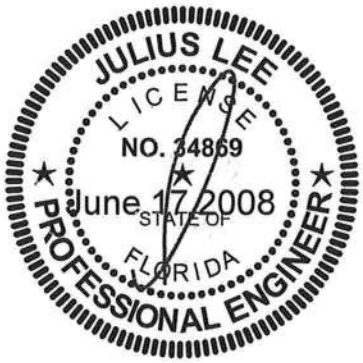
Builders FirstSource



Project Information for: L280800

Builder: Woodman Park Builders, Inc.
Address: 798 Southwest Utah Street
... Ft. White, FL 32038
County: Columbia
Truss Count: 26
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Mark E. Haddox Florida Certified Residental Contractor License No. CRC1329442
Address: Woodman Park Builders, Inc. 4816 W U.S. Highway 90 Suite# 100 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

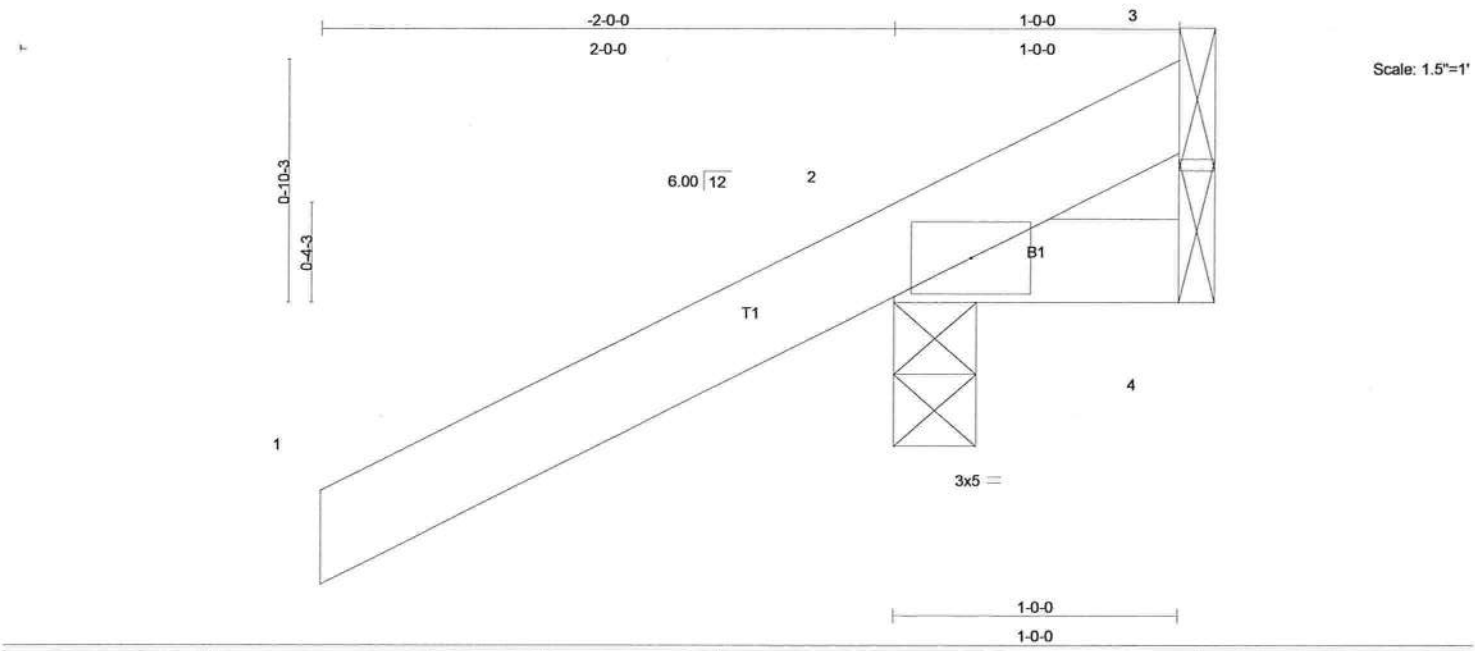
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1973297	CJ1	6/16/08
2	J1973298	CJ3	6/16/08
3	J1973299	CJ5	6/16/08
4	J1973300	EJ7	6/16/08
5	J1973301	HJ9	6/16/08
6	J1973302	T01	6/16/08
7	J1973303	T01G	6/16/08
8	J1973304	T02	6/16/08
9	J1973305	T03	6/16/08
10	J1973306	T03G	6/16/08
11	J1973307	T04	6/16/08
12	J1973308	T05	6/16/08
13	J1973309	T06	6/16/08
14	J1973310	T07	6/16/08
15	J1973311	T08	6/16/08
16	J1973312	T09	6/16/08
17	J1973313	T10	6/16/08
18	J1973314	T11	6/16/08
19	J1973315	T12	6/16/08
20	J1973316	T13	6/16/08
21	J1973317	T14	6/16/08
22	J1973318	T15	6/16/08
23	J1973319	T16	6/16/08
24	J1973320	T17	6/16/08
25	J1973321	T18	6/16/08
26	J1973322	T19	6/16/08

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973297
	CJ1	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-274(load case 6), 3=-90(load case 1)
Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973297
	CJ1	JACK	4	1	Job Reference (optional)

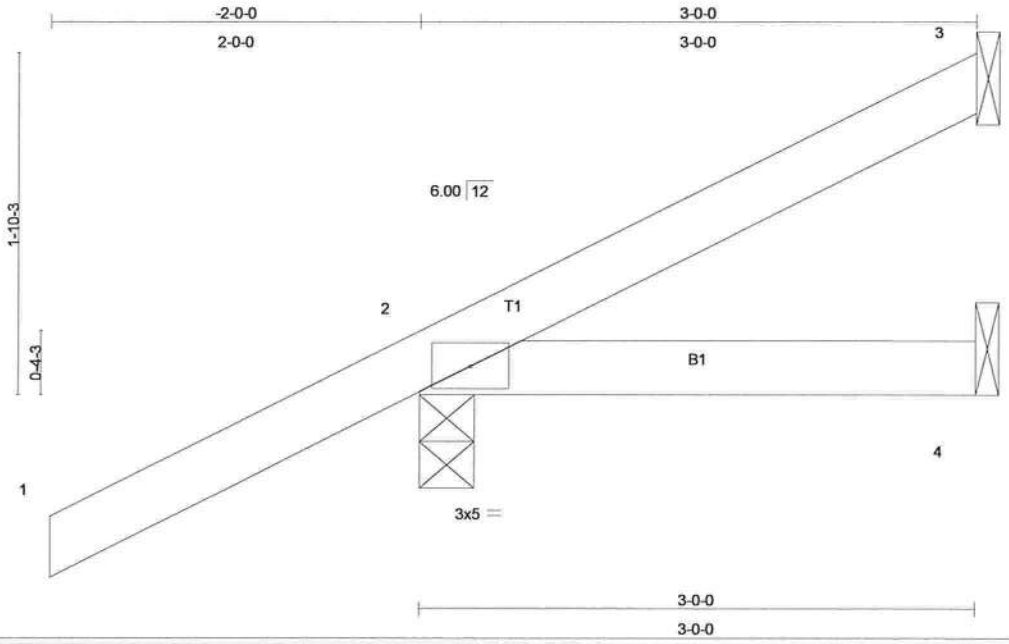
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	CJ3	JACK	4	1	J1973298
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-203(load case 6)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1 1/2 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973298
	CJ3	JACK	4	1	Job Reference (optional)

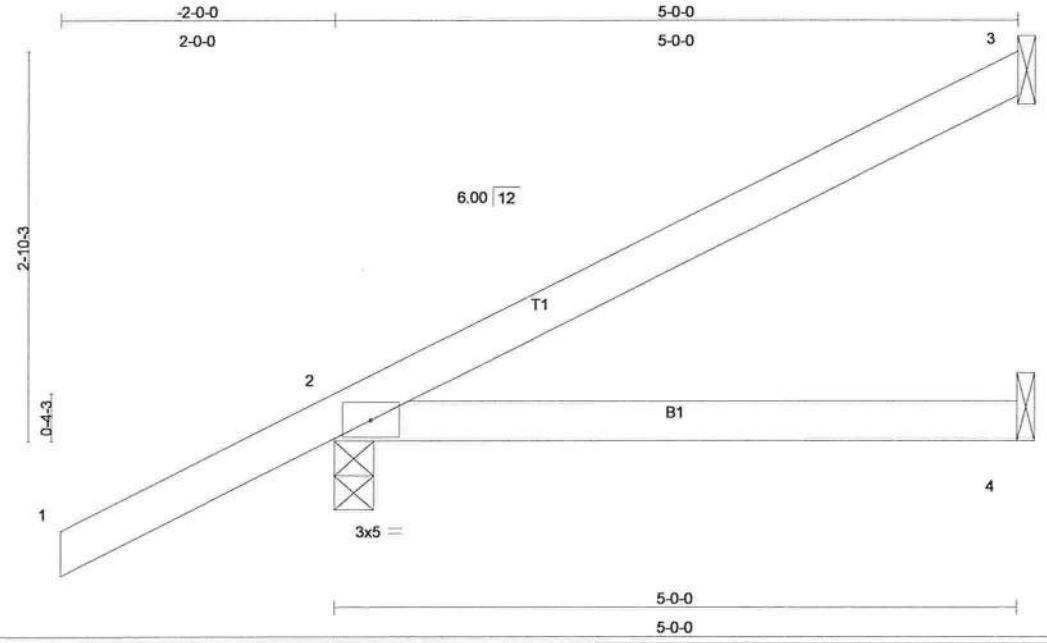
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
	CJ5	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:46 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-87(load case 6), 2=-199(load case 6)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE License No. 34183
1106 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
	CJ5	JACK	4	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34834
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973300
	EJ7	JACK	25	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1

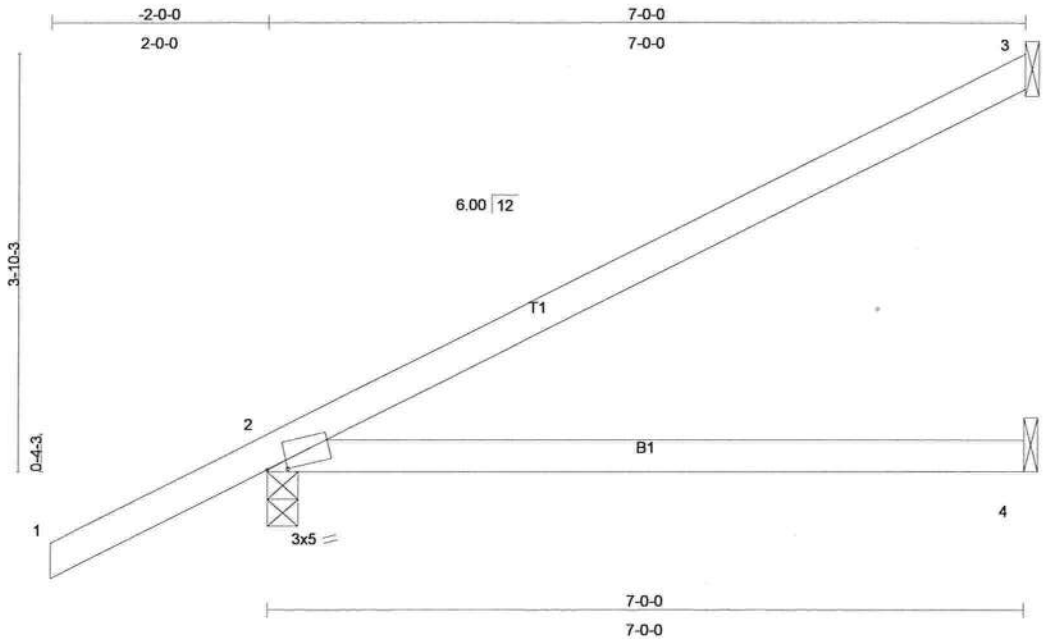


Plate Offsets (X,Y): [2:0-2-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>501	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-84(load case 6), 2=-139(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.81

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 139 lb uplift at joint 2.

Julius Lee
Truss Design Engineer
Florida PE No. 34169
1105 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973300
	EJ7	JACK	25	1	Job Reference (optional)

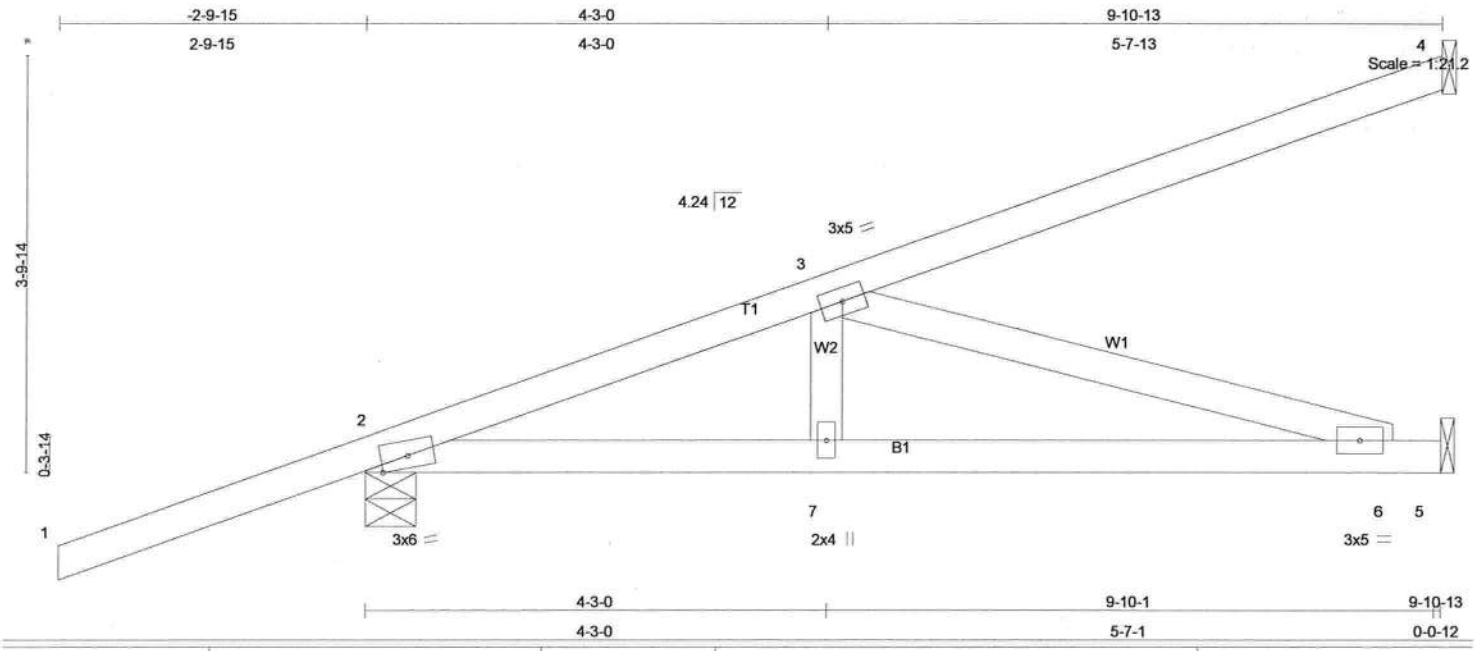
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973301
	HJ9	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>986	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift 4=-232(load case 3), 2=-281(load case 3), 5=-62(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-647/120, 3-4=-105/65
BOT CHORD 2-7=-308/599, 6-7=-308/599, 5-6=0/0
WEBS 3-7=0/190, 3-6=-624/321

JOINT STRESS INDEX
2 = 0.77, 3 = 0.18, 6 = 0.21 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Builders FirstSource

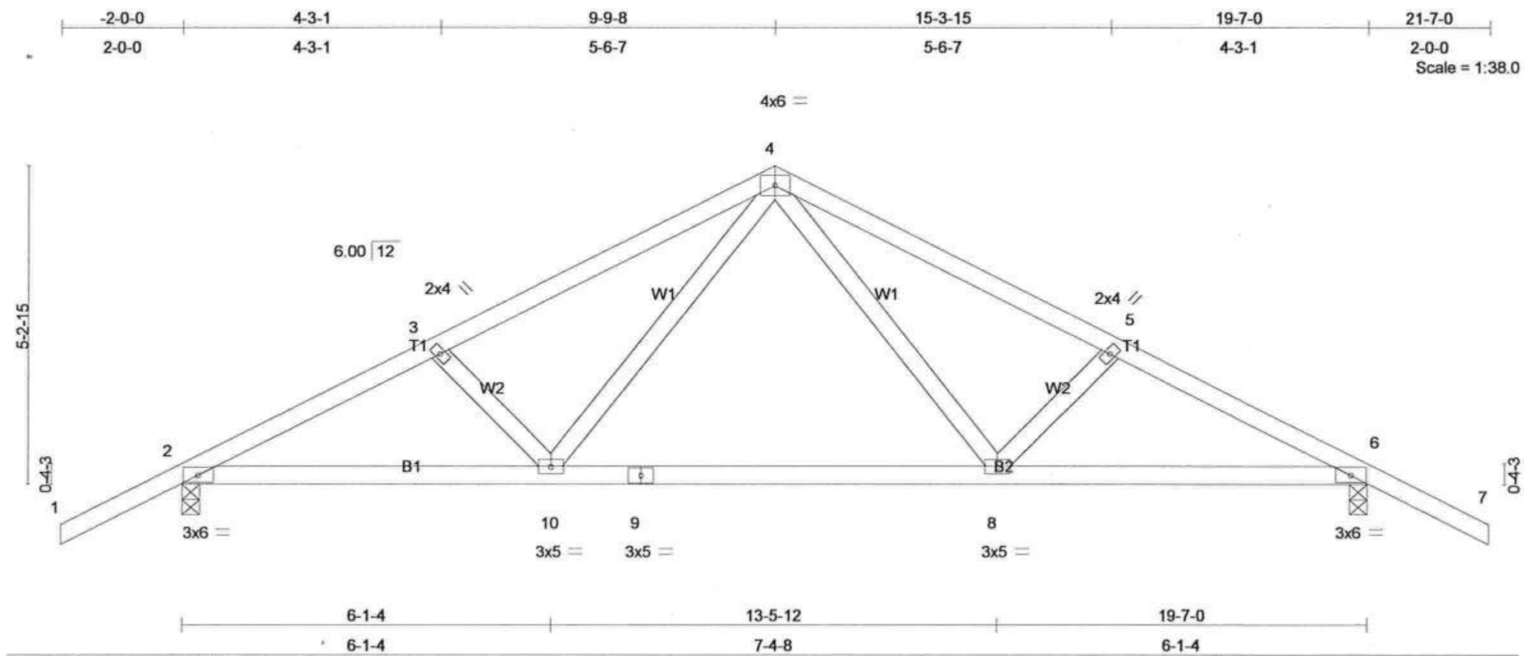
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54
   Trapezoidal Loads (plf)
   Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T01	COMMON	3	1	J1973302 Job Reference (optional)



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.26	8-10	>901	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.40	8-10	>572	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 94 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-9 oc bracing.

REACTIONS (lb/size) 2=955/0-3-8, 6=955/0-3-8
Max Horz 2=-94(load case 7)
Max Uplift 2=-292(load case 6), 6=-292(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1603/857, 3-4=-1456/828, 4-5=-1456/828, 5-6=-1603/857, 6-7=0/47
BOT CHORD 2-10=-608/1365, 9-10=-310/911, 8-9=-310/911, 6-8=-608/1365
WEBS 3-10=-195/185, 4-10=-287/583, 4-8=-287/583, 5-8=-195/185

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.76, 5 = 0.34, 6 = 0.71, 8 = 0.42, 9 = 0.60 and 10 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973302
	T01	COMMON	3	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973303
	T01G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:18:03 2008 Page 1

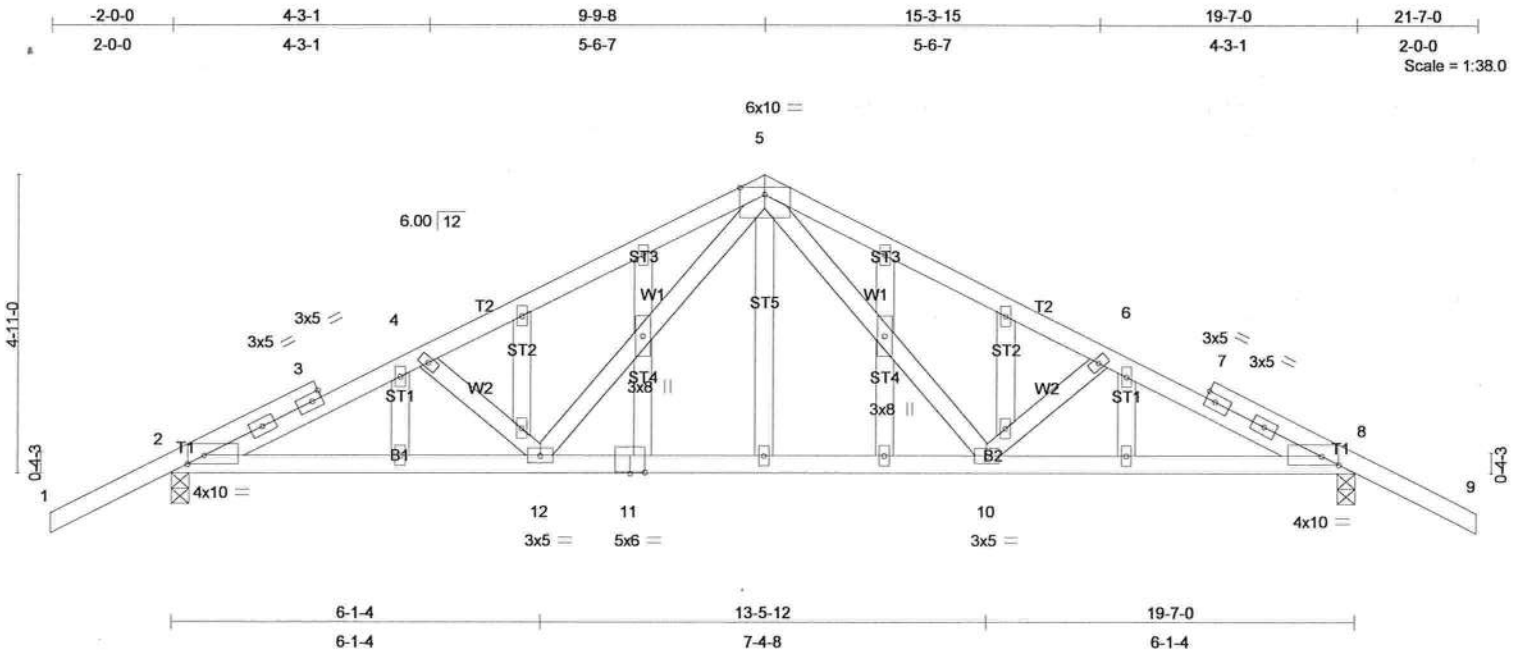


Plate Offsets (X,Y): [2:0-3-4,0-1-12], [8:0-3-4,0-1-12], [11:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.21	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.35	10-12	>656	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-9-15 oc bracing.

REACTIONS (lb/size) 2=1551/0-3-8, 8=1551/0-3-8
Max Horz 2=-102(load case 7)
Max Uplift 2=-796(load case 6), 8=-796(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-23/100, 2-3=-2682/1452, 3-4=-2575/1414, 4-5=-2272/1256, 5-6=-2272/1256, 6-7=-2575/1414, 7-8=-2682/1452, 8-9=-23/100
BOT CHORD	2-12=-1176/2364, 11-12=-612/1424, 10-11=-612/1424, 8-10=-1176/2364
WEBS	4-12=-618/422, 5-12=-401/787, 5-10=-401/787, 6-10=-618/422

JOINT STRESS INDEX

2 = 0.90, 3 = 0.00, 3 = 0.62, 3 = 0.72, 4 = 0.34, 5 = 0.48, 6 = 0.34, 7 = 0.00, 7 = 0.72, 7 = 0.62, 8 = 0.90, 10 = 0.56, 11 = 0.43, 12 = 0.56 , 13 = 0.34, 14 = 0.48, 15 = 0.34, 16 = 0.00, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.48, 24 = 0.34, 25 = 0.34, 26 = 0.34 and 27 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34985
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973303
	T01G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 796 lb uplift at joint 2 and 796 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-12=-10, 10-12=-40(F=-30), 8-10=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973304
	T02	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:07:39 2008 Page 1

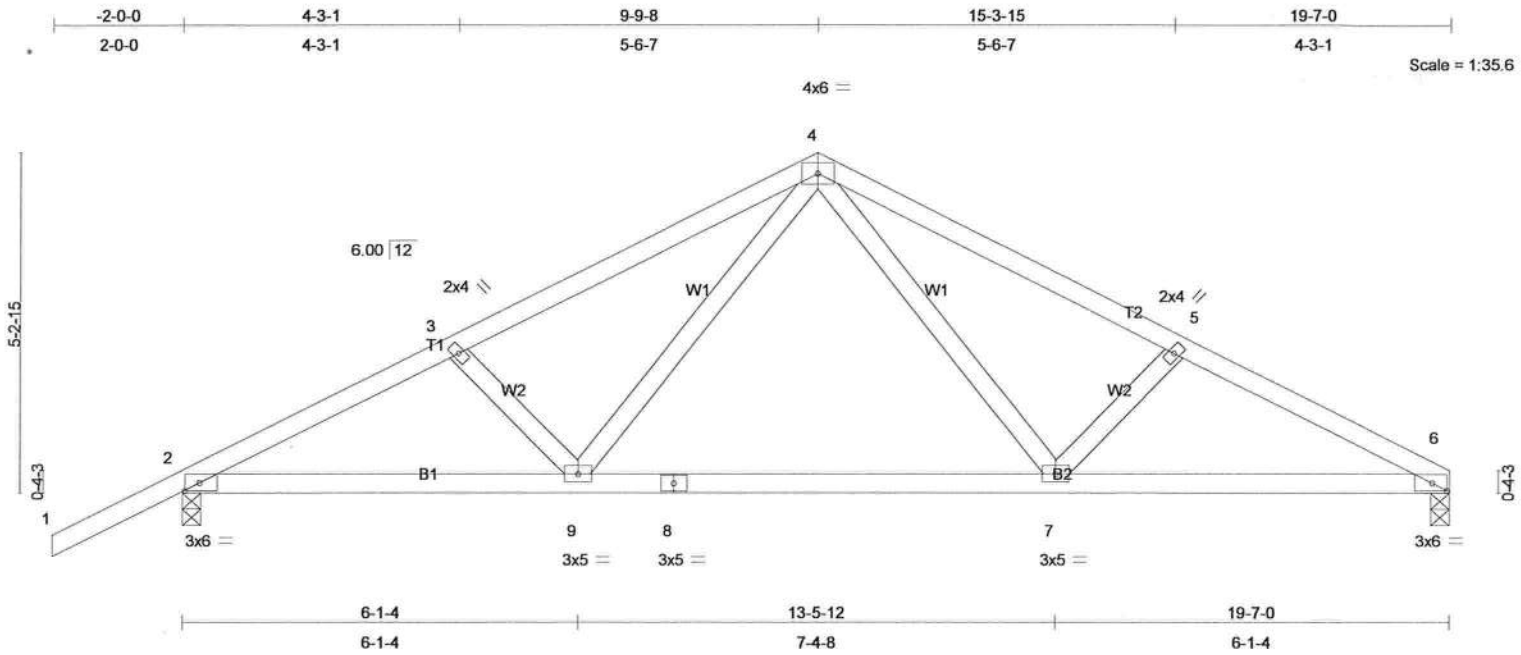


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.25	7-9	>921	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.40	7-9	>580	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-0-2 oc bracing.

REACTIONS (lb/size) 6=832/0-3-8, 2=961/0-3-8
Max Horz 2=107(load case 6)
Max Uplift 6=-195(load case 7), 2=-293(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-1615/878, 3-4=-1468/849, 4-5=-1495/894, 5-6=-1648/932
BOT CHORD	2-9=-704/1376, 8-9=-409/924, 7-8=-409/924, 6-7=-766/1413
WEBS	3-9=-195/187, 4-9=-282/582, 4-7=-347/619, 5-7=-217/221

JOINT STRESS INDEX

2 = 0.76, 3 = 0.34, 4 = 0.73, 5 = 0.34, 6 = 0.76, 7 = 0.45, 8 = 0.60 and 9 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 6 and 293 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

LOAD CASE(S) Standard

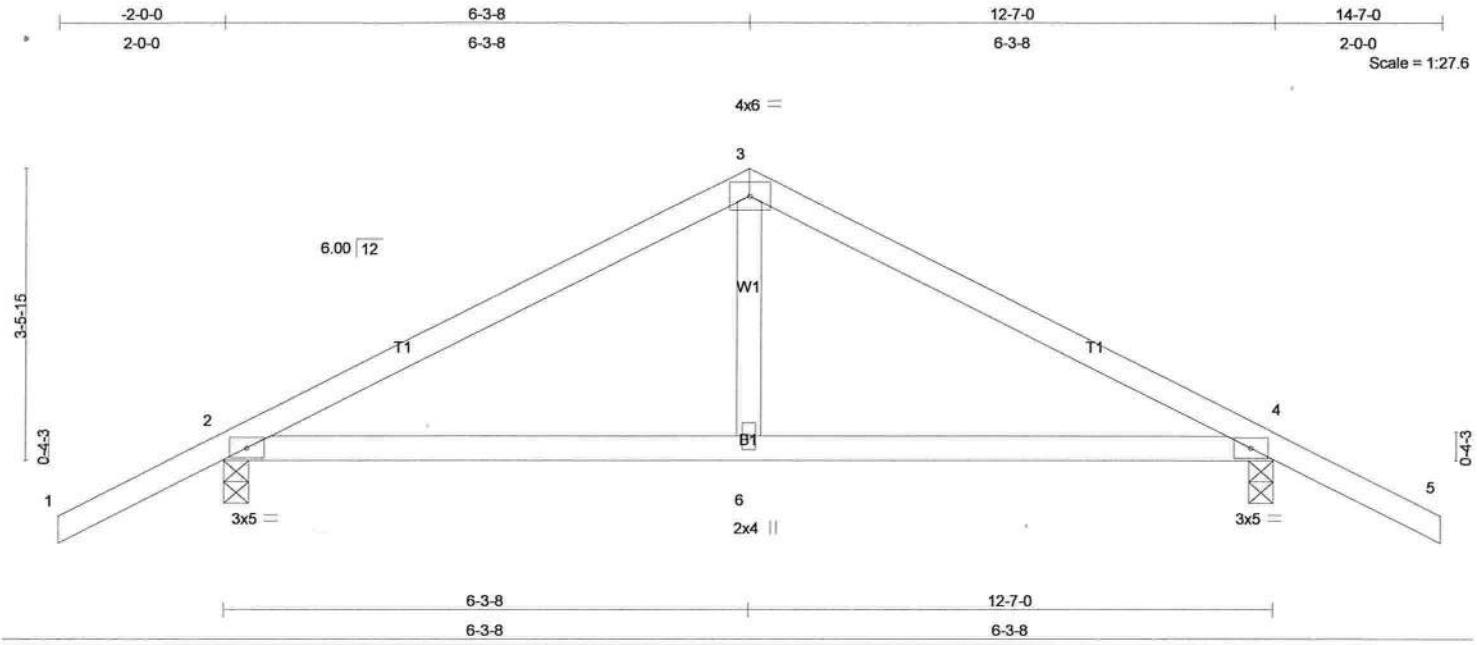
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T03	COMMON	3	1	J1973305
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:51 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=509/0-3-8, 4=509/0-3-8
Max Horz 2=-73(load case 7)
Max Uplift 2=-184(load case 6), 4=-184(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-537/276, 3-4=-537/276, 4-5=0/47
BOT CHORD 2-6=-66/412, 4-6=-66/412
WEBS 3-6=0/210

JOINT STRESS INDEX
2 = 0.40, 3 = 0.73, 4 = 0.40 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 2 and 184 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973305
	T03	COMMON	3	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973306
	T03G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:52 2008 Page 1

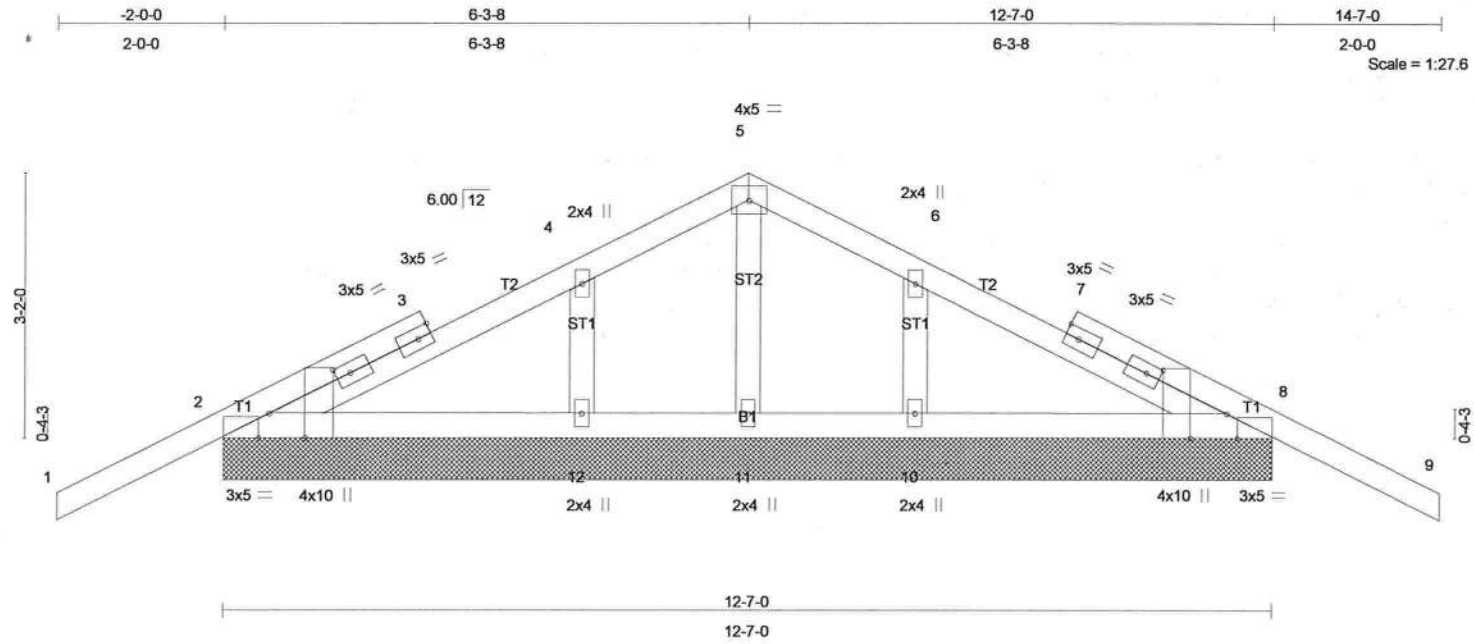


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [3:0-1-15,0-1-8], [7:0-1-15,0-1-8], [8:0-3-8,Edge], [8:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.05	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 61 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=489/12-7-0, 8=489/12-7-0, 11=206/12-7-0, 12=416/12-7-0, 10=416/12-7-0
Max Horz 2=-78(load case 7)
Max Uplift 2=-319(load case 6), 8=-332(load case 7), 11=-52(load case 6), 12=-205(load case 6), 10=-208(load case 7)
Max Grav 2=494(load case 10), 8=494(load case 11), 11=206(load case 1), 12=417(load case 10), 10=417(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-27/99, 2-3=-34/48, 3-4=-64/174, 4-5=-6/109, 5-6=-6/109, 6-7=-41/174, 7-8=-34/48, 8-9=-27/99
BOT CHORD 2-12=-71/144, 11-12=-71/144, 10-11=-71/144, 8-10=-71/144
WEBS 5-11=-206/56, 4-12=-374/295, 6-10=-374/295

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Crandal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.78, 2 = 0.00, 3 = 0.00, 3 = 0.49, 3 = 0.49, 4 = 0.15, 5 = 0.10, 6 = 0.15, 7 = 0.00, 7 = 0.49, 7 = 0.49, 8 = 0.78, 8 = 0.00, 10 = 0.16, 11 = 0.07 and 12 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973306
	T03G	GABLE	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2, 332 lb uplift at joint 8, 52 lb uplift at joint 11, 205 lb uplift at joint 12 and 208 lb uplift at joint 10.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973307
	T04	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:53 2008 Page 1

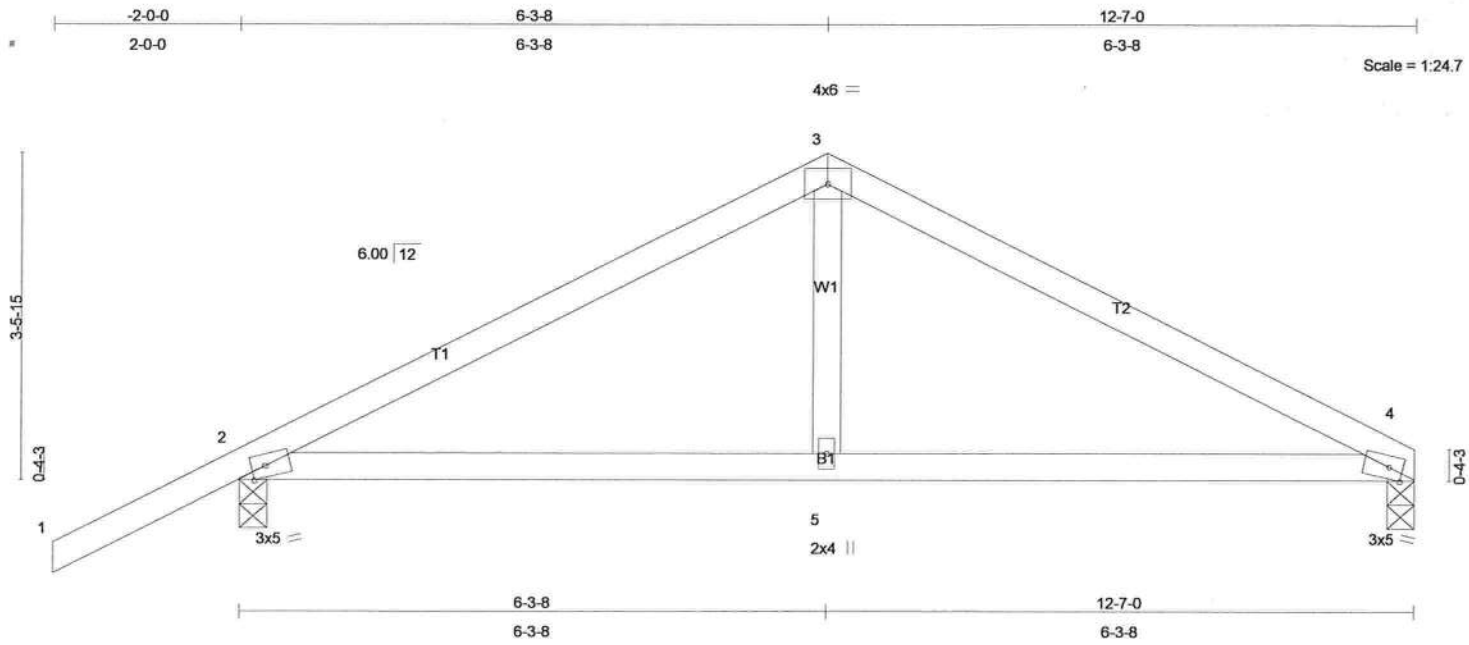


Plate Offsets (X,Y): [2:0-1-12,Edge], [4:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.05	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.07	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=383/0-3-8, 2=519/0-3-8
Max Horz 2=86(load case 6)
Max Uplift 4=-84(load case 7), 2=-186(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-568/328, 3-4=-564/320
BOT CHORD 2-5=-191/441, 4-5=-191/441
WEBS 3-5=0/214

JOINT STRESS INDEX
2 = 0.73, 3 = 0.81, 4 = 0.73 and 5 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1105 Coastal Key Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973307
	T04	COMMON	3	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 4 and 186 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973308
	T05	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

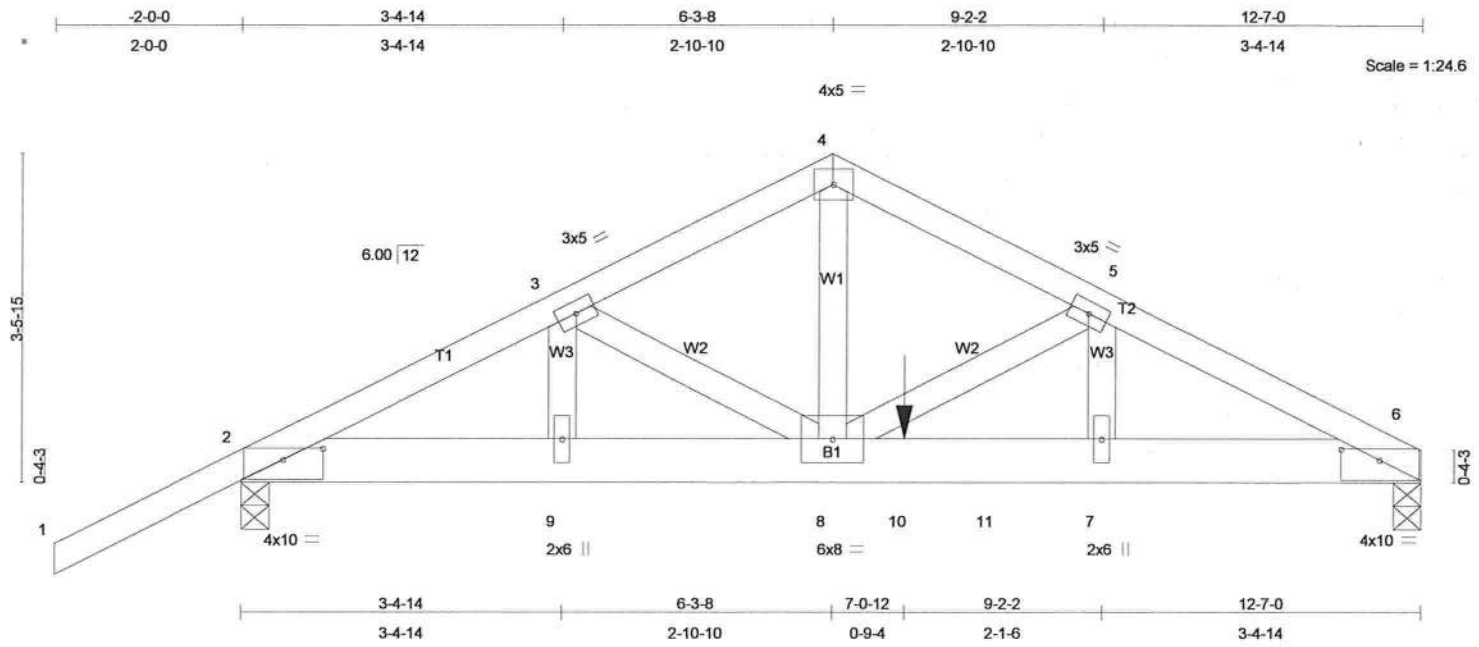


Plate Offsets (X,Y): [2:0-5-0,0-1-7], [6:0-5-0,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.11	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=3563/0-3-8, 2=1960/0-3-8
Max Horz 2=89(load case 5)
Max Uplift 6=-964(load case 6), 2=-585(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-3554/919, 3-4=-3847/1051, 4-5=-3843/1043, 5-6=-5951/1603
BOT CHORD 2-9=-785/3123, 8-9=-785/3123, 8-10=-1397/5275, 10-11=-1397/5275, 7-11=-1397/5275, 6-7=-1397/5275
WEBS 3-9=-410/169, 3-8=-115/445, 4-8=-864/3193, 5-8=-2145/625, 5-7=-507/1917

JOINT STRESS INDEX
2 = 0.65, 3 = 0.81, 4 = 0.75, 5 = 0.81, 6 = 0.65, 7 = 0.45, 8 = 0.39 and 9 = 0.45

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
3109 Coastal Ray Blvd
Boynton Beach, FL 33435

June 16,2008

NOTES

3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 6 and 585 lb uplift at joint 2.
8) Girder carries tie-in span(s): 33-9-8 from 8-0-0 to 12-7-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-519(F=-509)
   Concentrated Loads (lb)
   Vert: 10=-2363(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
	T06	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

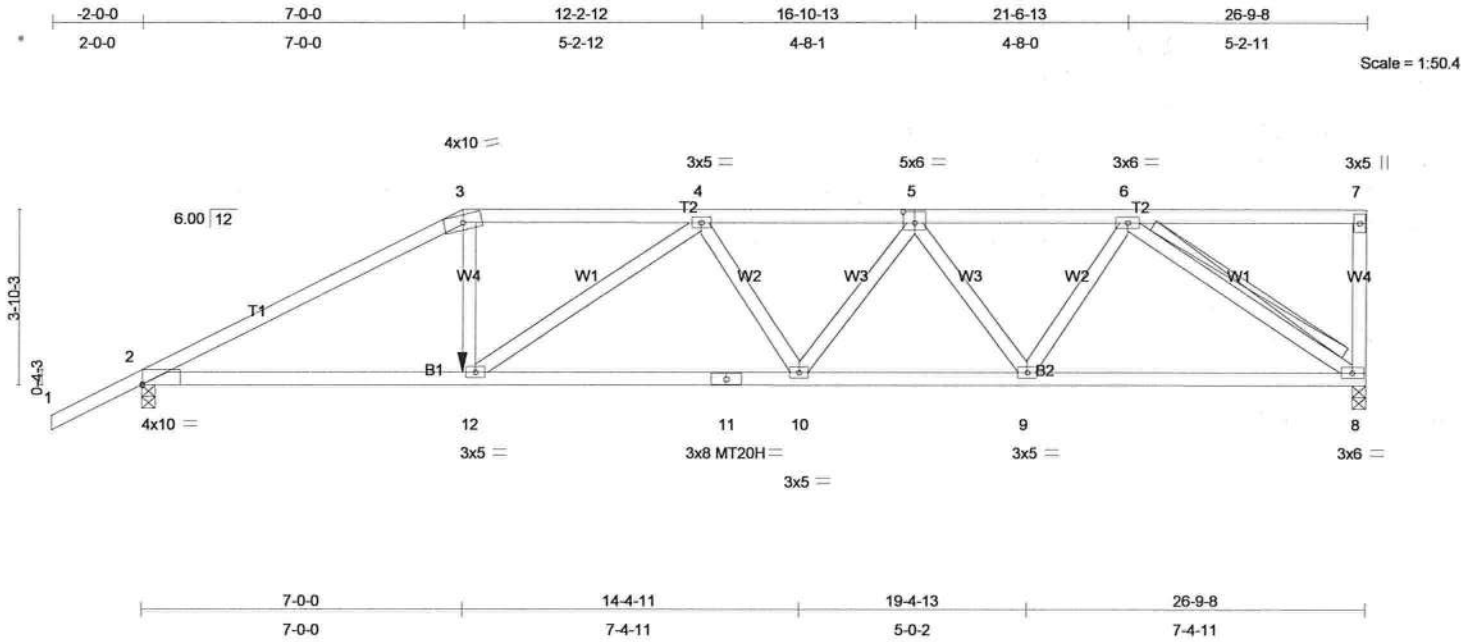


Plate Offsets (X,Y): [2:Edge,0-0-2], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.19	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.44	10-12	>728	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=1883/0-3-8, 2=1818/0-3-8
Max Horz 2=163(load case 5)
Max Uplift 8=-649(load case 4), 2=-578(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3360/1085, 3-4=-2956/1008, 4-5=-3532/1196, 5-6=-2790/938, 6-7=-85/15, 7-8=-288/143
BOT CHORD 2-12=-997/2914, 11-12=-1273/3610, 10-11=-1273/3610, 9-10=-1162/3346, 8-9=-798/2236
WEBS 3-12=-296/996, 4-12=-799/378, 4-10=-150/148, 5-10=-60/323, 5-9=-966/389, 6-9=-270/1065, 6-8=-2625/956

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.76, 3 = 0.77, 4 = 0.42, 5 = 0.55, 6 = 0.79, 7 = 0.64, 8 = 0.79, 9 = 0.79, 10 = 0.42, 11 = 0.87 and 12 = 0.73

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
	T06	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 649 lb uplift at joint 8 and 578 lb uplift at joint 2.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 12=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
	T07	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:55 2008 Page 1

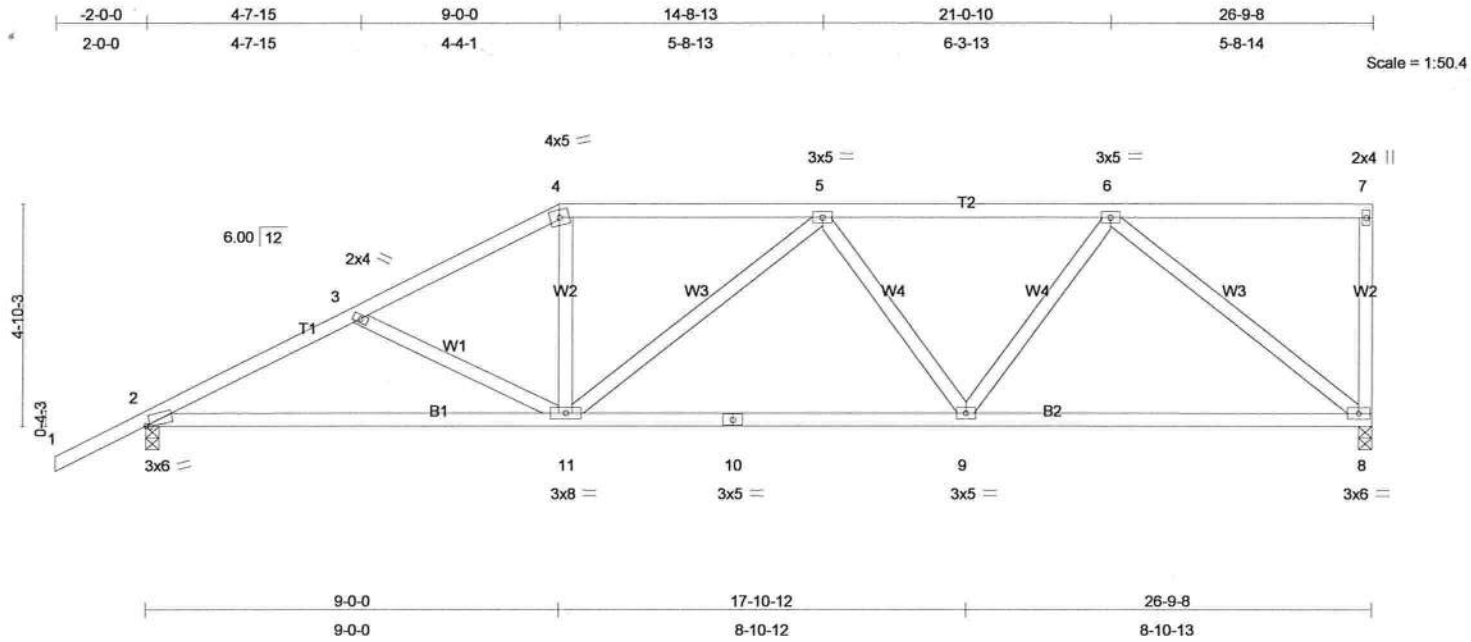


Plate Offsets (X,Y): [2:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.13	2-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.24	2-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.

REACTIONS (lb/size) 8=843/0-3-8, 2=969/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 8=-229(load case 5), 2=-248(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1537/763, 3-4=-1299/656, 4-5=-1126/644, 5-6=-1073/568, 6-7=-40/7, 7-8=-135/93
BOT CHORD 2-11=-827/1311, 10-11=-698/1238, 9-10=-698/1238, 8-9=-476/847
WEBS 3-11=-216/207, 4-11=-73/334, 5-11=-145/117, 5-9=-290/228, 6-9=-162/410, 6-8=-1052/605

JOINT STRESS INDEX
2 = 0.83, 3 = 0.33, 4 = 0.74, 5 = 0.41, 6 = 0.41, 7 = 0.82, 8 = 0.60, 9 = 0.41, 10 = 0.48 and 11 = 0.56

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
	T07	MONO HIP	1	1	Job Reference (optional)

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 8 and 248 lb uplift at joint 2.

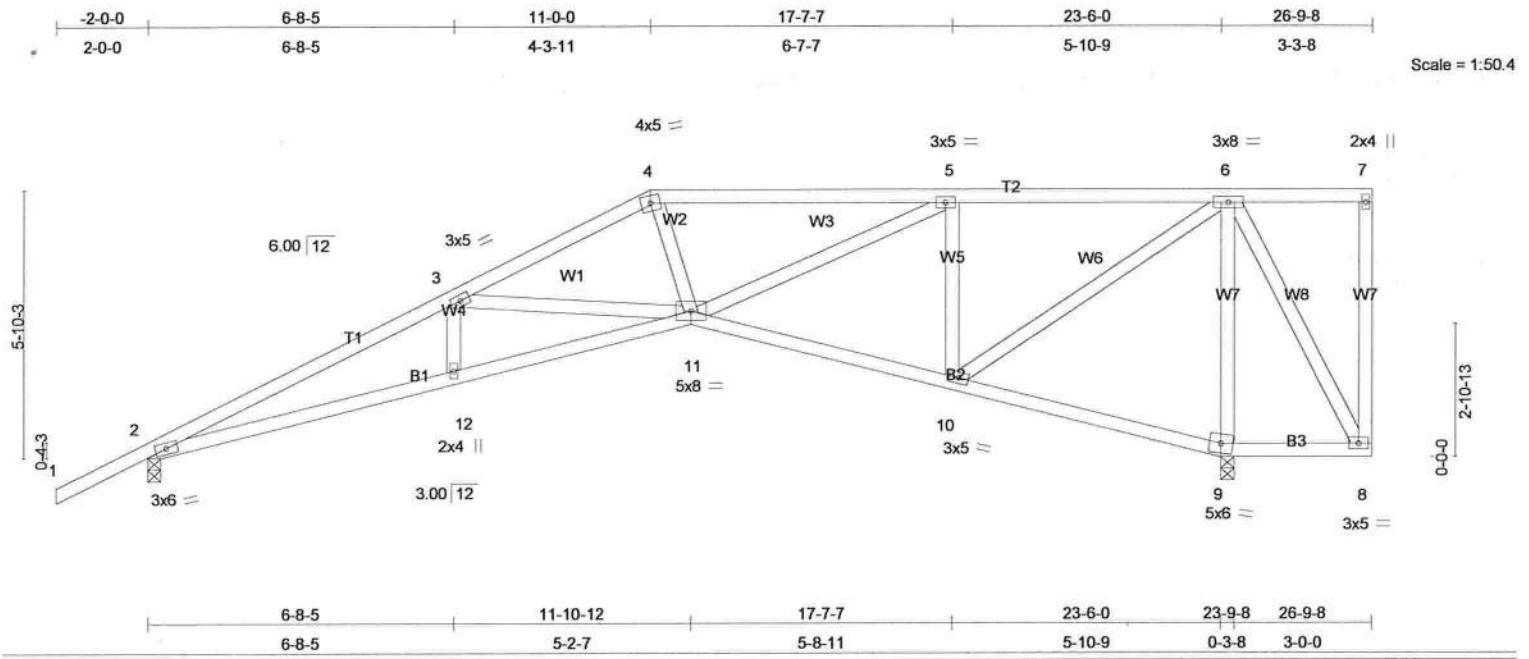
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:09:15 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-9 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=226(load case 6)
Max Uplift 2=-237(load case 6), 9=-284(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2229/1204, 3-4=-1600/921, 4-5=-1523/937, 5-6=-746/435, 6-7=-2/3, 7-8=-92/58
BOT CHORD 2-12=-1289/1967, 11-12=-1290/1966, 10-11=-453/776, 9-10=-105/59, 8-9=-69/37
WEBS 3-12=0/185, 3-11=-517/378, 4-11=-207/426, 5-11=-553/856, 5-10=-722/491, 6-10=-588/1015, 6-9=-966/593, 6-8=-74/139

JOINT STRESS INDEX
2 = 0.72, 3 = 0.48, 4 = 0.79, 5 = 0.56, 6 = 0.97, 7 = 0.34, 8 = 0.46, 9 = 0.45, 10 = 0.66, 11 = 0.61 and 12 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
	T08	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2 and 284 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

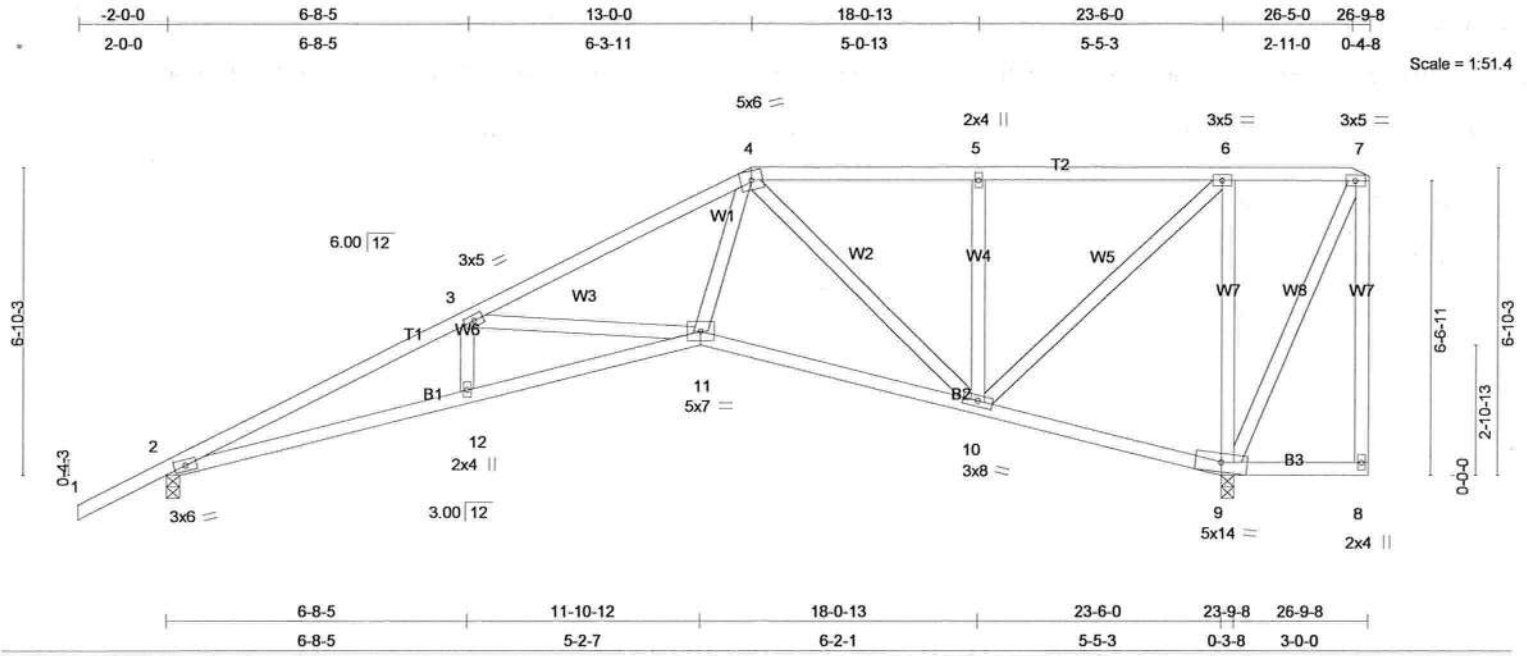
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 7=-54(F) 8=-30(F)

Julius Lee
Truss Design Engineer
Florida FIE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973312
	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:11:11 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-11-2 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-4-1 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=258(load case 6)
Max Uplift 2=-240(load case 6), 9=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2257/1244, 3-4=-1533/879, 4-5=-537/327, 5-6=-537/327, 6-7=-33/65, 7-8=-9/50
BOT CHORD	2-12=-1383/1997, 11-12=-1385/1997, 10-11=-720/1079, 9-10=-92/52, 8-9=-0/1
WEBS	3-12=0/180, 3-11=-633/487, 4-11=-550/899, 4-10=-719/526, 5-10=-308/194, 6-10=-508/847, 6-9=-846/542, 7-9=-152/77

JOINT STRESS INDEX
2 = 0.73, 3 = 0.48, 4 = 0.57, 5 = 0.34, 6 = 0.59, 7 = 0.48, 8 = 0.34, 9 = 0.42, 10 = 0.84, 11 = 0.66 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

NOTES

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2 and 281 lb uplift at joint 9.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 8=-30(F) 7=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973313
	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:02 2008 Page 1

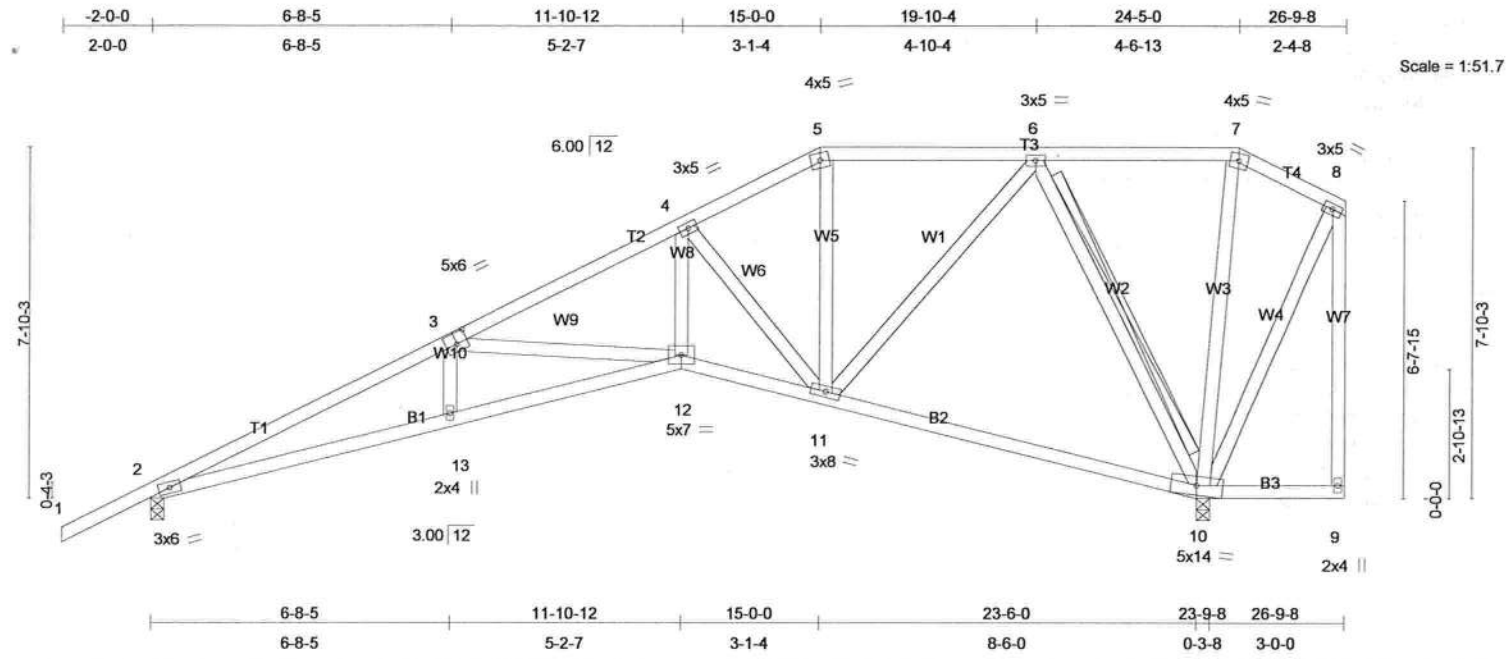


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.17	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-10-10 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-10 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=267(load case 6)
Max Uplift 2=-247(load case 6), 10=-221(load case 5)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2281/1240, 3-4=-1579/913, 4-5=-814/494, 5-6=-699/475, 6-7=-2/72, 7-8=-19/90, 8-9=-17/27
BOT CHORD	2-13=-1369/2017, 12-13=-1371/2014, 11-12=-922/1391, 10-11=-187/324, 9-10=-3/0
WEBS	3-13=0/198, 3-12=-604/436, 4-12=-582/903, 4-11=-1021/741, 5-11=-68/183, 6-11=-371/607, 6-10=-811/525, 7-10=-230/157, 8-10=-98/79

Julius Lee
Senior Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.55, 4 = 0.79, 5 = 0.42, 6 = 0.46, 7 = 0.60, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.64, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973313
	T10	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 221 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-7=-54, 7-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973314
	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:49 2008 Page 1

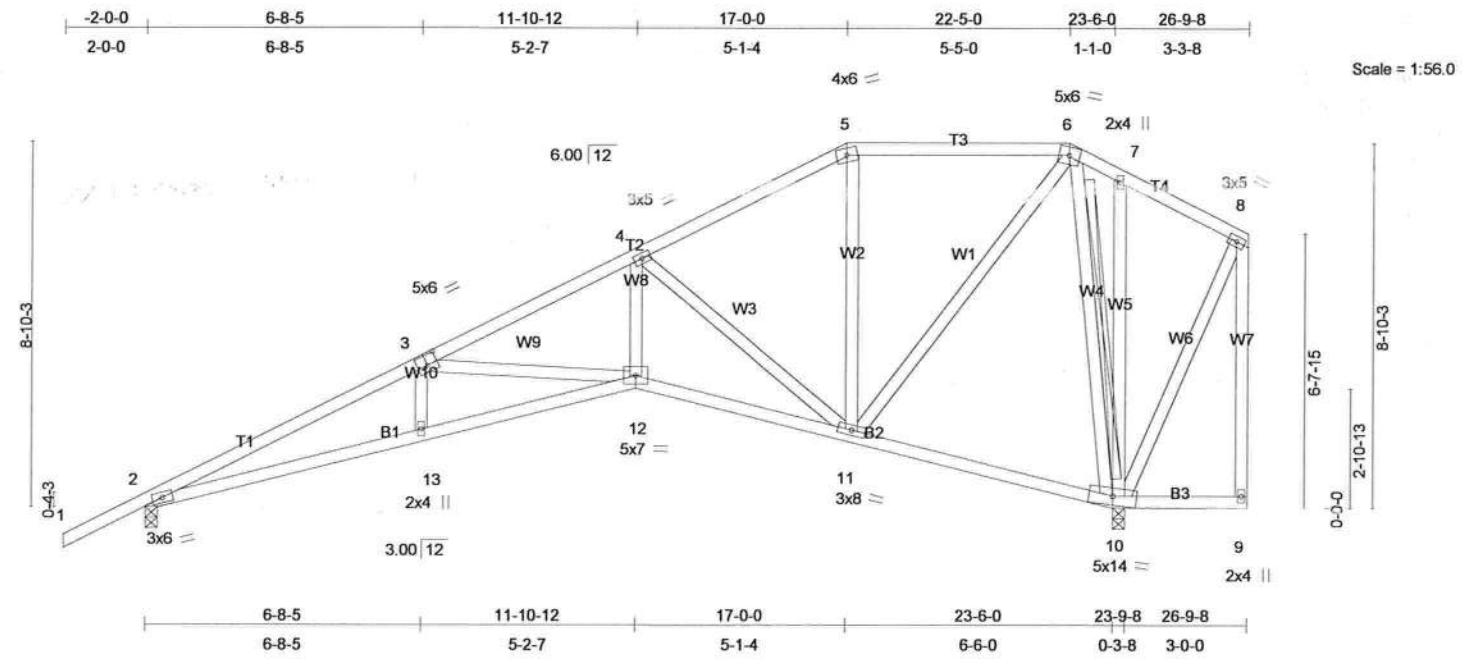


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.21	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.28	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=279(load case 6)
Max Uplift 2=-261(load case 6), 10=-272(load case 6)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2275/1247, 3-4=-1589/929, 4-5=-587/361, 5-6=-472/378, 6-7=0/94, 7-8=-16/110, 8-9=-13/39
BOT CHORD 2-13=-1375/2011, 12-13=-1377/2009, 11-12=-940/1408, 10-11=-35/66, 9-10=-3/2
WEBS 3-13=0/188, 3-12=-586/424, 4-12=-573/927, 4-11=-1160/830, 5-11=-76/75, 6-11=-429/690, 6-10=-779/449, 7-10=-143/119, 8-10=-145/105

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.52, 4 = 0.81, 5 = 0.60, 6 = 0.32, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.75, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973314
	T11	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 272 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:13:34 2008 Page 1

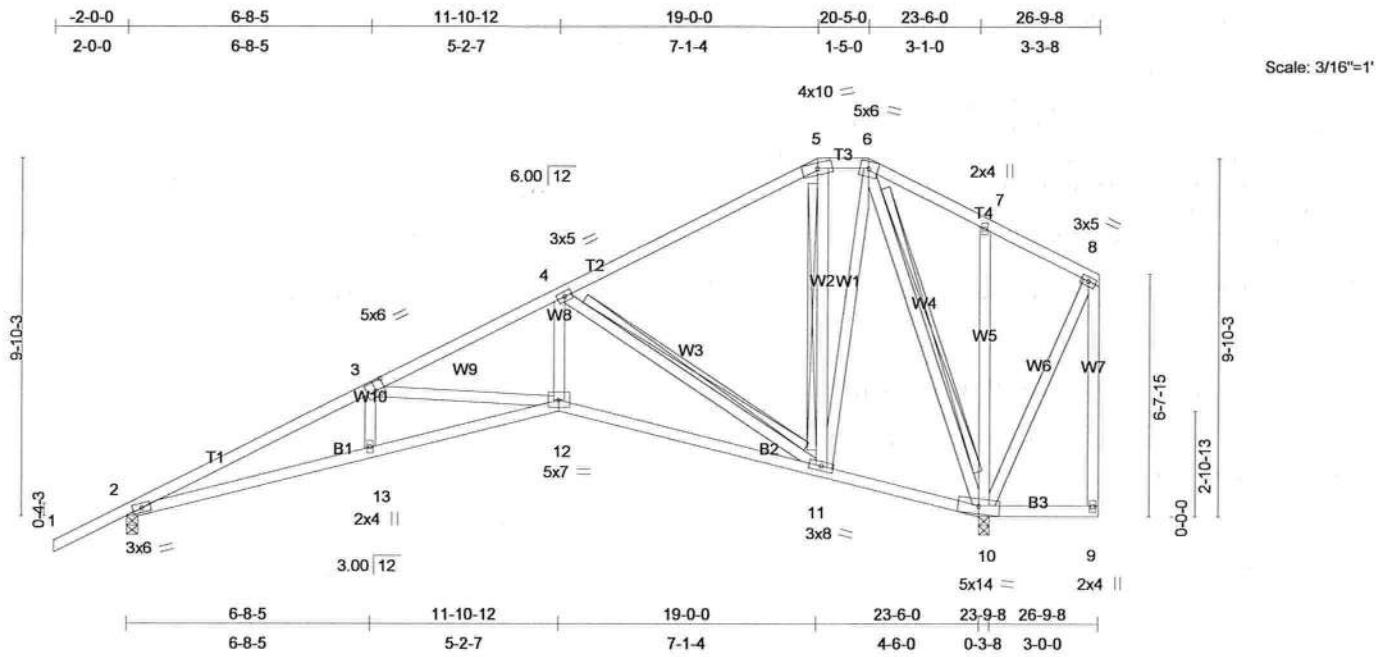


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.22	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.29	12-13	>967	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-11, 5-11, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=850/0-3-8, 10=1046/0-3-8
Max Horz 2=291(load case 6)
Max Uplift 2=-253(load case 6), 10=-239(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2260/1266, 3-4=-1611/970, 4-5=-418/274, 5-6=-295/322, 6-7=0/102, 7-8=-13/109, 8-9=-8/56
BOT CHORD 2-13=-1391/1996, 12-13=-1392/1996, 11-12=-987/1440, 10-11=-95/199, 9-10=-2/2
WEBS 3-13=0/174, 3-12=-543/395, 4-12=-575/942, 4-11=-1315/949, 5-11=-236/201, 6-11=-481/742, 6-10=-762/403, 7-10=-203/204, 8-10=-145/94

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.47, 4 = 0.82, 5 = 0.67, 6 = 0.43, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.93, 12 = 0.70 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
	T12	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2 and 239 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T13	HIP	1	1	J1973316
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:01 2008 Page 1

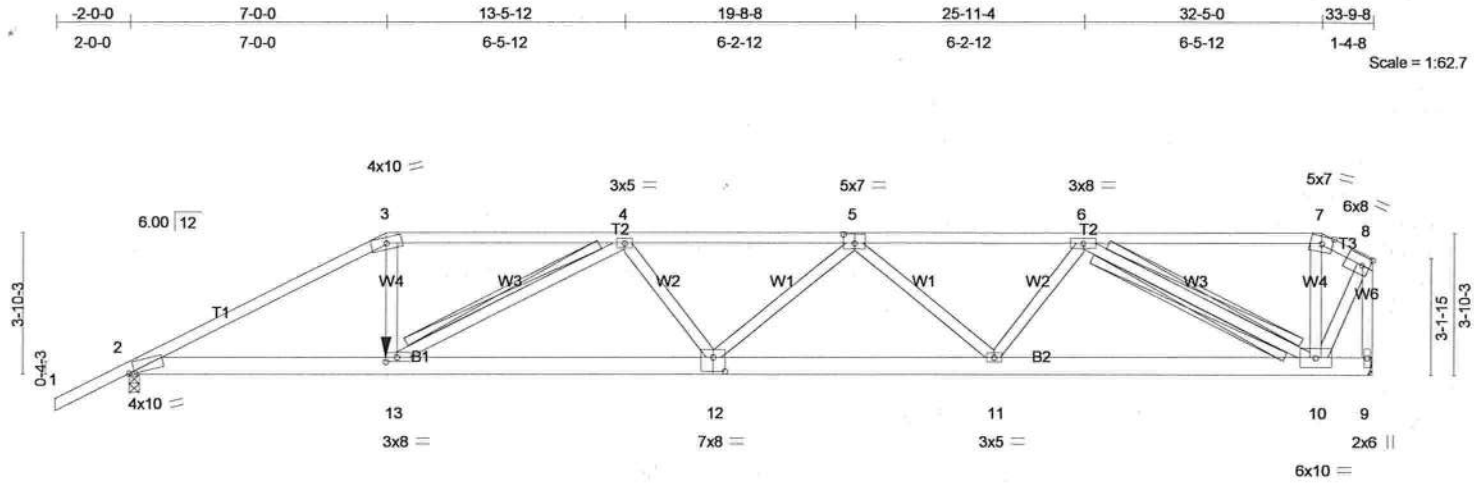


Plate Offsets (X,Y): [2:0-1-11,0-0-6], [5:0-3-8,0-3-0], [12:0-4-0,0-4-8], [13:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.36	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.68	12-13	>589	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS I-Brace: 2 X 4 SYP No.3 - 6-10
T-Brace: 2 X 4 SYP No.3 - 4-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2313/0-3-8, 9=2363/Mechanical
Max Horz 2=122(load case 5)
Max Uplift 2=-716(load case 5), 9=-787(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4517/1485, 3-4=-4027/1378, 4-5=-5721/1945, 5-6=-4785/1629, 6-7=-1016/363, 7-8=-1050/329, 8-9=-2367/718
BOT CHORD 2-13=-1324/3966, 12-13=-1954/5639, 11-12=-1955/5649, 10-11=-1442/4130, 9-10=-9/32
WEBS 3-13=-418/1454, 4-13=-1930/748, 4-12=0/271, 5-12=0/182, 5-11=-1166/489, 6-11=-265/1154, 6-10=-3553/1277, 7-10=-209/197, 8-10=-738/2278

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973316
	T13	HIP	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.81, 3 = 0.98, 4 = 0.62, 5 = 0.81, 6 = 0.84, 7 = 0.85, 8 = 0.86, 9 = 0.61, 10 = 0.92, 11 = 0.82, 12 = 0.90 and 13 = 0.91

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 716 lb uplift at joint 2 and 787 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

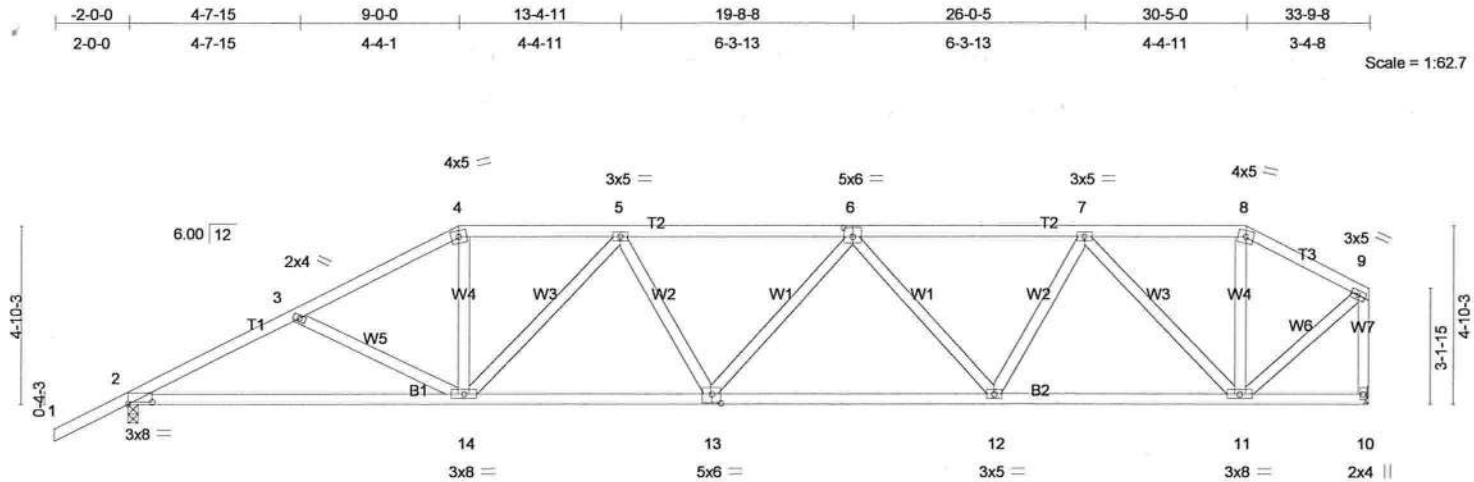
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 7-8=-118(F=-64), 2-13=-10, 9-13=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 13=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:02 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [6:0-3-0,0-3-0], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.31	2-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.09	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 2=1192/0-3-8, 10=1068/Mechanical
Max Horz 2=162(load case 6)
Max Uplift 2=-277(load case 6), 10=-241(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2014/1040, 3-4=-1776/933, 4-5=-1558/894, 5-6=-1933/1073, 6-7=-1637/909, 7-8=-727/447, 8-9=-843/455, 9-10=-1051/566
BOT CHORD 2-14=-990/1734, 13-14=-992/1893, 12-13=-1019/1938, 11-12=-728/1405, 10-11=-15/17
WEBS 3-14=-216/207, 4-14=-227/535, 5-14=-566/265, 5-13=-17/150, 6-13=-64/94, 6-12=-473/299, 7-12=-205/497, 7-11=-1009/534, 8-11=-52/208, 9-11=-460/932

JOINT STRESS INDEX
2 = 0.70, 3 = 0.33, 4 = 0.63, 5 = 0.43, 6 = 0.50, 7 = 0.43, 8 = 0.38, 9 = 0.66, 10 = 0.37, 11 = 0.89, 12 = 0.43, 13 = 0.56 and 14 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
	T14	HIP	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2 and 241 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1180 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T15	HIP	1	1	J1973318
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:03 2008 Page 1

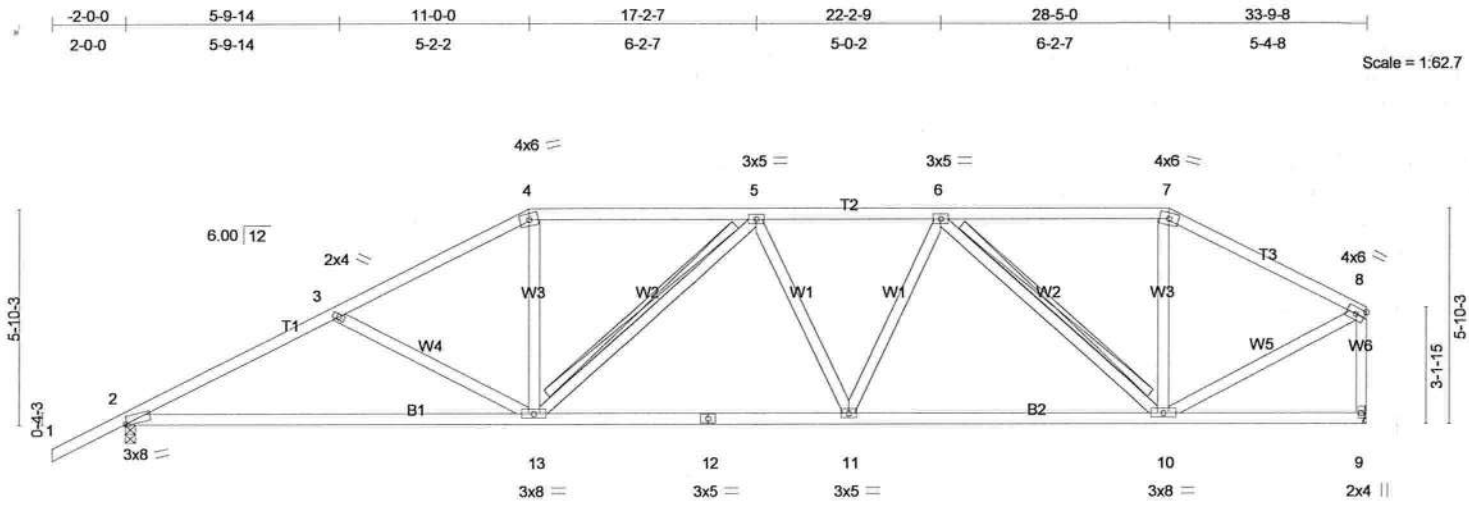


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.56	2-13	>713	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-14 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1192/0-3-8, 9=1068/Mechanical
Max Horz 2=173(load case 6)
Max Uplift 2=-292(load case 6), 9=-210(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1980/1046, 3-4=-1675/905, 4-5=-1453/876, 5-6=-1564/919, 6-7=-903/579, 7-8=-1061/577, 8-9=-1044/580
BOT CHORD 2-13=-985/1700, 12-13=-839/1627, 11-12=-839/1627, 10-11=-752/1465, 9-10=-34/36
WEBS 3-13=-291/278, 4-13=-152/439, 5-13=-350/150, 5-11=-166/131, 6-11=-80/278, 6-10=-803/416, 7-10=-11/229, 8-10=-469/983

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.88, 3 = 0.33, 4 = 0.74, 5 = 0.45, 6 = 0.45, 7 = 0.71, 8 = 0.68, 9 = 0.43, 10 = 0.90, 11 = 0.45, 12 = 0.57 and 13 = 0.56

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973318
	T15	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 210 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:04 2008 Page 1

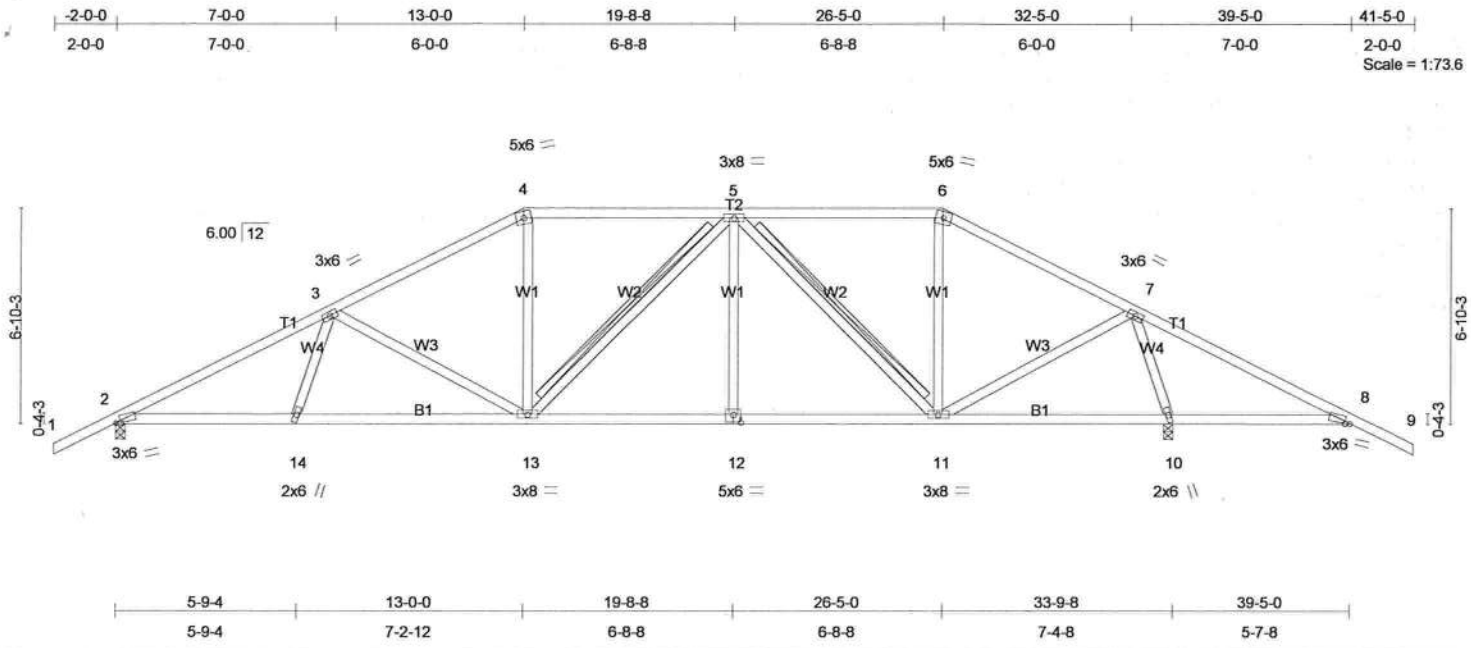


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [8:0-1-13,0-0-7], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1140/0-3-8, 10=1597/0-3-8
Max Horz 2=-113(load case 7)
Max Uplift 2=-303(load case 6), 10=-535(load case 7)
Max Grav 2=1144(load case 10), 10=1597(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1900/878, 3-4=-1440/745, 4-5=-1227/734, 5-6=-769/409, 6-7=-927/389, 7-8=-846/733, 8-9=0/47
BOT CHORD 2-14=-601/1616, 13-14=-615/1591, 12-13=-272/1220, 11-12=-272/1220, 10-11=-122/538, 8-10=-578/905
WEBS 3-14=0/221, 3-13=-422/336, 4-13=-68/340, 5-13=-134/95, 5-12=0/186, 5-11=-638/388, 6-11=-6/196, 7-11=-612/970, 7-10=-1579/1193

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.79, 3 = 0.72, 4 = 0.54, 5 = 0.56, 6 = 0.54, 7 = 0.72, 8 = 0.79, 10 = 0.54, 11 = 0.89, 12 = 0.44, 13 = 0.89 and 14 = 0.54

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
	T16	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 535 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:06 2008 Page 1

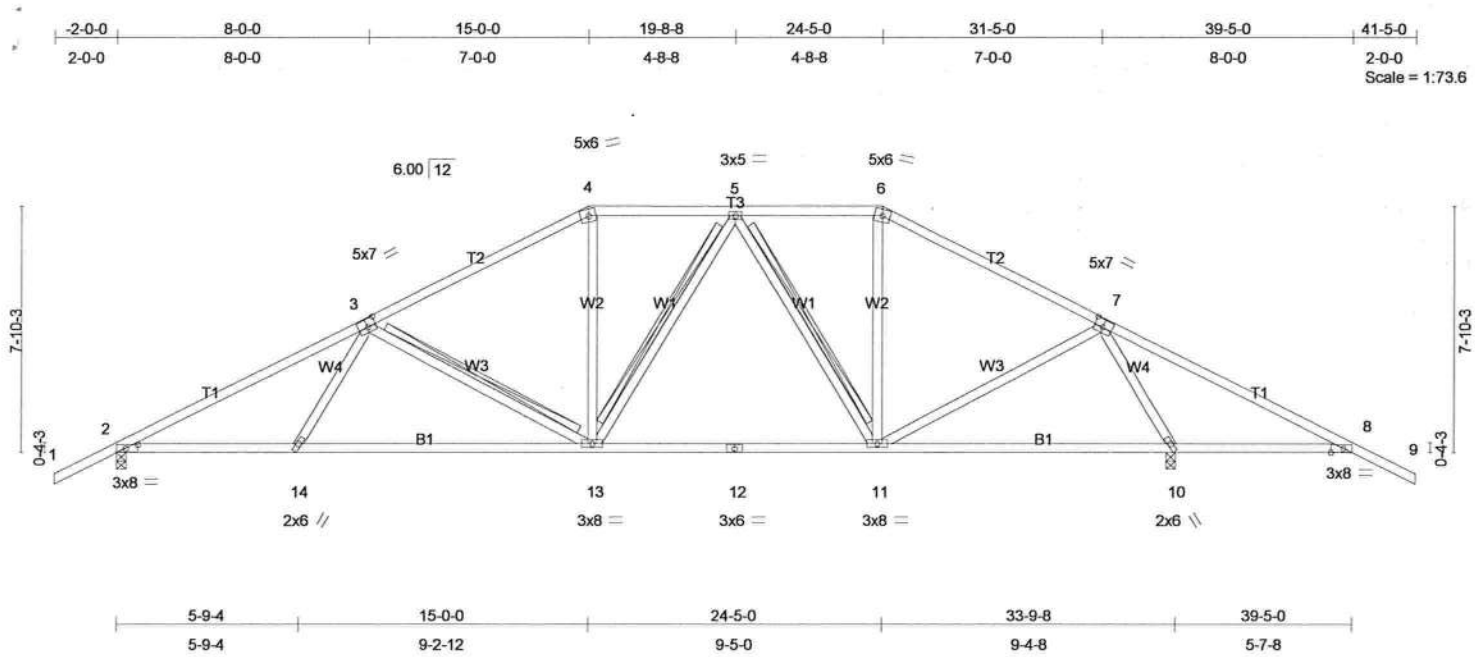


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-3-4,0-3-0], [7:0-3-4,0-3-0], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.16	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.33	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13, 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1142/0-3-8, 10=1595/0-3-8
Max Horz 2=-125(load case 7)
Max Uplift 2=-314(load case 6), 10=-549(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1887/872, 3-4=-1315/705, 4-5=-1099/703, 5-6=-823/505, 6-7=-1011/489, 7-8=-883/781, 8-9=0/47
BOT CHORD 2-14=-582/1595, 13-14=-606/1537, 12-13=-180/1023, 11-12=-180/1023, 10-11=-13/296, 8-10=-607/950
WEBS 3-14=0/291, 3-13=-511/421, 4-13=-54/307, 5-13=-117/156, 5-11=-433/266, 6-11=-11/221, 7-11=-388/666, 7-10=-1703/1317

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.70, 3 = 0.84, 4 = 0.54, 5 = 0.43, 6 = 0.54, 7 = 0.84, 8 = 0.70, 10 = 0.74, 11 = 0.61, 12 = 0.43, 13 = 0.61 and 14 = 0.74

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
	T17	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 549 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973321
	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:07 2008 Page 1

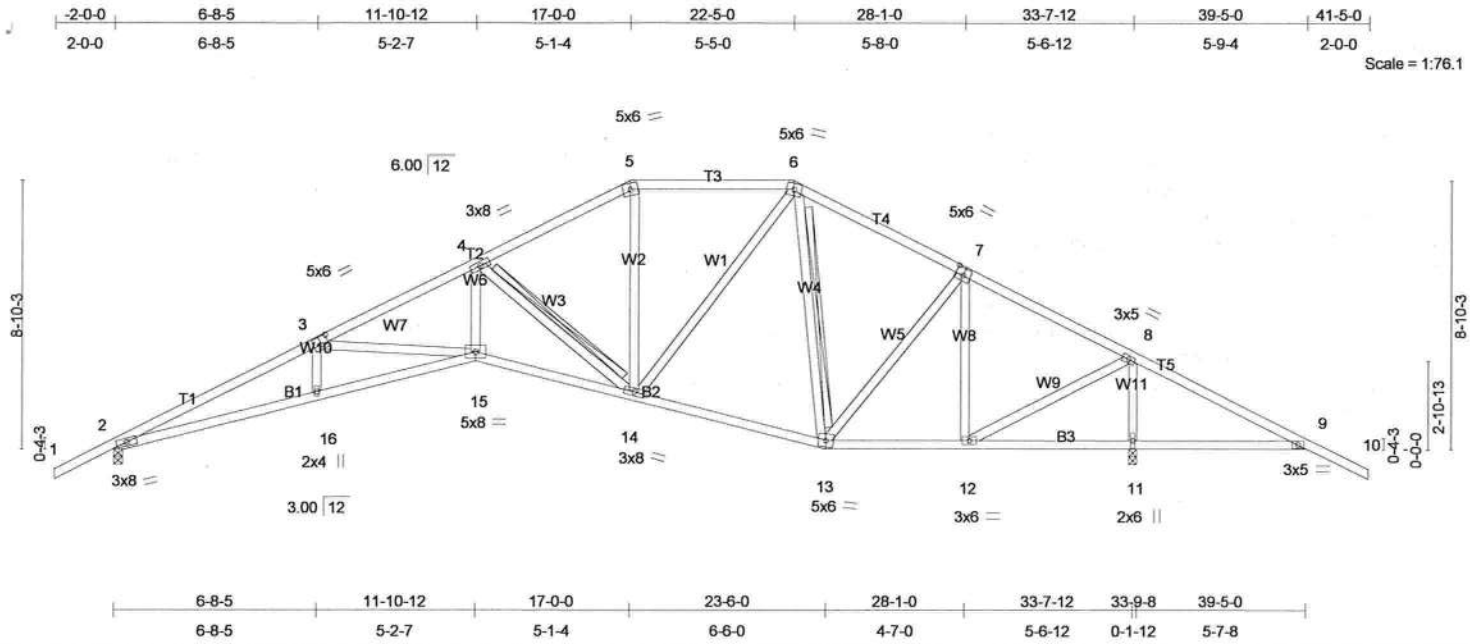


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.29	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.54	15-16	>743	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.29	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 221 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-136(load case 7)
Max Uplift 2=-350(load case 6), 11=-566(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3441/1524, 3-4=-2805/1216, 4-5=-1362/711, 5-6=-1172/693, 6-7=-959/544, 7-8=-774/276, 8-9=-807/679, 9-10=0/47
BOT CHORD 2-16=-1211/3075, 15-16=-1213/3074, 14-15=-787/2529, 13-14=-117/868, 12-13=-83/631, 11-12=-540/858, 9-11=-540/858
WEBS 3-16=0/184, 3-15=-537/413, 4-15=-495/1451, 4-14=-1676/759, 5-14=-105/336, 6-14=-213/590, 6-13=-347/156, 7-13=-150/259, 7-12=-581/474, 8-12=-899/1318, 8-11=-1520/1150

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973321
	T18	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.79, 3 = 0.53, 4 = 0.83, 5 = 0.43, 6 = 0.43, 7 = 0.45, 8 = 0.79, 9 = 0.54, 11 = 0.54, 12 = 0.74, 13 = 0.42, 14 = 0.85, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 2 and 566 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973322
	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:08 2008 Page 1

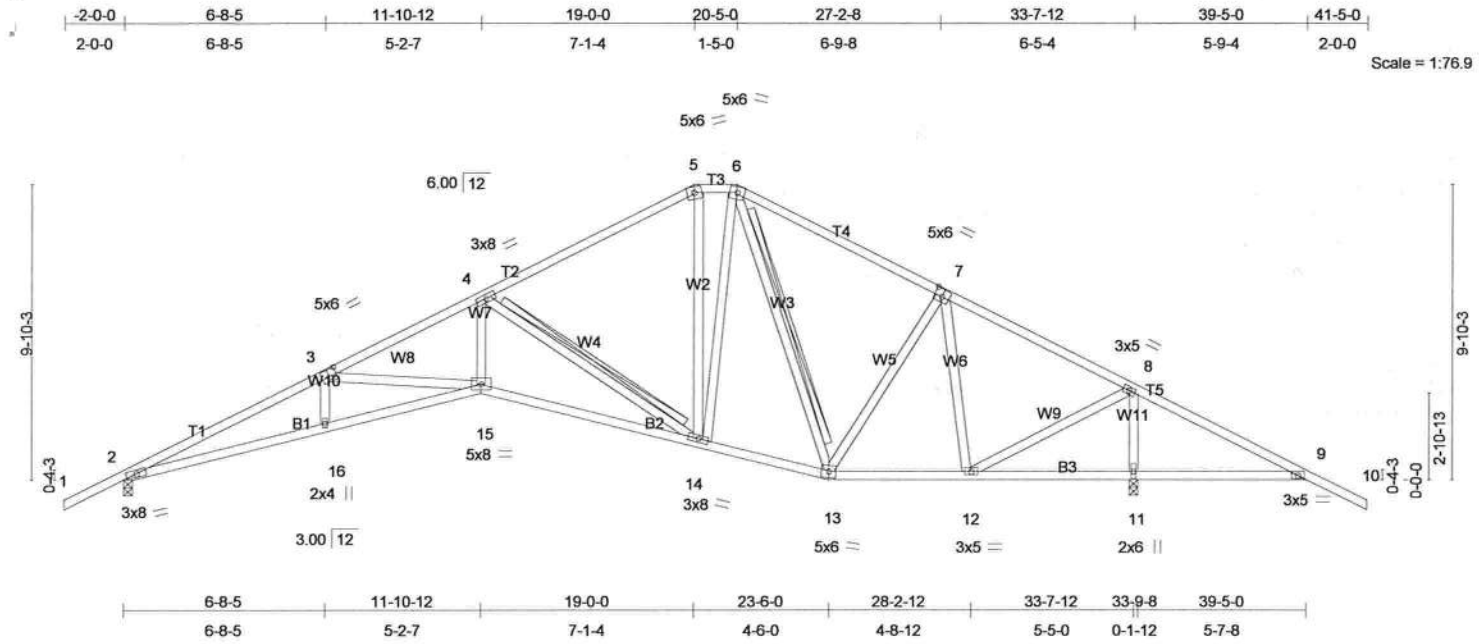


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.31	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.56	15-16	>712	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.32	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-148(load case 7)
Max Uplift 2=-331(load case 6), 11=-577(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3431/1543, 3-4=-2829/1255, 4-5=-1136/633, 5-6=-945/648, 6-7=-949/581, 7-8=-768/281, 8-9=-803/675, 9-10=0/47
BOT CHORD 2-16=-1227/3064, 15-16=-1227/3064, 14-15=-832/2561, 13-14=-82/904, 12-13=-73/693, 11-12=-535/854, 9-11=-535/854
WEBS 3-16=0/171, 3-15=-502/386, 4-15=-496/1468, 4-14=-1856/869, 5-14=-75/281, 6-14=-189/625, 6-13=-320/85, 7-13=-71/165, 7-12=-572/479, 8-12=-892/1306, 8-11=-1522/1159

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973322
	T19	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.79, 3 = 0.49, 4 = 0.84, 5 = 0.66, 6 = 0.58, 7 = 0.65, 8 = 0.79, 9 = 0.56, 11 = 0.55, 12 = 0.81, 13 = 0.28, 14 = 0.92, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2 and 577 lb uplift at joint 11.

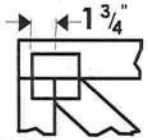
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

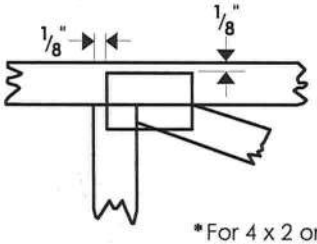
June 16,2008

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

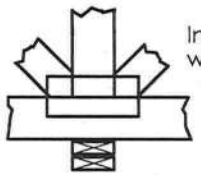
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

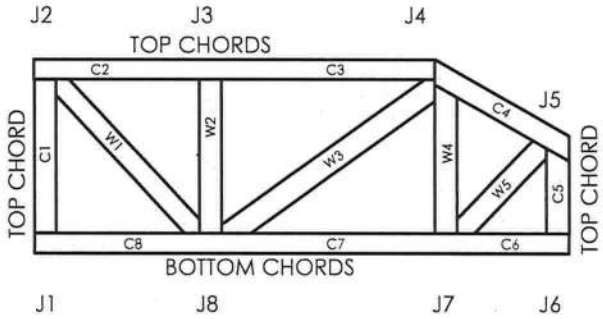
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 6"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 6"	13' 6"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 8"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 8"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 8"	8' 2"	10' 5"	10' 11"	13' 6"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 8"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

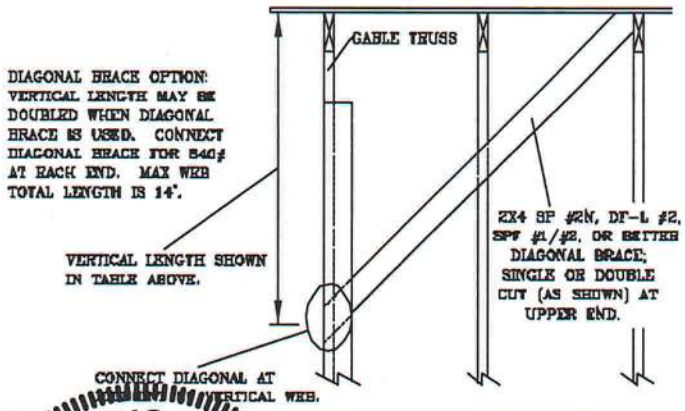
LIVE LOAD DEFLECTION CRITERIA IS L/240.

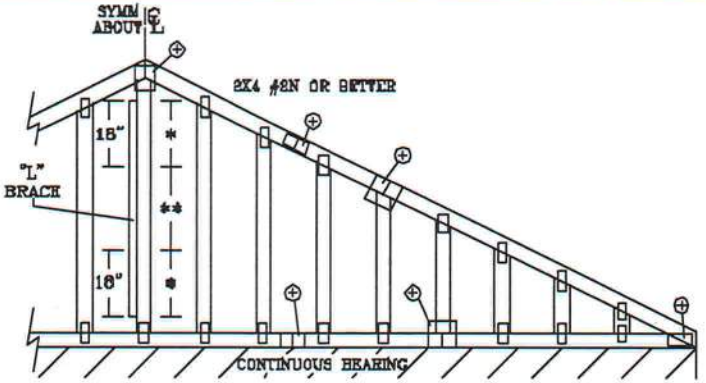
PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

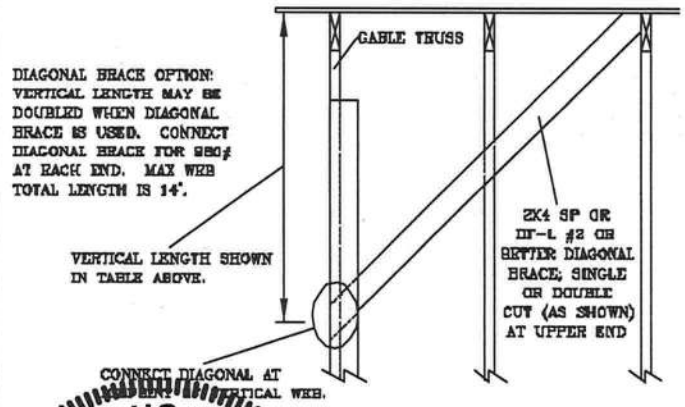
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

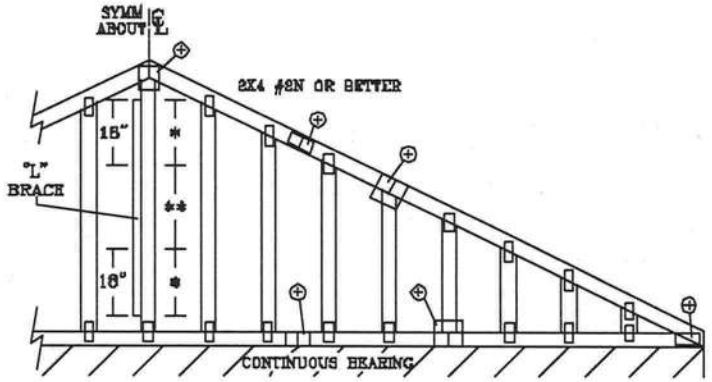
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

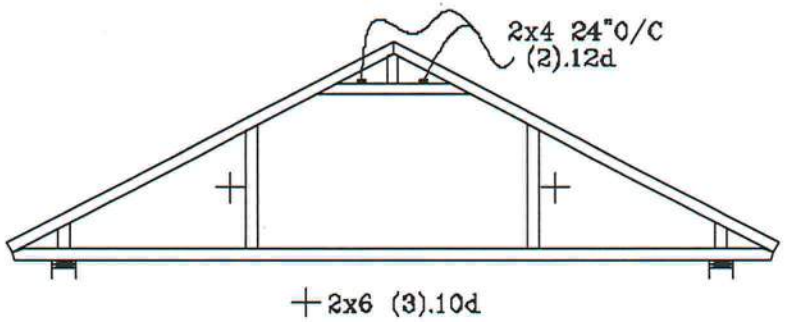
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

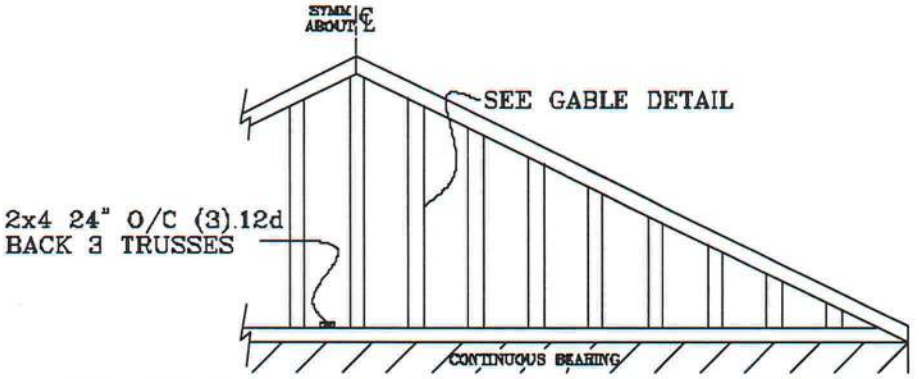
MAX. SPACING 24.0"

REF	ASCE7-02-GAB13030
DATE	11/26/03
DWG	MITEK STD GABLE 30' E WT
-ENG	

TYPICAL ATTIC TRUSS BRACING

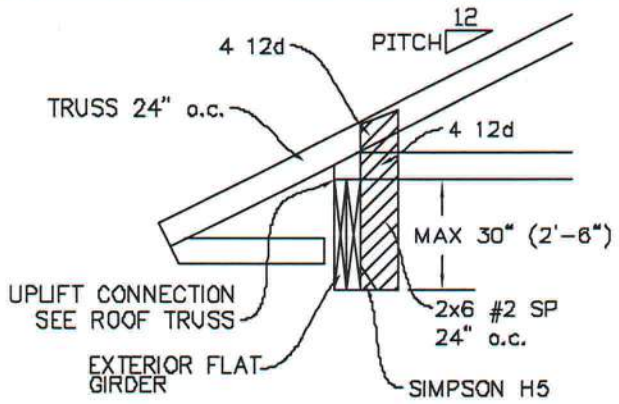


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

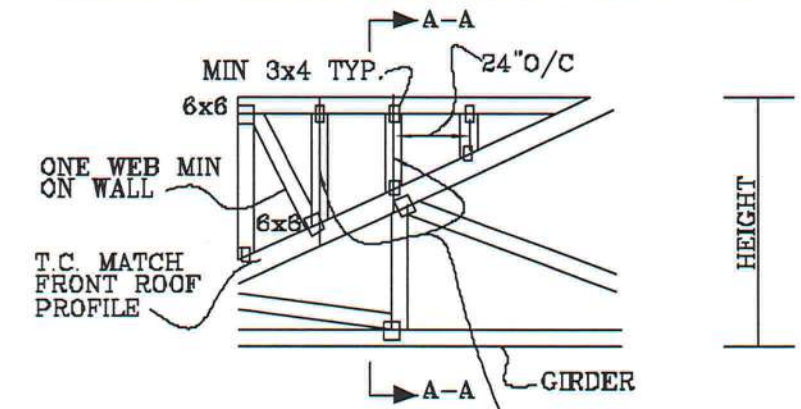
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

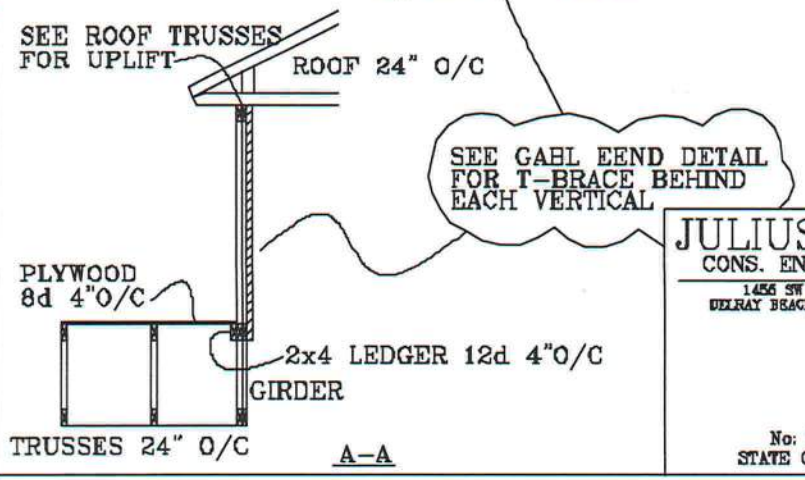


REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE
LICENSE
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

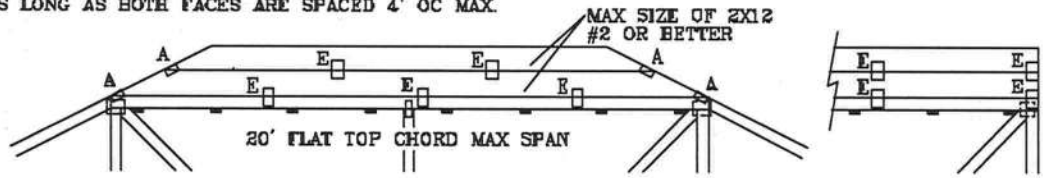
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

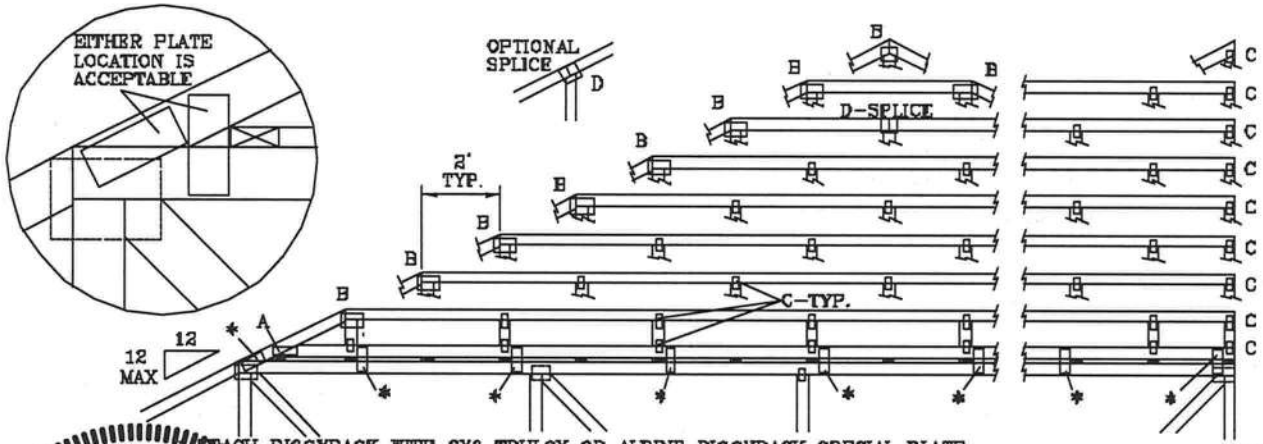
FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X8	5X8	6X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X6	6X5	5X8
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



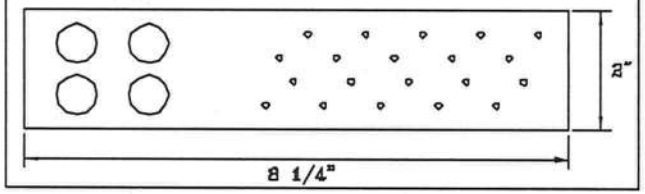
WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.



ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

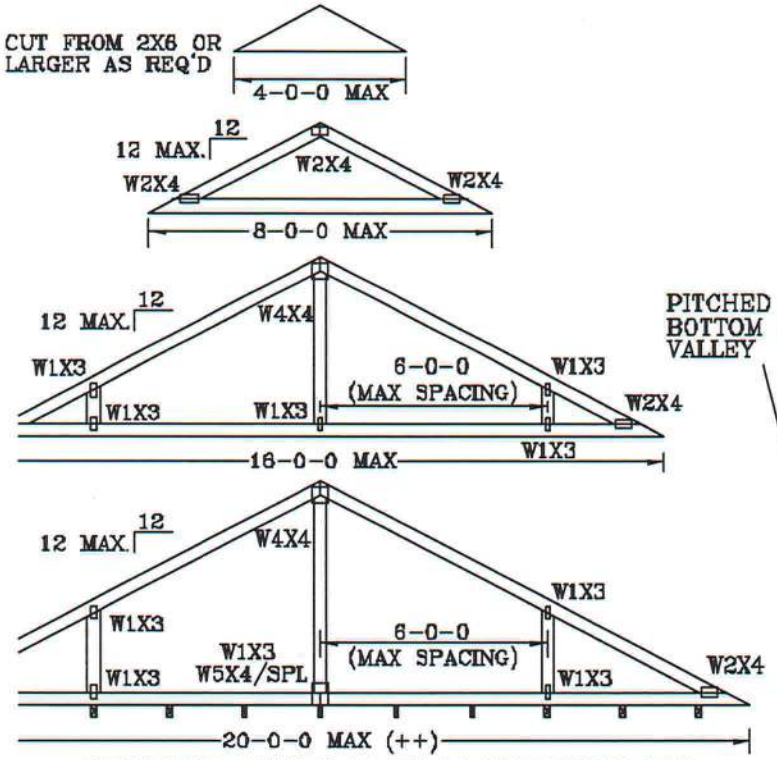
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

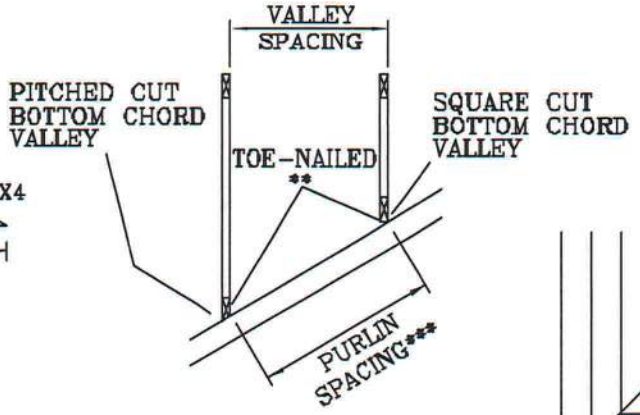
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

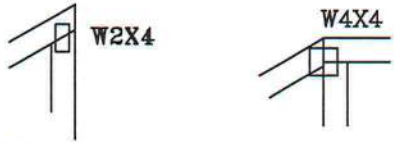
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

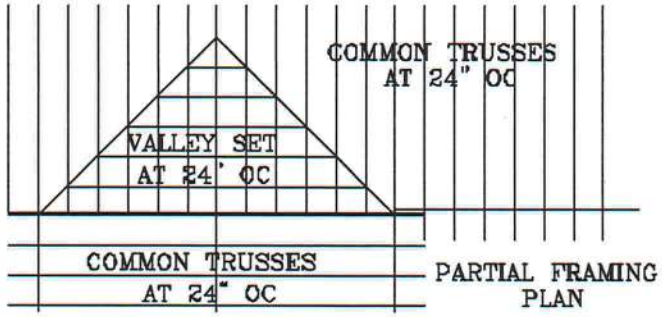
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.









SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

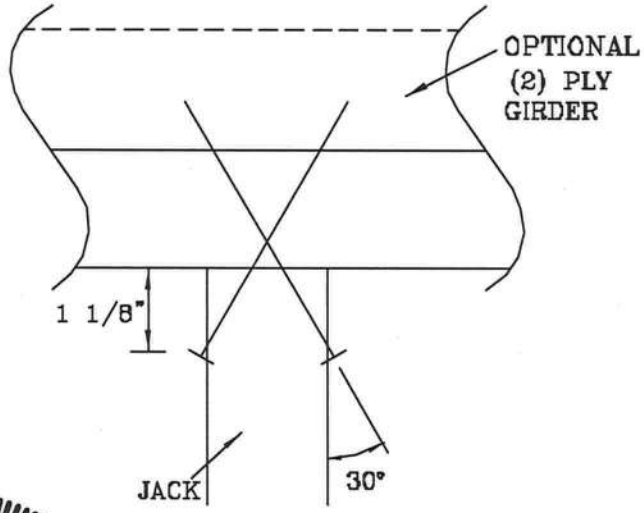
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

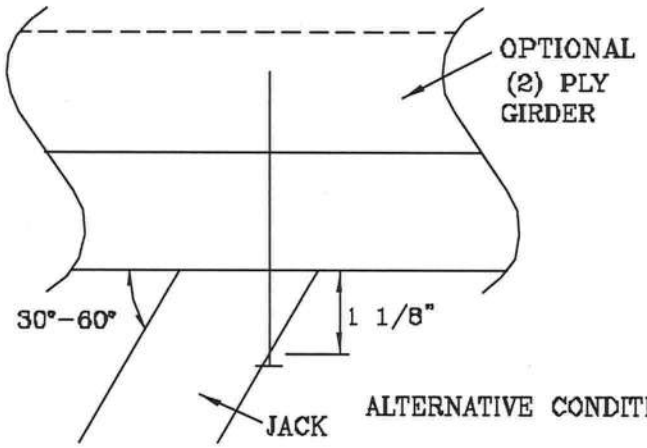
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

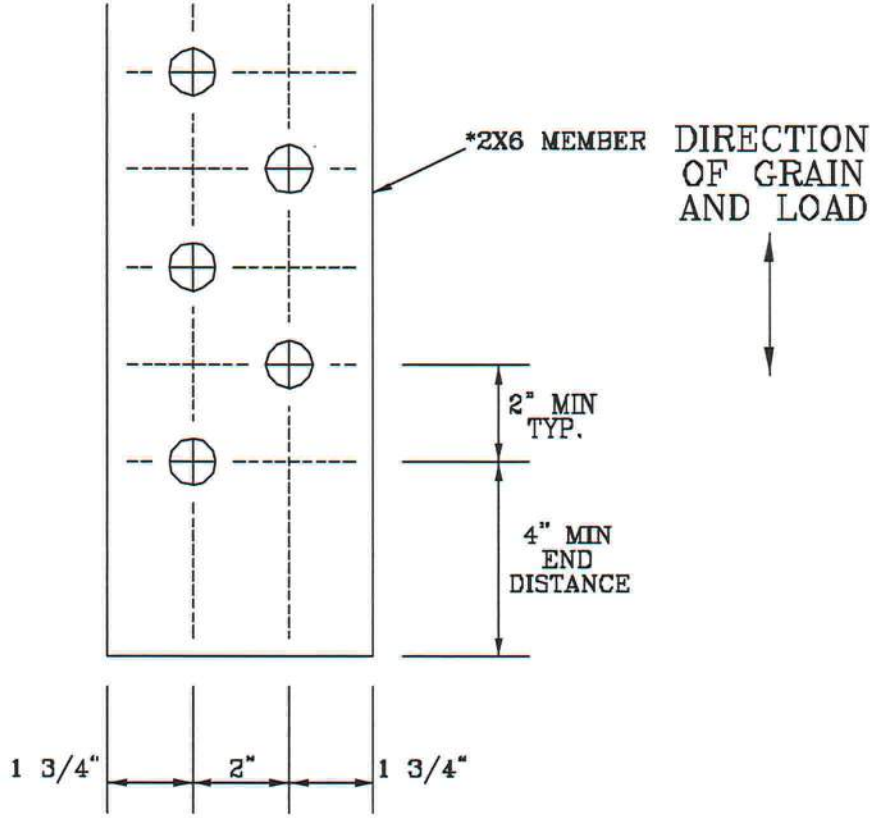
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

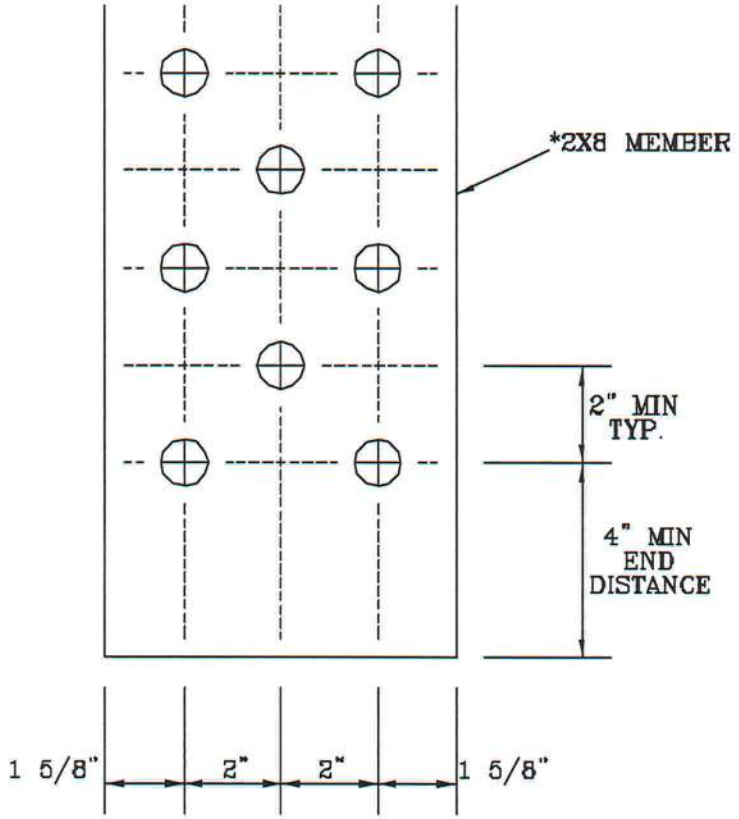
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

TC LL	PSF	
TC DL	PSF	
BC DL	PSF	
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

REF	BOLT SPACING
DATE	11/26/03
DRWG	CNBOLTSP1103
-ENG	JL

TRULOX CONNECTION DETAIL

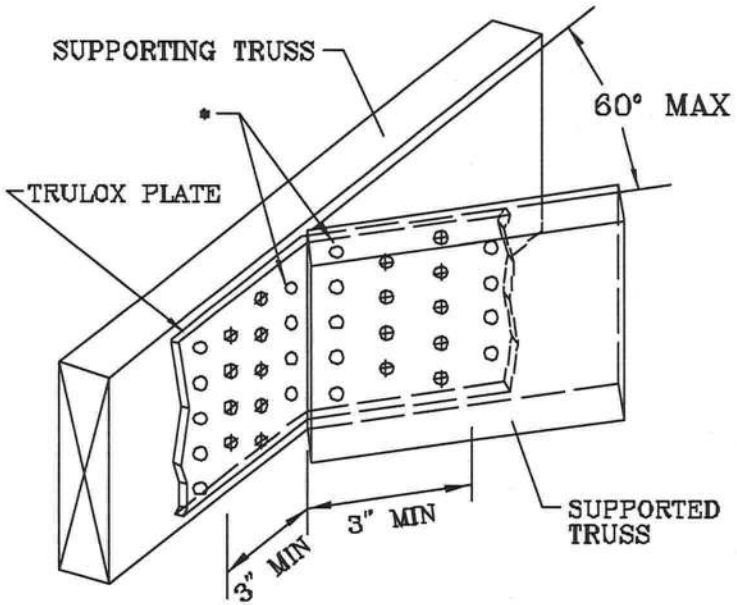
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

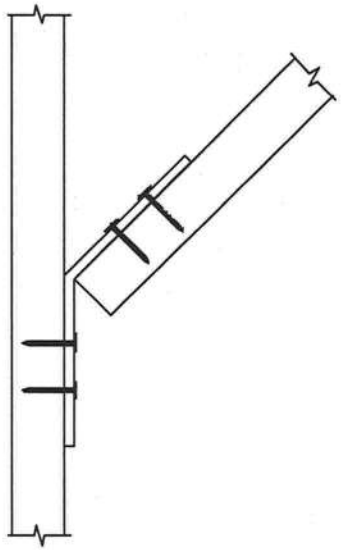
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

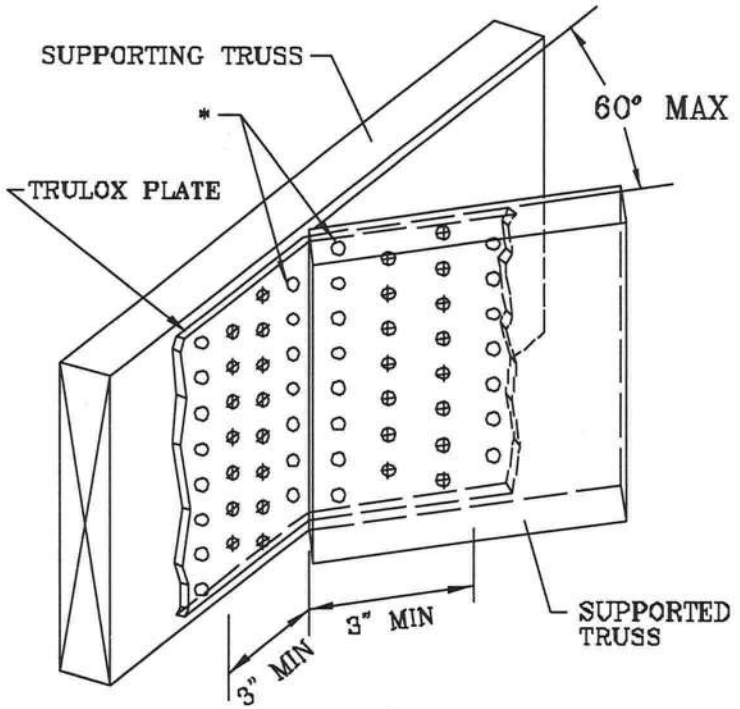
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

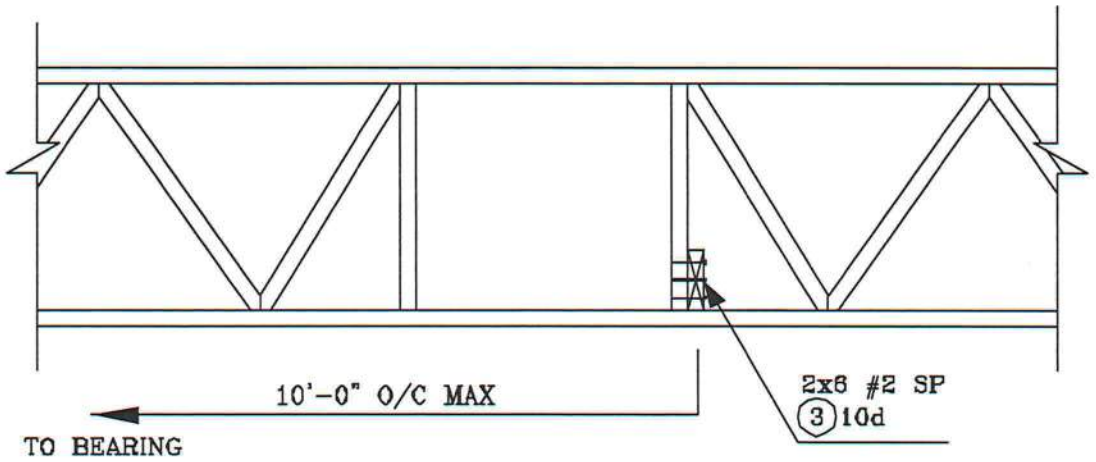
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

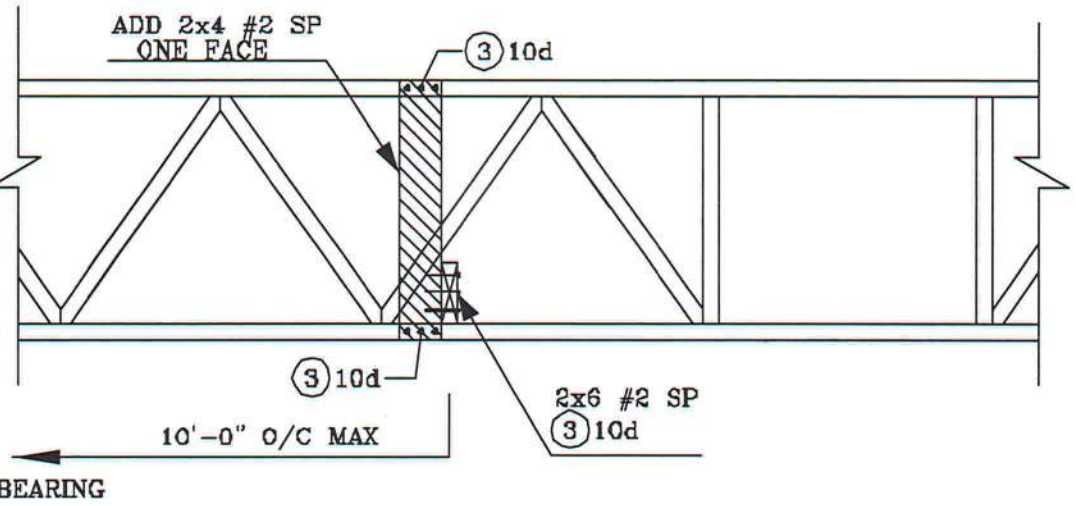
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

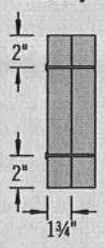
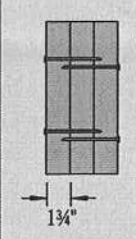
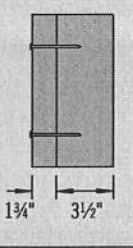
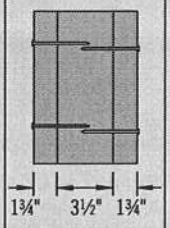
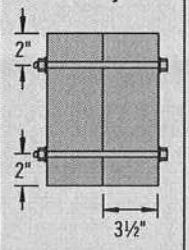
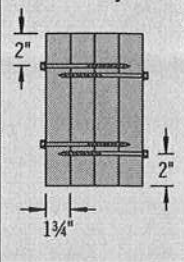
JULIUS LEE
LICENSE
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅜" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9/16" maximum.

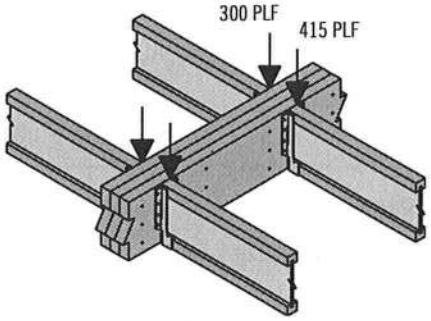
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

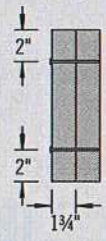
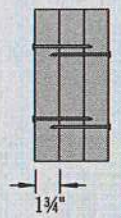
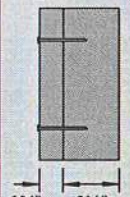
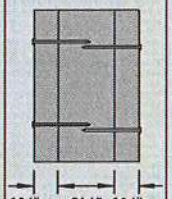
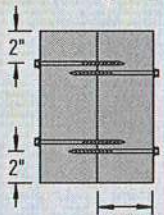
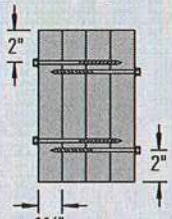
Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern: Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

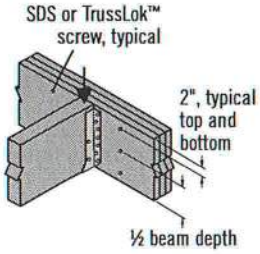
(2) 6" long screws required.

(3) 5" long screws required.

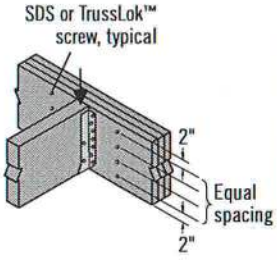
(4) 3½" and 3⅝" long screws must be installed on both sides.

Connections

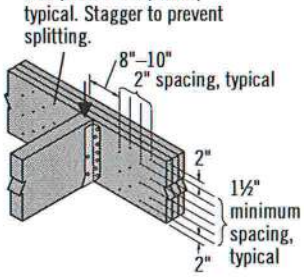
4 or 6 or Screw Connection



8 Screw Connection

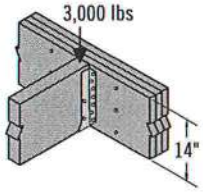


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

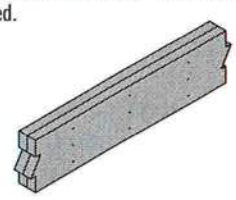
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

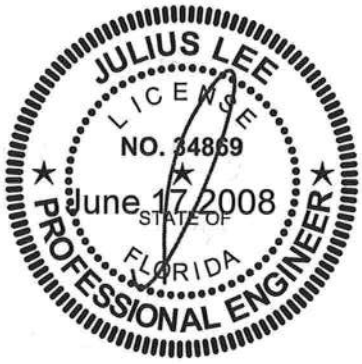
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

Builders FirstSource



Project Information for: L280800

Builder: Woodman Park Builders, Inc.
Address: 798 Southwest Utah Street
... Ft. White, FL 32038
County: Columbia
Truss Count: 26
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Mark E. Haddox Florida Certified Residental Contractor License No. CRC1329442
Address: Woodman Park Builders, Inc. 4816 W U.S. Highway 90 Suite# 100 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

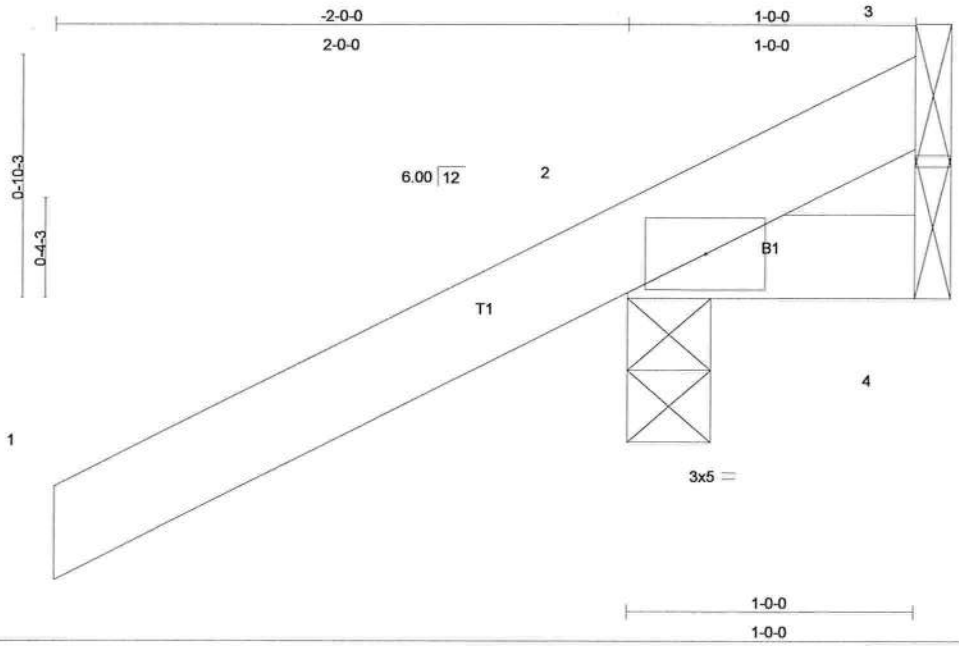
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1973297	CJ1	6/16/08
2	J1973298	CJ3	6/16/08
3	J1973299	CJ5	6/16/08
4	J1973300	EJ7	6/16/08
5	J1973301	HJ9	6/16/08
6	J1973302	T01	6/16/08
7	J1973303	T01G	6/16/08
8	J1973304	T02	6/16/08
9	J1973305	T03	6/16/08
10	J1973306	T03G	6/16/08
11	J1973307	T04	6/16/08
12	J1973308	T05	6/16/08
13	J1973309	T06	6/16/08
14	J1973310	T07	6/16/08
15	J1973311	T08	6/16/08
16	J1973312	T09	6/16/08
17	J1973313	T10	6/16/08
18	J1973314	T11	6/16/08
19	J1973315	T12	6/16/08
20	J1973316	T13	6/16/08
21	J1973317	T14	6/16/08
22	J1973318	T15	6/16/08
23	J1973319	T16	6/16/08
24	J1973320	T17	6/16/08
25	J1973321	T18	6/16/08
26	J1973322	T19	6/16/08

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973297
	CJ1	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-274(load case 6), 3=-90(load case 1)
Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

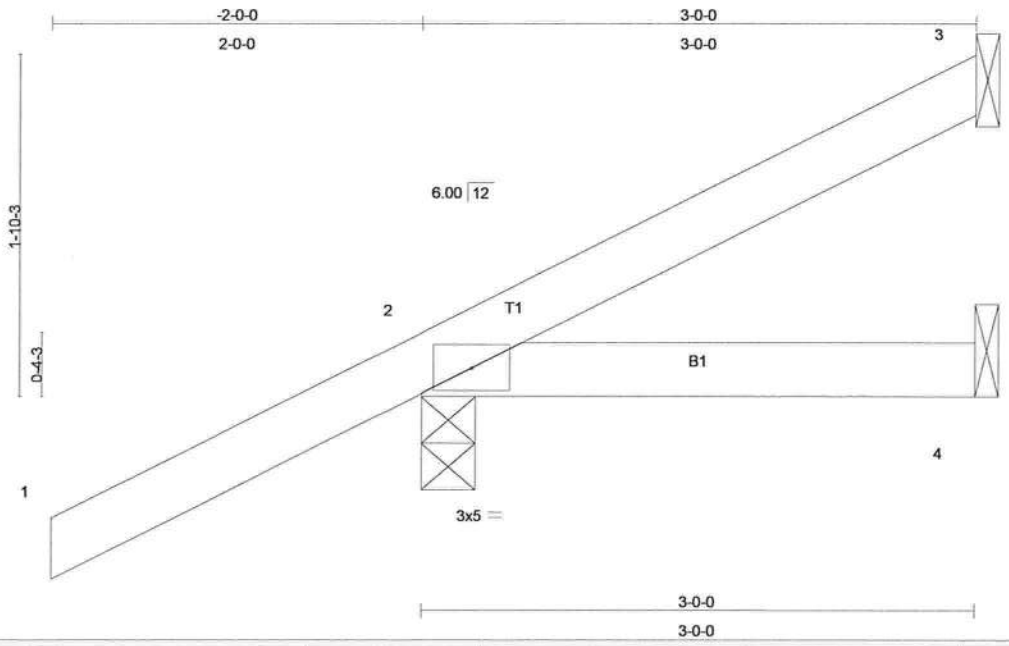
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973298
	CJ3	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-203(load case 6)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973298
	CJ3	JACK	4	1	Job Reference (optional)

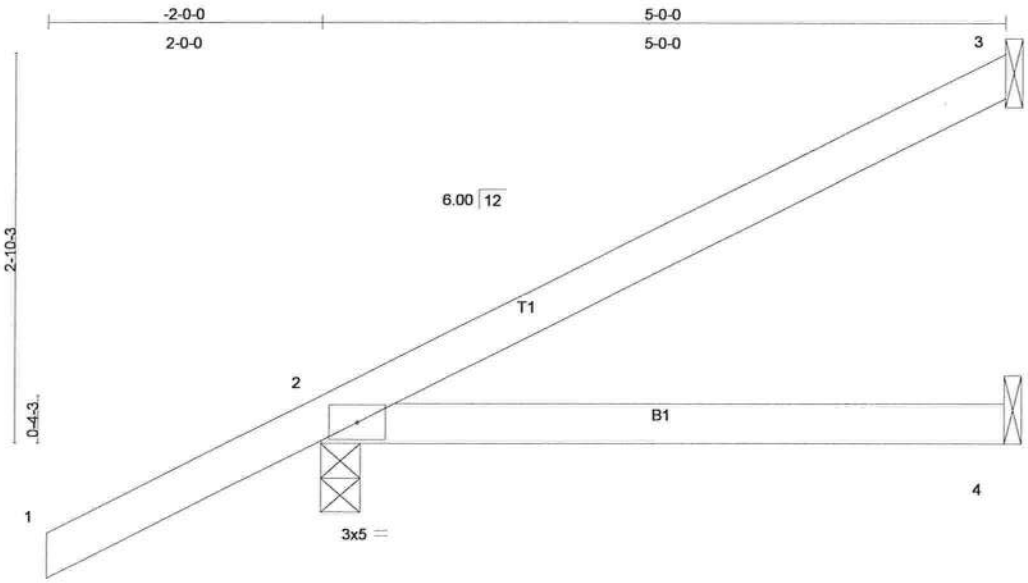
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
	CJ5	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:46 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-87(load case 6), 2=-199(load case 6)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
	CJ5	JACK	4	1	Job Reference (optional)

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	EJ7	JACK	25	1	J1973300
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1

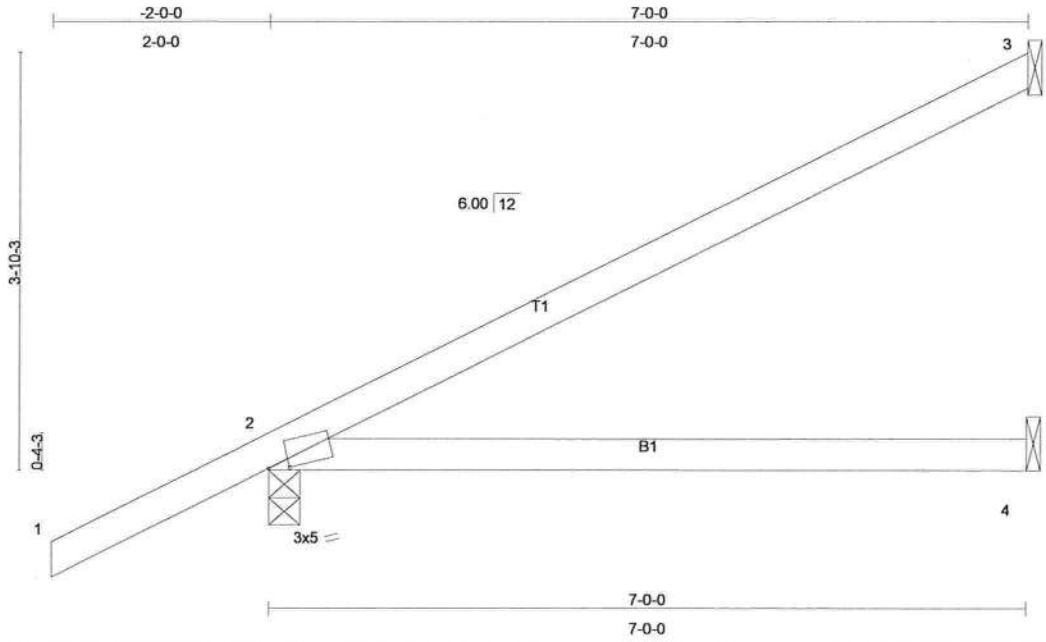


Plate Offsets (X,Y): [2:0-2-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>501	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-84(load case 6), 2=-139(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.81

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 139 lb uplift at joint 2.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

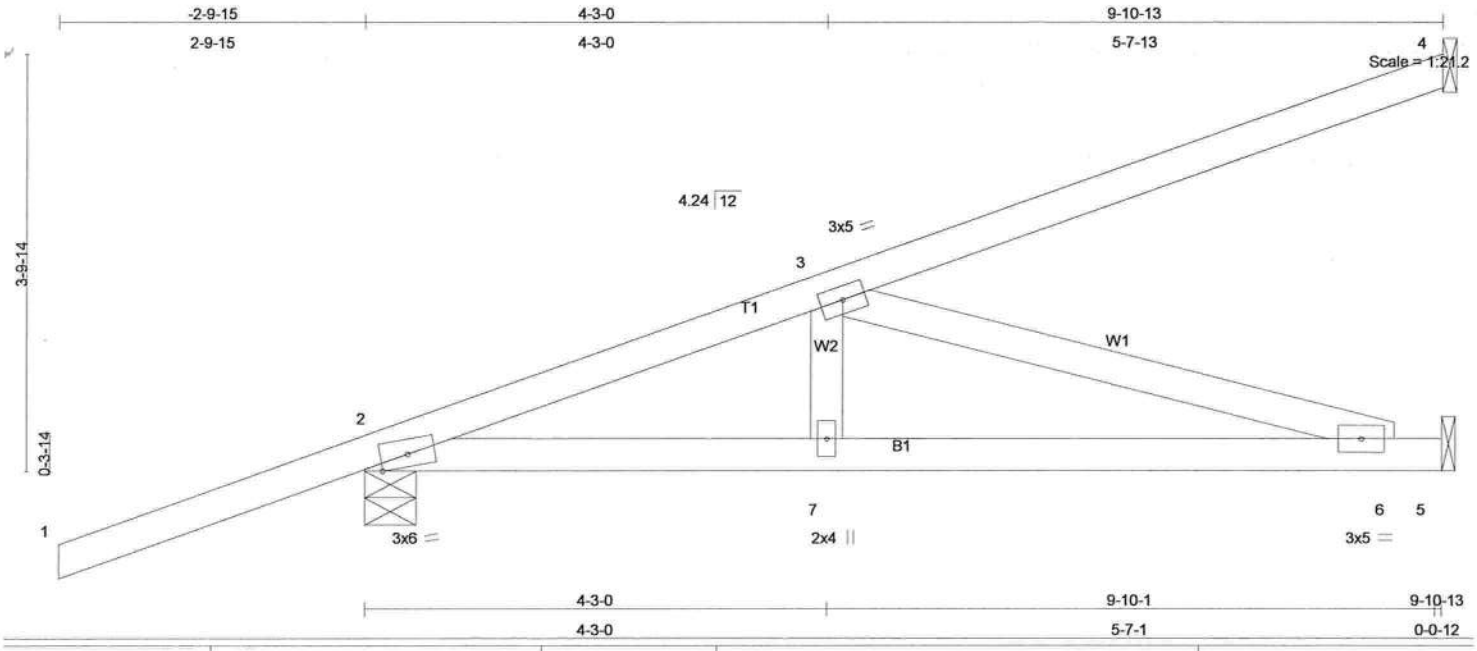
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973301
	HJ9	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>986	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift 4=-232(load case 3), 2=-281(load case 3), 5=-62(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-647/120, 3-4=-105/65
BOT CHORD 2-7=-308/599, 6-7=-308/599, 5-6=0/0
WEBS 3-7=0/190, 3-6=-624/321

JOINT STRESS INDEX
2 = 0.77, 3 = 0.18, 6 = 0.21 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973301
	HJ9	MONO TRUSS	2	1	Job Reference (optional)

LOAD CASE(S) Standard

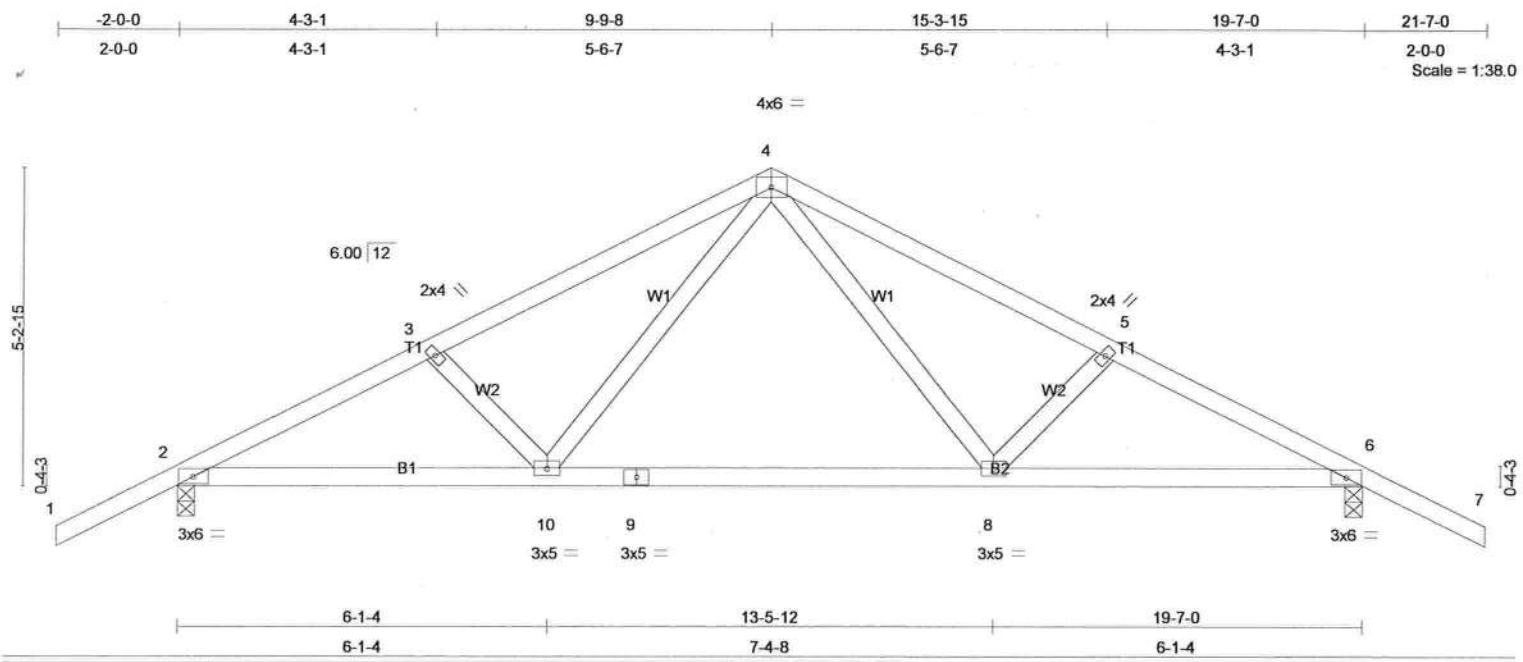
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-54
   Trapezoidal Loads (plf)
   Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1104 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973302
	T01	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:07:03 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.26	8-10	>901	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.40	8-10	>572	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 94 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-9 oc bracing.

REACTIONS (lb/size) 2=955/0-3-8, 6=955/0-3-8
Max Horz 2=-94(load case 7)
Max Uplift 2=-292(load case 6), 6=-292(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1603/857, 3-4=-1456/828, 4-5=-1456/828, 5-6=-1603/857, 6-7=0/47
BOT CHORD 2-10=-608/1365, 9-10=-310/911, 8-9=-310/911, 6-8=-608/1365
WEBS 3-10=-195/185, 4-10=-287/583, 4-8=-287/583, 5-8=-195/185

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.76, 5 = 0.34, 6 = 0.71, 8 = 0.42, 9 = 0.60 and 10 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34169
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973303
	T01G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:18:03 2008 Page 1

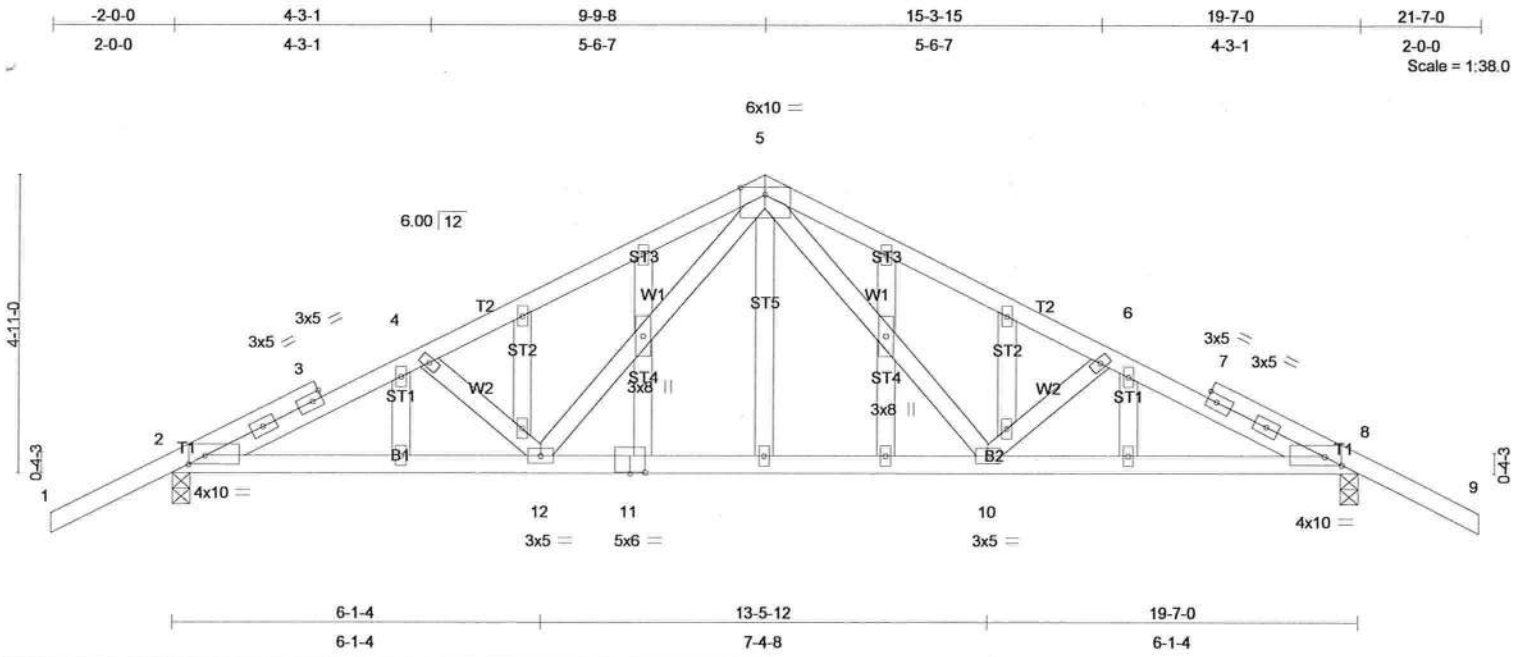


Plate Offsets (X,Y): [2:0-3-4,0-1-12], [8:0-3-4,0-1-12], [11:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.21	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.35	10-12	>656	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-9-15 oc bracing.

REACTIONS (lb/size) 2=1551/0-3-8, 8=1551/0-3-8
Max Horz 2=-102(load case 7)
Max Uplift 2=-796(load case 6), 8=-796(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-23/100, 2-3=-2682/1452, 3-4=-2575/1414, 4-5=-2272/1256, 5-6=-2272/1256, 6-7=-2575/1414, 7-8=-2682/1452, 8-9=-23/100
BOT CHORD	2-12=-1176/2364, 11-12=-612/1424, 10-11=-612/1424, 8-10=-1176/2364
WEBS	4-12=-618/422, 5-12=-401/787, 5-10=-401/787, 6-10=-618/422

JOINT STRESS INDEX

2 = 0.90, 3 = 0.00, 3 = 0.62, 3 = 0.72, 4 = 0.34, 5 = 0.48, 6 = 0.34, 7 = 0.00, 7 = 0.72, 7 = 0.62, 8 = 0.90, 10 = 0.56, 11 = 0.43, 12 = 0.56 , 13 = 0.34, 14 = 0.48, 15 = 0.34, 16 = 0.00, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.48, 24 = 0.34, 25 = 0.34, 26 = 0.34 and 27 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973303
	T01G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 796 lb uplift at joint 2 and 796 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-12=-10, 10-12=-40(F=-30), 8-10=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1104 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973304
	T02	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:07:39 2008 Page 1

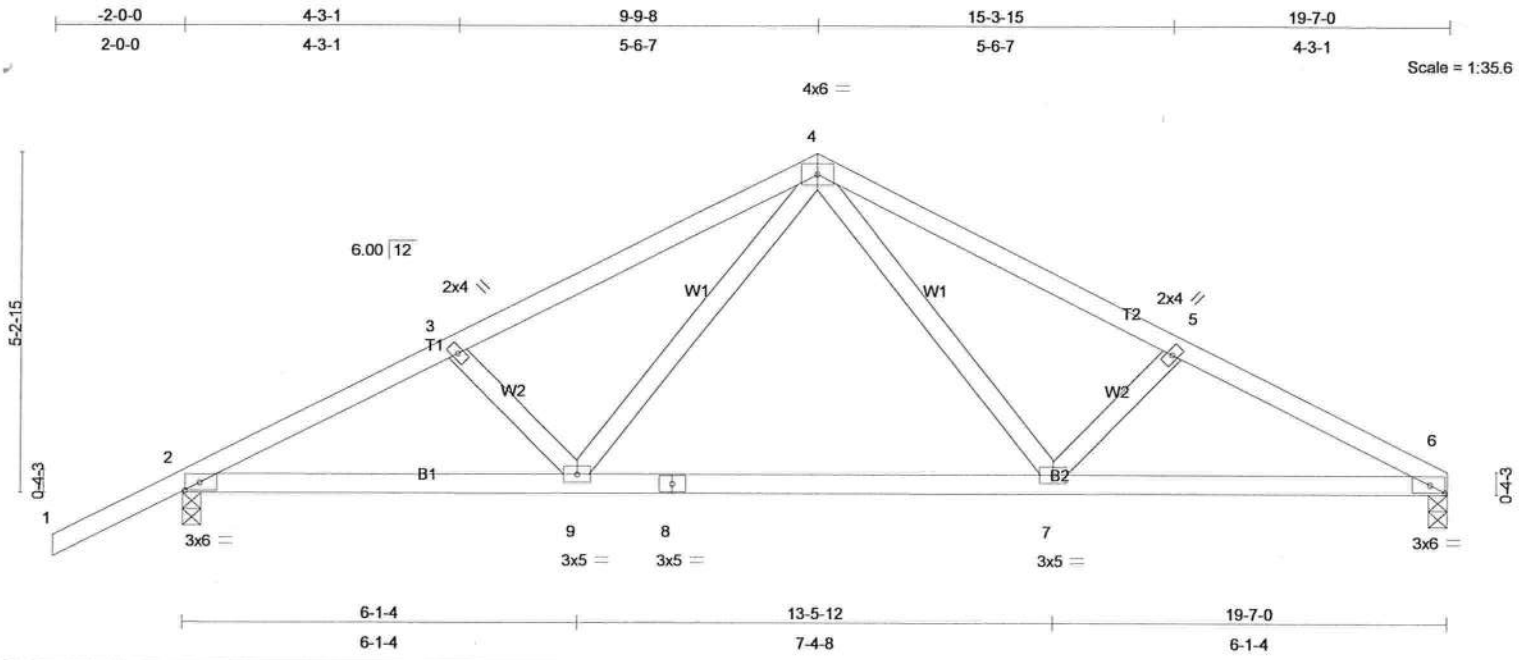


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.25	7-9	>921	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.40	7-9	>580	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-0-2 oc bracing.

REACTIONS (lb/size) 6=832/0-3-8, 2=961/0-3-8
Max Horz 2=107(load case 6)
Max Uplift 6=-195(load case 7), 2=-293(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-1615/878, 3-4=-1468/849, 4-5=-1495/894, 5-6=-1648/932
BOT CHORD	2-9=-704/1376, 8-9=-409/924, 7-8=-409/924, 6-7=-766/1413
WEBS	3-9=-195/187, 4-9=-282/582, 4-7=-347/619, 5-7=-217/221

JOINT STRESS INDEX

2 = 0.76, 3 = 0.34, 4 = 0.73, 5 = 0.34, 6 = 0.76, 7 = 0.45, 8 = 0.60 and 9 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 6 and 293 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973304
	T02	COMMON	6	1	Job Reference (optional)

LOAD CASE(S) Standard

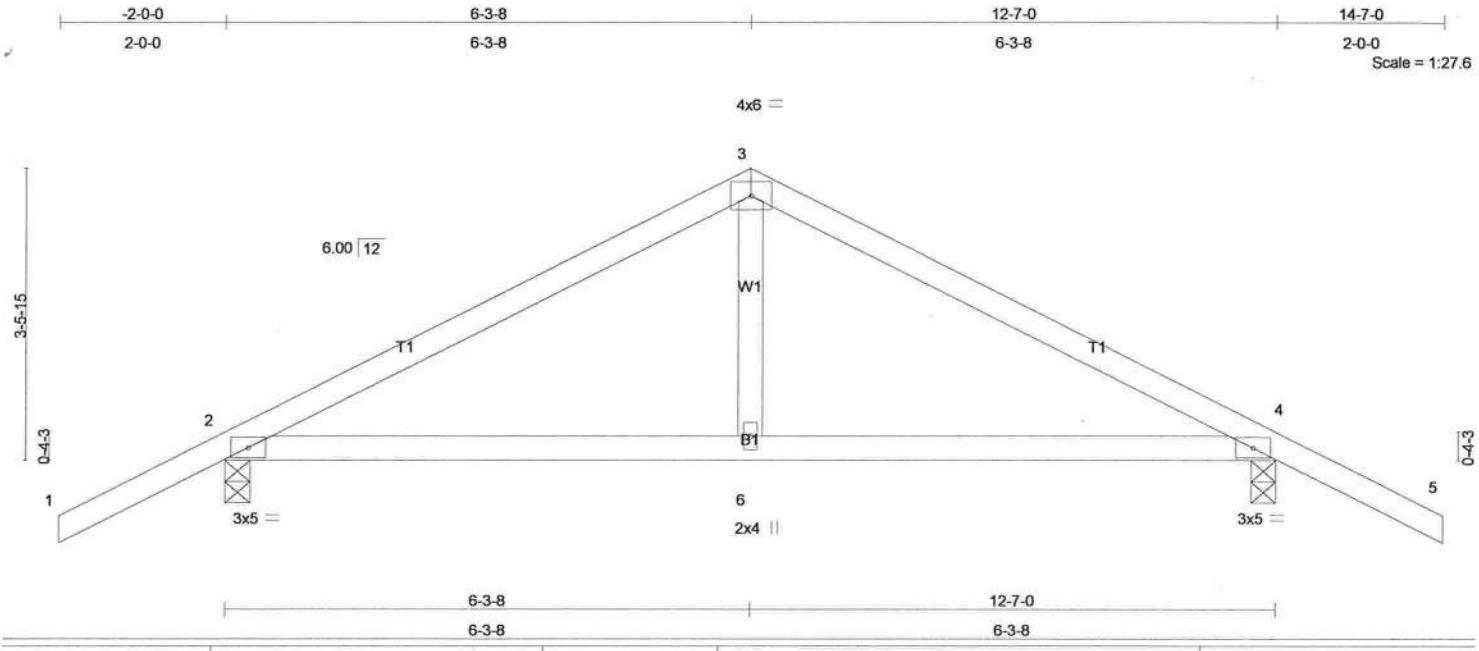
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Key Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T03	COMMON	3	1	J1973305
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:51 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=509/0-3-8, 4=509/0-3-8
Max Horz 2=-73(load case 7)
Max Uplift 2=-184(load case 6), 4=-184(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-537/276, 3-4=-537/276, 4-5=0/47
BOT CHORD	2-6=-66/412, 4-6=-66/412
WEBS	3-6=0/210

JOINT STRESS INDEX
2 = 0.40, 3 = 0.73, 4 = 0.40 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 2 and 184 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Builders FirstSource

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T03G	GABLE	1	1	J1973306
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:52 2008 Page 1

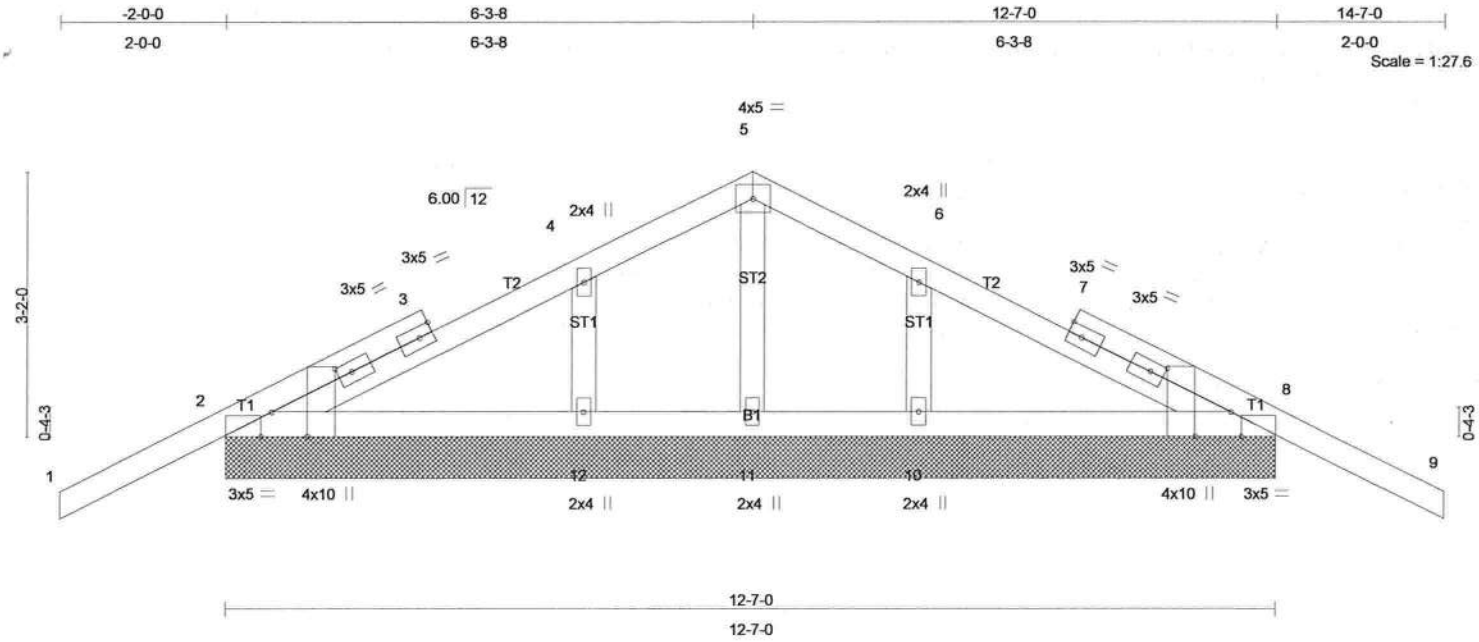


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [3:0-1-15,0-1-8], [7:0-1-15,0-1-8], [8:0-3-8,Edge], [8:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.05	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 61 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=489/12-7-0, 8=489/12-7-0, 11=206/12-7-0, 12=416/12-7-0, 10=416/12-7-0
Max Horz 2=-78(load case 7)
Max Uplift 2=-319(load case 6), 8=-332(load case 7), 11=-52(load case 6), 12=-205(load case 6), 10=-208(load case 7)
Max Grav 2=494(load case 10), 8=494(load case 11), 11=206(load case 1), 12=417(load case 10), 10=417(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-27/99, 2-3=-34/48, 3-4=-64/174, 4-5=-6/109, 5-6=-6/109, 6-7=-41/174, 7-8=-34/48, 8-9=-27/99
BOT CHORD	2-12=-71/144, 11-12=-71/144, 10-11=-71/144, 8-10=-71/144
WEBS	5-11=-206/56, 4-12=-374/295, 6-10=-374/295

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.78, 2 = 0.00, 3 = 0.00, 3 = 0.49, 3 = 0.49, 4 = 0.15, 5 = 0.10, 6 = 0.15, 7 = 0.00, 7 = 0.49, 7 = 0.49, 8 = 0.78, 8 = 0.00, 10 = 0.16, 11 = 0.07 and 12 = 0.16

NOTES

1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973306
	T03G	GABLE	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2, 332 lb uplift at joint 8, 52 lb uplift at joint 11, 205 lb uplift at joint 12 and 208 lb uplift at joint 10.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1180 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973307
	T04	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:53 2008 Page 1

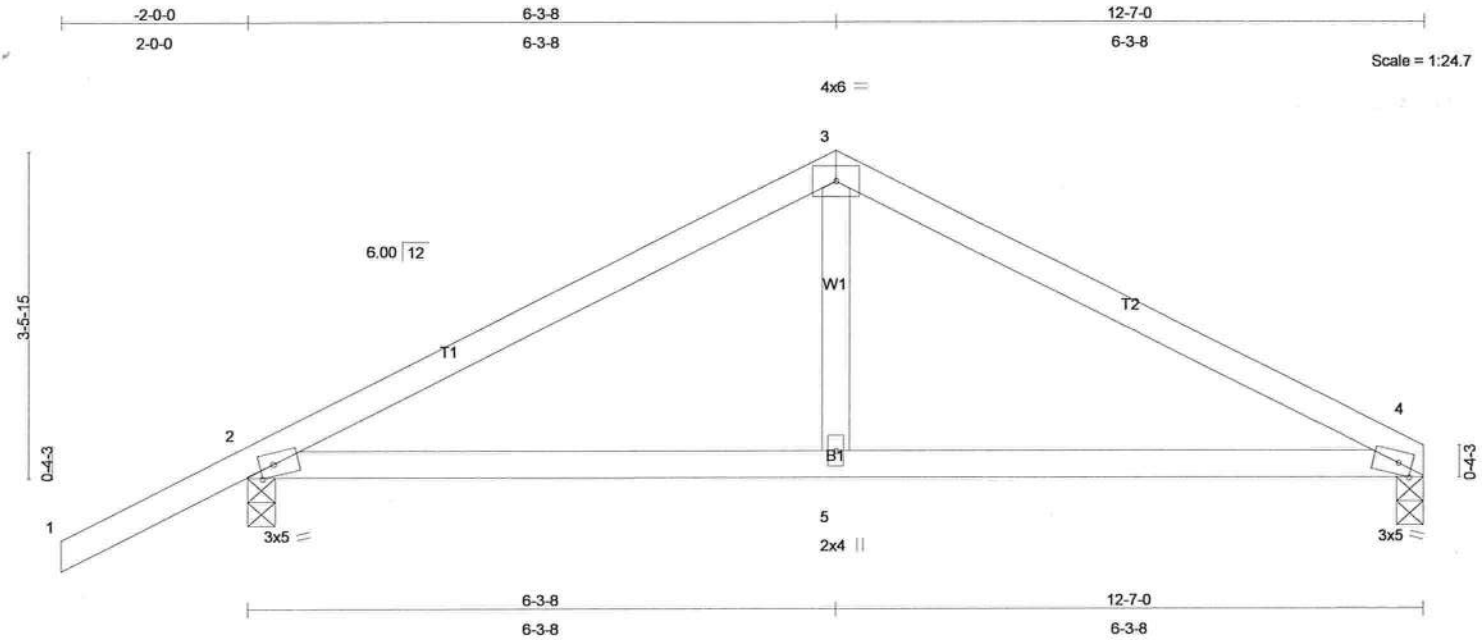


Plate Offsets (X,Y): [2:0-1-12,Edge], [4:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.05	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.07	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=383/0-3-8, 2=519/0-3-8
Max Horz 2=86(load case 6)
Max Uplift 4=-84(load case 7), 2=-186(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-568/328, 3-4=-564/320
BOT CHORD 2-5=-191/441, 4-5=-191/441
WEBS 3-5=0/214

JOINT STRESS INDEX
2 = 0.73, 3 = 0.81, 4 = 0.73 and 5 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973307
	T04	COMMON	3	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 4 and 186 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34-46610
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T05	COMMON	1	2	J1973308
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

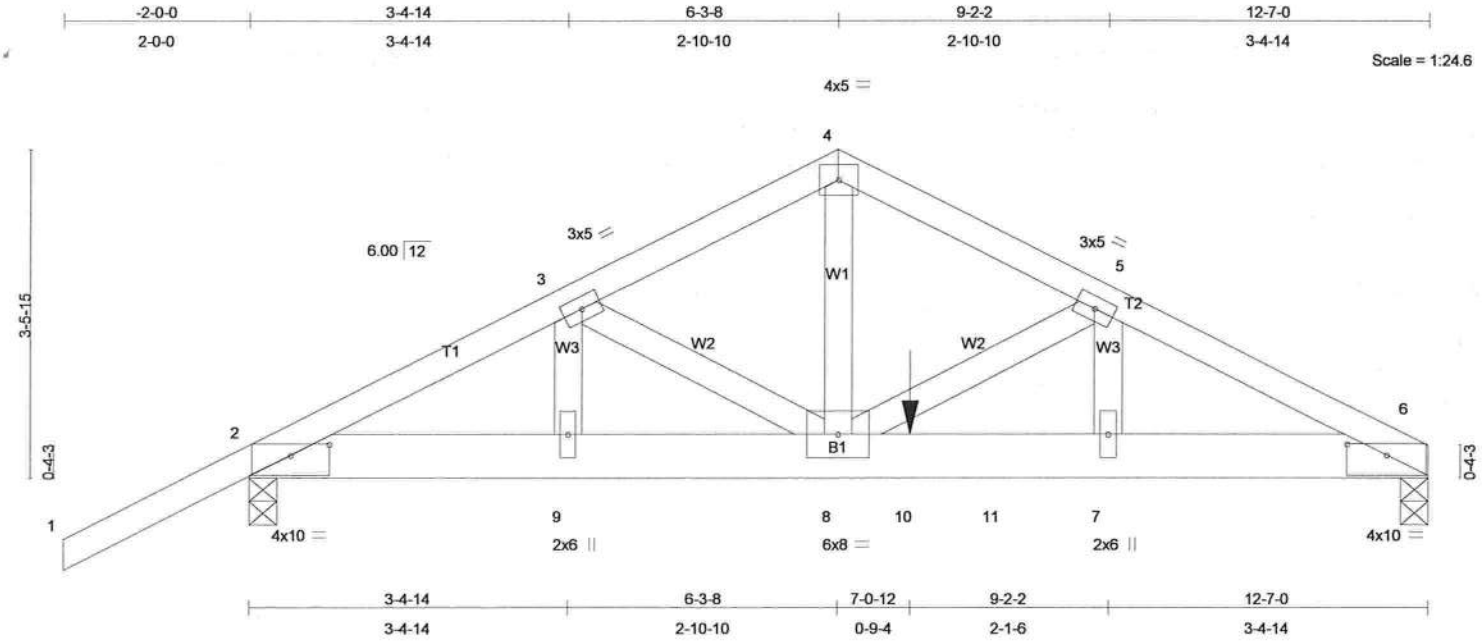


Plate Offsets (X,Y): [2:0-5-0,0-1-7], [6:0-5-0,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.11	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-4-13 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=3563/0-3-8, 2=1960/0-3-8
Max Horz 2=89(load case 5)
Max Uplift 6=-964(load case 6), 2=-585(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/51, 2-3=-3554/919, 3-4=-3847/1051, 4-5=-3843/1043, 5-6=-5951/1603
BOT CHORD	2-9=-785/3123, 8-9=-785/3123, 8-10=-1397/5275, 10-11=-1397/5275, 7-11=-1397/5275, 6-7=-1397/5275
WEBS	3-9=-410/169, 3-8=-115/445, 4-8=-864/3193, 5-8=-2145/625, 5-7=-507/1917

JOINT STRESS INDEX
2 = 0.65, 3 = 0.81, 4 = 0.75, 5 = 0.81, 6 = 0.65, 7 = 0.45, 8 = 0.39 and 9 = 0.45

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973308
	T05	COMMON	1	2	Job Reference (optional)

NOTES

3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 6 and 585 lb uplift at joint 2.
8) Girder carries tie-in span(s): 33-9-8 from 8-0-0 to 12-7-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-519(F=-509)
   Concentrated Loads (lb)
   Vert: 10=-2363(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
	T06	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

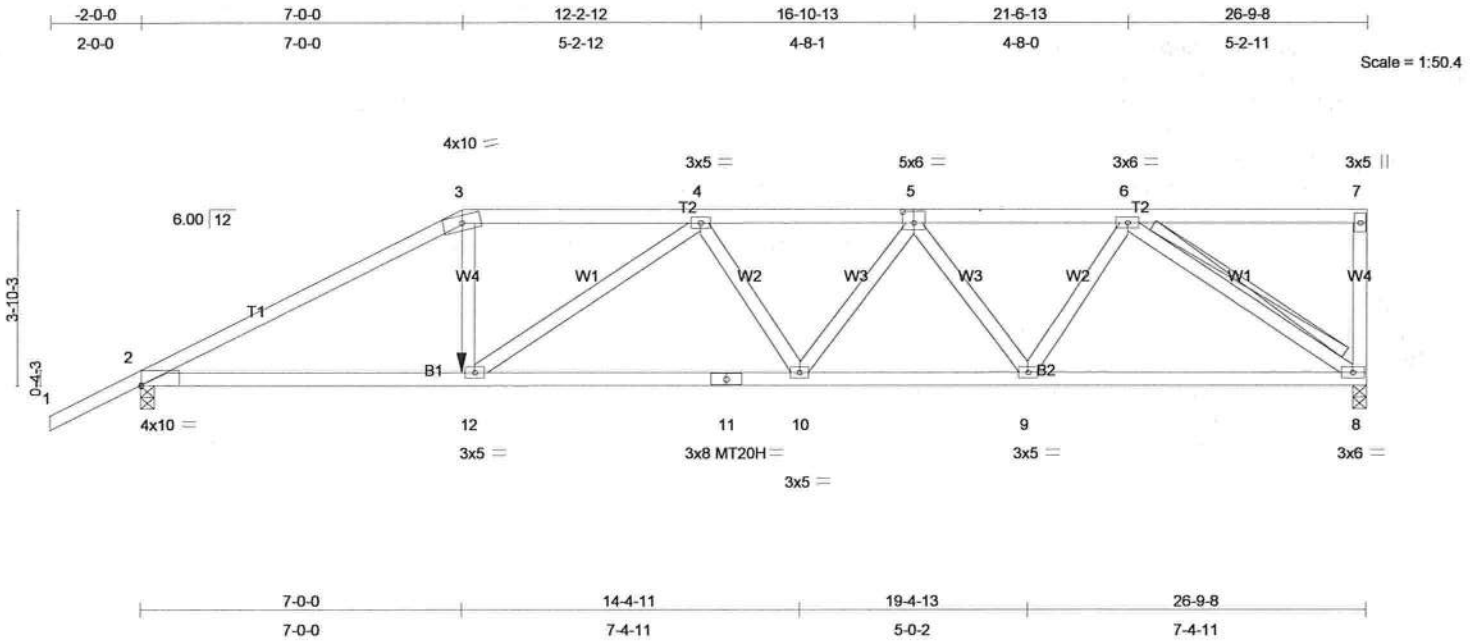


Plate Offsets (X,Y): [2:Edge,0-0-2], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.19	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.44	10-12	>728	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=1883/0-3-8, 2=1818/0-3-8
Max Horz 2=163(load case 5)
Max Uplift 8=-649(load case 4), 2=-578(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3360/1085, 3-4=-2956/1008, 4-5=-3532/1196, 5-6=-2790/938, 6-7=-85/15, 7-8=-288/143
BOT CHORD 2-12=-997/2914, 11-12=-1273/3610, 10-11=-1273/3610, 9-10=-1162/3346, 8-9=-798/2236
WEBS 3-12=-296/996, 4-12=-799/378, 4-10=-150/148, 5-10=-60/323, 5-9=-966/389, 6-9=-270/1065, 6-8=-2625/956

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.76, 3 = 0.77, 4 = 0.42, 5 = 0.55, 6 = 0.79, 7 = 0.64, 8 = 0.79, 9 = 0.79, 10 = 0.42, 11 = 0.87 and 12 = 0.73

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
	T06	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 649 lb uplift at joint 8 and 578 lb uplift at joint 2.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 12=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1893 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
	T07	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:55 2008 Page 1

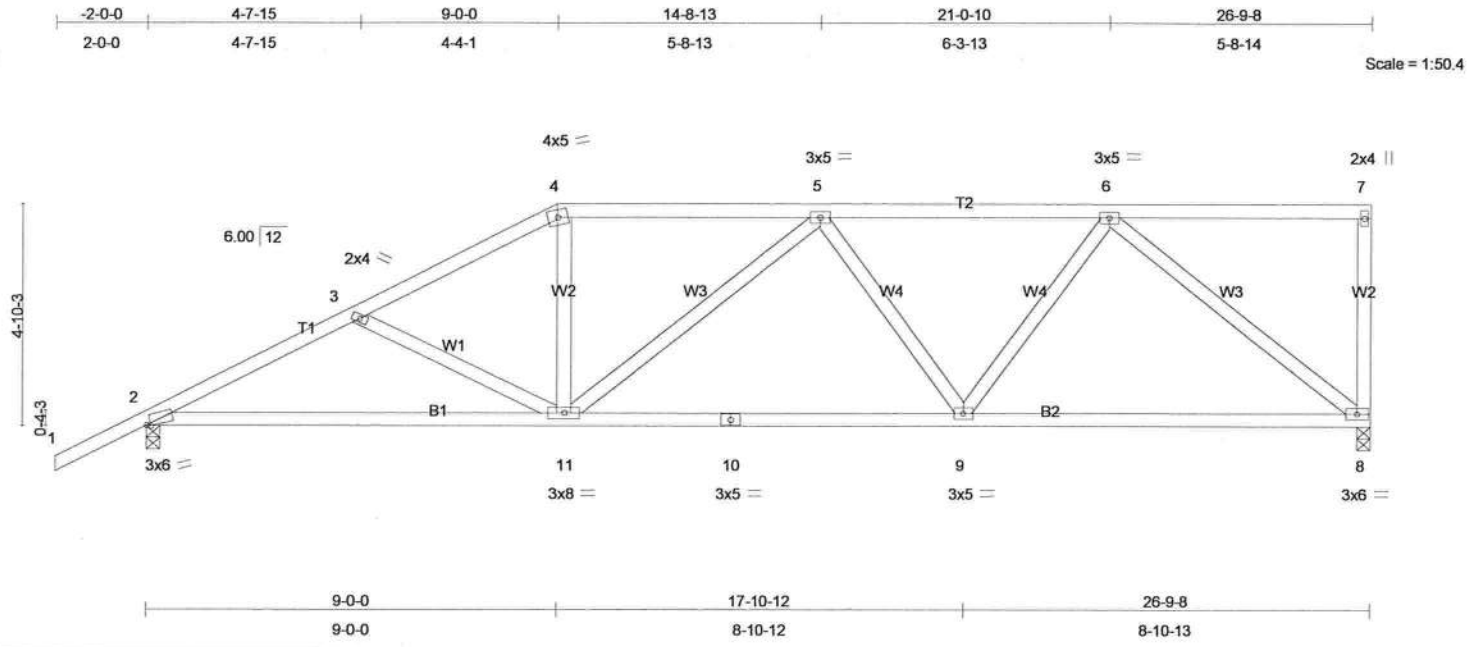


Plate Offsets (X,Y): [2:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.13	2-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.24	2-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.

REACTIONS (lb/size) 8=843/0-3-8, 2=969/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 8=-229(load case 5), 2=-248(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1537/763, 3-4=-1299/656, 4-5=-1126/644, 5-6=-1073/568, 6-7=-40/7, 7-8=-135/93
BOT CHORD 2-11=-827/1311, 10-11=-698/1238, 9-10=-698/1238, 8-9=-476/847
WEBS 3-11=-216/207, 4-11=-73/334, 5-11=-145/117, 5-9=-290/228, 6-9=-162/410, 6-8=-1052/605

JOINT STRESS INDEX
2 = 0.83, 3 = 0.33, 4 = 0.74, 5 = 0.41, 6 = 0.41, 7 = 0.82, 8 = 0.60, 9 = 0.41, 10 = 0.48 and 11 = 0.56

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
	T07	MONO HIP	1	1	Job Reference (optional)

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 8 and 248 lb uplift at joint 2.

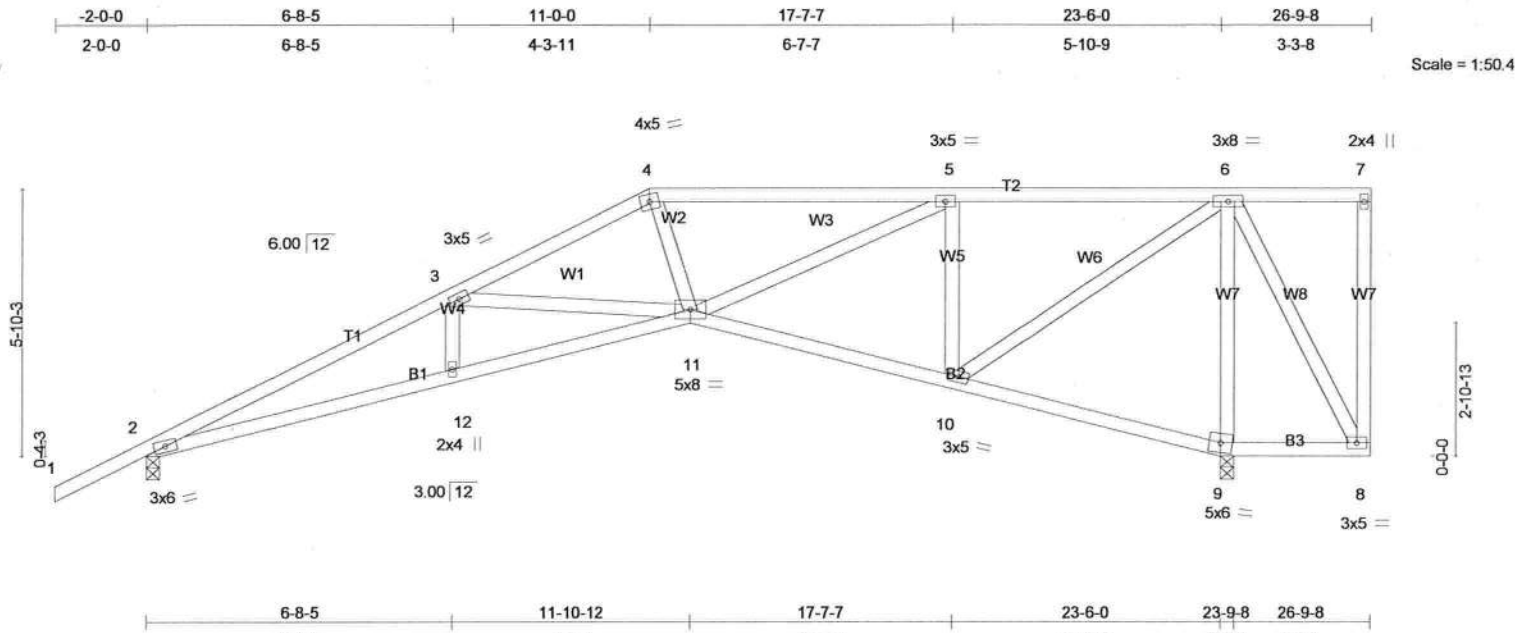
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:09:15 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-10-12 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-6-9 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=226(load case 6)
Max Uplift 2=-237(load case 6), 9=-284(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2229/1204, 3-4=-1600/921, 4-5=-1523/937, 5-6=-746/435, 6-7=-2/3, 7-8=-92/58
BOT CHORD	2-12=-1289/1967, 11-12=-1290/1966, 10-11=-453/776, 9-10=-105/59, 8-9=-69/37
WEBS	3-12=0/185, 3-11=-517/378, 4-11=-207/426, 5-11=-553/856, 5-10=-722/491, 6-10=-588/1015, 6-9=-966/593, 6-8=-74/139

JOINT STRESS INDEX

2 = 0.72, 3 = 0.48, 4 = 0.79, 5 = 0.56, 6 = 0.97, 7 = 0.34, 8 = 0.46, 9 = 0.45, 10 = 0.66, 11 = 0.61 and 12 = 0.34

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
	T08	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2 and 284 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

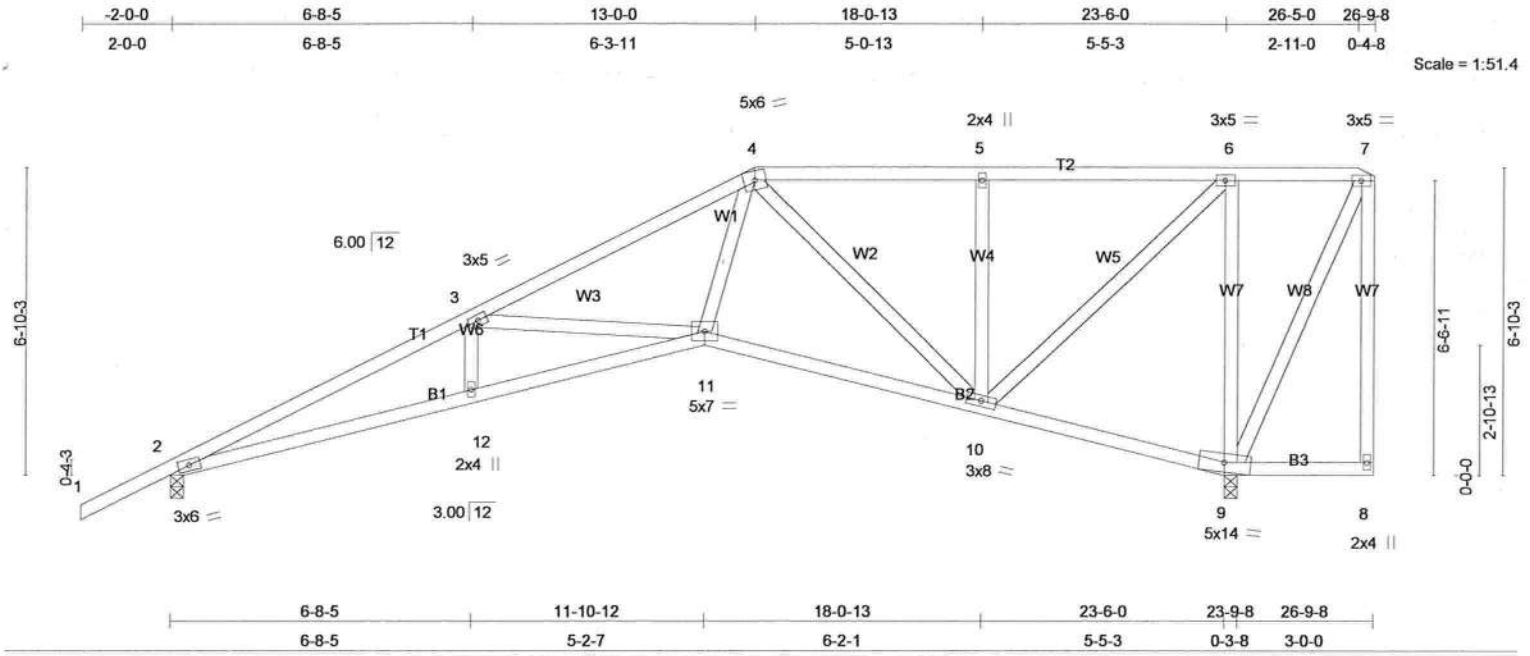
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 7=-54(F) 8=-30(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1105 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973312
	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:11:11 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-11-2 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-4-1 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=258(load case 6)
Max Uplift 2=-240(load case 6), 9=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2257/1244, 3-4=-1533/879, 4-5=-537/327, 5-6=-537/327, 6-7=-33/65, 7-8=-9/50
BOT CHORD	2-12=-1383/1997, 11-12=-1385/1997, 10-11=-720/1079, 9-10=-92/52, 8-9=-0/1
WEBS	3-12=0/180, 3-11=-633/487, 4-11=-550/899, 4-10=-719/526, 5-10=-308/194, 6-10=-508/847, 6-9=-846/542, 7-9=-152/77

JOINT STRESS INDEX

2 = 0.73, 3 = 0.48, 4 = 0.57, 5 = 0.34, 6 = 0.59, 7 = 0.48, 8 = 0.34, 9 = 0.42, 10 = 0.84, 11 = 0.66 and 12 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

NOTES

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2 and 281 lb uplift at joint 9.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 8=-30(F) 7=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 38868
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
	T10	SPECIAL	1	1	J1973313 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:02 2008 Page 1

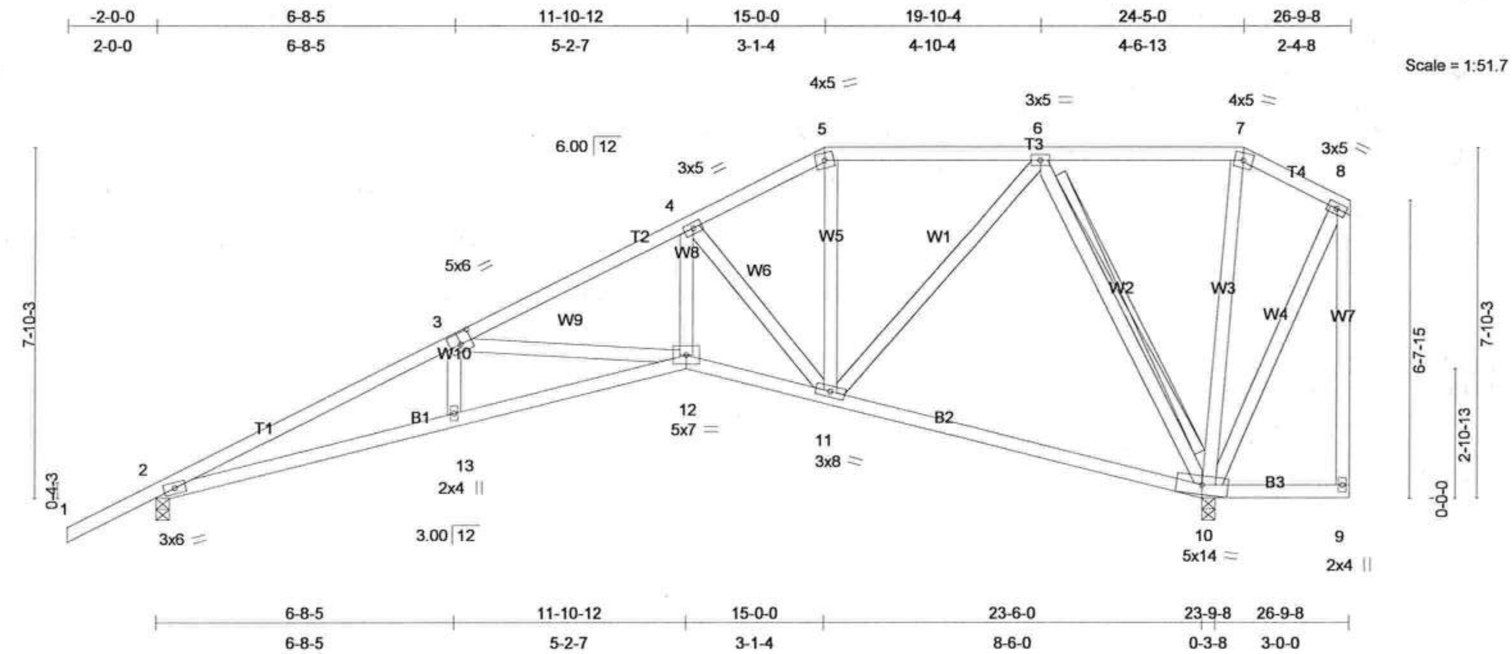


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		**SPACING**	2-0-0	**CSI**		**DEFL**	in	(loc)	l/defl	L/d	**PLATES**	**GRIP**
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.17	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=267(load case 6)
Max Uplift 2=-247(load case 6), 10=-221(load case 5)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2281/1240, 3-4=-1579/913, 4-5=-814/494, 5-6=-699/475, 6-7=-2/72, 7-8=-19/90, 8-9=-17/27
BOT CHORD 2-13=-1369/2017, 12-13=-1371/2014, 11-12=-922/1391, 10-11=-187/324, 9-10=-3/0
WEBS 3-13=0/198, 3-12=-604/436, 4-12=-582/903, 4-11=-1021/741, 5-11=-68/183, 6-11=-371/607, 6-10=-811/525, 7-10=-230/157, 8-10=-98/79

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.55, 4 = 0.79, 5 = 0.42, 6 = 0.46, 7 = 0.60, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.64, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 221 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-7=-54, 7-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973314
	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:49 2008 Page 1

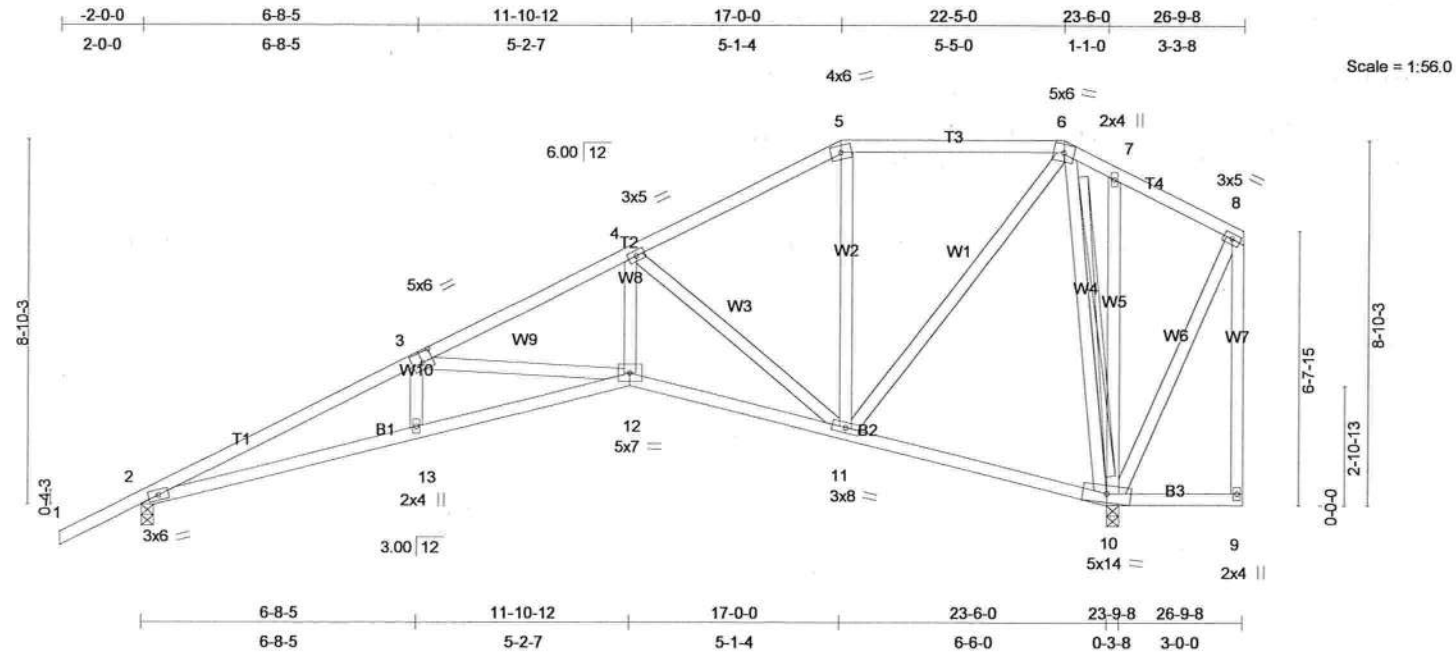


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.21	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.28	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=279(load case 6)
Max Uplift 2=-261(load case 6), 10=-272(load case 6)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2275/1247, 3-4=-1589/929, 4-5=-587/361, 5-6=-472/378, 6-7=0/94, 7-8=-16/110, 8-9=-13/39
BOT CHORD 2-13=-1375/2011, 12-13=-1377/2009, 11-12=-940/1408, 10-11=-35/66, 9-10=-3/2
WEBS 3-13=0/188, 3-12=-586/424, 4-12=-573/927, 4-11=-1160/830, 5-11=-76/75, 6-11=-429/690, 6-10=-779/449, 7-10=-143/119, 8-10=-145/105

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.52, 4 = 0.81, 5 = 0.60, 6 = 0.32, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.75, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973314
	T11	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 272 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:13:34 2008 Page 1

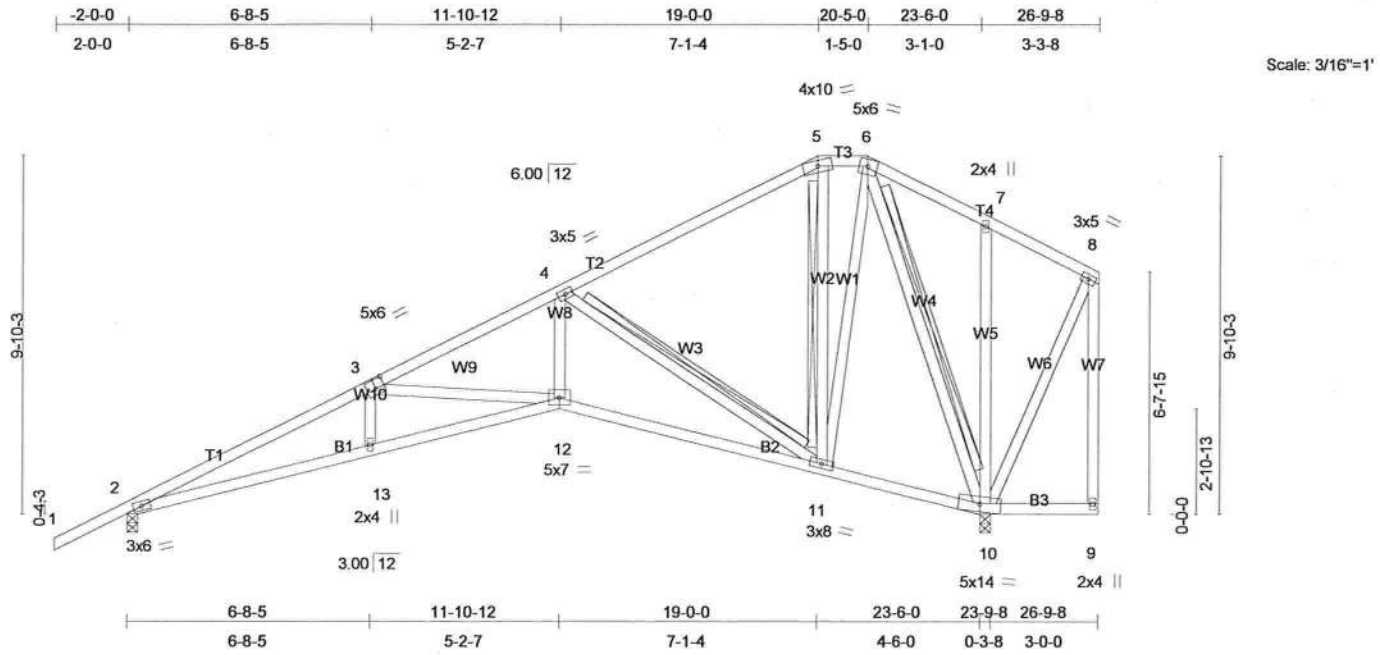


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.22	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.29	12-13	>967	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-10-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-4-2 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 4-11, 5-11, 6-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=850/0-3-8, 10=1046/0-3-8
Max Horz 2=291(load case 6)
Max Uplift 2=-253(load case 6), 10=-239(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2260/1266, 3-4=-1611/970, 4-5=-418/274, 5-6=-295/322, 6-7=0/102, 7-8=-13/109, 8-9=-8/56
BOT CHORD	2-13=-1391/1996, 12-13=-1392/1996, 11-12=-987/1440, 10-11=-95/199, 9-10=-2/2
WEBS	3-13=0/174, 3-12=-543/395, 4-12=-575/942, 4-11=-1315/949, 5-11=-236/201, 6-11=-481/742, 6-10=-762/403, 7-10=-203/204, 8-10=-145/94

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1165 Cresthill Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.73, 3 = 0.47, 4 = 0.82, 5 = 0.67, 6 = 0.43, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.93, 12 = 0.70 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
	T12	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2 and 239 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

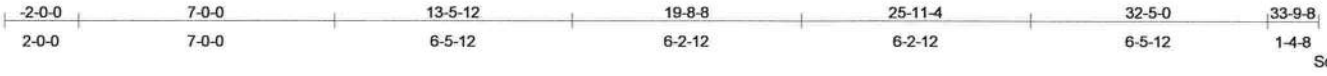
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

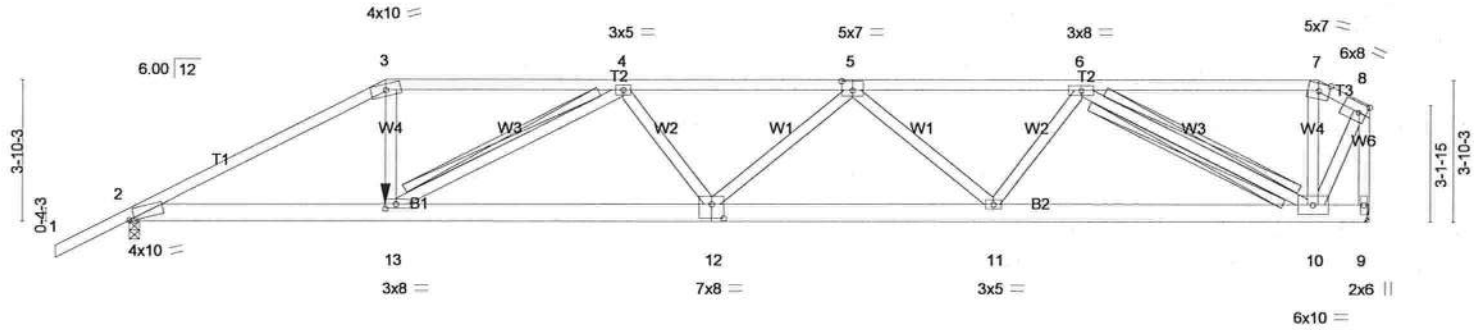
June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973316
	T13	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:01 2008 Page 1



Scale = 1:62.7



Simpson HTU26

Plate Offsets (X,Y): [2:0-1-11,0-0-6], [5:0-3-8,0-3-0], [12:0-4-0,0-4-8], [13:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.36	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.68	12-13	>589	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS I-Brace: 2 X 4 SYP No.3 - 6-10
T-Brace: 2 X 4 SYP No.3 - 4-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2313/0-3-8, 9=2363/Mechanical
Max Horz 2=122(load case 5)
Max Uplift 2=-716(load case 5), 9=-787(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4517/1485, 3-4=-4027/1378, 4-5=-5721/1945, 5-6=-4785/1629, 6-7=-1016/363, 7-8=-1050/329, 8-9=-2367/718
BOT CHORD 2-13=-1324/3966, 12-13=-1954/5639, 11-12=-1955/5649, 10-11=-1442/4130, 9-10=-9/32
WEBS 3-13=-418/1454, 4-13=-1930/748, 4-12=0/271, 5-12=0/182, 5-11=-1166/489, 6-11=-265/1154, 6-10=-3553/1277, 7-10=-209/197, 8-10=-738/2278

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

JOINT STRESS INDEX

2 = 0.81, 3 = 0.98, 4 = 0.62, 5 = 0.81, 6 = 0.84, 7 = 0.85, 8 = 0.86, 9 = 0.61, 10 = 0.92, 11 = 0.82, 12 = 0.90 and 13 = 0.91

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 716 lb uplift at joint 2 and 787 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

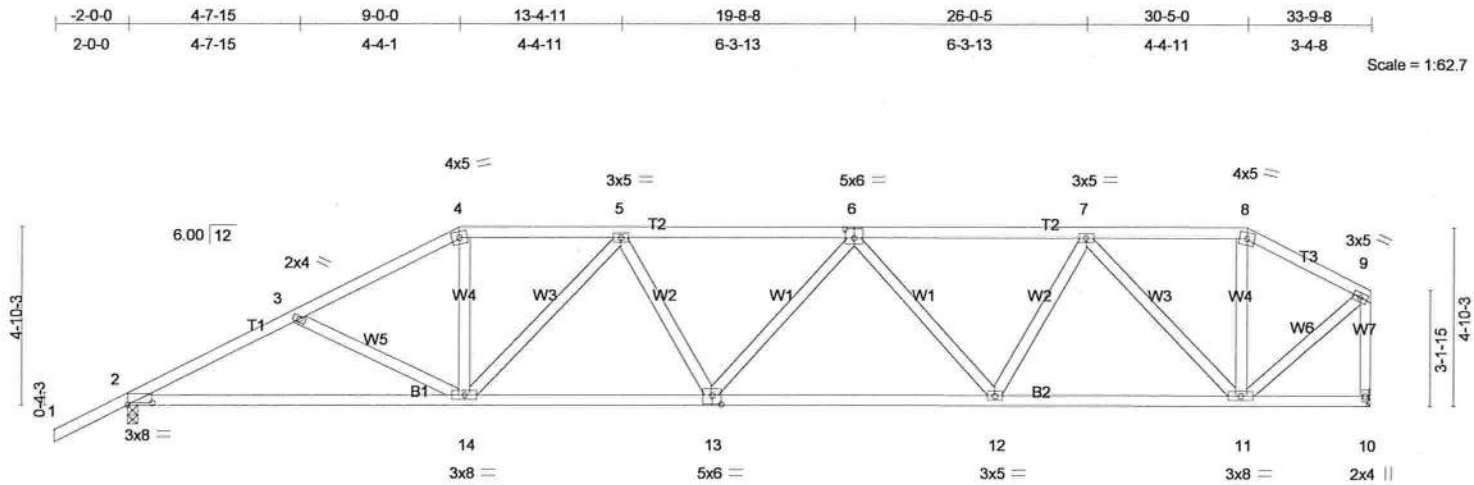
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 7-8=-118(F=-64), 2-13=-10, 9-13=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 13=-411(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:02 2008 Page 1



9-0-0 15-10-1 23-6-15 30-5-0 33-9-8
9-0-0 6-10-1 7-8-14 6-10-1 3-4-8

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [6:0-3-0,0-3-0], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.31	2-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.09	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 2=1192/0-3-8, 10=1068/Mechanical
Max Horz 2=162(load case 6)
Max Uplift 2=-277(load case 6), 10=-241(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2014/1040, 3-4=-1776/933, 4-5=-1558/894, 5-6=-1933/1073, 6-7=-1637/909, 7-8=-727/447, 8-9=-843/455, 9-10=-1051/566
BOT CHORD 2-14=-990/1734, 13-14=-992/1893, 12-13=-1019/1938, 11-12=-728/1405, 10-11=-15/17
WEBS 3-14=-216/207, 4-14=-227/535, 5-14=-566/265, 5-13=-17/150, 6-13=-64/94, 6-12=-473/299, 7-12=-205/497, 7-11=-1009/534, 8-11=-52/208, 9-11=-460/932

JOINT STRESS INDEX
2 = 0.70, 3 = 0.33, 4 = 0.63, 5 = 0.43, 6 = 0.50, 7 = 0.43, 8 = 0.38, 9 = 0.66, 10 = 0.37, 11 = 0.89, 12 = 0.43, 13 = 0.56 and 14 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1163 Crandal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
	T14	HIP	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2 and 241 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1102 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973318
	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:03 2008 Page 1

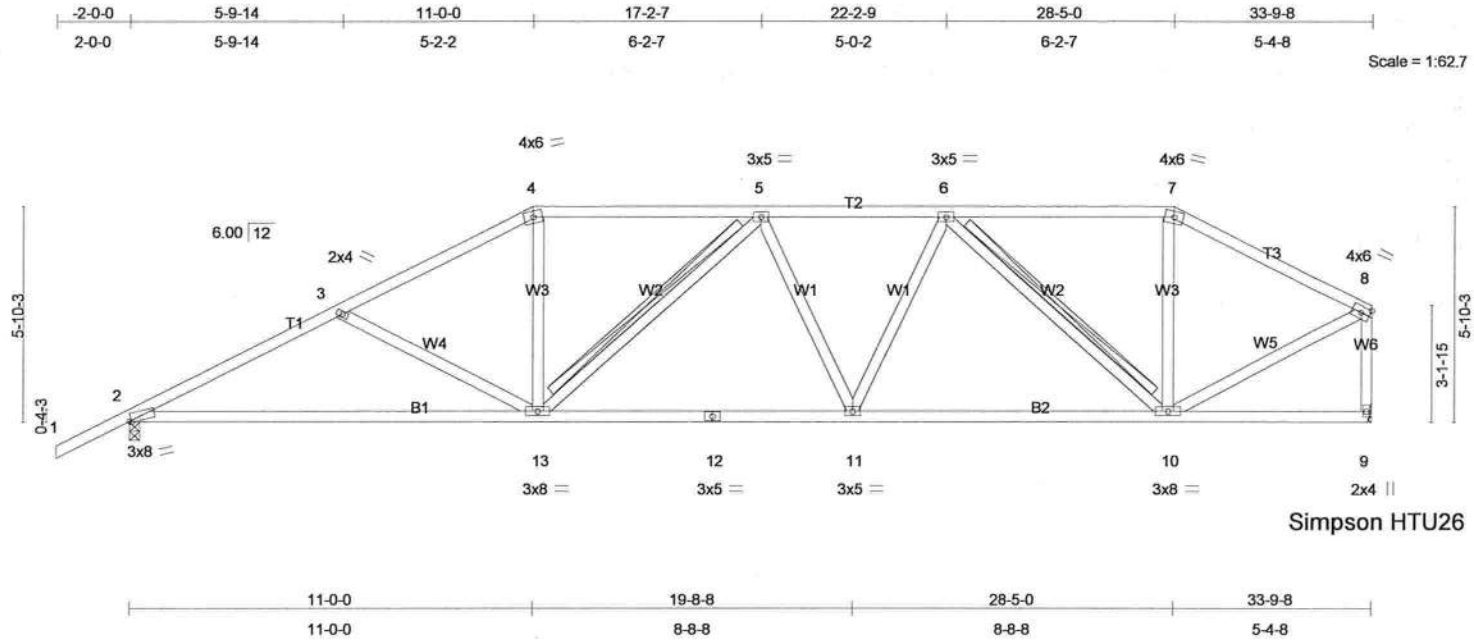


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.56	2-13	>713	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-14 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1192/0-3-8, 9=1068/Mechanical
Max Horz 2=173(load case 6)
Max Uplift 2=-292(load case 6), 9=-210(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1980/1046, 3-4=-1675/905, 4-5=-1453/876, 5-6=-1564/919, 6-7=-903/579, 7-8=-1061/577, 8-9=-1044/580
BOT CHORD 2-13=-985/1700, 12-13=-839/1627, 11-12=-839/1627, 10-11=-752/1465, 9-10=-34/36
WEBS 3-13=-291/278, 4-13=-152/439, 5-13=-350/150, 5-11=-166/131, 6-11=-80/278, 6-10=-803/416, 7-10=-11/229, 8-10=-469/983

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.88, 3 = 0.33, 4 = 0.74, 5 = 0.45, 6 = 0.45, 7 = 0.71, 8 = 0.68, 9 = 0.43, 10 = 0.90, 11 = 0.45, 12 = 0.57 and 13 = 0.56

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973318
	T15	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 210 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1103 Cnental Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:04 2008 Page 1

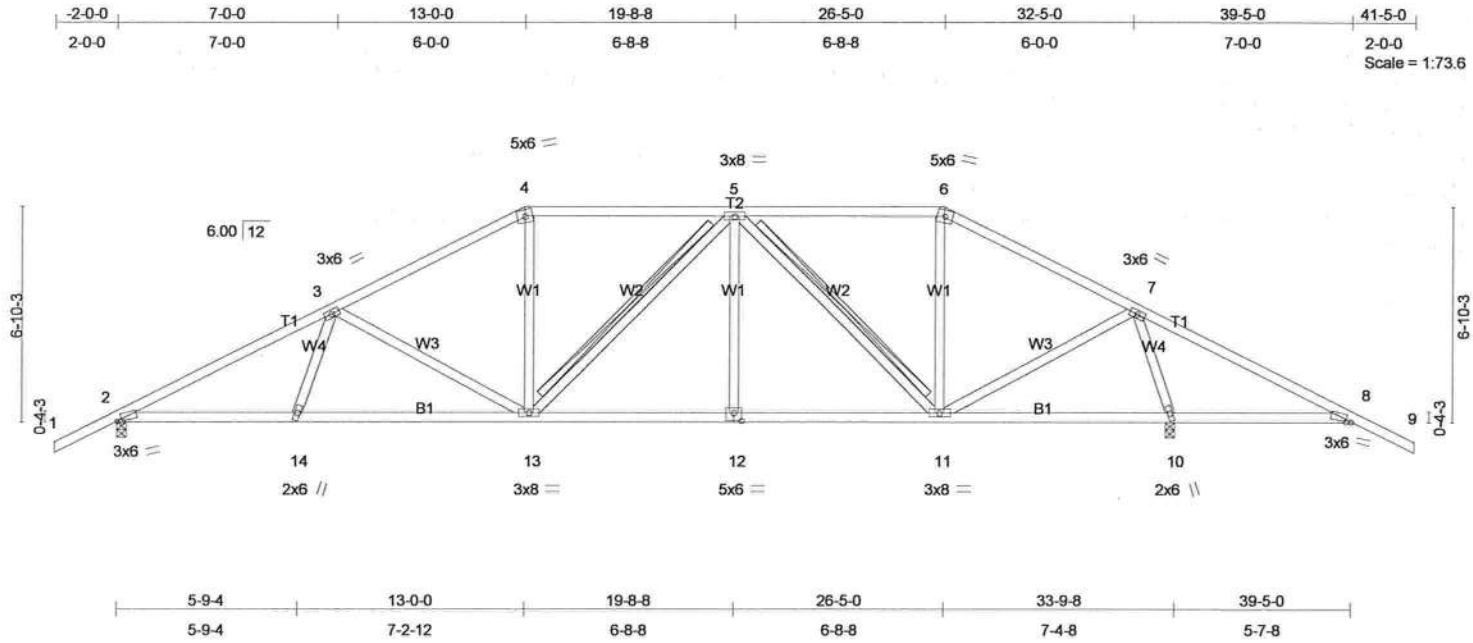


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [8:0-1-13,0-0-7], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1140/0-3-8, 10=1597/0-3-8
Max Horz 2=-113(load case 7)
Max Uplift 2=-303(load case 6), 10=-535(load case 7)
Max Grav 2=1144(load case 10), 10=1597(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1900/878, 3-4=-1440/745, 4-5=-1227/734, 5-6=-769/409, 6-7=-927/389, 7-8=-846/733, 8-9=0/47
BOT CHORD 2-14=-601/1616, 13-14=-615/1591, 12-13=-272/1220, 11-12=-272/1220, 10-11=-122/538, 8-10=-578/905
WEBS 3-14=0/221, 3-13=-422/336, 4-13=-68/340, 5-13=-134/95, 5-12=0/186, 5-11=-638/388, 6-11=-6/196, 7-11=-612/970, 7-10=-1579/1193

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.79, 3 = 0.72, 4 = 0.54, 5 = 0.56, 6 = 0.54, 7 = 0.72, 8 = 0.79, 10 = 0.54, 11 = 0.89, 12 = 0.44, 13 = 0.89 and 14 = 0.54

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
	T16	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 535 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:06 2008 Page 1

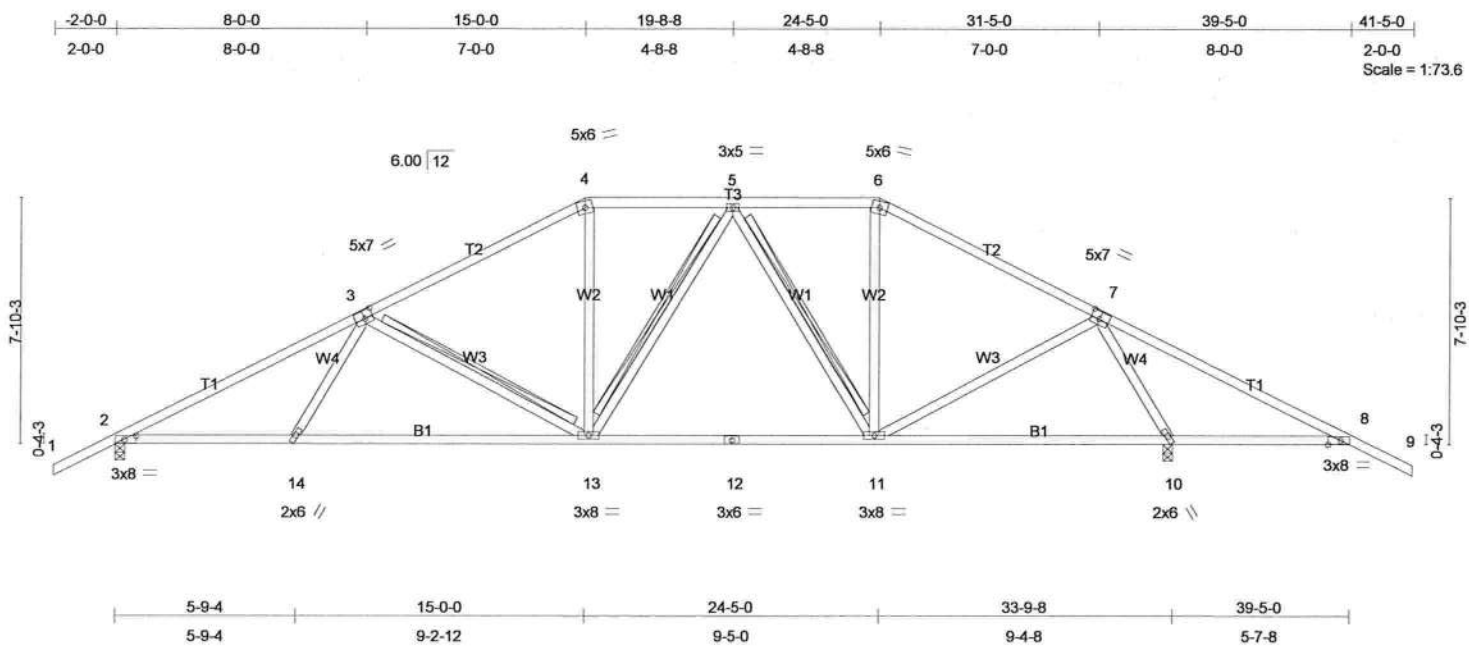


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-3-4,0-3-0], [7:0-3-4,0-3-0], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.16	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.33	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13, 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1142/0-3-8, 10=1595/0-3-8
Max Horz 2=-125(load case 7)
Max Uplift 2=-314(load case 6), 10=-549(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1887/872, 3-4=-1315/705, 4-5=-1099/703, 5-6=-823/505, 6-7=-1011/489, 7-8=-883/781, 8-9=0/47
BOT CHORD 2-14=-582/1595, 13-14=-606/1537, 12-13=-180/1023, 11-12=-180/1023, 10-11=-13/296, 8-10=-607/950
WEBS 3-14=0/291, 3-13=-511/421, 4-13=-54/307, 5-13=-117/156, 5-11=-433/266, 6-11=-11/221, 7-11=-388/666, 7-10=-1703/1317

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.70, 3 = 0.84, 4 = 0.54, 5 = 0.43, 6 = 0.54, 7 = 0.84, 8 = 0.70, 10 = 0.74, 11 = 0.61, 12 = 0.43, 13 = 0.61 and 14 = 0.74

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
	T17	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 549 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973321
	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:07 2008 Page 1

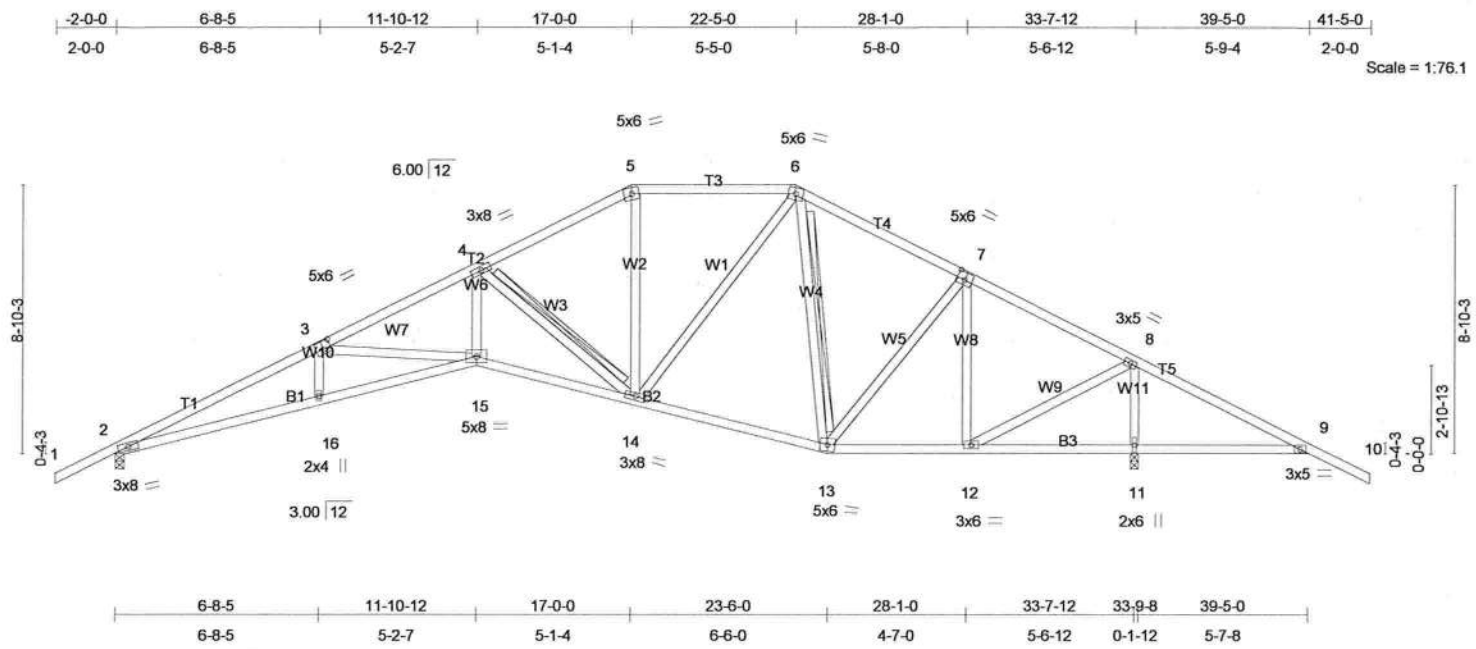


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.29	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.54	15-16	>743	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.29	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 221 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-136(load case 7)
Max Uplift 2=-350(load case 6), 11=-566(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3441/1524, 3-4=-2805/1216, 4-5=-1362/711, 5-6=-1172/693, 6-7=-959/544, 7-8=-774/276, 8-9=-807/679, 9-10=0/47
BOT CHORD 2-16=-1211/3075, 15-16=-1213/3074, 14-15=-787/2529, 13-14=-117/868, 12-13=-83/631, 11-12=-540/858, 9-11=-540/858
WEBS 3-16=0/184, 3-15=-537/413, 4-15=-495/1451, 4-14=-1676/759, 5-14=-105/336, 6-14=-213/590, 6-13=-347/156, 7-13=-150/259, 7-12=-581/474, 8-12=-899/1318, 8-11=-1520/1150

Julius Lee
Truss Design Engineer
Florida FIE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973321
	T18	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.79, 3 = 0.53, 4 = 0.83, 5 = 0.43, 6 = 0.43, 7 = 0.45, 8 = 0.79, 9 = 0.54, 11 = 0.54, 12 = 0.74, 13 = 0.42, 14 = 0.85, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 2 and 566 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34863
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973322
	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:08 2008 Page 1

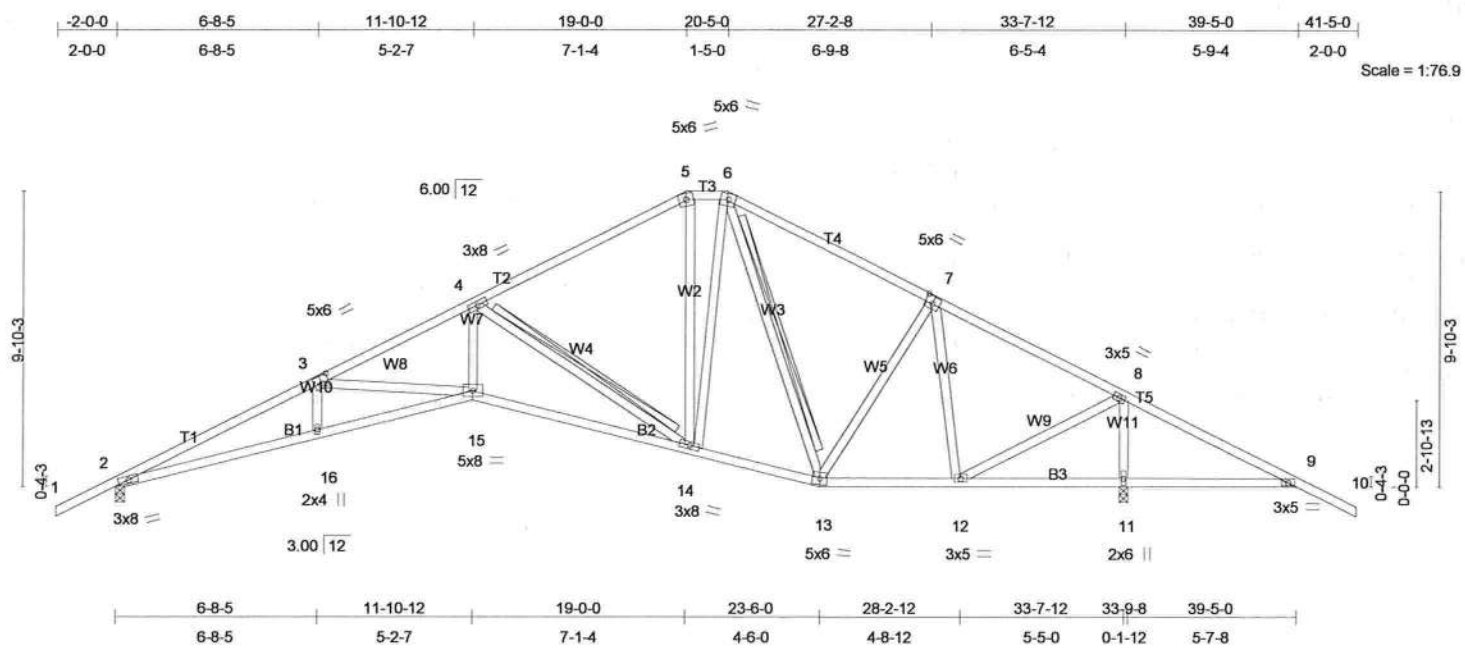


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.31	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.56	15-16	>712	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.32	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-148(load case 7)
Max Uplift 2=-331(load case 6), 11=-577(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3431/1543, 3-4=-2829/1255, 4-5=-1136/633, 5-6=-945/648, 6-7=-949/581, 7-8=-768/281, 8-9=-803/675, 9-10=0/47
BOT CHORD 2-16=-1227/3064, 15-16=-1227/3064, 14-15=-832/2561, 13-14=-82/904, 12-13=-73/693, 11-12=-535/854, 9-11=-535/854
WEBS 3-16=0/171, 3-15=-502/386, 4-15=-496/1468, 4-14=-1856/869, 5-14=-75/281, 6-14=-189/625, 6-13=-320/85, 7-13=-71/165, 7-12=-572/479, 8-12=-892/1306, 8-11=-1522/1159

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1163 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

JOINT STRESS INDEX

2 = 0.79, 3 = 0.49, 4 = 0.84, 5 = 0.66, 6 = 0.58, 7 = 0.65, 8 = 0.79, 9 = 0.56, 11 = 0.55, 12 = 0.81, 13 = 0.28, 14 = 0.92, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2 and 577 lb uplift at joint 11.

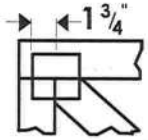
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

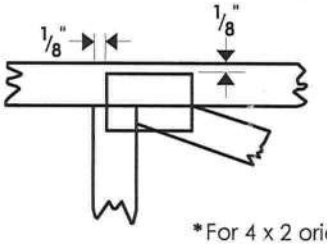
June 16,2008

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

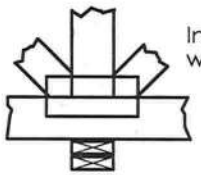
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

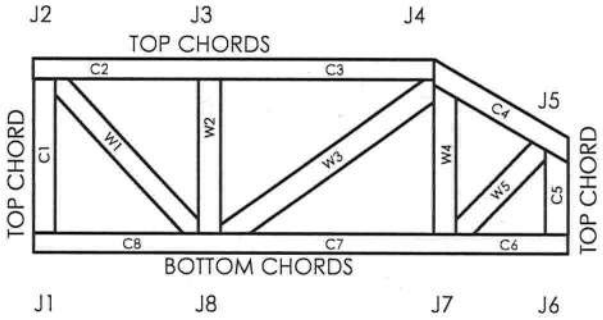
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 8"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 8"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 8"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 8"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

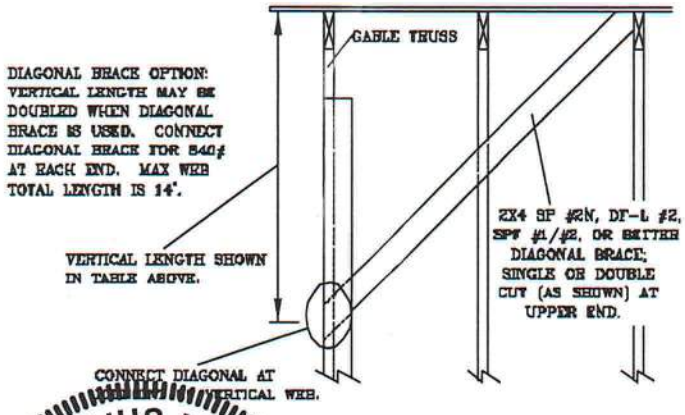
SOUTHERN PINE
#3
STUD
STANDARD

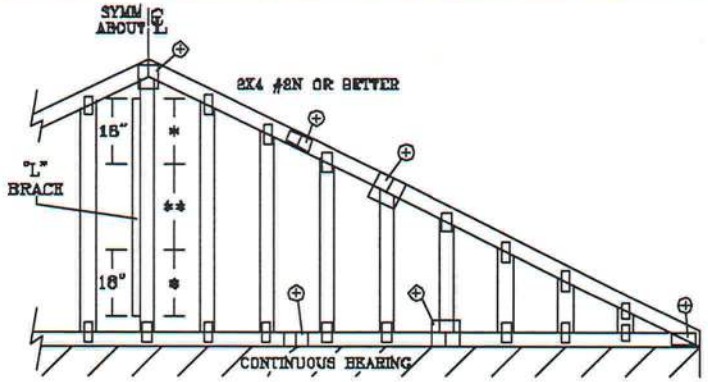
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

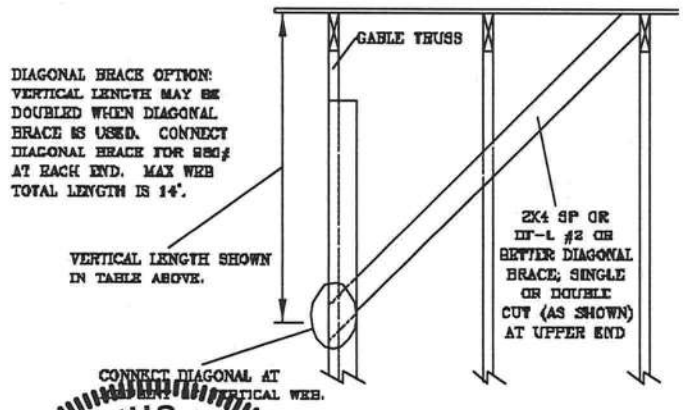
MAX. SPACING 24.0"

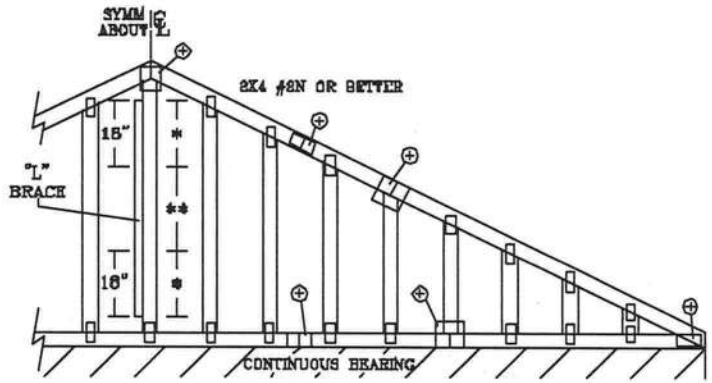
REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

WARNING!!! TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

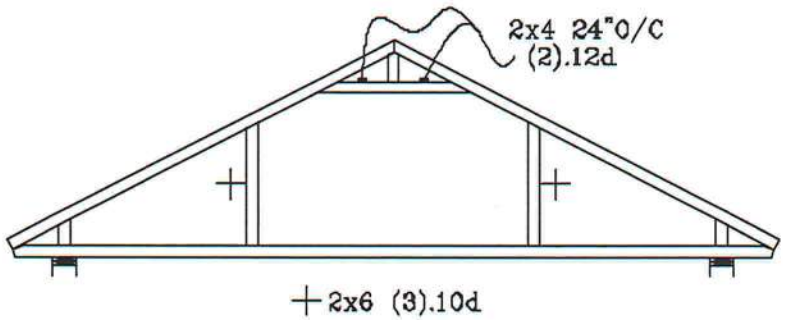
JULIUS LEE'S CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

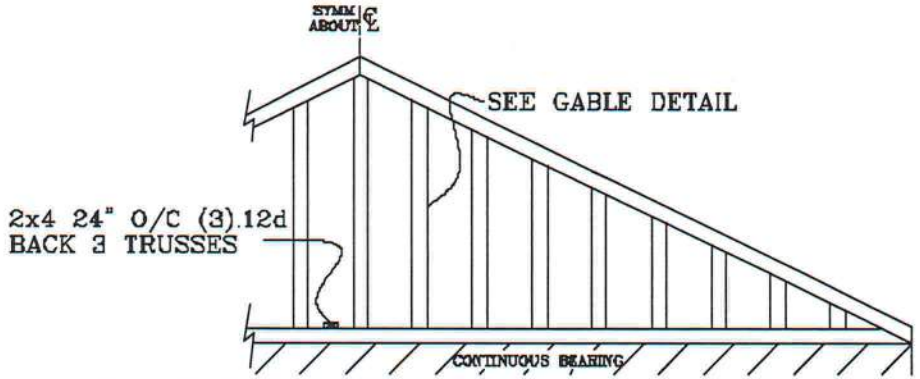
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 30' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

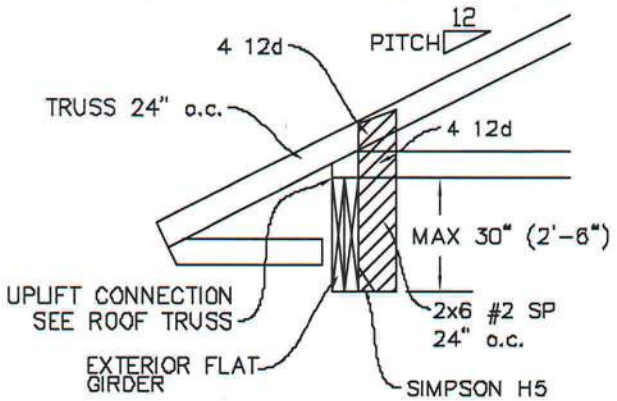


GABLE END TRUSS DETAIL

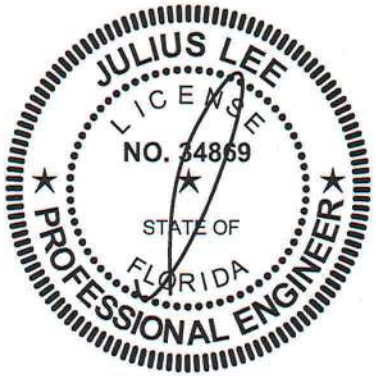


MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

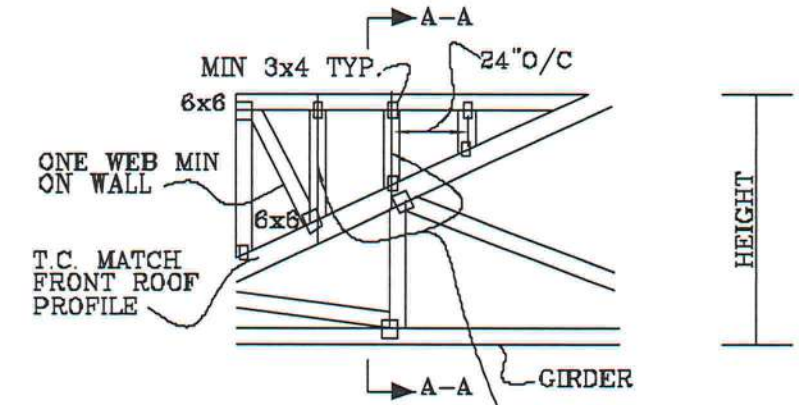
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

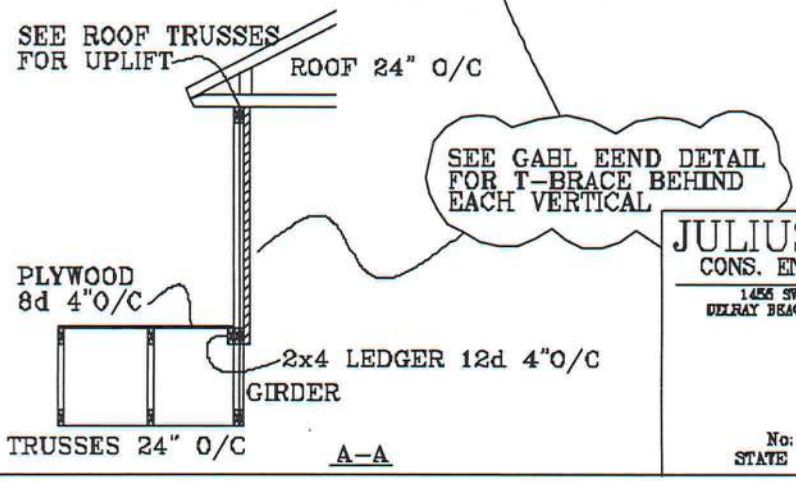


REVIEWED
By julius lee at 11:59 am, Jun 11, 2008



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

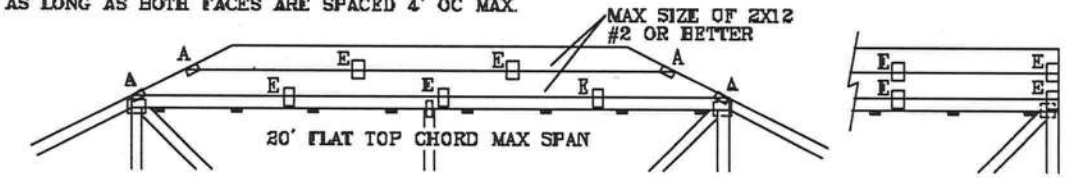
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

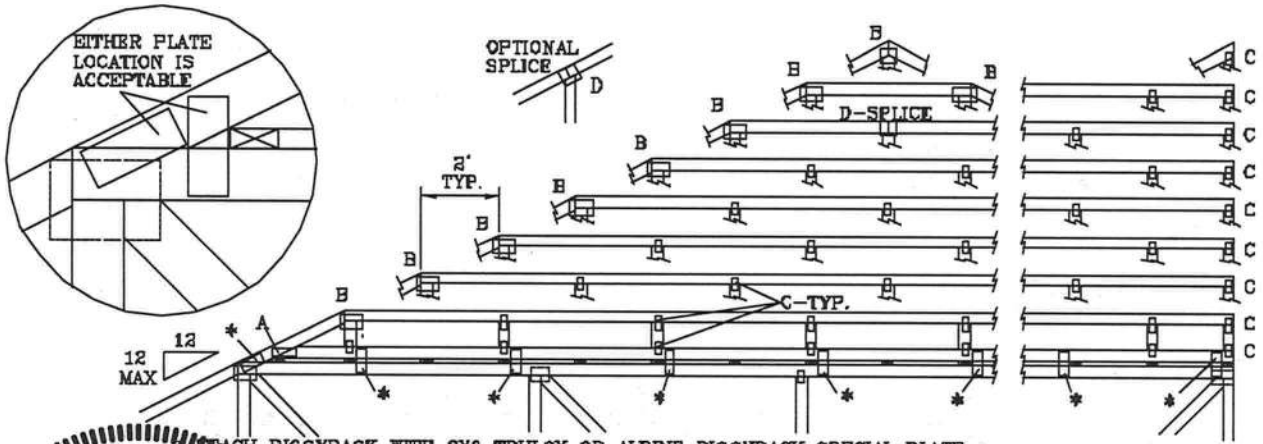
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

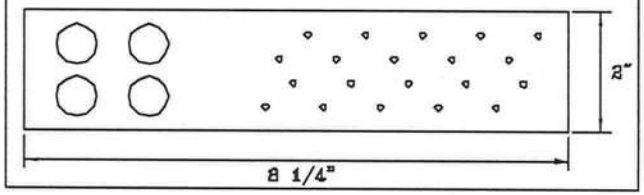
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1460 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

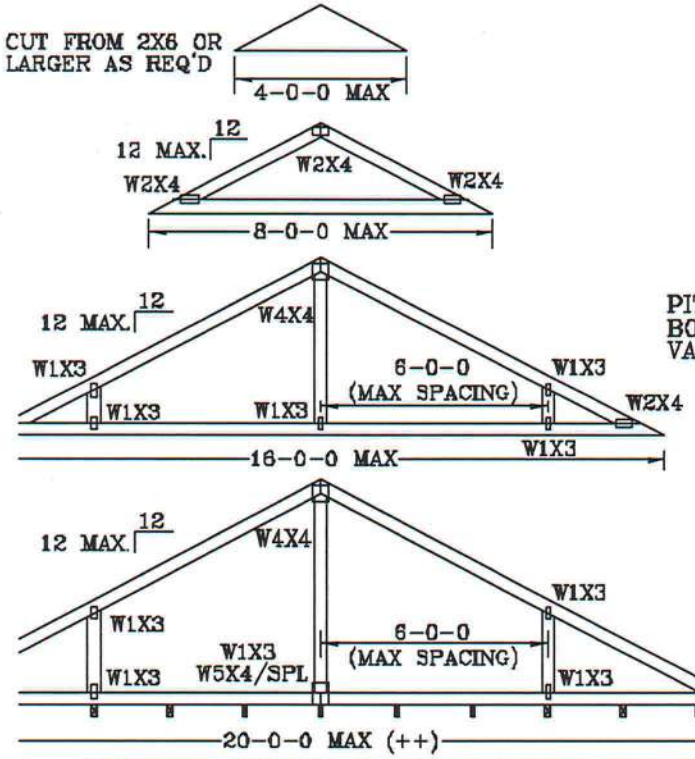
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

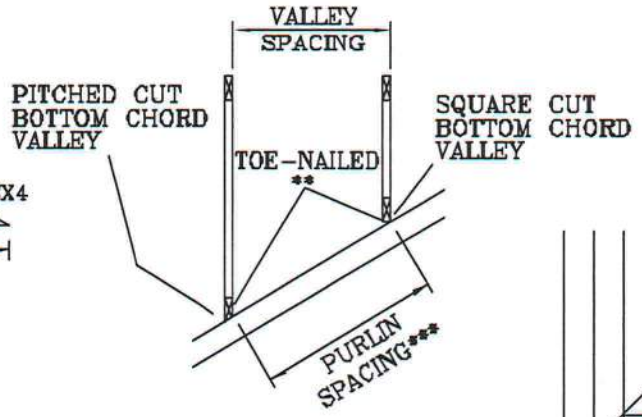
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

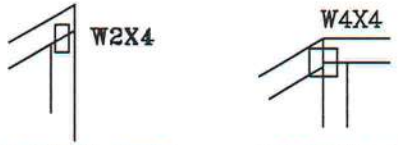
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

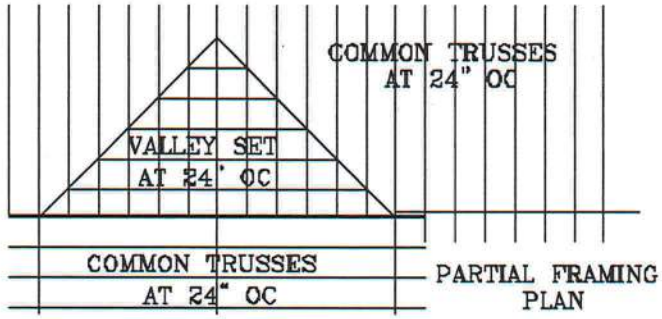
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.









SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

«WARNING» TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC. 1.25		1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

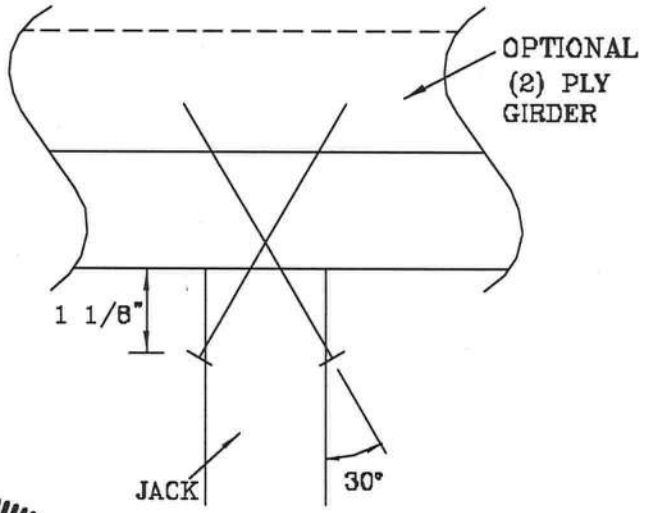
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

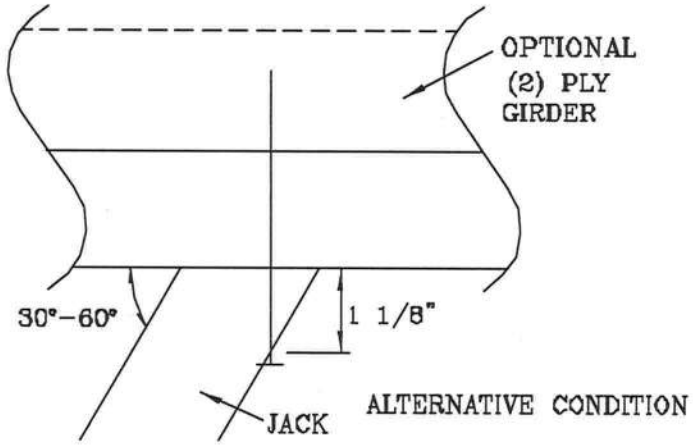
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

"WARNING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

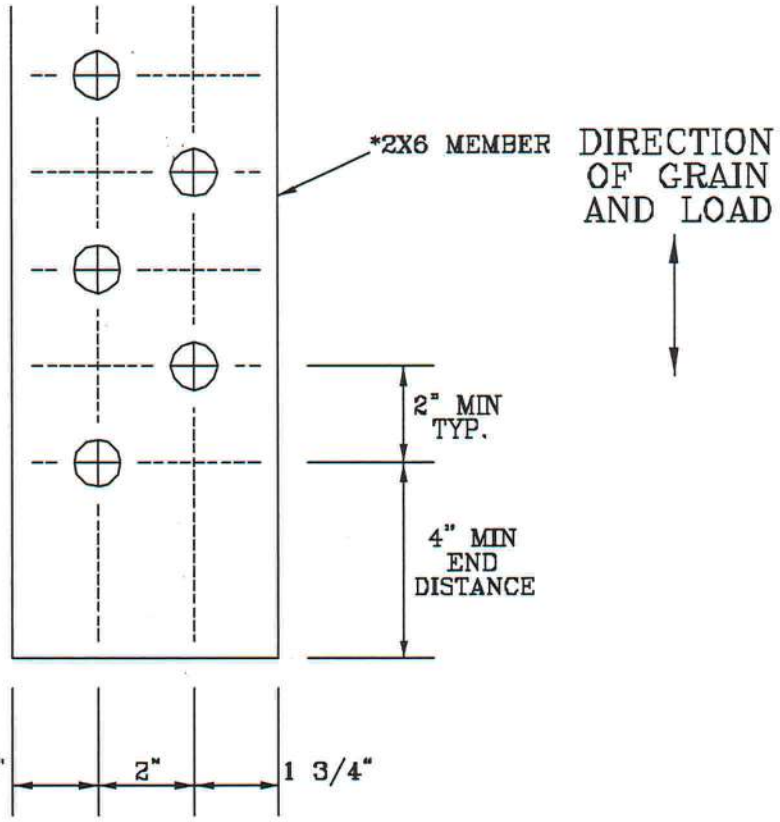
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

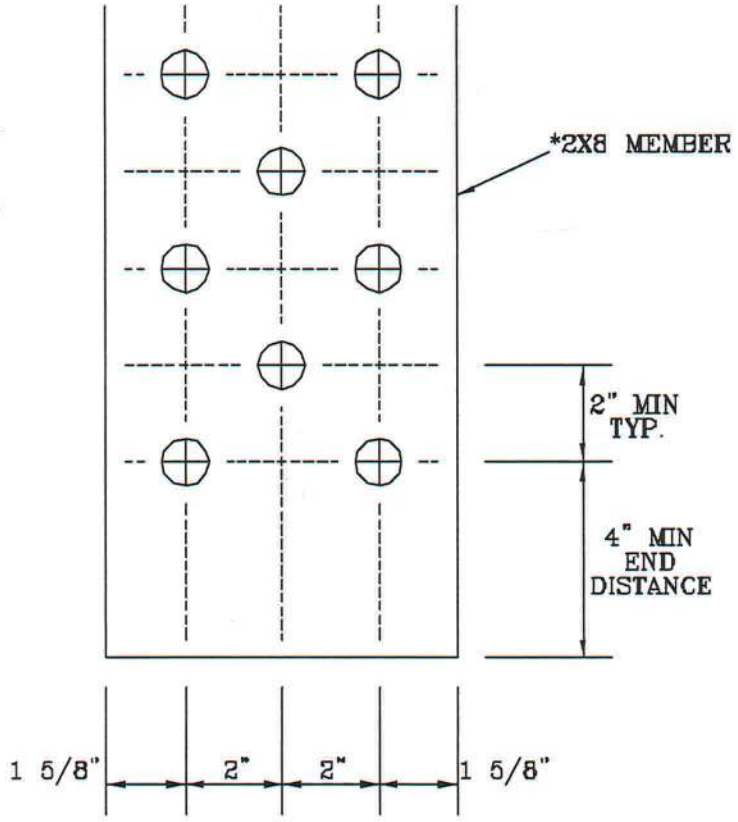
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

TRULOX CONNECTION DETAIL

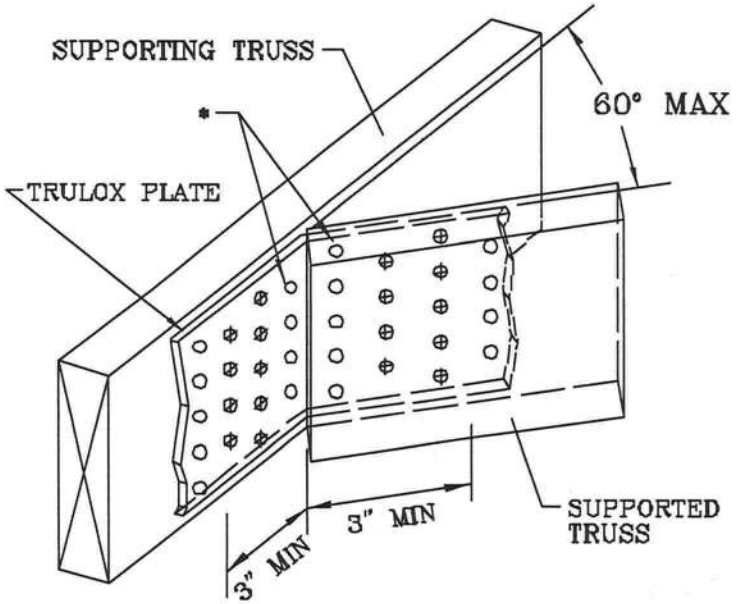
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

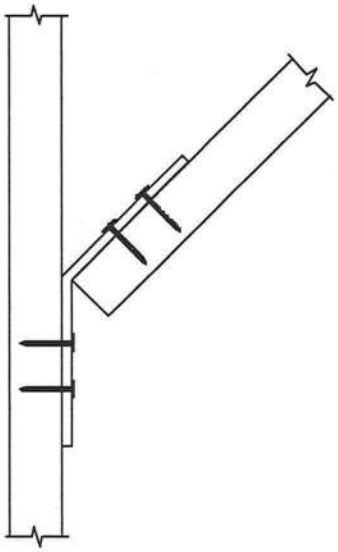
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

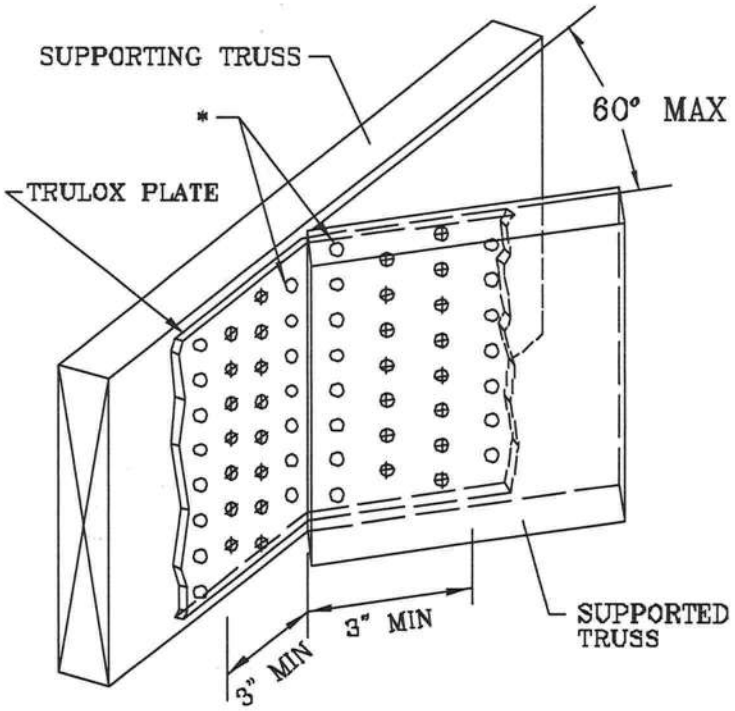
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

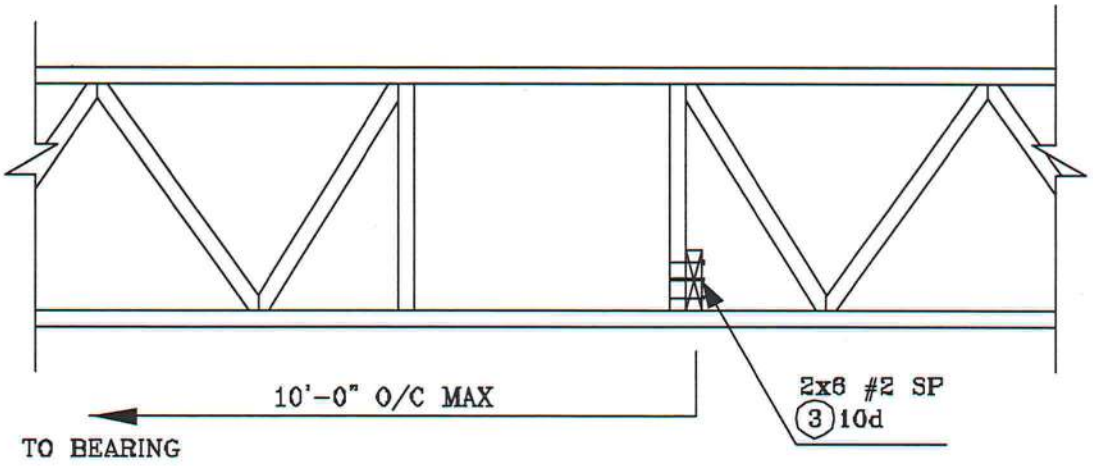
"WARNING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

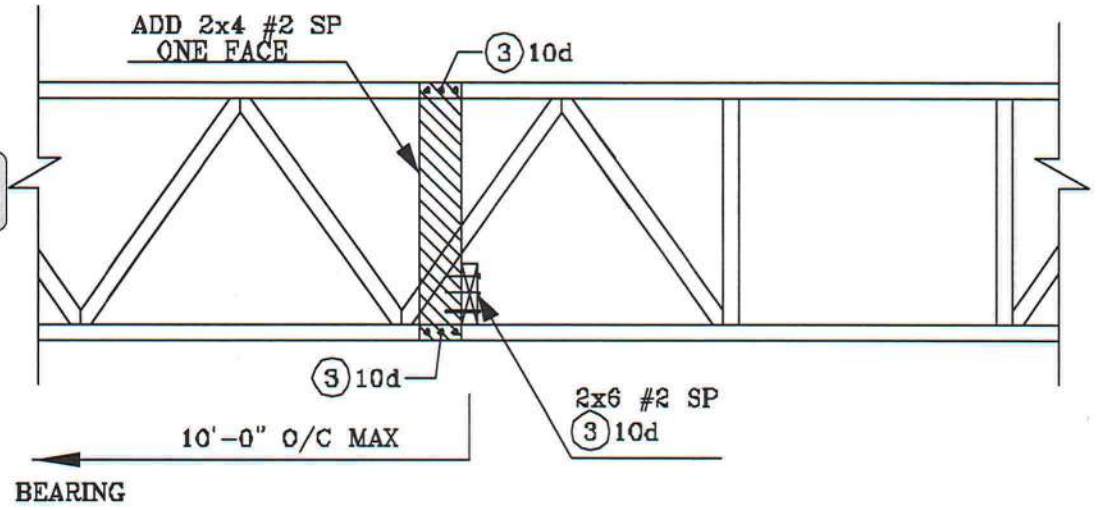
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

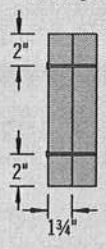
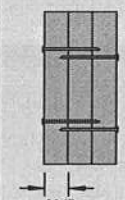
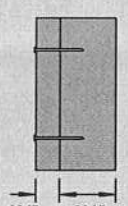
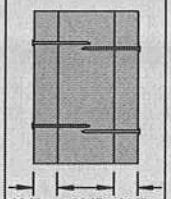
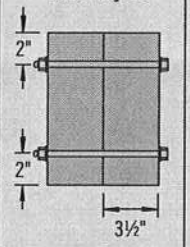
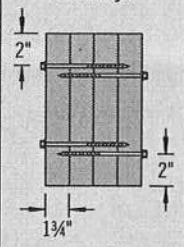
JULIUS LEE
LICENSE
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅜" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9/16" maximum.

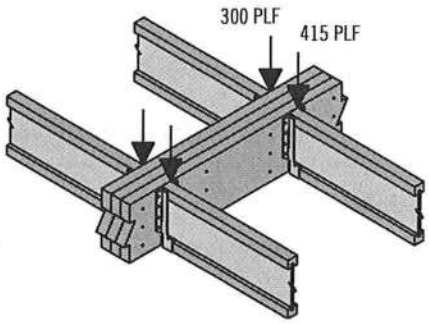
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

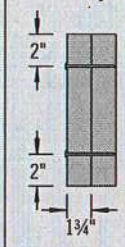
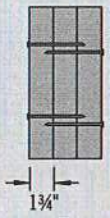
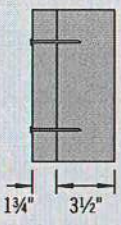
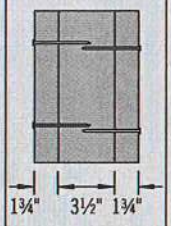
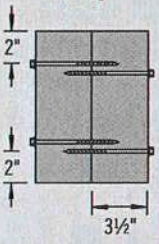
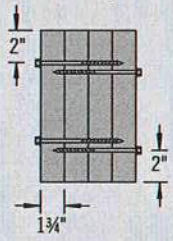
Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern: Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

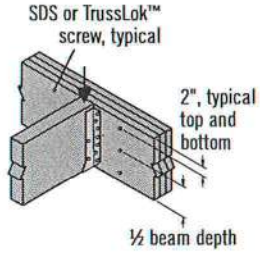
(2) 6" long screws required.

(3) 5" long screws required.

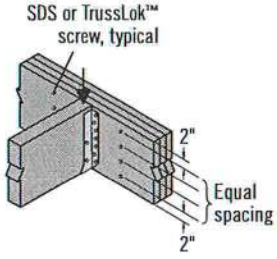
(4) 3½" and 3⅜" long screws must be installed on both sides.

Connections

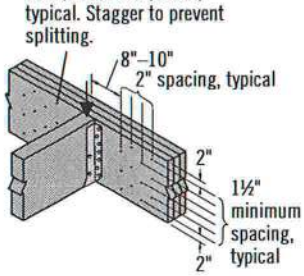
4 or 6 or Screw Connection



8 Screw Connection

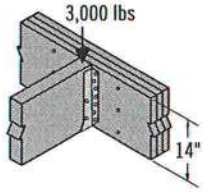


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

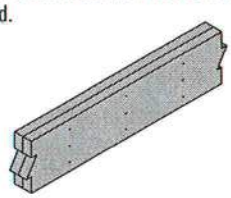
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



L6

Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"