" LARGER THAN REBAR SIZE AND 1/2" LARGER THAN TREADED ROD SIZE. HOLE SHALL BE BRUSHED OUT WITH BOTTLE BRUSH AND THEN BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURERS PRINTED INSTRUCTIONS.

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE OF THE PROJECT.

GENERAL CONTRACTOR MUST REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT/ENGINEER. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTORS SHOP DRAWING STAMP OR HAVE BEEN MERELY "RUBBER STAMPED" SHALL BE RETURNED WITHOUT REVIEW.

CHANGES TO THE CONTRACT DOCUMENTS SHALL BE CLOUDED ON SHOP DRAWINGS OR REQUESTED IN WRITING. THE CONTRACTOR IS LIABLE FOR ANY DEVIATIONS UNLESS REVIEWED AND ACKNOWLEDGED BY THE ENGINEER. SHOP DRAWING SUBMITTALS SHALL ONLY BE CHECKED FOR CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS.

SPECIFICATIONS

CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION), EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS.

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS", AISC SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A528 OR A588 BOLTS, AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, AND AISC D1.1 "STRUCTURAL WELDING CODE", EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.

A GEOTECHNICAL TESTING AND INSPECTION FIRM SHALL BE EMPLOYED TO PERFORM A SOIL SURVEY FOR SATISFACTORY SOIL MATERIALS, SAMPLING AND TESTING FOR QUALITY CONTROL, AS PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT FOR THIS PROJECT. ALL EARTHWORK OPERATIONS SHALL BE PERFORMED TO THE SATISFACTION OF THE GEOTECHNICAL TESTING FIRM.

TERMITE PROTECTION NOTES:

SOIL CHEMICAL BARRIER METHOD:

- 1. A PERMANENT SIGN WHICH IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHALL BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL. FBC 104.2.6
- 2. CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALLS. FBC 1603.4.4
- 3. IRRIGATION/SPRINKLER SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" FROM BUILDING SIDE WALLS. FBC 1603.4.4
- 4. TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION, BETWEEN WALL COVERINGS AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6". EXCEPTION: PAINT AND DECORATIVE CEMENTIOUS FINISH LESS THAN 5/8" THICK ADHERED DIRECTLY TO THE FOUNDATION WALL. FBC 1403.1.6
- 5. INITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE. FBC 1816.1.1
- 6. SOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLUDING SPACES BOXED OR FORMED. FBC 1816.1.2
- 7. BOXED AREAS IN CONCRETE FLOOR FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS. PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ELIMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT. FBC 1816.1.3
- 8. MINIMUM 6 MIL VAPOR RETARDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. RETREATMENT SHALL BE REMOVED ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR SOIL TREATMENT. FBC 1816.1.5
- 10. SOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS. FBC 1816.1.6
- 11. AN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL BE RETREATED. FBC 1816.1.6
- 12. ALL BUILDINGS ARE REQUIRED TO HAVE PER-CONSTRUCTION TREATMENT. FBC 1816.1.7
- 13. A CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES". FBC 1816.1.7
- 14. AFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STACKS, TUB TRAP BOXES, FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL. FBC 2303.1.3
- 15. NO WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0" OF ANY BUILDING OR PROPOSED BUILDING. FBC 2303.1.4

ISSUED FOR CONSTRUCTION



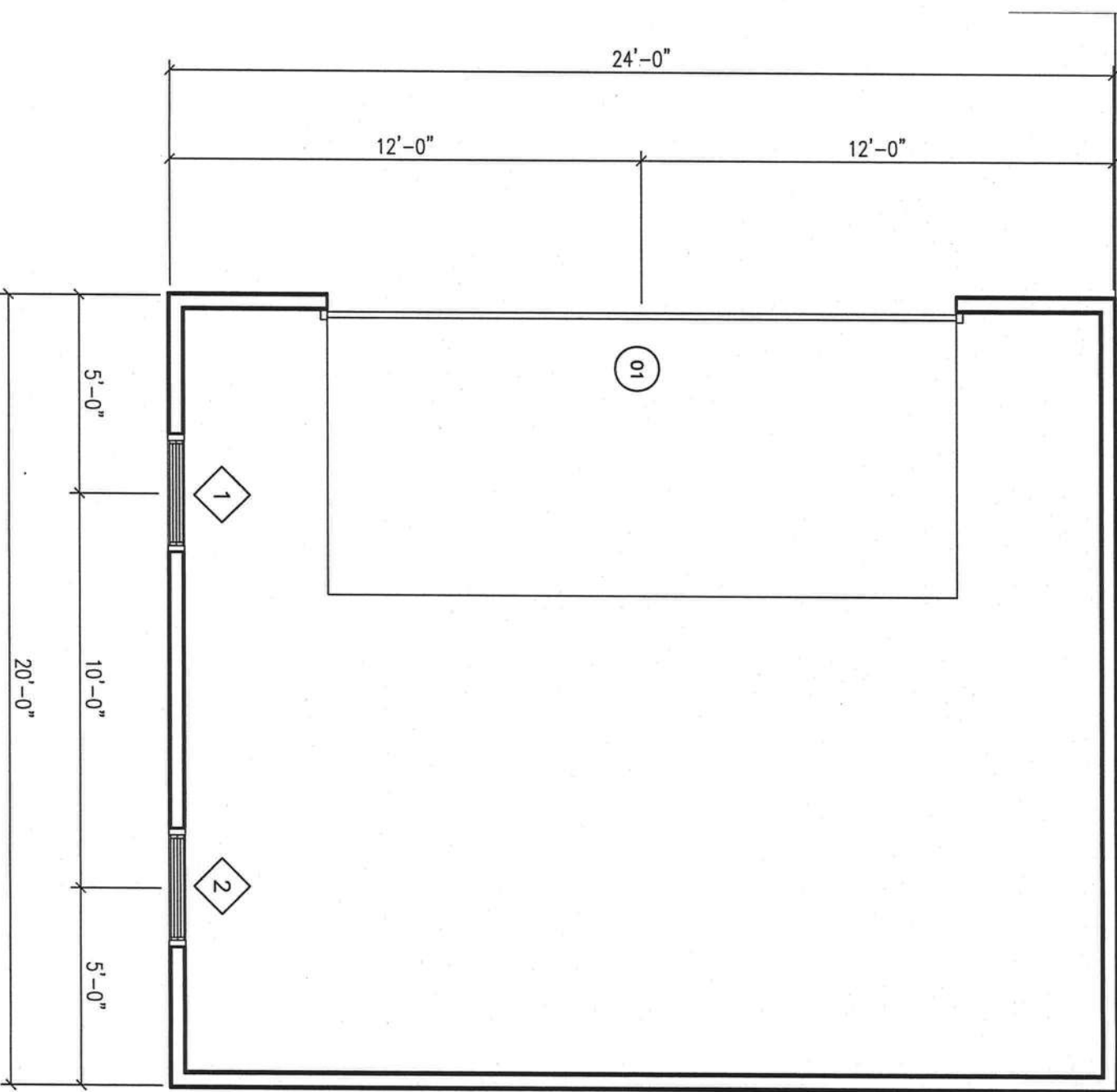
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Gary J. Gill, PE  
Auth. # 9461

IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA

GENERAL NOTES

PROJECT NUMBER: P008-146  
DESIGNED BY: F. VULTECH  
CHECKED BY: GS  
S-6.0

EXISTING STRUCTURE



DOOR SCHEDULE					
DOOR				NOTES	
MARK	SIZE				
	WD	HGT	THK		MATL
1	16'-0"	7'-0"	1 3/4"	WFL	

WINDOW SCHEDULE						
MARK		SIZE		TYPE	MATERIAL	NOTES
1	2	WIDTH	HEIGHT			
1	2	3'-0"	3'-0"	DOUBLE FRAME	WOOD	---
2	1	3'-0"	3'-0"	DOUBLE FRAME	WOOD	---

IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA



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Phone: (386) 362-3678  
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Gary J. Gill, PE  
Auth. # 9461

FLOOR PLAN

PROJECT NUMBER  
P008-146

DESIGNED BY  
F. VALENCH

CHECKED BY  
06

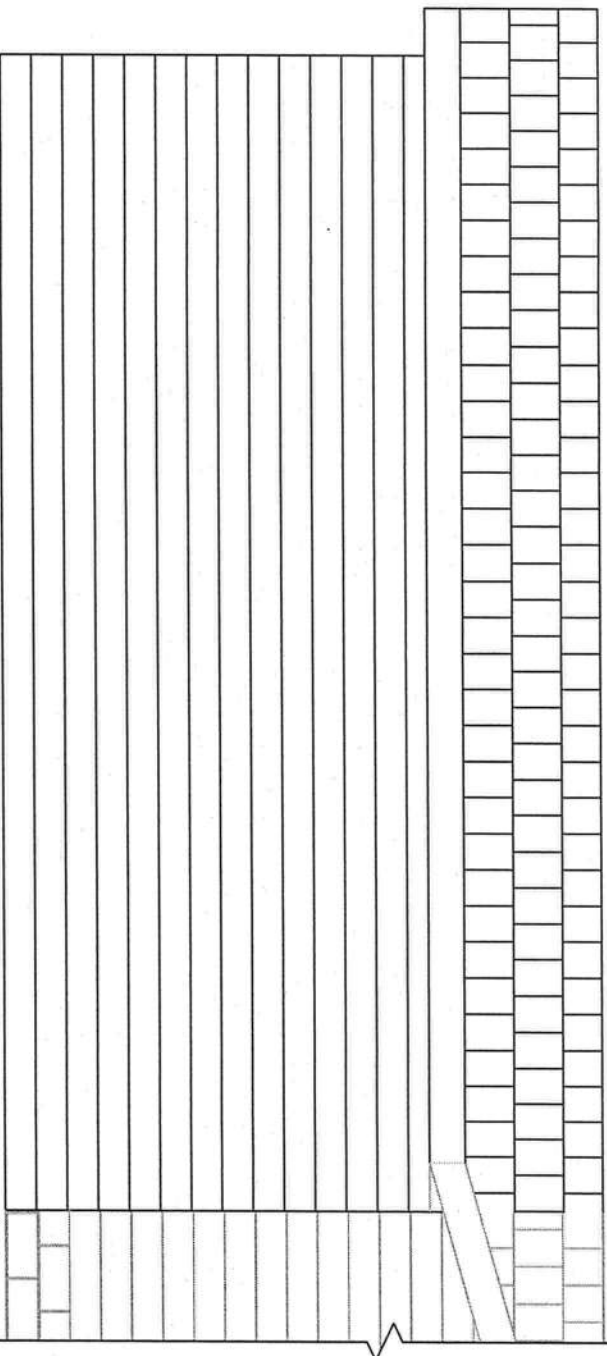
A-1.0

ISSUED FOR CONSTRUCTION

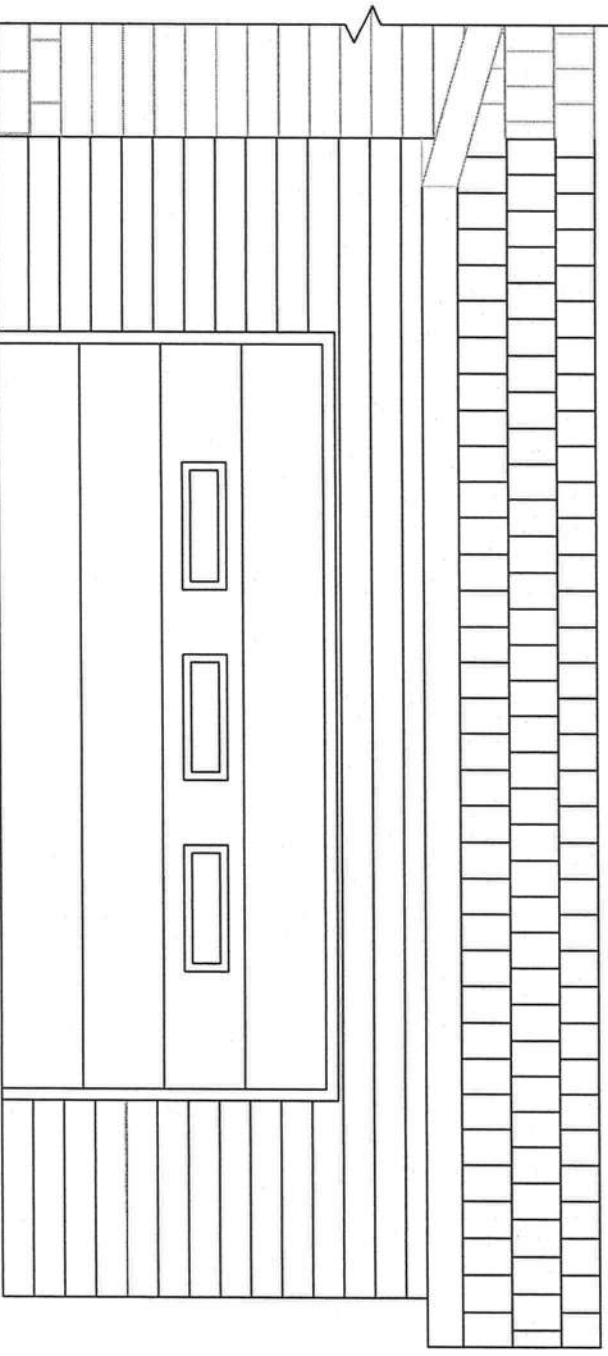
9/9/08

FLOOR PLAN

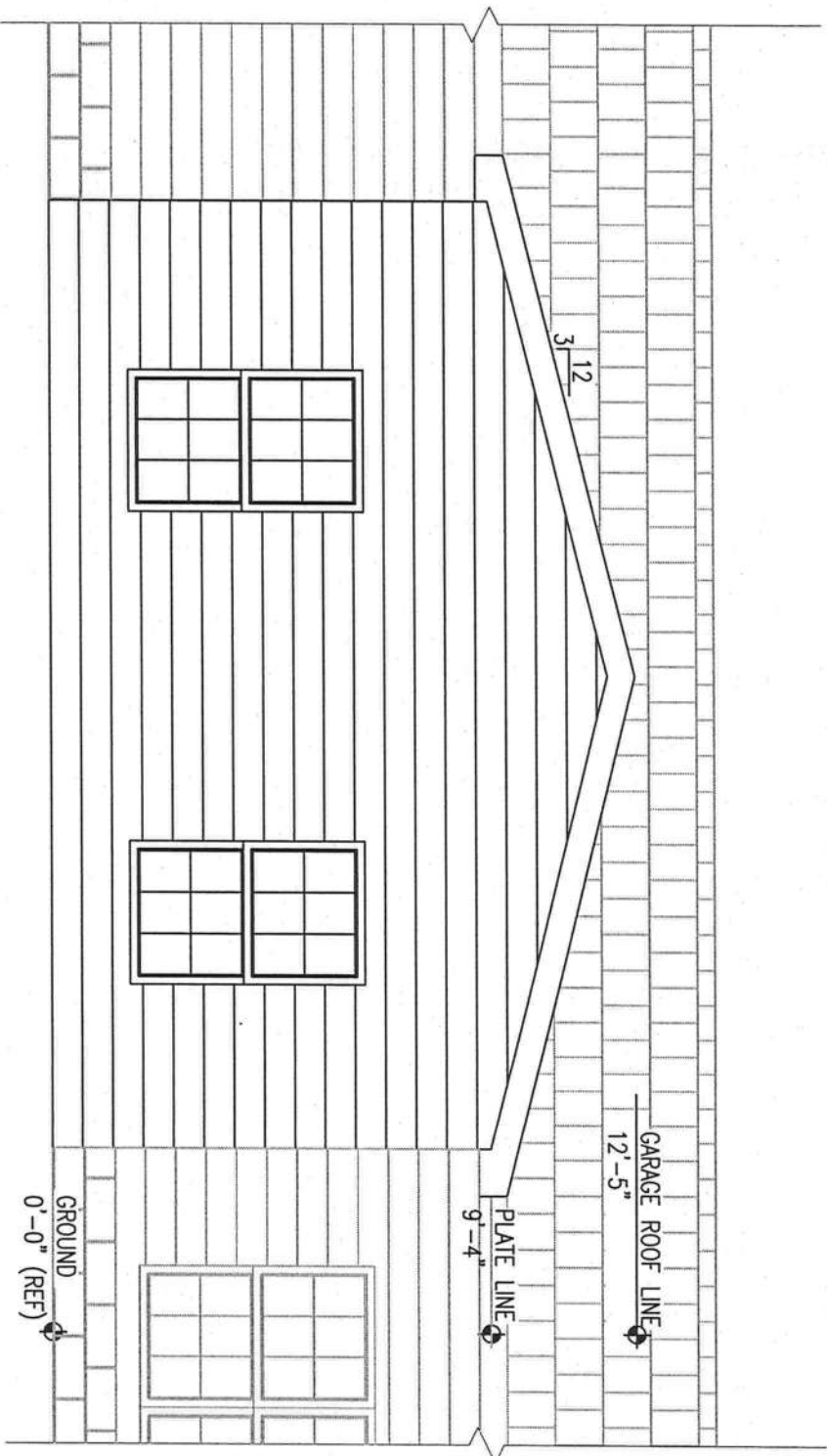
1/4" = 1'



EAST ELEVATION  
1/4" = 1'



WEST ELEVATION  
1/4" = 1'



NORTH ELEVATION  
1/4" = 1'

ISSUED FOR CONSTRUCTION

ELEVATIONS

IRONWOOD HOMES  
MIRA GARAGE ADDITION  
FLORIDA



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PROJECT NUMBER  
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OWNER  
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