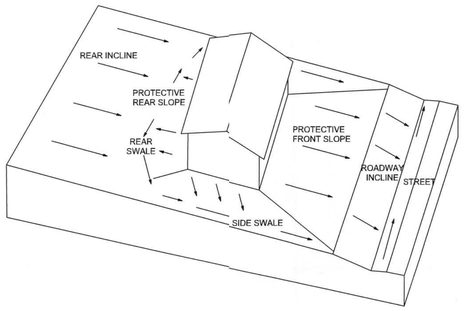


FHA / VA / HUD GRADING PATTERN TYPE "A"
(ALL DRAINAGE TOSTREET)

SITE PLAN NOTES:

- THIS SITE PLAN IS NOT VALID WITHOUT OWNERS SIGNATURE TO GUARANTE THE ACCURACY OF SETBACKS, LOT GEOMETRY, AND WELL, SEPTIC & BUILING

- MINIMUM ACCEPTABLE GRADE IS 1%

-100 YEAR FLOOR ELEVATION WAS ESTABLISHED AS 110.4' BY ARTHUR BEDINBAUGH AND REORDERED ON PLAT BOOK 6 PAGE 88 "SOUTHERN APPROACHES UN ONE" IN THE NE 1/4 OF THE NW 1/4 OF SECTION 13, TOWNSHIP 4 SOUTH, RANGE ! EAST, COLUMBIA COUNTY, FLORIDA.

- THE LOT WAS PERMITTED TO BE FILLED TO 110.5' OR ABOVE 100 YEAR FLOD ELEVATION AS SHOWN ON PLAT.

- THE FINISHED FLOOR ELEVATION MUST BE MINIMUM 6" ABOVE FINISHED GADE PER FBC2004. THE FINISHED GRADE SHOULD SLOPE DOWN FROM THAT ELVATION FOR ANOTHER 6" WITHIN 12 FEET AWAY FROM THE HOUSE AND HANGER IIALL DIRECTIONS SO THAT ALL RUNOFF DRAINS AWAY FROM THE HOUSE. THE WNER MUST MAINTAIN THE SWALES, SLOPES, AND DITCH TO PROVIDE FREE DRAIAGE TO THE DITCH AND PREVENT ANY POSSIBILITY OF STORM WATER BACKING UNTO THE HOUSE

- THE OWNER SHOULD BE AWARE THAT IF FREE DRAINAGE IS NOT MAINTAIED THRU FIELDS AND SUBDIVISIONS ACROSS ROADS AND THRU CULVERTS, OR IF FTURE DEVELOPMENT IN THE AREA CAUSES INCREASED STORM WATER RUN OFFOR IF RAINFALL OCCURS WITH GREATER FLOODING EFFECT THAN THE DESIGN CORM, THE LEVEL OF THE RUNOFF WATER COULD RISE HIGHER THAN ANTICIPATD AND NEARBY ZONE "A" AREAS COULD BE LARGER THAN INDICATED BY FEMA AN HIS HOUSE WOULD BE MORE SUSCEPTIBLE TO FLOODING (BASED ON PERSONAL OBSVATION WITHOUT MEASUREMENT, THE STORMS OF 2004 CAUSED FLOODING THAT EEMED TO BE BELOW THE LEVEL DEFINED BY FEMA AND THE PLAT PLAN IN AREASEAR THIS HOME.)

FOOTINGS AND FOUNDATIONS NOTES:
1 - Foundations shall be built on undisturbed soil or properly compacted fill material. Fundations shall be constructed of materials described in the FBC2004
2 - Pile foundations shall be designed and constructed in accordance with the FBC20l
3 - The bottom of foundations shall extend no less than 12 inches (305 mm) below finh grade.
4 - Temporary buildings and buildings not exceeding one story in height and 400 sq fh area shall be exempt from these requirements.
5 - Excavations for foundations shall be backfilled with soil which is free of organic marial, construction debris and large rocks.
6 - Where water impacts the ground from a roof valley, downspout, scupper or other rn water collection or diversion device, provisions shall be made to prevent soil erosion and direct thwater away from the foundation.
7 - Finish grade shall be sloped away from the foundation for drainage.
8 - The area under footings, foundations and concrete slabs on grade shall have all vjetation, stumps, roots and foreign materials removed prior to their construction. Fill material shall bfree of vegetation and foreign material.

DRAINAGE PLAN KEY

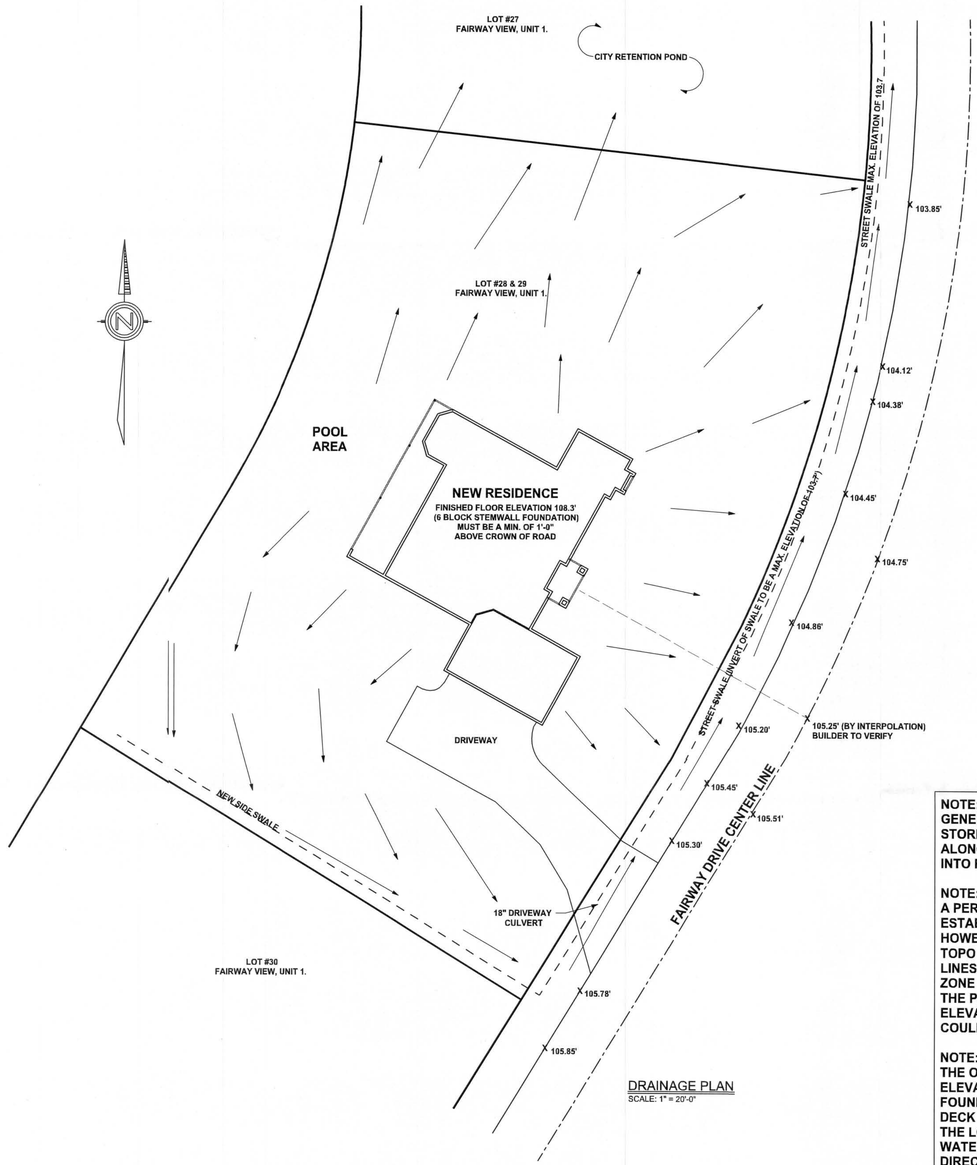
LOT LINE = ——————

INVERT OF DRAINAGE DITCH = – – – – –

CENTER LINE OF ROAD = — – — – —

DIRECTION OF WATER RUNOFF OF FINAL GRADE =

ELEVATION SHOT AS PROVIDED BY: DONALD F. LEE & ASSOCITATES, INC. (SEE ATTACHED SURVEY 06-5180) X 104.75'



DRAINAGE PLAN
SCALE: 1" = 20'-0"

NOTE: EXISTING GRADE ON LOTS # 28 & 29 GENERALLY SLOPES TO FRONT AND STORMWATER RUNNOFF RUNS INTO SWALE ALONG FAIRWAY DR RUNNING NORTHWARD INTO RETENTION POND.

NOTE: LOTS 28 AND 29 ARE IN FLOOD ZONE A PER FEMA RATE MAPS. THERE IS NO ESTABLISHED FLOOD ELEVATION. HOWEVER, VISUAL ALIGNMENT OF NGS TOPO MAP LINES AND FEMA RATE MAP LINES INDICATES IF THE AREA SHOWN AS ZONE A ON THE FEMA RATE MAP FLOODS THE PROPOSED FINISHED FLOOR ELEVATION OF THE PROPOSED HOUSE COULD BE ABOUT 5' UNDER WATER.

NOTE: THE CONTRACTOR STATES THAT THE ONLY FILL TO BE BROUGHT IN IS TO ELEVATE THE FILLED STEMWALL FOUNDATION OF THE HOUSE AND POOL DECK ABOUT 4' ABOVE EXISTING GRADE. THE LOT HAS NO EXISTING AREAS OF WATER STORAGE. THIS GRADING PLAN DIRECTS STORMWATER RUNOFF TO THE SAME LOCATION AS EXISTING STORMWATER RUNOFF.

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions, Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, comply, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

28 Jun 07

SEAL

Isaac Construction

Nick Patel Residence

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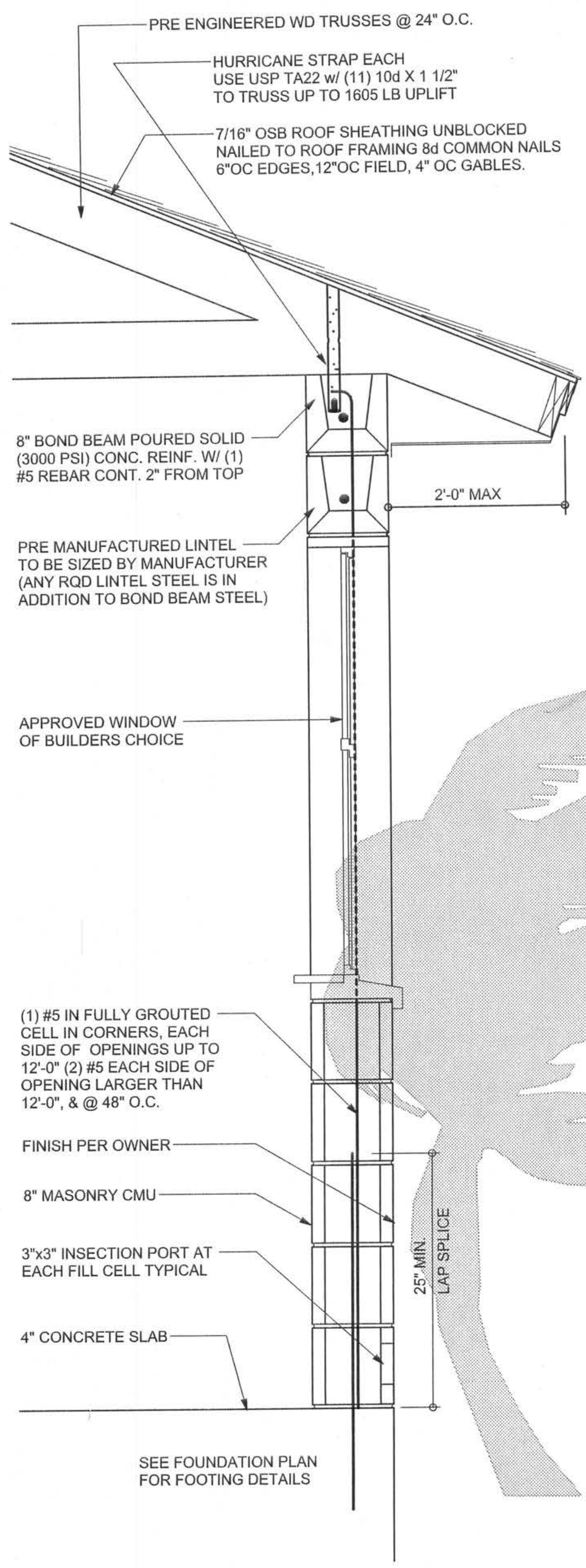
PRINTED DATE:
June 28, 2007

DRAWN BY: Evan Beamsley
CHECKED BY:

FINALS DATE:
28 / Jun / 07

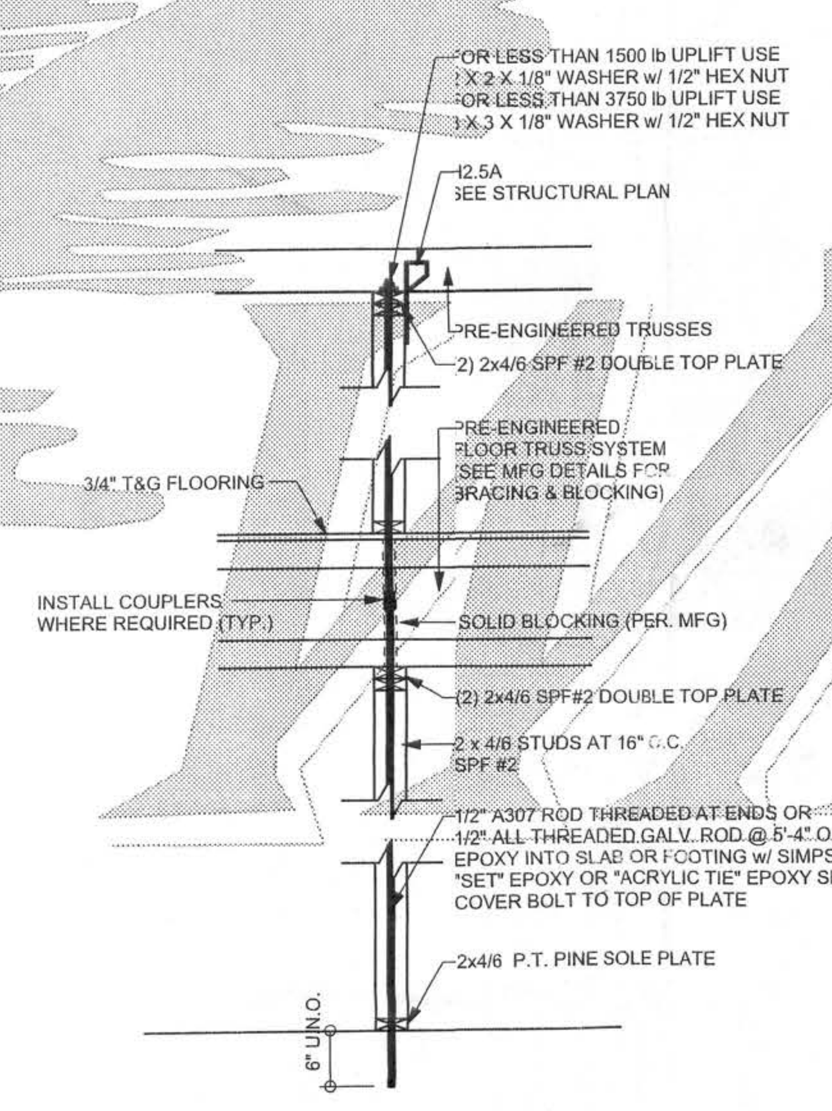
JOB NUMBER:
705154

DRAWING NUMBER
D1
OF 1 SHEET



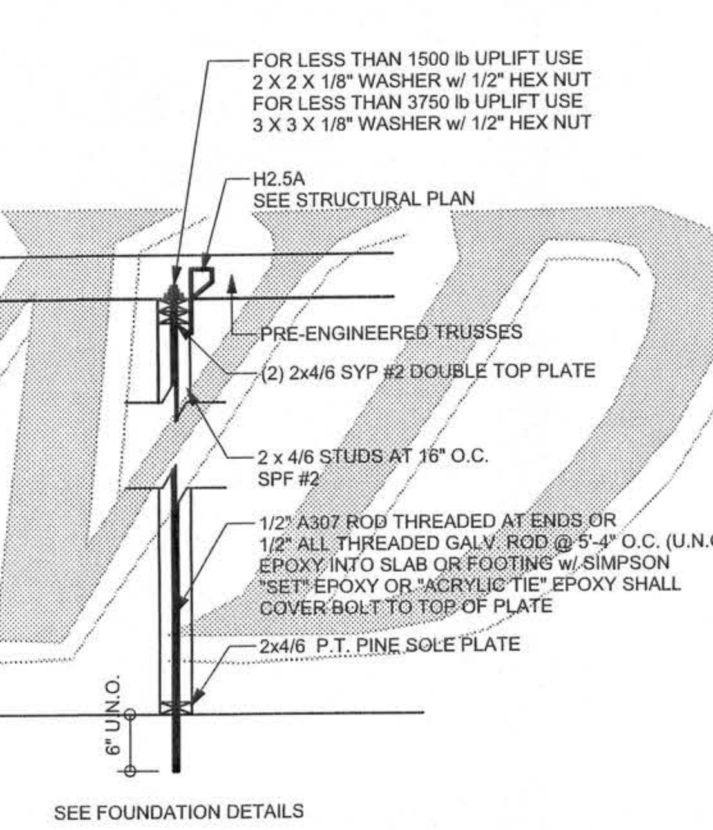
TYPICAL 1 STORY BLOCK WALL SECTION

SCALE: 1" = 1'-0"



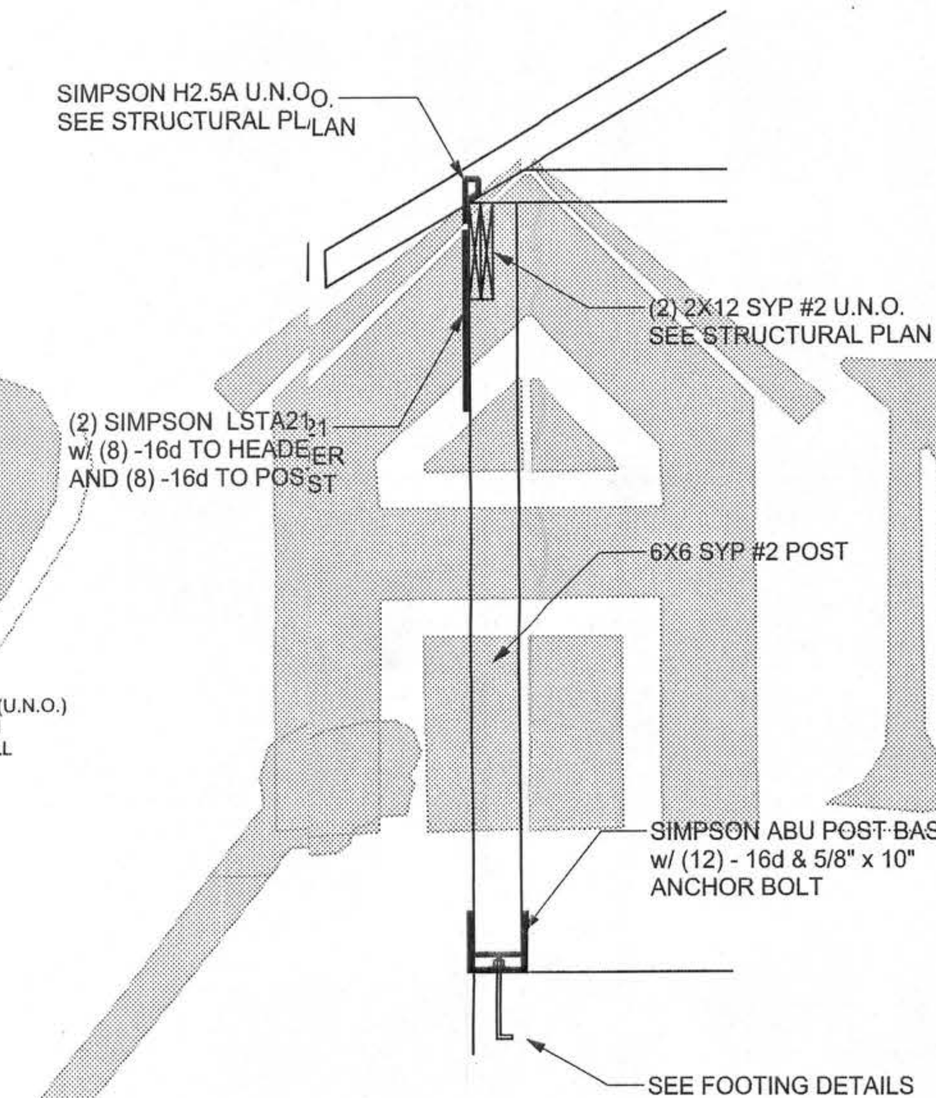
TWO STORY INTEFOR BEARING WALL

SCALE: 1/2" = 1'-0"



INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

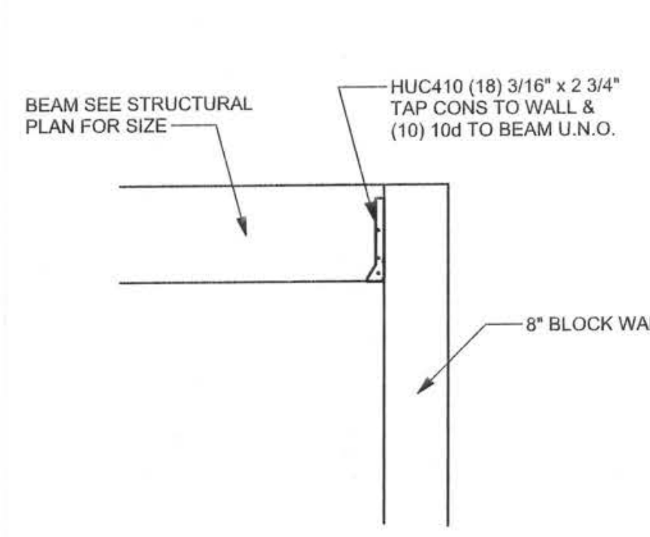
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

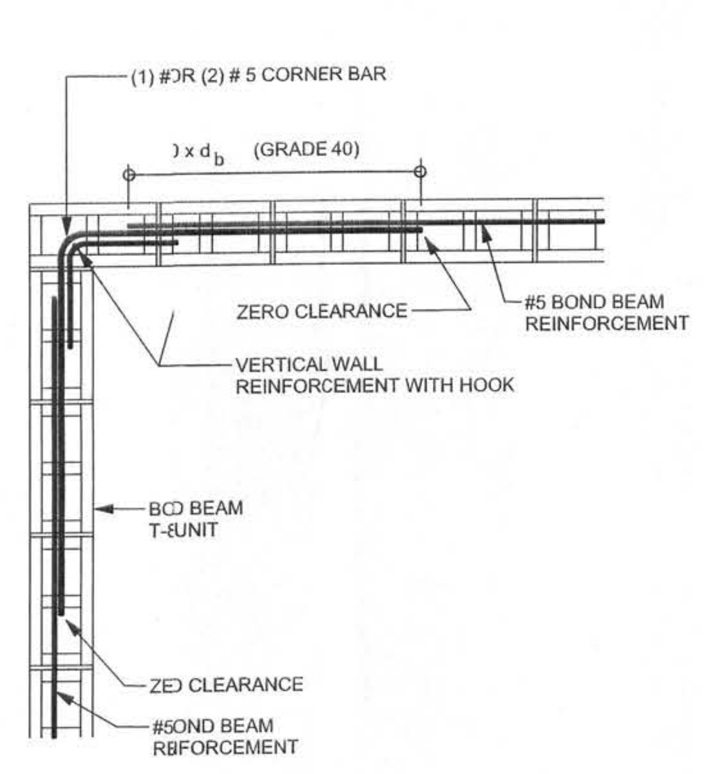
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT



BEAM TO BLOCK CONNECTION DETAIL

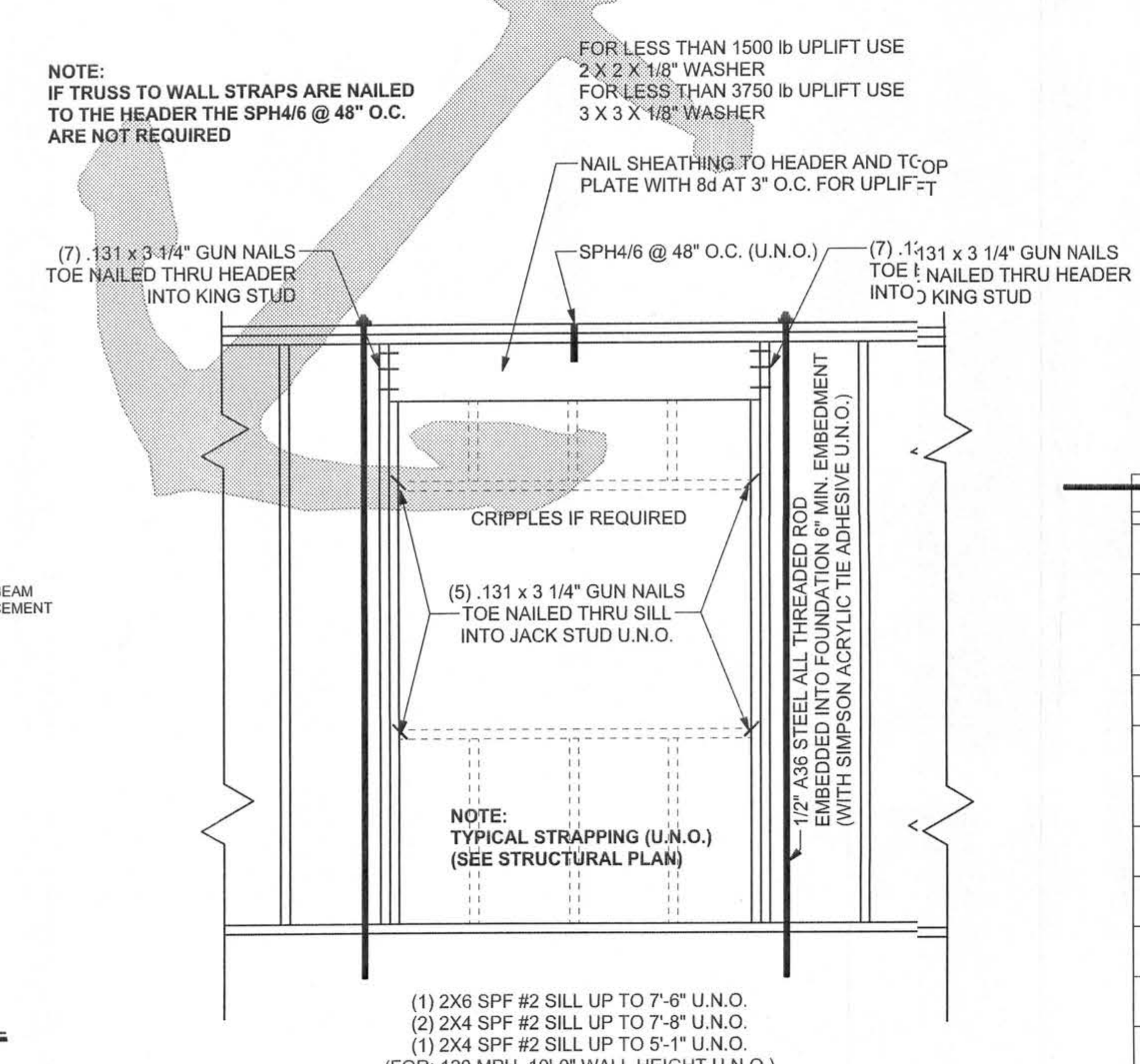
SCALE: 1/2" = 1'-0"



TYPICAL BOND BEAM CORNER DETAIL

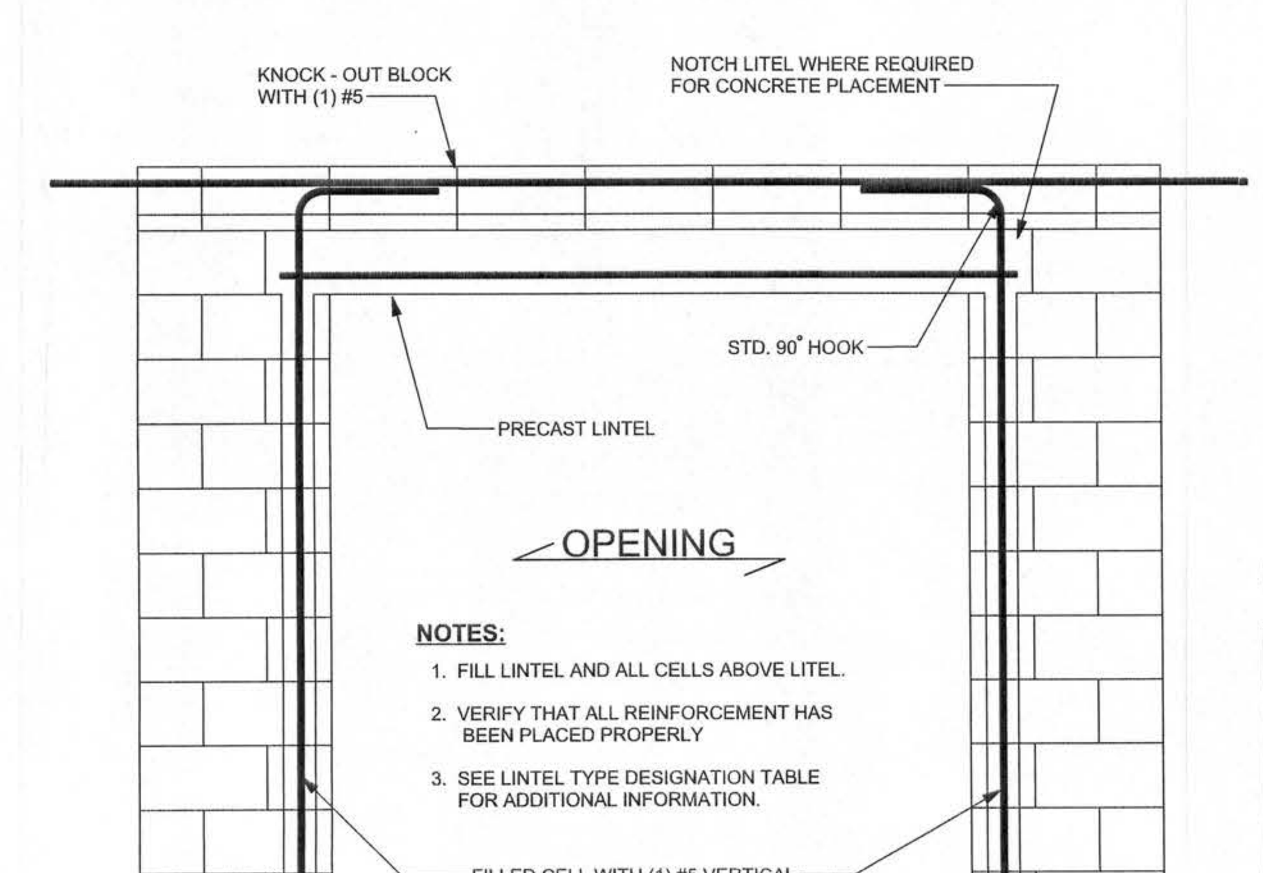
SCALE: 1/2" = 1'=0"

NOTE:
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



TYPICAL 1 STORY HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"



TYPICAL FILLED LINTEL ASSEMBLY

SCALE: 1/2" = 1'=0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

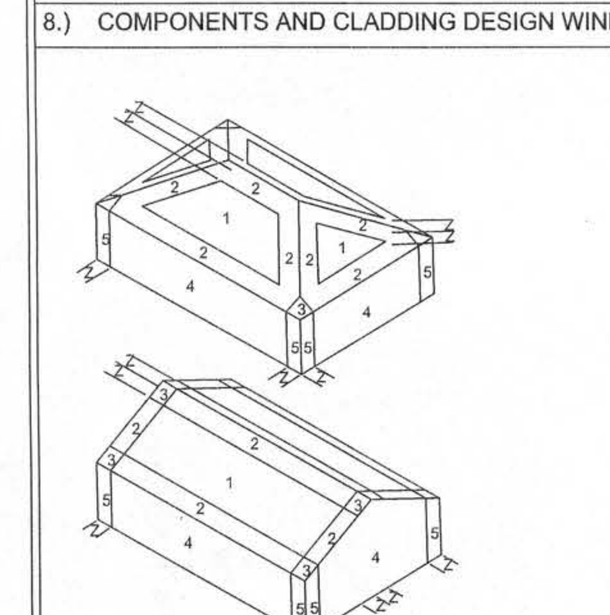
4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 386754-5419

DIMENSIOIS:
Stated dimensions supercede scaled dimensions Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

[illegible]
29 MAY 07

SEAL

Isaac Construction

Nick Patel Residence

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PRINTED DATE:
May 29, 2007

DRAWN BY:	CHECKED BY:
David Disosway	

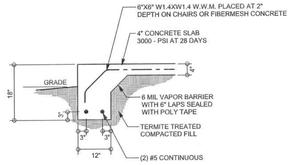
FINAL DATE:
28 / May / 07

JOB NUMBER:
705154

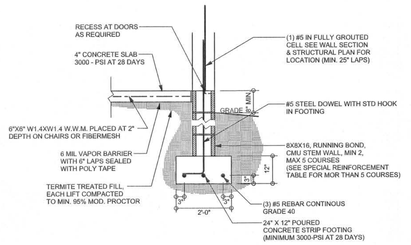
DRAWING NUMBER

S-1

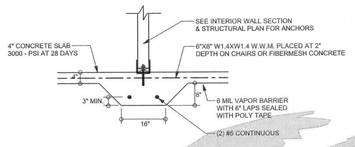
OF 4 SHEETS



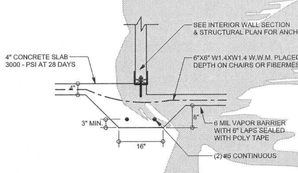
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

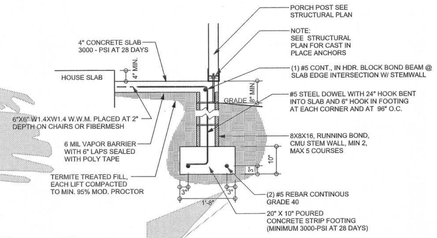


F17 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

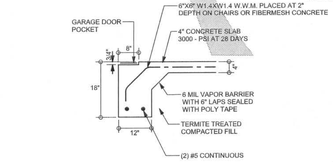


F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

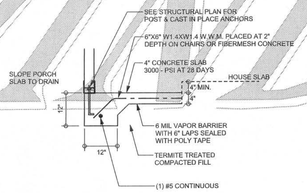




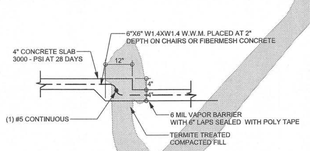
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

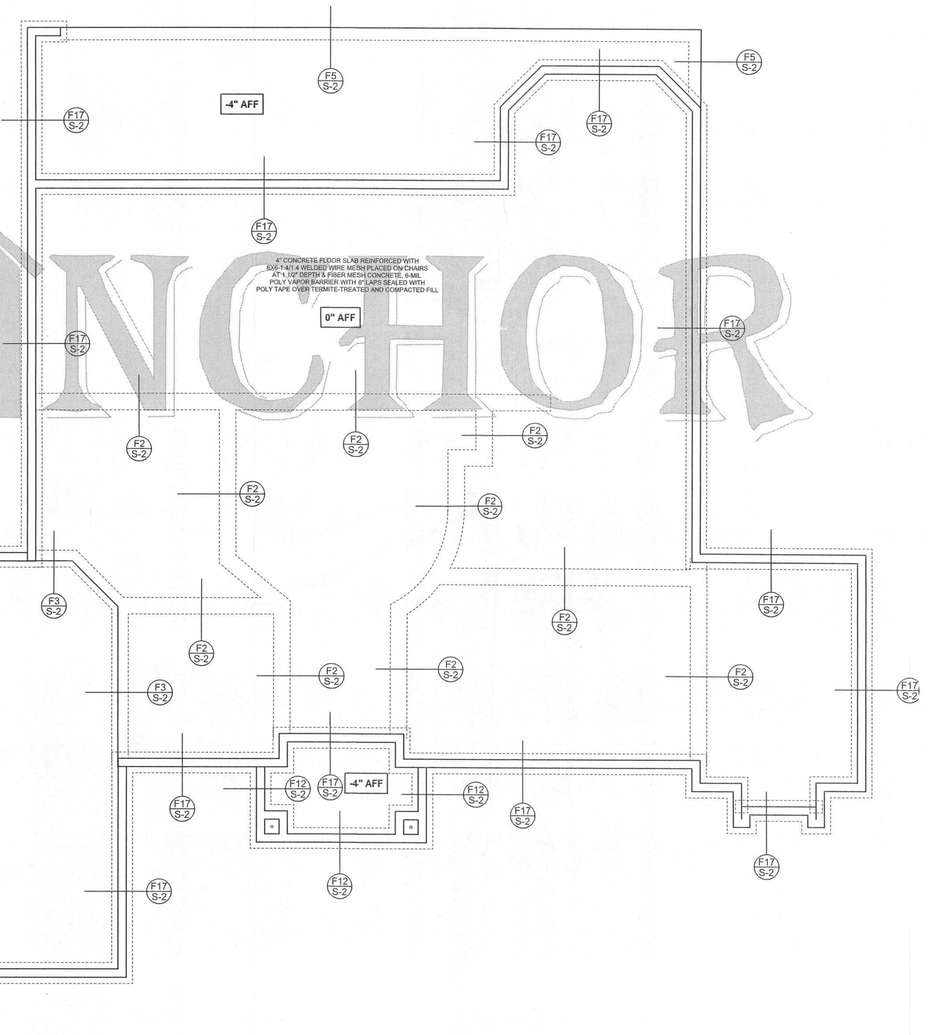
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN
SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT, REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.63915, POB 888, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

SEAL

Isaac Construction

Nick Patel Residence

ADDRESS:
442 NW Fairway Drive
Lake City, Florida 32055

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: May 29, 2007

DRAWN BY: David Disosway

CHECKED BY:

FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

DRAWING NUMBER
S-2
OF 4 SHEETS

MATERIALS

1. f'c 8" precast lintel = 3500 psi
2. f'c prestressed lintel = 6000 psi
3. Grout per ASTM C476 f'c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

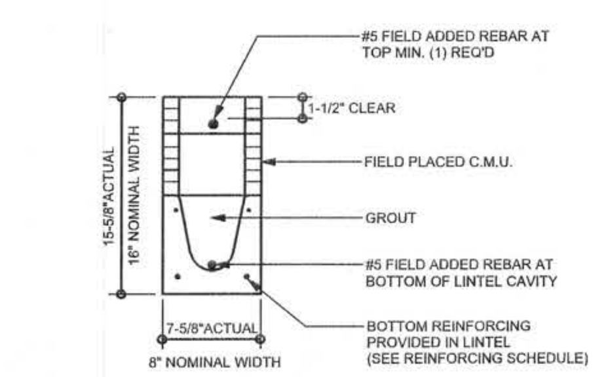
$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION

8F16-1B/1T-4'-0"

- F = FILLED WITH GROUT / U = UNFILLED / S = SOLID
- QUANTITY OF #5 FIELD ADDED REBAR AT BOTTOM OF LINTEL CAVITY
- NOMINAL WIDTH
- NOMINAL HEIGHT
- QUANTITY OF #5 FIELD ADDED REBAR AT TOP
- LENGTH



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintels lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction 6472 ⇒ (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECST & PRESTRESSED U-LINTELS

CAST-CRETE — SAFE LAD - POUNDS PER LINEAR FOOT

LENGTH		TYPE	8U8	8F8-0B / 8F8-1B	8F2-0B / 8F2-1B	8F16-0B / 8F16-1B	8F20-0B / 8F20-1B	8F24-0B / 8F24-1B	8F28-0B / 8F28-1B	8F32-0B / 8F32-1B
2'-10"	(34")	PRECAST	2231	3069	05	6113	7547	8974	10394	11809
				3069	05	6113	7547	8974	10394	11809
3'-6"	(42")	PRECAST	2231	3069	19	5163	6607	8054	9502	10951
				3069	05	6113	7547	8974	10394	11809
4'-0"	(48")	PRECAST	1966	2561	751	3820	4890	5961	7034	8107
				2693	05	6113	7547	8974	10394	11809
4'-6"	(54")	PRECAST	1599	1969	10	2931	3753	4576	5400	6224
				2189	75	6113	7547 (7)	8672	10294	11809
5'-4"	(64")	PRECAST	1217	1349	38	1999	2560	3123	3686	4249
				1663	90	5365	7547 (36)	7342 (19)	8733 (19)	10127 (19)
5'-10"	(70")	PRECAST	1062	1105	73	1631	2090	2549	3009	3470
				1451	22	4360	7168 (45)	6036 (19)	7181 (19)	8328 (20)
6'-6"	(78")	PRECAST	908	1238	77	3480	3031	3707	4383	5061
				1238	77	3480	5381	8360	10394 (37)	8825 (14)
7'-6"	(90")	PRECAST	743	1011	29	2632	2205	2698	3191	3685
				1011	29	2661	3898	5681	8467 (44)	6472 (15)
9'-4"	(112")	PRECAST	554	699	60	1625	2564	3486	2818	3302
				752	45	1843	2564	3486	4705 (37)	6390 (47)
10'-6"	(126")	PRECAST	475	535	90	1247	2093	2777	2163	2536
				643	52	1533	2093	2781	3643 (38)	4754 (45)
11'-4"	(136")	PRECAST	362	582	45	1366	1846	2423	3127	4006
				582	45	1366	1846	2423	3127	4006
12'-0"	(144")	PRECAST	337	540	73	1254	1684	2193	2805	3552
				540	73	1254	1684	2193	2805	3552
13'-4"	(160")	PRECAST	296	471	55	1075	1428	1838	2316	2883
				471	55	1075	1428	1838	2316	2883
14'-0"	(168")	PRECAST	279	424	06	1002	1326	1697	2127	2630
				442	06	1002	1326	1697	2127	2630
14'-8"	(176")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				458	83	1370	1902	2245	2517	2712
15'-4"	(184")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				412	10	1250	1733	2058	2320	2513
17'-4"	(208")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				300	36	950	1326	1609	1849	2047
19'-4"	(232")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				235	18	750	1037	1282	1515	1716
21'-4"	(256")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				180	40	598	845	1114	1359	1468
22'-0"	(264")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				165	15	550	784	1047	1285	1399
24'-0"	(288")	PRESTRESSED	N.R.	NR	R	NR	NR	NR	NR	NR
				129	50	450	654	884	1092	1222

SAFE UPLIFT LOADS FOR 8" PRECAST PRESTRESSED U-LINTELS

CAST-CRETE — SAFE LOAD - OUNDS PER LINEAR FOOT

LENGTH		TYPE	8F8-1T / 8F8-2T	8F12-1T / 8F12-2T	8F-1T / 8F-2T	8F20-1T / 8F20-2T	8F24-1T / 8F24-2T	8F28-1T / 8F28-2T	8F32-1T / 8F32-2T
2'-10"	(34")	PRECAST	1972	3173	60	5747	7034	8321	9608
			1972	3173	60	5747	7034	8321	9608
3'-6"	(42")	PRECAST	1569	2524	47	4569	5591	6613	7636
			1569	2524	47	4569	5591	6613	7636
4'-0"	(48")	PRECAST	1363	2192	79	3966	4853	5740	6627
			1363	2192	79	3966	4853	5740	6627
4'-6"	(54")	PRECAST	1207	1940	24	3508	4292	5077	5861
			1207	1940	24	3508	4292	5077	5861
5'-4"	(64")	PRECAST	1016	1632	90	2949	3607	4265	4924
			1016	1632	90	2949	3607	4265	4924
5'-10"	(70")	PRECAST	909	1492	93	2694	3295	3897	4498
			929	1492	93	2694	3295	3897	4498
6'-6"	(78")	PRECAST	835 (12)	1340	80	2419	2959	3498	4038
			835	1340	80	2419	2958	3498	4038
7'-6"	(90")	PRECAST	727 (23)	1021	34 (12)	2102 (11)	2571 (10)	3039 (10)	3508 (9)
			727	1166	34	2102	2571	3039	3508
9'-4"	(112")	PRECAST	591	680	33 (15)	1471 (15)	1811 (15)	2152 (16)	2494 (15)
			591	851	26	1705	2084	2463	2842
10'-6"	(126")	PRECAST	530	552	14 (15)	1185 (15)	1458 (15)	1732 (15)	2007 (15)
			530	686	83	1526	1865	2204	2544
11'-4"	(136")	PRECAST	474	485	38 (15)	1034 (15)	1272 (15)	1510 (15)	1749 (15)
			494	599	28	1422	1738	2053	2369
12'-0"	(144")	PRECAST	470 (9)	441	23 (14)	936 (14)	1151 (15)	1366 (15)	1582 (15)
			470	543	28	1349	1649	1948	2247
13'-4"	(160")	PRECAST	418 (15)	373	06 (14)	783 (14)	962 (14)	1141 (14)	1321 (14)
			428	455	70	1145	1444	1718	1993
14'-0"	(168")	PRECAST	384 (15)	346	59 (14)	723 (14)	887 (14)	1052 (14)	1218 (14)
			410	420	39	1050	1434 (8)	1694 (8)	1954 (7)
14'-8"	(176")	PRESTRESSED	239	323	19 (13)	671 (13)	823 (13)	976 (14)	1129 (14)
			246	390	55	968	1324 (8)	1625 (11)	1874 (11)
15'-4"	(184")	PRESTRESSED	224	302	15 (13)	626 (13)	767 (13)	909 (13)	1052 (13)
			230	364	39	897	1224 (8)	1562 (14)	1801 (14)
17'-4"	(208")	PRESTRESSED	187	255	14 (12)	520 (12)	637 (12)	754 (12)	872 (12)
			192	303	00	732	993 (8)	1268 (14)	1470 (14)
19'-4"	(232")	PRESTRESSED	162	222	17 (11)	446 (11)	546 (12)	646 (12)	746 (12)
			166	261	24	616	831 (8)	1057 (14)	1225 (14)
21'-4"	(256")	PRESTRESSED	142	198	06 (11)	393 (11)	480 (11)	567 (11)	654 (11)
			142	230	69	531	713 (7)	903 (13)	1046 (13)
22'-0"	(264")	PRESTRESSED	137	192	35 (10)	378 (11)	461 (10)	545 (11)	629 (11)
			137	221	54	508	681 (7)	861 (13)	997 (13)
24'-0"	(288")	PRESTRESSED	124	175	37 (10)	341 (10)	416 (10)	491 (10)	566 (10)
			124	200	16	450	600 (7)	756 (12)	875 (13)

SAFE GRAVITY LOADS FOR 8" PRECAT w/ 2" RECESS DOOR U-LINTELS

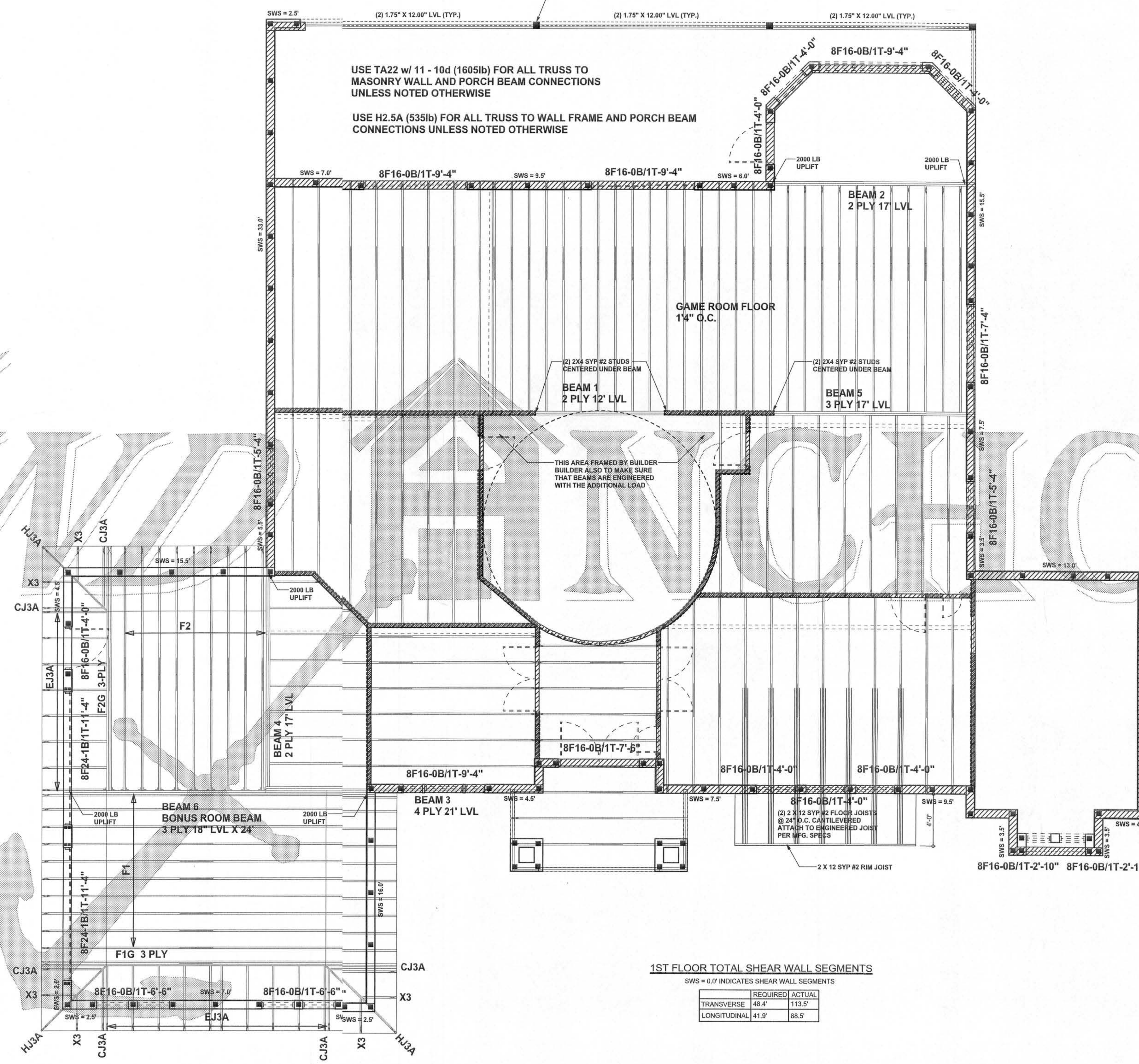
CAST-CRETE — SAFE LAD - POUNDS PER LINEAR FOOT

LENGTH		TYPE	8RU6	8RF6-0B / 8RF6-1B	8R0-0B / 8R0-1B	8RF14-0B / 8RF14-1B	8RF18-0B / 8RF18-1B	8RF22-0B / 8RF22-1B	8RF26-0B / 8RF26-1B	8RF30-0B / 8RF30-1B
4'-4"	(52")	PRECAST	1635	1749	55	3280	4349	5421	6493	7567
				1891	99	5206	6639	8060	9479	10893
4'-6"	(54")	PRECAST	1494	1596	63	2992	3968	4946	5924	6904
				1756	99	5206	6639	8060	9479	10893
5'-8"	(68")	PRECAST	866	920	70	1716	2277	2839	3402	3966
				1167	81	4567	6389	8060 (34)	7917 (19)	9311 (19)
5'-10"	(70")	PRECAST	810	859	53	1600	2124	2649	3174	3700
				1113	42	4242	6639 (10)	8060 (39)	7402 (19)	8706 (19)
6'-8"	(80")	PRECAST	797	901	25	3120	5048	7747	9448	7360
				901	25	3120	5048	7915	9479	10893 (32)
7'-6"	(90")	PRECAST	669	755	90	2459	3776	5743	7239	5623
				755	90	2459	3776	5743	8998 (19)	10893 (48)
9'-8"	(116")	PRECAST	411	466	39	1568	2253	3129	4091	3146
				526	39	1568	2253	3129	4150	5891 (47)

SAFE UPLIFT LOADS FOR 8" PRECAST \ 2" RECESS DOOR U-LINTELS

CAST-CRETE — SAFE LOAD POUNDS PER LINEAR FOOT

LENGTH		TYPE	8RF6-1T / 8RF6-2T	8RF10-1T / 8RF10-2T	8R4-1T / 8R4-2T	8RF18-1T / 8RF18-2T	8RF22-1T / 8RF22-2T	8RF26-1T / 8RF26-2T	8RF30-1T / 8RF30-2T
4'-4"	(52")	PRECAST	905	1748	35	3522	4409	5296	6183
			905	1748	35	3522	4409	5296	6183
4'-6"	(54")	PRECAST	867	1675	25	3374	4224	5074	5924
			867	1675	25	3374	4224	5074	5924
5'-8"	(68")	PRECAST	675	1301	60	2618	3277	3935	4594
			675	1301	60	2618	3277	3935	4594
5'-10"	(70")	PRECAST	655	1262	10	2538	3176	3815	4453
			655	1262	10	2538	3176	3815	4453
6'-8"	(80")	PRECAST	570	1012	51	2204	2758	3312	3865
			570	1097	51	2204	2758	3312	3865
7'-6"	(90")	PRECAST	506	797	12 (8)	1952 (7)	2442 (6)	2931 (6)	3257
			506	967	12	1952	2442	2931	3421
9'-8"	(116")	PRECAST	395	491	1 (12)	1301 (15)	1640 (15)	1980 (15)	2322 (16)
			395	589	15	1514	1893	2272	2652



STRUCTURAL FLOOR PLAN
SCALE: 3/16" = 1'-0"

1ST FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	48.4'	113.5'
LONGITUDINAL	41.9'	88.5'

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C.
1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C.
1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W

3/16" TAPCONS @ 12" O.C.
1/4" TAPCONS @ 18" O.C.

GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C.
(2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND

(2) 2X12X0',1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. (JOB #7-100)

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Isaac Construction

Nick Patel Residence

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Mark Disosway P.E.
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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: May 29, 2007

DRAWN BY: David Disosway

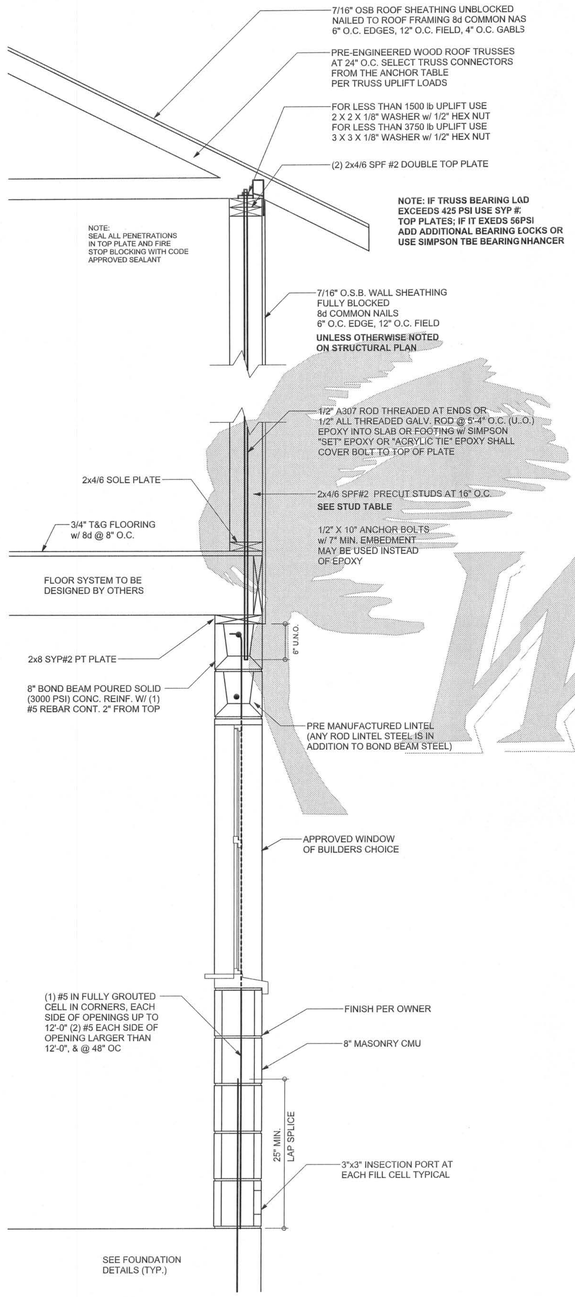
CHECKED BY:

FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

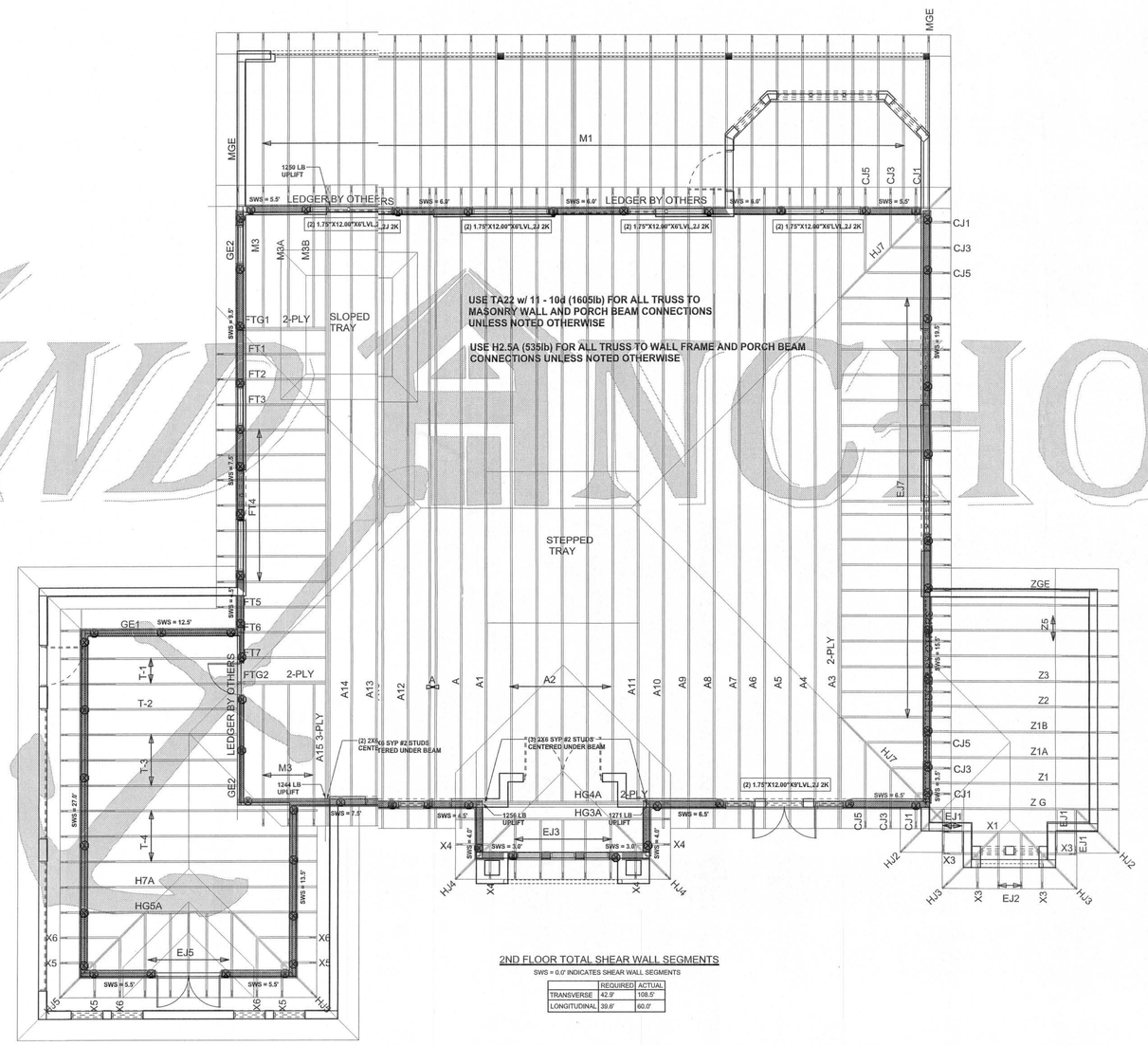
DRAWING NUMBER **S-3**

OF 4 SHEETS



TYPICAL 2 STORY BLOCK / FRAME WALL SECTION

SCALE: 1" = 1'-0"



2ND FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	42.9'	108.5'
LONGITUDINAL	39.6'	60.0'

STRUCTURAL ROOF PLAN

SCALE: 3/16" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

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1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

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3/16" TAPCONS @ 12" O.C.
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GARAGE DOOR UP TO 10'W

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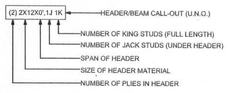
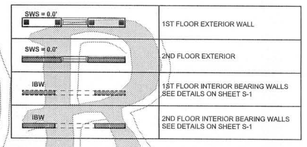
GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND



WALL LEGEND



INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

THREADED ROD LEGEND

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REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE №33915, POB 868, Lake City, FL 32056,386-754-5419

DIMENSIONS:
Stated fimensions supercede scaled dimencns. Refer all questions to Mark Isosway, P.E. for resolution. Do notproceed without clarification.

CERTFICATION: I hereby certify that I have examind this plan, and that the applicable portion of the plan, relating to wind engineering complywith section R301.2.1, florida building code residential 2004, to the best of my knowidge.

LIMITATION: This design is valid for one buildin, at specified location.

MARK DISOSWAY
P.E. 53915

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Nck Patel Residence

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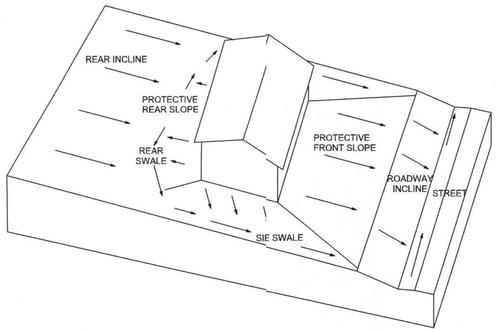
PRINTED DATE: May 29, 2007

DRWN BY: Dand Disosway | CHECKED BY:

FINLS DATE: 28' May / 07

JOB NUMBER: 705154

DRAWING NUMBER
S-4
OF 4 SHEETS



FHA / VA / HUD GRADING PATERN TYPE "A"
(ALL DRAINAGE TO S'REET)

SITE PLAN NOTES:

- THIS SITE PLAN IS NOT VALID WITHOUT OWNERS SIGNATURE TO GUARANTEE THE ACCURACY OF SETBACKS, LOT GEOMETRY, AND WELL, SEPTIC & BUILDING

- MINIMUM ACCEPTABLE GRADE IS 1%

-100 YEAR FLOOR ELEVATION WAS ESTABLISHED AS 110.4' BY ARTHUR BEDENBAGH AND REORDERED ON PLAT BOOK 6 PAGE 88 "SOUTHERN APPROACHES UNIT ON" IN THE NE 1/4 OF THE NW 1/4 OF SECTION 13, TOWNSHIP 4 SOUTH, RANGE 16 EAT, COLUMBIA COUNTY, FLORIDA.

- THE LOT WAS PERMITTED TO BE FILLED TO 110.5' OR ABOVE 100 YEAR FLOOD ELEVATION AS SHOWN ON PLAT.

- THE FINISHED FLOOR ELEVATION MUST BE MINIMUM 6" ABOVE FINISHED GRADE PER FBC2004. THE FINISHED GRADE SHOULD SLOPE DOWN FROM THAT ELEVAON FOR ANOTHER 6" WITHIN 12 FEET AWAY FROM THE HOUSE AND HANGER IN ALL DIRECTIONS SO THAT ALL RUNOFF DRAINS AWAY FROM THE HOUSE. THE OWNR MUST MAINTAIN THE SWALES, SLOPES, AND DITCH TO PROVIDE FREE DRAINAG TO THE DITCH AND PREVENT ANY POSSIBILITY OF STORM WATER BACKING UP INT THE HOUSE.

- THE OWNER SHOULD BE AWARE THAT IF FREE DRAINAGE IS NOT MAINTAINED 'IRU FIELDS AND SUBDIVISIONS ACROSS ROADS AND THRU CULVERTS, OR IF FUTUE DEVELOPMENT IN THE AREA CAUSES INCREASED STORM WATER RUN OFF, OR' RAINFALL OCCURS WITH GREATER FLOODING EFFECT THAN THE DESIGN STOR THE LEVEL OF THE RUNOFF WATER COULD RISE HIGHER THAN ANTICIPATED AD NEARBY ZONE "A" AREAS COULD BE LARGER THAN INDICATED BY FEMA AND HIHOUSE WOULD BE MORE SUSCEPTIBLE TO FLOODING (BASED ON PERSONAL OBSERVTION WITHOUT MEASUREMENT, THE STORMS OF 2004 CAUSED FLOODING THAT SEED TO BE BELOW THE LEVEL DEFINED BY FEMA AND THE PLAT PLAN IN AREAS NEA THIS HOME.)

FOOTINGS AND FOUNDATIONS NOTES:
1 - Foundations shall be built on undisturbed soil or properly compacted fill material. Foundions shall be constructed of materials described in the FBC2004
2 - Pile foundations shall be designed and constructed in accordance with the FBC2004
3 - The bottom of foundations shall extend no less than 12 inches (305 mm) below finish gde.
4 - Temporary buildings and buildings not exceeding one story in height and 400 sq ft in an shall be exempt from these requirements.
5 - Excavations for foundations shall be backfilled with soil which is free of organic material construction debris and large rocks.
6 - Where water impacts the ground from a roof valley, downspout, scupper or other rain wer collection or diversion device, provisions shall be made to prevent soil erosion and direct the wateaway from the foundation.
7 - Finish grade shall be sloped away from the foundation for drainage.
8 - The area under footings, foundations and concrete slabs on grade shall have all vegeton, stumps, roots and foreign materials removed prior to their construction. Fill material shall be freof vegetation and foreign material.

DRAINAGE PLAN KEY

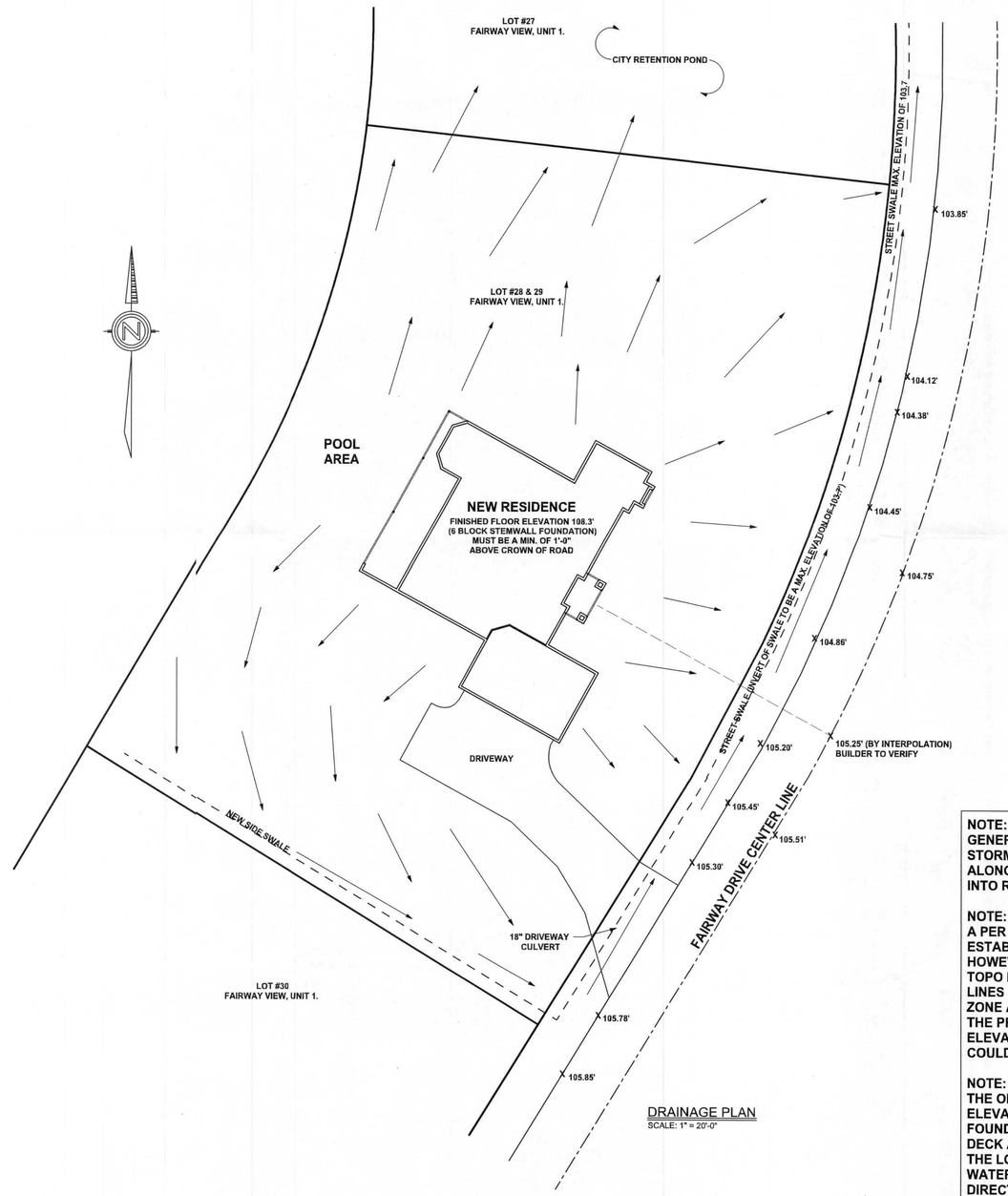
LOT LINE = ———————

INVERT OF DRAINAGE DITCH = – – – – –

CENTER LINE OF ROAD = — - — - —

DIRECTION OF WATER RUNOFF OF FINAL GRADE =

ELEVATION SHOT AS PROVIDED BY: DONALD F. LEE & ASSOCITATES, INC. (SEE ATTACHED SURVEY 06-5180) X 104.75'



DRAINAGE PLAN
SCALE: 1" = 20'-0"

NOTE: EXISTING GRADE ON LOTS # 28 & 29 GENERALLY SLOPES TO FRONT AND STORMWATER RUNNOFF RUNS INTO SWALE ALONG FAIRWAY DR RUNNING NORTHWARD INTO RETENTION POND.

NOTE: LOTS 28 AND 29 ARE IN FLOOD ZONE A PER FEMA RATE MAPS. THERE IS NO ESTABLISHED FLOOD ELEVATION. HOWEVER, VISUAL ALIGNMENT OF NGS TOPO MAP LINES AND FEMA RATE MAP LINES INDICATES IF THE AREA SHOWN AS ZONE A ON THE FEMA RATE MAP FLOODS THE PROPOSED FINISHED FLOOR ELEVATION OF THE PROPOSED HOUSE COULD BE ABOUT 5' UNDER WATER.

NOTE: THE CONTRACTOR STATES THAT THE ONLY FILL TO BE BROUGHT IN IS TO ELEVATE THE FILLED STEMWALL FOUNDATION OF THE HOUSE AND POOL DECK ABOUT 4' ABOVE EXISTING GRADE. THE LOT HAS NO EXISTING AREAS OF WATER STORAGE. THIS GRADING PLAN DIRECTS STORMWATER RUNOFF TO THE SAME LOCATION AS EXISTING STORMWATER RUNOFF.

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

25986

WINLOAD ENGINEER: Mark Disosway, PE N.53915, POB 868, Lake City, FL 32055, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to MarkDisosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, comply, florida building code residential 2004, to the best of my knowndge.

LIMITATION: This design is valid for one buildig, at specified location.

MARK DISOSWAY
P.E. 53915
28 JUN 07
SEAL

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PRINTED DATE: June 28, 2007

DRAWN BY: Evn Beamsley | CHECKED BY:

FINALS DATE: 28/ Jun / 07

JOB NUMBER: 705154

DRAWING NUMBER
D1
OF 1 SHEET

Daniel Shaheen

FRONT ELEVATION

SCALE: 1/4" = 1'

REAR ELEVATION

SCALE: 1/4" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PREEDENCE OVER SCALED DIMENSIONS

December 29, 2006

D.D.S. Studios

ARCHTECTURAL © DESIGN

P.O. Box 273
LAKE CITY FL. 32056
(386) 754-0181

DESIGNED BY:

NICHOLAS PAUL GEISLER ARCHITECT

ENGINEERED BY:

A CUSTOM HOME DESIGNED FOR:

NICK & RUPAL PATEL

EXTERIOR ELEVATIONS

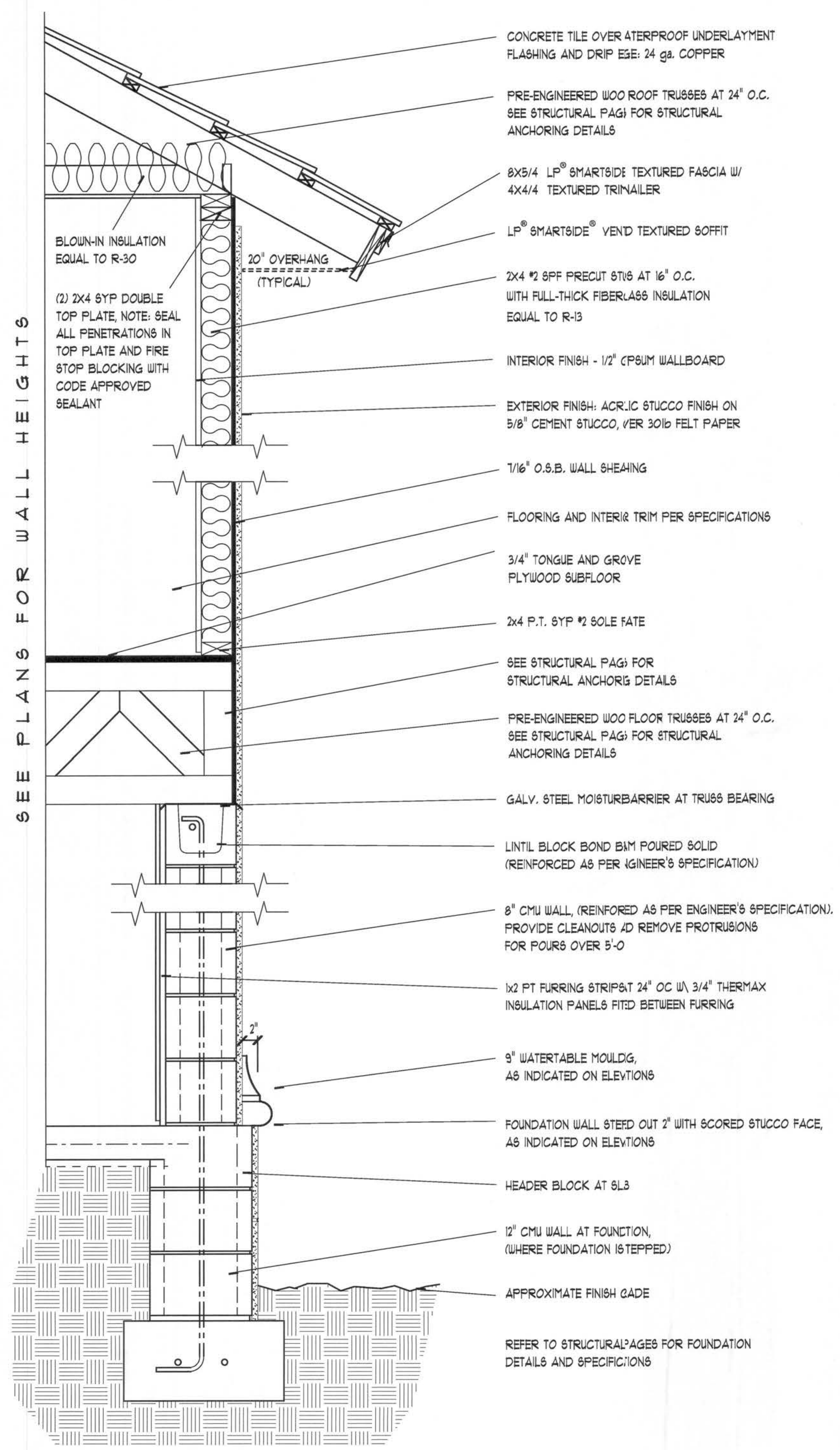
SHEET NUMBER

1 of 6

All work shall comply with the standard building code, and all applicable local codes ad ordinances.

Contractor shall verify all dimensions prior to commencing construction.

Daniel Shaheen



TWO-STORY TYPICAL WALL SECTION

SCALE: 1" = 1'
REFER TO STRUCTURAL PAGE FOR STRUCTURAL SPECIFICATIONS

LEFT ELEVATION

SCALE: 1/4" = 1'

RIGHT ELEVATION

SCALE: 1/4" = 1'

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December 29, 2006

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Lake City, FL 32055
386/755-9021

ENGINEERED BY:

A CUSTOM HOME DESIGNED FOR:

NICK & RUPAL PATEL

EXTERIOR ELEVATIONS

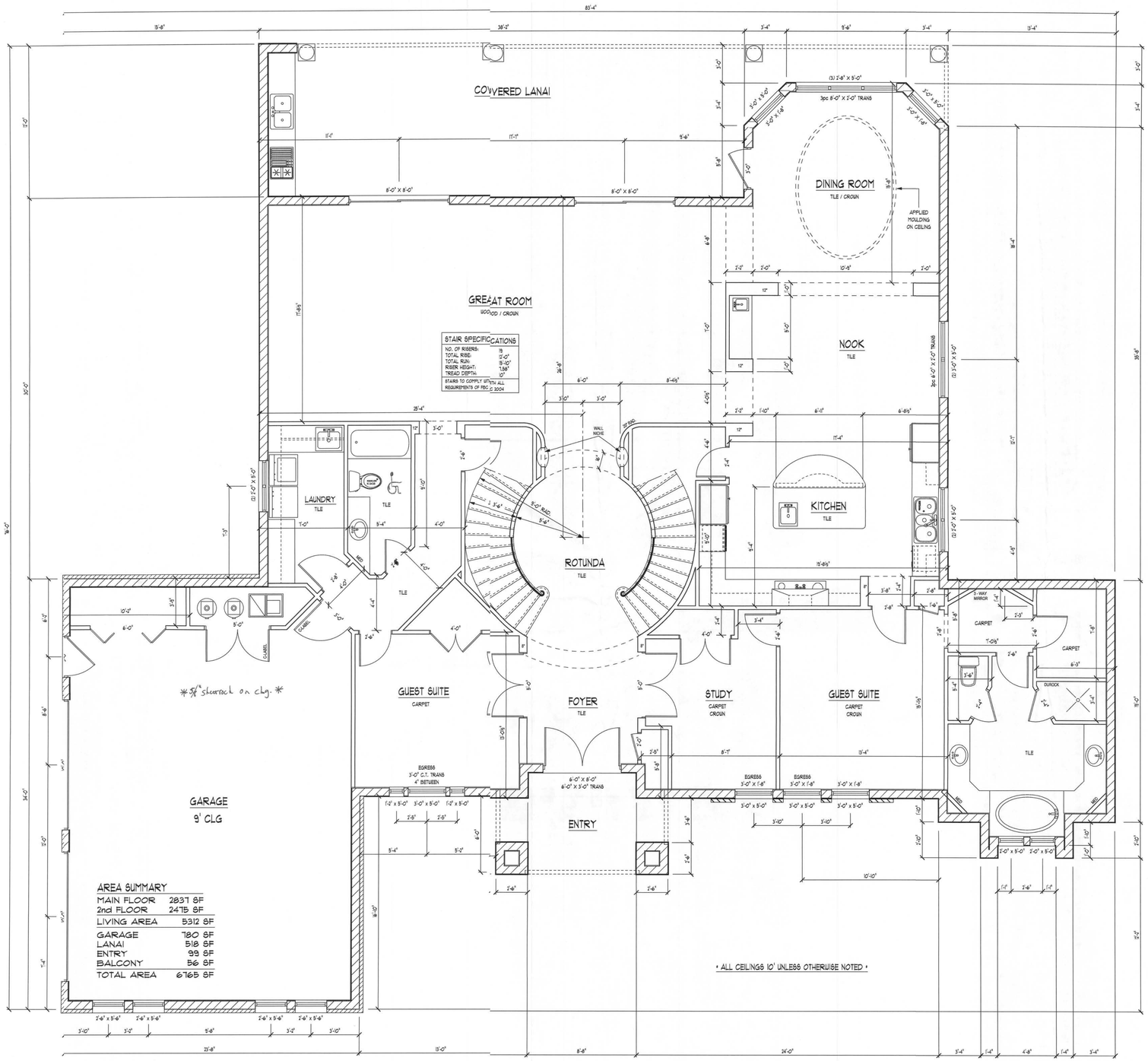
TYPICAL WALL SECTION

SHEET NUMBER
2 of 6

All wck shall comply with the stndard building code, and al applicable local code and ordinances.

Contrctor shall verify all dimensions prior to commencing construction.

Daniel Shaheen



FLOOR PLAN

SCALE: 1/4" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAK: PRECEDENCE OVER SCALED DIMENSIONS

December 29, 2006

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FLOOR PLAN

SHEET NUMBER
3 of 6

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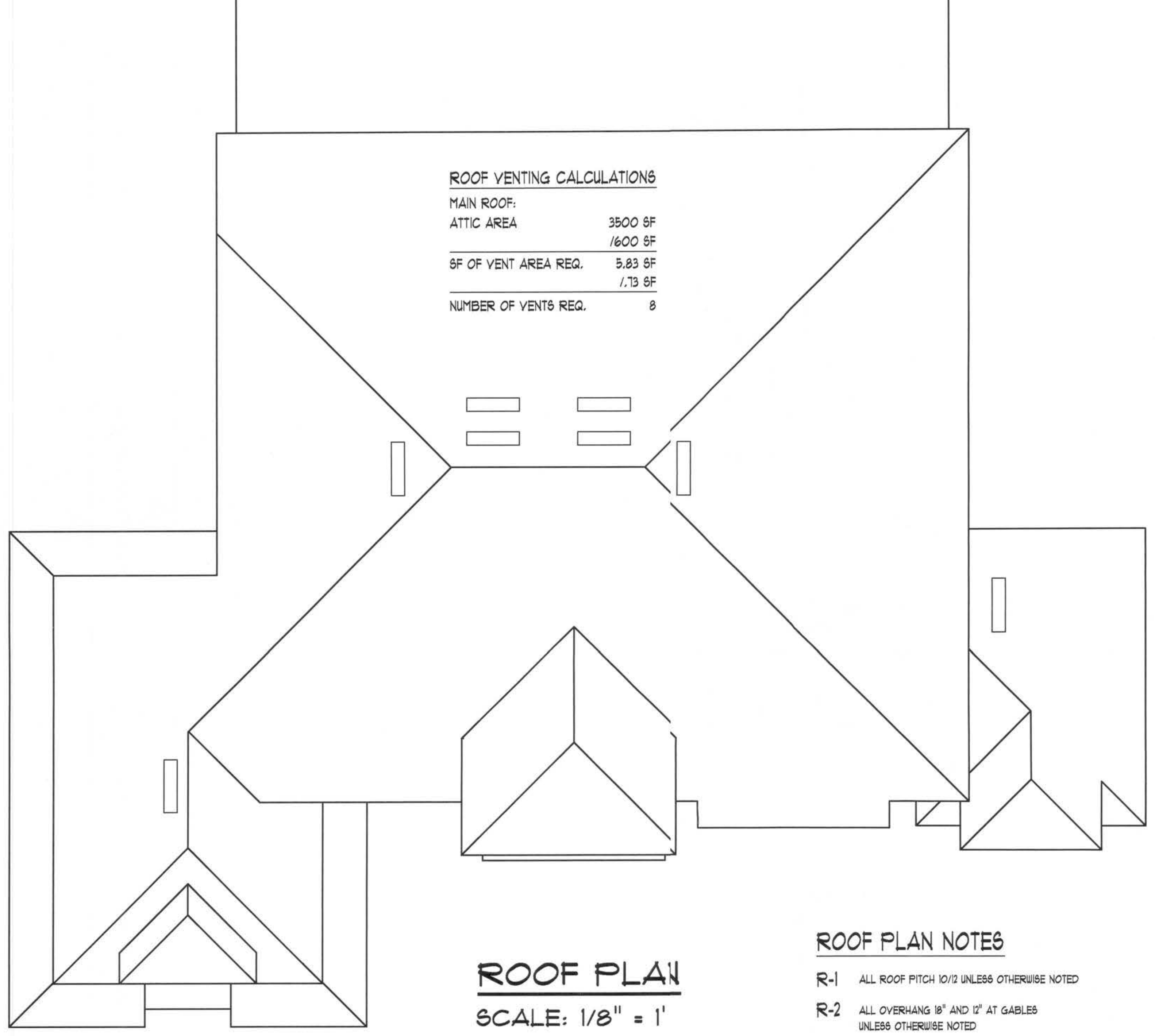
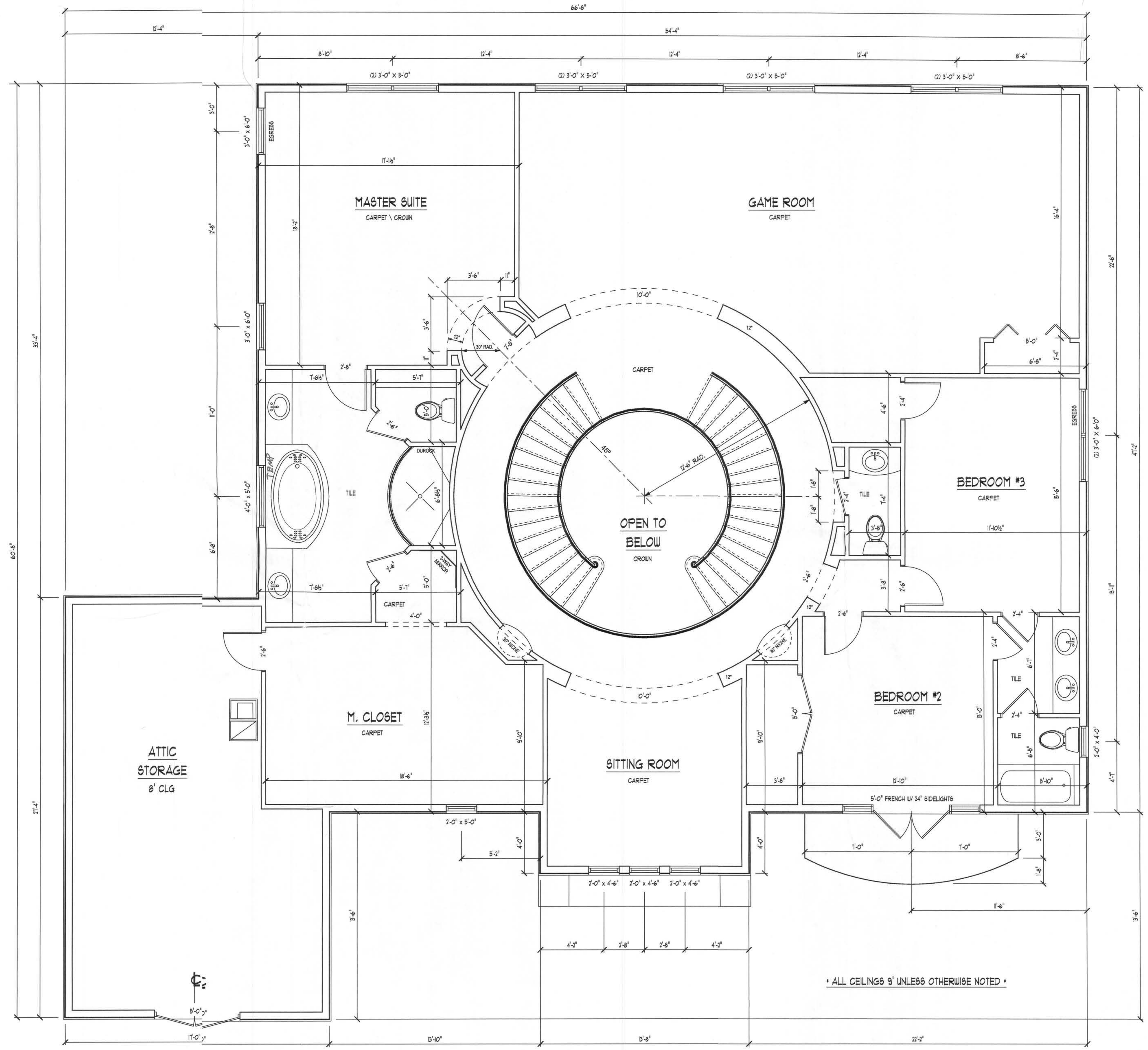


ROOF PLAN
SCALE: 1/8" = 1'

ROOF PLAN NOTES

- R-1 ALL ROOF PITCH 10/12 UNLESS OTHERWISE NOTED
- R-2 ALL OVERHANG 18" AND 12" AT GABLES UNLESS OTHERWISE NOTED
- R-3 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
- R-4 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HEEL HEIGHTS
- R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR

Daniel Shaheen



2nd FLOOR PLAN
SCALE: 1/4" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

December 29, 2006

D.D.S. Studios
ARCHITECTURAL © DESIGN

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(386 754-0181

DESIGNED BY:

NICHOLAS PAUL GEISLER ARCHITECT
N.C.A.R.B. Certified
Route 17, Box 1038
Lake City, FL 32055
386/755-9021

ENGINEERED BY:

A CUSTOM HOME DESIGNED FOR:
NICK & RUPAL PATEL

2nd FLOOR PLAN
ROOF PLAN

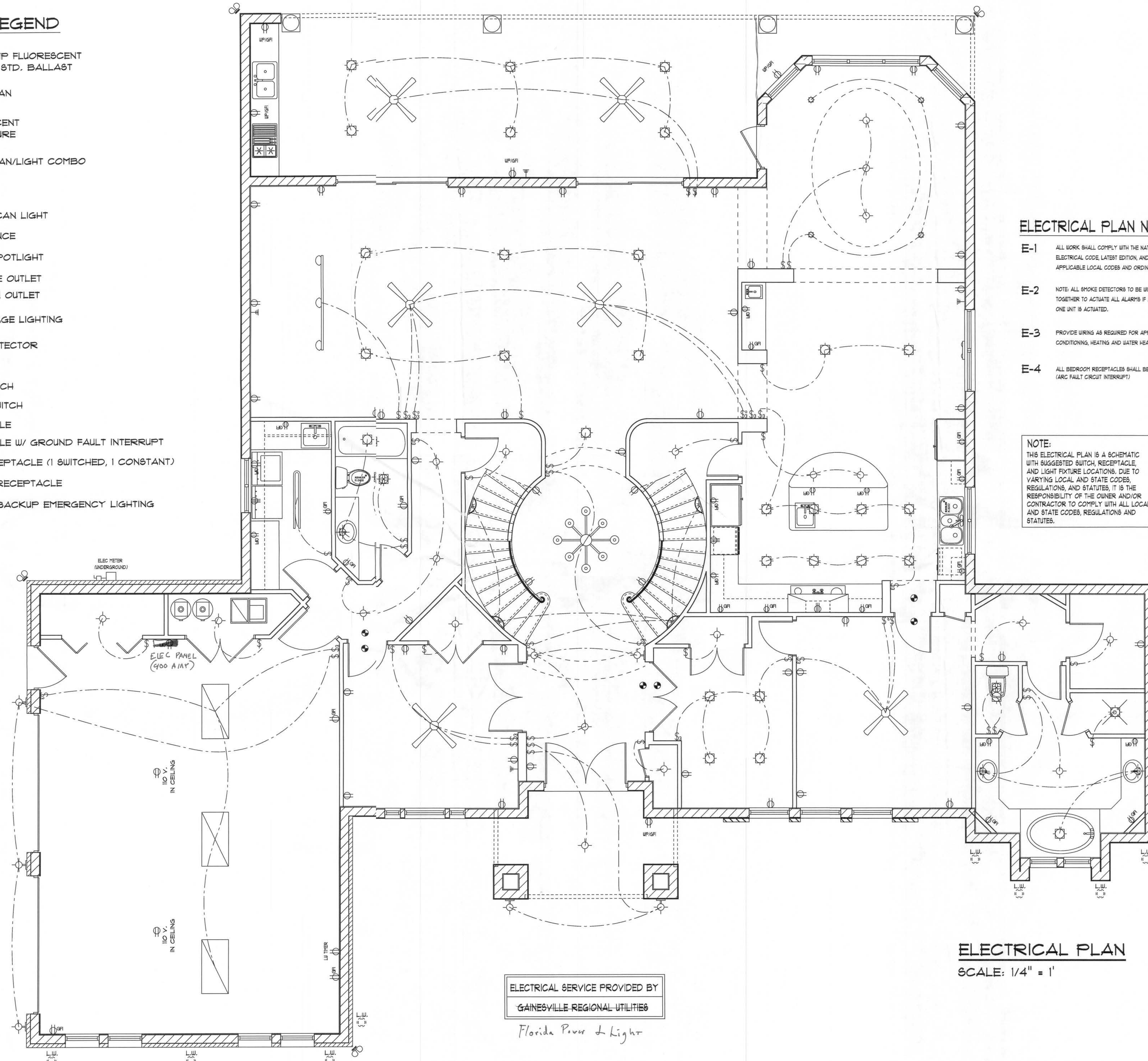
SHEET NUMBER
4 of 6

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

ELECTRICAL LEGEND

⧄	2'X4' 4-LAMP FLUORESCENT LIGHT WITH STD. BALLAST
	EXHAUST FAN
	INCANDESCENT LIGHT FIXTURE
	EXHAUST FAN/LIGHT COMBO
	CAN LIGHT
	EYEBALL CAN LIGHT
	WALL SCONCE
	DOUBLE SPOTLIGHT
	TELEPHONE OUTLET
	TELEVISION OUTLET
	LOW WATTAGE LIGHTING
	SMOKE DETECTOR
	SWITCH
$_3$	3-WAY SWITCH
$	DIMMER SWITCH
	RECEPTACLE
	RECEPTACLE W/ GROUND FAULT INTERRUPT
◐	SPLIT RECEPTACLE (1 SWITCHED, 1 CONSTANT)
	220 VOLT RECEPTACLE
	BATTERY BACKUP EMERGENCY LIGHTING



ELECTRICAL PLAN NOTES

E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.

E-2 NOTE: ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTUATE ALL ALARMS IF ANY ONE UNIT IS ACTUATED.

E-3 PROVIDE WIRING AS REQUIRED FOR APPLIANCES, AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.

E-4 ALL BEDROOM RECEPTACLES SHALL BE AFCI. (ARC FAULT CIRCUIT INTERRUPT)

NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

ELECTRICAL PLAN

SCALE: 1/4" = 1'

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

[illegible]
Daniel Shaheen

December 29, 2006

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Route 17, Box 1038
Lake City, FL 32055
386/755-9021

ENGINEERED BY:

A CUSTOM HOME DESIGNED FOR:
NICK & RUPAL PATEL

ELECTRICAL PLAN

SHEET NUMBER
5 of 6

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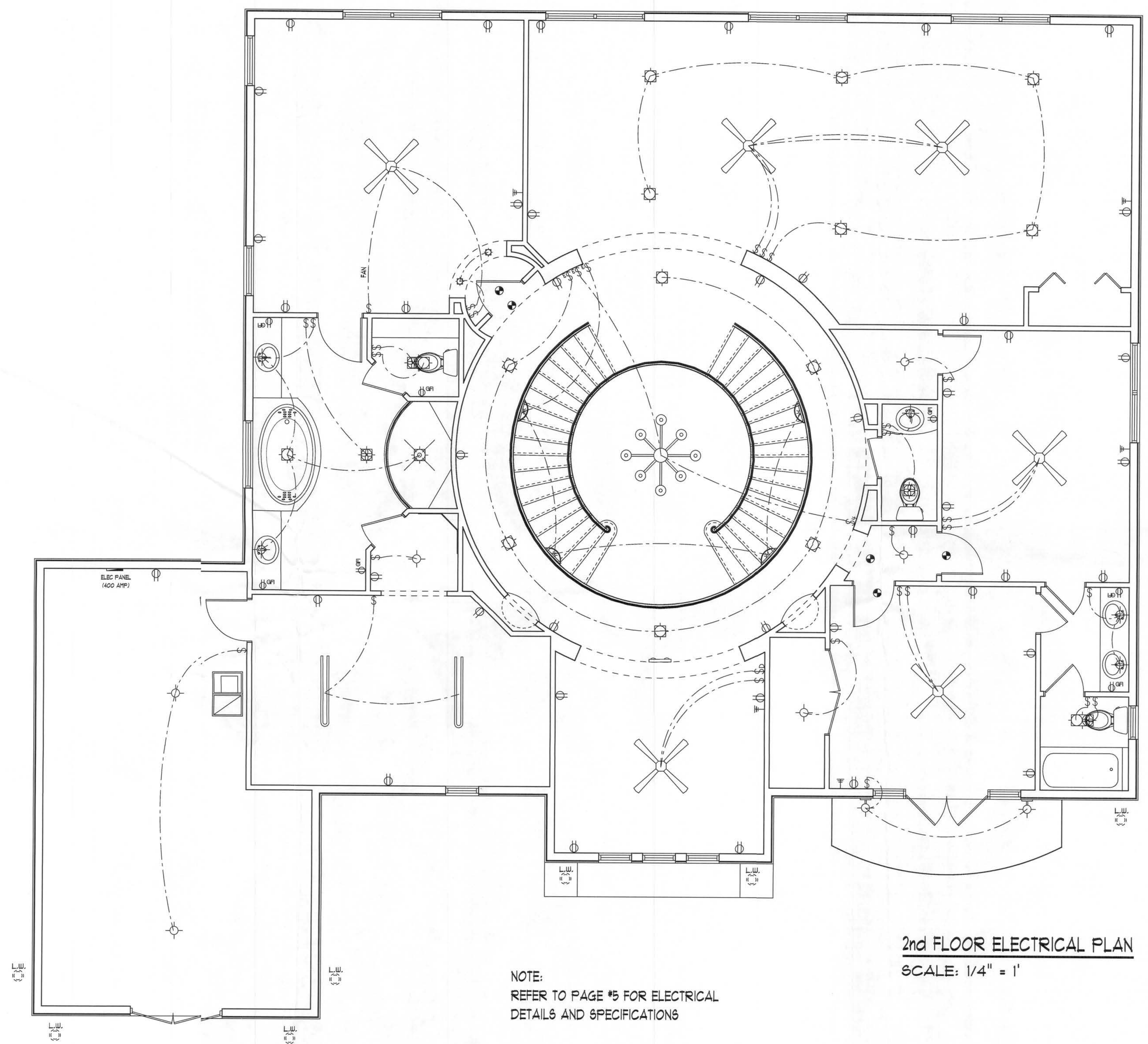
Daniel Shaheen



2nd FLOOR ELECTRICAL PLAN

SCALE: 1/4" = 1'

NOTE:
REFER TO PAGE #5 FOR ELECTRICAL DETAILS AND SPECIFICATIONS

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECIDENCE OVER SCALED DIMENSIONS

December 29, 2006

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P.O. Box 273
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DESIGNED BY:

NICHOLAS PAUL GEISLER ARCHITECT

ENGINEERED BY:

A CUSTOM HOME DESIGNED FOR:

NICK & RUPAL PATEL

2nd FLOOR ELECTRICAL PLAN

SHEET NUMBER
6 of 6

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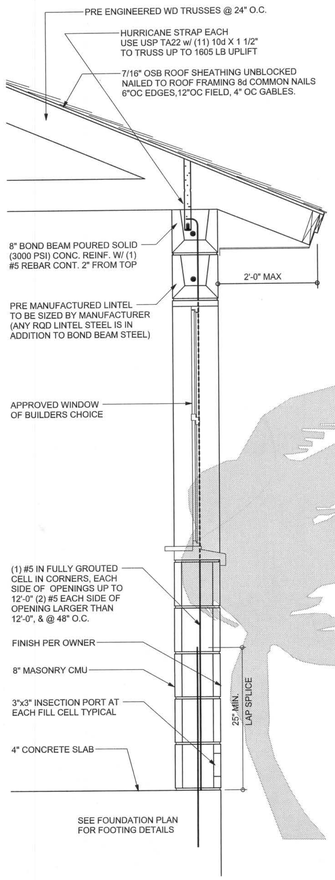
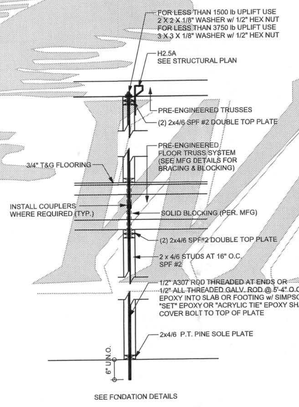


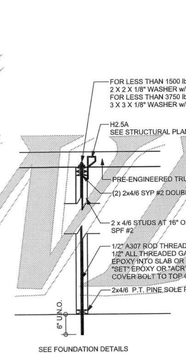
TYPICAL 1 STORY BLOCK WALL SECTION

SCALE: 1" = 1'-0"



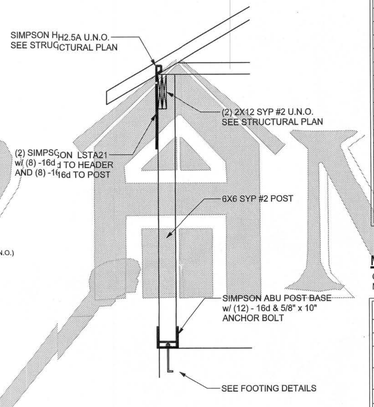
TWO STORYNTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1085	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004, TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE, STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

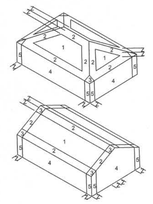
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL, FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



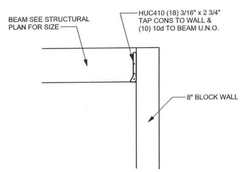
Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

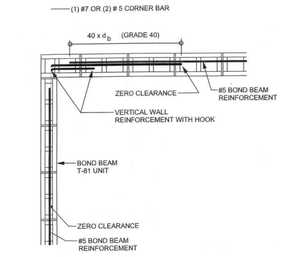
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT



BEAM TO BLOCK CONNECTION DETAIL

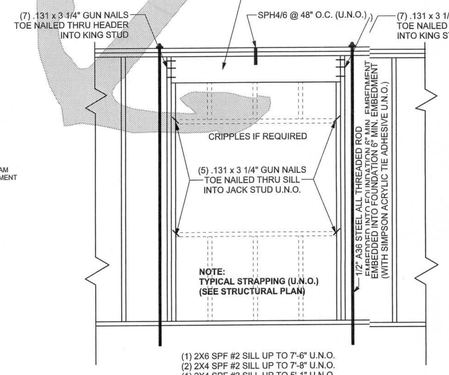
SCALE: 1/2" = 1'-0"



TYPICAL BOND BEAM CORNER DETAIL

SCALE: 1/2= 1"=0"

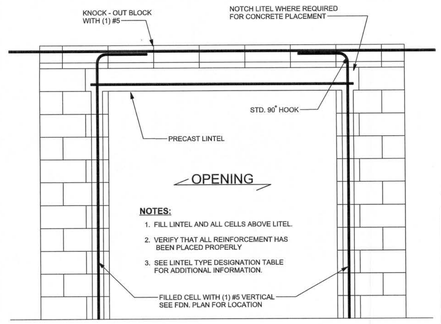
NOTE:
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.
(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.
(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.
(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING D) DETAIL

SCALE: 1/2" = 1'-0"



TYPICAL FILLED LINTEL ASSEMBLY

SCALE: 1/2" = 1'=0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

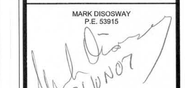
WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915



SEAL

Isaac Construction

Nick Patel Residence

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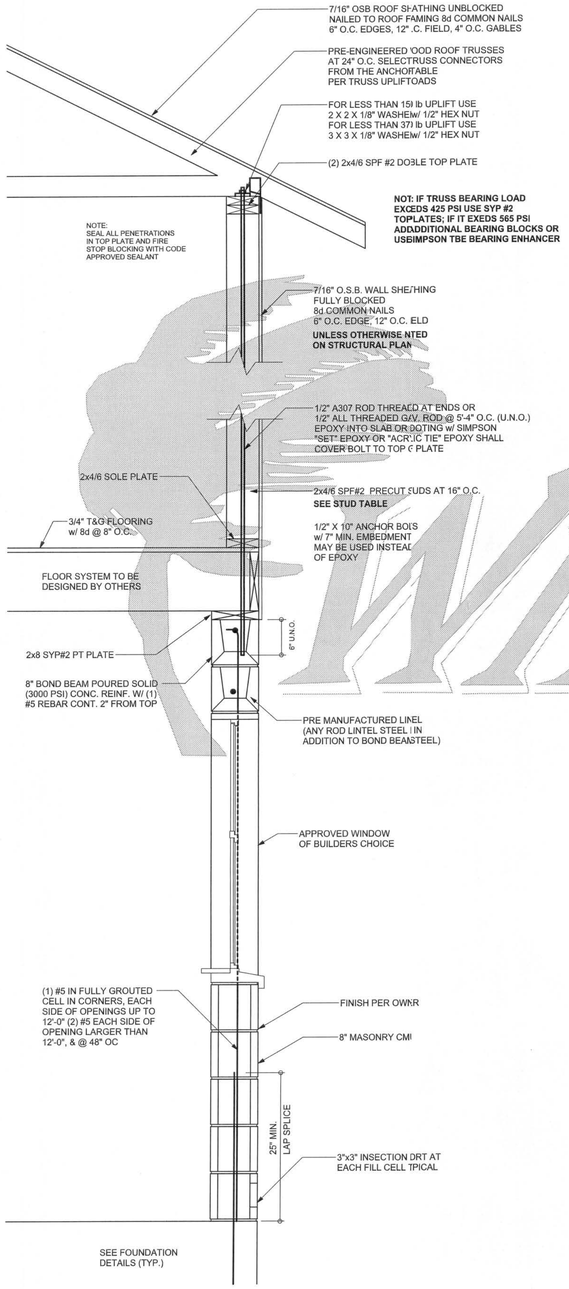
PRINTED DATE:
June 21, 2007

DRAWN BY: David Disosway
CHECKED BY:

FINALS DATE:
28 / May / 07

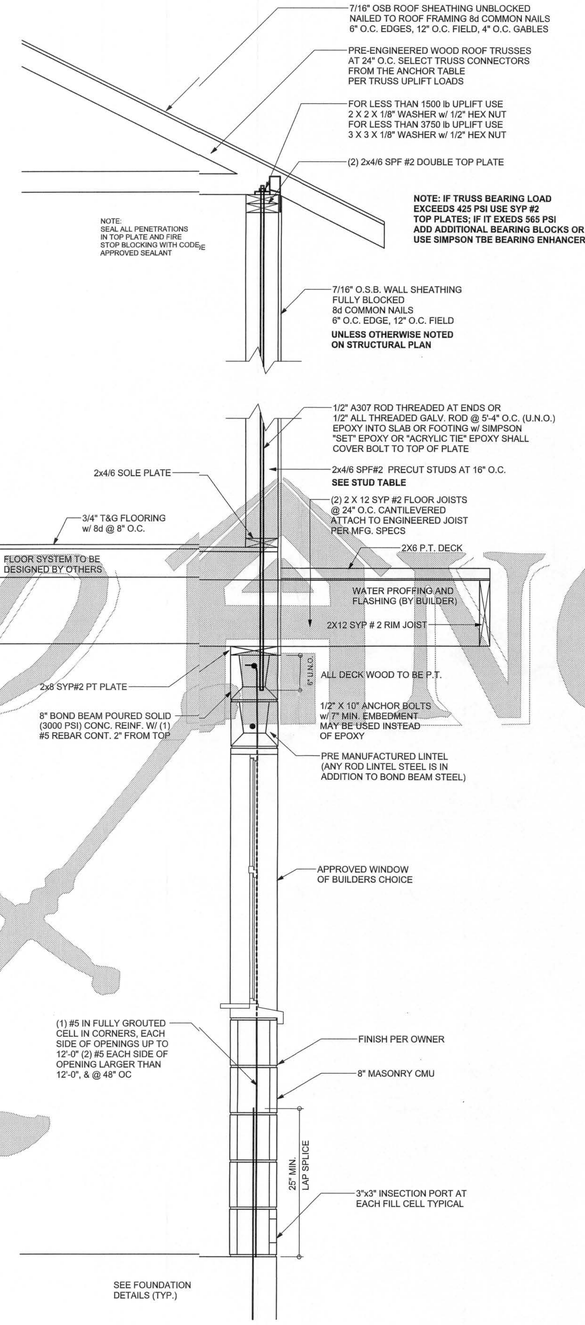
JOB NUMBER:
705154

DRAWING NUMBER
S-1
OF 5 SHEETS



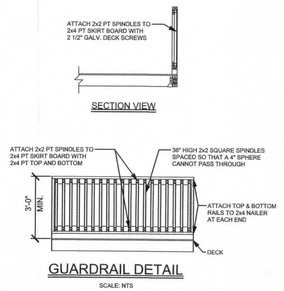
TYPICAL 2 STORY BLOCK / FRAME WALL SECTION

SCALE: 1" = 1'-0"



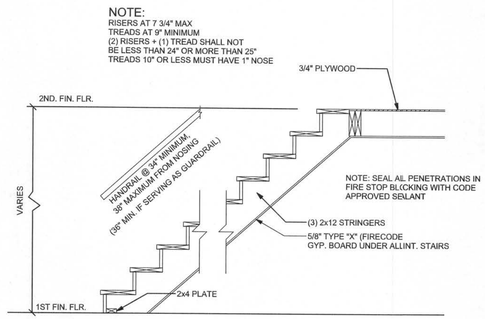
TYPICAL 2 STORY BLOCK / FRAME WALL SECTION @ CANTILEVERED BALCONIES

SCALE: 1" = 1'-0"



GUARDRAIL DETAIL

SCALE: NTS



1403 04/01/03 STAIR DETAIL

SCALE: NTS

REVISIONS	
6/21/2007	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.5385, POB 868, Lake City, FL 32056, 386754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proved without clarification.

CERTIFICTION: I hereby certify that I have examined 'is plan, and that the applicable portions ofhe plan, relating to wind engineering comply wit section R301.2.1, florida building code residntial 2004, to the best of my knowledge

LIMITATIO: This design is valid for one building, aspecified location.

MARK DISOSWAY P.E. 53915

SEAL

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PRINTED DATE: June 21, 2007

DRAWI BY: David Disosway | CHECKED BY:

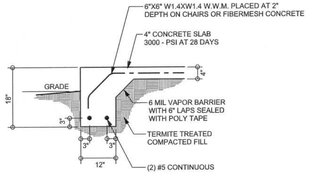
FINALSDATE: 28 / My / 07

JOB NUMBER: 705154

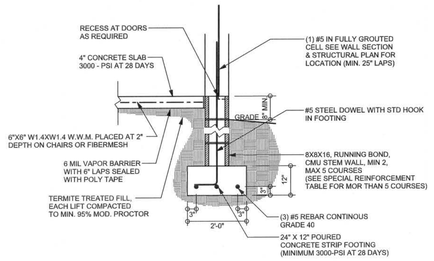
DRAWING NUMBER

S-1.1

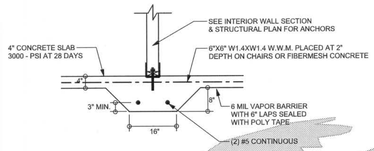
OF 5 SHEETS



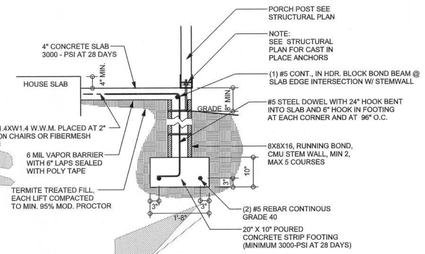
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



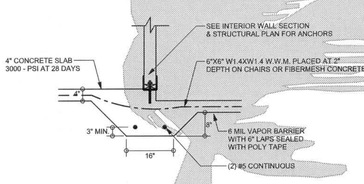
F17 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



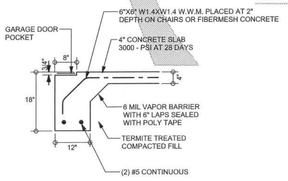
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



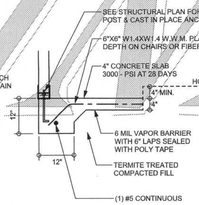
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



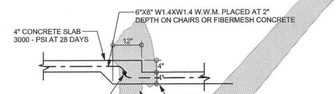
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

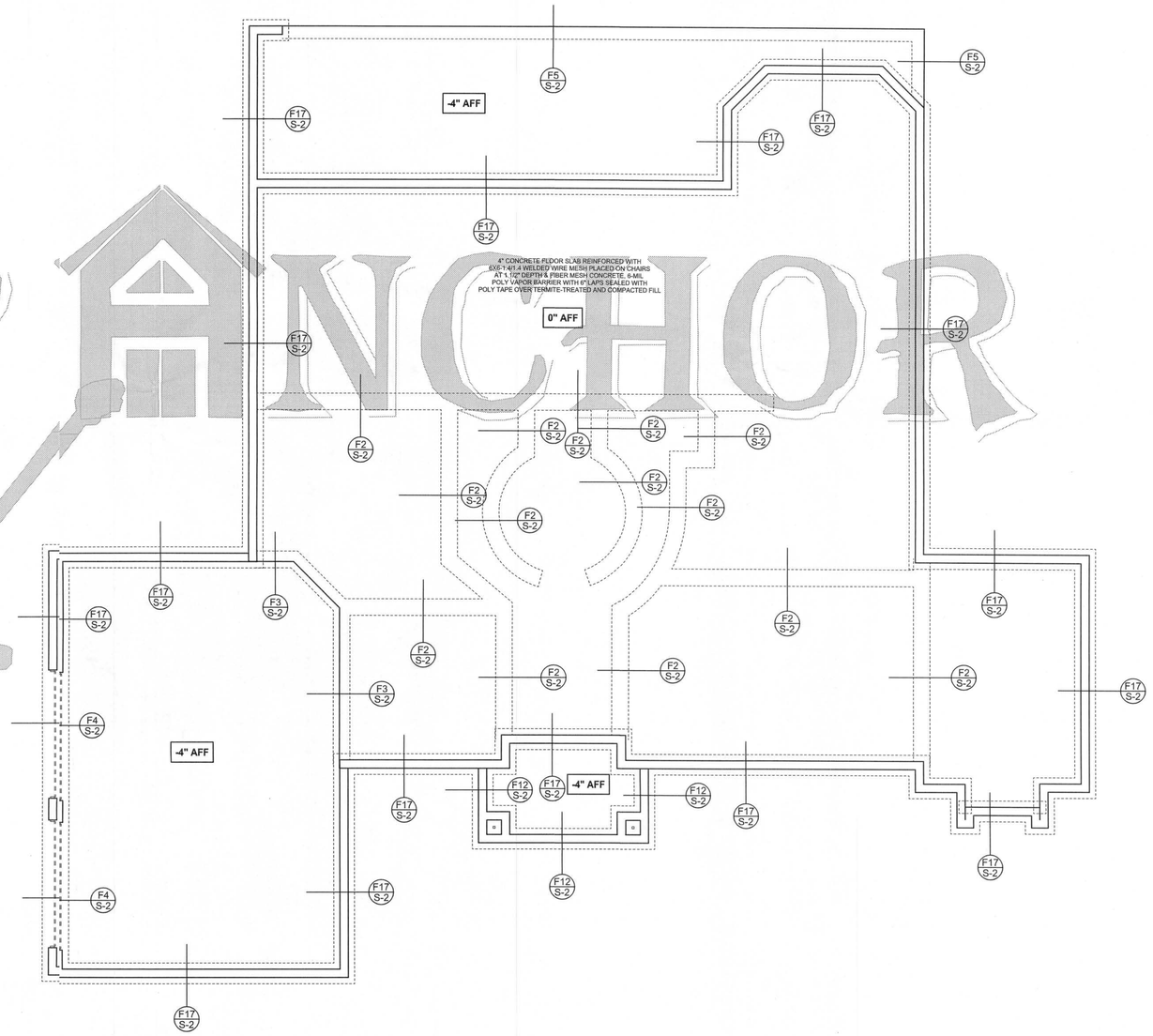
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN
SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE N OT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, FL No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Sated dimensions supercede scaled dmensions. Refer all questions to fark Disosway, P.E. for resolution. D not proceed without clarification.

CRTIFICATION: I hereby certify that I have eamined this plan, and that the applicable prtions of the plan, relating to wind engineering cmply with section R301.2.1, florida building cde residential 2004, to the best of my kowledge.

LMITATION: This design is valid for one bilding, at specified location.

MARK DISOSWAY P.E. 53915

SEAL

Isaac Construction

Nick Patel Residence

ADDRESS:
442 NW Fairway Drive
Lake City, Florida 32055

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: June 21, 2007

DRAWN BY: David Disosway

CHECKED BY:

FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

DRAWING NUMBER
S-2
OF 5 SHEETS

MATERIALS

1. f'c 8" precast lintel = 3500 psi
2. f'c prestressed lintel = 6000 psi
3. Grout per ASTM C476 f'c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

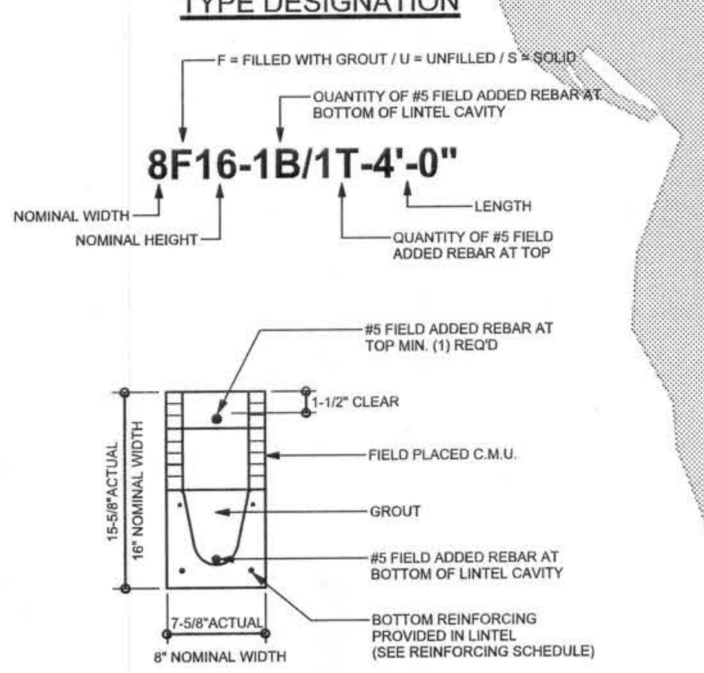
1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintels lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction 6472 ⇒ (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAS & PRESTRESSED U-LINTELS

CAST-CRETE — SAFE LOA - POUNDS PER LINEAR FOOT

LENGTH		TYPE	8U8	8F8-0B / 8F8-1B	8F12- / 8F12-	8F16-0B / 8F16-1B	8F20-0B / 8F20-1B	8F24-0B / 8F24-1B	8F28-0B / 8F28-1B	8F32-0B / 8F32-1B
2'-10"	(34")	PRECAST	2231	3069	460	6113	7547	8974	10394	11809
				3069	460	6113	7547	8974	10394	11809
3'-6"	(42")	PRECAST	2231	3069	371	5163	6607	8054	9502	10951
				3069	460	6113	7547	8974	10394	11809
4'-0"	(48")	PRECAST	1966	2561	275	3820	4890	5961	7034	8107
				2693	460	6113	7547	8974	10394	11809
4'-6"	(54")	PRECAST	1599	1969	211	2931	3753	4576	5400	6224
				2189	437	6113	7547 (7)	8672	10294	11809
5'-4"	(64")	PRECAST	1217	1349	143	1999	2560	3123	3686	4249
				1663	309	5365	7547 (36)	7342 (19)	8733 (19)	10127 (19)
5'-10"	(70")	PRECAST	1062	1105	117	1631	2090	2549	3009	3470
				1451	262	4360	7168 (45)	6036 (19)	7181 (19)	8328 (20)
6'-6"	(78")	PRECAST	908	1238	217	3480	3031	3707	4383	5061
				1238	217	3480	5381	8360	10394 (37)	8825 (14)
7'-6"	(90")	PRECAST	743	1011	172	2632	2205	2698	3191	3685
				1011	172	2661	3898	5681	8467 (44)	6472 (15)
9'-4"	(112")	PRECAST	554	699	116	1625	2564	3486	2818	3302
				752	124	1843	2564	3486	4705 (37)	6390 (47)
10'-6"	(126")	PRECAST	475	535	89	1247	2093	2777	2163	2536
				643	105	1533	2093	2781	3643 (38)	4754 (45)
11'-4"	(136")	PRECAST	362	582	94	1366	1846	2423	3127	4006
				582	94	1366	1846	2423	3127	4006
12'-0"	(144")	PRECAST	337	540	87	1254	1684	2193	2805	3552
				540	87	1254	1684	2193	2805	3552
13'-4"	(160")	PRECAST	296	471	75	1075	1428	1838	2316	2883
				471	75	1075	1428	1838	2316	2883
14'-0"	(168")	PRECAST	279	424	70	1002	1326	1697	2127	2630
				442	70	1002	1326	1697	2127	2630
14'-8"	(176")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				458	78	1370	1902	2245	2517	2712
15'-4"	(184")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				412	71	1250	1733	2058	2320	2513
17'-4"	(208")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				300	53	950	1326	1609	1849	2047
19'-4"	(232")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				235	41	750	1037	1282	1515	1716
21'-4"	(256")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				180	34	598	845	1114	1359	1468
22'-0"	(264")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				165	31	550	784	1047	1285	1399
24'-0"	(288")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
				129	25	450	654	884	1092	1222

SAFE UPLIFT LOADS FOR 8" PRECAST & RESTRESSED U-LINTELS

CAST-CRETE — SAFE LOAD - PUNDS PER LINEAR FOOT

LENGTH		TYPE	8F8-1T / 8F8-2T	8F12-1T / 8F12-2T	8F16T / 8F16T	8F20-1T / 8F20-2T	8F24-1T / 8F24-2T	8F28-1T / 8F28-2T	8F32-1T / 8F32-2T
2'-10"	(34")	PRECAST	1972	3173	44	5747	7034	8321	9608
			1972	3173	44	5747	7034	8321	9608
3'-6"	(42")	PRECAST	1569	2524	35	4569	5591	6613	7636
			1569	2524	35	4569	5591	6613	7636
4'-0"	(48")	PRECAST	1363	2192	30	3966	4853	5740	6627
			1363	2192	30	3966	4853	5740	6627
4'-6"	(54")	PRECAST	1207	1940	27	3508	4292	5077	5861
			1207	1940	27	3508	4292	5077	5861
5'-4"	(64")	PRECAST	1016	1632	22	2949	3607	4265	4924
			1016	1632	22	2949	3607	4265	4924
5'-10"	(70")	PRECAST	909	1492	20	2694	3295	3897	4498
			929	1492	20	2694	3295	3897	4498
6'-6"	(78")	PRECAST	835 (12)	1340	18	2419	2959	3498	4038
			835	1340	18	2419	2959	3498	4038
7'-6"	(90")	PRECAST	727 (23)	1021	16 (12)	2102 (11)	2571 (10)	3039 (10)	3508 (9)
			727	1166	16	2102	2571	3039	3508
9'-4"	(112")	PRECAST	591	680	11 (15)	1471 (15)	1811 (15)	2152 (16)	2494 (15)
			591	851	13	1705	2084	2463	2842
10'-6"	(126")	PRECAST	530	552	91 (15)	1185 (15)	1458 (15)	1732 (15)	2007 (15)
			530	686	11	1526	1865	2204	2544
11'-4"	(136")	PRECAST	474	485	79 (15)	1034 (15)	1272 (15)	1510 (15)	1749 (15)
			494	599	10	1422	1738	2053	2369
12'-0"	(144")	PRECAST	470 (9)	441	72 (14)	936 (14)	1151 (15)	1366 (15)	1582 (15)
			470	543	92	1349	1649	1948	2247
13'-4"	(160")	PRECAST	418 (15)	373	60 (14)	783 (14)	962 (14)	1141 (14)	1321 (14)
			428	455	77	1145	1444	1718	1993
14'-0"	(168")	PRECAST	384 (15)	346	55 (14)	723 (14)	887 (14)	1052 (14)	1218 (14)
			410	420	70	1050	1434 (8)	1694 (8)	1954 (7)
14'-8"	(176")	PRESTRESSED	239	323	51 (13)	671 (13)	823 (13)	976 (14)	1129 (14)
			246	390	65	968	1324 (8)	1625 (11)	1874 (11)
15'-4"	(184")	PRESTRESSED	224	302	48 (13)	626 (13)	767 (13)	909 (13)	1052 (13)
			230	364	60	897	1224 (8)	1562 (14)	1801 (14)
17'-4"	(208")	PRESTRESSED	187	255	40 (12)	520 (12)	637 (12)	754 (12)	872 (12)
			192	303	50	732	993 (8)	1268 (14)	1470 (14)
19'-4"	(232")	PRESTRESSED	162	222	34 (11)	446 (11)	546 (12)	646 (12)	746 (12)
			166	261	42	616	831 (8)	1057 (14)	1225 (14)
21'-4"	(256")	PRESTRESSED	142	198	30 (11)	393 (11)	480 (11)	567 (11)	654 (11)
			142	230	31	531	713 (7)	903 (13)	1046 (13)
22'-0"	(264")	PRESTRESSED	137	192	29 (10)	378 (11)	461 (10)	545 (11)	629 (11)
			137	221	35	508	681 (7)	861 (13)	997 (13)
24'-0"	(288")	PRESTRESSED	124	175	26 (10)	341 (10)	416 (10)	491 (10)	566 (10)
			124	200	3	450	600 (7)	756 (12)	875 (13)

SAFE GRAVITY LOADS FOR 8" PRECAS w/ 2" RECESS DOOR U-LINTELS

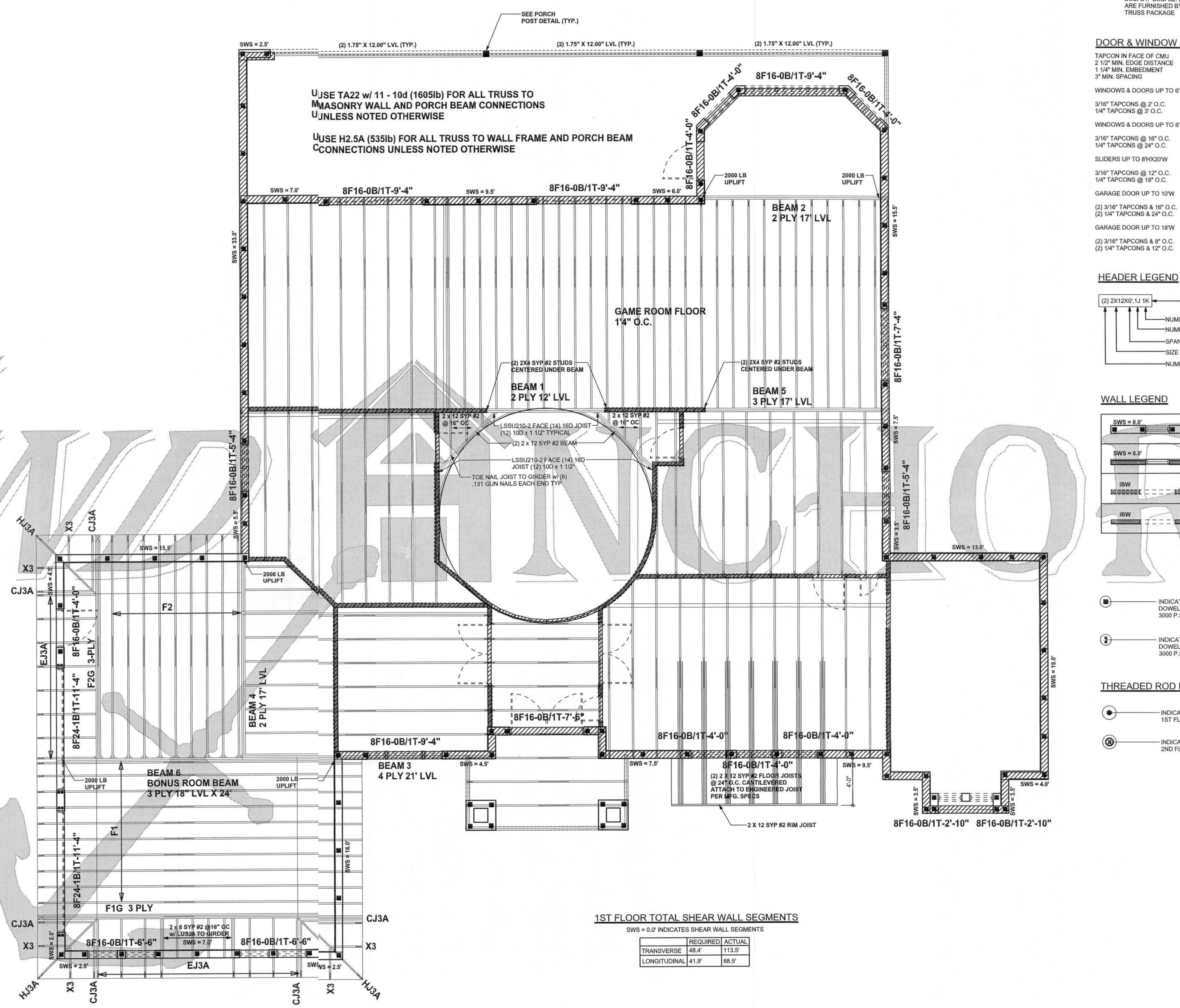
CAST-CRETE — SAFE LOD - POUNDS PER LINEAR FOOT

LENGTH		TYPE	8RU6	8RF6-0B / 8RF6-1B	8RF10B / 8RF1B	8RF14-0B / 8RF14-1B	8RF18-0B / 8RF18-1B	8RF22-0B / 8RF22-1B	8RF26-0B / 8RF26-1B	8RF30-0B / 8RF30-1B
4'-4"	(52")	PRECAST	1635	1749	33	3280	4349	5421	6493	7567
				1891	36	5206	6639	8060	9479	10893
4'-6"	(54")	PRECAST	1494	1596	30	2992	3968	4946	5924	6904
				1756	36	5206	6639	8060	9479	10893
5'-8"	(68")	PRECAST	866	920	17	1716	2277	2839	3402	3966
				1167	24	4567	6389	8060 (34)	7917 (19)	9311 (19)
5'-10"	(70")	PRECAST	810	859	16	1600	2124	2649	3174	3700
				1113	23	4242	6639 (10)	8060 (39)	7402 (19)	8706 (19)
6'-8"	(80")	PRECAST	797	901	18	3120	5048	7747	9448	7360
				901	18	3120	5048	7915	9479	10893 (32)
7'-6"	(90")	PRECAST	669	755	14	2459	3776	5743	7239	5623
				755	14	2459	3776	5743	8998 (19)	10893 (48)
9'-8"	(116")	PRECAST	411	466	9	1568	2253	3129	4091	3146
				526	9	1568	2253	3129	4150	5891 (47)

SAFE UPLIFT LOADS FOR 8" PRECAST w2" RECESS DOOR U-LINTELS

CAST-CRETE — SAFE LOAD - DUNDS PER LINEAR FOOT

LENGTH		TYPE	8RF6-1T / 8RF6-2T	8RF10-1T / 8RF10-2T	8RF11T / 8RF12T	8RF18-1T / 8RF18-2T	8RF22-1T / 8RF22-2T	8RF26-1T / 8RF26-2T	8RF30-1T / 8RF30-2T
4'-4"	(52")	PRECAST	905	1748	26	3522	4409	5296	6183
			905	1748	26	3522	4409	5296	6183
4'-6"	(54")	PRECAST	867	1675	25	3374	4224	5074	5924
			867	1675	25	3374	4224	5074	5924
5'-8"	(68")	PRECAST	675	1301	19	2618	3277	3935	4594
			675	1301	19	2618	3277	3935	4594
5'-10"	(70")	PRECAST	655	1262	19	2538	3176	3815	4453
			655	1262	19	2538	3176	3815	4453
6'-8"	(80")	PRECAST	570	1012	16	2204	2758	3312	3865
			570	1097	16	2204	2758	3312	3865
7'-6"	(90")	PRECAST	506	797	14 (8)	1952 (7)	2442 (6)	2931 (6)	3257
			506	967	14	1952	2442	2931	3421
9'-8"	(116")	PRECAST	395	491	9 (12)	1301 (15)	1640 (15)	1980 (15)	2322 (16)
			395	589	11	1514	1893	2272	2652



STRUCTURAL FLOOR PLAN
SCALE: 3/16" = 1'-0"

1ST FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	48.4'	113.5'
LONGITUDINAL	41.9'	88.5'

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C.
1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C.
1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W

3/16" TAPCONS @ 12" O.C.
1/4" TAPCONS @ 18" O.C.

GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C.
(2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND

(2) 2X12X0',1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. (JOB #7-100)

REVISIONS

WNDLOAD ENGINEER: Mark Disosway, PI No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
SEAL

Isaac Construction

Nick Patel Residence

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442 NW Fairway Drive
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PRINTED DATE: June 21, 2007

DRAWN BY:	CHECKED BY:
David Disosway	

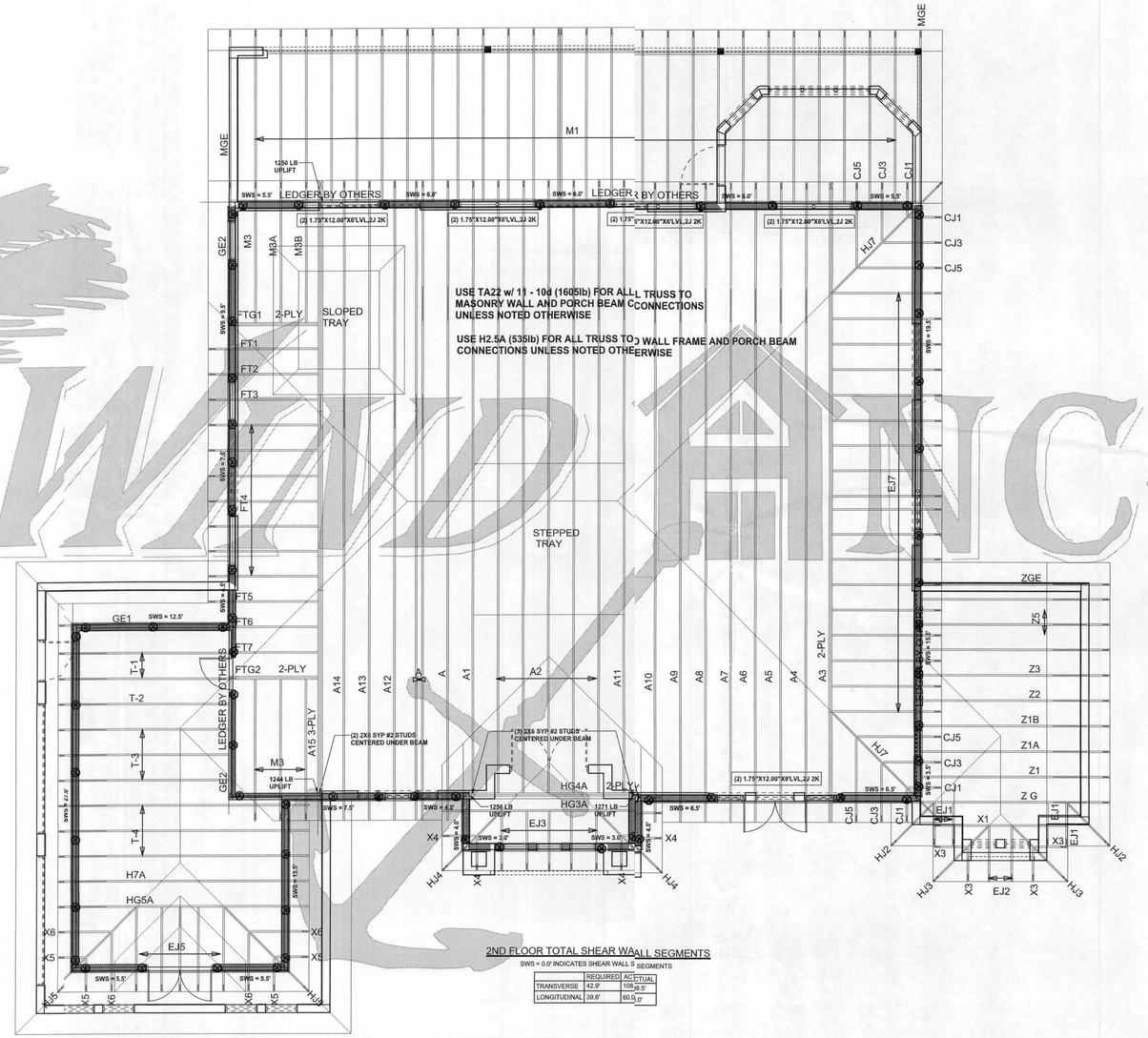
FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

DRAWING NUMBER

S-3

OF 5 SHEETS



2ND FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	42.9'	108.5'
LONGITUDINAL	39.6'	60.0'

STRUCTUAL ROOF PLAN
SCALE: 3/16" = 1"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C.
1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C.
1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'6X20'W

3/16" TAPCONS @ 12" O.C.
1/4" TAPCONS @ 18" O.C.

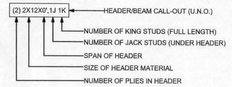
GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C.
(2) 1/4" TAPCONS & 24" O.C.

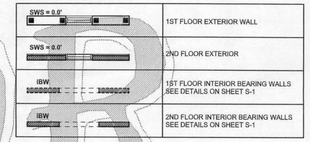
GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND



WALL LEGEND



INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

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REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, IE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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