

REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

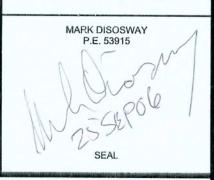
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building at specified location. In case of conflict structural requirements, scope of work, and builder responsibilities on sheet S-1 control.



Rick Bicknell Residence

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June 30, 2005

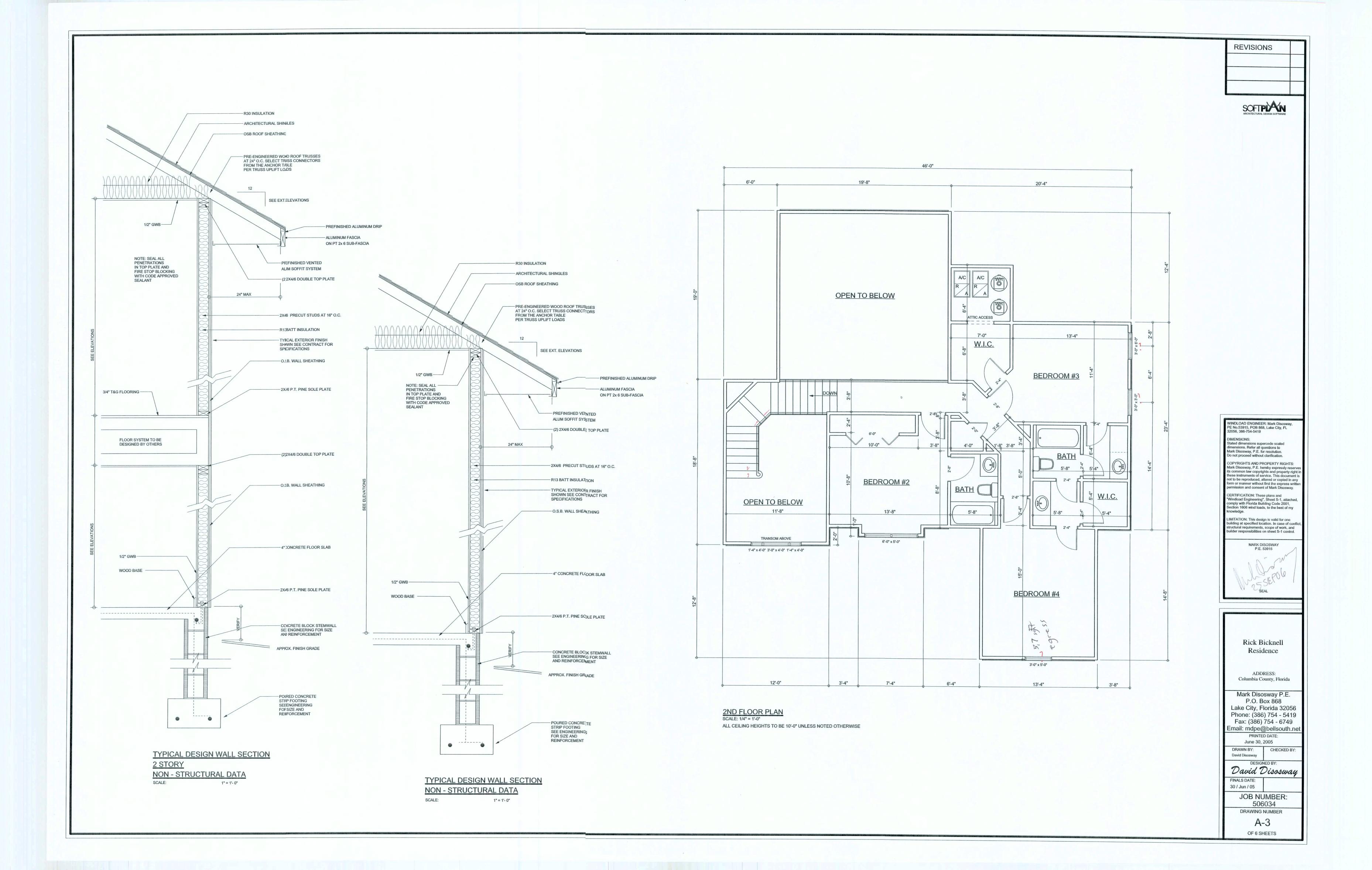
DRAWN BY: CHECKED BY:
David Disosway

DESIGNED BY:
David Disosway

FINALS DATE: 30 / Jun / 05

JOB NUMBER: 506034 DRAWING NUMBER

> A-2 OF 6 SHEETS



E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS. E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED. E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE. E -4

ALL SMOKE DETECTORS SHALL BE 120V W/ BATERY
BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
BE INTERLOCKED TOGETHER. INSTALL INSIDEAND
NEAR ALL BEDROOMS. E -5

TELEPHONE, TELEVISION AND OTHER LOW VOITAGE
DEVICES OR OUTLETS SHALL BE AS PER THE (WNER'S
DIRECTIONS, & IN ACCORDANCE W/ APPLICABIE
SECTIONS OF NEC-LATEST EDITION. E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS. E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY. E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT) TWO SIDED GAS FIREPLACE E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

IN CEILING

1 1

<u>[</u>

ELECTRICAL PLAN NOTES

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
♦	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
₩	220v OUTLET
⊕ _{eu}	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$	4 WAY WALL SWITCH
dv _{P/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
P	GARAGE DOOR OPENER
Maccommonance	WALL HEATER

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS:
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June 30, 2005 DRAWN BY: CHECKED BY:

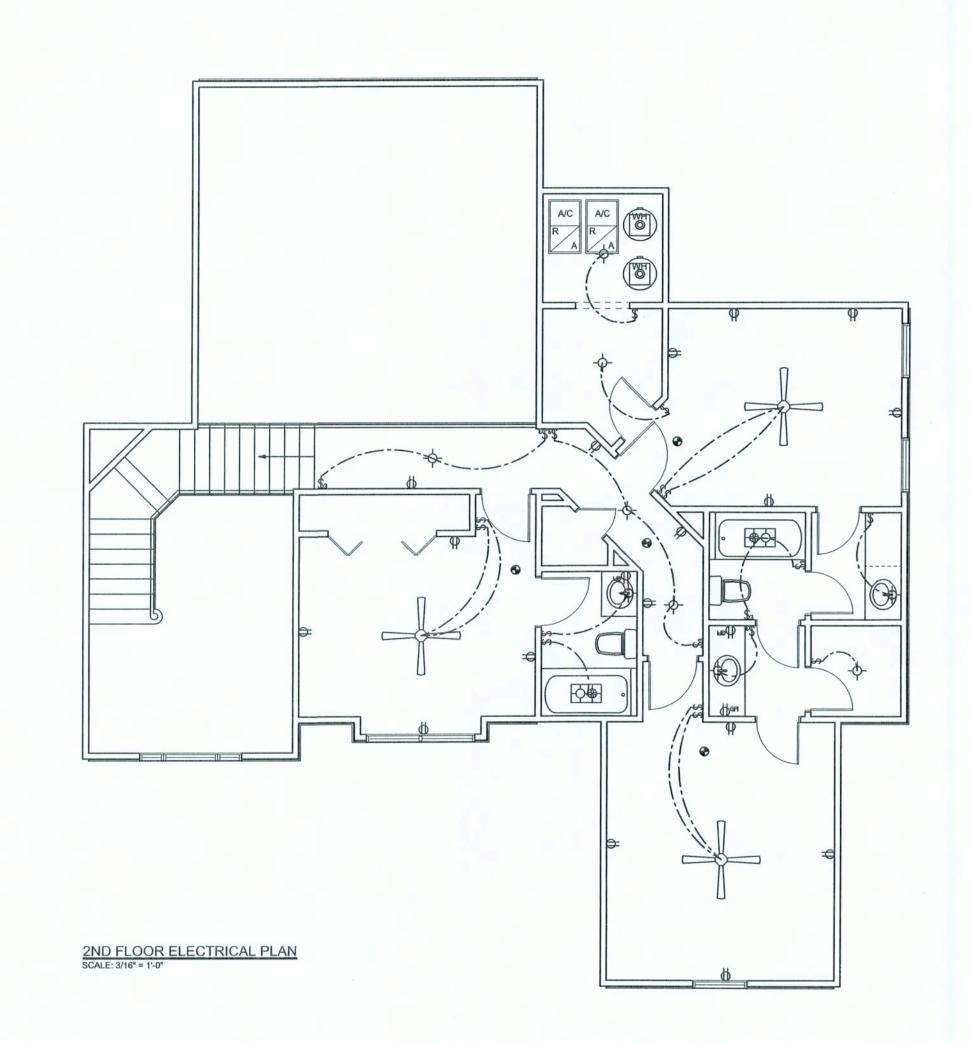
David Disosway DESIGNED BY:

David Disosway

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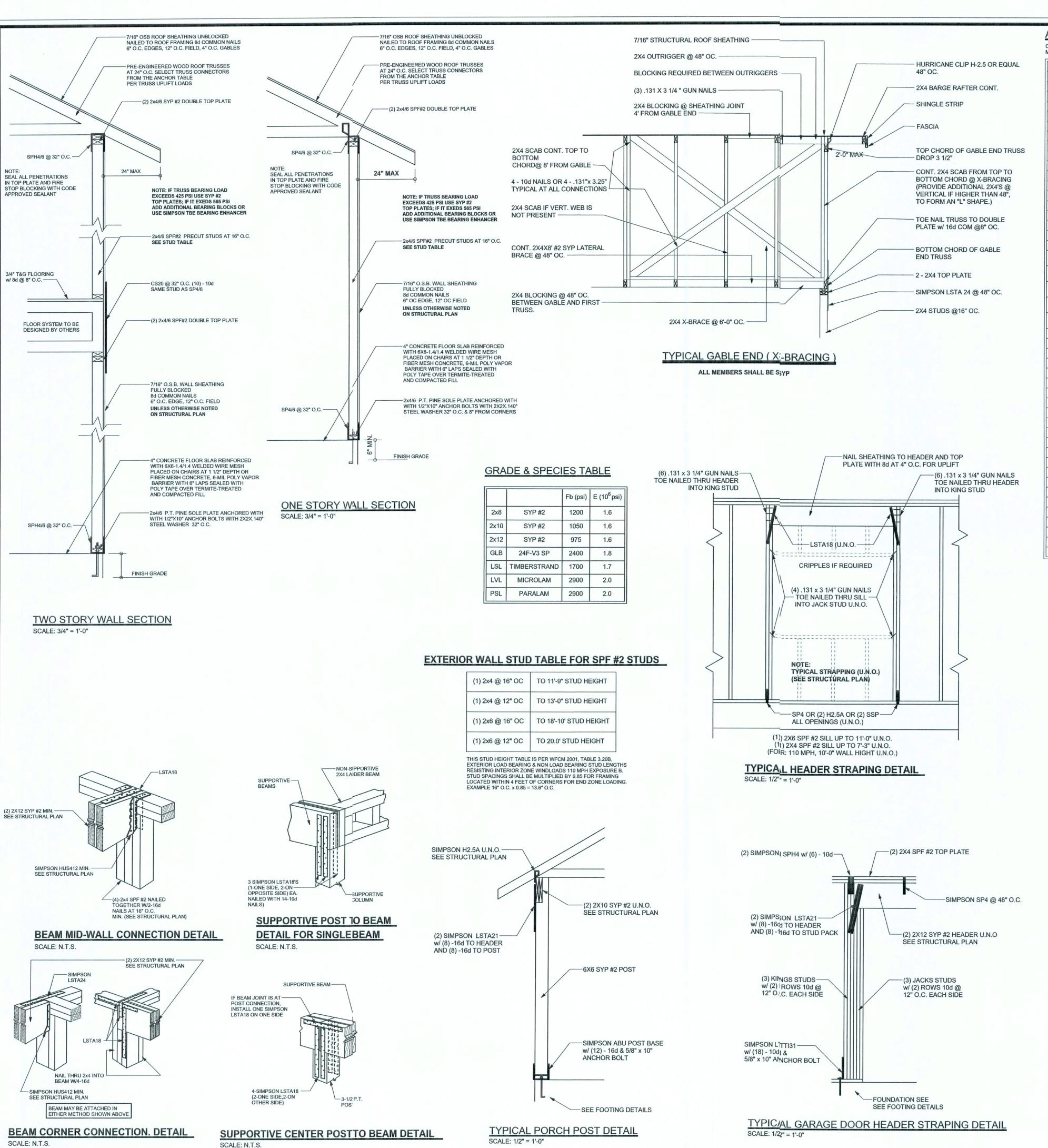
A-4 OF 6 SHEETS



CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING TWO SIDED GAS FIREPLACE IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN. F1 S-1 4" CONCRETE FLOOR SLAB REINFORCED WITH
6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS
AT 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL
POLY VAPOR BARRIER WITH 6" LAPS SEALED WITH
POLY TAPE OVER TERMITE-TREATED AND COMPACTED FILL f===3 REF. ----F4 S-1 GAS FIREPLACE FOUNDATION PLAN SCALE: 1/4" = 1'-0" DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SHEET #

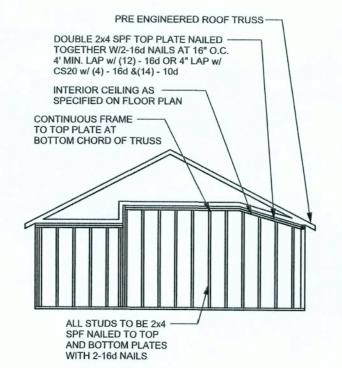
A-5 OF 6 SHEETS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

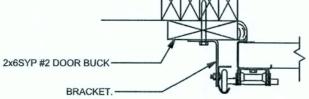
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
					1/2" AB
< 1350	< 1305	LTT19	8-160		
		LTT19 LTTI31	8-16d 18-10d 1 1/2"		
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2310 < 2775	< 2310 < 2570	LTTI31 HD2A	18-10d, 1 1/2" 2-5/8" BOLTS		1/2" AB 5/8" AB
< 2310 < 2775 < 4175	< 2310 < 2570 < 3695	LTTI31 HD2A HTT16	18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d		1/2" AB
< 2310 < 2775 < 4175 < 1400	< 2310 < 2570 < 3695 < 1400	LTTI31 HD2A HTT16 PAHD42	18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d 16-16d		1/2" AB 5/8" AB
< 2310 < 2775 < 4175 < 1400 < 3335	< 2310 < 2570 < 3695 < 1400 < 3335	LTTI31 HD2A HTT16 PAHD42 HPAHD22	18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d 16-16d 16-16d		1/2" AB 5/8" AB 5/8" AB
< 2310 < 2775 < 4175 < 1400	< 2310 < 2570 < 3695 < 1400	LTTI31 HD2A HTT16 PAHD42	18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d 16-16d		1/2" AB 5/8" AB



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAI SCALE: N.T.S.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

D	OOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
	8' - 10'	24" O.C.	5" O.C.	5" O.C.
	11' - 15'	18" O.C.	4" O.C.	4" O.C.
	16' - 18'	16" O.C.	3" O.C.	3" O.C.



GN PER TABLE BELOW:

GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

GENERAL NOTES

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE

FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTMIC 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PPLICATION OF FBC 2001 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

TRUSS SHEETS. **DESIGN DATA** WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS. BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) Zone Effective Wind Area (ft2) 19.9 -21.8 18.1 -18.1 2 19.9 -25.5 18.1 -21.8 2 O'hg -40.6 3 | 19.9 | -25.5 | 18.1 | -21 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows | 21.8 | -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door | 19.5 | -22.9 16x7 Garage Door 18.5 -2

DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF NOT IN FLOOD ZONE (BUILDER TO VERIFY)

PE No.53915, POB 868, Lake City, FL

REVISIONS

SOFTPLAN

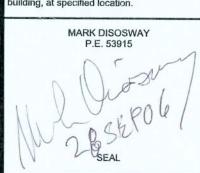
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rmission and consent of Mark Disosway. ERTIFICATION: I hereby certify that I have mined this plan, and that the applicable rtions of the plan, relating to wind engine emply with section R301.2.1, florida buildi de residential 2004, to the best of my

LIMITATION: This design is valid for one uilding, at specified location.



Stanley Crawford Construction

Rick Bicknell

Residence

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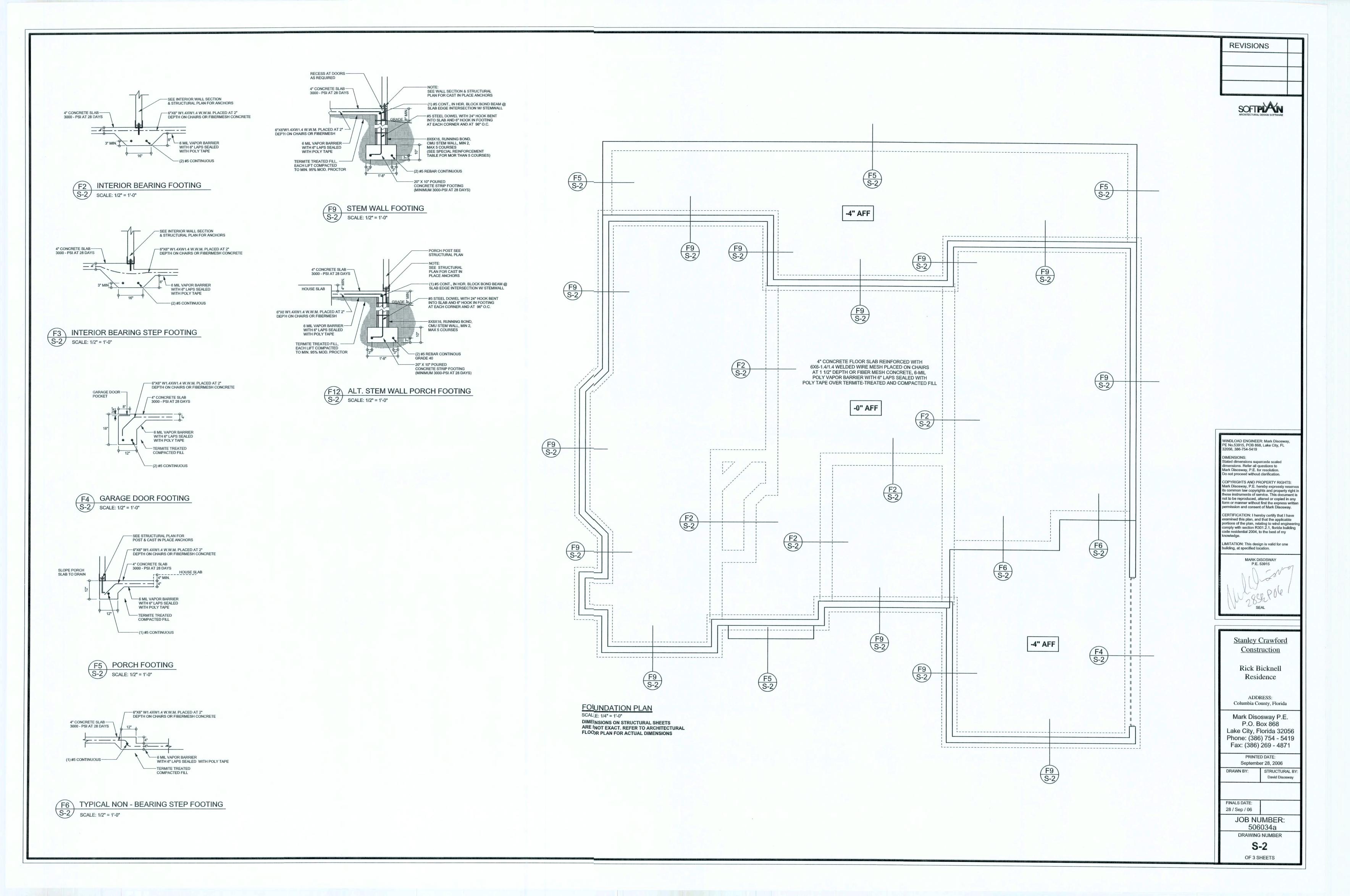
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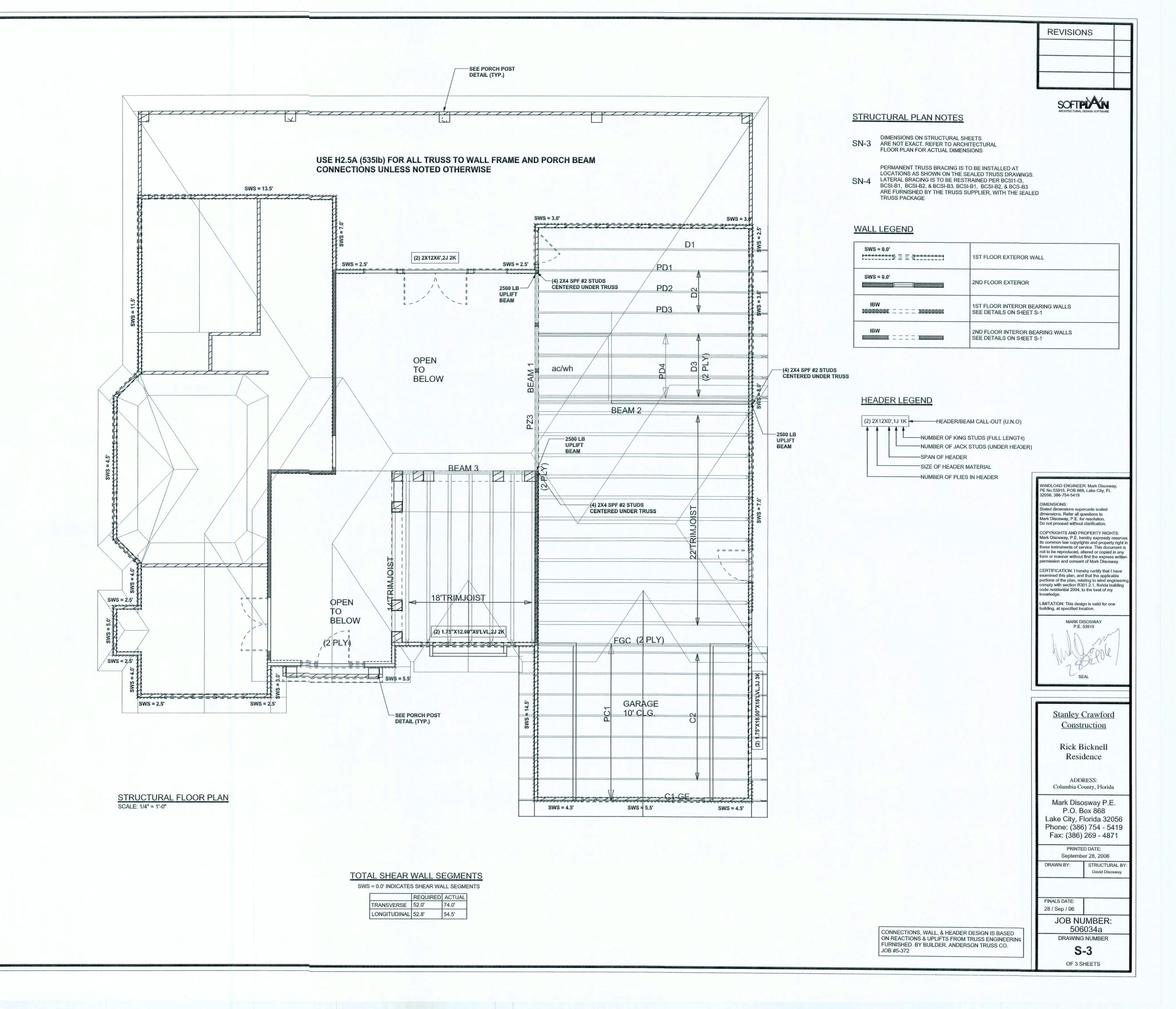
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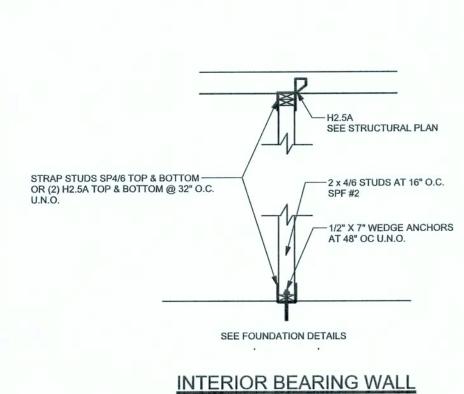
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> **S-1** OF 3 SHEETS

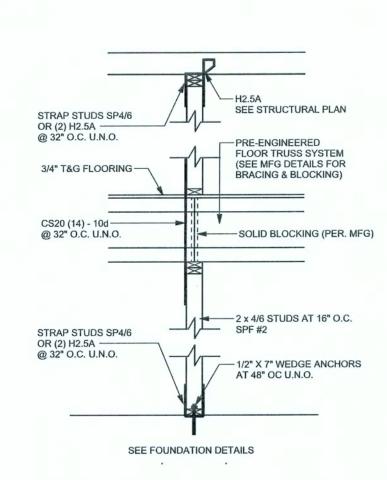
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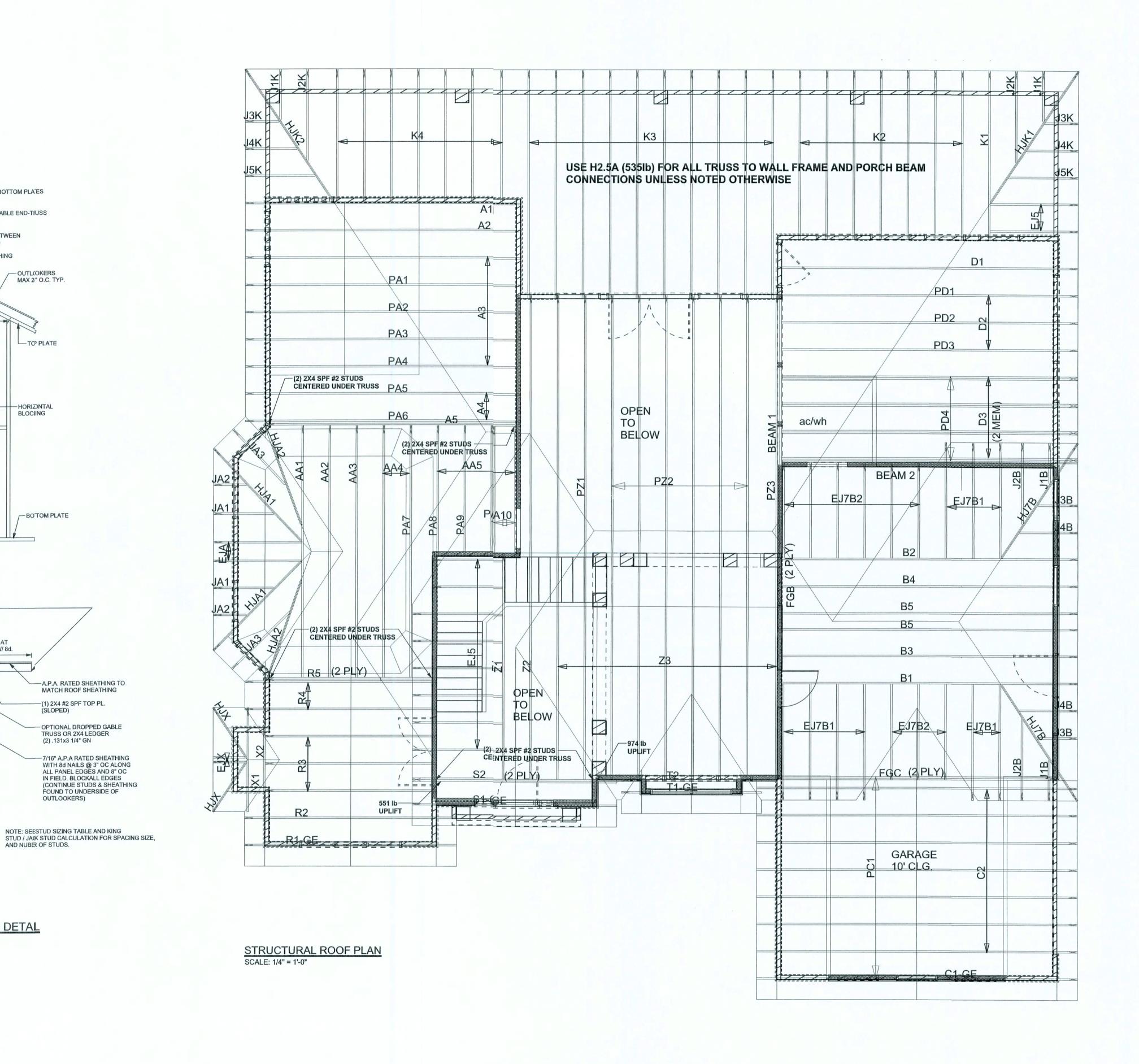


SCALE: 1/2" = 1'-0"



2 STORY INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



BLOCKING MUST BE PARALLEL TO TOP AND BOTTOM PLA'ES

NOTE: MUST STRAP EACH OUTLOOKER TO GABLE END-TIUSS

CRIPPLE

- SOLID BLOCKING BETWEEN

-OUTL(OKERS MAX 2." O.C. TYP.

TO PLATE

-HORIZONTAL

-BOTOM PLATE

-A.P.A. RATED SHEATHING TO MATCH ROOF SHEATHING

OPTIONAL DROPPED GABLE TRUSS OR 2X4 LEDGER (2) .131x3 1/4" GN

-7/16" A.P.A RATED SHEATHING

WITH 8d NAILS @ 3" OC ALONG ALL PANEL EDGES AND 8" OC

IN FIELD. BLOCKALL EDGES (CONTINUE STUDS & SHEATHING FOUND TO UNDERSIDE OF

OUTLOOKERS)

NOTE: SEESTUD SIZING TABLE AND KING

AND NUBER OF STUDS.

-(1) 2X4 #2 SPF TOP PL.

BLOCIING

OUTLOOKERS (TYP.)

WITH A MINIMUM OF 2-16d NAILS

/-DOUBLE

HEADER

/-JACK STUD

FASTEN SHEATHING TO FRAMING AT

4" OC ON EDGES. 6" OC IN FIELD W/ 8d.

(3) .131 X 3 1/4 "

1/2" GYP-

2x4 BLOCKING

GABLE END WALL BALLOON FRAMING DETAL

(3) .131x3 1/4 " GN

GUN NAILS

OVERHANGS IN EXCESS OF 12" AND UP

TO 24" USE 2X4 SYP OUT LOOKER AT

24" OC CLIP TO DBL TOP PL. WITH (1) -

H2.5 W/ (8) 8d. (12" OVERHANGS DO NOT NEÈD OUTLOOKERS OR DROPPED

TRUSSES/RAFTERS ---/

CEILING HEIGHT VARIES-

SCALE: 1/2" = 1'-0"

BLOCK SHEATHING EDGES WITHIN 4'

OF GABLE TRUSS ATTACH EACH END WITH (3) .131 X 3 1/4" GUN NAILS

STUD

DOUBLE STUD

WITH SIMPSON H2.5 OR EQUIVALENT

SOFTPIAN

REVISIONS

STRUCTURAL PLAN NOTES

EACH SIDE (U.N.O.)

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD

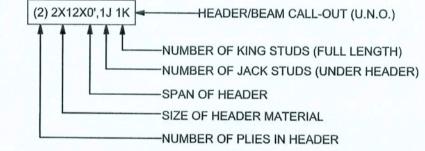
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

sws = 0.0,	1ST FLOOR EXTERIOR WALL		
SWS = 0.0'	2ND FLOOR EXTERIOR		
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1		
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1		

HEADER LEGEND



PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Stanley Crawford Construction

> Rick Bicknell Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: September 28, 2006 STRUCTURAL BY DRAWN BY: David Disosway

FINALS DATE: 28 / Sep / 06 JOB NUMBER: 506034a

> **S-4** OF 3 SHEETS

DRAWING NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB #5-372