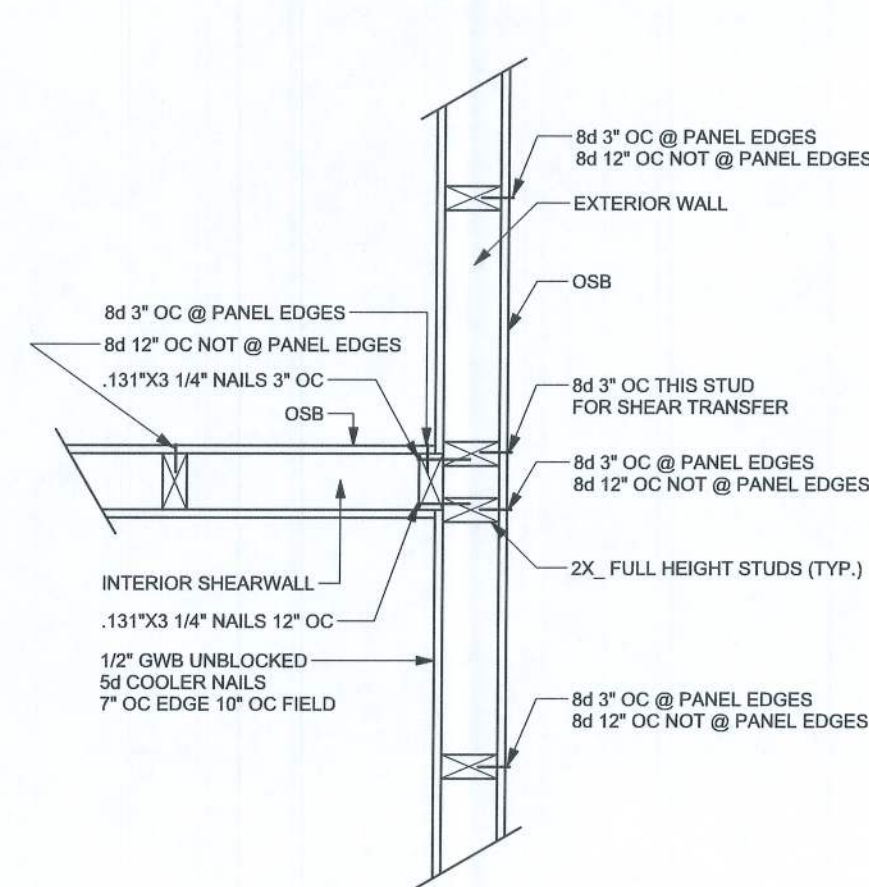
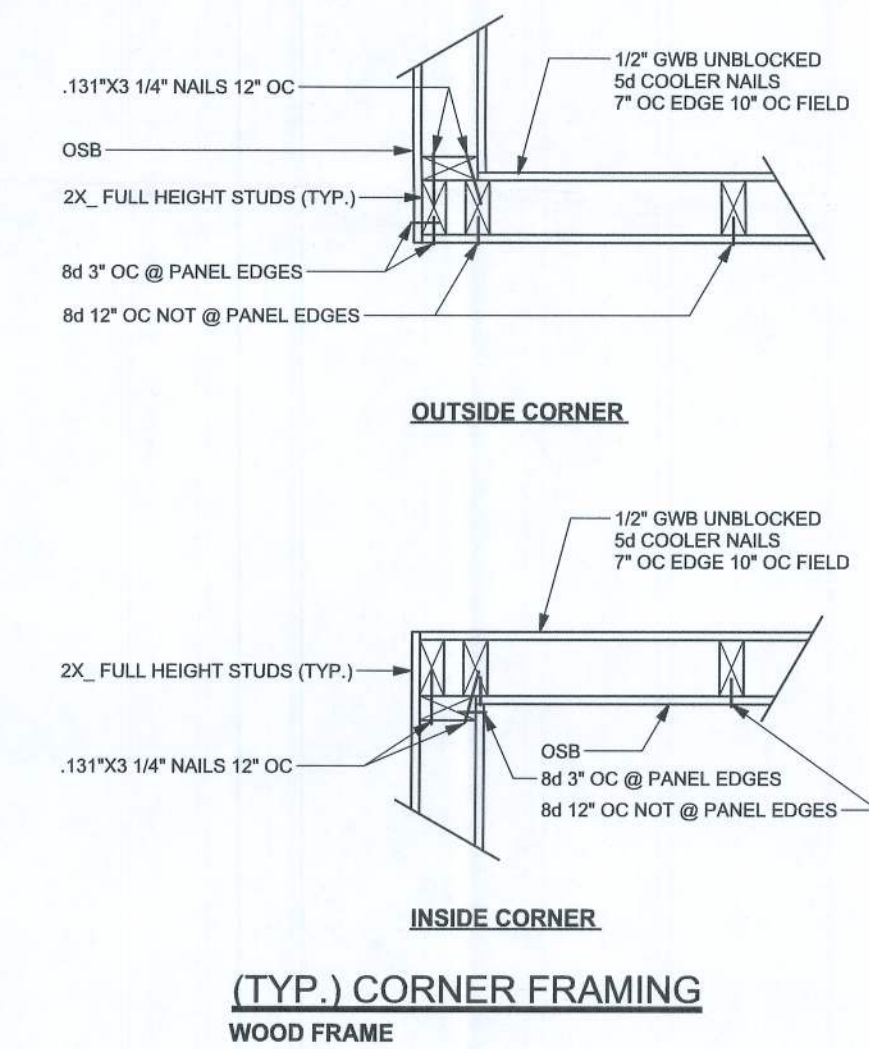
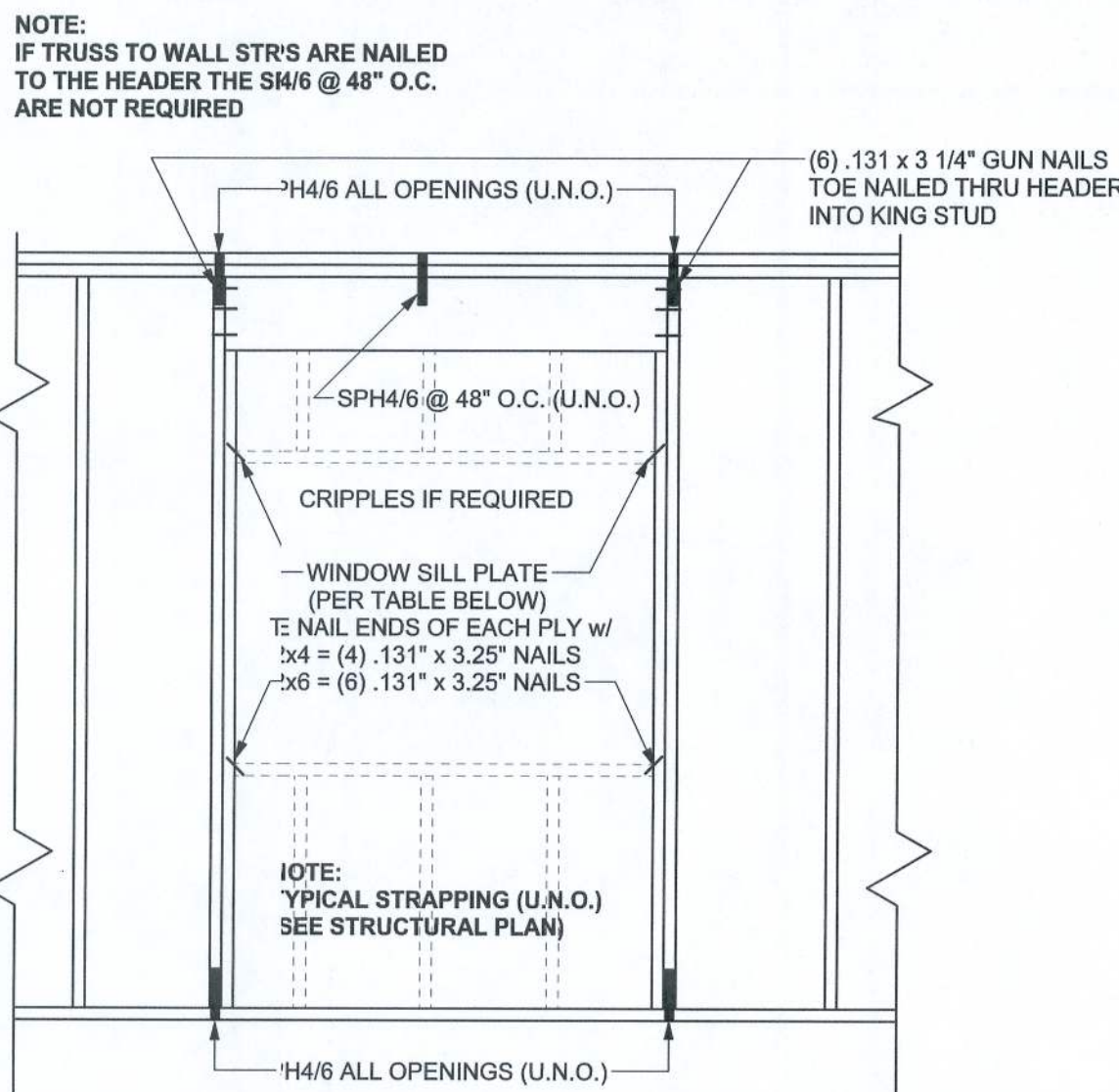


ONE STORY WALL SECTION
SCALE: 3/4\"/>



(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

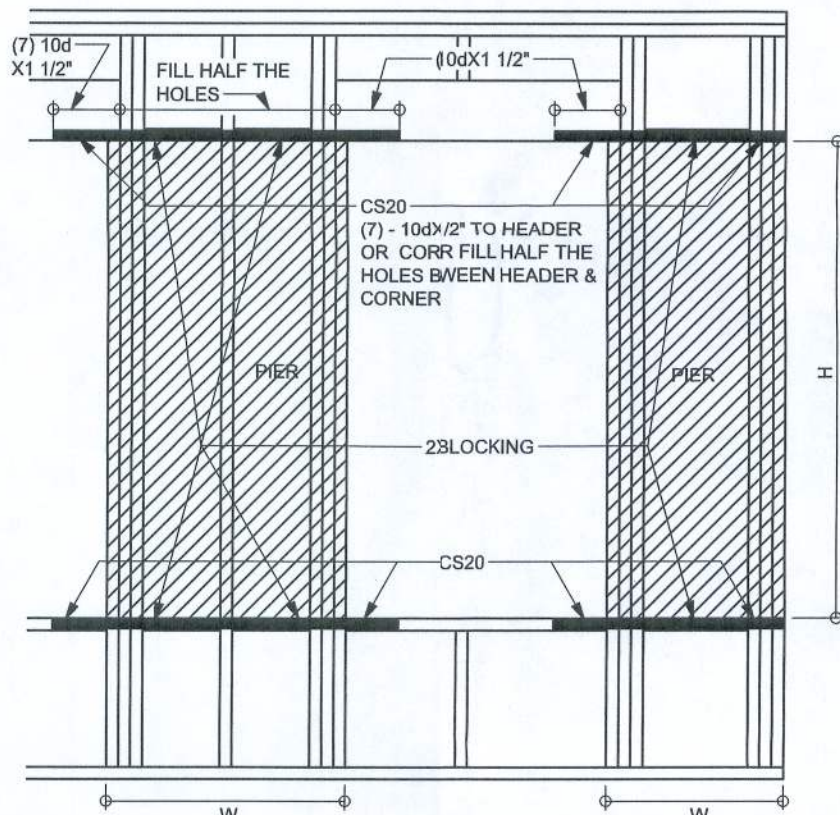


SILL PILE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	114	124	134	144
90-100 MPH	2	3	4	5
110-120 MPH	4	5	6	7
130 MPH	6	7	8	9

TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2\"/>

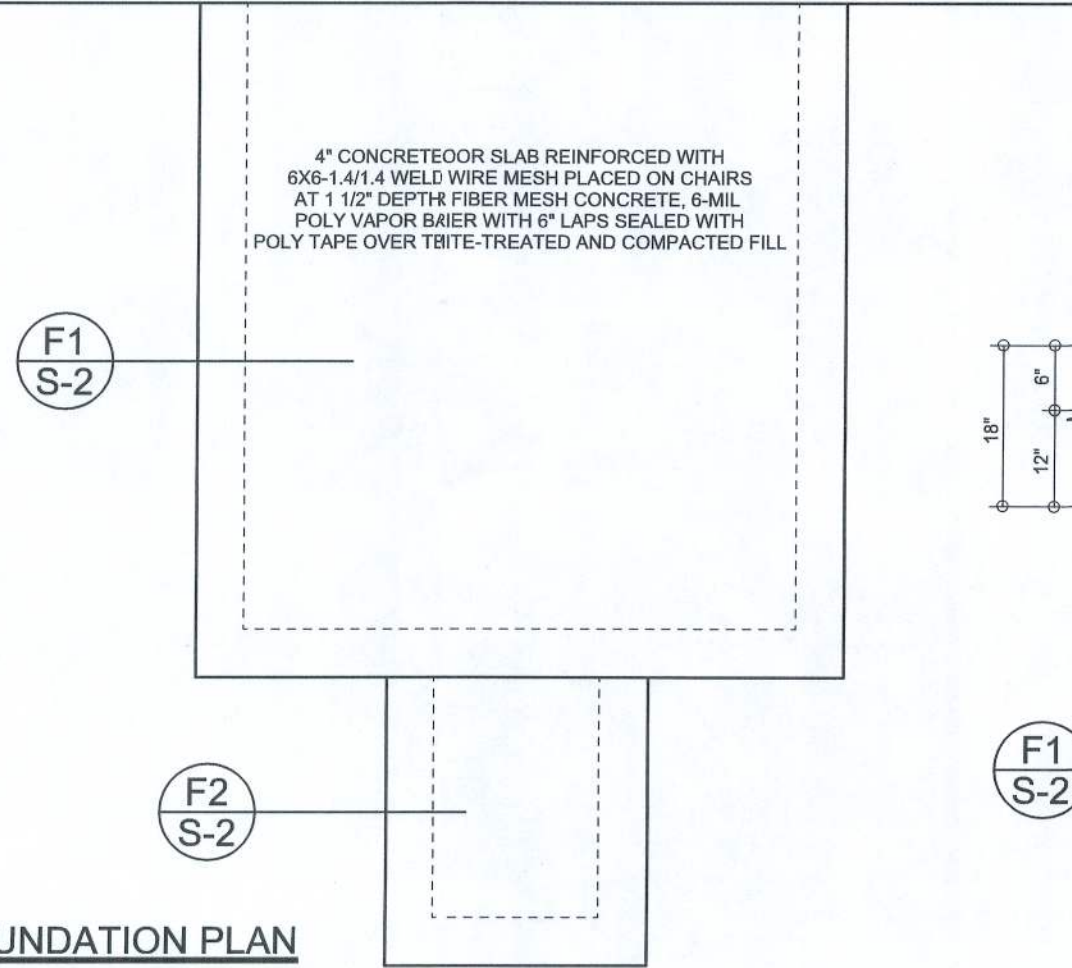
TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	10.0'	10.0'
LONGITUDINAL	8.0'	12.0'

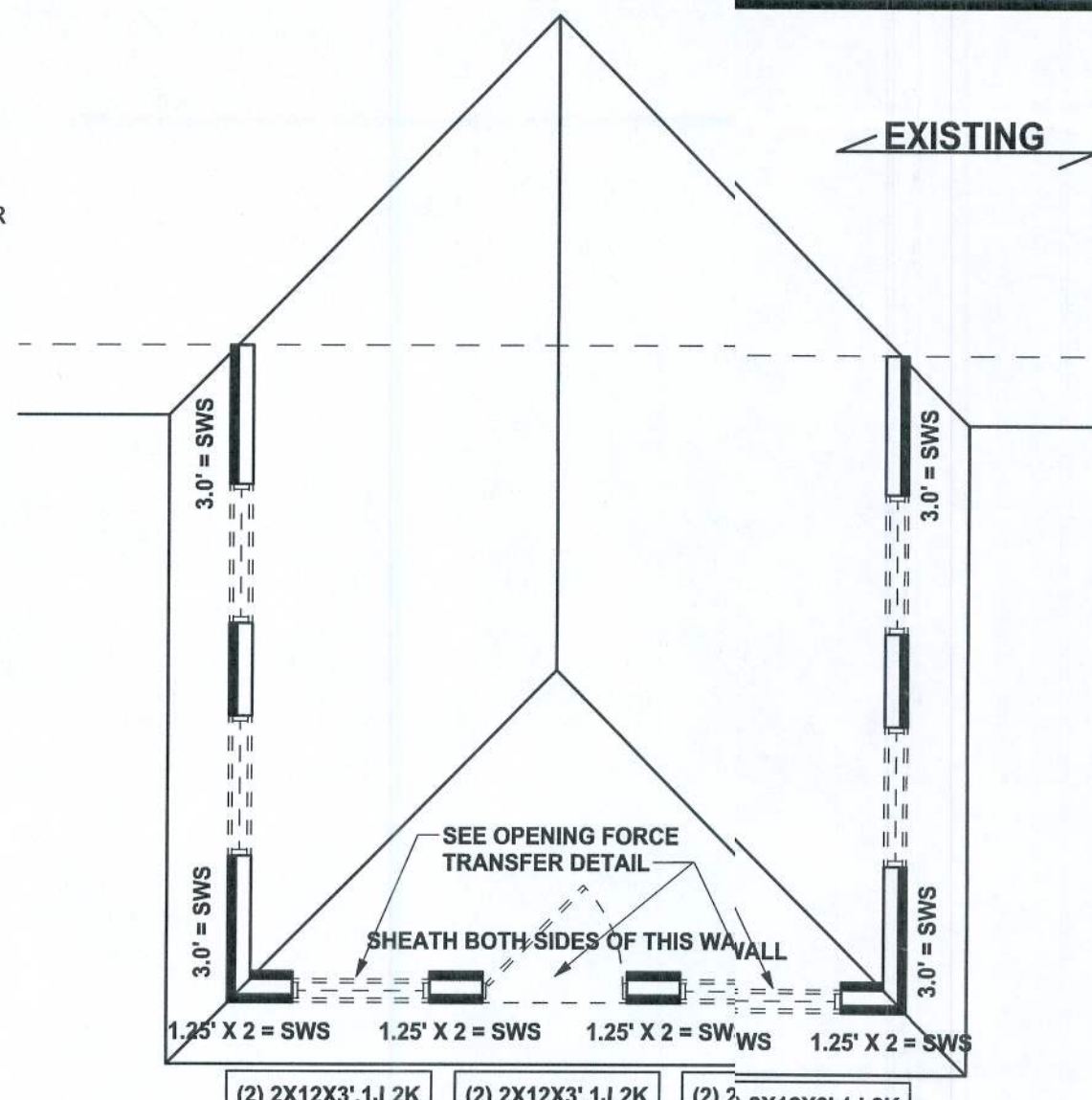


NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEAR WALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER RESIDE TAPERING MUST MEET THE ASPECT RATIO REQUIREMENT HW < 3.5:1 WHERE H IS THE HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGH OR 5' WIDE THE WIDTH OF THE ENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTHEN STRAPPED ACCORDING TO THIS DETAIL.

OPENING FORCE TRANSFER
WOOD FRAME



FOUNDATION PLAN
SCALE: 1/4\"/>



USE H2.5A (535lb) FOR ALL TRUSSES TO WALL
CONNECTIONS UNLESS NOTED OTHERWISE

STRUCTURAL PLAN
SCALE: 1/4\"/>

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORTCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03, BCSH-B1, BCSH-B2, & BCSH-B3. BCSH-B1, BCSH-B2, & BCSH-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (F) FULL LENGTH
	NUMBER OF JACK STUDS (U) UNDER HEADER
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLIES IN HEADER

(F1 S-2) MONOLITHIC FOOTING
SCALE: 1/2\"/>

(F2 S-2) PATIO FOOTING
SCALE: 1/2\"/>

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB #10-119

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCT 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 41LB EACH END; 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF 30:1 IS TYPICAL. SPACING OF CUTS TO BE 12\"/>

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 24ksi, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCT 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCT 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCT 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL STUD TABLE

FOR SPF #2 STUDS

(1) 2x4 @ 16\"/>
(1) 2x4 @ 12\"/>
(1) 2x6 @ 16\"/>
(1) 2x6 @ 12\"/>

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20R, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\"/>

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

1.4A	Compressive strength	Specific Requirements
2.1	Mortar	8\"/>
2.2	Grout	ASTM C 270, Type N, UNO
2.3	CMU standard	ASTM C 476, admixtures require approval
2.3	Clay brick standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8\"/>
2.4	Reinforcing bars, #3 - #11	ASTM A 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30\"/>
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 980	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6465	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GRADE & SPECIES TABLE

	SYP #2	Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1	
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND > 10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = C	
3.) WIND IMPORTANCE FACTOR = 1.0	
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	

Zone	10	100
1	27.8 -30.5	25.3 -25.3
2	27.8 -35.7	25.3 -30.5
2 Chg	-56.8	-56.8
3	27.8 -35.7	25.3 -30.5
3 Chg	-95.6	-99.3
4	30.5 -33.0	25.9 -28.5
5	30.5 -40.7	25.9 -31.6
Doors & Windows		30.5 -40.7
Worst Case (Zone 5, 10 Hz)		
8x7 Garage Door	27.3	-32.0
16x7 Garage Door	25.9	-29.4

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway,
PE No.53915, PO# 868, Lake City, FL
32066, 386-754-519

DIMENSIONS: Stated dimensions/supercede scaled dimensions. Referall questions to Mark Discoway, PE, for resolution. Do not proceed without clarification.

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CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with section R301.2.1, Florida building code residential 207, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY,
P.E. 53915

2024.10
P.E. SEAL

Jones Addition

ADDRESS:
140 SW Woodgrass Glen
Lake City, FL 32024

Mark Dsoway P.E.
P.O Box 868
Lake City Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

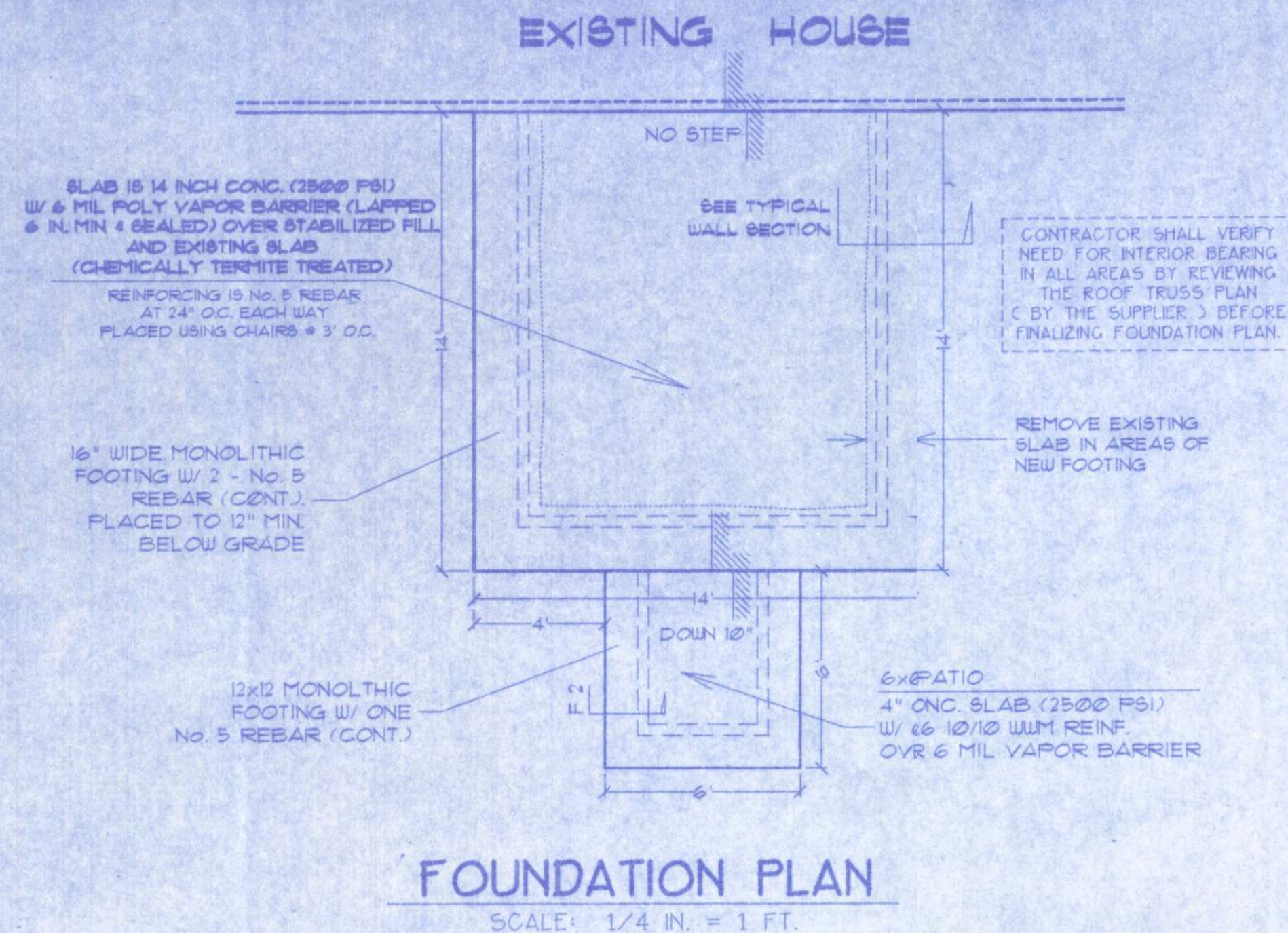
PRINTED DATE:
July 2, 2010

DRAWN BY: STRUCTURAL BY:
David Discoway

FINALS DATE:
22Jul10

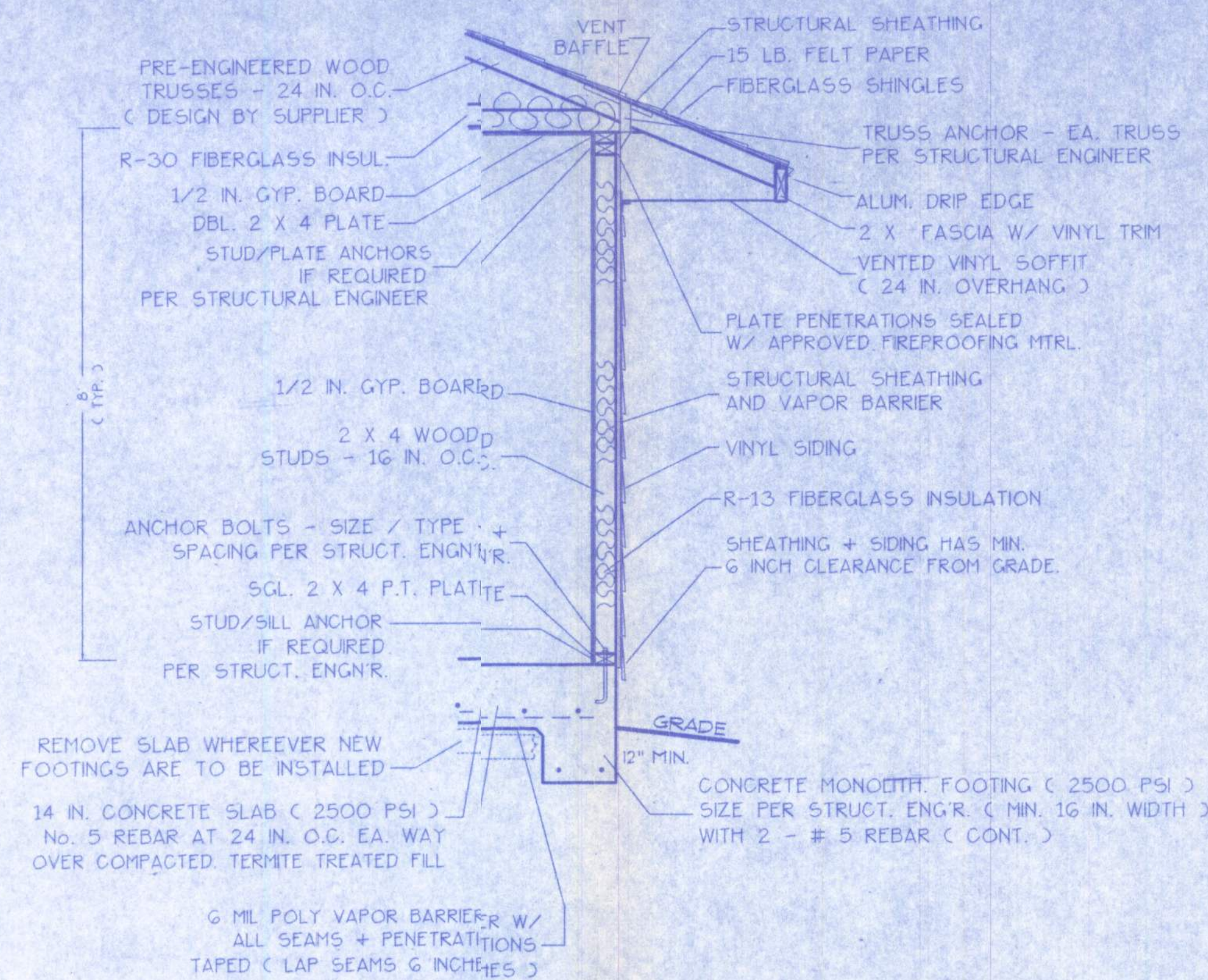
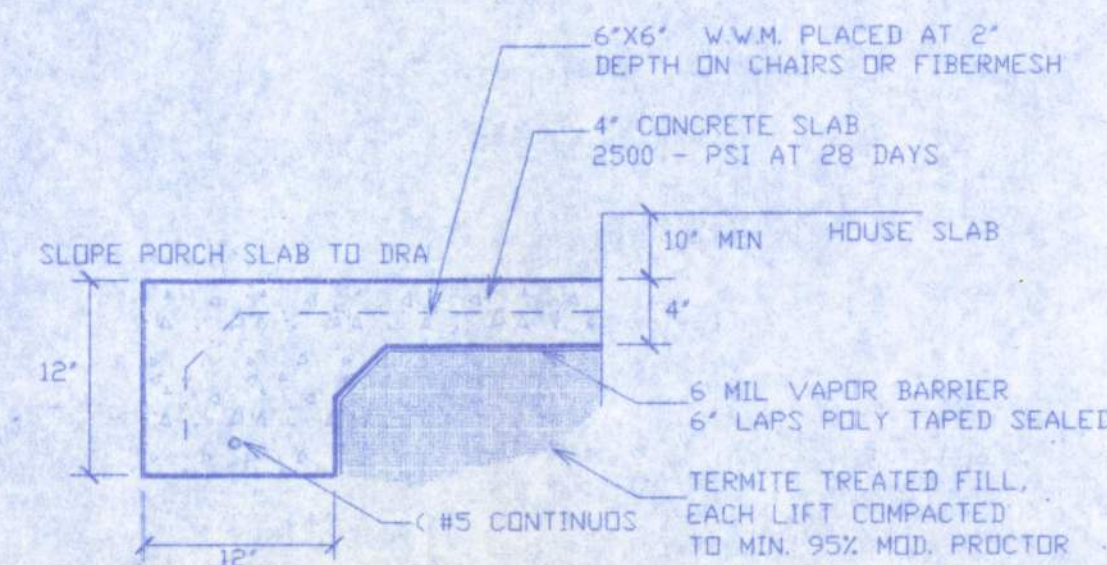
JOB NUMBER:
107038
DRAWING NUMBER

S-1
OF1 SHEET

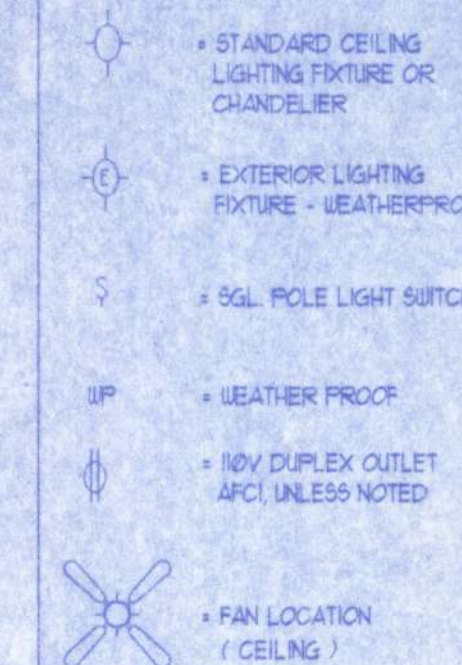


FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

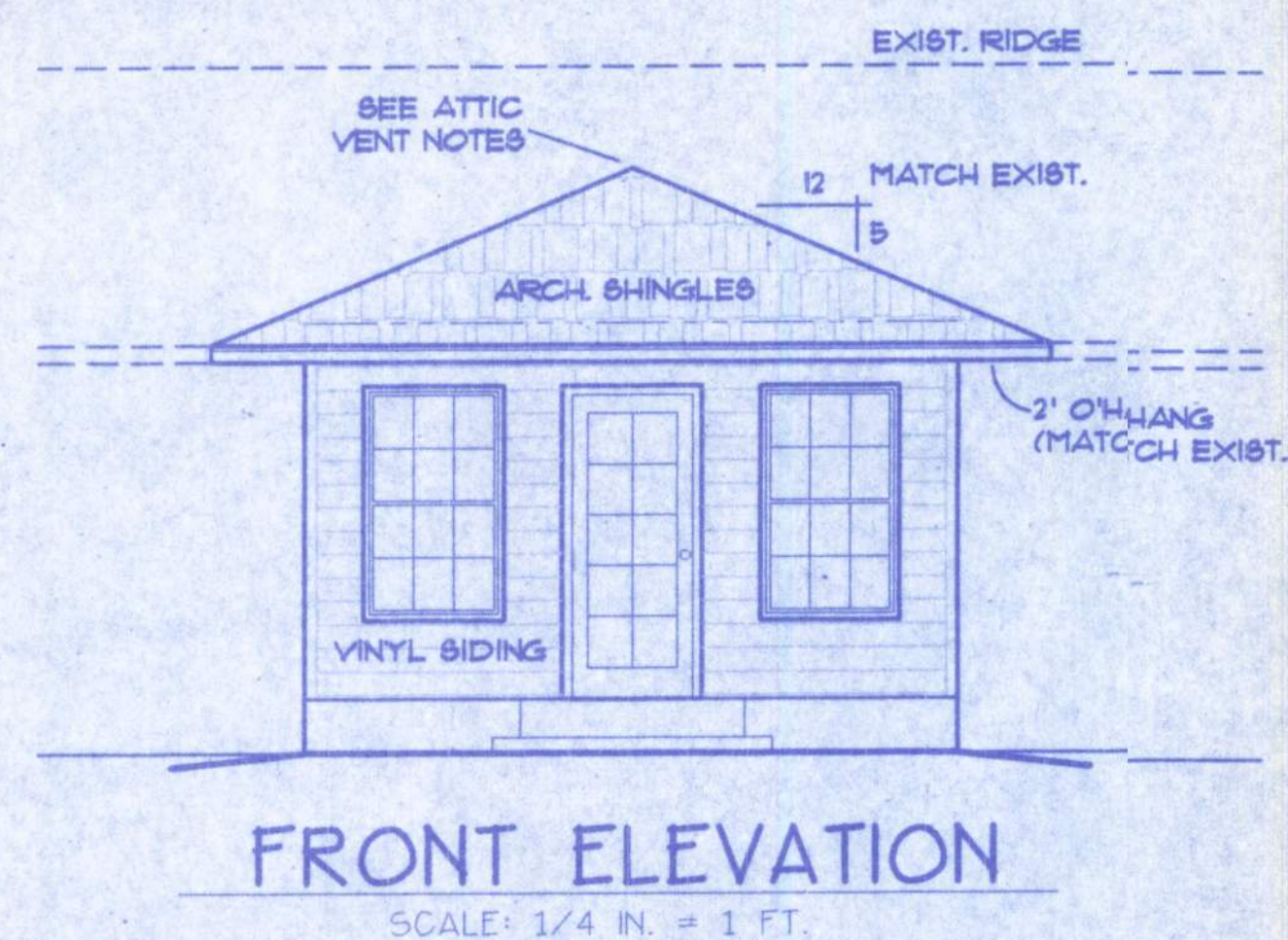
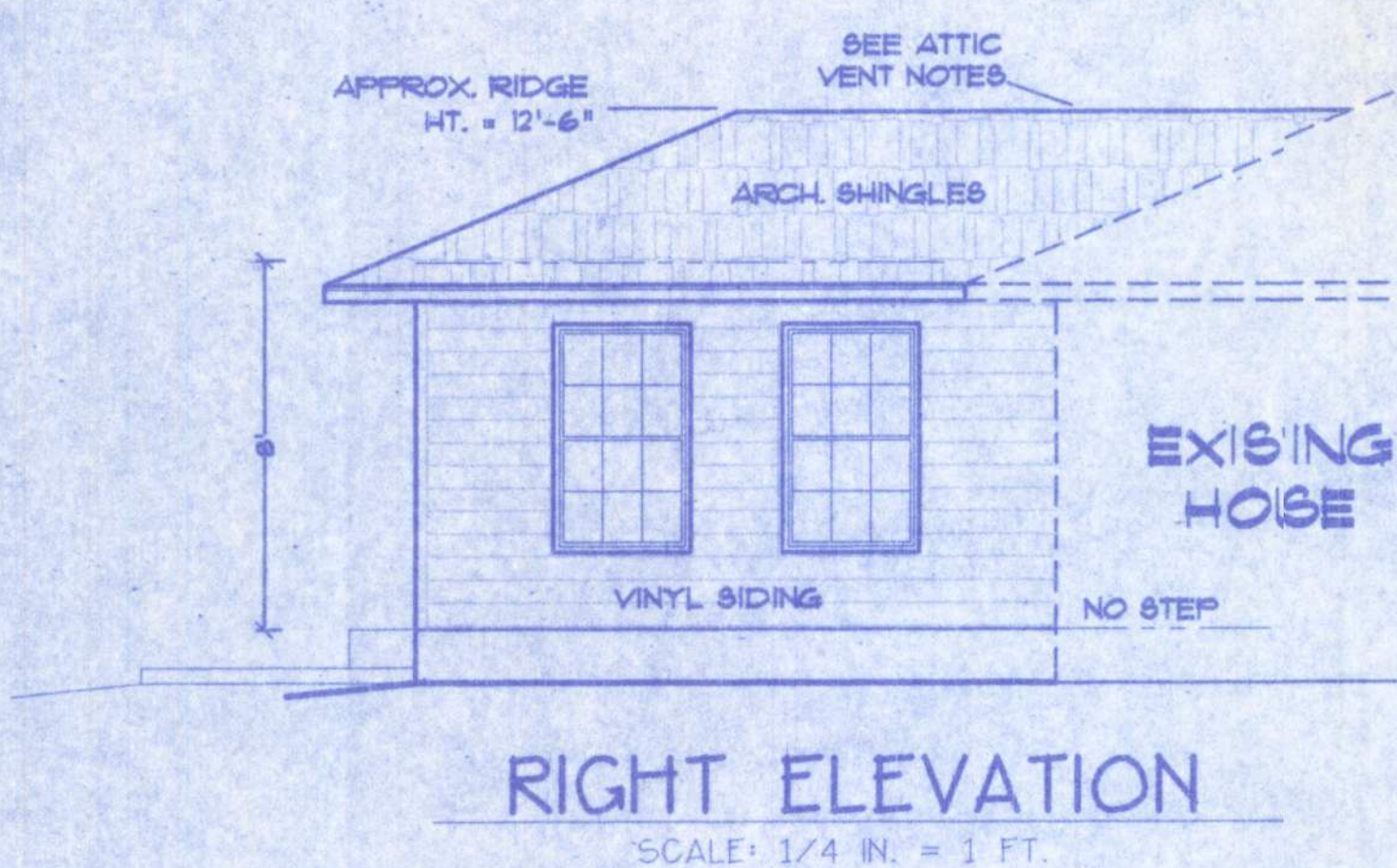
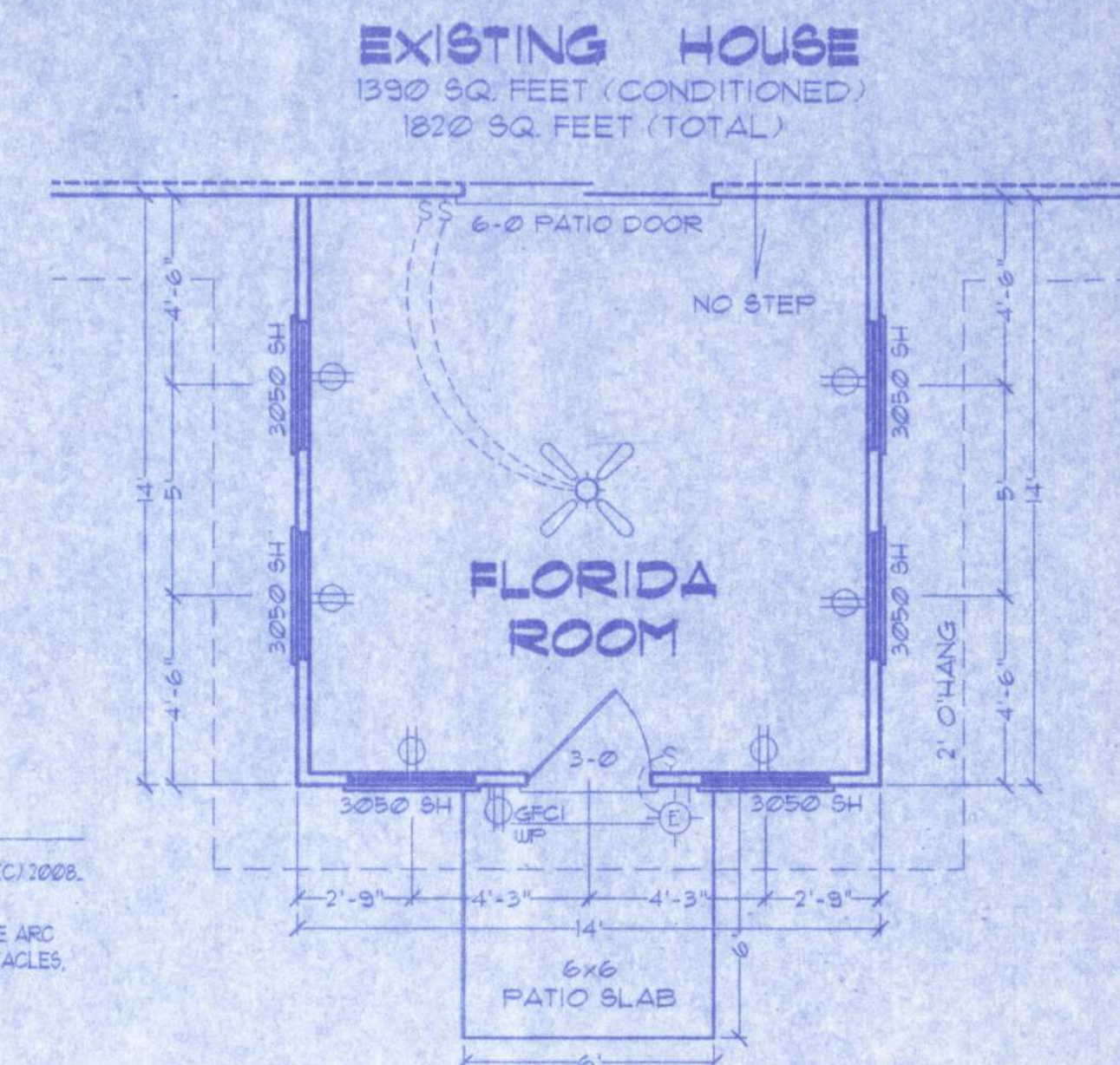


ELECTRICAL SYMBOLS



ELECTRICAL PLAN NOTES

- ALL INSTALLATIONS ARE PER NAT'L. ELECTRIC CODE (NEC) 2008.
- ALL RECEPTACLES UNLESS NOTED OTHERWISE, SHALL BE ARC FAULT CIRCUIT INTERRUPTER (AFCI) TYPE. ALSO, RECEPTACLES UNLESS NOTED, SHALL BE TAMPER RESISTANT.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 2008.
- EXTENSION OF EXISTING ELECTRICAL CIRCUIT(S) TO NEW ADDITION SHALL BE DESIGNED BY THE ELEC. CONTRACTOR.



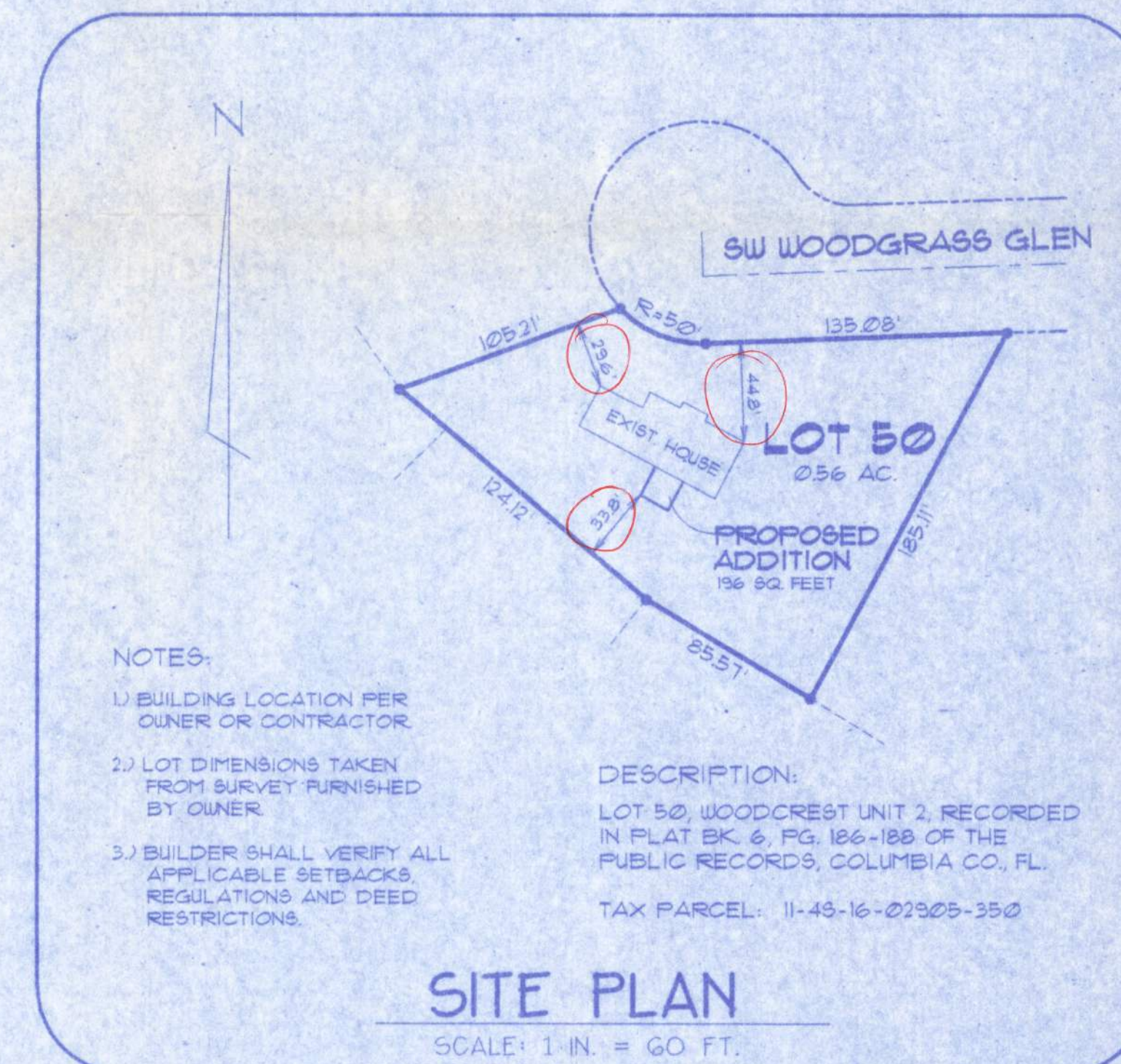
ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1 / 4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 sq. ft. of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

GENERAL NOTES

- See "Wind Load Detail Sheet S-1" and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- All concrete used to be 2500 PSI strength or greater.
- Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- Roof Truss design is the responsibility of the supplier.
- The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Prod. Modification.
- Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.



NOTES:

- BUILDING LOCATION PER OWNER OR CONTRACTOR.
- LOT DIMENSIONS TAKEN FROM SURVEY FURNISHED BY OWNER.
- BUILDER SHALL VERIFY ALL APPLICABLE SETBACKS, REGULATIONS AND DEED RESTRICTIONS.

DESCRIPTION:

LOT 50, WOODCREST UNIT 2, RECORDED IN PLAT BK 6, PG. 166-180 OF THE PUBLIC RECORDS, COLUMBIA CO., FL.

TAX PARCEL: 11-43-16-02305-350

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

140 SW WOODGRASS GLEN
LAKE CITY, FL 32024

Job No.: 1007038
2/3/2010

SEE SHEET S-1 FOR ENGINEERING DETAILS

FILE: 10-007	JONES ADDITION	SHEET: 1 OF 1
DATE: 7-18-10		CAD FILE: 10-C07
DRAWN: T A D		REV:
CHECK: T A D		REV:

PREPARED BY:
TIM DELBENE
Drafting + Technical Services

192 SW Woodcrest Gln, Lake City, FL 32024
Phone: (386) 755-5891